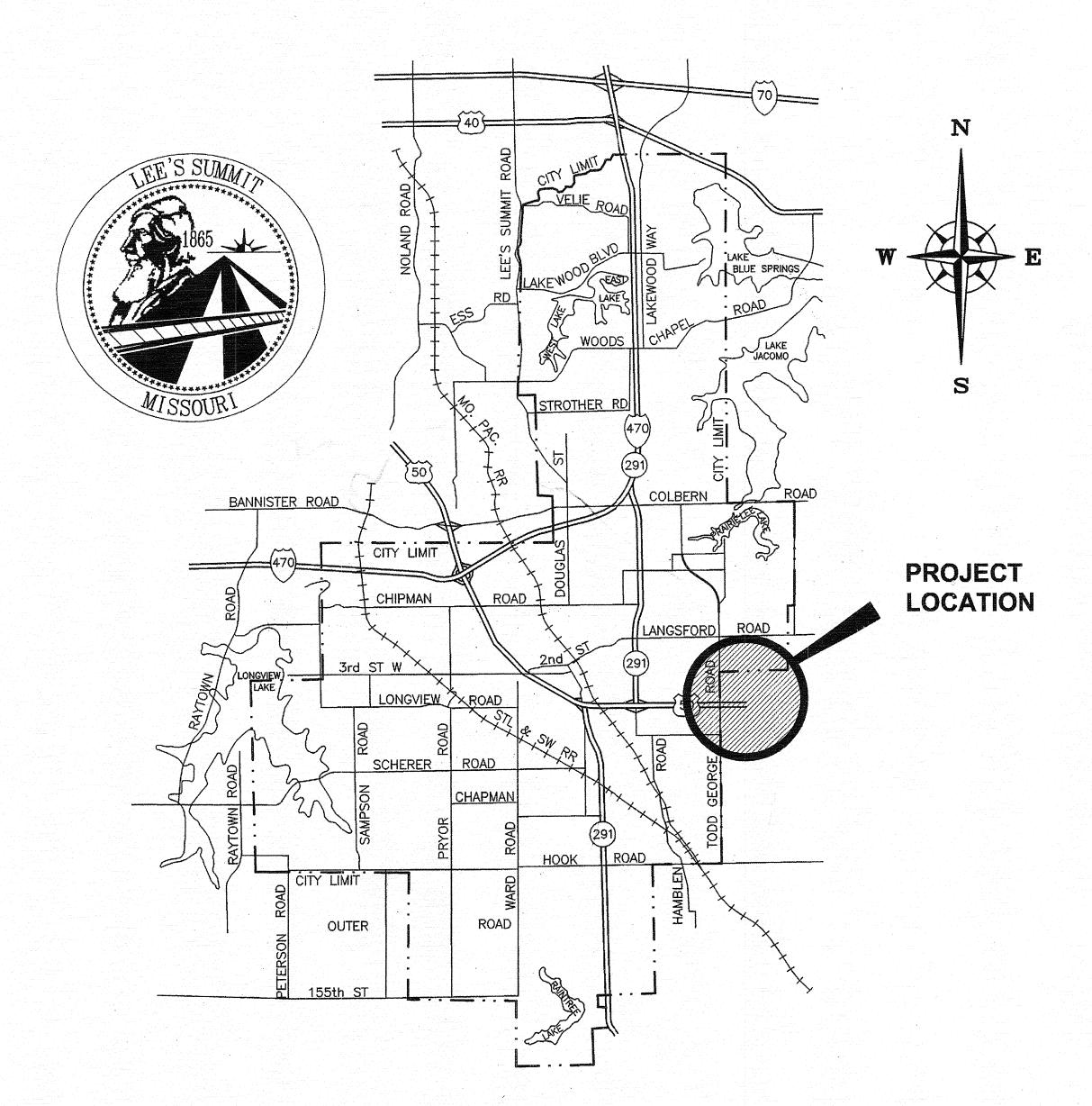
### SITE CONSTRUCTION PLANS

# LEE'S SUMMIT HOSPITAL - REPLACEMENT HOSPITAL

## 2100 SE Blue Parkway

LOCATED IN THE

# CITY OF LEE'S SUMMIT JACKSON COUNTY, MISSOURI CIVIL/SITEWORK PACKAGE



### INDEX OF DRAWINGS

Sht. No.	Description
1	Cover Sheet
2	General Layout Sheet
3 - 6	Site Grading Plans
7 - 10	Site Utility Plans
11 - 14	Site Dimension Plans
15	Concrete Pavement Jointing
16 - 17	Storm Sewer Profiles
18	Sanitary Sewer Profile and Details
19	Storm Drainage Map
20	Storm Drainage Calculations
21	Site Accessible Route Enlargements
22	Site Accessible Route and Ambulance Entry Enlargements
23	Site Accessible Route Enlargements
24	Erosion Control Plan
25 - 26	Detention Basin Details
27	Construction Details
28	Storm Sewer Details
29	Erosion Control Details

### ESTIMATE OF QUANTITIES

DESCRIPTION	UNIT	QUANTITY
Curb and Gutter	L.F.	14,845)
Riprag	^S.Y.	398
Drive Area Pavement	S. Y.	15,580
Heavy Duty Concrete	S.Y.	3,095
Parking Stall Pavement	<i>S.Y.</i>	10,590 ~
8" HOPE	~ L.F.	132
8" HDPE	L.F.	990
10" HDPE	L.F.	545
12" HDPE	L.F.	226
15" HDPE	L.F.	28
18" HDPE	L.F.	306
24" HDPE	L.F.	814
30" HDPE	L.F.	147
18" RCP	L.F.	91
30" RCP	L.F.	497
8" PVC	L.F.	315
6" Cleanout	EA.	3
8" Cleanout	EA.	13
10" Cleanout	EA.	8
12" Cleanout	EA.	3
Concrete Apron	L.F.	6
Concrete Encasement	L.F.	30
48" Band & Gasket C.S.P.	L.F.	33
18" RCP Flared End Section w/Conc. Toewall	EA.	1
24" RCP Flared End Section w/Cons. Toewall	EA.	1
30" RCP Flared End Section w/Conc. Toewall	EA.	8
4'x4' J.B.	EA.	1
5'x3' C.I.	EA.	15
5'x4' C.I.	EA.	2
6'x3' C.I.	EA.	1
6'x5' C.I.	EA.	1
11'x6' Weir Box w/ Concrete Wingwalls	EA.	1
Connect Roof Drain to Curb Inlet	EA.	7
15" Nyloplast Drain w/Standard Lock Down Grate	EA.	7
4' Diameter Manholes	EA.	3

### PROJECT CONTACTS

Engil	neer	
George B	utler Asso	ciates, Inc
One Rent	ner Ridge	
9801 Rei	nner Boule	vard
	V C6	

Lenexa, Kansas 66219-9745 Phone: (913) 492-0400 Contact: Mr. Brad Burton

Architect Perkins + Will 6200 North Central Expressway Dallas, Texas 75206 Phone: (214)775-4826 Contact: Mr. Dwight Burns

First Issue Date: 06-02-06 (As ASI #2)

Ochsner Hare & Hare 2600 Grand, Mezzanine Suite Kansas City, Missouri 64108 Phone: (816)842-8844 Contact: Mr. Ken Boone

Landscape Architect

Cover Sheet

BRADLEY D.

PROJECT NUMBER 10367.00

First Issue as: ASI #2 - 06/02/069 ASI #7 - 10/20/06

> DESIGNED H.T.R. / J.W.M REVIEWED B.D.B. SHEET TITLE

> > Cover Sheet

SHEET NUMBER

1 of 29

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### UTILITY COMPANY CONTACTS

CITY OF LEE'S SUMMIT WATER UTILITIES ATTN: WES OWENS WATER & SEWER 616 NE DOUGLAS ST LEE'S SUMMIT, MO 64063 NATURAL GAS

MISSOURI GAS AND ENERGY ATTN: KEVIN DRISKELL LEE'S SUMMIT, MO 64086 (816) 969-2217

ELECTRICAL POWER ATTN: JEFF WILLIAMS

215 N. SPRING ST., 2ND FLOOR INDEPENDENCE, MO 64050-2822

CABLE TELEVISION ATTN: GREG JOHNSON KANSAS CITY, MO 64113 (816) 222-5533

BEFORE EXCAVATING CALL: 1-800-DIG-RITE

### FLOOD PLAIN:

TELEPHONE

The subject property lies within Zone C "Areas of minimal flooding" as shown on and according to FIRM Community—Panel Number 290174 0007 C, Dated August 3, 1989.

## EXISTING ZONING:

The subject property is currently zoned as: CP-2

This zoning has the following setback provisions: Front Yard Setback-20' Rear Yard Setback-20' Street Side Yard Setback-15' Interior Side Yard Setback-10' Maximum Building Height - 5 Stories or 60'

### SITE DEVELOPMENT DATA

Total Site Area in Acres: 39.00±

Currently Proposed Developed Area = 20.69 Acres.

Conceptual Developed Area = 18.31 Acres.

Parking Required: See Parking Data Table (herein)

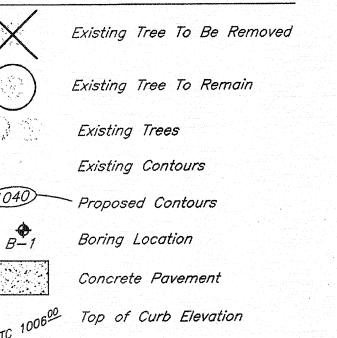
### PROJECT BENCHMARK:

" \_ " Cut on the North side of Concrete Base of North Post of Todd George Road Exit Sign for Westbound U.S. Highway 50. Approximately 30' South of the Centerline of the Outer

*Elevation = 1012.79* 

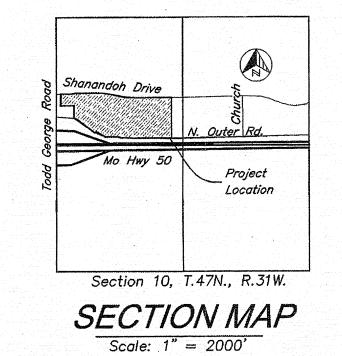
### I FGEND OF SYMBOLS

	JEIVE OF OTIVIE		
	Signs	<del>(</del>	Guy Anchor
$\boxtimes$	Gas Test Station		Flood Light
₩v ⊠	Water Meter		Fire Hydrant
$\otimes$	Sprinkler Valve/Boxes _		= Existing Storm Sewer Line
	Water Vault	5	— Existing Sanitary Sewer Line
	Sanitary Sewer Manhole	W	<i>— Existing Water Line</i>
E	Electric Manhole	———GAS	— Existing Gas Line
$\phi^{LP}$	Street Light	U.D	— Underdrain
0 <sup>PP</sup> ·	Power Pole x	× × ×	— Existing Fence Line
Q	Traffic Signal	<u>7. V.</u> ]	Telephone Vault
	Elec. Box	<i>B.P</i> ]	Backflow Preventer
4	Guy Pole		— Existing Easement
)W MRKR	Right of Way Marker		- Property Line



Spot Grade Elevation

Proposed Building Layout



Owner / Developer Lee's Summit Hospital 530 NW Murray Road Lee's Summit, Missouri Contact: Mr. Scott Montgomery Phone: (816)969-6060

1. The construction covered by these plans shall conform to all applicable standards and specifications of the Public Works Department of the City of Lee's Summit, Missouri, current usage. Contractor to contact public works inspections at (816) 969-7450 (48) hours prior to commencement of any construction activity.

2. Existing topography shown hereon is based on contour and topographic information as noted on the ALTA/ACSM Land Title Survey prepared for HCA, Midwest Division by Olsson Associates Consulting Engineers (dated 02/10/04) as provided to this engineer by the client. Additional land surveying field information gathered and prepared by this engineer has also been incorporated.

3. Soils Report – A soils report has been completed by Kleinfelder Geotechnical Engineering Services. ALL grading operations shall conform to the findings and recommendations noted within the soils report. A copy of the soils report and all boring logs is available for review at the offices of J.E. Dunn Construction. Any geo-technical information in this set of plans has been provided by Kleinfelder. George Butler Associates, Inc. is not responsible for the adequacy or accuracy of the soils information shown or provided. It is provided for informational purposes only. The content of these plans may change based on recommendations found in the final geotechical report.

4. This site is currently under construction. Grading operations have begun per a Mass Grading Package previously submitted and approved by the City of Lee's Summit. Contractor shall obtain a set of those drawings for his review to determine the extent of the improvements under way. The contractor of these plans shall be responsible for ALL erosion control installed per the Mass Grading Package and shall maintain those erosion control measures (in addition to those measures shown in these plans) once mass grading operations are complete and construction of these plans begins.

5. Existing Utilities — The locations of existing underground utilities are approximate and have not been field verified by the Owner or it's representative. The Contractor shall determine the exact location of all existing utilities before commencing work. The Contractor is fully responsible for any and all damages occurring from his failure to do so. The Contractor shall coordinate the relocation of any utilities that may be encountered prior to the start of construction.

6. Slopes - Slopes shall be graded at a maximum slope of 3:1 (Horz.:Vert.). It is critical that grading shown in and around building pad be accomplished accurately so drainage away from building pad is maintained at all times.

7. Existing Site Conditions — The Contractor shall, prior to commencing work, investigate surface and subsurface conditions to be encountered across the project site and notify the Engineer if any discrepancies or changed conditions are noted.

8. The contractor is responsible for the protection of all property corners and section corners. Any property corners and/or section corners disturbed or damaged by construction activities shall be reset by a Registered Land Surveyor licersed in the State of Missouri, at the contractor's expense.

9. Cut/Fill - All fills are to be made with suitable structural fill material in accordance with the project geo-technical engileer recommendations. Special inspections are required. Contractor shall coordinate inspections with the Owner.

10. The Contractor shall be responsible for the restoration of the right-of-way and for damaged improvements such as curbs, sidewalks, street light and traffic signal junction boxes, traffic signal loop lead ins, signal poles, etc. Damaged improvements shall be repaired in conformance with the latest City standards and to the City's satisfaction.

11. The Contractor shall coordinate and conduct a pre-construction walk-thru with the City of Lee's Summit Public Works Department to review and document the condition of all existing public improvements (i.e. pavements, walks landscaping, etc. surrounding the site.

12. All disturbed areas within the Public right-of-way shall be sodded. All other disturbed areas shall be seeded in accordance with the project specifications.

Proposed Phase One	Req'd.	Prov'd.
Hospital 64 Bed (2—Story)(123,460 Sq. Ft.)	116	
City Code Requires a Minimum of 1.8 Spaces per Proposed Hospital Bed.		
Outpatient/Medical Office (MOB) 66,296 Sq. Ft. Bldg. (2—Story)	332	
City Code Requires a Minimum of 5 Spaces per 1000 Square Feet of Proposed Medical Office Use.		
Sub-Total Parking	448*	606**
Accessible Parking Spaces City Code Requires that a Minimum of 2% of the Total Spaces for Parking Areas of 501 to 1000 Spaces be Accessible, with 1 in 8 of those Spaces being Van Accessible.		
Total Accessible Spaces	12	23
Van Accessible Spaces	(2)	(5)
Ambulance Parking Spaces  No Code Requirement for Ambulance  Spaces		
Ambulance Spaces	0	8

\* Parking Count Includes Required Accessible Spaces \*\* Parking Count Includes Required Accessible

Spaces and Provided Ambulance Spaces. Note: It is intended that 158 of the parking spaces constructed with Phase One will be "credited" to the required number of spaces for the future construction of 

### Trash Enclosures

There are no trash enclosures planned for the site. Trash will be disposed of via trash compactor located in the loading dock area on the north side of the hospital building.

### Site ADA Accessibility

See Sheets 21, 22, 23 and 27 for Site Accessibility Routes, Notes and Details.

### Impervious Coverage

Total Site Area	1,698,840 sq. ft. or 39.00 Acres
Phase 1	392,677.39 sq. ft. or 23.11%
Phase 2	416,633.85 sq. ft. or 24.52%
Phase 3	757,214.92 sq. ft. or 44.57%

Impervious Coverage based on a platted Lot 1 Area of 1,261,800.481 Sq. Ft. is equal to 405,697.82 Sq. Ft. or 33.2%.

### Floor Area Ratios - (FAR)

Phase 1 –	Hospital	11% FAR
Phase 2 -	Hospital with all Expansions	18% FAR
Phase 3 -	Hospital with all Expansions and West Sites Medical Office Bldgs.	22% FAR

- 1. Connection materials shall be pre-assembled and excavation of the existing water main shall be completed at each of two points of connection prior to water service shutdown and connection installation in order to minimize disruption to water service.
- 2. When a disruption of water service will occur, the Contractor shall notify Water Utilities Operations at least 48 hours in advance to make the necessary arrangements. Water Utilities Operations can be reached at (816)969-1940.
- 3. The interior of the connection assembly shall be completely swabbed with a 100 ppm chlorine solution immediately prior to installation.
- 4. Excavation bottoms for interconnections shall have gravel base and sumps as required to prevent the entry of trench water and debris into the water main and assemblies during installation. Water mains shall be capped at all times when construction is not occurring on them. 5. Contractor shall excavate and expose location, size, and material of construction of existing water mains for each interconnection with the new 12" Dia. water, including locations of pipe joints. Contractor shall provide all required
- interconnecting material and fittings at his expense. 6. The Contractor shall be responsible for providing all required temporary disinfection and water main flushing fittings and
- 7. All flushing of water mains shall be coordinated with and approved by the City of Lee's Summit.
- 8. Contractor shall provide all equipment and material required for disinfection and dechlorination of the flush water.
- 9. Contractor shall be responsible for the collection and testing of all required water samples for verification of disinfection of installed watermains.
- 10. All water main valves and fire hydrants shall be operated only by City personnel, unless prior written approval has been issued by the City to the Contractor. 11. Minimum water main depth of burial is 42 inches (3.5 feet) as measured from the top of the pipe to the existing
- ground or finished grade, whichever is lower. 12. Maximum water main depth of burial is 84 inches (7 feet) from the top of the pipe to the finished grade.
- 13. The Contractor shall disinfect and test all mains and fire lines regardless of existing conditions. This may include repairing existing facilities that must be included in the test and are not capable of holding test pressures. All thrust blocks or other bracing facilities shall be in place at least 48 hours before the initial filling of the line. All tests will be administered by the City.

General Layout

BRADLEY D. BURTON NUMBER E-258872

PROJECT NUMBER 10367.00

First Issue as: ASI #2 - 06/02/06  $\triangle$  ASI #7 - 10/20/06

> DESIGNED H.T.R. / J.W.M. DRAWN REVIEWED SHEET TITLE

General Layout Sheet

SHEET NUMBER

2 of 29

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66,296 Sq. Ft. Bldg. (2—Story)		
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Van Accessible Spaces	(2)	(5)
Ambulance Parking Spaces No Code Requirement for Ambulance Spaces		
Ambulance Spaces	0	8

Parking Count Includes Required Accessible Spaces

\*\* Parking Count Includes Required Accessible Spaces and Provided Ambulance Spaces.

Note: It is intended that 164 of the parking spaces constructed with Phase One will be "credited" to the required number of spaces for the future construction of Phase Two.

Total Site Area	1,698,840 sq. ft. or 39.00 Acres
Phase 1	392,677.39 sq. ft. or 23.11%
Phase 2	416,633.85 sq. ft. or 24.52%
Phase 3	757,214.92 sq. ft. or 44.57%

Impervious Coverage based on a platted Lot 1 Area of 1,261,800.481 Sq. Ft. is equal to 405,697.82 Sq. Ft. or

### Floor Area Ratios - (FAR)

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Phase 2 — Hospital with all Expansions	18% FAR
Phase 3 — Hospital with all Expansions and West Sites Medical Office Bldgs.	22% FAR

- 7. All flushing of water mains shall be coordinated with and approved by the City of Lee's Summit.
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General Layout

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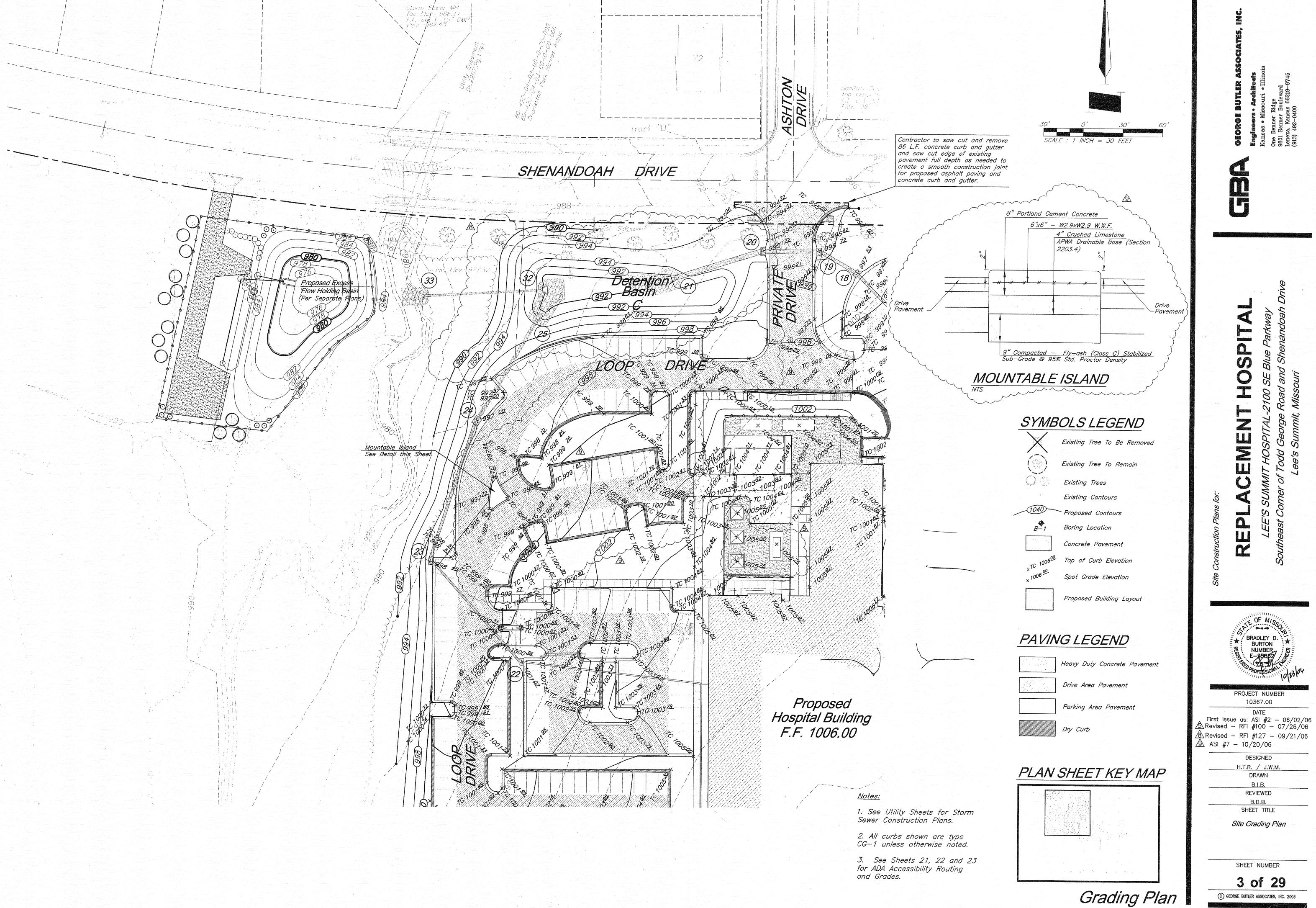
BURTON NUMBER E-25862 PROJECT NUMBER 10367.00

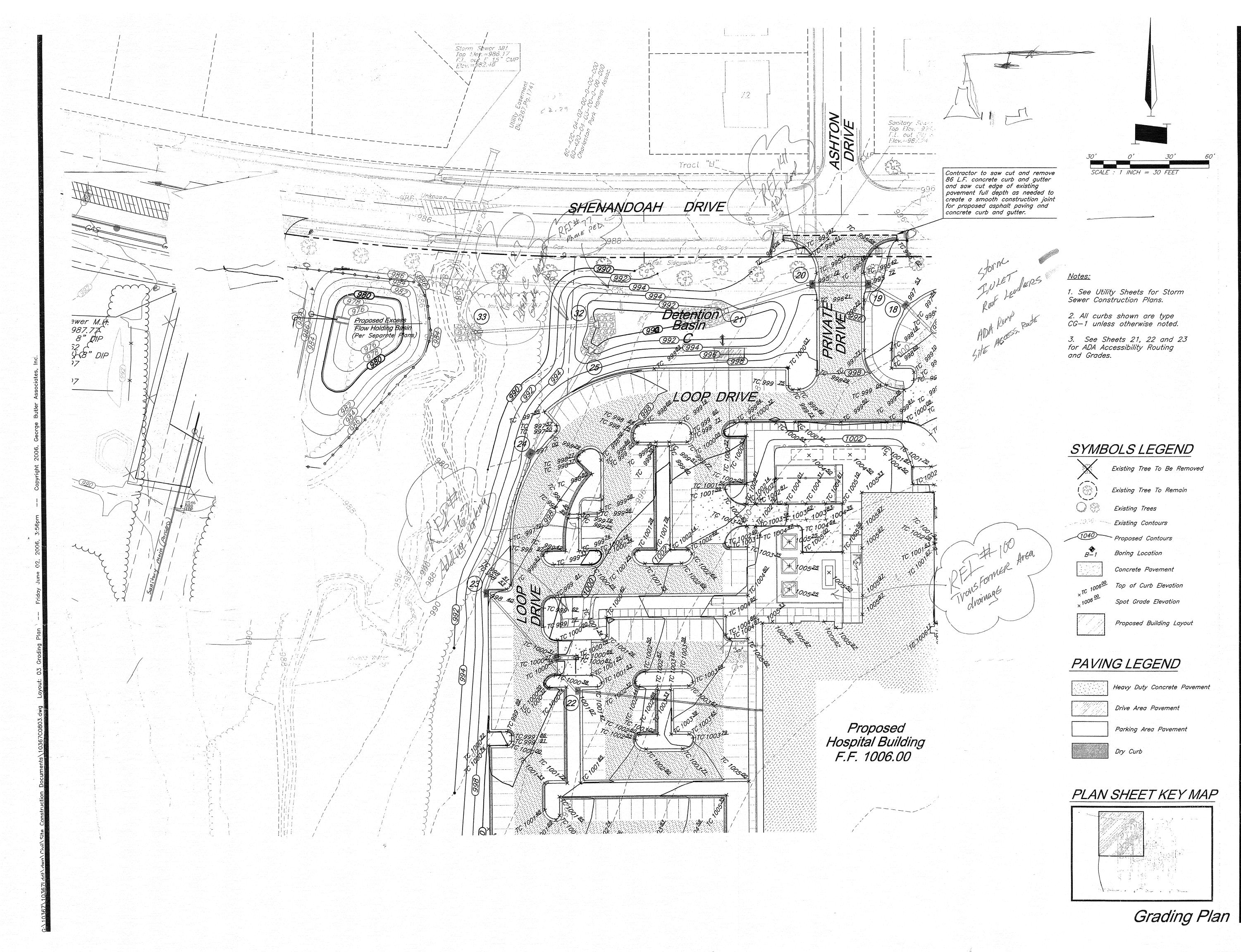
First Issue as: ASI #2 - 06/02/06

DESIGNED H.T.R. / J.W.M DRAWN B.I.B. REVIEWED B.D.B.

SHEET TITLE General Layout Sheet

HEET NUMBER 2 of 29





KGE BUTLER ASSOCI
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- Missouri - Illinois
nner Ridge

Kansas • Missouri One Renner Ridge 9801 Renner Bouleva Lenexa, Kansas 66218

HOSPITAL 3 SE Blue Parkway

Sourchold rights

BRADLEY D.
BURTON
NUMBER
E-25862

PROJECT NUMBER 10367.00

DATE
First Issue as: ASI #2 - 06/02/06

DESIGNED
H.T.R. / J.W.M.
DRAWN

B.I.B.

REVIEWED

B.D.B.

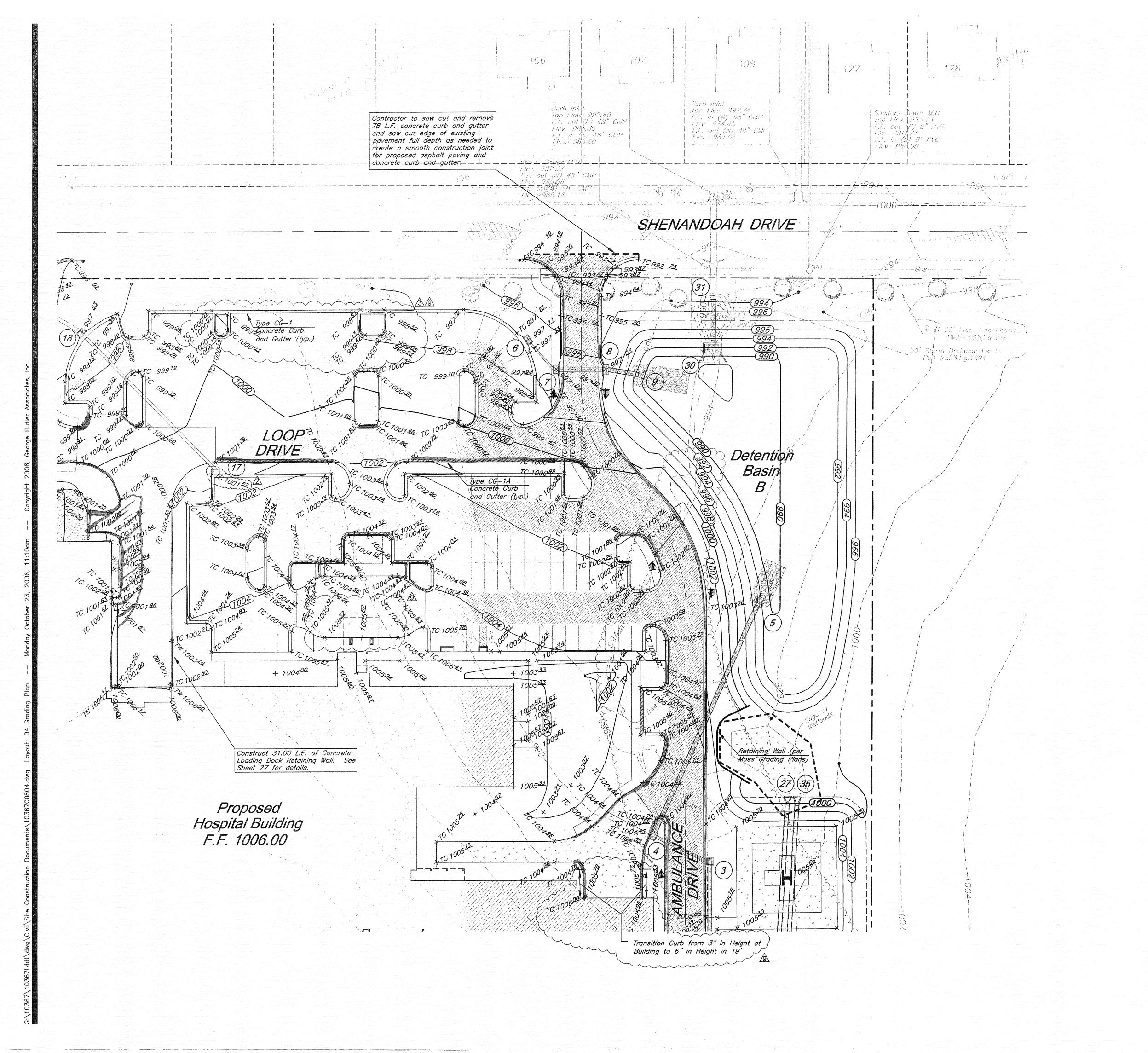
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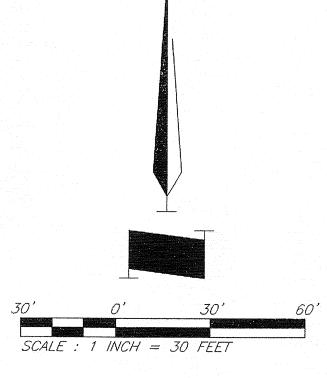
Site Grading Plan

SHEET NUMBER

3 of 29

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### <u>Notes:</u>

1. See Utility Sheets for Storm Sewer Construction Plans.

2. All curbs shown are type CG-1 unless otherwise noted.

3. See Sheets 21, 22 and 23 for ADA Accessibility Routing and Grades.

### SYMBOLS LEGEND

Existing Tree To Be Remove

Existing Tree To Remain

Existing Trees

Existing Contours

1040 Proposed Contours

B-1 Boring Location

Concrete Pavement

1006<sup>00</sup> Top of Curb Elevation

× 1006 Spot Grade Elevation

Proposed Building Layout

### PAVING LEGEND

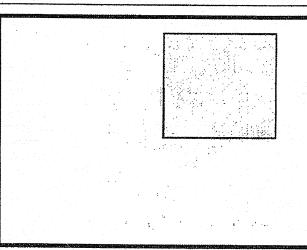
Heavy Duty Concrete Pavement

Drive Area Pavement

Parking Area Pavement

Type CG-1A Dry Curb

### PLAN SHEET KEY MAP



Grading Plan



LEE'S SUMMIT HOSPITAL-2100 SE Blue utheast Corner of Todd George Road and Sh

BRADLEY D. BURTON NUMBER E-28862

PROJECT NUMBER 10367.00

DATE
First Issue as: ASI #2 - 06/02/06
Revised - RFI #082 - July 11, 2006
Revised - RFI #090 - July 14, 2006
ASI #7 - 10/20/06

DESIGNED
H.T.R. / J.W.M.

H.T.R. / J.W.M.

DRAWN

B.I.B.

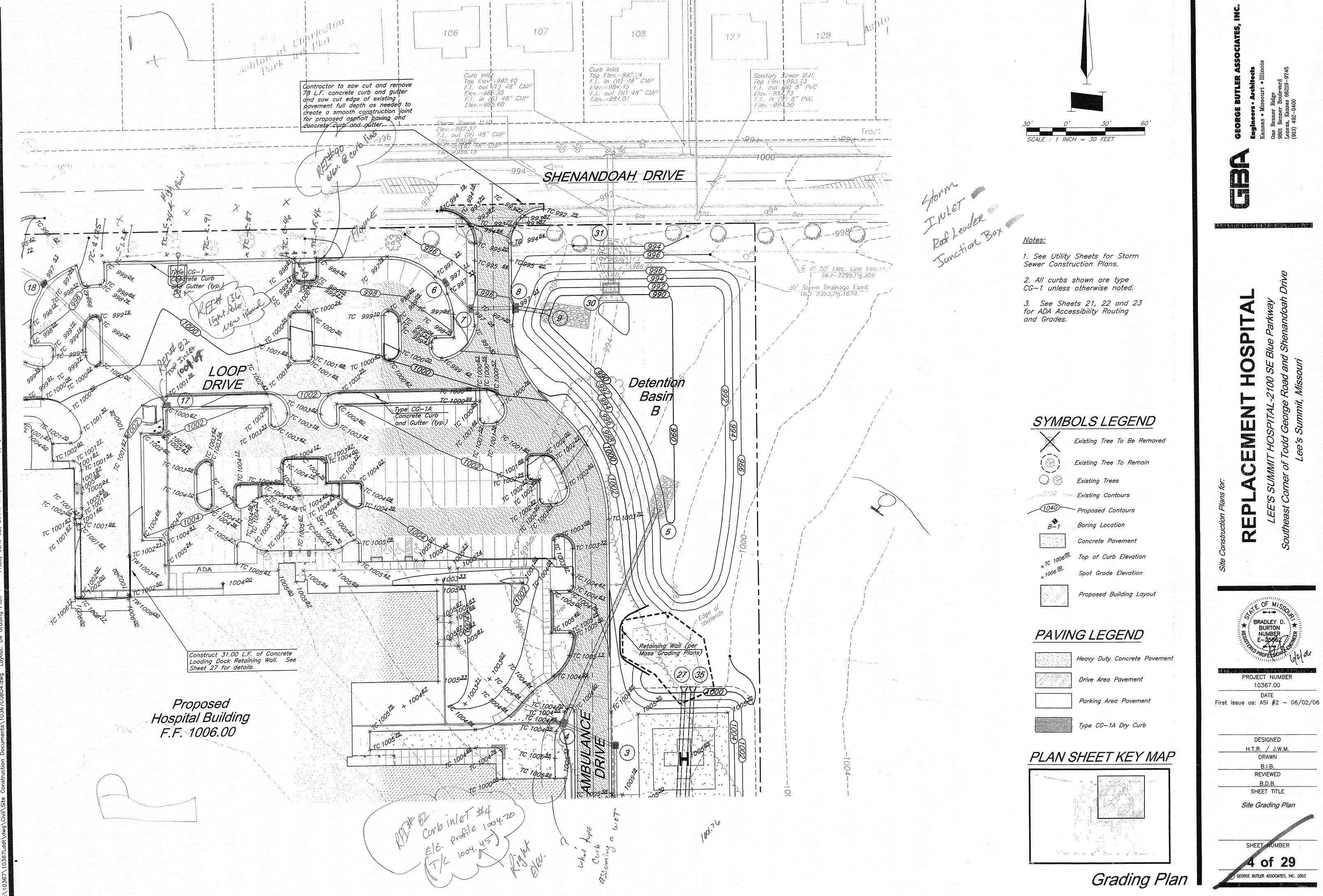
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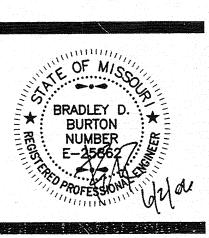
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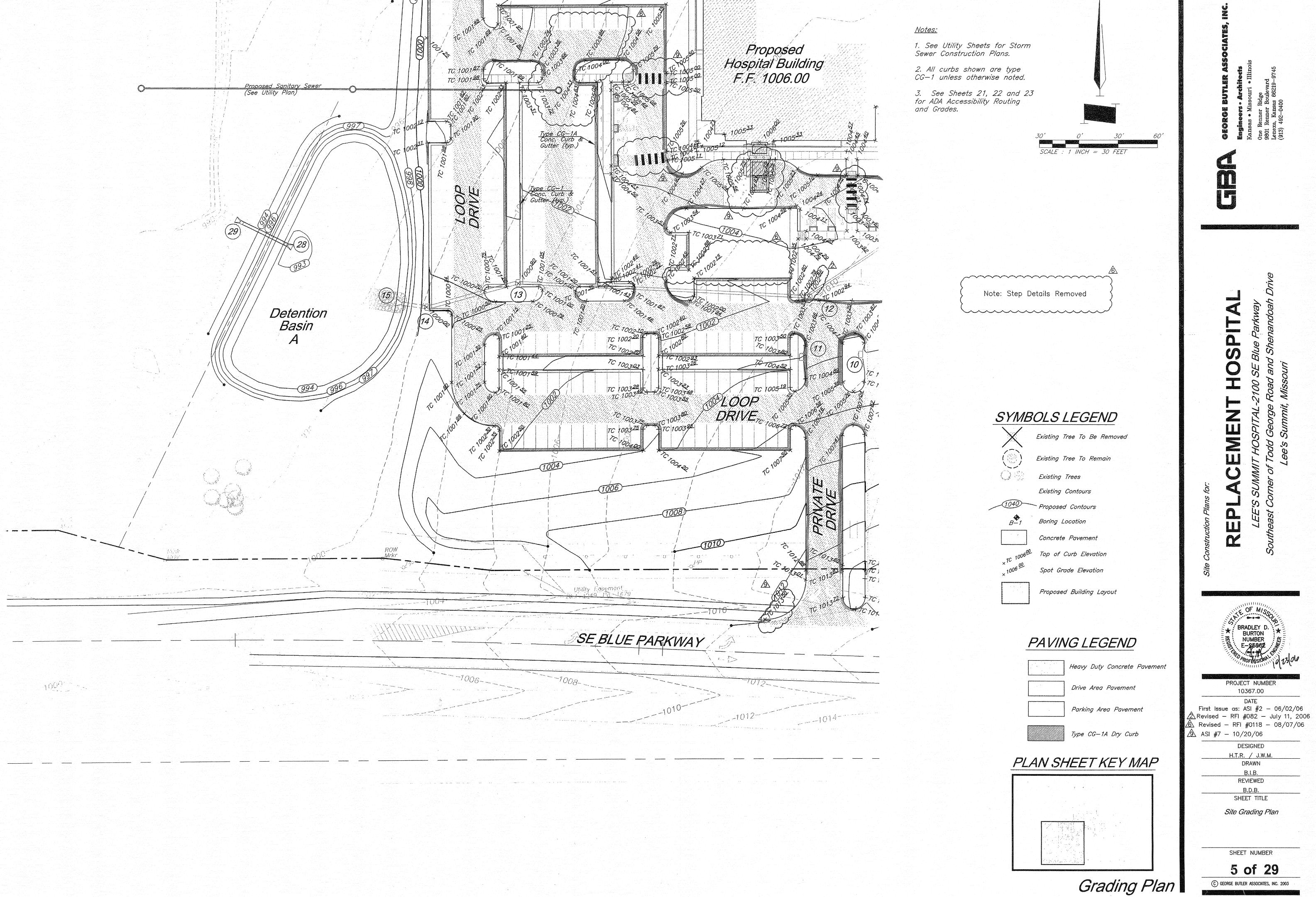
Site Grading Plan

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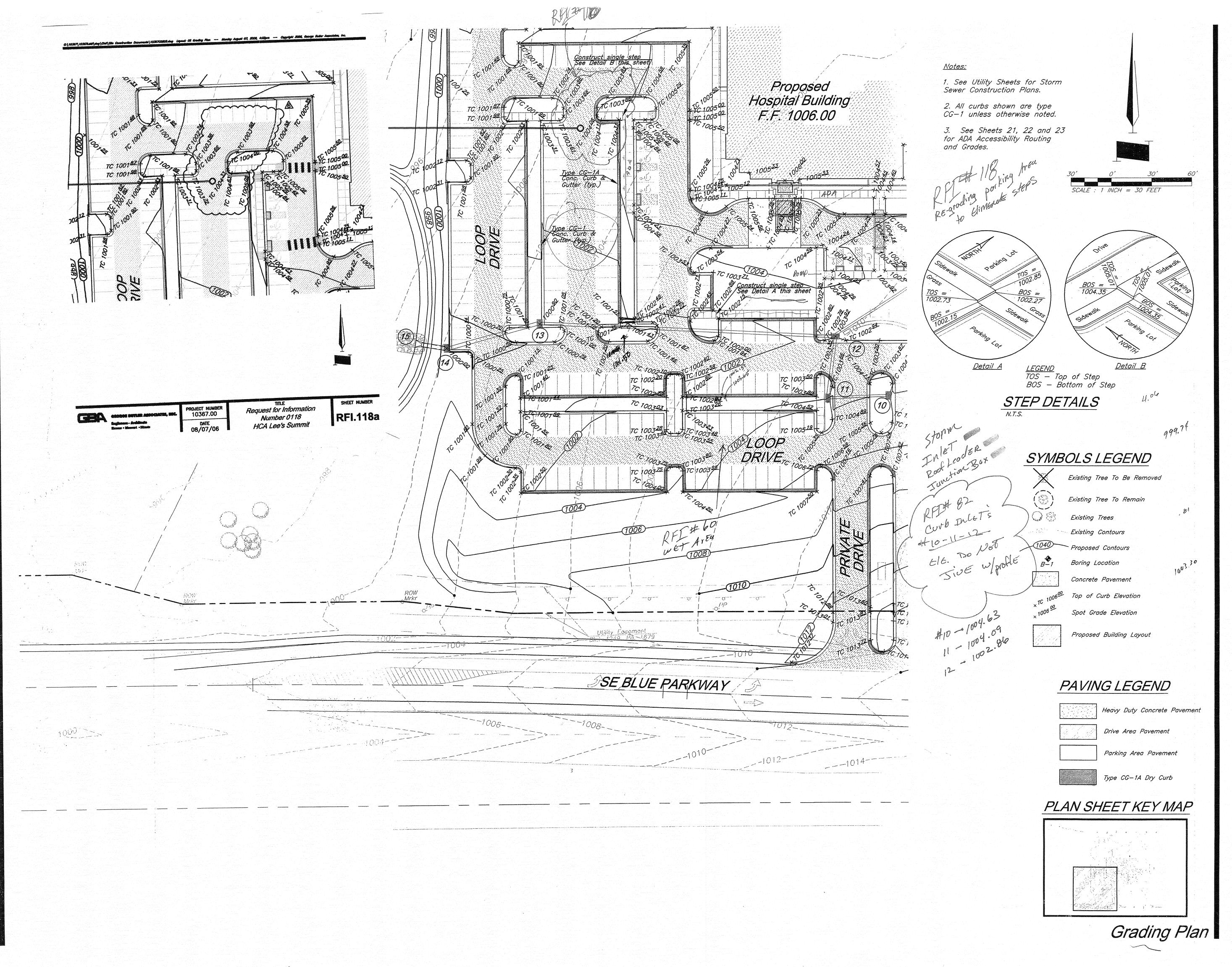
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Revised - RFI #0118 - 08/07/06



Engiroers • Architects
Kansas • Missouri • Illinois
One Renner Ridge
9801 Renner Boulevard

REPLACEMENT HOSPITALISE STATES OF THE STATES

rte Construction Plans for:

BRADLEY D.

BURTON
NUMBER
E-25862

PROJECT NUMBER
10367.00

PROJECT NUMBER

10367.00

DATE

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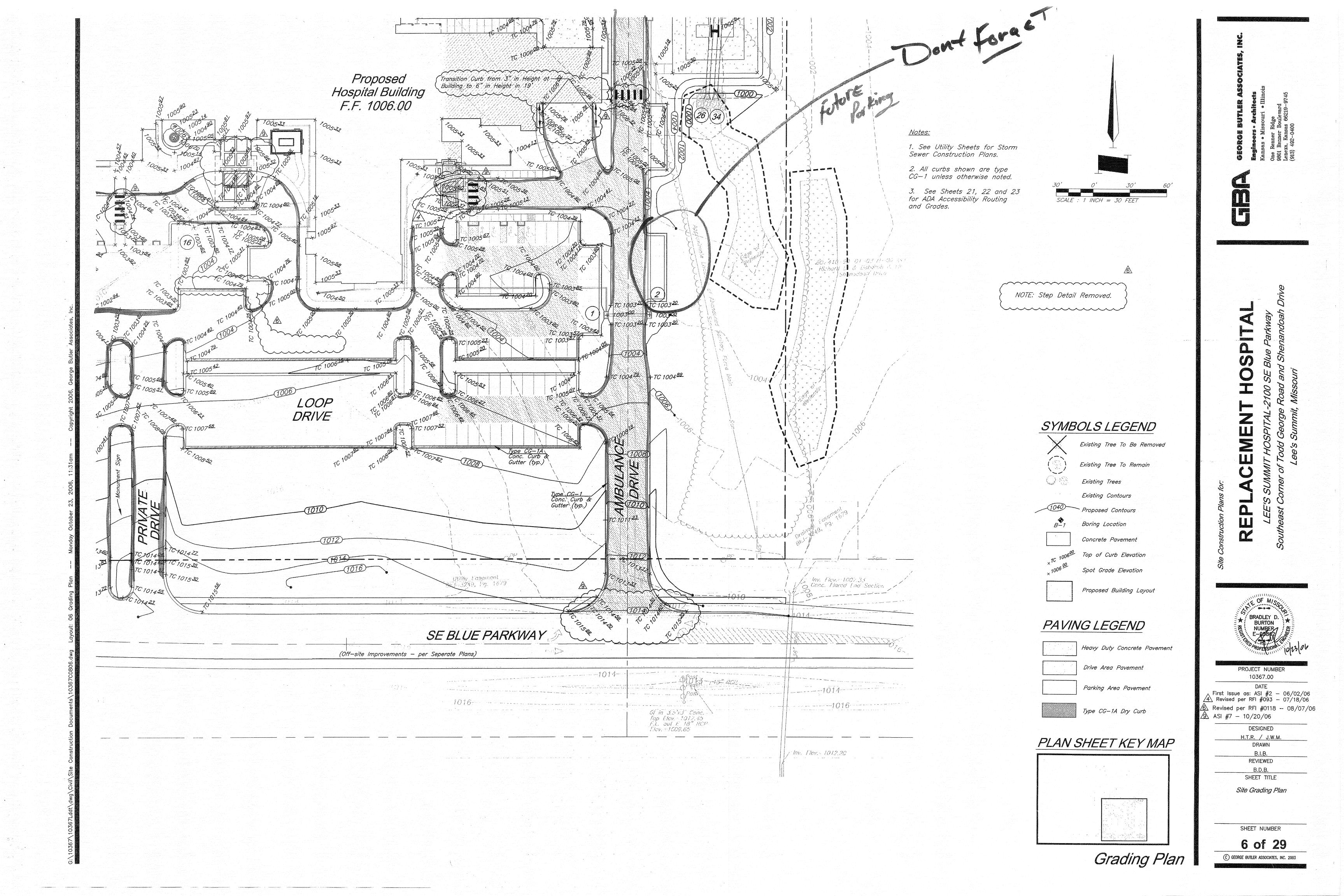
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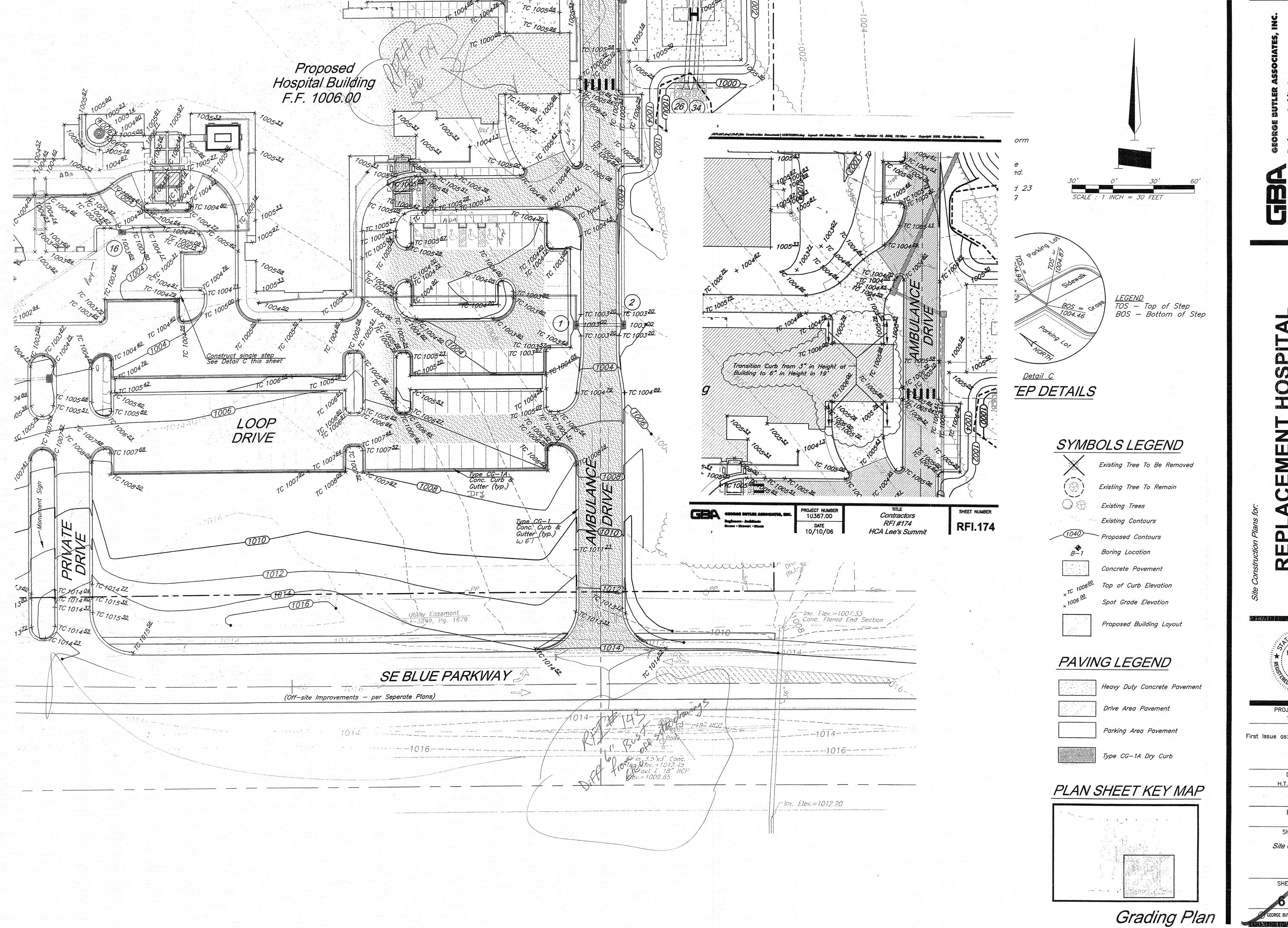
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SHEET TITLE

Site Grading Plan

5 of 29
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BURTON
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E-258627

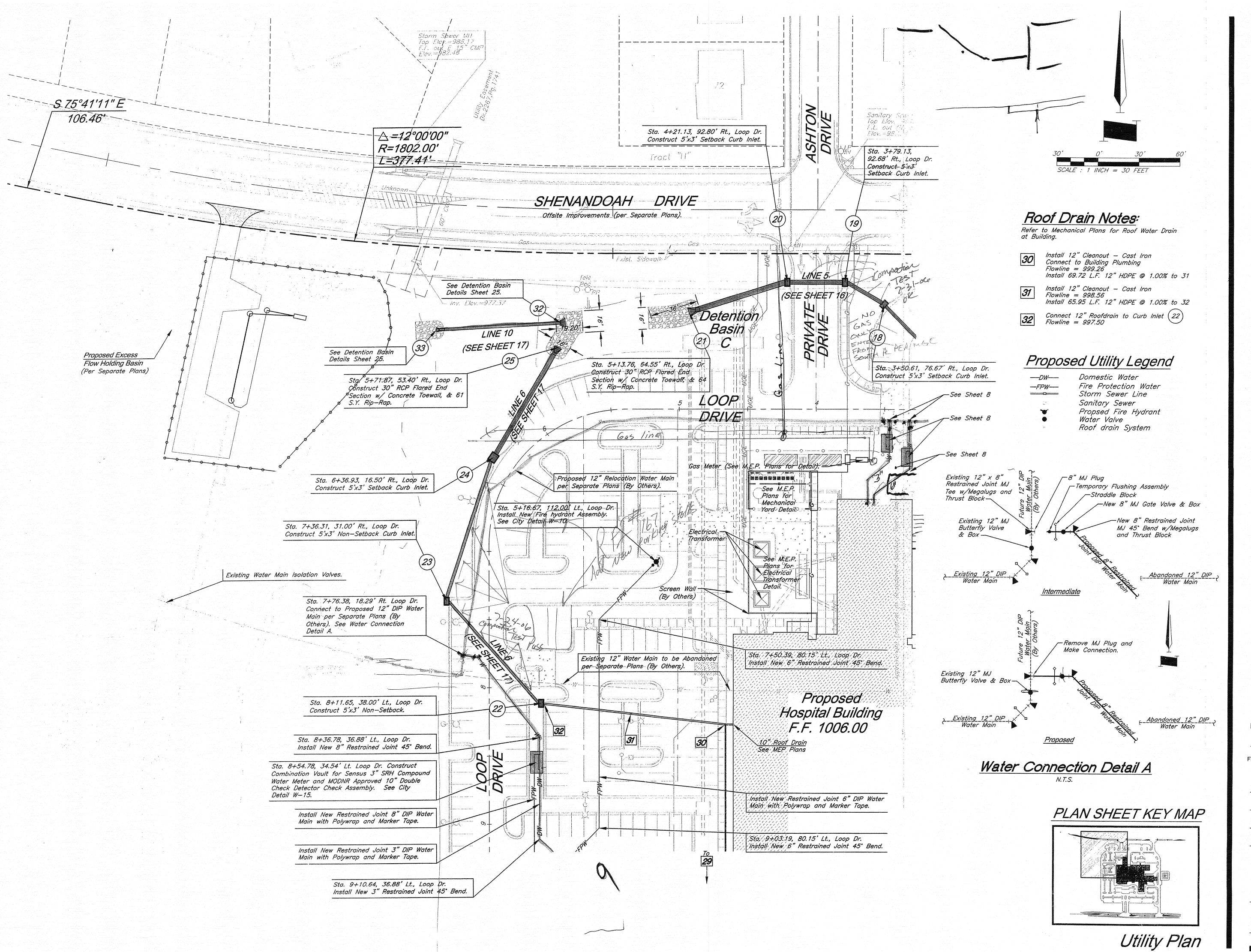
PROJECT NUMBER 10367.00 First Issue as: ASI #2 - 06/02/06

> DESIGNED H.T.R. / J.W.M.

REVIEWED B.D.B. SHEET TITLE

Site Grading Plan

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Missouri • Illinois

Kansas • Missouri • Illii One Renner Ridge 9801 Renner Boulevard

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BRADLEY D.

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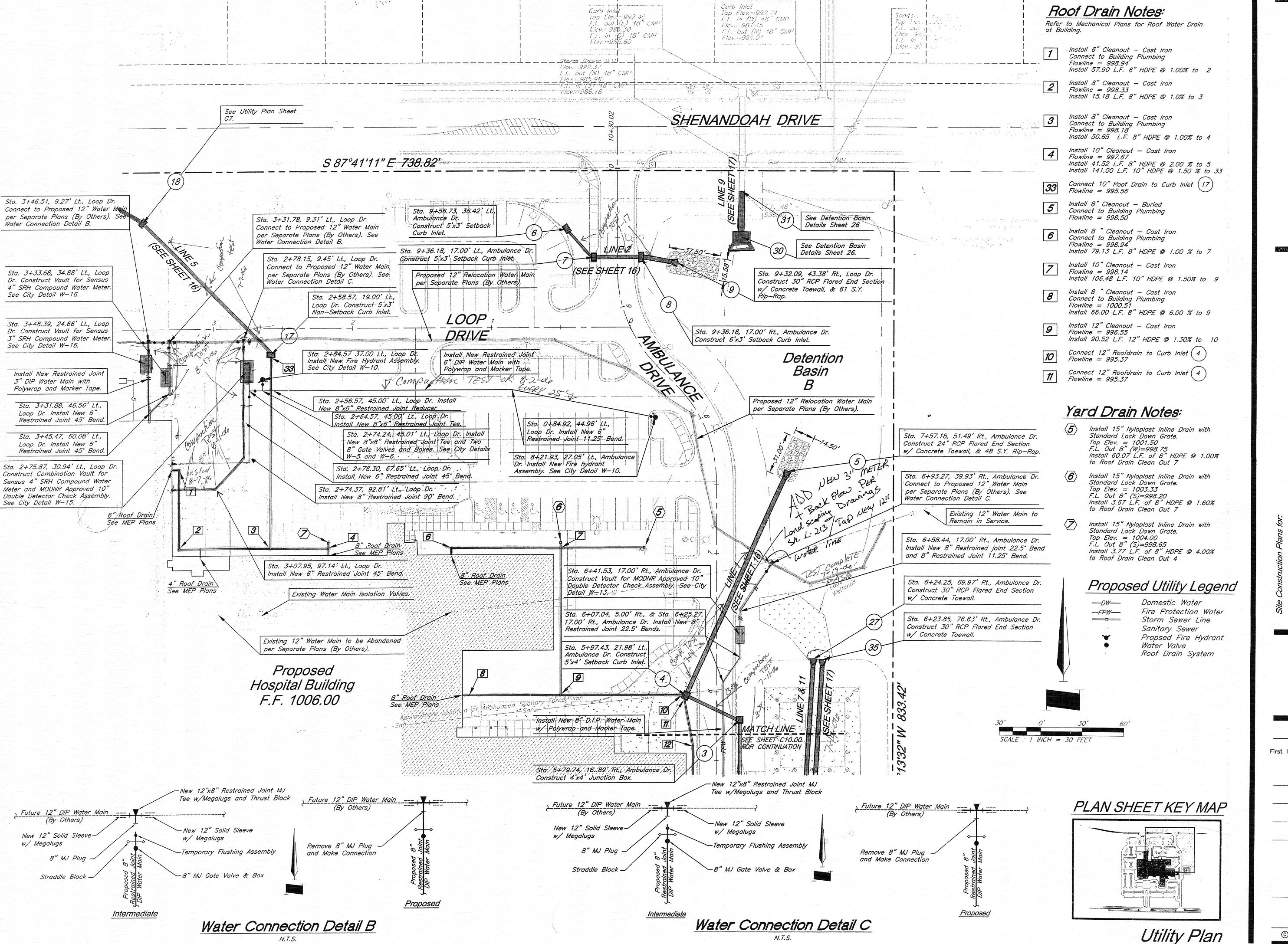
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REVIEWED
B.D.B.
SHEET TITLE

Site Utility Plan

SHEET NUMBER

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BUTLER ASSOCIATES, I. Architects

Kansas • Missouri • One Renner Ridge 9801 Renner Boulevar Lenexa, Kansas 66219

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BRADLEY D. BURTON NUMBER E-28862

PROJECT NUMBER 10367.00 DATE

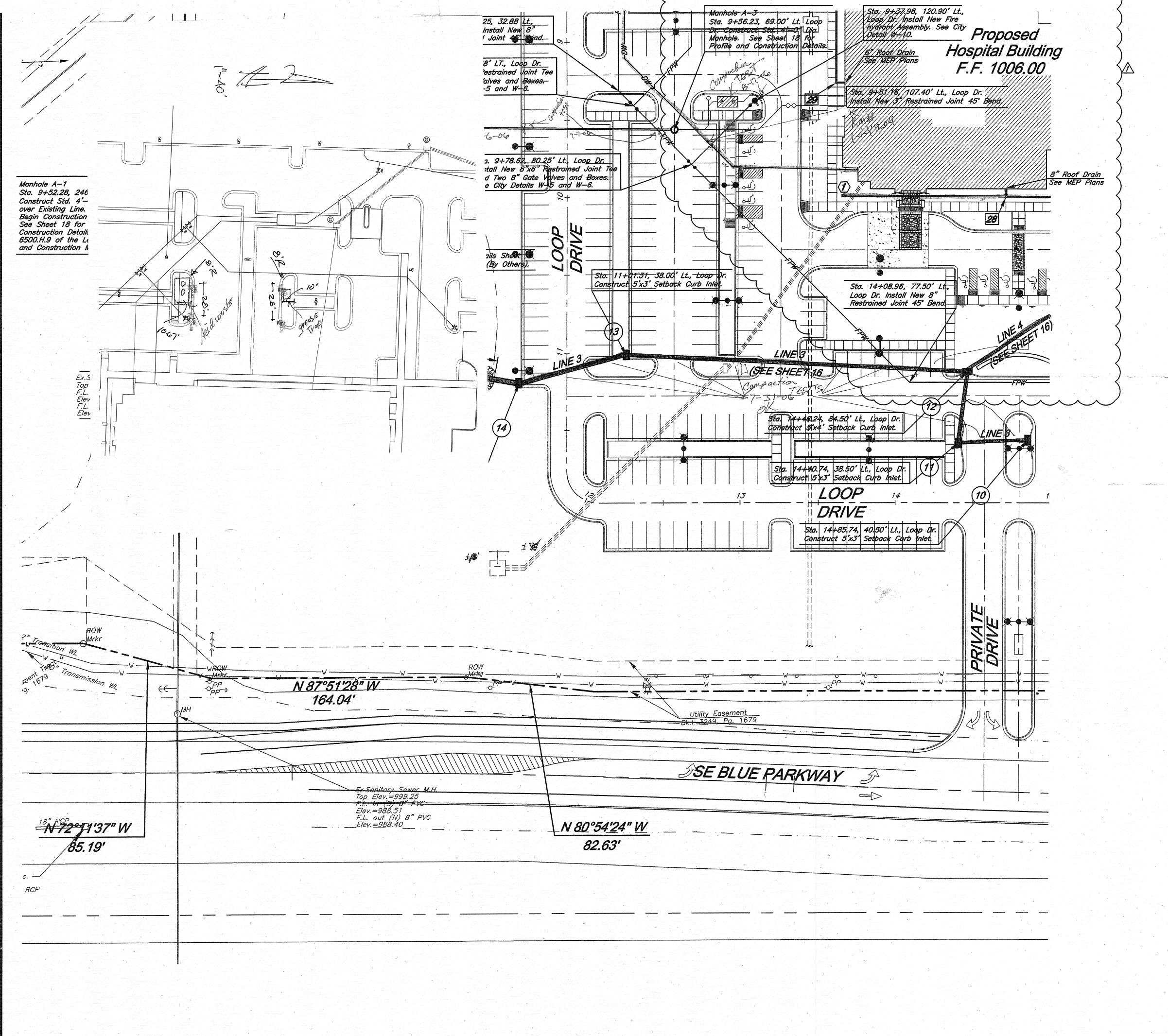
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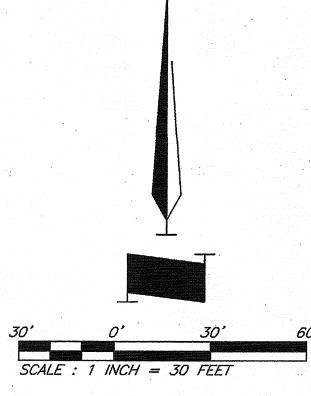
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B.I.B.
REVIEWED
B.D.B.
SHEET TITLE

Site Utility Plan

SHEET NUMBER

8 of 29
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### Roof Drain Notes:

Refer to Mechanical Plans for Roof Water Drain at Building.



Install 8" Cleanout — Cast Iron Connect to Building Plumbing Flowline = 1001.59 Install 38.99 L.F. 8" HDPE @ 1.00% to 26



Install 6" Cleanout — Cast Iron Connect to Building Plumbing Flowline = 1000.25 Install 98.93 L.F. 6" HDPE @ 1.00% to 30

### Yard Drain Notes:

Install 15" Nyloplast Inline Drain with
Standard Lock Down Grate.
Top Elev. = 1004.70
F.L. Out 8" (E)=1002.65
Install 105.56 L.F. of 8" HDPE @
1.00% to Roof Drain Clean—out 28

### Proposed Utility Legend

Domestic Water
Fire Protection Water
Storm Sewer Line
Sanitary Sewer
Propsed Fire Hydrant
Water Valve
Roof Drain System

# PLACEMENT HOSPI

Southeast Corner of Todd George Road and

BRADLEY D.

BURTON
NUMBER
E-25862

PROJECT NUMBER 10367.00

DATE
First Issue as: ASI #2 - 06/02/06

Revised: ASI #04 - 06/23/06

DESIGNED H.T.R. / J.W.M.

H.T.R. / J.W.M.
DRAWN
B.I.B.
REVIEWED
B.D.B.

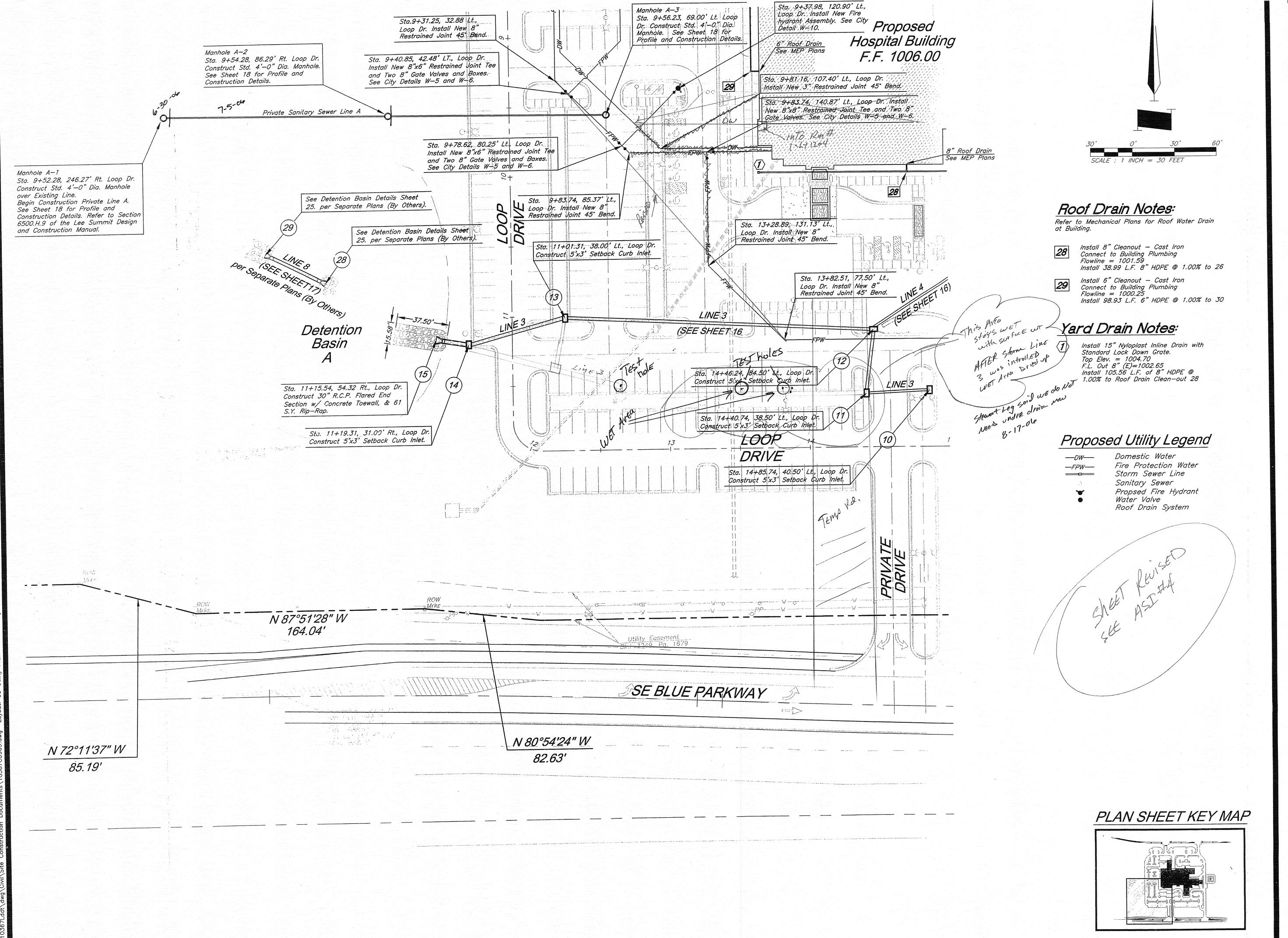
SHEET TITLE
Site Utility Plan

SHEET NUMBER

9 of 29
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Utility Plan

PLAN SHEET KEY MAP



HOSPITAL 

PROJECT NUMBER 10367.00

DATE
First Issue as: ASI #2 - 06/02/06

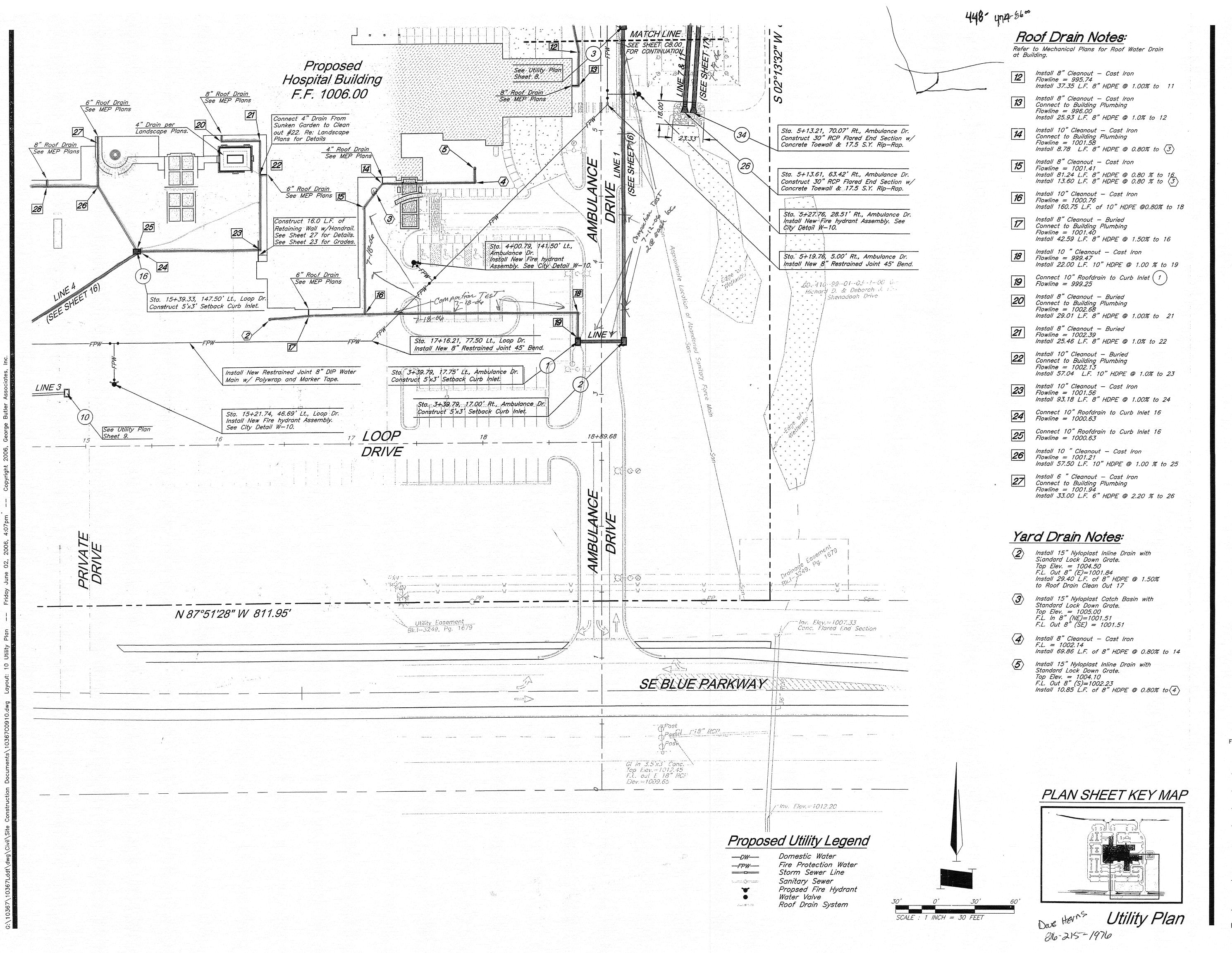
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SHEET TITLE

Site Utility Plan

SHEET NUMBER

Utility Plan



GE BUTLER ASSOCIA ers - Architects • Missouri • Illinois

One Renner Ridge 9801 Renner Bou Lenexa, Kansas 6

REPLACEMENT HOSPITAL

e Construction Plans for.

BRADLEY D.

BURTON

NUMBER

E-25862

PROJECT NUMBER

10367.00

DATE

First Issue as: ASI #2 - 06/02/06

DESIGNED
H.T.R. / J.W.M.
DRAWN

H.T.R. / J.W.M.

DRAWN

B.I.B.

REVIEWED

B.D.B.

SHEET TITLE

Site Utility Plan

SHEET NUMBER

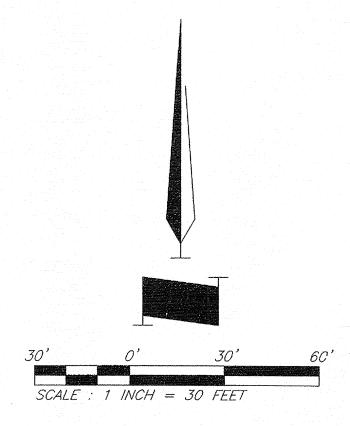
10 of 29

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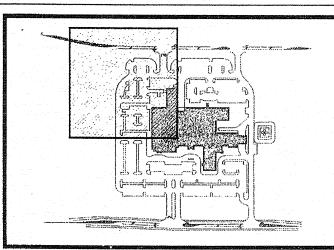
### PROPERTY DESCRIPTION

All that part of the Northwest Quarter of Section 10, Township 47 North, Range 31 West, in the City of Lee's Summit, Jackson County, Missouri, described as follows:

COMMENCING at the Southwest corner of the Northwest Quarter of Section 10, Township 47 North, Range 31 West; thence North 2 degrees 08 minutes 13 seconds East along the West line of the Northwest Quarter of said Section 10 a distance of 227.10 feet to a point; thence South 87 degrees 52 minutes 00 seconds East a distance of 7.4.22 feet to a point on the East right of way line of Todd George Road, said point also being the Northwest corner of Charlestown Park 3rd Plat, a subdivision in Lee's Summit, Jackson County, Missouri, the PONTO F BEGINNING; thence North 2 degrees 08 minutes 00 seconds East along the East right of way of Todd George Road a distance of 236.73 feet to a point on the South might of way line of Shenandoah Drive; thence South 87 degrees 41 minutes 11 seconds East along the South right of way line of Shenandoah Drive a distance of 225.97 feet to a point; thence North 83 degrees 00 minutes 25 seconds East along the South right of way line of Shenandoah Drive a distance of 111.30 feet to a point; thence South 87 degrees 41 minutes 11 seconds East along the South right of way line of Shenandoah Drive a distance or 400.03 feet to a point; thence in a Southeasterly direction along the South right of way line of Shenandoah Drive and along a curve to the right, having a radius of 1738.00 feet, through a central angle of 12 degrees 00 minutes 00 seconds, an arc distance of 364.01 feet to a point; thence South 87 degrees 41 minutes 11 seconds East along the South right of way line of Shenandoah Drive and along a curve to the left, having a radius of 100.46 feet to a point; thence in a Southeasterly direction along the South right of way line of Shenandoah Drive and distance of 364.01 feet to a point; thence South 87 degrees 41 minutes 11 seconds East along the South right of way line of Shenandoah Drive a distance of 106.46 feet to a point; thence have a second seet along the South right of way line of Shenandoah Drive and distance of 364.02 feet, through a central rangle of 12 degrees 00 minutes 00 seco





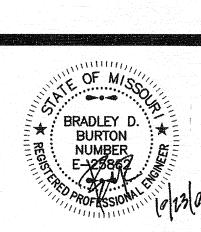


Dimension Plan

OSPITAL SE Blue Parkway

ACENENT HOSPITAL 2100 SE BILLO D.

Construction Plans for:



PROJECT NUMBER 10367.00

DATE
First Issue as: ASI #2 — 06/02/06

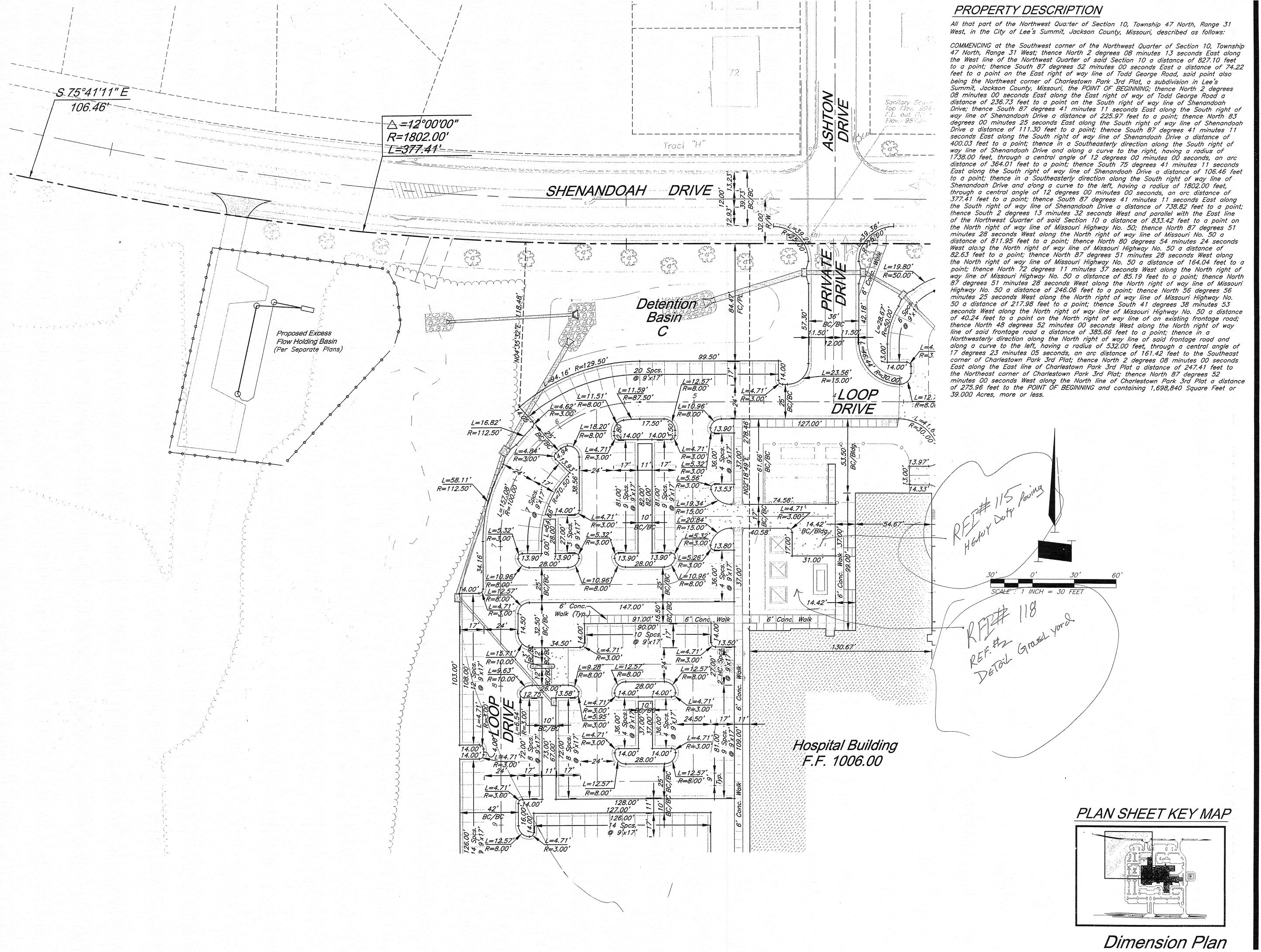
ASI #7 — 10/20/06

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H.T.R. / J.W.M.
DRAWN
B.I.B.
REVIEWED
B.D.B.
SHEET TITLE

Site Dimension Plan

SHEET NUMBER

11 of 29

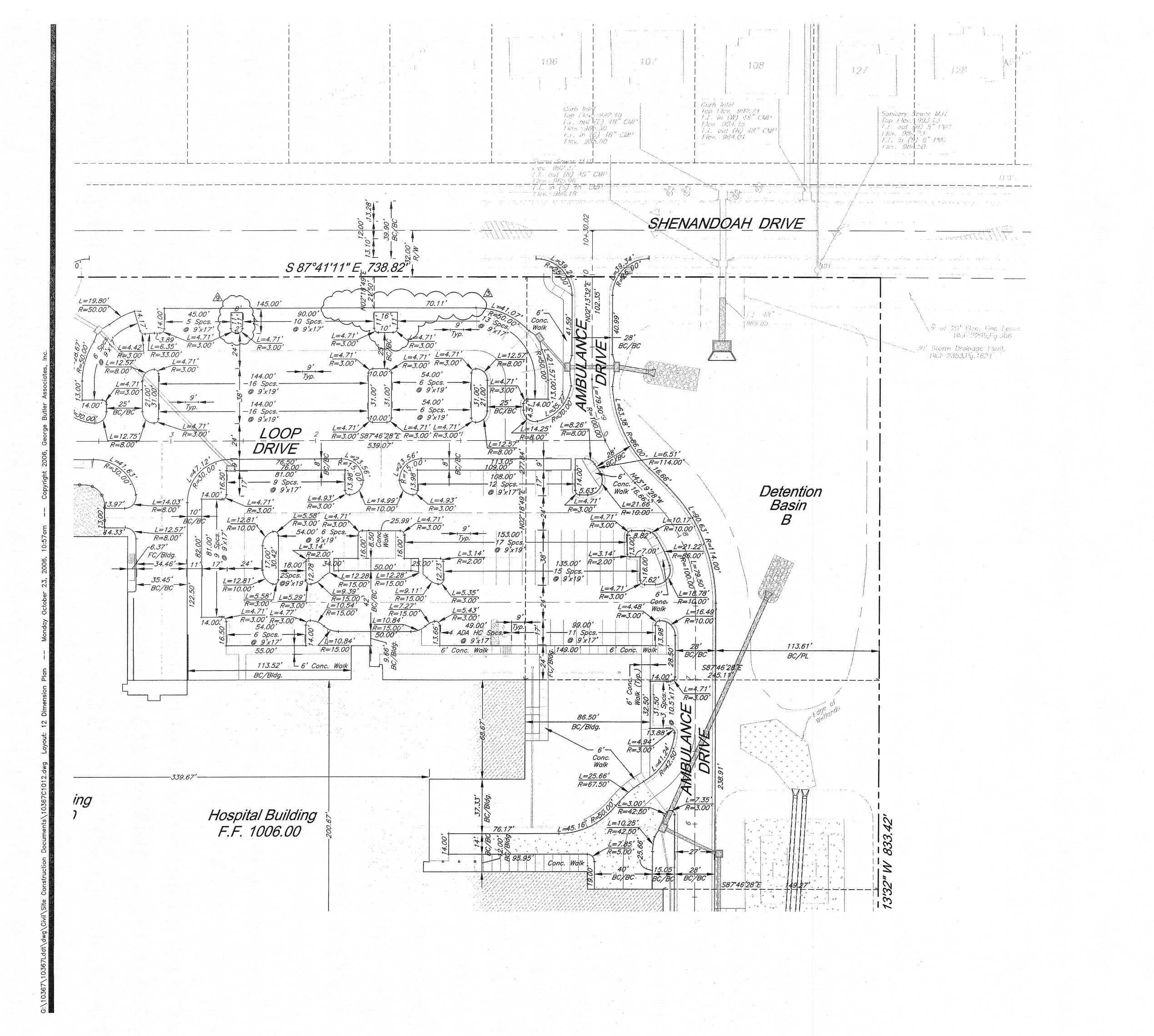


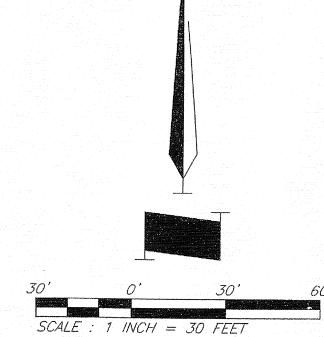
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First Issue as: ASI #2 - 06/02/06

DESIGNED **REVIEWED** 

B.D.B. SHEET TITLE Site Dimension Pl





PROJECT NUMBER 10367.00

DATE
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ARevised - RFI #090 - July 14, 2006  $\triangle$  ASI #7 - 10/20/06

> DESIGNED H.T.R. / J.W.M DRAWN

REVIEWED B.D.B. SHEET TITLE

Site Dimension Plan

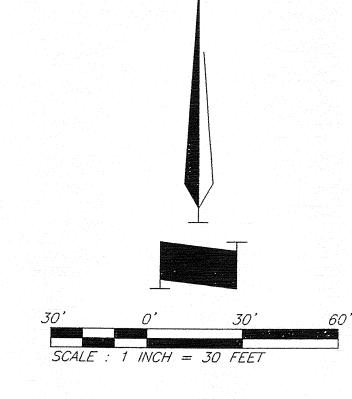
SHEET NUMBER

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Dimension Plan

PLAN SHEET KEY MAP

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# HOSPITAL-2100 SE Blue Parkway

BRADLEY D.
BURTON
NUMBER
E-25862
PROFESSION

PROJECT NUMBER
10367.00

DATE

First Issue as: ASI #2 - 06/02/06

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B.I.B.
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B.D.B.
SHEET TITLE
Site Dimension Plan

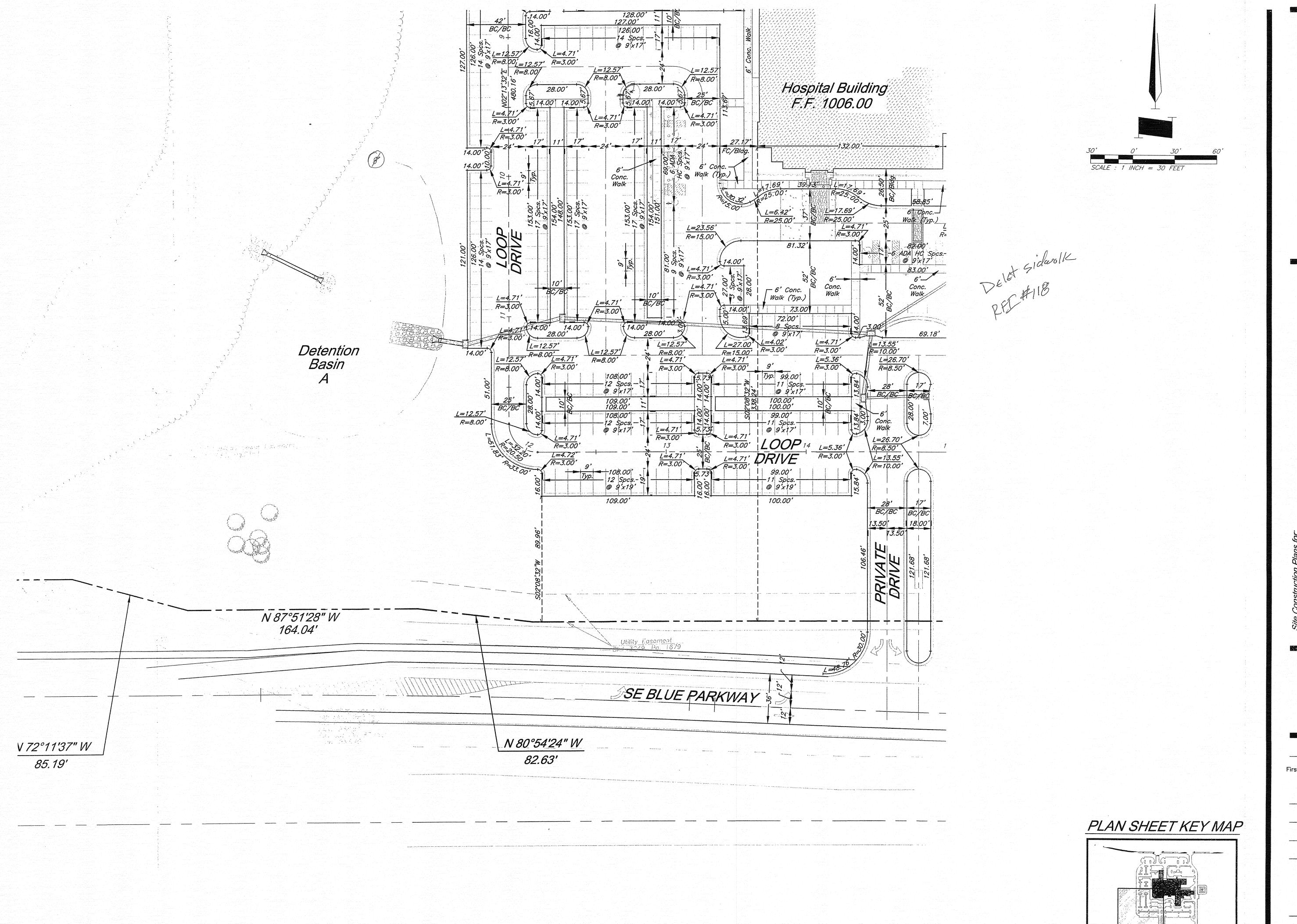
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12 of 29

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Dimension Plan

PLAN SHEET KEY MAP



Engineers - Architects
Kansas - Missouri - Illinois
One Renner Ridge
9801 Renner Boulevard
Lenexa, Kansas 66219-9745

# SUMMIT HOSPITAL-2100 SE Blue Parkwal

BRADLEY D.

BURTON

NUMBER

E-25862

PROJECT NUMBER

10367.00

DATE

First Issue as: ASI #2 - 06/02/06

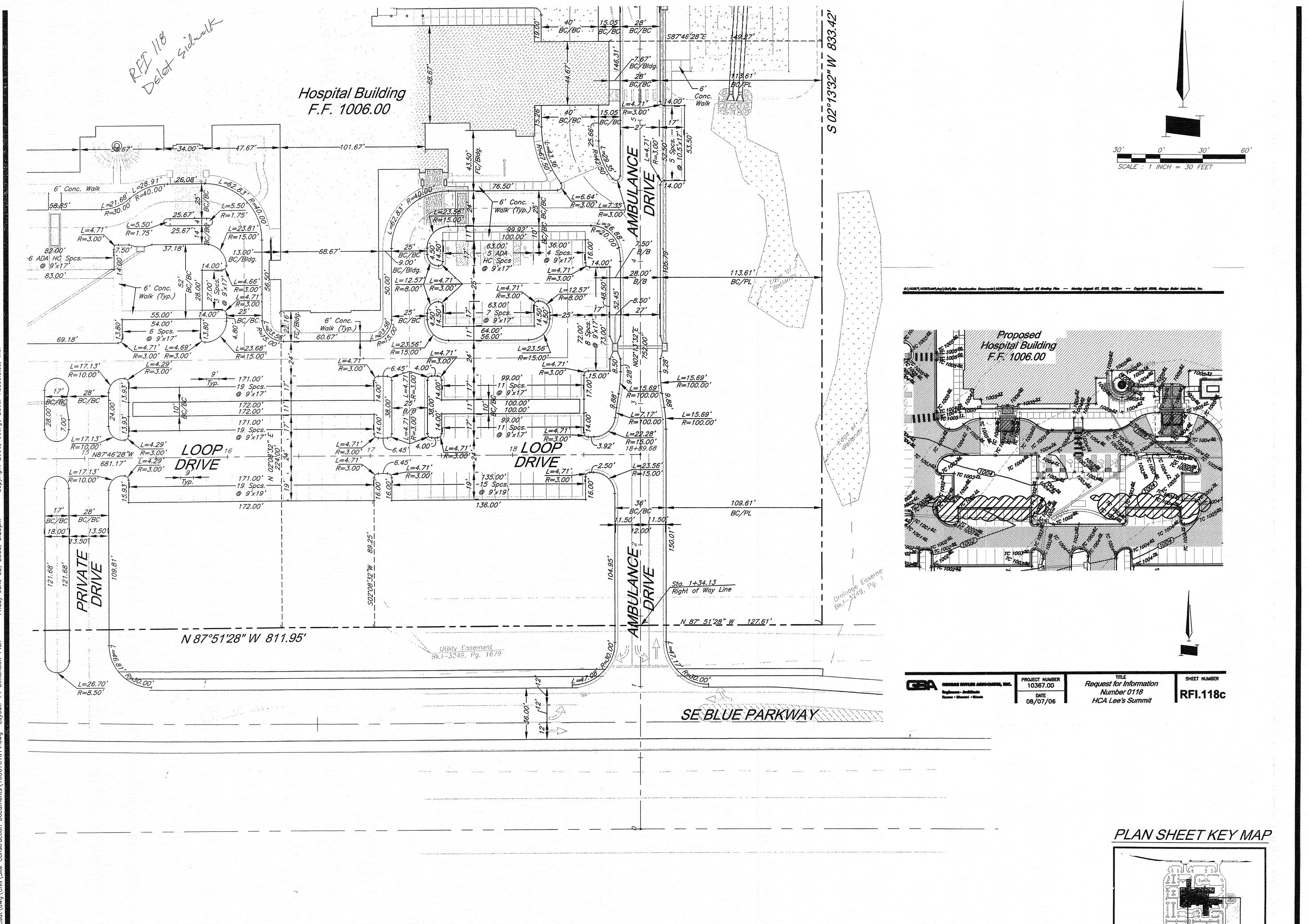
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SHEET TITLE

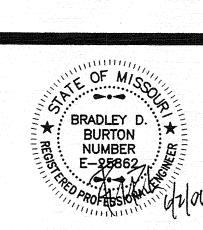
Site Dimension Plan

SHEET NUMBER

13 of 29
© GEORGE BUTLER ASSOCIATES, INC. 2

Dimension Plan





PROJECT NUMBER 10367.00

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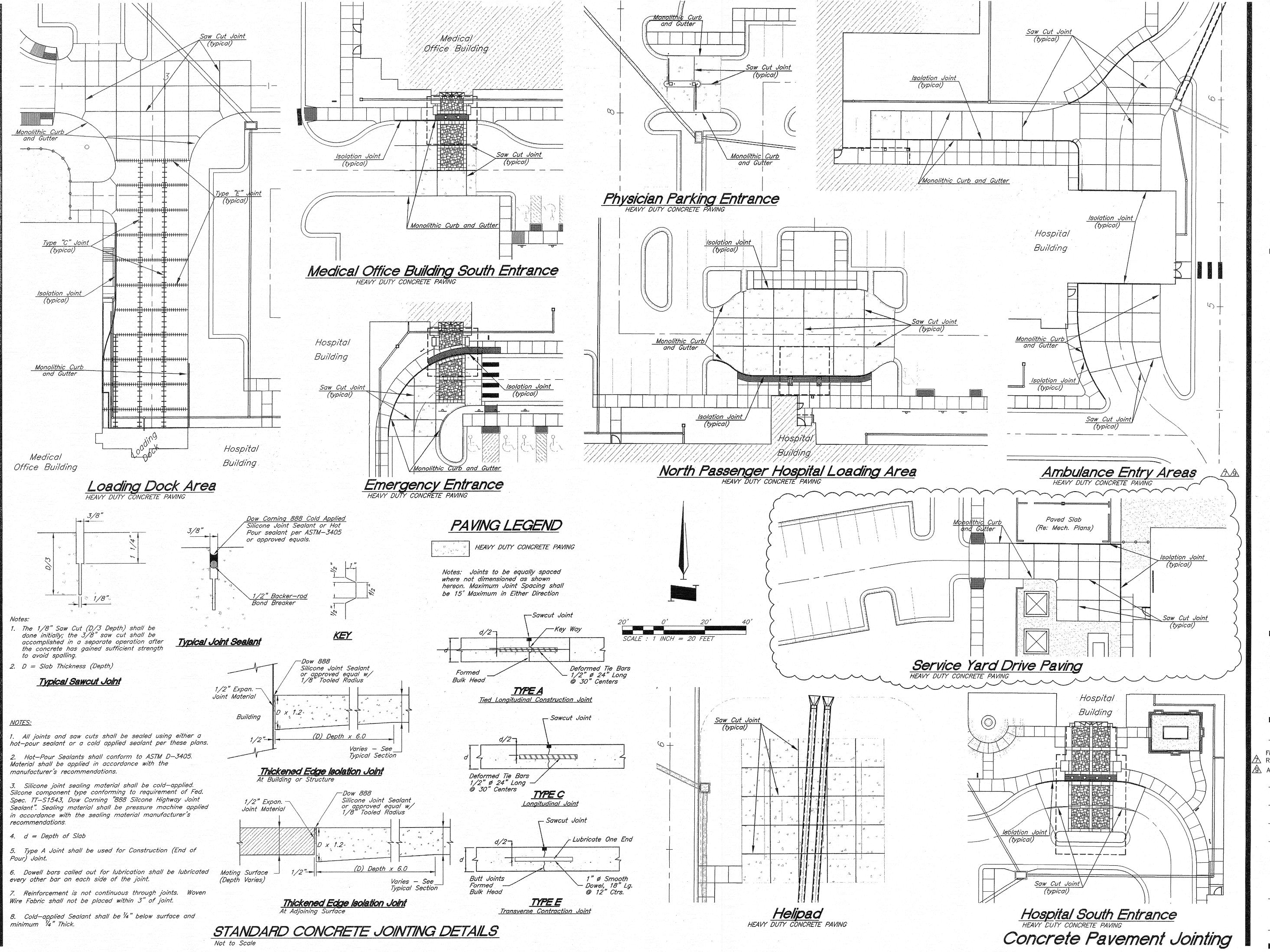
REVIEWED SHEET TITLE

Site Dimension Plan

SHEET NUMBER

14 of 29

Dimension Plan



BRADLEY D. BURTON NUMBER

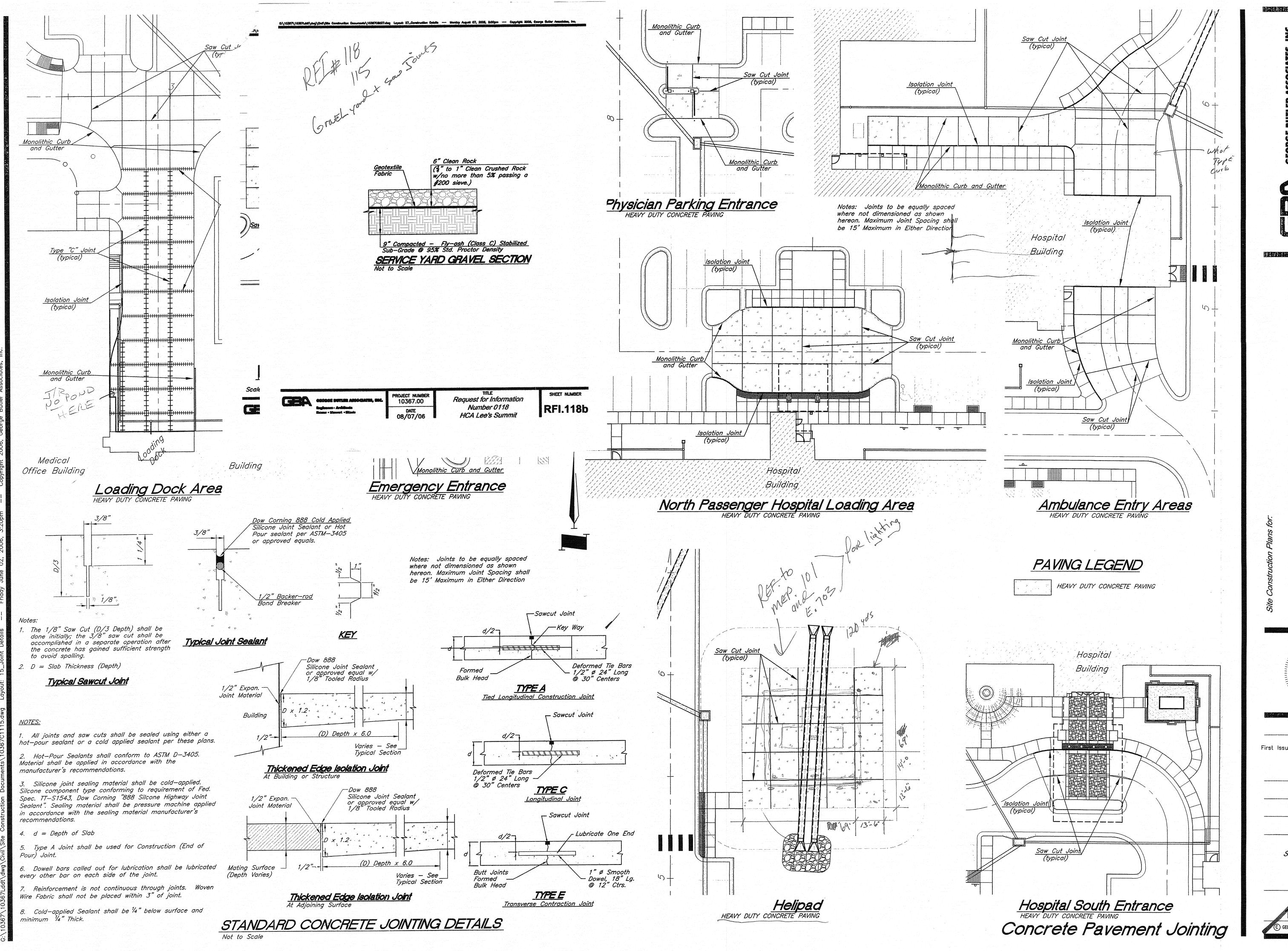
PROJECT NUMBER 10367.00 First Issue as: ASI #2 - 06/02/06 A Revised - RFI #0115 - 08/07/06 <u>∕9</u> ASI #7 — 10/20/06

DESIGNED J.W.M. DRAWN J.W.M. REVIEWED

> SHEET TITLE Site Dimension Plan

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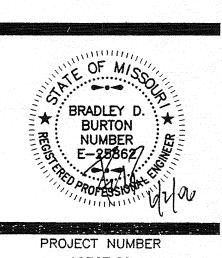
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Lenexa, Kansas 66219-9745
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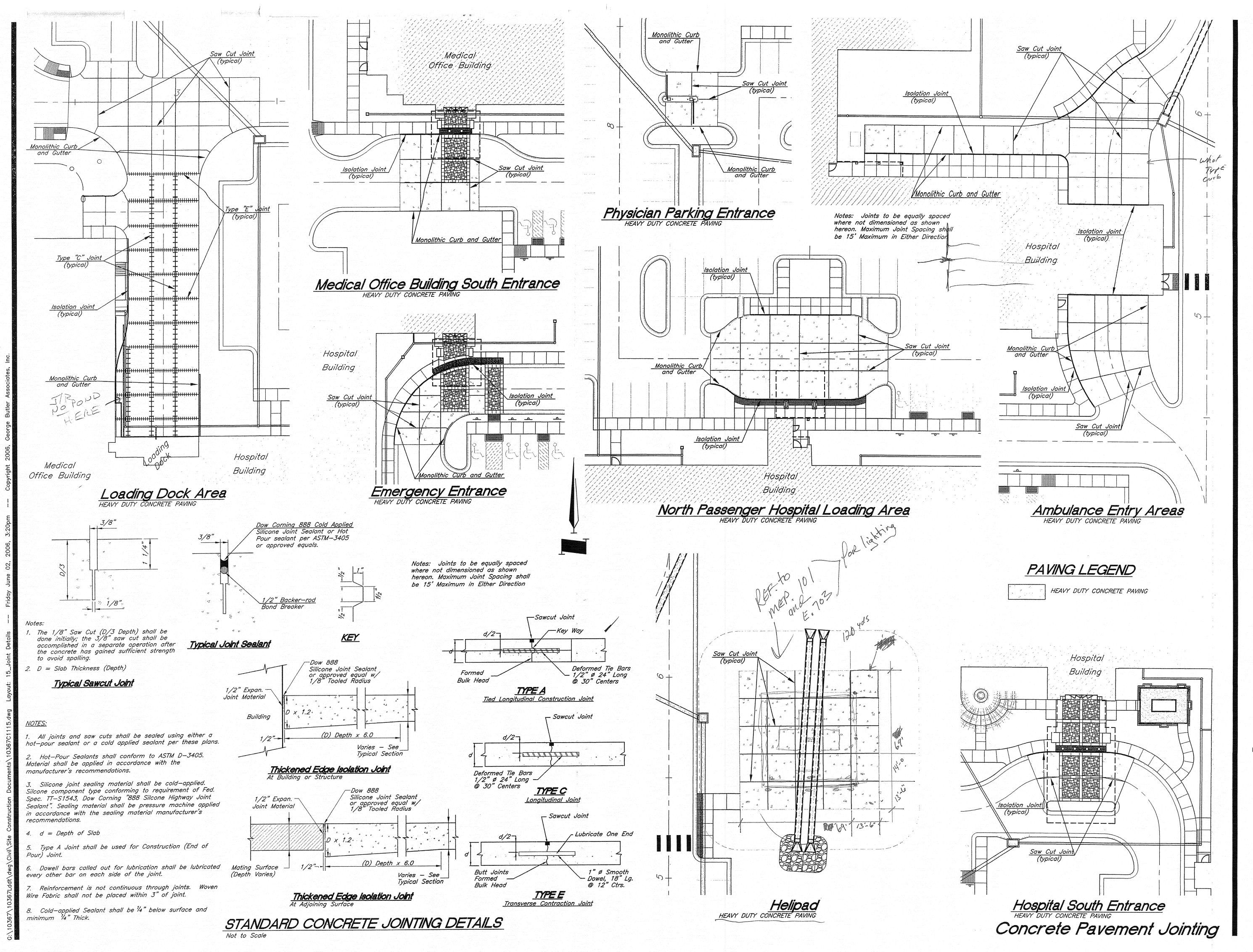
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J.W.M.
DRAWN
J.W.M.
REVIEWED
B.D.B
SHEET TITLE

B.D.B
SHEET TITLE
Site Dimension Plan

SHEET NUMBER

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9801 Renner Boulevard
Lenexa, Kansas 66219-9745
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SPTAL

Nue Parkway

Shenandoah Drive

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BRADLEY D.
BURTON
NUMBER
E-28862
PROJECT NUMBER
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PROJECT NUMBER
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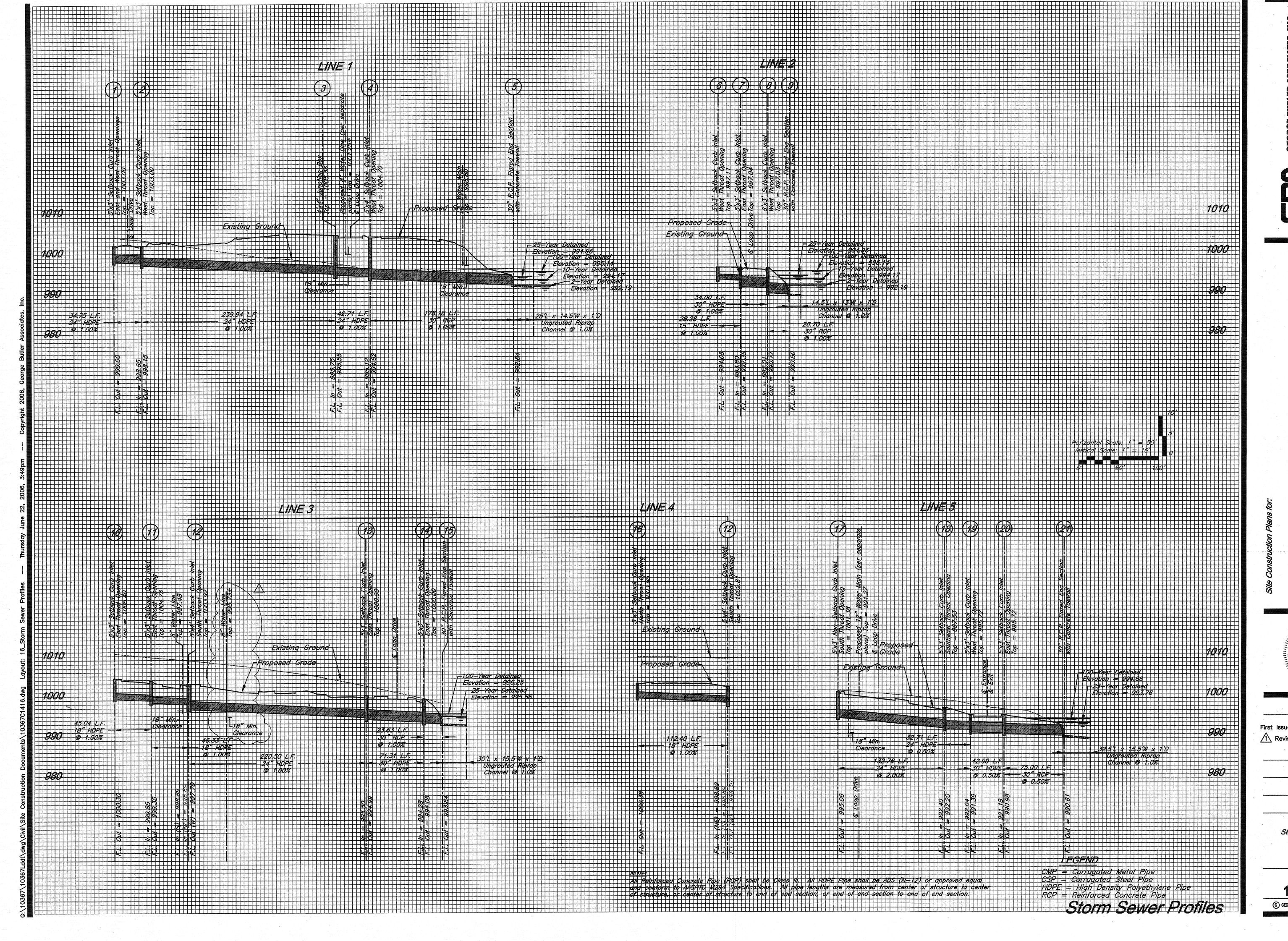
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B.D.B
SHEET TITLE
Site Dimension Plan

SHEET NUMBER

15 of 29
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BRADLEY D.
BURTON
NUMBER
E-25862

PROJECT NUMBER 10367.00

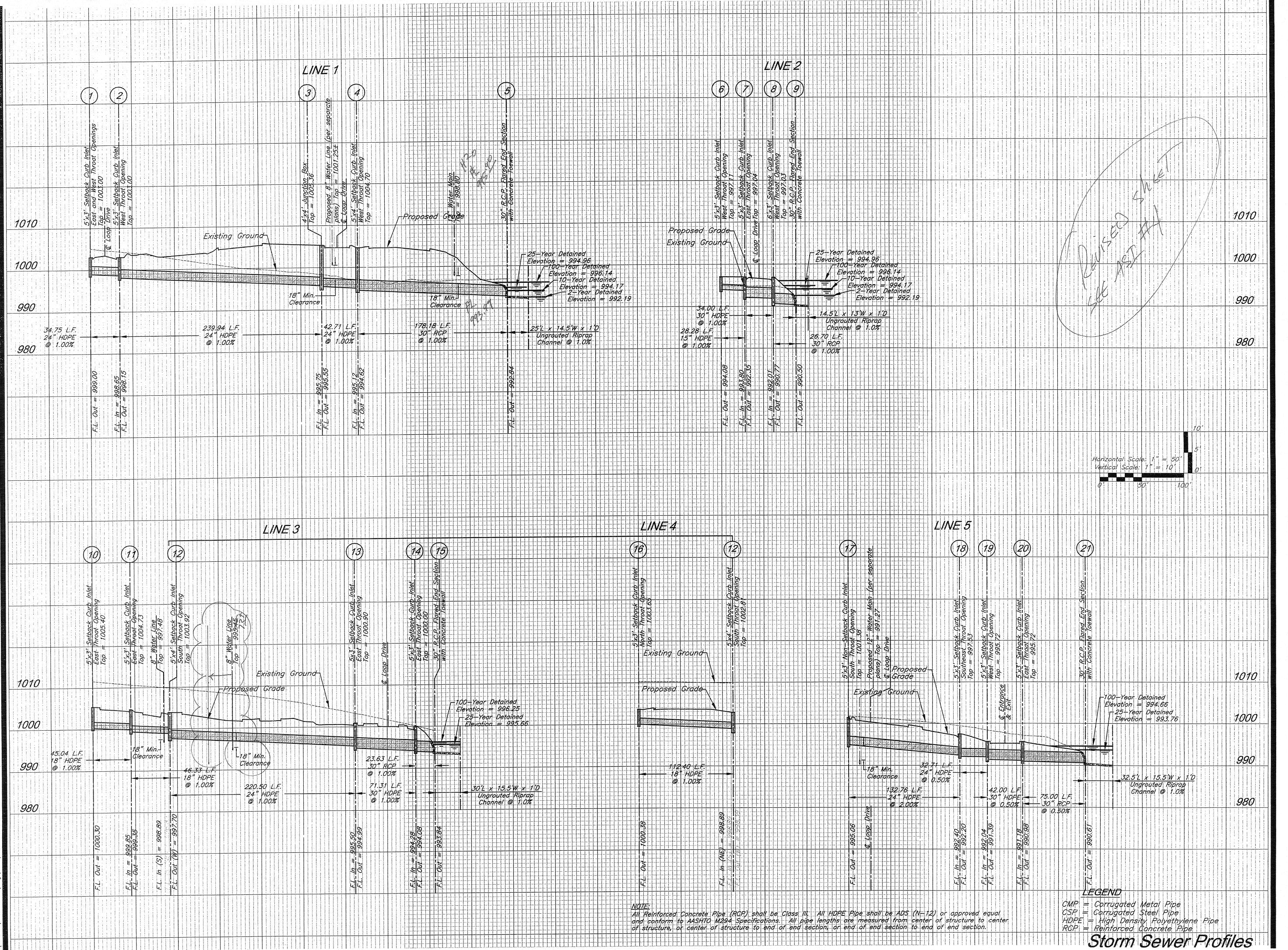
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One Renner Ridge
9801 Renner Boulevard

REPLACEMENT TO SPIRE TO SPIRE

BRADLEY D.

BURTON
NUMBER
E-25662
PROJECT NUMBER

PROJECT NUMBER

10367.00

DATE

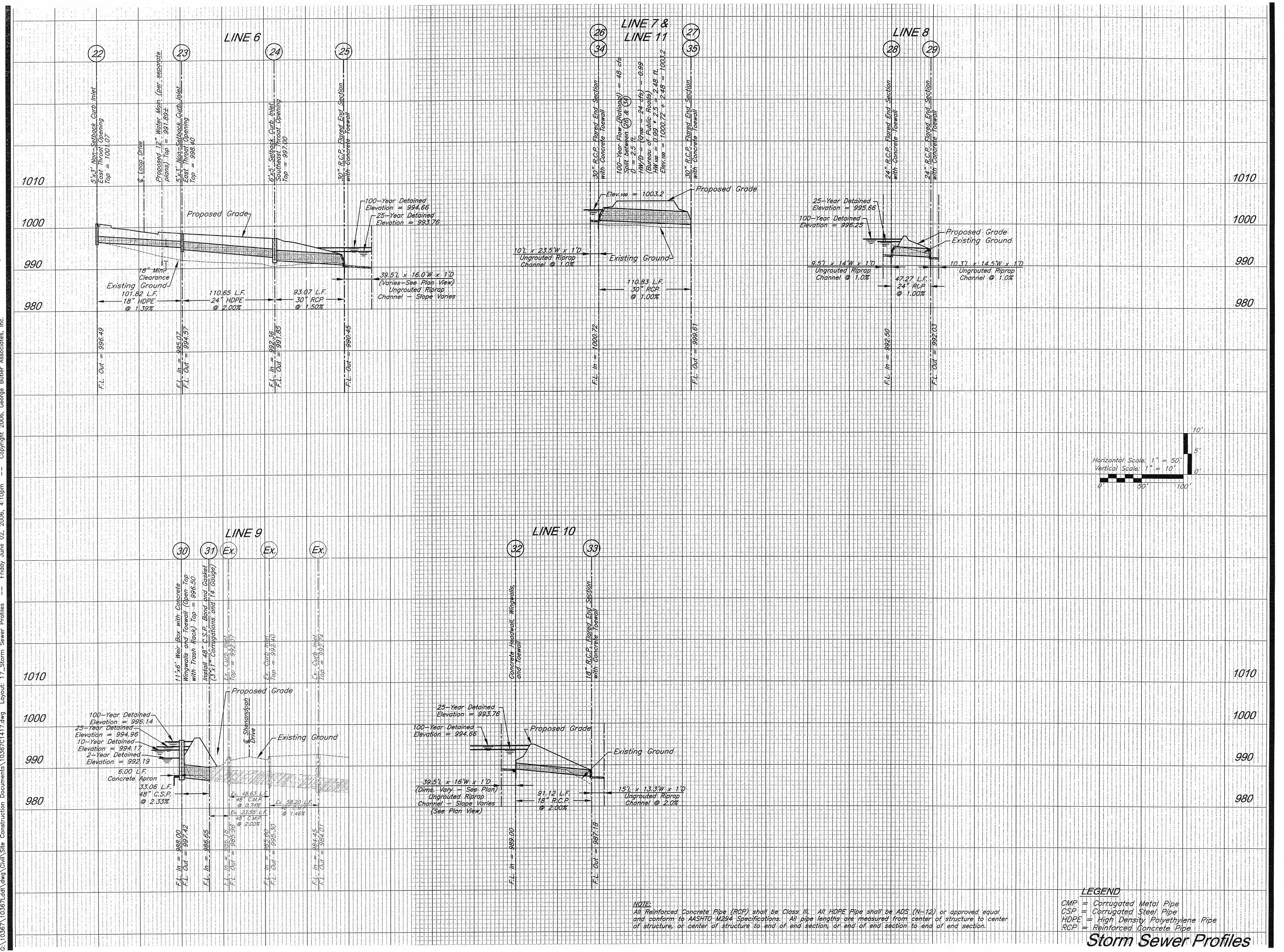
First Issue as: ASI #2 - 06/02/06

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REVIEWED
B.D.B.
SHEET TITLE

Storm Sewer Profiles

SHEET NUMBER

16 of 29
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BURTON
NUMBER
E-258627

PROJECT NUMBER 10367.00

DATE First Issue as: ASI #2 - 06/02/06

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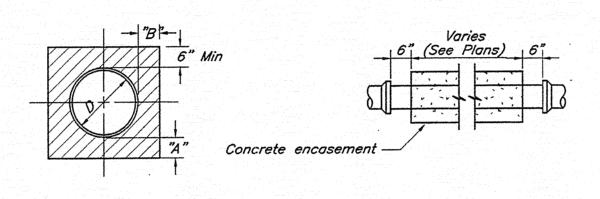
Storm Sewer Profiles

SHEET NUMBER

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# FLEXIBLE WALL CONNECTION DETAIL

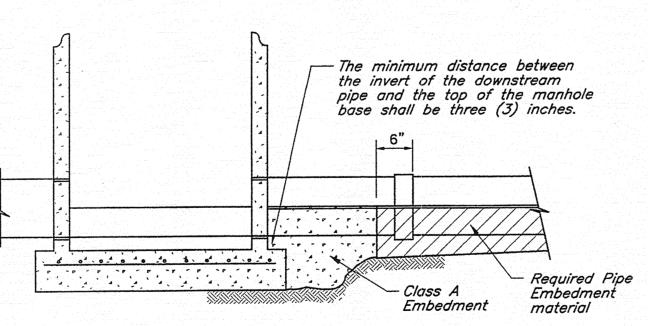


D Nominal Pipe Size A Fill Below Pipe (See Table) B Side Clearances (See Table)

Table of Bedding Depths and Side Clearances 4"-18" 6" 6" Granular Fill Concrete

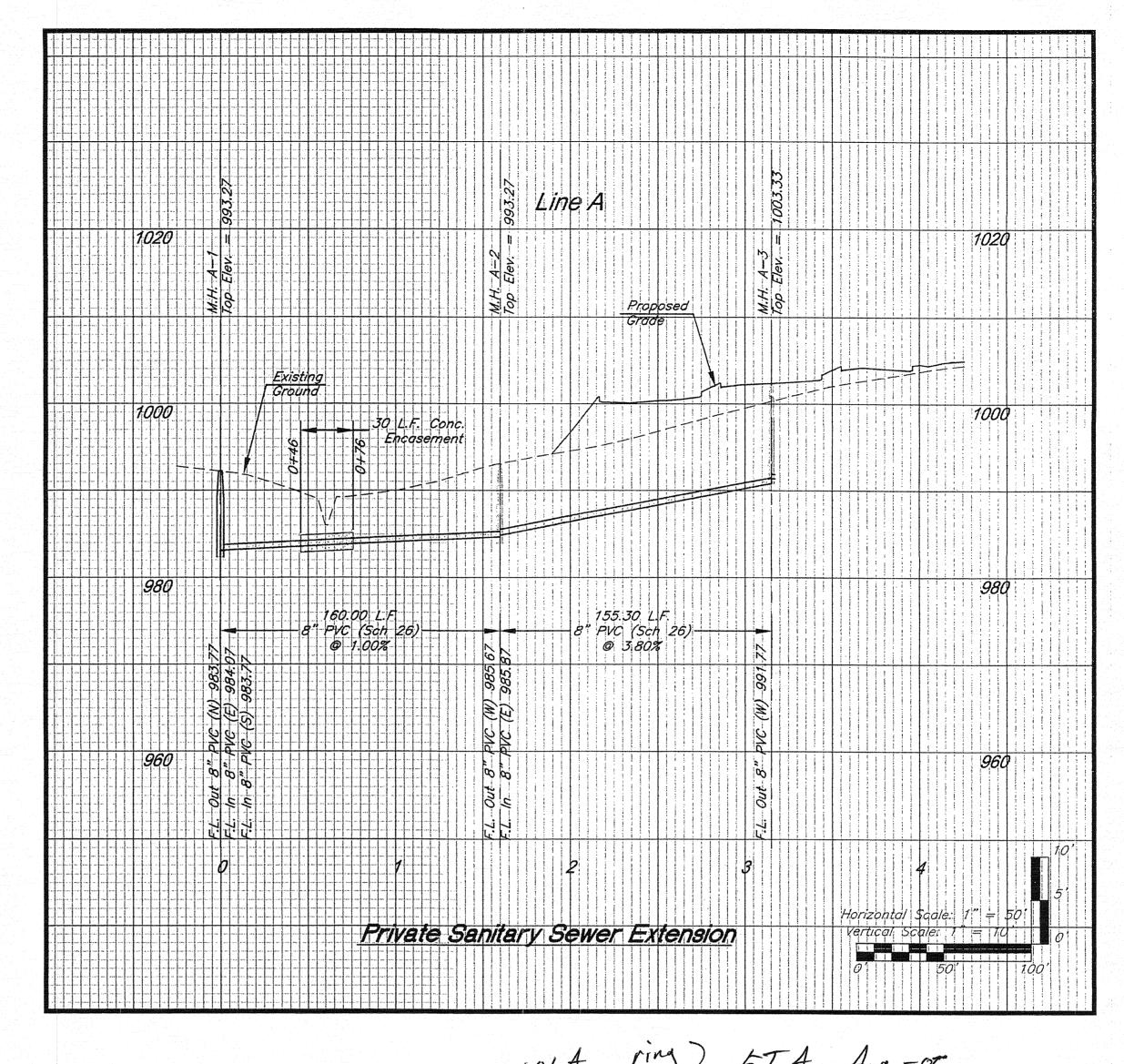
Contractor shall provide documentation supporting the selected strength class of the pipe based on earth loadings and the chosen pipe bedding.

### STANDARD EMBEDMENTS



### MANHOLE BASE SECTION

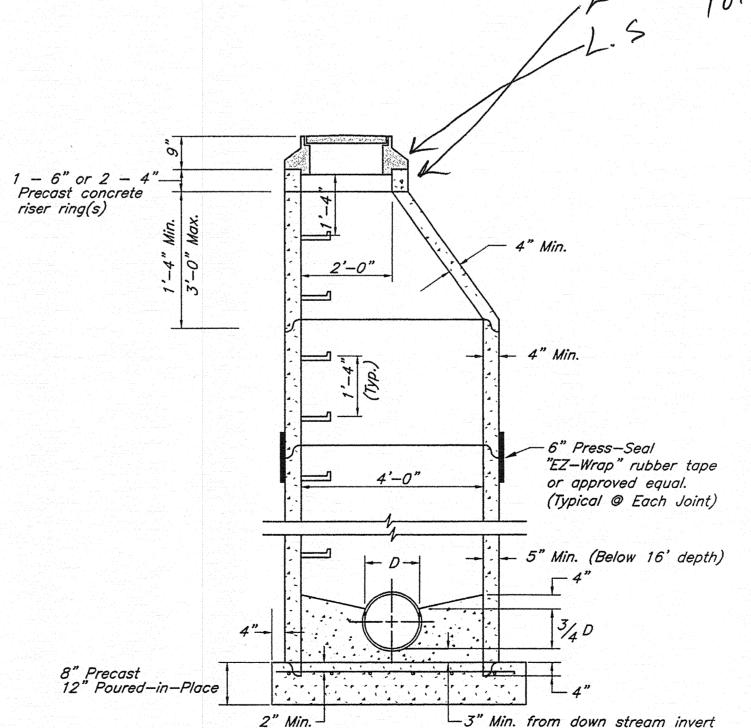
- 1st joint of pipe shall be embedded in concrete to within 6" of the 1st joint for RCP & VCP.
- If a flexible wall connection is used, class B embedment may be used.
- If flexible or semi-flexible pipe is used, flexible wall connector must be used.
- Flexible wall connections shall be press wedge, A-Lock, Press-Seal (PSX Gasket) or approved equal.



# 1.5 101A ring 2 ETA Aug-96 12.5

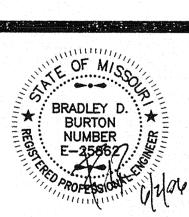
### MANHOLE NOTES:

- 1. All manhole rings shall be set in a minimum of two (2) rows of 3/4 to 1 inch pre-formed butyl joint sealant.
- 2. All manholes shall be painted with an approved bituminuous waterproofing material. The coating shall be a minimum 14 mil. dry thickness and shall be applied in accordance with the manufacturer's recommendations. Waterproofing material shall not be applied within 12" of "doghouse" or "knock—out"
- 3. Unless otherwise noted on the plans, the inside diameter of the manhole shall be 4'-0".
- 4. All manhole bases (pre-cast or poured-in-place) shall have No. 4 reinforcing bars placed on 6" centers both ways.
- 5. All standard manhole rings and covers shall be Clay & Bailey 2007MR, Neenah R—1669 with full mud ring, Deeter R—1916E with full mud ring, or approved equal.
- 6. Steel core, plastic coated steps shall be used (M.A. Ind., Inc. No. PS2-PF, or approved equal).
- 7. Reinforcement in all sections shall equal or exceed A.S.T.M. C-478 specifications.
- 8. Butyl material to be used at all manhole joints. O-Rings may be used for joints below the cone section, but the cone section itself shall not have O-ring joints.
- 9. Approved waterstop gasket and concrete mortar to be used around pipe in manholes where "doghouse" or "knock-out" openings exist.
- 10. The manhole flowline invert elevations shown on the plans shall be adjusted (up and/or down) to account for the sewer slope from the center of the manhole to the manhole wall.



4' DIA. STANDARD PRECAST MANHOLE (ECCENTRIC CONE) No Scale

HOSPIA 



PROJECT NUMBER 10367.00

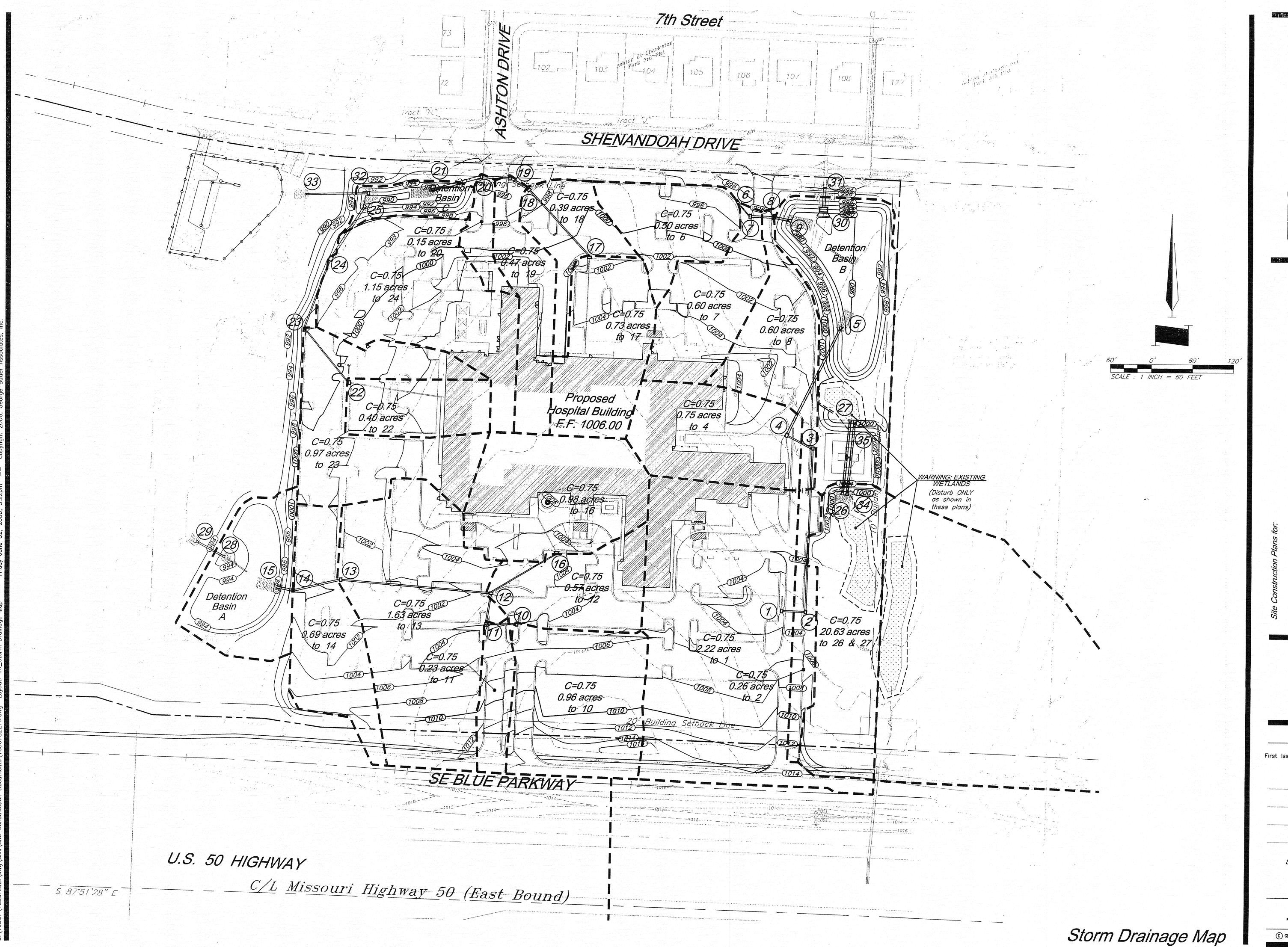
First Issue as: ASI #2 - 06/02/06

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SHEET TITLE Sanitary Sewer Profile & Details

SHEET NUMBER

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CEORGE BUTLER ASSOCIATES

Engineers - Architects

Kansas - Missouri - Illinois

One Renner Ridge

9801 Renner Boulevard

Lenexa, Kansas 66219-9745

S.S.IMMIT HOSPITAL 2400 SF Blue Parkway



PROJECT NUMBER
10367.00

DATE

First Issue as: ASI #2 - 06/02/06

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R.G.Z.
DRAWN
R.G.Z./B.I.B.
REVIEWED
B.D.B.
SHEET TITLE

Storm Drainage Map

SHEET NUMBER

19 of 29

Hospital Corporation of America	
Lee's Summit, Jackson County,	
25 Year Return Period (4% Fred	quency)

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6 23 0.97 1.10 0.75 0.83 5.00 8.53 6.83 5.37 Non-Setbock Curb Inlet  24 1.37 1.10 0.75 0.83 5.00 8.53 8.63 5.37 Non-Setbock Curb Inlet  40 1.15 1.10 0.75 0.83 5.00 1.78 8.53 8.63 5.37 Non-Setbock Curb Inlet  41 1.15 1.10 0.75 0.83 5.00 1.78 8.53 8.63 5.37 Non-Setbock Curb Inlet  42 1.15 1.10 0.75 0.83 5.00 1.78 8.53 8.64 24" HPPE Round 110.65 24 0.010 2.00 41.70 3.14 13.27 10.79 0.86 996.29 0.42 994.64 994.57 992.36  42 1.15 1.10 0.75 0.83 5.00 0.15 8.53 17.73 30" RCP Round 93.07 30 0.013 1.50 50.37 4.91 10.26 9.36 0.88 994.04 0.48 994.24 991.85 990.45  42 1.15 1.10 0.24 0.26 10.00 0.75 0.83 5.00 0.15 8.53 17.73 30" RCP Round 93.07 30 0.013 1.50 50.37 4.91 10.26 9.36 0.88 994.04 0.48 994.24 991.85 990.45  42 1.15 1.10 0.24 0.26 10.00 0.20 7.05 44.79 30" End Section  43 0 0.68 1.10 0.51 0.56 5.00 0.16 8.53 3.25 30" RCP Round 47.27 30 0.013 1.00 41.13 4.91 8.38 5.00 0.68 994.19 0.01 993.29 992.50 992.03	996.29	6.29	997.60	399.24	
24 1.15 1.10 0.75 0.83 5.00 0.77 0.83 5.00 0.77 0.83 5.00 0.77 0.83 5.00 0.77 0.83 5.00 0.77 0.83 5.00 0.78 0.83 5.00 0.78 0.85 8.00 6.85 8.00 6.85 8.00 6.85 8.00 0.78 0.00 0.00 0.00 0.00 0.00 0.00	994.4		996.29	996.57	Sized for 100-Year Storm Runoff
25	994.4	14.4	99424	995.17	Sized for 100-Year Storm Runoff
7 26 3.92 1.10 0.24 0.26 10.00 0.20 7.05 44.79 30" RCP Round 110.83 30 0.013 1.00 41.13 4.91 8.38 9.13 2.00 1005.71 3.26 1004.12 1000.72 999.61  8 28 0.68 1.10 0.51 0.56 5.00 8.53 3.25 30" End Section 997.00  8 29 0.68 1.10 0.51 0.56 5.00 0.16 8.53 3.25 30" RCP Round 47.27 30 0.013 1.00 41.13 4.91 8.38 5.00 0.68 994.19 0.01 993.29 992.50 992.03	993.76	3.76	007.76		05 V 01 D 1 1 5 11
7 26 3.92 1.10 0.24 0.26 10.00 0.20 7.05 44.79 30° RCP Round 110.83 30 0.013 1.00 41.13 4.91 8.38 9.13 2.00 1005.71 3.26 1004.12 1000.72 999.61  8 28 0.68 1.10 0.51 0.56 5.00 8.53 3.25 30° End Section 997.00  8 10 12 1.10 0.51 0.56 5.00 8.53 5.79 48° End Section 997.00			993.76		25—Year Storm Detention Elevation
8 28 0.68 1.10 0.51 0.56 5.00 0.16 8.53 3.25 30" End Section 8.50 0.013 1.00 41.13 4.91 8.38 5.00 0.68 994.19 0.01 993.29 992.03			1005.71	1005.30	Under Helipad
8 28 0.68 1.10 0.51 0.56 5.00 0.16 8.53 3.25 30" RCP Round 47.27 30 0.013 1.00 41.13 4.91 8.38 5.00 0.68 994.19 0.01 993.29 992.50 992.03 992.03 992.03 992.03 992.03 992.03 992.00 992.	1000.86	00.86	1000.86		HW/D = 4.58/2.5 = 1.83
8 28 0.68 1.70 0.51 0.56 5.00 0.16 8.53 3.25 30" RCP Round 47.27 30 0.013 1.00 41.13 4.91 8.38 5.00 0.68 994.19 0.01 993.29 992.50 992.03 992.03 992.03 992.03 992.03 992.03 992.00 992.				997.00	
29 0.68 7.70 0.51 0.56 5.00 0.76 0.53 5.79 48" End Section 997.00	993.28	33.28	994.19	997.00	Detention Basin A Outlet
1 - 30 + 30 + 30 + 30 + 30 + 30 + 30 + 30			993.28		
$1 - \frac{3}{3} + $			990.11	997.00	
	988.65	88.65			Detention Basin B Outlet
			988.65		
32 0.21 1.10 0.51 0.56 5.00 8.53 1.00 18" End Section 995.00			990.02	995.00	0.90 ft. Diameter Steel, Orifice Pla
10 32 0.21 1.10 0.51 0.56 5.00 0.32 8.53 1.00 18" RCP Round 91.12 18 0.013 2.00 14.90 1.77 8.43 4.79 0.68 990.02 0.02 987.94 989.00 987.18	987.93	37.93	987.93		Detention Basin C Outlet
11 34 3.92 1.10 0.24 0.26 10.00 7.05 44.79 30" End Section RCP Round 110.83 30 0.013 1.00 41.13 4.91 8.38 9.13 2.00 1005.71 3.26 1004.12 1000.72 999.61	1000.86	00.86	1005.71	1005.30	Under Helipad HW/D = 4.58/2.5 = 1.83
11 35 3.92 1.10 0.24 0.26 10.00 0.20 7.05 44.79 30" RCP Round 110.83 30 0.013 1.00 41.13 4.91 8.38 9.13 2.00 1005.71 3.26 1004.12 1000.72 999.61			1000.86		

STORM SEWER PIPE AND STRUCTURE TABLE

Hospital Corporation of America

				equency)											T + + 1995		18-8 1 (1-16) 1 (1)																
		<b>T</b>			Puna	ff Calcu	lations										F	ipe Desig	n						galerakerin (ki di		Design (	Checks					
	Structures	Direct	Total	Precipitation	10110	·/ CO/CE	<i>notione</i>	Flow		7.	Tributary	Structure and Pipe	Pipe	Pipe	Pipe	Round Pipe	Manning's	Pipe	Full Pipe	Pipe	Full Pipe	Partial Pipe	Inlet Control	Headwater	Outlet	Headwater	Structure Top	Upstream	Downstream	Dowrstream	Hydraulic	Hydraulic	
	aa gaalaa			Coefficient, K	C	K×G	Tc	Time		sity Ri	Punoff, Q	Description	Material				п		Discharge, C	Area, A		Velocity, V	HW/D	Inlet Ctrl Elev.		Outlet Ctrl Elev.	Elevation	Flowline Elev.	Flowline Elev.	Water Elev.	Grade Elev.	Grade Allow	. Comments
mber Fi	0111 10	(acre)					(min.)	(min.		(hr.)	(cfs)				(l.f.)	(in.)		(%)	(cfs)	(s.f.)	(fps)	(fps)	(ft./ft.)		(ft.)								
	7	2.22		1.25	0.75	0.94	6.40	e. Transition				'x3' Setback Curb Inlet															1003.00				1004.30	1002.50	East and West Throat Opening
	1 2		2.22	1.25	0.75	0.94	6.40	0.06	9.7	7	20.34 24	4" HDPE	HDPE	Round	34.75	24	0.010	1.00	29.49	3.14	9.39	10.11	1.50	1002.01	1.26	1004.30		999.00	998.65	1003.04			
	2	0.26		1.25	0.75	0.94	5.00		10.	32	2.52 5	'x3' Setback Curb Inlet							to be to people of the								1003.00				1003.04	1002.50	
	3		2.48	1.25	0.75	0.94	6.46	0.39	9.7	5	22.66 24	4" HDPE	HDPE	Round .	239.94	24	0.010	1.00	29.49	3.14	9.39	10.34	1.71	1001.56	3.61	1003.04		998.15	995.75	999.43			
	3	0.00		1.25	0.75	0.94	5.00	ver	10.3	32	0.00 4	'x4' Junction Box															1005.36				999.43	1004.86	
	4	3.40	2.48	1.25	0.75	0.94	6.88	0.07	9.6	0	22.31 2	4" HDPE	HDPE	Round	42.71	24	0.010	1.00	29.49	3.14	9.39	10.32	1.67	998.90	1.59	999.43		995.55	995.12	997.84		-	
	4	0.75		1.25	0.75	0.94	5.00	turit. Ali kitti	10.3	32	7.26 5	'x4' Setback Curb Inlet															1004.70				997.84	1004.20	Sized for 100-Year Storm I
	5		3.23	1.25	0.75	0.94	6.95	0.33	9.5	7	28.98 30	O" RCP	RCP	Round	178.18	30	0.013	1.00	41.13	4.91	<i>8.38</i>	9.06	1.23	997.68	1.70	997.84		994.62	992.84	<i>996.14</i>			
																															996.14		100-Year Storm Detention E
1.2																																	
,	17	0.73		1.25	0.75	0.94	5.00					'x3' Non-Setback Curb Inlet															1001.38				996.60	1000.88	
	18		0.73	1.25	0.75	0.94	5.00	0.22	10.3	32	7.06 2	4" HDPE	HDPE	Round	132.76	24	0.010	2.00	41.70	3.14	13.27	9.88	0.77	996.60	0.25	995.59		995.06	992.40	995.70			
<u> </u>	18	0.39		1.25	0.75	0.94	5.00		10.	32	3.77 5	'x3' Setback Curb Inlet								<b></b>					<del>                                     </del>		997.53				995.70	997.03	Sized for 100-Year Storm I
	19		1.12	1.25	0.75	0.94	5.00	0.06	10.	32	10.84 2	4" HDPE	HDPE	Round	32.71	24	0.010	0.50	20.85	3.14	6.64	6.70	0.91	994.01	0.35	995.70		992.20	992.04	<i>995.35</i>			
	19	0.47					5.00					'x3' Setback Curb Inlet								<del>                                     </del>							995.72				995.22	995.22	Sized for 100-Year Storm I
	20		1.59	1.25	0.75	0.94	5.00	0.10	10.	32	15.39 30	6" HDPE	HDPE	Round	42.00	36	0.010	0.50	37.81	4.91	7.70	7.29	0.83	993.46	0.29	995.35	205 70	991.39	991.18	995.06			
	20	0.15					5.00					'x3' Setback Curb Inlet												~~~	+	20-00	995.72				995.06	995.22	Sized for 100-Year Storm F
	21		1.74	1.25	0.75	0.94	5.00	0.29	10	32	16.84 30	6" RCP	RCP	Round	75.00	36	0.013	0.50	29.08	4.91	5.92	6.13	0.86	993.12	0.40	995.06	<u> </u>	990.98	990.61	994.66			
																												<b> </b>			994.66		100-Year Storm Detention E
																											2002.07						
	22	0.40					5.00					'x3' Non-Setback Curb Inlet								1		<b>-</b>	0.00	00700	++	000.07	1001.07	000.45			997.69	1000.57	
	23		0.40					0.23	10.	32	J.87 18	8" HDPE	HDPE	Round	101.82	18	0.010	1.39	16.14	1.77	9.13	7.51	0.80	997.69	0.25	996.87	000.40	996.49	995.07	996.62			
	23	0.97				0.94	5.00					'x3' Non-Setback Curb Inlet												000.00	+	000 70	998.40	001.55		005.55	995.62	997.90	Sized for 100-Year Storm R
	24		1.37	1.25	0.75	0.94	5.00	0.16			13.26 2		HDPE	Round	110.65	24	0.010	2.00	41.70	3.14	13.27	11.78	1.02	996.62	0.79	996.32	007.00	994.57	992.36	995.56			
	24	1.15			0.75		5.00					'x5' Setback Curb Inlet		Round					50.37	1	10.26	10.17	1.06	994.51	0.90	995.56	997.00	991.85	990.45	994.66	995.56	996.50	Sized for 100-Year Storm R
	67										24.39 30					30	1 0017	1 150	EA 77	1 101	10 26	1 1017	1 106	00151	1 000	005 5E	1	001.85	1 000 45	001 66	1		

Storm Drainage Calculations

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PROJECT NUMBER 10367.00

DATE First Issue as: ASI #2 — 06/02/06

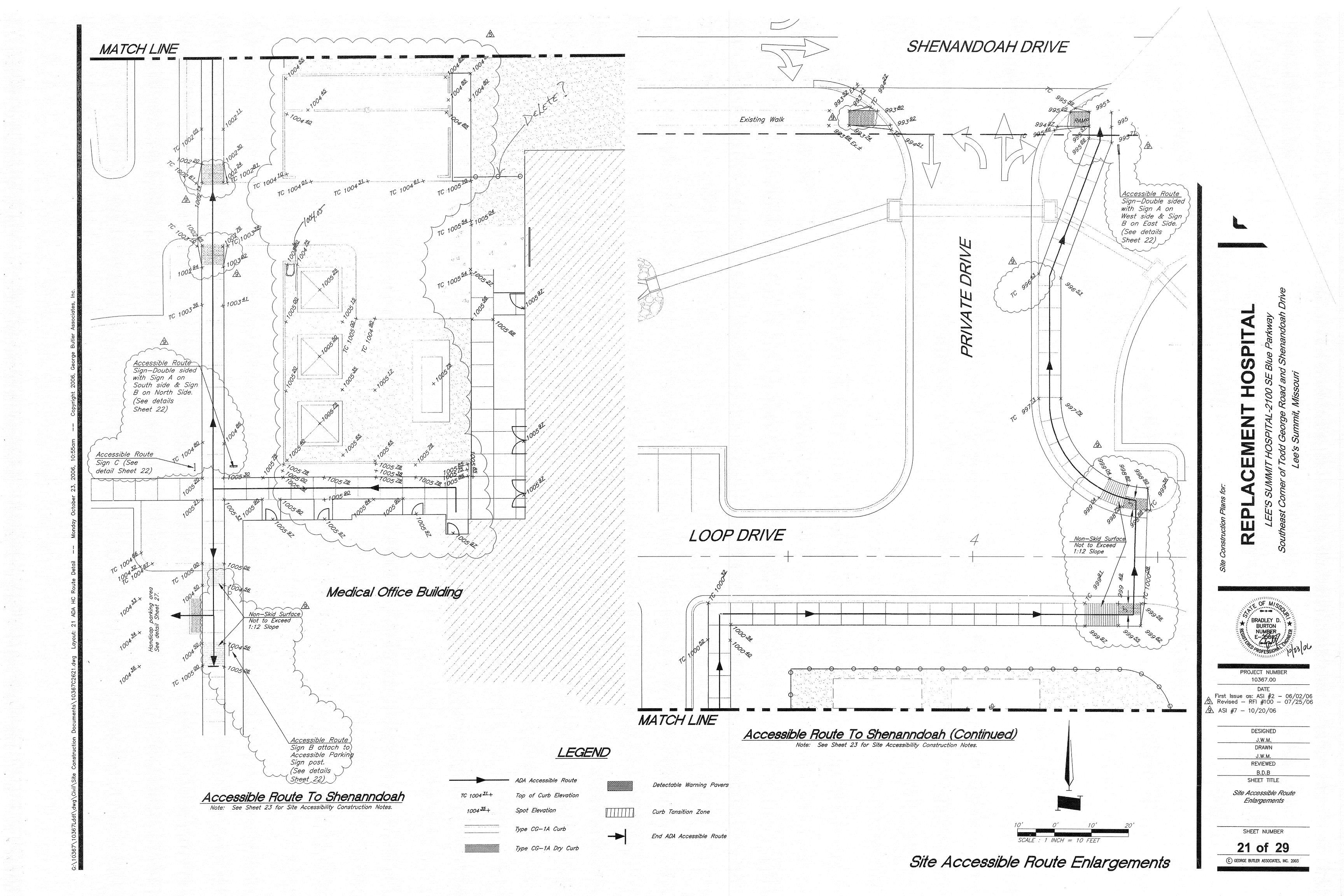
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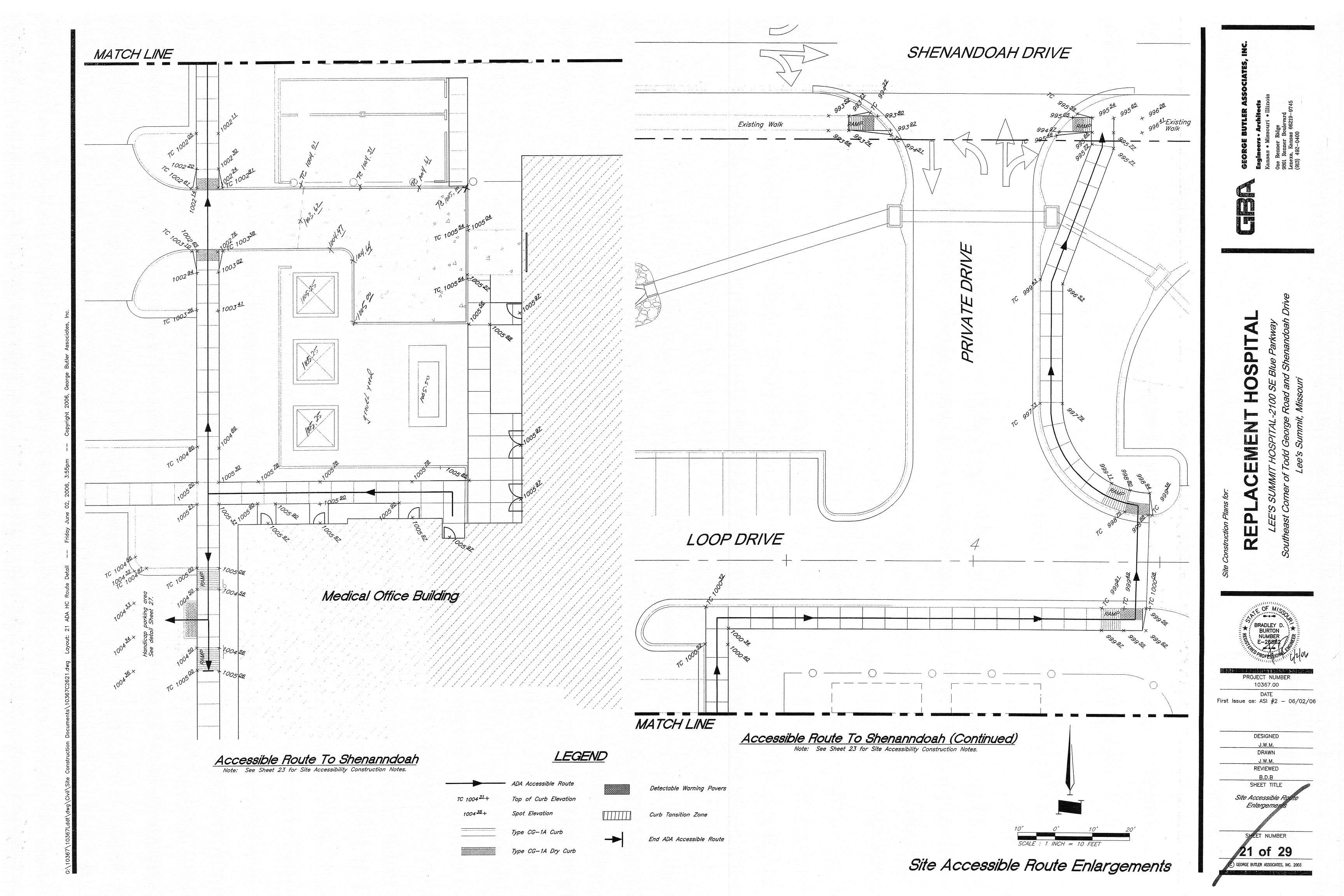
Storm Drainage Calculations

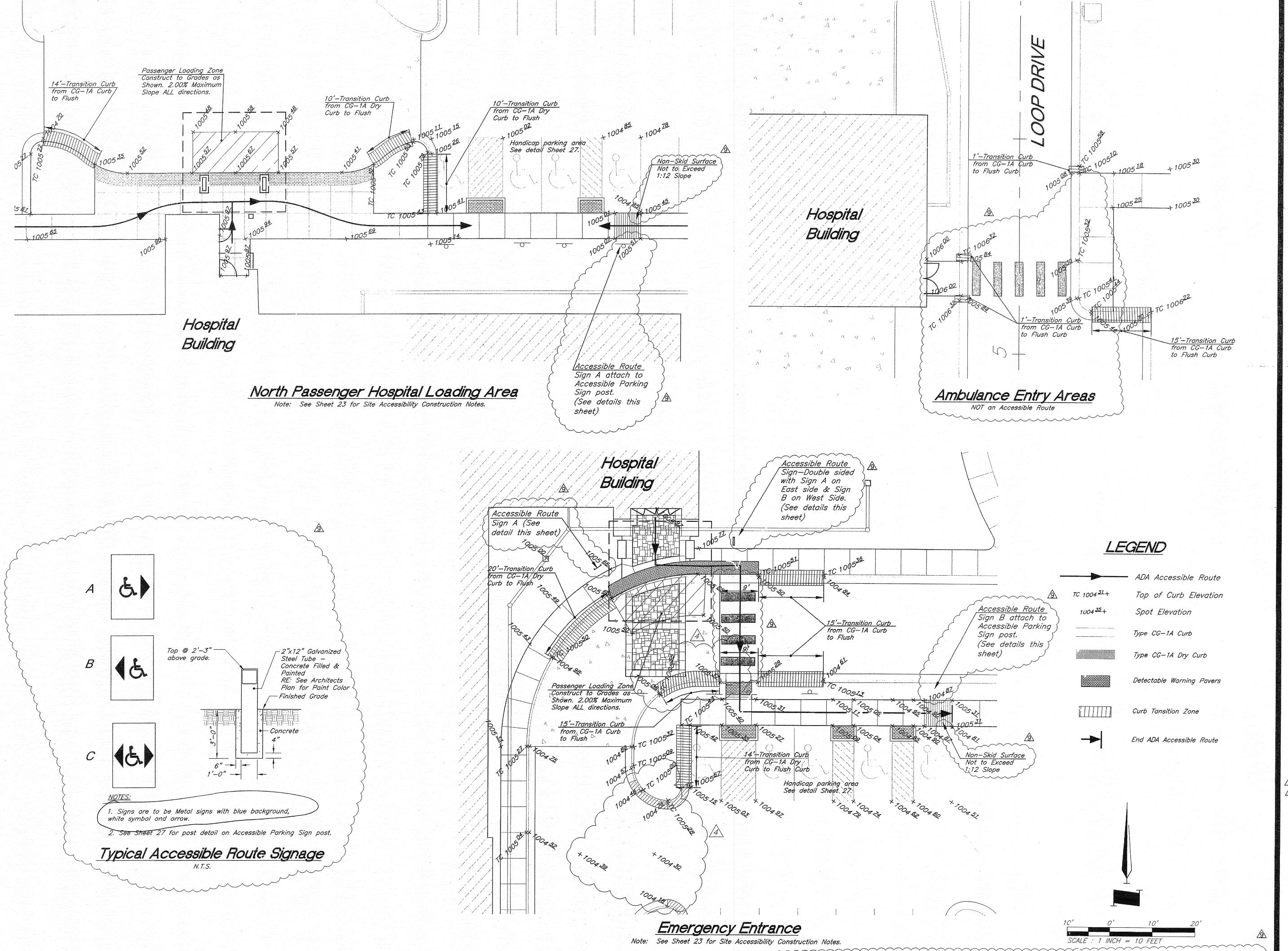
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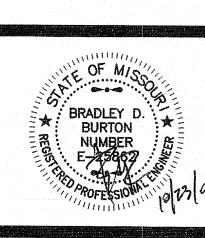


TE ASSOCIATES, I

Engineers - Architects
Kansas • Missouri • Illin
One Renner Ridge
9801 Renner Boulevard

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Construction Plans for:



PROJECT NUMBER 10367.00 DATE

DATE
First Issue as: ASI #2 - 06/02/06
Revised per RFI #093 - 07/18/06
ASI #7 - 10/20/06

DESIGNED
J.W.M.
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J.W.M.
REVIEWED
B.D.B

SHEET TITLE

Site Accessible Route
and Ambulance Entry
Enlargements

SHEET NUMBER

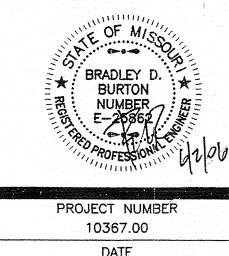
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Site Accessible Route and Ambulance Entry Enlargements

Site Accessible Route Enlargements

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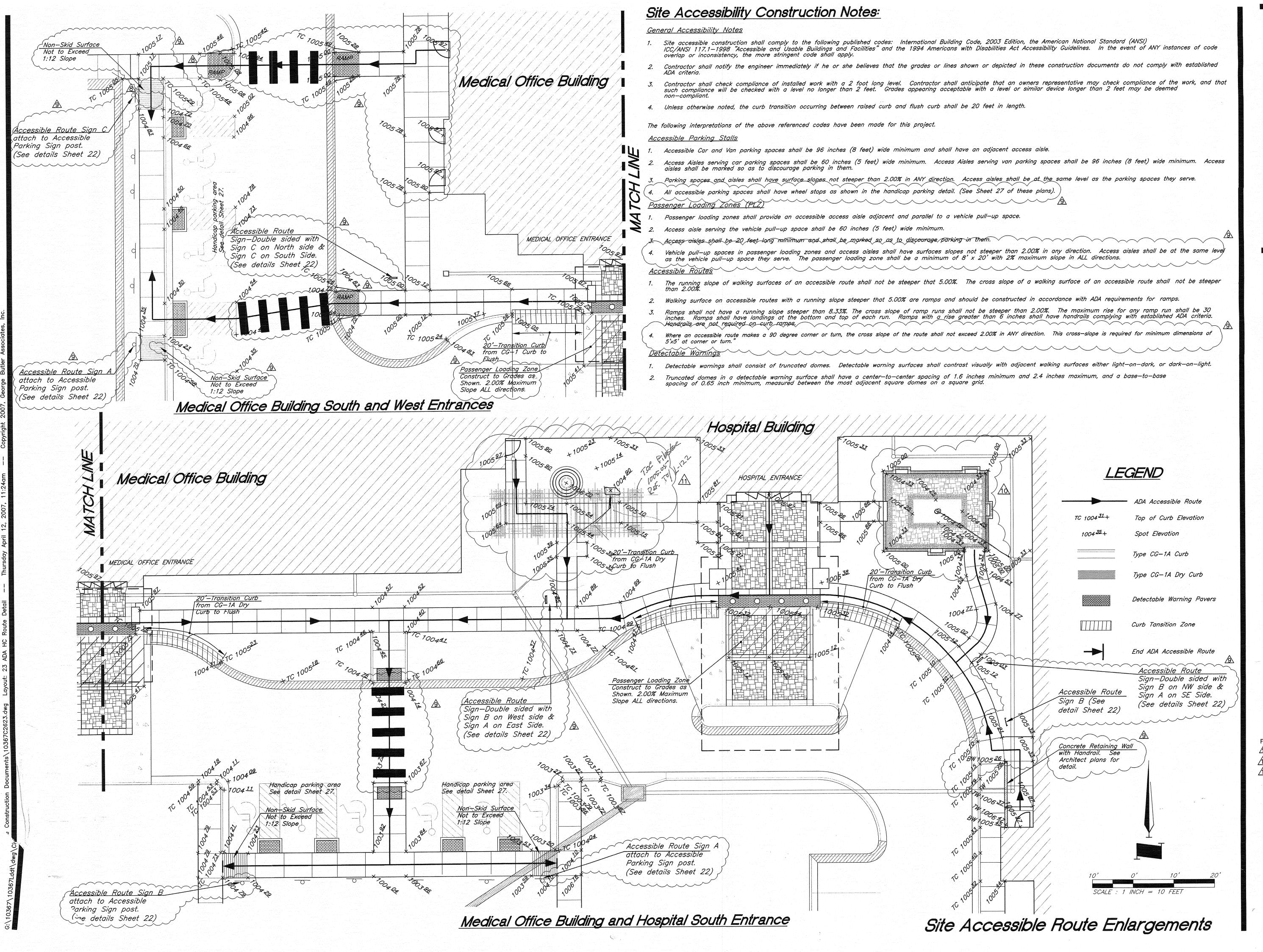
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SHEET TITLE

Site Accessible Rout Enlargements

GEORGE BUTLER ASSOCIATES, INC. 2003



BRADLEY D. BURTON NUMBER E-25862

PROJECT NUMBER First Issue as: ASI #2 - 06/02/06<u></u> ASI #7 − 10/20/06

10 RFI #349 - 4/5/07

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Site Accessible Route Enlargements

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ACEMENT HOSPITAL-2100 SE Blue Parkway

BRADLEY D.

BURTON
NUMBER
E-25862

PROJECT NUMBER
10367.00

DATE First Issue as: ASI #2 - 06/02/06

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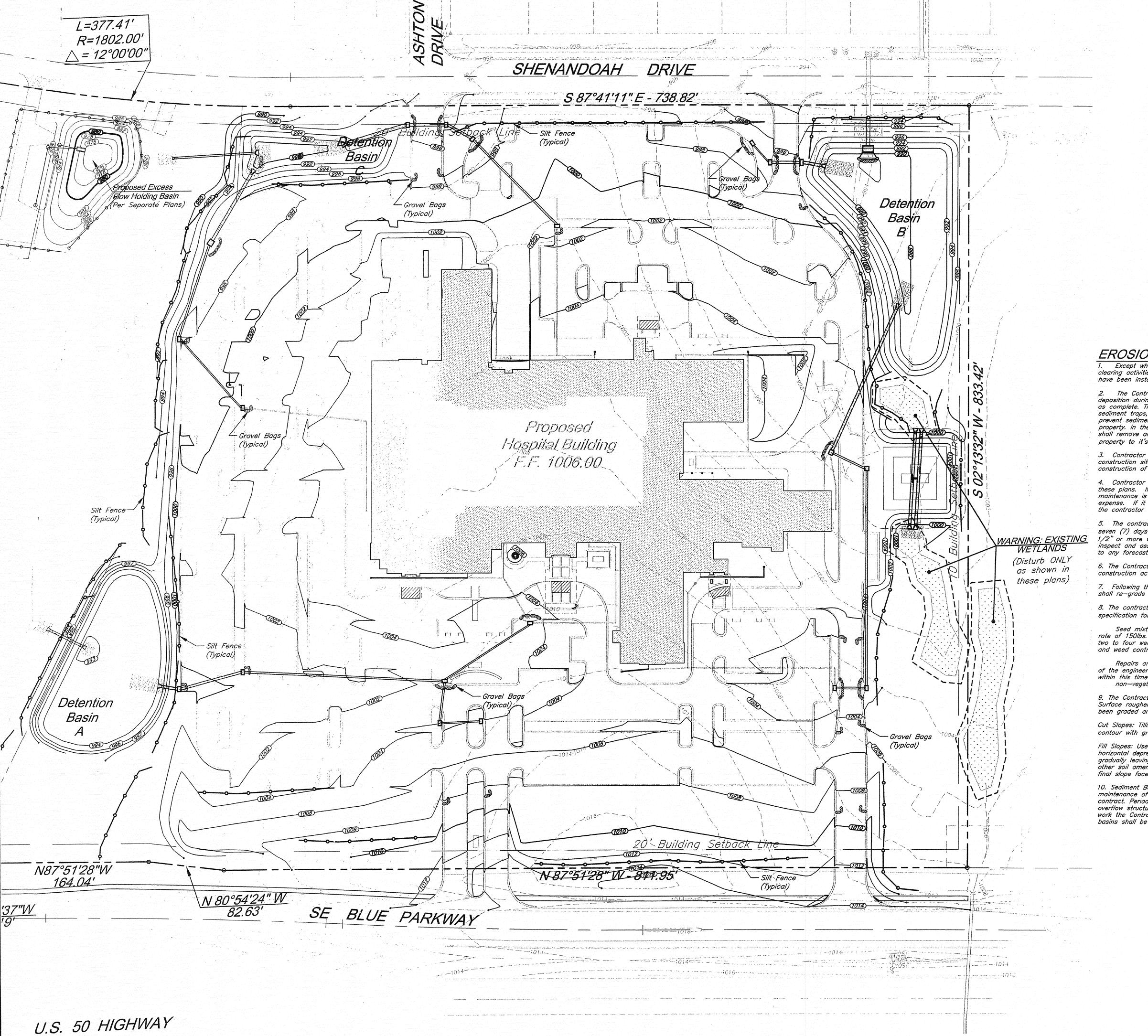
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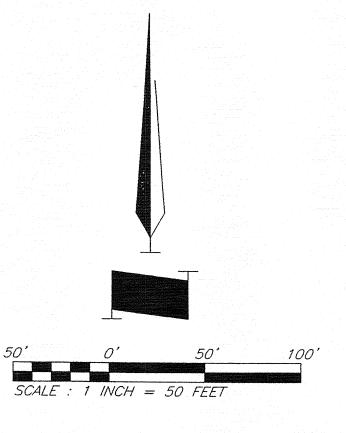
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Enlargement

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Site Accessible Route Enlargements





### LEGEND

Gravel Bags

### EROSION CONTROL NOTES

1. Except where necessary to install erosion and sediment control devices, clearing activities shall not begin until all erosion and sediment control devices have been installed and the soil has been stabilized.

2. The Contractor shall provide for control of surface erosion and sediment deposition during all phases of construction and until the owner accepts the work as complete. The Contractor shall provide temporary seeding, berms, silt fence, sediment traps, rock dam ditch checks, surface roughening or other means to prevent sediment from reaching the public right-of-way, streams or adjacent property. In the event the prevention measures are not effective, the contractor shall remove any debris sediment and restore the right-of-way and adjacent property to it's original or better condition.

3. Contractor is responsible for keeping all public roadways adjacent to the construction site free of dirt and debris resulting from activities related to the construction of this project.

4. Contractor must install and maintain the erosion control measures shown on these plans. If the City or the Engineer determines that the installation or the maintenance is inadequate, the contractor must immediately correct at his expense. If it is determined that additional erosion control measures are needed the contractor will be directed to install and maintain those measures.

5. The contractor shall inspect the erosion control measures at least once every seven (7) days and within twenty-four (24) hours following each rainfall event of 1/2" or more within any twenty—four (24) hour period. The contractor shall also inspect and assure that all sediment control devices are in working condition prior to any forecasted rainfall.

6. The Contractor shall temporarily seed all disturbed areas if there has been no construction activity on them for a period of 14 calendar days.

7. Following the final removal of all erosion control measures the Contractor shall re-grade and re-seed all areas that were disturbed by the removal.

8. The contractor shall seed or hydro seeded in accordance with city

specification for seeding and/or hydroseeding.

Seed mixture to be winter wheat or winter ryegrass sowed at an application rate of 150lbs. per acre. The seeded areas shall be inspected by the Engineer two to four weeks after seeding for adequate seed germination, erosion control and weed control.

Repairs and reseeding shall be performed by the contractor at the direction of the engineer at no additional cost. If vegetative measures are not effective within this time frame, Contractor may be required to reseed or employ a non-vegetative option to stabilize the disturbed area.

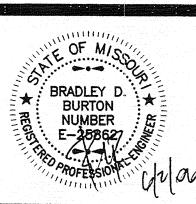
9. The Contractor shall surface roughen all slopes steeper than 8:1 on site.
Surface roughening by the Contractor shall be performed after the slopes have been graded and dressed. Acceptable methods of surface roughening are:

Cut Slopes: Tilling, disking, and harrowing running across the slope and on the contour with grooves less than 10 inches apart and not less than 1" deep.

Fill Slopes: Use tracking with tracked machinery up and down the slope to leave horizontal depressions in the soil. Each pass should move across the slope gradually leaving a continuous tracking pattern. Apply fertilizer, mulch, topsoil or other soil amendments as necessary prior to tracking. Do not blade or scrape the final slope face.

10. Sediment Bcsin Maintenance — This Contractor is responsible for continuous maintenance of ali sediment basins shown on these plans for the duration of this contract. Periodic sediment removal will be necessary to keep outlet pipes and overflow structures functioning properly. Immediately prior to acceptance of this work the Contractor shall perform a final cleaning of all basins. Sediments form basins shall be dried and placed in on—site embankments.

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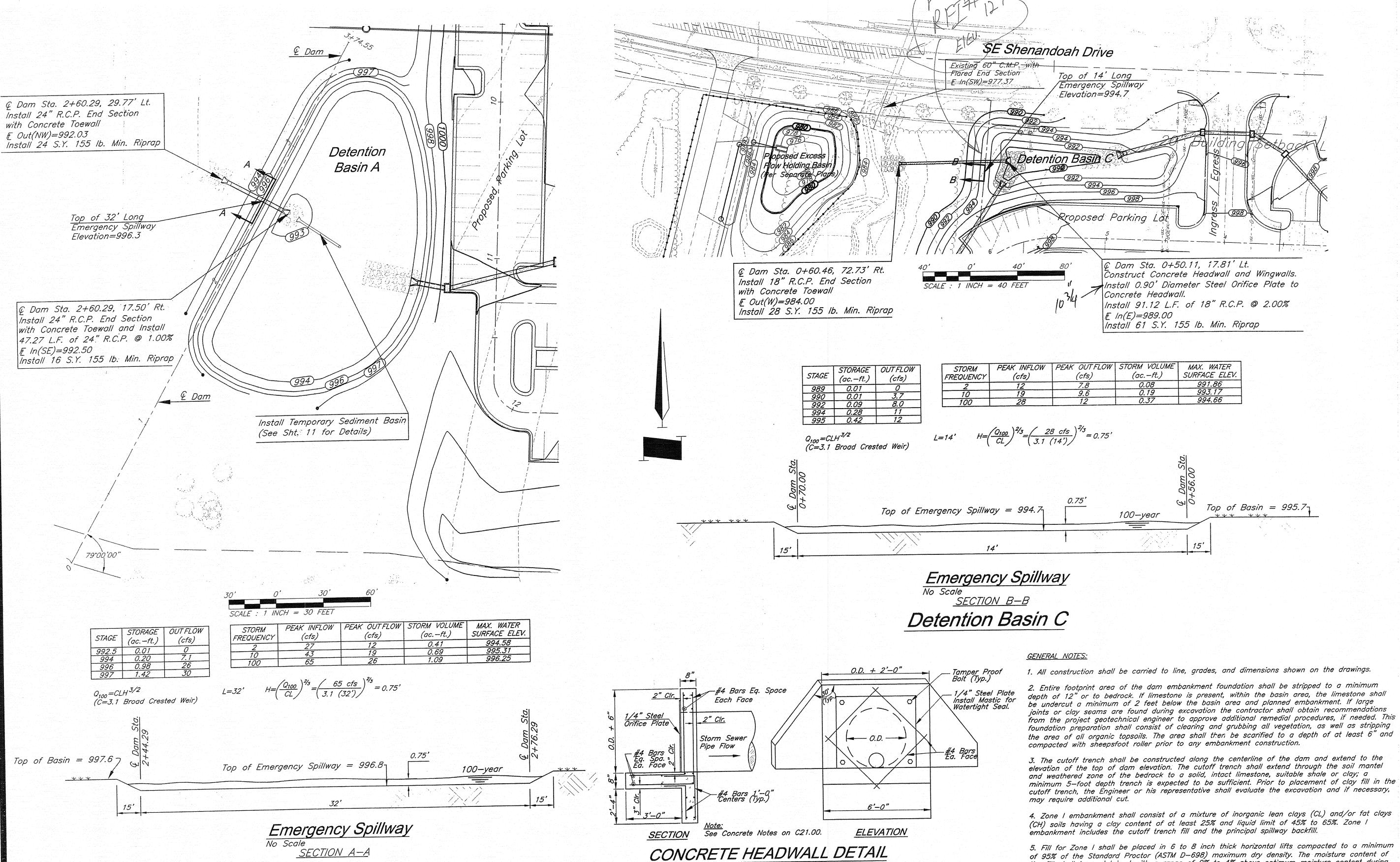
B.D.B. SHEET TITLE

Erosion Control Plan

SHEET NUMBER

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depth of 12" or to bedrock. If limestone is present, within the basin area, the limestone shall be undercut a minimum of 2 feet below the basin area and planned embankment. If large joints or clay seams are found during excavation the contractor shall obtain recommendations from the project geotechnical engineer to approve additional remedial procedures, if needed. This foundation preparation shall consist of clearing and grubbing all vegetation, as well as stripping

3. The cutoff trench shall be constructed along the centerline of the dam and extend to the elevation of the top of dam elevation. The cutoff trench shall extend through the soil mantel and weathered zone of the bedrock to a solid, intact limestone, suitable shale or clay; a minimum 5-foot depth trench is expected to be sufficient. Prior to placement of clay fill in the cutoff trench, the Engineer or his representative shall evaluate the excavation and if necessary,

5. Fill for Zone I shall be placed in 6 to 8 inch thick horizontal lifts compacted to a minimum of 95% of the Standard Proctor (ASTM D-698) maximum dry density. The moisture content of the fill shall be maintained with a range of 0% to 4% above optimum moisture content during placement and compaction. Compaction of each layer shall be conducted in a systematic and continuous manner so as to ensure the specified density. Rolling shall be done, parallel to the dam profile wherever possible. The embankment shall be brought up in layers such that the surface is essentially level except for a slight slope for drainage.

6. Field density test will be performed by the Contractor to determine the degree of compaction achieved and the suitability of the materials incorporated into the embankment. The Contractor shall obtain the services of a qualified geotechnical testing firm, pre-approved by the Engineer, to monitor and test the fills. The Contractor shall adjust the compactive effort, material, or method of placement to achieve satisfactory results.

7. No fill shall be placed in adverse weather conditions or as determined by the Geo-technical Engineer. Frozen materials or soils containing organic material shall not incorporated into the

8. The surfaces of the dam embankment shall be covered with no less than 6", nor more than 12" of topsoil measured vertically.

Top of Emergency Spillway Detention Basin — End Section End Section Existing Grade Outlet Pipe Zone 2 90% Std. Max. Density Compacted Clay Core 95% Std. Max. Density

Top of Dam = Top of Basin

Typical Detention Basin Dam Construction Detail

No Scale

Turf Reinforcement Mat

(See Details)

Detention Basin A

Peak

Runoff

2-Yr.

Peak

Runoff

(mins.)(cfs)(cfs)(cfs)PrePostPrePostPrePostPrePost

Time of

Concentration

15.6 6.0

93 | 9.0 | 6.0

Detention Basin calculations shown here are for the final constructed configuration of the storm water detention basins once sediment ponds

Drainage

Area

(ac.)

are removed at a later date.

Drainage

Area Name

Watershed A

Watershed C

Curve

Number

100-Yr.

Runoff

Detention Basin Details

BRADLEY D. BURTON

Fas

12 PBM 1015 FOR CONTROL BEET 1655 (1655)

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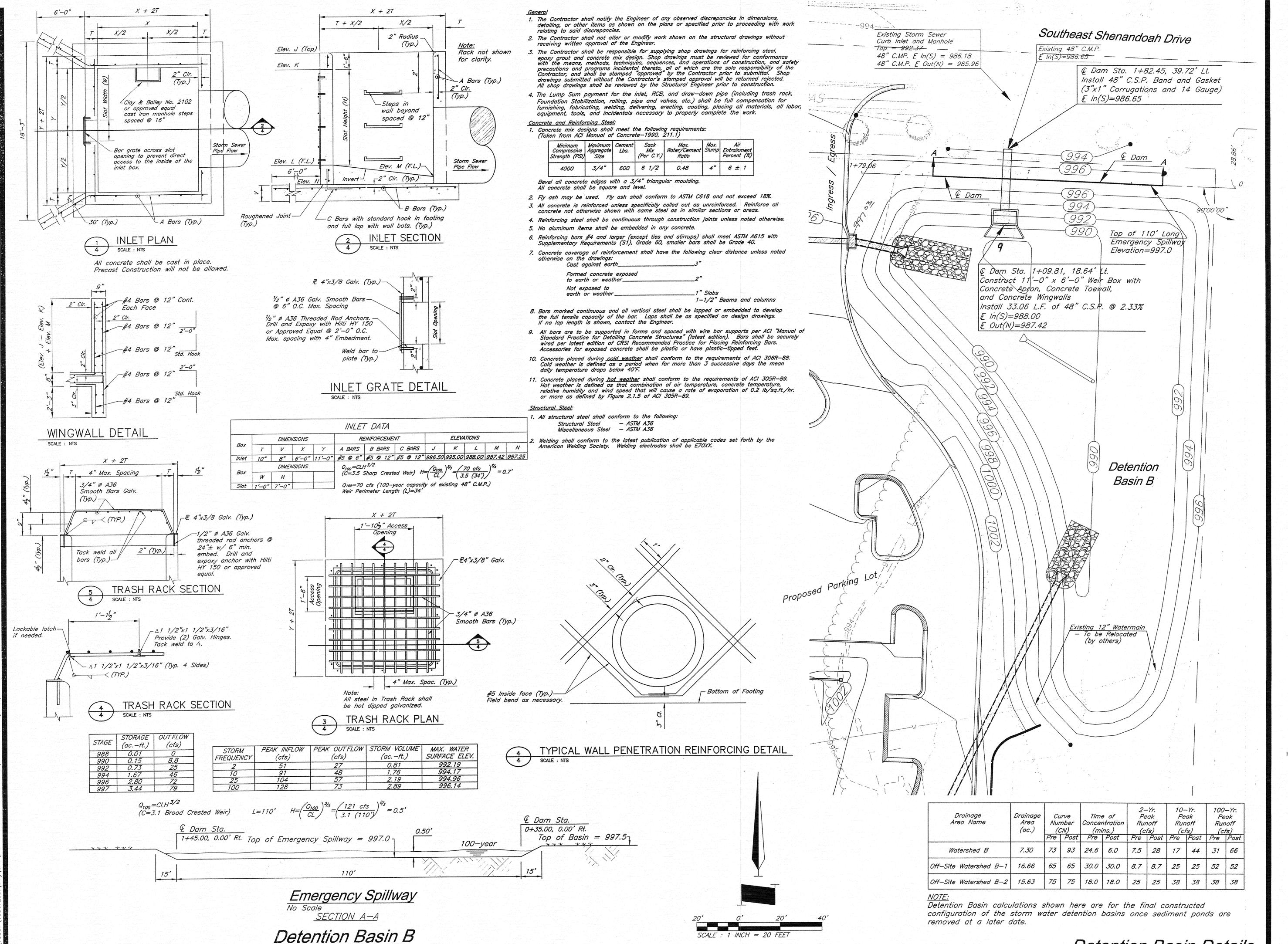
> A.G.L./B.D.B. SHEET TITLE

Detention Basin Details

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GEORGE BUTLER ASSOCIATER ASSOCIATE Engineers - Architects
Kansas - Missouri - Illinois
One Renner Ridge
9901 Renner Boulevard

SPITAL

EPLACEMENT HOSPITAL-2100 SE Blue Pa

Southeast (

BRADLEY D.

BURTON

NUMBER

E-25862

BRADLEY D.

BURTON

NUMBER

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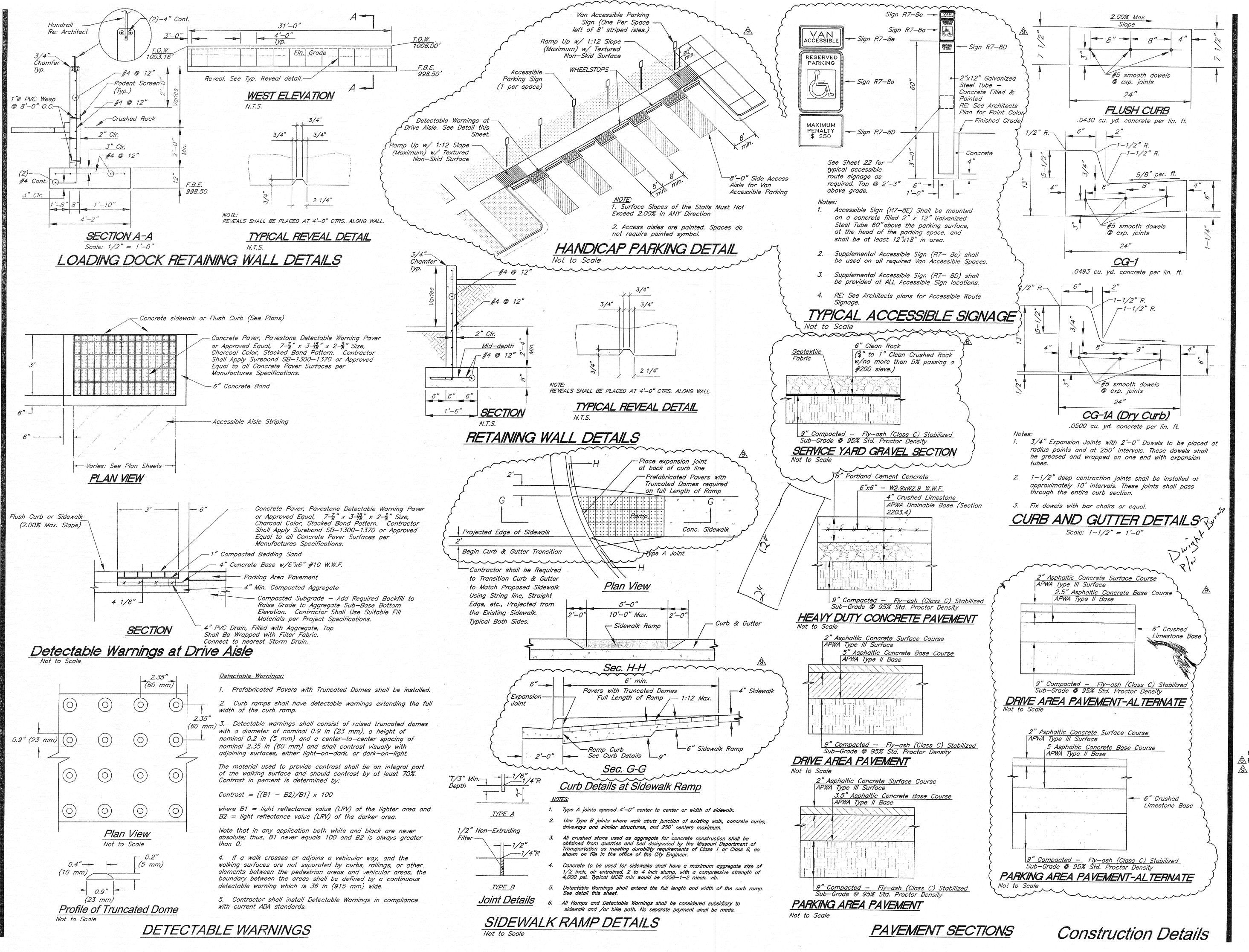
Detention Basin Details

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Detention Basin Details



Engineers - Architects
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PLACEMENT HOSPITAL-2100 SE Blue Parkway

BRADLEY D.

BURTON
NUMBER
E-25862

PROJECT NUMBER 10367.00 DATE

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Revised - RFI #0118 - 08/07/06

ASI #7 - 10/20/06

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B.I.B.
REVIEWED

DRAWN
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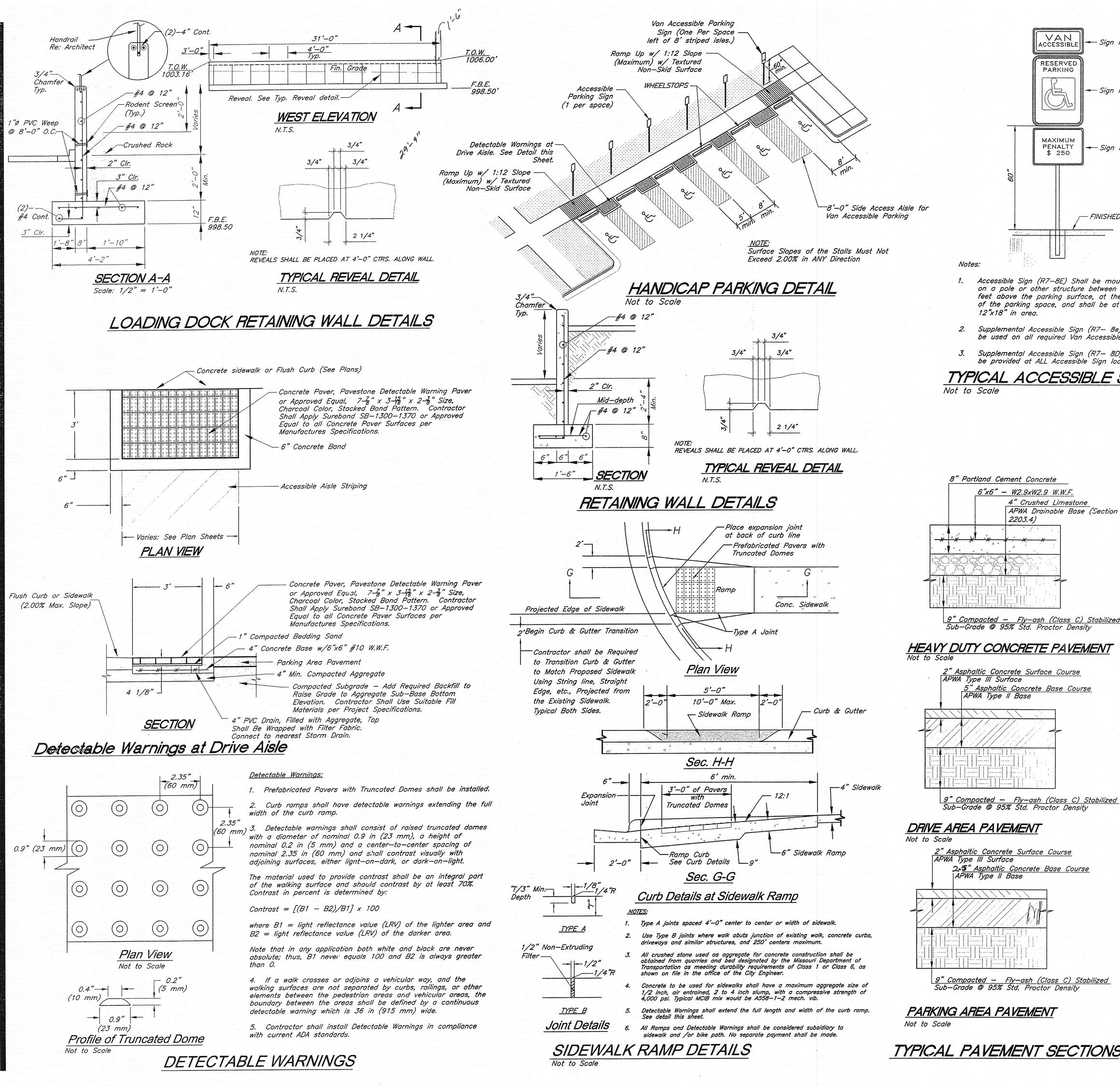
REVIEWED
B.D.B.

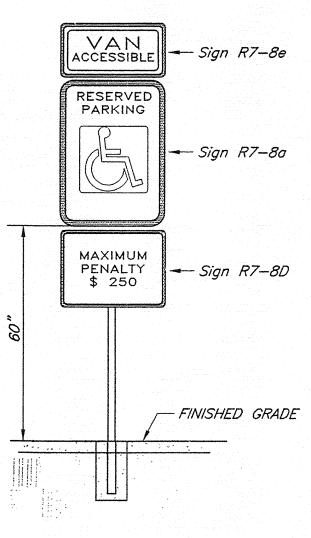
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Construction Details

SHEET NUMBER

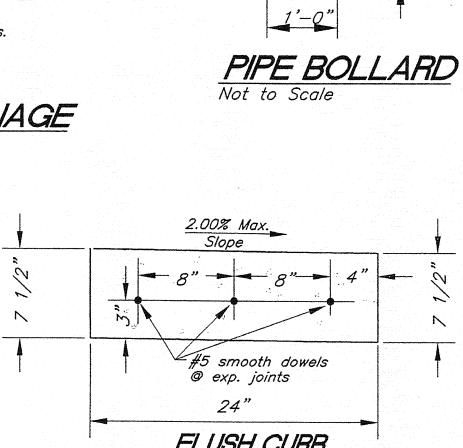
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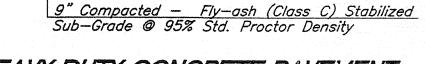




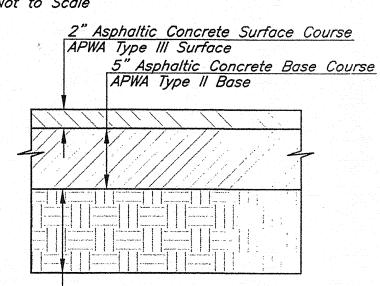
- Accessible Sign (R7-8E) Shall be mounted on a pole or other structure between 3 and 5 feet above the parking surface, at the head of the parking space, and shall be at least
- 2. Supplemental Accessible Sign (R7- 8e) shall be used on all required Van Accessible Spaces.
- 3. Supplemental Accessible Sign (R7- 8D) shall be provided at ALL Accessible Sign locations.

# TYPICAL ACCESSIBLE SIGNAGE

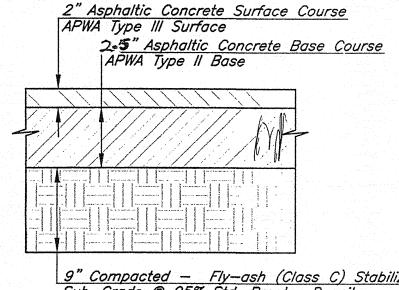




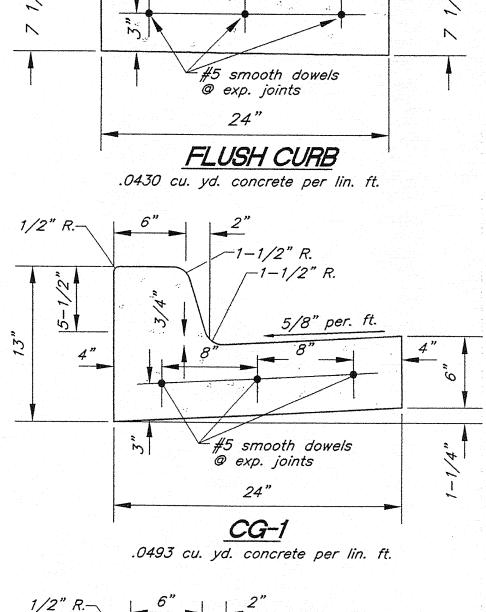
### HEAVY DUTY CONCRETE PAVEMENT

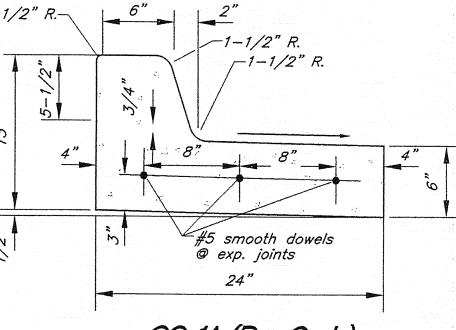


9" Compacted — Fly—ash (Class C) Stabilized Sub—Grade @ 95% Std. Proctor Density



TYPICAL PAVEMENT SECTIONS





CG-1A (Dry Curb) .0500 cu. yd. concrete per lin. ft.

3/4" Expansion Joints with 2'-0" Dowels to be placed at radius points and at 250' intervals. These dowels shall be greased and wrapped on one end with expansion

2. 1-1/2" deep contraction joints shall be installed at approximately 10' intervals. These joints shall pass through the entire curb section.

3. Fix dowels with bar chairs or equal.

CURB AND GUTTER DETAILS Scale: 1-1/2" = 1'-0"

Construction Details

-6" Dia. Schedule

Galvanized Pipe

-Proposed

Pavina

filled with Concrete.

80 Painted

0 

A. 000 BRADLEY D. BURTON NUMBER E-25862 PROJECT NUMBER

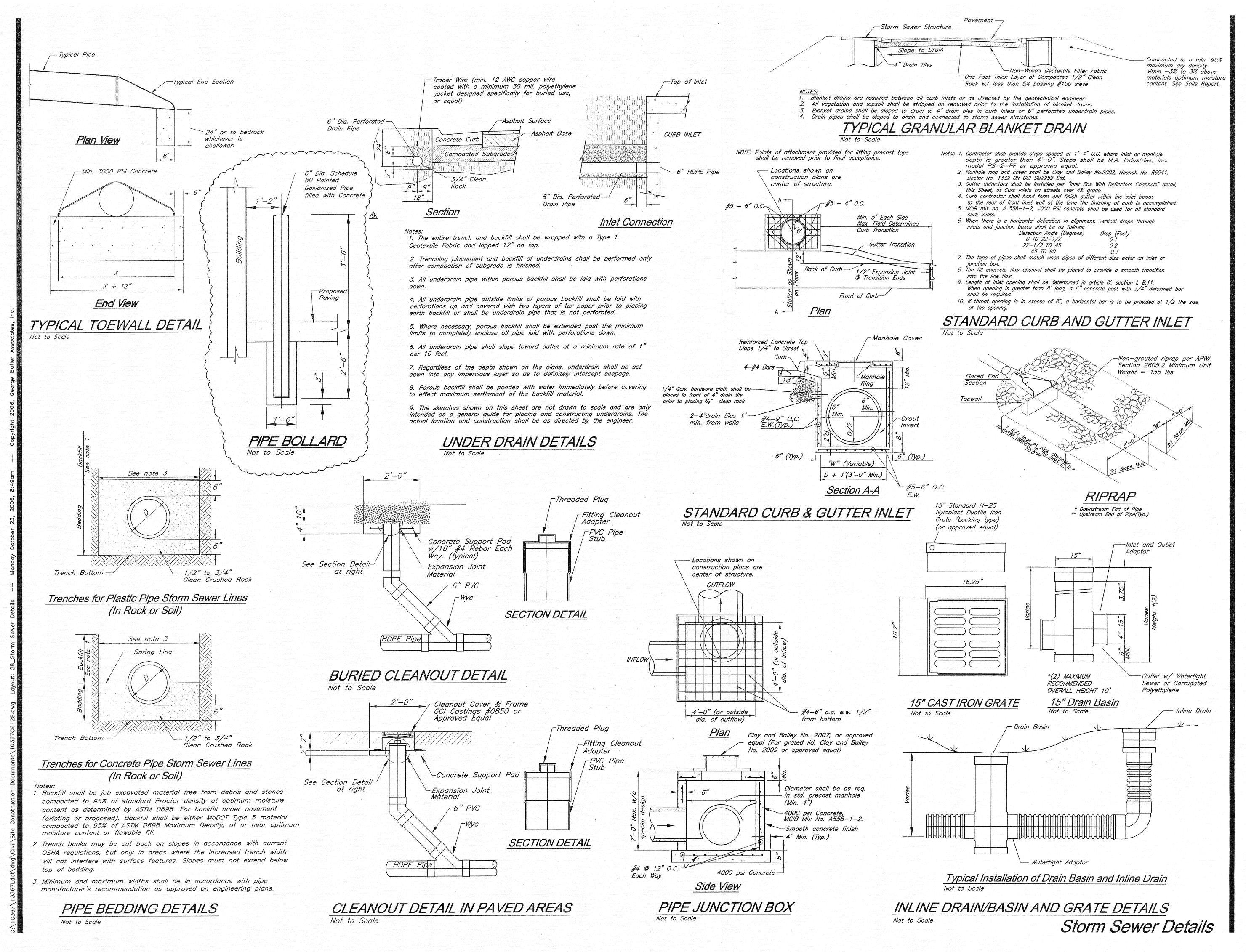
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CONSIDERON FIGURES TOF.

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Storm Sewer Details

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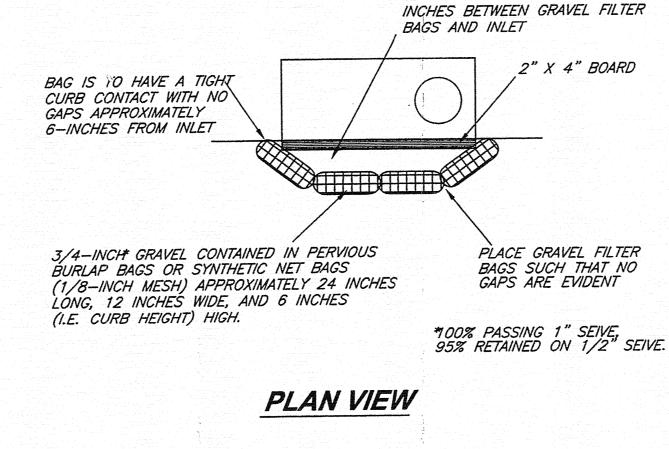
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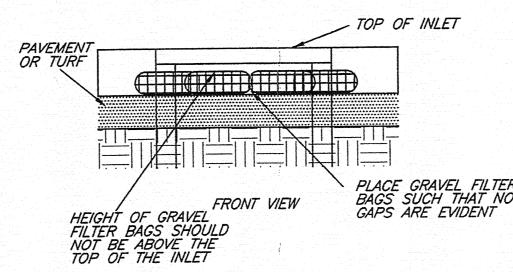
28 of 29

Storm Sewer Details

SEDIMENT FENCE INSTALLATION Not to Scale

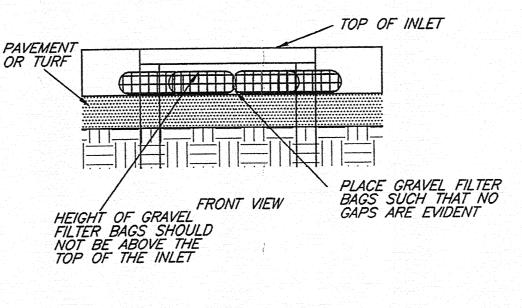
APPROXIMATE 4-INCH BY 4-INCH TRENCH



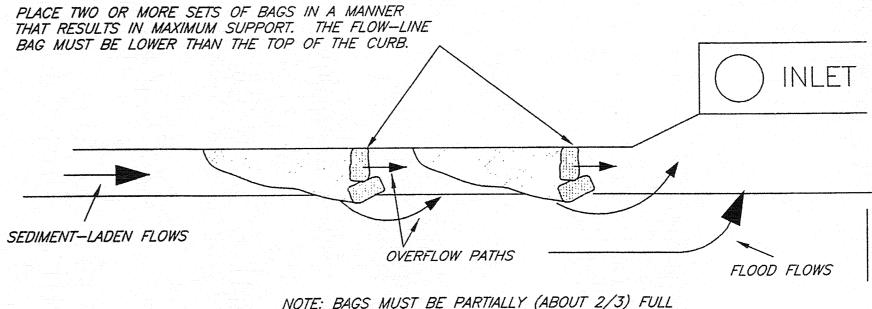


### FRONT VIEW

INLET PROTECTION IN A SUMP Not to Scale



LEAVE APPROXIMATELY 4 TO 6

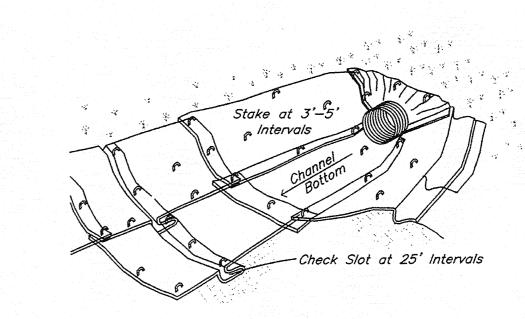


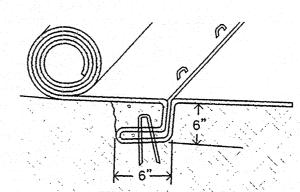
### PLAN VIEW

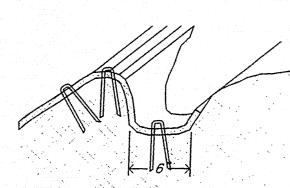
Grade	Spacing
(%)	(ft.)
 (/0/	(/1./
0.5	100
1.0	50
2.0	25
3.0	16
4.0	13
5.0	10

### SPACING OF ROCK BAGS

INLET PROTECTION ON A SLOPE Not to Scale

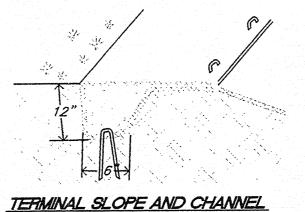


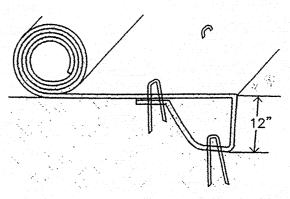




INTERMITTENT CHECK SLOT

LONGITUDINAL ANCHOR TRENCH





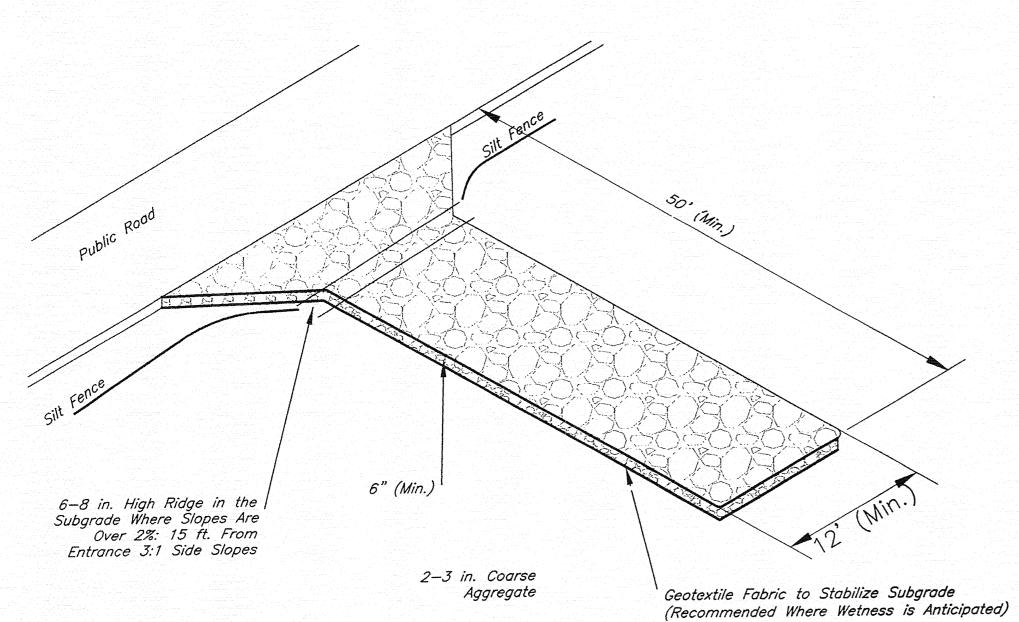
ANCHOR TRENCH

# INITIAL CHANNEL ANCHOR TRENCH

All drainage swale flowlines and sideslopes are to be lined with turf reinforcement mat. Contractor to install North American Green C350 (or approved equal) at swale bottoms and to a minimum of 3' above flowline grades. Remaining swale sideslopes are to be lined with North American Green DS150 (or approved equal) to the top of all sideslopes. All installation (check slots, staking, stapling, etc. shall conform to manufacturers specifications and recommendations.

EROSION BLANKETS AND TURF REINFORCEMENT MAT CHANNEL INSTALLATION Not to Scale

Erosion Control Details



1. Avoid locating on steep slopes or at curves on public roads. If possible, locate where permanent roads will eventually be Gravel Construction Entrance Notes:

2. Remove all vegetation and other unsuitable material from the subgrade area, grade and crown for positive drainage.

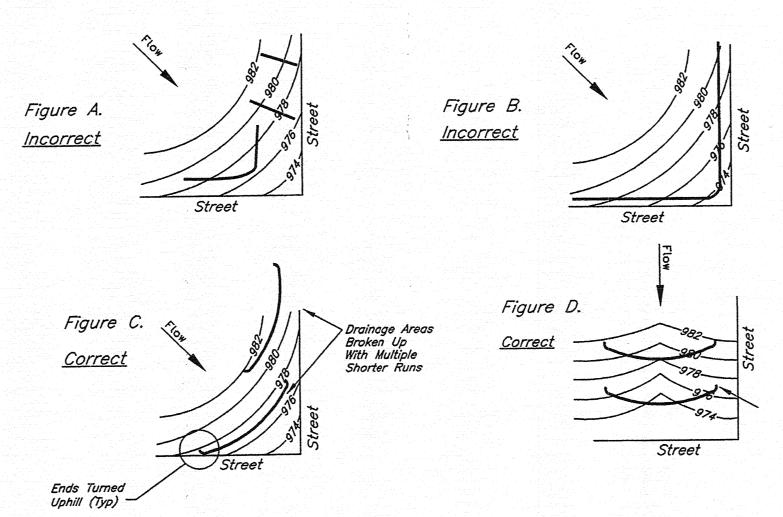
3. If slope towards the road exceeds 2%, construct a 6-8 inch high ridge with 3:1 side slopes across the approach approximately 15 feet from the entrance to divert runoff away from public road.

4. Aggregate size shall be 2- 3-inch wash stone

5. Pad Design thickness 6 inches minimum. Width - 12 feet minimum or full width roadway, whichever is greater. Length -

6. Washing Facility (Optional): Level area with minimum of 3 inches of washed stone.

GRAVEL CONSTRUCTION ENTRANCE



sediment fence must be turned uphill (Figures A & C).

2. Long perimeter runs of sediment fence must be limited to 200'. Runs should be broken up into several smaller segments to minimize water concentrations (Figures B & C).

3. Areas should be broken up with interior sediment

fence to minimize water concentrations and slow runoff velocities (Figure D).
4. Long slopes should have intermediate rows of sediments fence, not just at the top and toe of the slope (Figure D).

5. Install sediment fence at the top of the slope to slow velocity and volume of water and 6' to 10' away from the toe to create a sediment storage area (Figure E).

> SEDIMENT FENCE LAYOUT Not to Scale

Figure E.

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