

8787 RENNER BLVD. SUITE 100 LENEXA, KS 66219

913 + 498 - 1550 finklewilliams.com

December 5, 2024

### Paragon Star North Village - Multifamily Permit No: PL2024082 December 4, 2024 Final Development Plan Review Response

Drawings have been updated to conform with the comments from the plan review per the responses below.

### **Planning Review**

Reviewed By: Hector Soto Jr.

1. SCALE. The scale bar on Sheet C008 is labeled as 1"=40', but the sheet is actually drawn at 20' scale. Correct.

Response: Scale has been updated.

2. FIRE ACCESS ROAD. Will the fire access road be secured at both ends with a gate or similar device so that it can only be accessed by the Fire Department when needed, or will it be open at both ends at all times and thus be useable by the general public as a connection between the two parking lot areas? If the latter, then the fire access road shall be considered an extension of the parking lot and shall have a CG-1 boundary along both sides of the lane for the entire length of the connection between the two parking lot areas. If considered an extension of the parking lot, the 26' pavement width for the fire access road/drive aisle shall not include the width of the CG-1 boundary.

Response: The road will be secured with bollards and Knox Box per the A-101 floor plan sheet.

3. DRIVEWAY WIDTH. The two parking lot driveway connections to NW Paragon Pkwy are dimensioned as having widths of 27' BC/BC (east driveway) and 27.35' BC/BC (west driveway), resulting in the actual asphalt paving width (excluding curb and gutter) being less than 24' in width. City ordinance requires a minimum pavement width of 24' (28' BC/BC). Revise.

Response: Driveway width revised.

4. ADA PARKING SPACES. Every parking space required by this section shall be identified by a sign, mounted on a pole or other structure a minimum 60 inches (5 feet) above the ground measured from the bottom of the sign, at the head of the parking space.

Response: ADA parking signage is identified on Architectural Sheets A1.70G- A1.75G as well as detail 3/A1.76G attached.

5. LIGHT FIXTURES. No information for any building-mounted lights fixtures is provided on Sheets E100 or E300. Show the location of building-mounted fixtures on the building elevations to the extent possible and include the proposed building-mounted fixtures on the photometric plans. Building-mounted fixtures shall comply with the lighting standards under UDO Sections 8.220, 8.260, 8.270 and 8.280.

Response: No exterior wall mounted lighting is anticipated at this time.

6. PARKING GARAGE PARKING SPACES. 1) Dimension the proposed parking garage spaces. Spaces shall be a minimum 9' wide x 19' deep. 2) Dimension the drive aisle widths in the parking garage. Two-way drive aisles shall be a minimum 24' wide.

Response: This was discussed with staff early in design and based upon industry standards for Parking Garage Construction, the reduced depth of stalls was accepted. See attached email correspondence. Dimensions are shown in the attached garage set.

7. MECHANICAL EQUIPMENT SCREENING. Dash in the location of all proposed roof-top units on the building elevations to confirm compliance with the requirement that RTUs shall be fully screened from view on all sides by using parapet walls of a height at least equal to the height of the RTUs. Please take into account the added height from any curbs on which the RTUs will sit. If any ground-mounted equipment will be used, it shall be fully screened from view on all sides using evergreen shrubs or a masonry wall at least equal in height to the units being screened.

Response: There is a 5' parapet along Paragon Parkway. Sightlines from the public way to the roof top from anywhere surrounding the building obscure any rooftop equipment planned.

8. BUILDING ELEVATIONS. All of the text on the building elevations is unintelligible. All of the copy on the elevations are random symbols and numbers.

Response: The A-102 sheet has been replaced.

9. DATA TABLE. Include the impervious coverage information (in sq. ft. and as a percentage) in the table.

Response: Data table has been updated.

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10. TRASH ENCLOSURES. A detailed drawing of enclosure and screening methods to be used in connection with trash storage containers on the property shall be provided. Each trash enclosure shall be constructed of masonry walls or steel architecturally designed walls with either a solid steel opaque gate painted to be compatible with the color of the masonry or steel walls and building it is to serve or a steel framed semi-opaque gate with a screen mesh material approved by the Director that provides an appropriate visual barrier.

Response: See attached A6.07 for trash enclosure details used for Phase 1. The trash enclosure will be within the building footprint when Phase 2 is built out.

### **Engineer Review**

Reviewed By: Sue Pyles, P.E.

- 1. General:
  - Wall sections are shown for 2 of the walls, but not all. Do all walls utilize one of those sections? Please add a separate retaining wall layout and detail plan for clarity of locating all walls.
  - Please review the plan set and relocate elevations, etc. as necessary for clarity.
  - Please include an Engineer's Estimate of Probable Construction Costs.

Response: Wall sections are for areas specified on plan view. Retaining wall plan added. Retaining wall design to be provided as deferred submittal. Plan updated for clarity. Estimate provided.

### 2. Sheet C006:

- Include TW/BW elevations for the east wall in the middle, not just the ends.
- Highlight Enlargement #1 in Plan view in the same way as on the Utility Plan. That's very helpful.
- Include slopes and dimensions for the ramps in Enlargement #1.

Response: TW/BW added. Enlargement area added. Slopes and dimensions added for enlargement #1.

3. Sheets C008-C009: Are Grass Pavers being proposed? If not, please remove from Legend.

Response: No grass pavers proposed. Removed from legend.

- 4. Sheet C012:
  - Please relocate the water meters shown in Enlargement #1 & #4 to outside of paved areas.
  - Please revise to specify soft type "K" copper service line extending a minimum of 10' on each side of water meters.

• Please indicate irrigation and domestic meters for clarity.

Response: Water meter location shown in pavement due to lack of adequate green space. Copper lines specified 10' each side of meter. Meters indicated as domestic.

### 5. Sheet C017:

• Please revise the Class C Pipe Bedding Detail to show 12" of backfill over the pipe.

• The curb and gutter detail or pavement detail must show that the aggregate base and compaction of native subgrade extends a minimum of one (1) foot beyond the back of curb.

• The City now requires KCMMB asphalt. Please include in the asphalt pavement sections for clarity.

Response: Building detail revised. Curb detail revised. Asphalt pavement detail revised.

### Fire Review By: Jim Eden

2. Paved access and all hydrants shall be installed and operable prior to bringing combustible materials on site or going vertical with combustible construction.

Response: Acknowledged

3. IFC 507.1 - An approved water supply capable of supplying the required fire flow for fire protection shall be provided to premises upon which facilities, buildings or portions of buildings are hereafter constructed or moved into or within the jurisdiction.

Confirm available fire flow per Table B105.1(2). only a 50% reduction is allowed for having a sprinkler system.

Response: The available flow is 169 psi static and 2200 GPM at 84 psi residual which equals 2979 GPM at 20 psi. Table B105.1(2) requires 4,000 GPM @ 20 psi for 4 hours with a reduction of 50% to 2,000GPM meeting the required flow per Table B105.1(2)

4. IFC 903.3.7 - Fire department connections. The location of fire department connections shall be approved by the fire code official. Connections shall be a 4 inch Storz type fitting and located within 100 feet of a fire hydrant, or as approved by the code official.

The FDC(s) not shown for the north riser and parking garage standpipe.

Response: Proposed FDC locations are indicated on sheet A-101 and civil utility plan C013.

5. IFC 503.2.1 - Fire apparatus access roads shall have an unobstructed width of not less than 20 feet (6096 mm), exclusive of shoulders, except for approved security gates in accordance with Section 503.6, and an unobstructed vertical clearance of not less than 13 feet 6 inches (4115 mm)

What is the height of the lane under the parking garage?

Response: 13'-10" Min Clear

6. The access barrier to the fire lane shall be a drop down bollard or a swing gate with a Knox padlock.

Response: Acknowledged. Drop down bollards and Knox Box are indicated on the A-101 floor plan sheet.

Building Codes Review By: Joe Frogge

1. In this case, water piping must be copper for at least 10' on both sides of meters.

Response: Copper pipe noted 10' each side of meter

2. Provide water meter pit detail. Water meter vault detail on C016 is for backflow preventer.

Response: Water meter pit detail added to civil utility details.

3. Provide complete retaining wall design. Must be sealed/signed by engineer.

Response: Deferred submittal

4. Light pole base is incomplete. Refers to structural design. Provide complete detail.

Response: See sheet C018

Sincerely,

Sai Oa. W Ollini

Dave Williams

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### John Gaar

From:	Mike Weisenborn < Mike.Weisenborn@cityofls.net>
Sent:	Wednesday, September 22, 2021 1:46 PM
То:	John Gaar
Cc:	Ellen Foster; Kelsie Carnes; Scott Grause
Subject:	RE: Paragon Star

Good afternoon everyone. I have heard back from staff on everything know. Fire and Codes are in agreement with the first two items. Item 3 will take additional conversations and I can look at scheduling a Zoom meeting for that. On item 4, the DTLS apartments ran into the same issue with their parking garage. The reduced parking stalls were approved as part of the PDP due to the industry standards for these facilities. Staff is fine with allowing the reduction on this project based on these reasons.

Let me know if you would like me to schedule a meeting.

From: John Gaar <jgaar@finklewilliams.com>
Sent: Monday, September 20, 2021 8:48 AM
To: Mike Weisenborn <Mike.Weisenborn@cityofls.net>
Cc: Ellen Foster <efoster@finklewilliams.com>; Kelsie Carnes <kcarnes@finklewilliams.com>; Scott Grause
<sgrause@brinkmannconstructors.com>
Subject: RE: Paragon Star

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Mike,

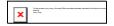
The file is a SketchUp file. You can download a viewer for free at <u>https://www.sketchup.com/products/sketchup-viewer</u>. Otherwise we will just share with you on screen when we get back together.

Thanks,

John

From: Mike Weisenborn <<u>Mike.Weisenborn@cityofls.net</u>>
Sent: Monday, September 20, 2021 8:24 AM
To: John Gaar <<u>jgaar@finklewilliams.com</u>>
Cc: Ellen Foster <<u>efoster@finklewilliams.com</u>>; Kelsie Carnes <<u>kcarnes@finklewilliams.com</u>>; Scott Grause
<<u>sgrause@brinkmannconstructors.com</u>>
Subject: RE: Paragon Star

Good morning. I am not able to open the Code Model. Can you let me know the format or send it in another? Thanks



Mike Weisenborn | Project Manager, Development Center 220 SE Green Street | Lee's Summit, MO 64063 816.969.1240 | <u>cityofLS.net</u> | <u>Mike.Weisenborn@cityofls.net</u>

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From: John Gaar <jgaar@finklewilliams.com>
Sent: Friday, September 17, 2021 11:23 AM
To: Mike Weisenborn <<u>Mike.Weisenborn@cityofls.net</u>>
Cc: Ellen Foster <<u>efoster@finklewilliams.com</u>>; Kelsie Carnes <<u>kcarnes@finklewilliams.com</u>>; Scott Grause
<<u>sgrause@brinkmannconstructors.com</u>>
Subject: Paragon Star

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Mike,

Attached is a record of our meeting last week along with more plan information hopefully communicating our approach to building life safety (specifically building separation requirements). Please review and let me know when next week you are available to meet. In the event you have access to a free SketchUp service, I've included the model for you to look around.

Thanks,

John

### JOHN L. GAAR, AIA

FINKLE + WILLIAMS Architecture



#### WE'VE MOVED! Please note our new address above.

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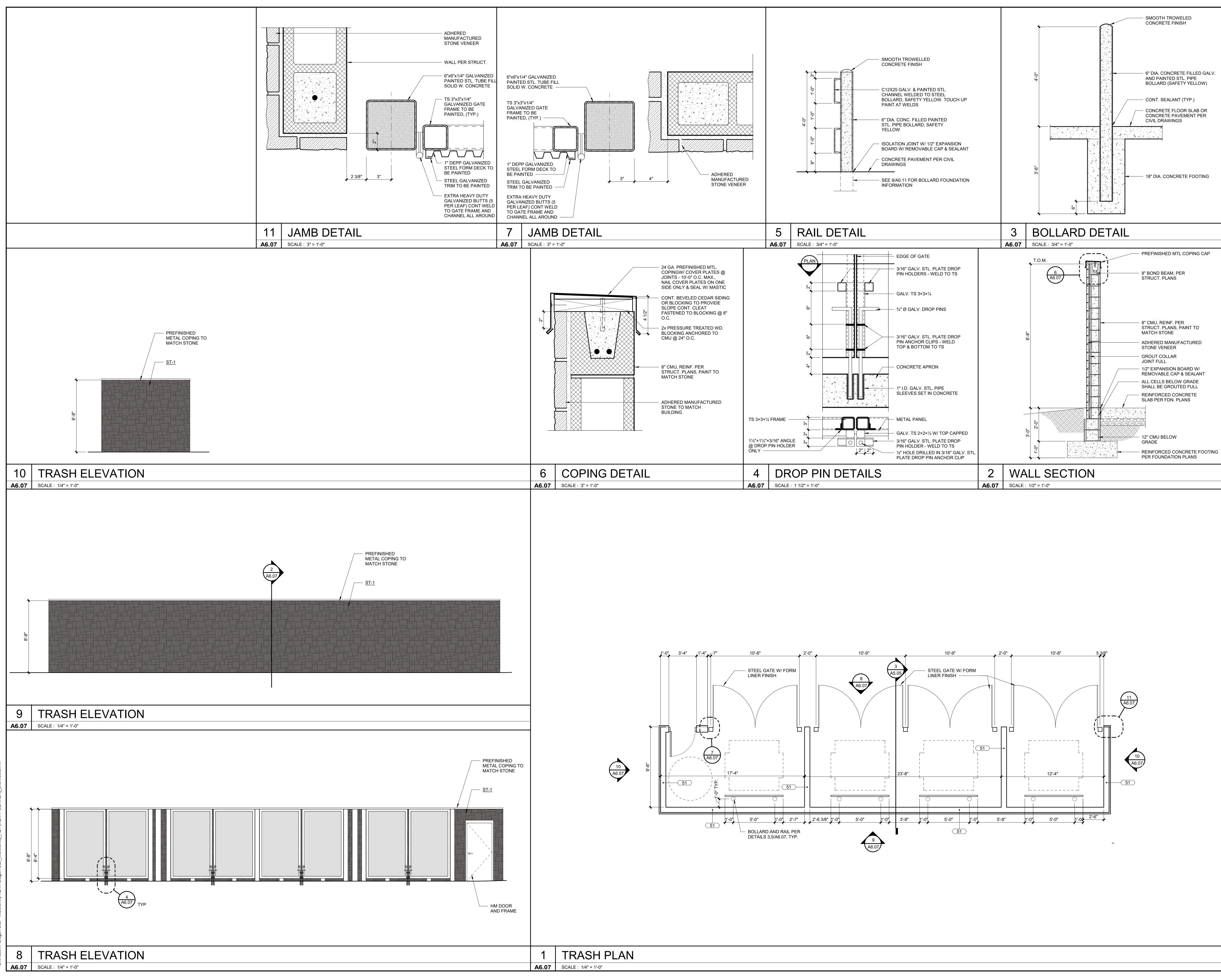
### Paragon Star North Village Site Work Engineer's Estimate

	12/10/202	24				
ltem No.	Description	Unit	Approx. Unit Quantity	Unit Price	Unit Total	
1	Heavy Duty Concrete Pavement Section (8")	SY	484	\$70.00	\$33,880.00	
2	Light Duty Asphalt Pavement Section (6")	SY	2,543	\$28.00	\$71,204.00	
3	Heavy Duty Asphalt Pavement Section (8")	SY	5,270	\$32.00	\$168,640.00	
4	Flyash modified subgrade	SY	8,297	\$7.50	\$62,227.50	
5	Brick Pavers/Crosswalk Pavers	SF LF	7,450	\$24.00	\$178,800.00	
6 7	Concrete Curb and Gutter (Type "CG-1") Concrete Sidewalk	SF	2,806 33,044	\$18.00 \$6.00	\$50,508.00 \$198,264.00	
7 8	ADA Ramp	EA	33,044	\$6.00 \$2,400.00	\$7,200.00	
9	No Parking Pavement Striping (4" white) (Trash Dumpster)	LF	242	\$5.00	\$1,210.00	
10	Traffic Signs	EA	3	\$1,500.00	\$4,500.00	
11	Storm Sewer (18") (HDPE)	LF	178	\$75.00	\$13,346.25	
12	Storm Sewer (4") (SDR 26 PVC)	LF	55	\$65.00	\$3,575.00	
13	Storm Sewer (6") (SDR 26 PVC)	LF	316	\$80.00	\$25,240.00	
14	Storm Sewer (8") (SDR 26 PVC)	LF	795	\$100.00	\$79,500.00	
15	Storm Sewer (10") (SDR 26 PVC)	LF	150	\$110.00	\$16,445.00	
16	Storm Sewer (12") (SDR 26 PVC)	LF	176	\$125.00	\$22,000.00	
17	Storm Sewer (4") Wye	EA	1	\$250.00	\$250.00	
18	Storm Sewer (6") Wye	EA	3	\$250.00	\$750.00	
19	Storm Sewer (8") Tee	EA	7	\$350.00	\$2,450.00	
20	Storm Sewer (10") Tee	EA	3	\$450.00	\$1,350.00	
21	Storm Sewer (12") Tee	EA	2	\$600.00	\$1,200.00	
22	Storm Sewer (12"x12"x8") Tee	EA	1	\$750.00	\$750.00	
23	Connect to 6' Dia. MH (Storm)	EA	2	\$7,000.00	\$14,000.00	
24	4' Grate inlet	EA	2	\$5,000.00	\$10,000.00	
25	4" HDPE PerforatedUnderdrain	LF	1,531	\$25.00	\$38,275.00	
26	Sanitary Sewer (4") (SDR 26 PVC)	LF	6	\$65.00	\$390.00	
27	Sanitary Sewer (6") (SDR 26 PVC)	LF	484	\$80.00	\$38,720.00	
28 29	Sanitary Sewer (6") 45° Bend Sanitary Sewer (6") 22-1/2° Bend	EA EA	5	\$250.00 \$250.00	\$1,250.00 \$500.00	
30 30	Sanitary Sewer (6') Wye	EA	2	\$250.00	\$500.00	
30 31	Sanitary Sewer (6') Vye Sanitary Sewer (6'') Cut in Tee	EA	2	\$1,500.00	\$3,000.00	
32	Sanitary Sewer (6") Clean out	EA	8	\$500.00	\$4,000.00	
33	Sanitary Sewer (6") Concrete Encasement	LF	50	\$200.00	\$10,000.00	
34	Erosion Control Wash-Out Station	EA	1	\$2,500.00	\$2,500.00	
35	Silt Fence	LF	2,800	\$5.00	\$14,000.00	
36	Curb Inlet Protection	EA	6	\$300.00	\$1,800.00	
37	Area Inlet Protection	EA	4	\$300.00	\$1,200.00	
38	Construction entrance and staging area	LS	1	\$2,500.00	\$2,500.00	
39	2" Dia. C901 HDPE	LF	663	\$15.00	\$9,945.00	
10	2" Type "K" Copper Pipe	LF	120	\$20.00	\$2,400.00	
11	6" Class 305 C900 PVC	LF	257	\$80.00	\$20,560.00	
42	8" Class 305 C900 PVC	LF	1,427	\$100.00	\$142,700.00	
43	Water (2") Water Meter	EA	8	\$3,500.00	\$28,000.00	
14	Water (8"x6"x8") Tee	EA	7	\$1,000.00	\$7,000.00	
	Water (8"x8"x8") Tee	EA	9	\$1,500.00	\$13,500.00	
16	Water (8"x2" Reducer)	EA	9	\$800.00	\$7,200.00	
47	Water (8") 45° Bend w/Thrust Block	EA	6	\$1,500.00	\$9,000.00	
18	Water (8") 22-1/2° Bend w/Thrust Block	EA	6	\$1,500.00	\$9,000.00	
19	Water (2" Gate Valve) Water (8" Gate Valve)	EA	5	\$1,000.00	\$5,000.00 \$17,500.00	
50 51	Water (8" Gate Valve) Water Fire Hydrant Assembly	EA EA	5	\$3,500.00 \$4,000.00	\$17,500.00 \$28,000.00	
57 52	Water Fire Hydrant Assembly Water Connect to 12" Water Main	EA	2	\$4,000.00 \$7,500.00	\$28,000.00 \$15,000.00	
52 53	Telecom (12"x12") Utility Quartzite Box	EA	2	\$1,500.00	\$1,500.00	OF MISS
53 54	Telecom Utility Pull Box	EA	4	\$1,500.00	\$6,000.00	OF MIG
55	Telecom (2") Dia. PVC Conduit	LF	530	\$15.00	\$7,950.00	OF MISS
56	Telecom (1") Dia. HDPE Conduit	LF	74	\$10.00	\$740.00	>
57	Telecom (3 - 5") Dia. HDPE	LF	817	\$20.00	\$16,340.00	souther the state of the state
58	Telecom (2 - 5") Dia. HDPE	LF	82	\$20.00	\$1,640.00	<b>FEATY</b>
59	Electrical (4") Dia. Schedule 40 PVC Conduit	LF	1,384	\$20.00	\$27,680.00	Jh La
50	Electrical (8 - 3") Dia. Schedule 40 PVC Conduit	LF	422	\$125.00	¢52 750 00 to	11151
61	Electrical Evergy 9'x9' Concrete Pad	EA	5	\$10,000.00	\$50,000.00	NMBER
62	Electrical 200Amp Sectionalizer Pad	EA	2	\$8,000.00	\$16,000.00 <b>P</b>	2018003126
63	Retaining Wall	SF	3,927	\$60.00	\$235,620.00	12/10/24
						NVMBER 2018003126 12/10/24 ONAL EN
	Total Cost				\$1,816,949.75	ALLESS

12/10/2024

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# PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Deri	4 N I -	18017 10050 07 10050 09	
Proj	Project No.: 18017,19050.07,19050.08		
Date	Date: 06.28.22		
Issued For: FOR CONSTRUCTION		FOR CONSTRUCTION	
REVISIONS			
No.	Date	Description	

REGISTRATION

PROJECT TEAM		
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE	
CIVIL	GBA ENGINEERS	
LANDSCAPE	LAND 3	
STRUCTURAL	BOB D. CAMPBELL	
PLUMBING	LATIMER SOMMERS	
MECHANICAL	LATIMER SOMMERS	
ELECTRICAL	LATIMER SOMMERS	
FIRE PROTECTION	LATIMER SOMMERS	
CONTRACTOR	BRINKMANN CONSTRUCTORS	





# DRAWING INDEX

ARCHITE	ECTURAL
A0.01G	LEGENDS & GEN. NOTES
A0.03	CODE
A1.70G	GARAGE FIRST FLOOR PLAN AT GRADE
A1.71G	GARAGE FIRST FLOOR PLAN
A1.72G	GARAGE SECOND FLOOR PLAN
A1.73G	GARAGE THIRD FLOOR PLAN
A1.74G	GARAGE FOURTH FLOOR PLAN
A1.75G	GARAGE FIFTH FLOOR PLAN
A1.76G	GARAGE SECTIONS
A1.77G	GARAGE STAIRS
A1.78G	GARAGE STAIRS AND SECTIONS
A1.79G	GARAGE DETAILS
A8.01G	DOOR SCHEDULE AND DETAILS
STRUCT	URAL
	URAL GENERAL NOTES
S0.01	
S0.01 S0.05	GENERAL NOTES
S0.01 S0.05	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE
S0.01 S0.05 S0.10	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN
S0.01 S0.05 S0.10 S1.71G	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN
S0.01 S0.05 S0.10 S1.71G S1.72G	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN
S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN
S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN
S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G S1.75G	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN
S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G S1.75G S1.76G	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN
S0.01 S0.05 S0.10 S1.71G S1.72G S1.72G S1.73G S1.74G S1.75G S1.76G S3.00	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN GARAGE FIFTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN TYPICAL FOUNDATION DETAILS
S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G S1.75G S1.76G S3.00 S3.01	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN GARAGE FIFTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN TYPICAL FOUNDATION DETAILS GARAGE FOUNDATION DETAILS
S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G S1.75G S1.76G S1.76G S3.00 S3.01 S3.02	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN GARAGE FIFTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN TYPICAL FOUNDATION DETAILS PILE & PODIUM FOUNDATION DETAILS

MECHAN	IICAL
ME-000	COVER
ME-001	SITE P
MECHAN	IICAL
MP1.71G	GARAG
MP1.72G	GARAC
MP1.73G	GARAC
MP1.74G	GARAC
MP1.75G	GARAC
MP2.01	MECH
ELECTRI	CAL
E1.71G	GARAC
E1.72G	GARAC

E1./1G	GARA
E1.72G	GARA
E1.73G	GARA
E1.74G	GARA
E1.75G	GARA
E3.01	ELECT
E3.02	ELECT

'ER SHEET E PLAN

AGE FIRST FLOOR- PLUMBING AGE SECOND FLOOR- PLUMBING AGE THIRD FLOOR- PLUMBING AGE FOURTH FLOOR- PLUMBING AGE FIFTH FLOOR- PLUMBING HANICAL DETAILS/SCHEDULES

AGE FIRST FLOOR PLAN- ELECTRICAL RAGE SECOND FLOOR PLAN- ELECTRICAL RAGE THIRD FLOOR PLAN- ELECTRICAL RAGE FOURTH FLOOR PLAN- ELECTRICAL RAGE FIFTH FLOOR PLAN- ELECTRICAL ECTRICAL DETAILS/ SCHEDULES ECTRICAL SCHEDULES

# **PROJECT** TEAM

ARCHITECT FINKLE + WILLIAMS ARCHITECTURE 8787 Renner Blvd., Suite 100 Lenexa, Kansas 66219 PH. 913.498.1550

**CIVIL** GBA 9801 Renner Blvd, Suite 300 Lenexa, Kansas 66219 PH. 913.492.0400

LANDSCAPE TBD

**STRUCTURAL** BOB D. CAMPBELL & CO., INC. 4338 Belleview Kansas City, Missouri 64111 PH. 816.531.4144 F. 816.531.8572

PRECAST CONTRACTOR CORESLAB STRUCTURES, INC. 759 S. 65th St. Kansas City, Kansas 66111 PH. 913.287-5725

**CONTRACTOR** BRINKMANN CONSTRUCTORS 11101 Switzer Rd., Suite 310 Overland Park, Kansas 66210 PH. 913.717.9007 F. 913.717.9407

**MECHANICAL** LS&A, P.A. 8625 College Blvd., Suite 102 Overland Park, Kansas 66210 PH. 785.233.0647 F. 785.233.0647

**PLUMBING** LS&A, P.A. 8625 College Blvd., Suite 102 Overland Park, Kansas 66210 PH. 785.233.0647 F. 785.233.0647

ELECTRICAL LS&A, P.A. 8625 College Blvd., Suite 102 Overland Park, Kansas 66210 PH. 785.233.0647 F. 785.233.0647

FIRE PROTECTION TBD







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PROJECT NUMBER 18017,19050.07,1905

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PERM				ENTS	DR	
		S AND SPACES				EXISTING CONSTRUCTION T
SIGNS W	HICH DESIGNATE PERM		CES SHALL COMPLY WITH			EXISTING CONSTRUCTION T
		4.30.5, 4.30.6			• XX	WALL TYPE DESIGNATION - REFERENCE FLOOR PLAN(S
ALARN SOUND I OPE	A WILL SPRINKLER	EXIT STAIR			X	ROOM NAME AND NUMBER - REFERENCE FLOOR PLAN(S REFERENCE FINISH SCHEDU
TYP	PE 'A' TYPE 'B'	TYPE 'C'			XX	DOOR AND FRAME DESIGNA REFERENCE FLOOR PLAN(S REFERENCE DOOR AND FRA
	RESERVE	<u>ה</u>				
18"	PARKING		EACCESSIBLE		X.	REFERENCE ELE REFERENCE SHE
~	VAN ACCESSIBL	PARKING ST REQUIREME REQUIREME	ALL SIGNAGE NTS, INCLUDING FINE NTS, WITH LOCAL		SECTION S	YMBOL
	\$250 Fine				X.	REFERENCE SHE
INTE		SNAGE SC	HEDULE GA	RAGE	X•	DETAIL / ENLARGED PLAN SYN
<b>DOOR N</b> 03.A	O. ROOM NAM	NE TYPE 'B'	SIGN TYPE		X.	
05.A 06.A 06.B	STAIR STAIR STAIR	TYPE 'A' TYPE 'C' TYPE 'C'				
06.A 06.A 06.A	STAIR STAIR STAIR	TYPE 'C' TYPE 'C' TYPE 'C'			INTERIOR E	ELEVATION SYMBOL
1. <u>TA(</u> a.	CTILE & BRAILE CHARAC CHARACTERS SHALL	<u>CTERS</u> . BE RAISED MINIMUM 1/3	2"		• X/X•	REFERENCE ELE REFERENCE SHE
b. 2. <u>TYI</u>	CHARATERS SHALL E PESTYLES	BE ACCOMPANIED BY GR	ADE 2 BRAILLE		FINISH DES	IGNATION SYMBOL
a. b.	CHARACTERS SHALL		SERIF OR SERIF TYPESTYLE IGH AND MAXIMUM 2" HIGH		XX-1 XX-1	<ul> <li>WALL FINISH DESIGNATION</li> <li>BASE FINISH DESIGNATION</li> <li>LIMITS OF WALL AND BASE F</li> </ul>
5. <u>Pic</u> a. b.	PICTOGRAMS SHALL PLACED DIRECTLY B	ELOW THE PICTORGRAM	HE EQUIVALENT VERBAL DESCR AS INDICATED. AM SHALL BE 6" MIN. IN HEIGHT	IPTION		REFERENCE FLOOR PLAN(S REFERENCE FINISH SCHEDU
4. <u>MA</u> a.		ACKGROUND SHALL BE	EGGSHELL, MATTE OR OTHER N GN MANUFACTURER.	ON-	(XX-#)	FLOOR FINISH DESIGNATION REFERENCE FLOOR PLAN(S REFERENCE FINISH SCHEDU
b. c.	BACKGROUND SHALI "GRIZZLE GREY"		LIC, COLOR TO MATCH SW 7068		×	REVISION NOTE
5. <u>MC</u> a. b.		<u>) HEIGHT</u> E FINISH FLOOR TO THE JACENT TO THE LATCH S			XX	CONSTRUCTION NOTE
C.	IF NO WALL SPACE E LEAF DOORS, MOUN		DE OF THE DOOR, INCLUDING DO	DUBLE	#	DEMOLITION NOTE
6. <u>FIR</u> a.			IN. 0.5" STROKE READING "SPRI 8, AS REQUIERD BY LOCAL FIRE		CJ——	GYPSUM BOARD CONTROL REFERENCE FLOOR PLAN(S REFERENCE DETAIL <b>1,2/A7.0</b>
	CTIONAL INFOR	RMATION DIRECTION TO OR INFOR			CMJ ———	CONCRETE MASONRY CONT REFERENCE FLOOR PLAN(S REFERENCE DETAIL 2/41 100
FUNCTIC		UILDING SHALL COMPLY				REFERENCE DETAIL 2/A1.10
		NICATION DEVIC	E SIGNAGE	FOR	FE FE	COMPANY, WWW.LARSENMI BRACKET, REFERENCE FLOG CENTERLINE OF EXTINGUIS
SUMMON IDENTIFI COMMUN	NING ASSISTANCE CIA T CATION OF THE LOCATION NICATION SYSTEM. EAC	HE TWO-WAY COMMUNIO ON SHALL BE POSTED AI H SIGN SHALL COMPLY V	CATION SYSTEM, INSTRUCTIONS CATION SYSTEM AND WRITTEN DJACENT TO EACH TWO-WAY /ITH ICC A117.1 FOR VISUAL CHA E PER DRAWING BELOW.		FEC	SEMI-RECESSED FIRE EXTIN COMPANY, WWW.LARSENMI ARCHITECTURAL SERIES, M (2 ½" PROTRUSION FROM W. RECESSED HANDLE, ENGRA "FIRE EXTINGUISHER" ON DO
						FIRE EXTINGUISHER AND MA BRACKET. MOUNT SO CENT

MBOLS LEGEND	GENERAL NOTES		
N TO REMAIN N TO BE DEMOLISHED	<ol> <li>ALL CONSTRUCTION SHALL CONFORM TO THE MINIMUM STANDARDS OF THE APPLICABLE CODE INDICATED IN THE BUILDING SUMMARY COLUMN AND ALL LOCAL CODES PRESENTLY IN EFFECT UNLESS MORE STRINGENT REQUIREMENTS ARE INDICATED.</li> <li>ALL NEW CONSTRUCTION SHALL COMPLY W/THE AMERICANS WITH DISABILITIES ACT</li> </ol>	THIS DISCLA DISCLAIMEF PROJECT N 18017,19050.	
	ACCESSIBILITY GUIDELINES (ADAAG) AND CHAPTER 11 OF THE INTERNATIONAL BUILDING CODE (INCLUDES ICC A117.1 PER IBC)	THE UNDER PREPARATI	
N-N(S) FOR LOCATIONS.	3. THE GENERAL CONTRACTOR AND SUBCONTRACTORS SHALL OBTAIN AND PAY FOR ALL REQUIRED PERMITS, LICENSES, AND ALL UTILITY CHARGES, AND ARRANGE FOR ALL REQUIRED INSPECTIONS.	<u>NO.</u>	
ER - N(S) FOR LOCATIONS. EDULE FOR FINISHES.	4. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING BUILDING & SITE UTILITIES BETWEEN CIVIL & MEP DRAWINGS. THE CONTRACTOR SHALL ALSO CONTACT ALL APPLICABLE UTILITY COMPANIES & PROVIDE CONDUIT & OTHER FACILITIES AS REQUIRED.	A0.01G A0.03 A1.70G	
SNATION - N(S) FOR LOCATIONS. FRAME SCHEDULE FOR REQUIREMENTS.	5. THE GENERAL CONTRACTOR AND ALL SUBCONTRACTORS SHALL VERIFY ALL DIMENSIONS & CONDITIONS ON THE JOB SITE PRIOR TO THE BIDDING OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES.	A1.71G A1.72G A1.73G	
	IN CASES OF DISCREPANCY CONCERNING DIMENSIONS, QUANTITIES AND LOCATION, THE CONTRACTOR SHALL, IN WRITING, CALL TO THE ATTENTION OF THE ARCHITECT ANY DISCREPANCIES BETWEEN SPECIFICATIONS, PLANS, DETAILS OR SCHEDULES. THE ARCHITECT WILL THEN INFORM THE CONTRACTOR, IN WRITING, WHICH DOCUMENT TAKES PRECEDENCE. THERE SHALL BE NO ADJUSTMENT TO THE COST OR TIME OF THE WORK RESULTING FROM CLARIFICATION OF SUCH DISCREPANCIES.	A1.74G A1.75G A1.76G A1.77G A1.78G	
ELEVATION SHEET	<ul> <li>6. DIMENSIONS ON DRAWINGS ARE SHOWN TO FINISHED FACE OF WALLS AND PARTITIONS OF EXISTING OR NEW CONSTRUCTION UNLESS OTHERWISE NOTED. CEILING HEIGHT DIMENSIONS AND ALL OTHER VERTICAL DIMENSIONS ARE TO THE FINISHED FLOOR SURFACE UNLESS OTHERWISE NOTED.</li> </ul>	A1.79G A8.01G	
SECTION SHEET SYMBOL	7. CONTRACTOR TO FOLLOW ALL PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS FOR ALL BULIDING PRODUCTS. IN THE EVENT OF A CONFLICT BETWEEN INFORMATION SHOWN ON THE CONTRACT DOCUMENTS AND PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS, PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS SHALL GOVERN. CONTRACTOR SHALL NOTIFY ARCHITECT OF AN CONFLICTS BETWEEN PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS AND THE CONTRACT DOCUMENTS PRIOR TO INSTALLATION.	THE UN ALL OTH ESTIMA PART O GEOTEC THIS NC	
DETAIL SHEET	8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING SHOP DRAWINGS, PRODUCT DATA, OR SAMPLES FOR CASEWORK, FINISHES, DOORS, FRAMES, HARDWARE, MECHANICAL, ELECTRICAL, AND PLUMBING FIXTURES, AND OTHER ITEMS REQUIRING ARCHITECT'S REVIEW FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS, AND FOR ALL ITEMS WHICH ALLOWED CONTRACTOR OPTIONS. PRIOR TO FORWARDING TO THE ARCHITECT FOR REVIEW. THESE SUBMITTALS MUST BE REVIEWED BY THE CONTRACTOR FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATIONS OF CONSTRUCTION AND SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO, ALL OF WHICH ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL AFFIX A STAMP TO SUBMITTAL INDICATING HIS REVIEW. SUBMITTALS FORWARDED WITHOUT A STAMP WILL BE RETURNED. ALL SUBMITTALS MUST BE REVIEWED BY THE ARCHITECT PRIOR TO CONSTRUCTION.	ARCHIT	
ELEVATION	9. CONTRACTOR SHALL GUARANTEE ALL WORK AGAINST FAULT OF ANY MATERIAL OR WORKMANSHIP FOR A PERIOD OF NOT LESS THAN ONE YEAR AFTER COMPLETION OR ACCEPTANCE. FAULTY WORK SHALL BE REPLACED OR REPAIRED AS REQUIRED AT NO COST TO THE OWNER.	ARCHIT	
	10. CONTRACTOR SHALL COORDINATE WITH OWNER ALL ITEMS TO BE SALVAGED PRIOR TO SUBMISSION OF BIDS AND START OF CONSTRUCTION. OWNER SHALL HAVE SALVAGE RIGHTS TO RETAIN ALL REMOVED ITEMS.		
DN DN SE FINISHES	11. ALL CHANGES PROPOSED DURING CONSTRUCTION WHICH RESULT IN A CHANGE TO THE CONTRACT TIME AND/OR SUM SHALL BE SUBMITTED TO THE ARCHITECT IN WRITING AND APPROVED BY THE ARCHITECT AND OWNER BEFORE SUCH WORK SHALL COMMENCE.		
N(S) FOR LOCATIONS. EDULE FOR DESCRIPTIONS.	12. CONTRACTOR SHALL COORDINATE CLEAR OPENINGS FOR ALL APPLIANCES PRIOR TO CONSTRUCTION OF CASEWORK.		
ION - N(S) FOR LOCATIONS. EDULE FOR DESCRIPTIONS.	13. CONTRACTOR SHALL FURNISH AND INSTALL CONCEALED FIRE-RETARDANT TREATED WOOD BLOCKING BEHIND ALL CABINETS, TOILET ACCESSORIES, PLUMBING FIXTURES, AND OTHER WALL MOUNTED ITEMS AS REQUIRED FOR ADEQUATE SUPPORT.		
	14. CONTRACTOR SHALL COORDINATE ALL LOCK AND LATCH SETS AND FINAL KEYING WITH OWNER. DOUBLE KEYED LOCKS ARE NOT PERMITTED ON ANY REQUIRED OR MARKED EXIT. MATCH EXISTING KEYING SYSTEM IF ONE IS EXISTING.		
	15. ALL DOOR HARDWARE ON EXIT DOORS SHALL BE READILY OPERABLE FROM THE EGRESS SIDE WITHOUT THE USE OF A KEY, SPECIAL KNOWLEDGE, OR EFFORT.		
DL JOINT -	16. CONTRACTOR SHALL PREPARE ALL NEW AND EXISTING SURFACES SCHEDULED TO RECEIVE NEW FINISHES IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR THE SUBSTRATE & FINISH BEING APPLIED.		
N(S) FOR LOCATIONS. 17.01 FOR CONSTRUCTION REQUIREMENTS.	17. CONTRACTOR SHALL COORDINATE FINAL QUANTITY AND LOCATIONS OF FIRE EXTINGUISHERS WITH THE FIRE DEPARTMENT AND/OR BUILDING DEPARTMENT. SEE SYMBOLS LEGEND FOR TYPE OF EXTINGUISHER.		
ONTROL JOINT - N(S) FOR LOCATIONS. . <b>10E</b> FOR CONSTRUCTION REQUIREMENTS.	18. ALL CONSTRUCTION MATERIALS EXPOSED WITHIN PLENUMS SHALL BE NON-COMBUSTIBLE OR SHALL HAVE A MAXIMUM FLAME SPREAD RATING OF 25 AND MAXIMUM SMOKE DEVELOPED RATING OF 50.		
TINGUISHER BY LARSEN'S MANUFACTURING NMFG.COM, MODEL MP10 W/B2 MOUNTING LOOR PLAN(S) FOR LOCATIONS. MOUNT SO	<ol> <li>ALL PIPING, LOW VOLTAGE WIRE AND CABLE, OPTICAL FIBER, PNEUMATIC TUBING, AND ALL DUCT AND DUCT COVERINGS, LININGS AND CONNECTORS INSTALLED WITHIN PLENUMS MUST BE RATED FOR PLENUM USE.</li> </ol>		
TINGUISHER BY LARSEN'S MANUFACTURING	20. TENANT SHALL BE RESPONSIBLE FOR COORDINATION AND INSTALLATION OF VOICE AND DATA CABLING AND EQUIPMENT.		
MFG.COM OR APPROVED EQUAL: , MODEL # AL-2409-6R. ALUMINUM, SEMI-RECESSED WALL WITH ROLLED EDGES), SOLID DOOR WITH	21. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE AUTOMATIC SPRINKLER SYSTEM. THE DESIGN SHALL BE PER NFPA REQUIREMENTS.		
RAVED VERTICAL LETTERS WITH NO BACKFILL DOOR. CABINET TO BE PROVIDED WITH MP10 MANUFACTURER'S STANDARD MOUNTING	22. ALL NEW GLASS AND GLAZING LOCATED IN HAZARDOUS LOCATIONS AS DEFINED IN IBC SECTION 2406.3 SHALL MEET THE REQUIREMENTS FOR SAFETY GLAZING AS DEFINED IN IBC SECTION 2406.		
INTERLINE OF CABINET HANDLE IS 46" A.F.F.	23. IF THE CONTRACTOR FAILS TO SUBMIT A MATERIAL FOR APPROVAL, THE MATERIAL MAY BE REQUIRED TO BE REMOVED BY THE CONTRACTOR EITHER BY DIRECTION OF THE OWNER OR ARCHITECT.		
	24. ALL HIGH-PILED STORAGE SHALL COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE APPLICABLE EDITION OF THE INTERNATIONAL FIRE CODE.		
	25. THE CONTRACTOR IS TO PROVIDE AS BUILT DRAWINGS IN HARD COPY & AN ELECTRONIC AUTOCAD FILE TO THE OWNER AT THE CONCLUSION OF THE PROJECT.		
	26. INSTALL ELASTOMERIC JOINT SEALER AROUND ALL PIPES, DUCTWORK, & STRUCTURE PASSING THRU INTERIOR NON-RATED CONCRETE AND MASONRY WALLS, GYPSUM BOARD PARTITIONS, AND CONCRETE FLOOR/ROOF SLABS. FOR FIRE RATED INTERIOR CONCRETE AND MASONRY WALLS, GYPSUM BOARD PARTITIONS, AND CONCRETE FLOOR/ROOF SLABS SEAL ALL PIPES, DUCTWORK, AND STRUCTURE. INSTALL FIRESTOP MATERIALS IN ALL GAPS PRIOR TO SEALANT APPLICATION. INSTALL SEALER ACCORDING TO MANUFACTURER'S WRITTEN INSTRUCTIONS.		
	27. CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTION OF ALL EXISTING CONSTRUCTION INDICATED TO REMAIN AND SHALL REPAIR AND/OR REPLACE ALL AREAS AND /OR MATERIAL DAMAGED DURING CONSTRUCTION AT A MINIMUM TO THE CONDITION WHICH EXISTED PRIOR TO CONSTRUCTION.		
	28. CONTRACTOR SHALL BE RESPONSIBLE FOR PRICING RADIO COVERAGE AMPLIFIER FOR EMERGENCY RESPONDERS AS AN ALTERNATE. PRIOR TO CONSTRUCTION COMPLETION, AMPLIFIER SHALL BE PROVIDED ONLY IF REQUIRED BY AHJ .		

# FESSIONAL SERVICES DISCLAIMER

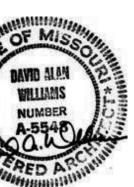
SCLAMER SERVES NOTICE OF ACCEPTANCE OF RESPONSIBILITY AND MER OF RESPONSIBILITY AS TO THE CONTRACT DOCUMENTS PREPARED FOR NUMBER:

050.07,19050.08, PARAGON STAR NORTH VILLAGE BY FINKLE + WILLIAMS, INC. DERSIGNED ARCHITECT, AND FINKLE + WILLIAMS, INC., ARE RESPONSIBLE FOR RATION OF ONLY THE NOTED CONSTRUCTION DRAWINGS BELOW:

TITLE	DATE
LEGENDS & GEN. NOTES	10.18.24
CODE	10.18.24
GARAGE FIRST FLOOR PLAN AT GRADE	10.18.24
GARAGE FIRST FLOOR PLAN	10.18.24
GARAGE SECOND FLOOR PLAN	10.18.24
GARAGE THIRD FLOOR PLAN	10.18.24
GARAGE FOURTH FLOOR PLAN	10.18.24
GARAGE FIFTH FLOOR PLAN	10.18.24
GARAGE SECTIONS	10.18.24
GARAGE STAIRS	10.18.24
GARAGE STAIRS AND SECTIONS	10.18.24
GARAGE DETAILS	10.18.24
DOOR SCHEDULE AND DETAILS	10.18.24

UNDERSIGNED ARCHITECT AND FINKLE + WILLIAMS DISCLAIM RESPONSIBILITY FOR OTHER CONSTRUCTION DOCUMENTS, AND ANY OTHER SPECIFICATIONS, REPORTS, MATES, SHOP DRAWINGS, ETC. RELATING TO OR INTENDED TO BE USED FOR ANY RT OF THE ARCHITECTURAL OR ENGINEERING PROJECT, INCLUDING ANY TECHNICAL ENGINEERING SERVICES, OR ENVIRONMENTAL REPORTS.

S NOTICE IS EXECUTED BY THE UNDERSIGNED AND AUTHENTICATED BY THE CHITECTURAL SEAL OF THE PERSON PREPARING THS NOTICE.



HITECT: DAVID A. WILLIAMS

# **BUILDING SUMMARY**

GENERAL BUILDING INFORMATION

PROJECT NAME: PARAGON STAR NORTH VILLAGE ADDRESS: 3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

PROPOSED USE: MIXED USE MULTI-FAMILY RESIDENTIAL AND RETAIL

## APPLICABLE CODES

INTERNATIONAL BUILDING CODE (IBC)	2018 EDITION
INTERNATIONAL MECHANICAL CODE (IMC)	2018 EDITION
INTERNATIONAL PLUMBING CODE (IPC)	2018 EDITION
NATIONAL ELECTRIC CODE (NEC)	2017 EDITION
INTERNATIONAL FIRE CODE (IFC)	2018 EDITION
INTERNATIONAL FUEL GAS CODE (IFGC)	2018 EDITION

# **GENERAL BUILDING LIMITATIONS**

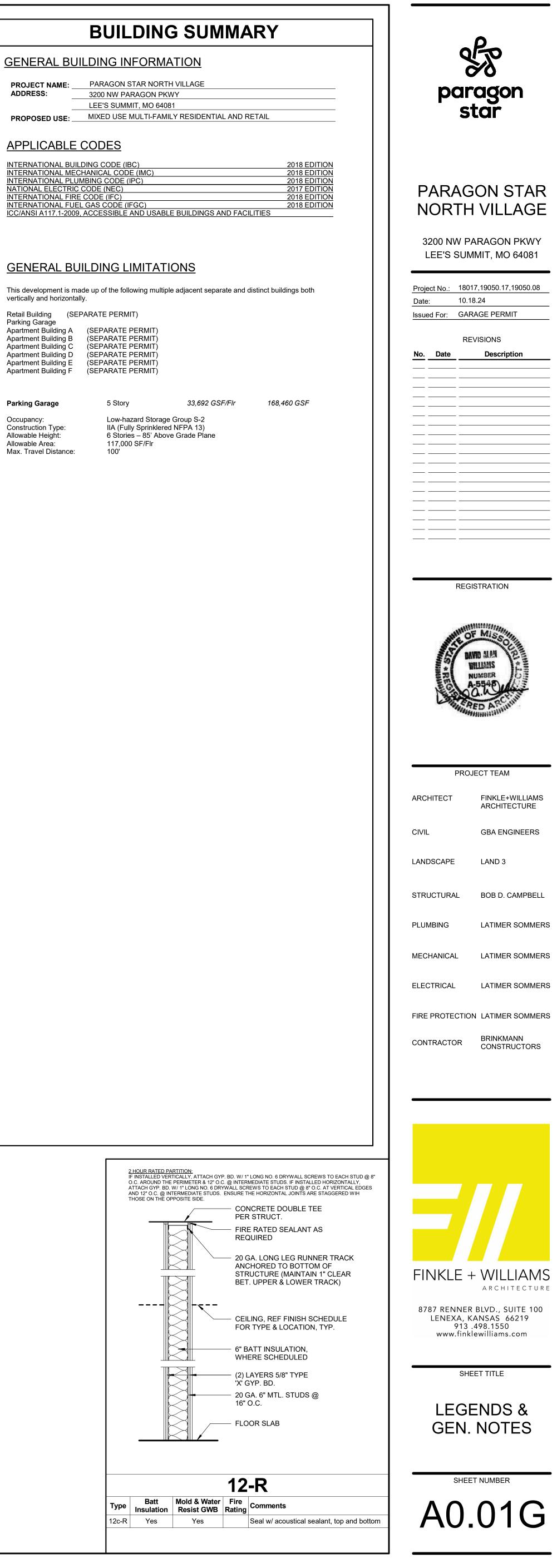
This development is made up of the following multiple adjacent separate and distinct buildings both vertically and horizontally.

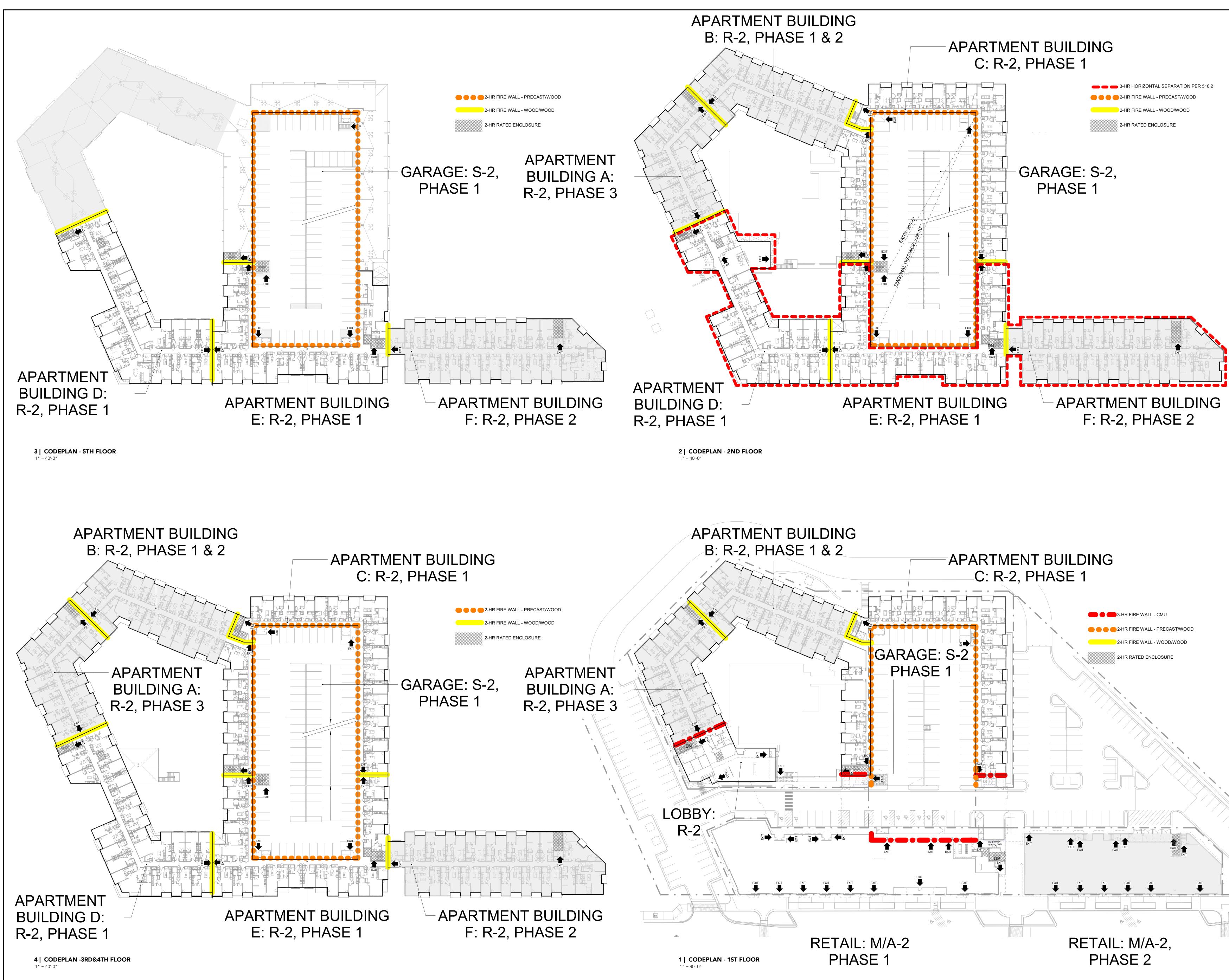
Retail Building Parking Garage	(SEPA	ARATE PER
Apartment Building	А	(SEPARAT
Apartment Building		<b>(SEPARAT</b>
Apartment Building		<b>(SEPARAT</b>
Apartment Building	D	(SEPARAT
Apartment Building	Е	<b>(SEPARAT</b>
A	-	JOEDADAT

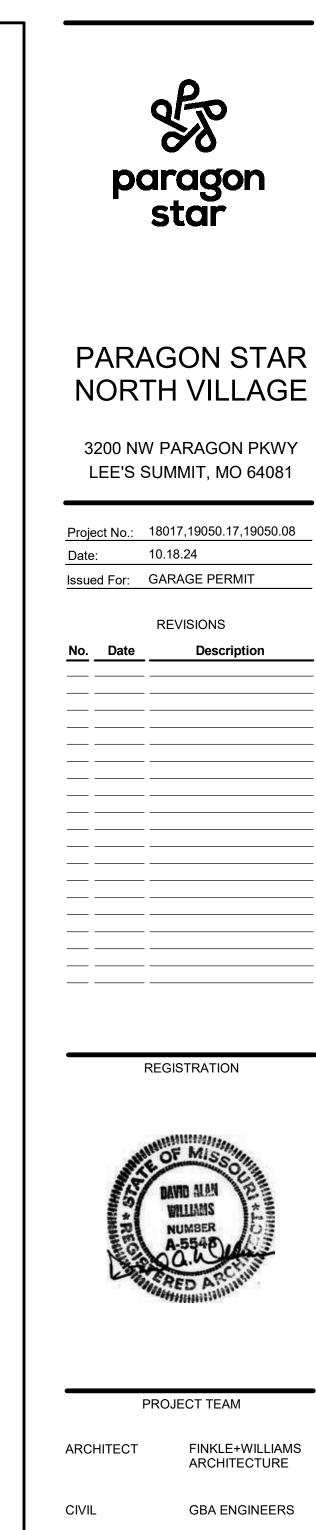
(SEPARATE PERMIT) Apartment Building F

Occupancy: Construction Type: Allowable Height: Allowable Area: Max. Travel Distance:

Parking Garage







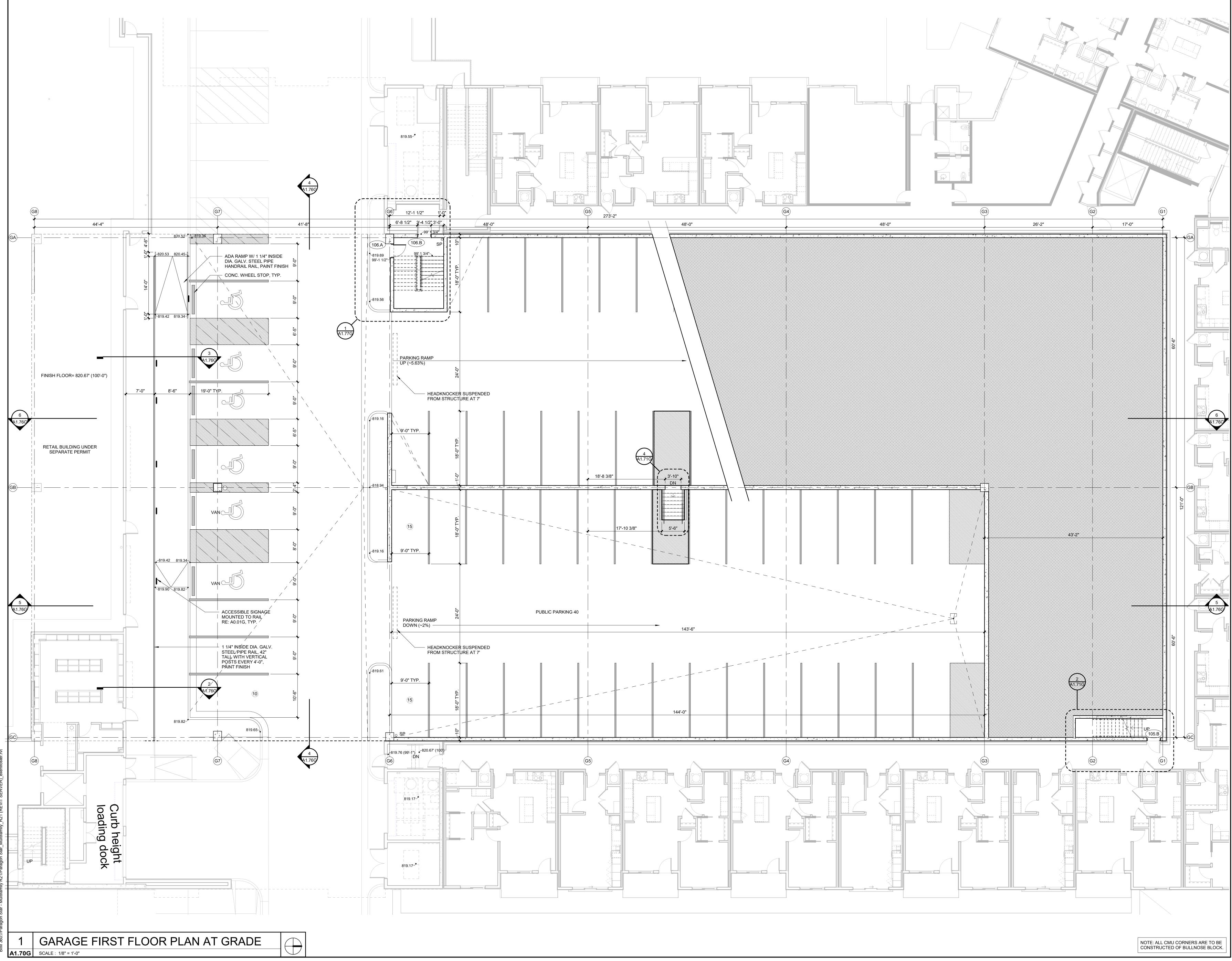
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
ANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS



CODE



SHEET NUMBER





# PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.:		8017,19050.17,19050.08	
Date:	1	10.18.24	
Issued Fo	r. (	GARAGE PERMIT	
13306010	1. 、		
		REVISIONS	
No. Da	te	Description	

# REGISTRATION

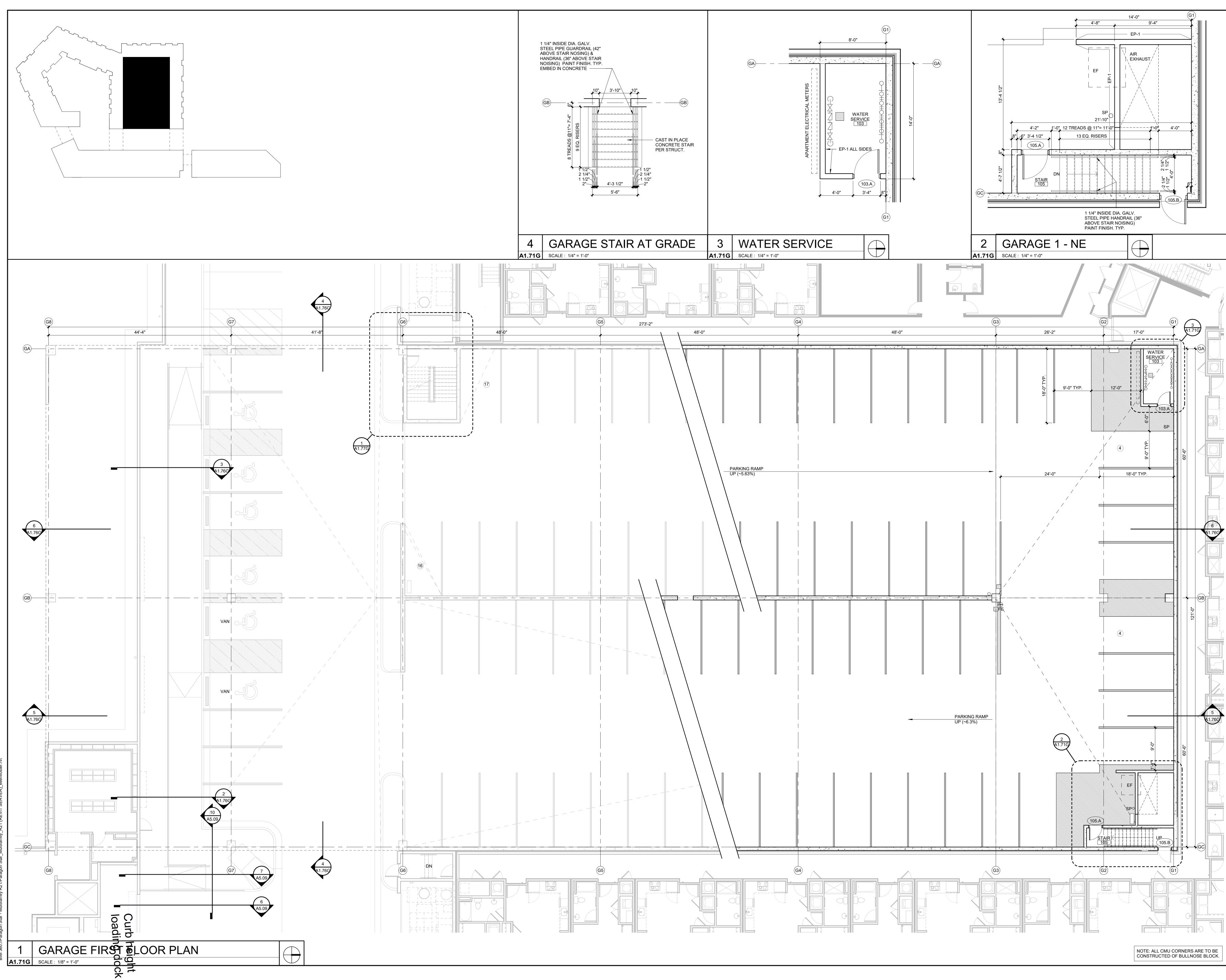


PROJECT TEAM		
FINKLE+WILLIAMS ARCHITECTURE		
GBA ENGINEERS		
LAND 3		
BOB D. CAMPBELL		
LATIMER SOMMERS		
BRINKMANN CONSTRUCTORS		



SHEET NUMBER

A1.70G





# PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

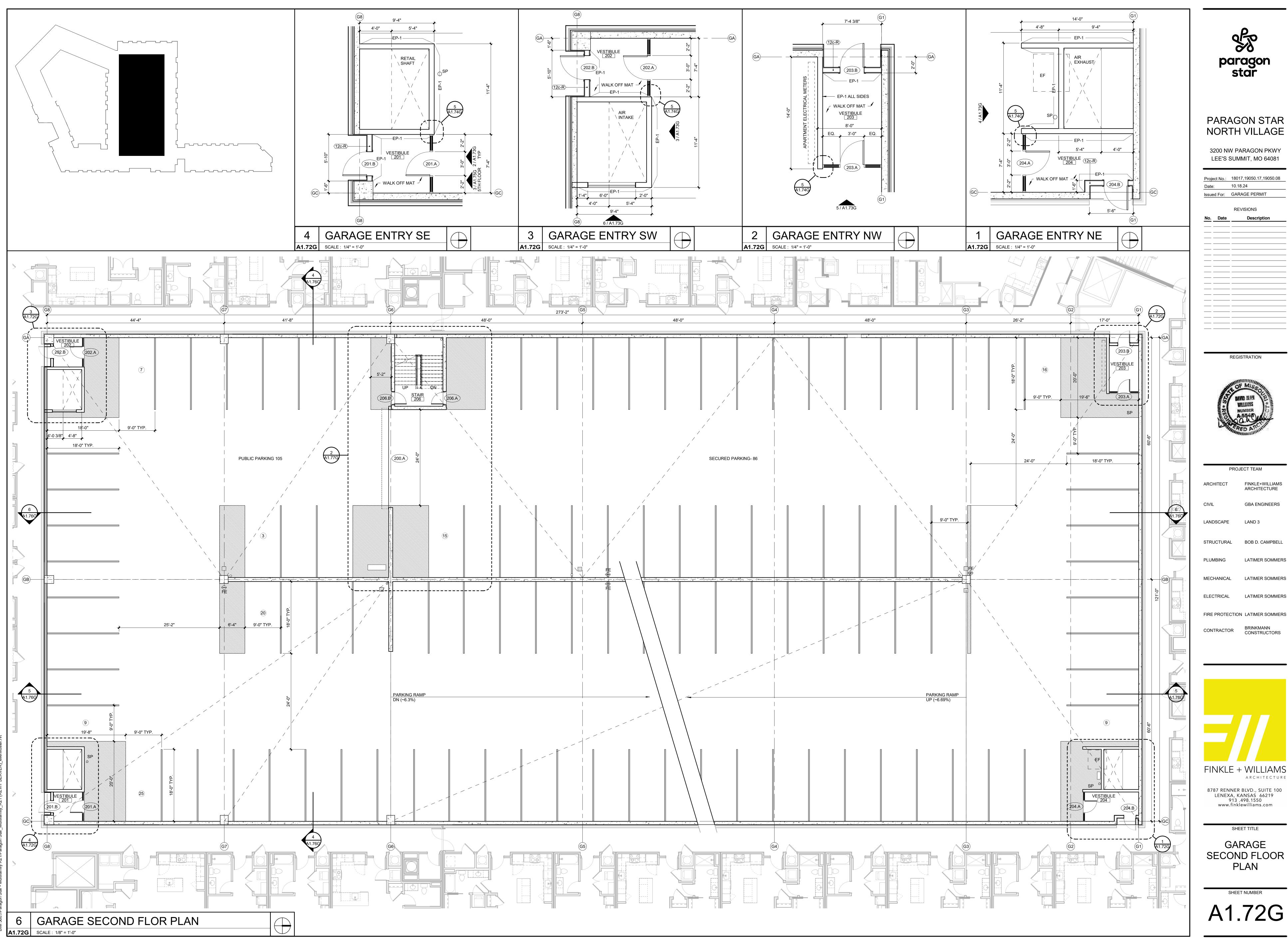
Project No.:		18017,19050.17,19050.08	
Date:		10.18.24	
Issue	ed For:	GARAGE PERMIT	
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		REVISIONS	
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# REGISTRATION



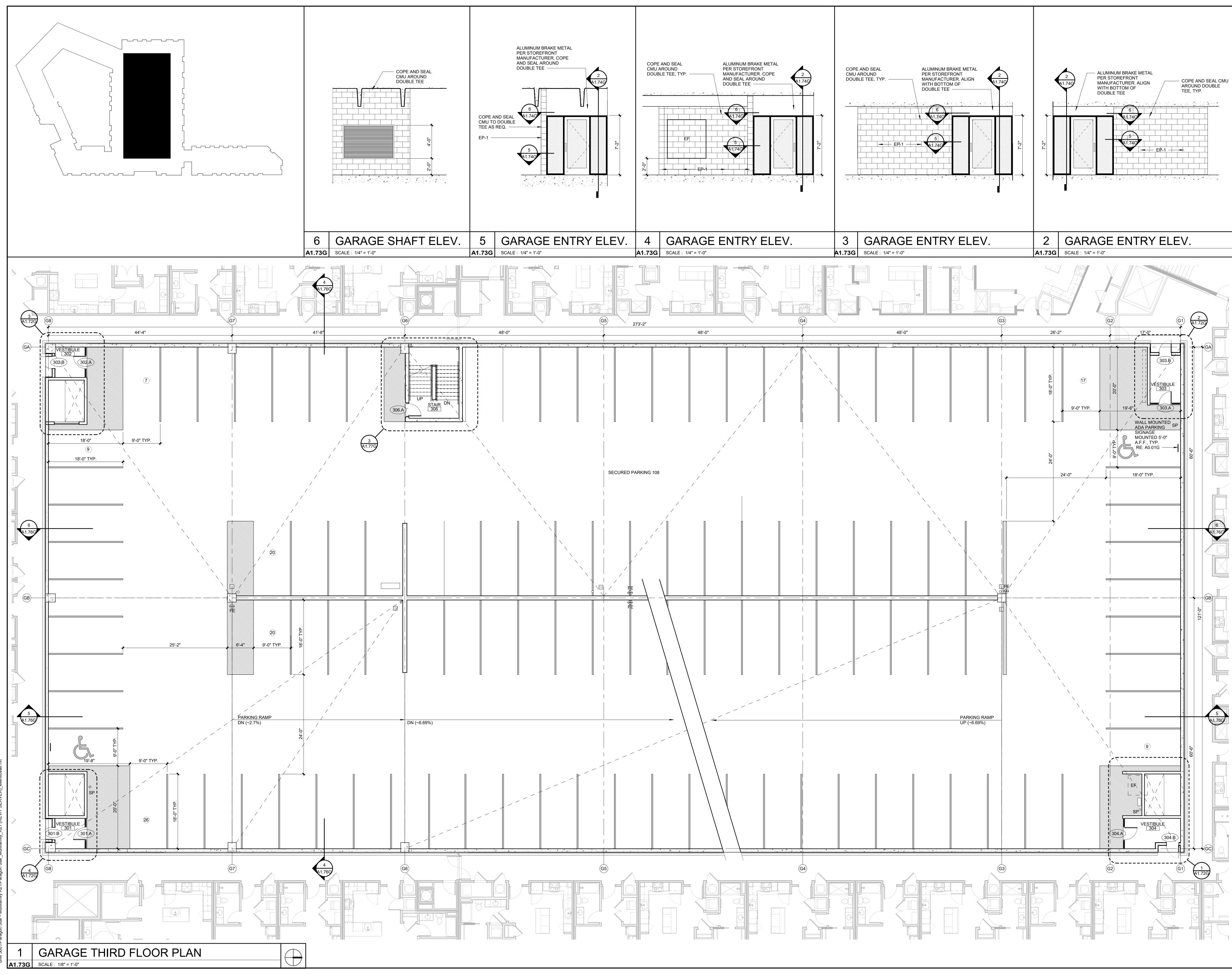
PROJECT TEAM			
FINKLE+WILLIAMS ARCHITECTURE			
GBA ENGINEERS			
LAND 3			
BOB D. CAMPBELL			
LATIMER SOMMERS			
BRINKMANN CONSTRUCTORS			





LATIMER SOMMERS

FINKLE + WILLIAMS ARCHITECTURE 8787 RENNER BLVD., SUITE 100 LENEXA, KANSAS 66219 913 .498.1550 www.finklewilliams.com





# PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.:		18017,19050.17,19050.08
Date:		10.18.24
Issue	ed For:	GARAGE PERMIT
10000		
		REVISIONS
No.	Date	Description
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# REGISTRATION

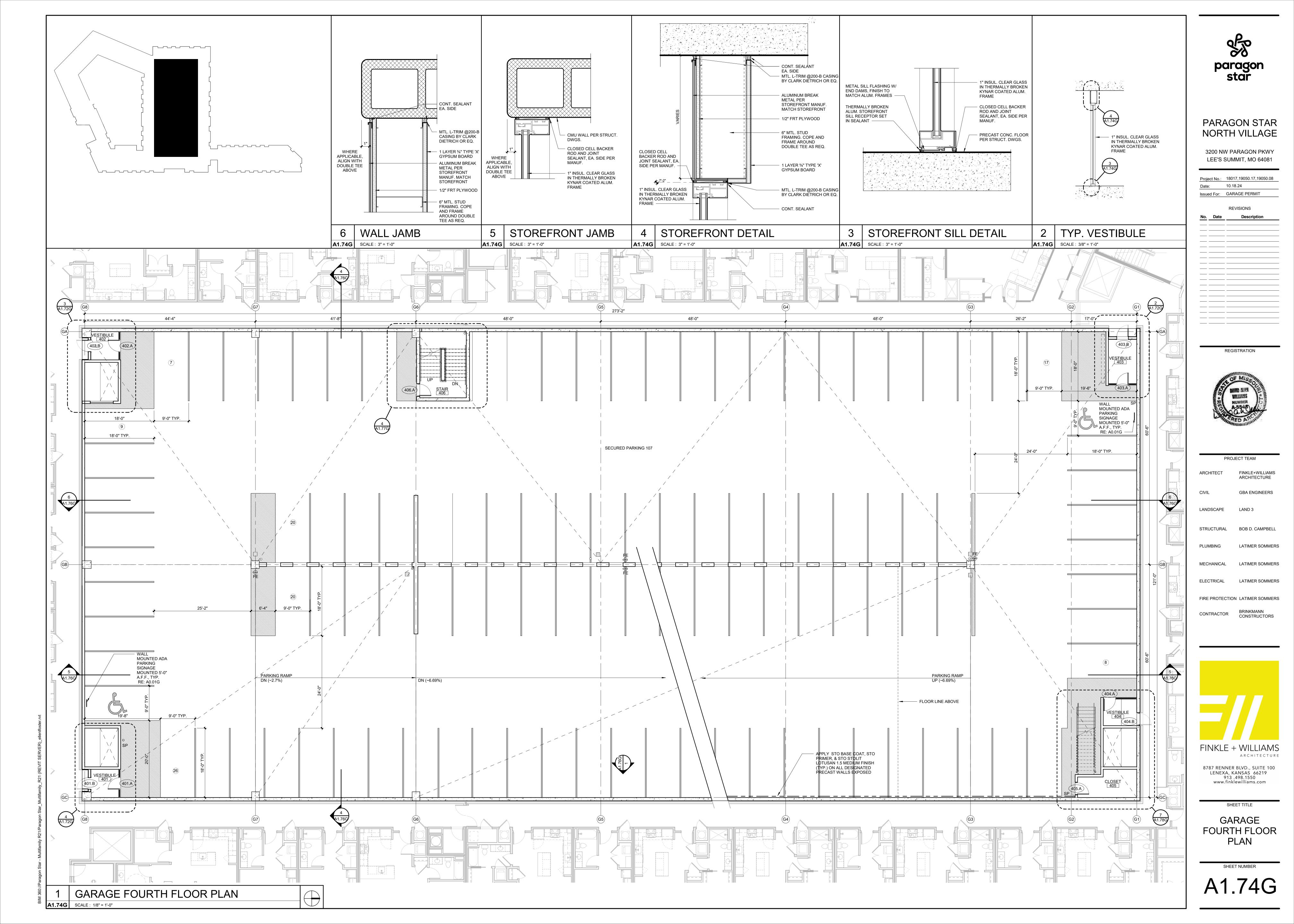


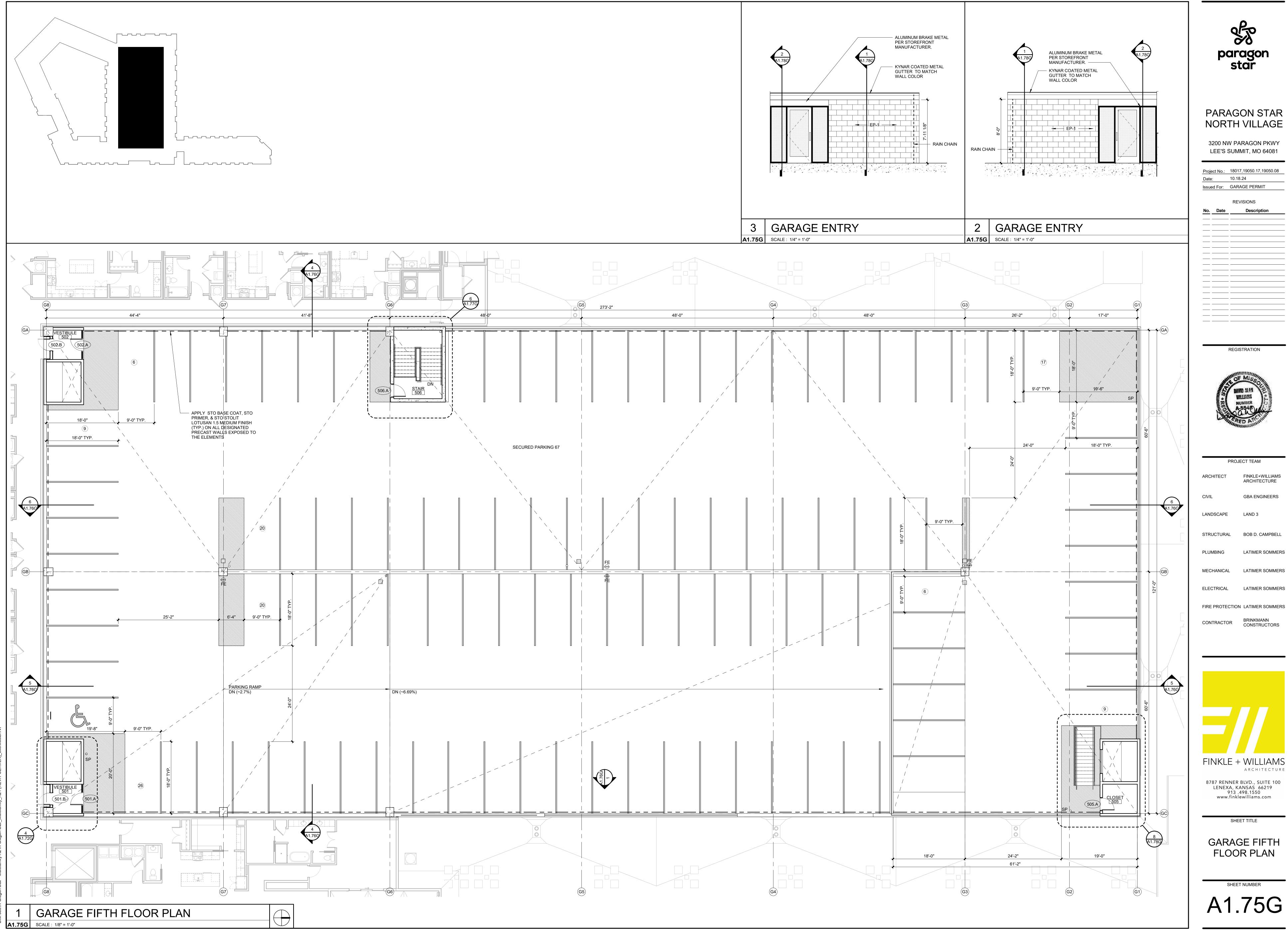
PROJECT TEAM		
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE	
CIVIL	GBA ENGINEERS	
LANDSCAPE	LAND 3	
STRUCTURAL	BOB D. CAMPBELL	
PLUMBING	LATIMER SOMMERS	
MECHANICAL	LATIMER SOMMERS	
ELECTRICAL	LATIMER SOMMERS	
FIRE PROTECTION	LATIMER SOMMERS	
CONTRACTOR	BRINKMANN CONSTRUCTORS	



SHEET NUMBER

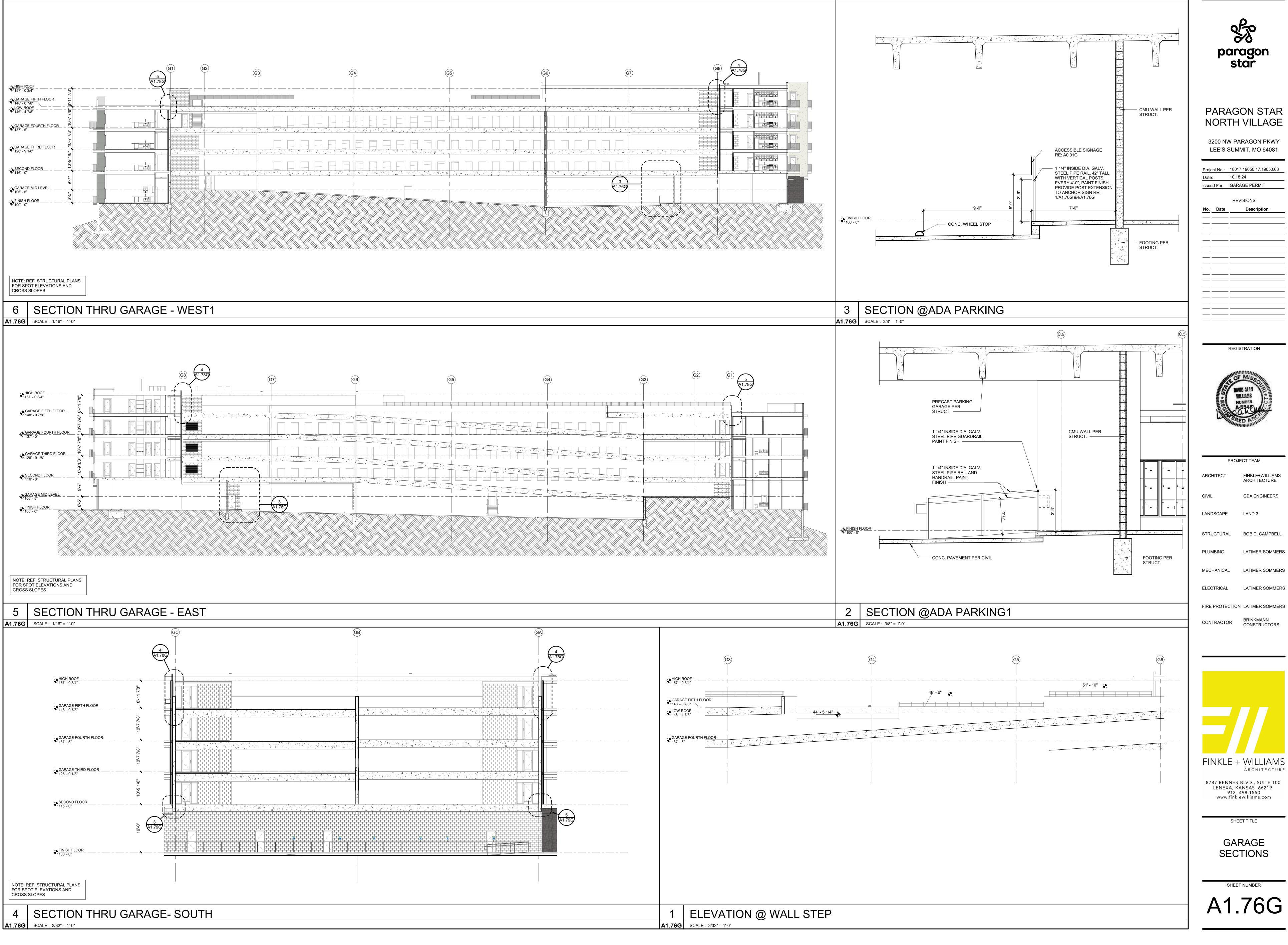
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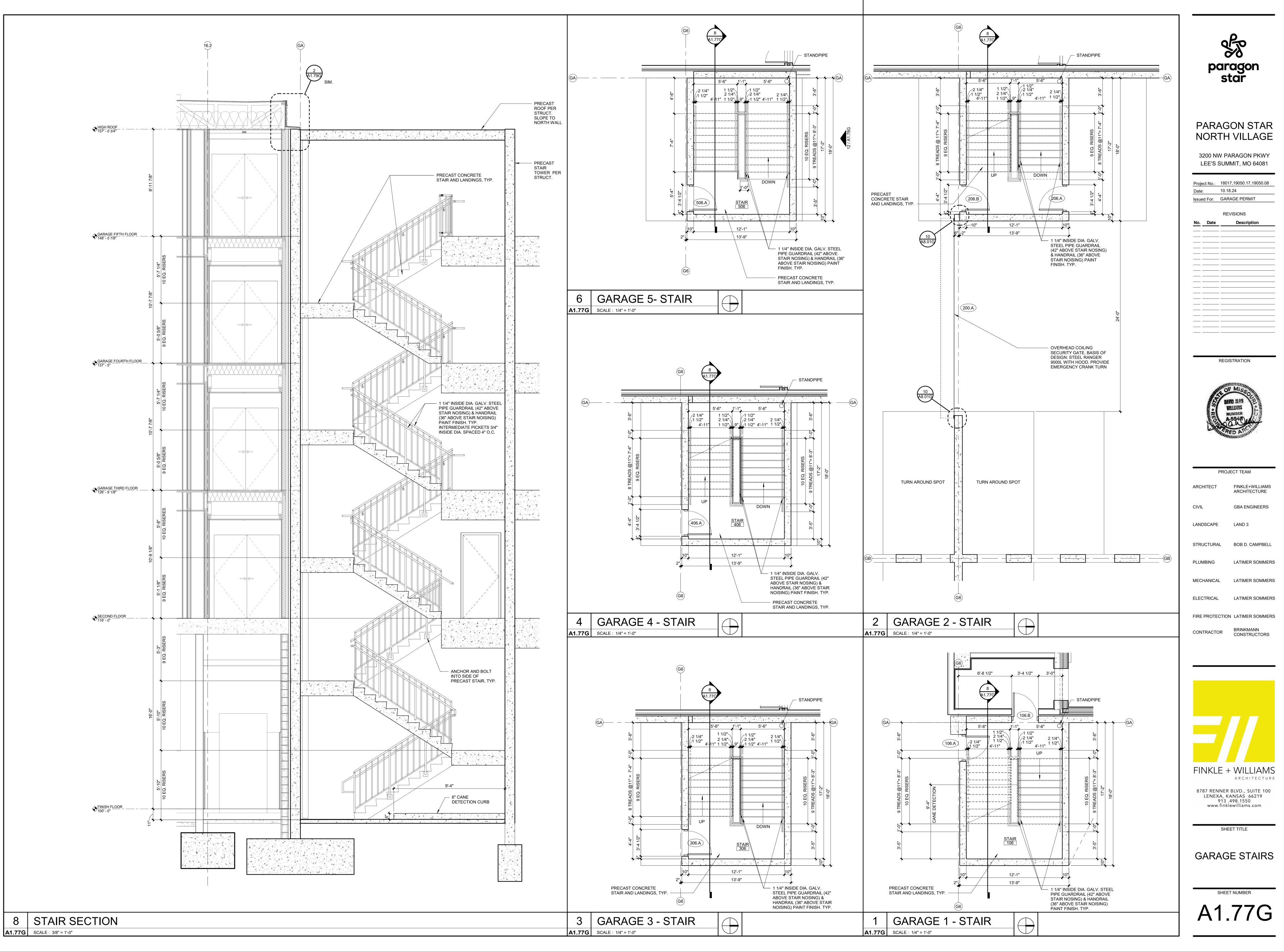




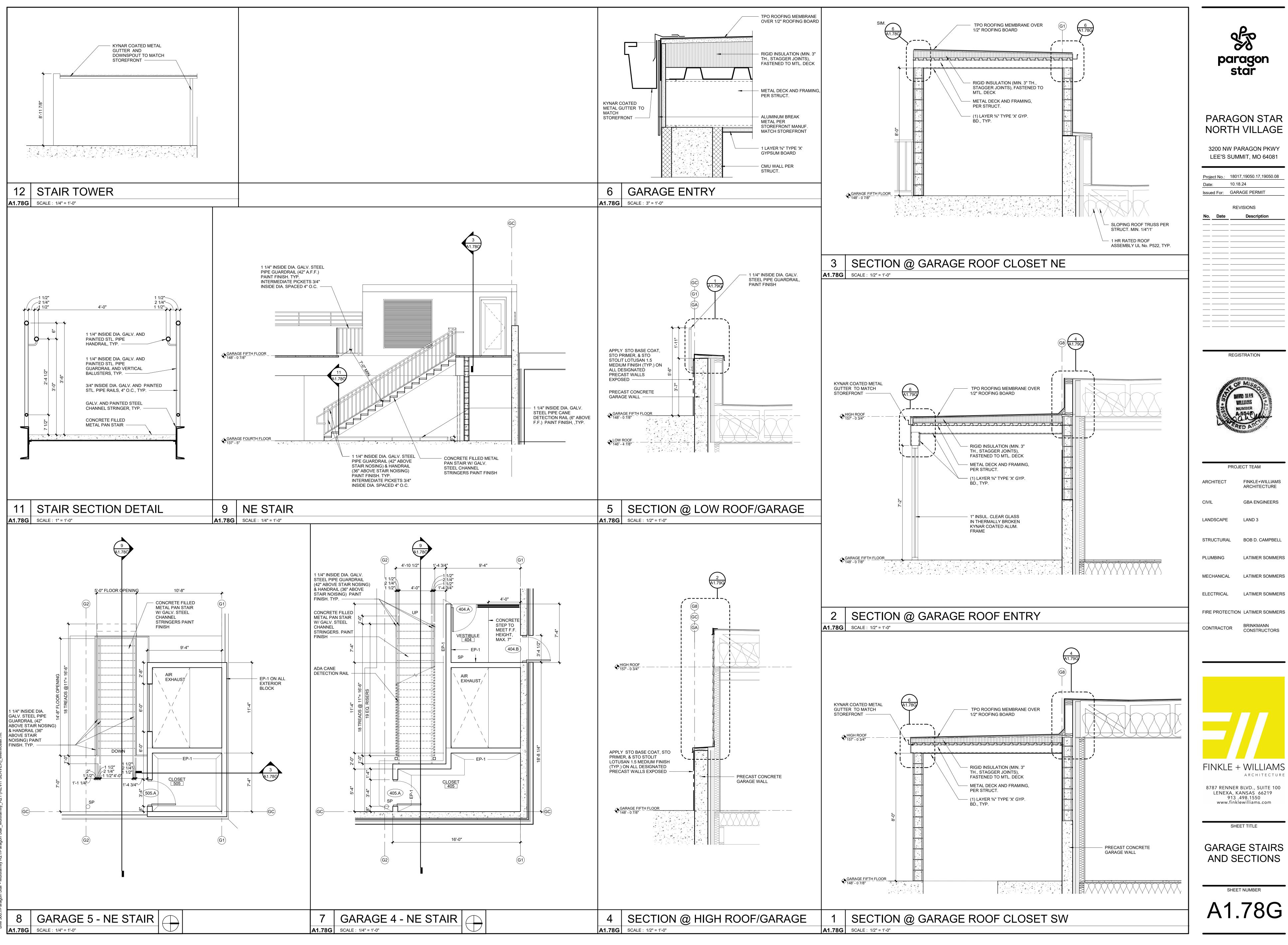
30://Paragon Star - Multifamily R21/Paragon Star\_Multifamily\_R21 (REVIT SERVER)\_ellenlfoster.

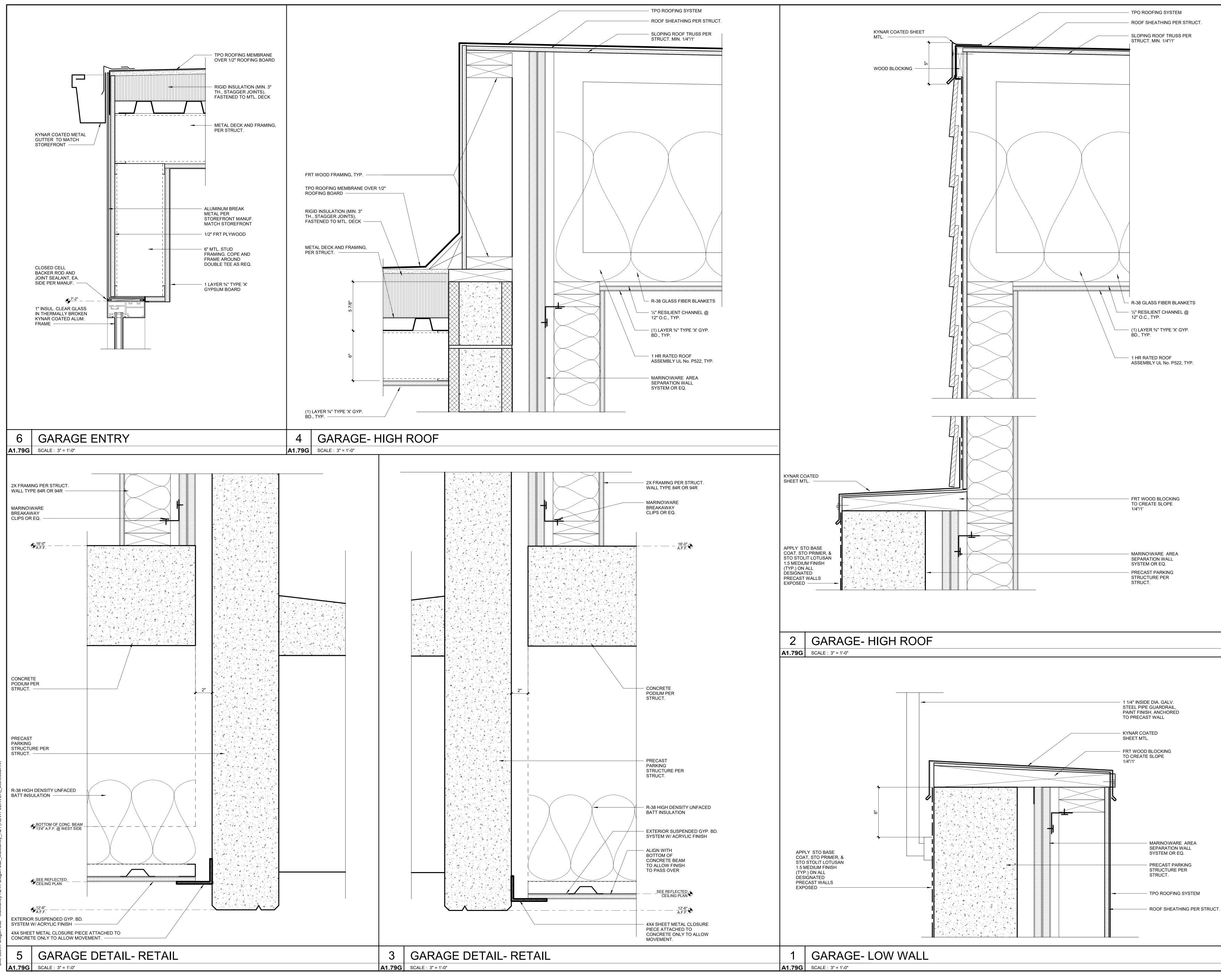


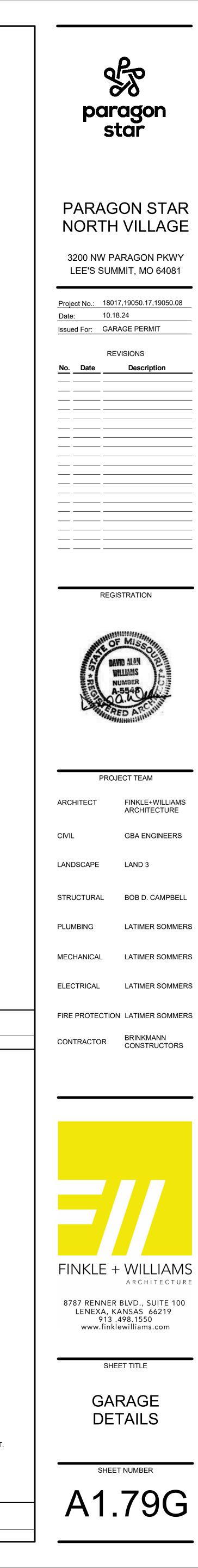


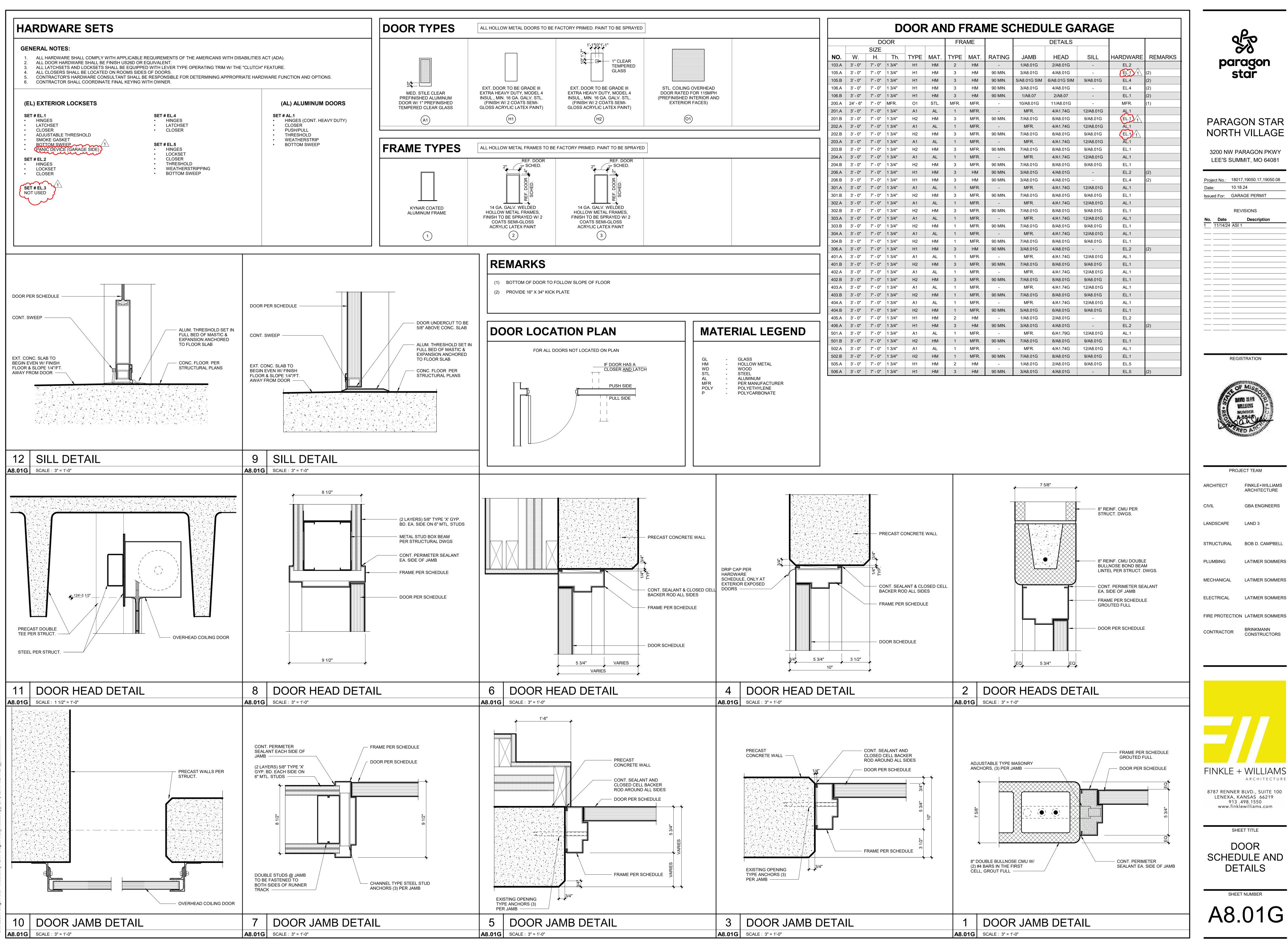


60://Paragon Star - Multifamily R21/Paragon Star\_Multifamily\_R21 (REVIT SERVER)\_ellenlfoster









		DC	DOR			FRA	ME			DETAILS			
		SIZE											
NO.	W.	Н.	Th.	TYPE	MAT.	TYPE	MAT.	RATING	JAMB	HEAD	SILL	HARDWARE	REM
103.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	2	HM	-	1/A8.01G	2/A8.01G	-	EL.2	
105.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	HM	90 MIN.	3/A8.01G	4/A8.01G	-	EL.1 1	(2)
105.B	3' - 0"	7' - 0"	1 3/4"	H1	HM	3	HM	90 MIN.	5/A8.01G SIM	6/A8.01G SIM	9/A8.01G	EL.4	(2)
106.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	HM	90 MIN.	3/A8.01G	4/A8.01G	-	EL.4	(2)
106.B	3' - 0"	7' - 0"	1 3/4"	H1	HM	3	HM	90 MIN.	1/A8.07	2/A8.07	-	EL.1	(2)
200.A	24' - 6"	7' - 0"	MFR.	01	STL.	MFR.	MFR.	-	10/A8.01G	11/A8.01G	-	MFR.	(1)
201.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
201.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
202.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
202.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1 1	
203.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
203.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
204.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
204.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
206.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	3/A8.01G	4/A8.01G	-	EL.2	(2)
206.B	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	3/A8.01G	4/A8.01G	-	EL.4	(2)
301.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
301.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
302.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
302.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
303.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
303.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	1	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
304.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
304.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	1	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
306.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	3/A8.01G	4/A8.01G	-	EL.2	(2)
401.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
401.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
402.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
402.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
403.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
403.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	1	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
404.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
404.B	3' - 0"	7' - 0"	1 3/4"	H2	НМ	1	MFR.	90 MIN.	5/A8.01G	6/A8.01G	9/A8.01G	EL.1	
405.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	2	НМ	-	1/A8.01G	2/A8.01G	_	EL.2	
406.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	3/A8.01G	4/A8.01G	-	EL.2	(2)
501.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	6/A1.79G	12/A8.01G	AL.1	
501.B	3' - 0"	7' - 0"	1 3/4"	H2	HM	1	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
502.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
502.B	3' - 0"	7' - 0"	1 3/4"	H2	HM	1	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
505.A	3' - 0"	7' - 0"	1 3/4"	H1	HM	2	HM	-	1/A8.01G	2/A8.01G	9/A8.01G	EL.5	
506.A	3' - 0"	7' - 0"	1 3/4"	H1	HM	3	HM	90 MIN.	3/A8.01G	4/A8.01G	-		(2)

#### **GENERAL NOTES - STRUCTURAL** General Information A. The contractor shall verify dimensions and conditions before construction and notify the engineer of any discrepancies, inconsistencies, or difficulties affecting the work before proceeding. B. The contractor shall coordinate all disciplines, verifying size and location of all openings, whether shown on structural drawings or not, as called for on architectural, mechanical, or electrical drawings. In the case of work in an existing building the contractor shall scan existing structure to locate all rebar in the area of the new core/opening using ground penetrating radar and notify the engineer of record for review prior to coring/cutting. Conflicts, inconsistencies, or other difficulties affecting structural work shall be called to the architect or engineer's attention for direction before proceeding. All design and construction work for this project shall conform to the requirements of the following governing design codes: 1. International Building Code (IBC 2018) as amended by the city of Lee's Summit, Minimum Design Loads for Buildings and Other Structures (ASCE7-16) Specification for Structural Steel Buildings (AISC 360-16) Member Design Basis is Allowable Stress Design (ASD) Connection Design Basis is Allowable Stress Design (ASD) 4. Structural Welding Code (AWS D1.4-17) 5. Building Code Requirements for Structural Concrete (ACI 318-14) Building Code Requirements for Masonry Structures (TMS 402-16) . North American Specification for the Design of Cold-Formed Steel Structural Members (AISI S100-16) 8. National Design Specification (NDS) for Wood Constriction with 2018 Supplements (ANSI/AWC NDS-2018) 9. Special Design Provisions for Wind and Seismic (AWC SDPWS-2015) D. These drawings are for this specific project and no other use is authorized. 2. Structural Load Design Criteria A. Dead Load: = 35 psf Floor, Apartment = 15 psf Floor, Balony Floor, Corridor (Above Podium) = 25 psf Garage Collateral = 5 psf = 25 psf Stair, Metal Pan = 60 psf Stair, Wood = 25 psf B. Live Load: Floor, Apartment = 40 psf = 60 psf Floor, Balcony Floor, Corridor (Serving Apartment) = 40 psf Floor, Corridor )Serving Public) = 100 psf Floor, Public (Clubhouse) = 100 psf Floor, Storage = 125 psf = 40 psfGarage = 20 psf Roof, MEP Equipment Zone = 45 psf = 100 psf C. Snow: Pg = 20psf, Ce = 1.0 Pf = 14 psf (Apt) & 16.8 psf (Garage), Pm = 20 psf ls = 1.0, Cs = 1.0, Ct = 1.0 (Apt) & 1.2 (Garage) Drift & unbalanced snow loads per ASCE/SEI 7-16 D. Lateral Loads: 1.) Wind V(ult) = 109 mph, exposure C. lw=1.0 GCpi=+/-0.18 Design wind pressures to be used for the design of exterior component and cladding materials on the designated zones of wall and roof surfaces shall be per section 30.7 and Table 30.7-2 of ASCE/SEI 7-16. Tabulated pressures shall be multiplied by effective area reduction factors, exposure adjustment factors, and topographic factors where applicable 2.) Seismic: Ss = 0.099, S1 = 0.068, le=1.0, Site Classification D Seismic Design Category B Basic Seismic Force-resisting System: At Podium A.2 - Ordinary Reinforced Concrete Shear Walls R = 4, Omega = 4, Cd = 2 1/2, V = 0.053W At Apartments Above Podium: A.17 - Light-Framed Walls with Shear Panels of All Other Materials R = 2, Omega = 2 1/2, Cd = 2, V = 0.053W At Precast Garage: A.6 - Ordinary Precast Shear Walls (N/S Direction) R = 3, Omega = 2 1/2, Cd = 3, V = 0.035W B.9 - Ordinary Precast Shear Walls (E/W Direction) R = 4, Omega = 2 1/2, Cd = 4, V = 0.026W E. This project is designed to resist the most critical effects resulting from the load combinations of section 1605.3 of the International Building Code. 3. Concrete A. All concrete for foundations (walls, grade beams, footings and piers) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 6 gallons of water per 100 pounds of cement and not over 4 inches of slump. B. All concrete for interior flatwork (without floor covering) shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 525 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.75 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only). All concrete for interior flatwork (with floor covering) shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 540 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.40 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only). All concrete for exterior flatwork shall have a minimum design compressive strength of 4500 psi in 28 days, with not less than 560 pounds of cement per cubic yard of concrete, not over 5 gallons of water per 100 pounds of cement, with 6% +/- 1% air entrainment, and a maximum of 4 inches of slump. All concrete for columns shall develop a minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 560 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5 gallons of water per 100 pounds of cement and not over 4 inches of slump. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's dosage rates for improved workability. G. The preceding minimum mix requirements may have up to 15% maximum of the cement content replaced with an approved ASTM C618 Class C fly ash, provided the total minimum cementitious content is not reduced. H. Combined aggregate (coarse plus fine) for all concrete shall be well graded from coarsest to finest with no more than 18 percent and not less than 8 percent retained on an individual sieve, except that less than 8 percent may be retained on coarsest sieve and on No. 50 and finer sieves. Submit this gradation report with the concrete mix design shop drawings. All interior concrete slabs on grade shall be placed over 15 mil, Class A Vapor Barrier per ASTM E1745 with less than 0.01 perms, tested after mandatory conditioning. All joints shall be lapped and sealed per manufacturer's recommendations. All penetrations, as well as damaged vapor barrier material shall also be sealed per manufacturer's recommendation prior to concrete placement. Install barrier per manufacturer recommended details at all discontinuous edges (at interior columns, exterior edge of slab, etc.) to ensure terms of warranty are followed. The vapor barrier shall be placed over freedraining granular material as prescribed by the project soils report. Basement foundation walls shall be braced at the base and top of wall by the contractor until the slab on grade at the base and the floor framing/slab at the top of wall is complete and the concrete has achieved 75% of the design strength. The contractor is responsible for engineering and design of the wall bracing, if required. K. All concrete is reinforced concrete unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any details not shown shall be detailed per ACI 315 and meet requirements of ACI 318, current editions. Control joints in dirt formed slab to be as shown on plans. Where not shown, limit controlled areas to not more than 144 square feet, or 12 feet on any side. Slab panel side ratio shall not exceed 1 1/2 to 1. M. Contractor shall verify that all concrete inserts, reinforcing and embedded items are correctly located and rigidly secured prior to concrete placement. N. Construction joints in beams, slabs, and grade beams shall occur at midspan (middle third) unless noted otherwise. Provide 2 x 4 horizontal keys at construction joints for shear transfer. O. No aluminum items shall be embedded in any concrete. 4. Reinforcing Steel A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel. Welded plain wire fabric shall be supplied in sheets and conform to the requirements of ASTM A185. B. Clear minimum coverage of concrete over reinforcing steel shall be as follows: Concrete placed against earth: 3" Formed concrete against earth: 2" Slabs: 1-1/2" 4. Beams or Columns: Other All coverage shall be nominal bar diameter minimum. All dowels shall be the same size and spacing as adjoining main bars (splice lap 48 bar diameters or 24" minimum unless noted otherwise). . At corners of all walls, beams, and grade beams supply corner bars (minimum 2'-0" in each direction or 48 bar diameters) in outside face of wall, matching size and spacing of horizontal bars. Where there are no vertical bars in outside face of wall, supply 3 - #4 vertical support bars for corner bars. Bars marked continuous and all vertical steel shall be lapped 48 bar diameters (2'-0" minimum) at splices and embedments, unless shown otherwise. Splice top bars near midspan and splice bottom bars over supports, unless noted otherwise. At all holes in concrete walls and slabs, add 2 - #5 bars (opening dimension plus 96 diameters long) at each of four sides and add 2 - #5 x 5'-0" diagonally at each of four corners of hole. Openings in 8" thick walls are reinforced similar, but with 1 - #

5 instead of 2 - #5, respectively. G. Unless otherwise covered on architectural plans or specifications, vertical control joints in concrete wall shall be spaced at a maximum of 20'-0" on center and coordinated with the architect. Every other horizontal wall reinforcing bar shall be discontinuous at control joints except heavy top and bottom bars unless noted otherwise. Provide base seal waterstop style number 772 (by Greenstreak Inc. or approved equal) on dirt face side of wall at all walls below grade.

- H. Accessories shall be as specified in latest edition of the ACI Detailing Handbook and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet.
- I. All slabs and stairs not shown otherwise shall be 6" thick with #4 bars at 12" on center each way. All exterior porches and stoops not otherwise detailed may be constructed in any standard manner, solid or hollow, but must be reinforced with #4 bars at 12" on center each way minimum. Porches shall be doweled to adjacent walls or grade beams with #4 bars at 12" on center, hooked or embedded 48 diameters into both members. Slope porches 1/8" per foot for drainage unless noted otherwise.
- J. Allow 2 tons of reinforcing bars #4 or larger to be used as directed in the field for special conditions by the engineer of record (labor for placing same to be included).
- 5. Structural Steel
- A. All structural steel beams and columns shall be ASTM A992, grade 50 steel and all miscellaneous steel shall be ASTM A36 grade steel (except at moment connections where plates shall be ASTM A572, grade 50). Hollow Structural Sections (HSS) shall be ASTM A500, grade C. Fabrication and erection shall be in accordance with AISC 303-05 "Code of Standard Practice for Steel Buildings and Bridges" in the 13th Edition
- of the AISC Steel Construction Manual. B. All welding shall conform to the recommendations of the AWS. C. All exterior steel and connections, and brick relief angles shall be hot-dip galvanized. D. All bolts not otherwise specified shall be 3/4" diameter high strength (ASTM A325-N). All bolts shall be fully pretensioned. All beam connections shall be designed per the AISC Manual of Steel Construction "Framed Beam Connections" for the indicated reactions or at least 0.4 x beam total shear capacity, Vn/Omega, shown in the maximum total uniform load tables, whichever is greater; and, shall account for eccentricity when the bolt line is more than 2" from the center of the support. All connections must be two bolt minimum. Additional connection elements may not be specifically shown in the conceptual details in this set but may be required by the final connection design, such as stiffener plates, doubler plates, supplement/reinforcing plates or other connection material. Connection design and shop drawing preparation shall be completed under the direct supervision of a professional engineer licensed in the state the project is located and shop drawings and connection calculations shall
- bear his/her seal. E. All anchor bolts shall be 3/4" diameter, ASTM F1554, Grade 36 unless noted otherwise. Washers of minimum size and thickness for the given anchor diameter in Table 14-2 of the AISC Steel Construction Manual shall be provided at every column anchor bolt. Washers shall have a standard size hole for the anchor bolt. At braced frames washers shall be welded all around to the column base plate with 3/16" fillet weld.
- F. Allow 2.0 tons structural steel to be used as directed in field for special conditions by the engineer of record. Cost for shop drawings, fabrication, delivery, detailing, and erection to be included. 50% of structural steel allowance shall be bid as miscellaneous galvanized angle and plate.
- 6. Post Installed Anchors
- A. Post-installed anchors shall be used only where specified on the drawings unless approved in writing by the engineer of record. See drawings for anchor diameter, spacing and embedment. Performance values of the anchors shall be obtained for specified products using appropriate design procedures and/or standards as required by the governing building code. Anchors installed in concrete shall have an ICC-ES Evaluation Service Report. Special inspection is required for all post installed anchors. The contractor shall coordinate an on-site meeting with the post installed anchor manufacturer field representative to educate the construction team on the anchor
- installation guidelines and requirements B. Mechanical anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ACI 355.2 and ICC-ES AC193. All anchors
- shall be installed per the anchor manufacturer's written instructions. C. Adhesive anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions.
- D. Mechanical anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC01. All anchors shall be installed per the anchor manufacturer's written instructions. E. Adhesive anchors used in solid grouted masonry shall have been tested and qualified
- for use in accordance with ICC-ES AC58. All anchors shall be installed per the anchor manufacturer's written instructions. F. Anchors used in hollow concrete masonry shall have been tested and qualified in accordance with ICC-ES AC106 or ICC-ES AC58 as appropriate. All anchors shall be installed per the anchor manufacturer's written instructions with appropriate screen tubes used for adhesives
- 7. Foundations
- A. The soil investigation was prepared by Terracon, the report number is 02215169 and the telephone number is 913-492-777 B. Structural foundations consist of a network of stright shaft auger pressure grouted piles established on moderatly weathered shale capable of safely supporting 40ksf end
- bearing. Each pile shall penetrate 5'-0" minimum into the moderately weathered shale. Spread footing and shallow foundations for ancillary structures are designed to bear on engineered fill or undisturbed soil capable of safely supporting 1,500 psf. Retaining walls are designed for an active lateral load of 50 pcf equivalent fluid pressure. D. Basement walls are designed for an at rest lateral load of 70 pcf equivalent fluid
- pressure. See General Note 3.J for wall bracing requirements. E. Contractor shall provide for dewatering at excavations from either surface water or seepage
- F. All foundation excavations shall be inspected by a qualified soil engineer, approved by the architect and/or structural engineer, prior to placement of steel or concrete. This inspection shall be at the owner's expense. G. All concrete in the structural portion retaining the backfill shall have attained its design
- strength prior to being backfilled. H. Moisture content in soils beneath building locations should not be allowed to change after footing excavations and after grading for slabs on grade are completed. If subgrade materials become desiccated or softened by water or other conditions, recompact materials to the density and water content specified for engineered fill. Do not place concrete on frozen ground.
- 8. Drilled Auger Pressure-Grouted Piers
- A. Piers not otherwise indicated shall be 30" diameter.
- B. All piers shall have (4) #7x6'-0" hooked dowels unless otherwise indicated. C. Pier dowels shall extend 40 diameters above top of pier. Driving dowels into concrete after initial set is not allowed.
- D. Refer to the specifications (sections for excavation and concrete) for other detailed requirements
- E. Pier concrete to have 6" slump.

# 9. Concrete Masonry Units

- A. Concrete block used in exterior walls or load bearing walls shall meet the requirements of ASTM C90 and have a minimum net compressive strength of 2650 psi and laid up using type N mortar such that f'm equals 2000 psi. Mortar shall be volume proportion based cement lime mortar. Proportioning shall be completed by box measure. Any block in contact with earth shall be normal weight units, laid using type "S" mortar and
- arouted solid. B. The contractor shall provide adequate temporary bracing for all masonry walls during
- construction. C. All concrete block shall have 9 gage (or larger) horizontal joint reinforcing (ladder or truss) per architectural drawings and specifications (16" maximum vertical spacing).
- D. Cavity wall construction shall be reinforced as designed for specific concrete block used. The horizontal joint reinforcing shall be of the ladder or truss style per specification and continuous between brick and block, as prescribed by the
- architectural drawings. E. Concrete block shall be reinforced as follows in 6", 8", 10", and 12" walls: . Vertical reinforcing shall be a minimum of 1 - #4 bar in 6" and 8" walls and 2 - #4 bars in 10" and 12" walls at 4'-0" on center, at each corner, at each door and window jamb, each side of control joints and in the end void of each length of wall. Lap splices for masonry vertical reinforcing shall be 48 bar diameters or 24" minimum.
- 2. Horizontal reinforcing:
- A. Horizontal joint reinforcing as noted above. B. Continuous horizontal bars shall be included per section or detail in bond beam or optional running bond beam where noted. Where bond beams are continuous at corners of walls, supply corner bars matching size of horizontal bars (minimum 2'-0" or 40 bar diameters in each direction).
- F. Grout, where noted above, shall have a minimum design ultimate compressive strength of 2500 psi at 28 day test and 3/8" maximum aggregate size.
- G. Non-load bearing concrete block walls shall be isolated from adjacent structural elements with vertical 3/8" control joints and at the top of the wall with 1" air space or compressible material and support per architectural detail. H. Unless otherwise covered on architectural plans or specifications, vertical control joints
- in masonry construction shall be 3/8" wide, full height of wall. Joints shall be spaced at a maximum of 24'-0" on center and coordinated with the architect. All horizontal joint
- reinforcing shall be discontinuous at control joints in masonry. All bond beam horizontal reinforcing shall be continuous through control joints. I. Lintels over all openings up to 8'-0" wide in new and existing masonry walls not otherwise covered shall be one 6x3 1/2x5/16 angle for each 4" width of masonry. All
- exterior lintels to be galvanized. J. Walls shall be anchored top and bottom by dowels matching wall vertical reinforcing(unless noted otherwise) from floor slab bottom and bracing angles at the
- top, per details on the drawings.
- 10. Light Gage Metal Structural Framing
- A. All load bearing, light gage structural studs, track, and bridging shall be of the type, size, gage, and spacing as shown on the plans, minimum.
- heavier shall have a minimum vield of 50.000 psi.
- B. All materials shall be 33,000 psi minimum yield, except studs of 16 gage or C. All properties, fabrication, and erection shall be in accordance with latest editions of the AISI "Specifications for the Design of Cold-Formed Structural Members." D. All framing components shall be cut squarely or at an angle to fit squarely
- against abutting members. Splicing of axially loaded members is not permitted. Members shall be held firmly in place until properly fastened. Attachments of
- similar components shall be by welding, screw attachment, or bolting. Wire tying of components is not permitted. E. Tracks shall be securely anchored to floor and overhead members. Special anchorage requirements required for wind bracing shall be as shown on the plans. F. Prior to fabrication and/or erection, the contractor shall submit shop drawings
- complete with detail of erection, fabrication, attachments, anchorages, lintels, etc., for review by the architect/engineer.

### 11. Timber and Wood Framing

- A. Quality and construction of wood framing members and their fasteners for load supporting purposes not otherwise indicated on the drawings shall be in accordance with the 2018
- International Building Code. B. All studs and top and bottom plates shall be Douglas Fir No. 2 grade visually graded lumber, with an allowable fiber stress in bending of 900 psi minimum and an elastic modulus of 1,600,000psi unless noted otherwise. All joist, truss members, and headers to be No. 2
- grade (min.) unless noted otherwise. All lumber for exterior decks and balconies shall be treated Southern Yellow Pine No. 2 grade. C. Bridging of stud bearing walls and shear walls shall be solid, matching sheathing joints. D. Joist blocking and bridging shall be solid wood or cross bridging of either wood or metal
- straps. Spacing, in any case, shall not exceed 8'-0". E. Wood members and sheathing shall be fastened with number and size of fasteners not less than that set forth in Table 2304.9.1 of the 2018 International Building Code. Floor sheathing shall be APA rated tongue and groove Sturd-I-Floor, exposure 1, glued and nailed with 10d nails or # 10 screws at 12" on center field. Sheathing of shear walls or roof diaphragms shall be edge nailed with 8d common nails at 6" on center and nailed to intermediate framing and/or blocking members with 8d common nails at 12" on center unless otherwise noted on the drawings.
- F. Sill plates shall be bolted to concrete slabs with 1/2" diameter bolts at 32" on center (UNO, re: shearwall schedule.) Provide plate washers at sill plate anchors for shearwalls per shearwall schedule. Plates in direct contact with concrete or masonry shall be preservative-
- G. All hangers, ties and connections shown are based on Simpson Strong Tie as the basis of design, provide Simpson Strong Tie or an approved equal. Joist hangers shall be equal to "LUS" for wood application and "LB" for steel weld-on application. Roof truss ties shall be equal to "H2.5A" and tie the roof truss to the top plate (provide (2) "H2.5A" diagonally across from each other when uplift load shown in truss shop submittal exceeds 600lbs). Roof girder ties shall be equal to a "LGT2", "LTG3" or "LGT4" tie (dependent on number of plies) and tie the truss girder to the top plate. Provide "H2.5A" at the top of each stud to top track when the top track has roof truss attached.
- H. Service condition dry with moisture content at or below 19% in service. I. Laminated strand lumber (LSL) shall have an allowable flexural stress (Fb) of 1,700 psi (reduced by size factor) and an elastic modulus (E) of 1,300,000 psi.
- J. Laminated veneer lumber (LVL) shall have an allowable flexural stress (Fb) of 2.600 psi (reduced by size factor) and an elastic modulus (E) of 2.000.000 psi.
- K. Parallel Strand Lumber (PSL) shall have an allowable flexural stress (Fb) of 2,900 psi (reduced by size factor) and an elastic modulus (E) of 2,000,000 psi. ((E) = 2,200,000 psi for members > 18"). L. Glulams shall be 24F-V8 or better with an allowable flexural stress (Fb) of 2,400 psi and an
- elastic modulus (E) of 1,800,000 psi. Exterior glulams shall be moisture-resistant treated. M. Pre-engineered wood trusses shall be designed in accordance with the Truss Plate Institute's national design standard for metal-plate connected wood truss construction (ANSI/TPI-1 latest edition). Trusses shall be designed and manufactured by an authorized member of the Wood Truss Council of America (WTCA). Truss design shall conform to specified codes, allowable stress increases, deflection limitations and other applicable
- criteria of the governing code. N. Truss shop drawings showing complete erection and fabrication details and calculations (including connections) shall be submitted to the project architect/engineer for review prior to fabrication and/or erection. Calculations and layout plan shall bear the seal of a professional engineer, registered in the state of the project location. Layout plan shall incldue truss locations, spacing and all hanger designations used to support trusses to beams or other trusses. Calculations shall indicate max reactions in all directions, number of plies for the truss and dead, live and total load deflections along with a list or key of all standard and nonstandard utilized load combination. Shop drawings shall also be submitted to the local
- government controlling agency when requested by that agency. O. All trusses shall be securely braced both during erection and permanently, as indicated on the approved truss design drawings and in accordance with TPI's commentary and recommendations for handling, installing and bracing metal-plate connected wood trusses (HIB-91, booklet) and the latest edition of ANSI/TPI-1.
- P. The truss manufacturer shall supply all hardware and fasteners for joining truss members together and fastening truss members to their supports. Metal connector plates shall be manufactured by a member of the Wood Truss Council of America (WTCA) and shall be 20 gauge minimum. Connector plates shall meet or exceed ASTM A653, grade 33, with ASTM A924 galvanized coating designation G60. Q. Provide truss space directly above and centered over HVAC closets. Refer to Architectural
- and MEP drawings for exact locations. R. Shipment, handling, and erection of trusses shall be by experienced, qualified persons and shall be performed in a manner so as not to endanger life or property. Apparent truss damage shall be reported to the truss manufacturer for evaluation prior to erection.Cutting or alteration of trusses is not permitted.
- S. Pre-Engineered Floor Trusses: Top Chord Dead Load = 30 psf = Per General Note 5B Top Chord Live Load Bottom Chord Dead Load = 10 psf Live Load Deflection = L/480; (1/2" max) Total Load Deflection = L/360
- Roof Truss Design Criteria: Top Chord Dead Load Top Chord Live Load Top Chord Snow Load Bottom Chord Dead Load Bottom Chord Live Load
- Live Load Deflection = L/360 Total Load Deflection = L/300 U. Roof trusses shall be designed per IBC 2018 for net uplift resulting from wind loading as calculated using components and cladding loading. Top and bottom chord dead load used in
- V. Construction bracing shall be provided by the contractor as required to keep the building and studs plumb. W. Structural members shall not be cut for pipes, etc., unless specifically detailed. Nothing and boring of studs and top of plates shall conform to the provisions of section 2308.9.10 and 2308.9.11 of the IBC. Where top plates or sole plates are cut for pipes, a metal tension tie
- with minimum 0.058 inches thick and 1 1/2" inches wide shall be fastened to each plate across and to each side of the opening with not less than (6) 16d nails, in accordance with section 2308.9.8 of the IBC. X. All fasteners for wood to wood connections and wood connectors shall be as indicated in
- structural drawings or manufacturer literature to achieve full capacity of connector. Alternate fasteners may be submitted as a substitution request. Submittal must show that alternative fasteners will not reduce the capacity of the connection.

### 12. Precast Concrete Members

- A. The contractor/supplier is responsible for the design of all the precast members and connection between them and other structural members. Submit design calculations, sealed by an engineer licensed in the state of the project location, for review by the architect/engineer of record.
- B. All precast members are to be designed in accordance with ACI 318-11, 2012 IBC and other applicable codes, standards (see specs) and design criteria shown on design documents.
- C. Precast concrete members shall conform to the 2012 IBC for the required fire ratings (refer to architects documents). D. All wall panels should be designed for building wind loads, seismic loads, gravity loads,
- and transmit these loads to the foundation through properly designed connections. E. Provide blockouts and openings for mechanical/electrical equipment. Refer to mechanical/electrical documents.
- F. Shop drawings shall be complete and shall include a layout plan, fabrication details, estimated camber, connection and anchorage details and member identification marks. Identification marks shall appear on manufactured units to facilitate correct field placement.

### 13. Deferred Submittal and Shop Drawing

- A. Bob D. Campbell and Company, Inc. will review the General Contractor's (GC) shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to be a properly functioning integral element of the overall structural system designed by Bob D. Campbell and Company, Inc.
- B. Deferred submittals shall be submitted to the architect of record for review who shall forward to the building official for review and approval. Design calculations for deferred sub mittals shall be submitted at the same time as the shop drawings for review. Design calculations shall be prepared and sealed by a Professional Engineer licensed in the state of the project. The deferred submittal items shall not be installed until the deferred submittal documents have been approved by the building official.
- Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall: 1. Review each submission for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs incidental thereto, all of which are the sole responsibility of the GC.
- 2. Review and approve each submission. . Stamp each submission as approved. D. Bob D. Campbell and Company, Inc. shall assume that no submission comprises a
- variation unless the GC advises Bob D. Campbell and Company, Inc. with written documentation
- E. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has met the above requirements. Bob D. Campbell and Company, Inc. shall return without comment unrequired material or submissions without GC approval stamp.
- F. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify the GC. 1. Concrete mix designs and material certificates including admixtures and
- compounds applied to the concrete after placement. Reinforcing steel shop drawings including erection drawings and bending details.Bar list will not be reviewed for correct quantities.
- 3. Elevations of all reinforced concrete masonry walls at a scale no smaller than 3/8" = 1'-0" showing all required reinforcing.
- 4. Grout mix designs (for CMU). 5. Construction and control joint plans and/or elevations.
- 6. Structural steel shop drawings including erection drawings and piece details. Include joist, decking and connector submittals. Include miscellaneous framing specified on the structural drawings, but do not submit framing specified on nonstructural drawings for Bob D. Campbell and Company, Inc. review. Defferred Submittal: Railings and guardrails with sealed calculations
- 8. Defferred Submittal: Metal stair framing with sealed calculations 9. Defferred Submittal: Exterior cold-formed metal framing
- 10. Defferred Submittal: Exterior curtain wall 11. Deferred Submittal: Structural steel connection design calculations submitted
- concurrently with structural steel shop drawings. 12. Miscellaneous anchors shown on the structural drawings. 13. Deferred Submittal: Wood truss design calculations and detailed erection and
- fabrication drawings. Standard stick framing shop drawings need not be submitted. 14. Standard details and bridging information for light gage metal framing. Erection plans and details for light gage metal joists and lintels spanning more than 6'-0" shall be submitted. Standard wall framing
- need not be submitted.
- 15. Deferred Submittal: Augured pile foundation plans and details. 16. Deferred Submittal: Precast concrete shop drawings including erection drawings and connection details. 17. Deferred Submittal: Precast concrete connection design calculations.
- 18. Deferred Submittal: Cold-Formed metal framing for exterior walls.

14 Chat	encent of Structured Crossiel Inc						STRUCTURAL SHEET LIST		
Α. Τ	ement of Structural Special Ins	sed on com	pletion of special inspections			Sheet Number	Sheet Name	Current Revision	Current Revision
d C	uring construction in accordance with se code. The owner shall employ one or mo ne required special inspections.	ction 1704 c	of the International Building			S0.01	GENERAL NOTES	4	Date 9.28.22
B. T a	he special inspector shall furnish inspect rchitect and structural engineer, and any Il discrepancies shall be brought to the ir	other desig	nated person.			S0.02 S0.03	WOOD SCHEDULES & TYPICAL DETAILS WOOD SCHEDULES & TYPICAL DETAILS	3 3	7.20.22 7.20.22
Ci Si	orrection, then, if uncorrected, to the pro tructural engineer. he special inspector shall submit a final s	per design a	authority, building official and			S0.04 S0.05	STEEL SCHEDULES CMU DETAILS	3	7.20.22 7.11.22
s th	pecial inspection was, to the best of the i ne approved plans and specifications and ne building code.	inspector's k	nowledge, in conformance with			S0.06 S0.10	WOOD SHRINKAGE & MOVEMENT CONCRETE SCHEDULE	2	7.11.22
Е. Т р	he following inspections and tests are re eriodic) as defined within the referenced seneral Contractor shall provide notificati	section or s	tandard listed below. The			S1.01 S1.02	FOUNDATION PLAN SECOND FLOOR FRAMING PLAN		
ir 1	<ul> <li>spection are ready to be inspected and  </li> <li>Shop Fabrication – structural steel pe shop</li> </ul>	provide acce	ess for those inspections.			S1.03	THIRD FLOOR FRAMING PLAN		
	<ul> <li>Shop Fabrication – pre-engineered w TPI certified shop</li> <li>Shop Fabrication – precast concrete precision</li> </ul>					S1.04 S1.05	FOURTH FLOOR FRAMING PLAN         FIFTH FLOOR FRAMING PLAN		
4	<ul> <li>Shop Fabrication – precast concrete j shop</li> <li>Steel Construction per Section 1705.2 of AISC 341 Chapter J (as referenced)</li> </ul>	2 and the qu	ality assurance requirements			S1.06 S1.11A	ROOF FRAMING PLAN         BUILDING A FOUNDATION PLAN	3	7.20.22
5	. Concrete Construction per Section 17 a. Reinforcing Steel Placement					S1.12A S1.13A	BUILDING A SECOND FLOOR FRAMING PLANBUILDING A THIRD FLOOR FRAMING PLAN	3 3	7.20.22 7.20.22
	<ul><li>b. Reinforcing Steel Welding</li><li>c. Cast in Place Anchors</li><li>d. Post Installed Anchors</li></ul>					S1.14A S1.15A	BUILDING A FOURTH FLOOR FRAMING PLANBUILDING A ROOF FRAMING PLAN	3	7.20.22 9.28.22
	<ul><li>e. Design Mix Verification</li><li>f. Concrete Sampling and Testing</li><li>g. Concrete Placement</li></ul>					S1.16A S1.21B	BUILDING A SHEARWALL PLAN BUILDING B FOUNDATION PLAN	3	7.20.22 9.28.22
	<ul> <li>h. Concrete Curing</li> <li>i. Prestressed Concrete Stressing a</li> <li>j. Erection of Precast</li> </ul>		-			S1.22B S1.23B	BUILDING B SECOND FLOOR FRAMING PLAN BUILDING B THIRD FLOOR FRAMING PLAN	4	9.28.22 9.28.22
	<ul> <li>k. Verification of In-situ Concrete St Concrete</li> <li>I. Formwork Shape, Location and E</li> </ul>	Dimensions				S1.24B S1.25B	BUILDING B FOURTH FLOOR FRAMING PLAN         BUILDING B ROOF FRAMING PLAN	4	9.28.22 9.28.22
	<ul> <li>Masonry Construction per Section 17 requirements of TMS 402/ACI530/AS</li> <li>Verification of Soils per Table 1705.6</li> </ul>	CE5 and TM	/S602/A530.1/ASCE6 Level B			S1.26B	BUILDING B SHEARWALL PLAN	4	9.28.22
89	<ul> <li>Inspections and Tests of Cast-In-Plac</li> <li>Wood Lateral System (periodic)         <ul> <li>a. Wood shearwalls (include sheath</li> </ul> </li> </ul>	-				S1.31C S1.32C	BUILDING C FOUNDATION PLAN         BUILDING C SECOND FLOOR FRAMING PLAN	4	9.28.22 9.28.22
	attachments) b. Portal frames c. Shear wall and portal frame holdo	owns				S1.33C S1.34C	BUILDING C THIRD FLOOR FRAMING PLANBUILDING C FOURTH FLOOR FRAMING PLAN	4	9.28.22 9.28.22
1	<ul> <li>d. Shear wall tension rod system</li> <li>0. Wood Gravity Framing and Placemer where indicated as required)</li> </ul>					S1.35C S1.36C	BUILDING C ROOF FRAMING PLAN     BUILDING C SHEARWALL PLAN	4	9.28.22 9.28.22
	<ul> <li>a. Heavy timber/SCL/glulam beams</li> <li>b. Headers and jambs (random sam</li> <li>c. Bearing walls (random sampling)</li> </ul>	npling)				S1.41D S1.42Da	BUILDING D FOUNDATION PLATO BE BUILDING D PODIUM SLAB REINFORCEMENT PLAN	4 3	9.28.22 7.20.22
	<ul><li>d. Connector/hardware installation (</li><li>e. Floor and roof trusses (random s</li></ul>		ություց)			S1.42Db S1.43D	BUILDING D PODIUM SLAB REINFORCEMENT PLAN BUILDING D PODIUM DTLID RALAE GEOMETRYPLAN BUILDING D THIRD FLOOR FRAMING PLAN	3 4	7.20.22 9.28.22
-	yright and Disclaimer						BUILDING D FOURTHEED FRANK PETHELY BUILDING D FIFTS EOF FRANK FANTELY	4	9.28.22 9.28.22
B tr	Il drawings in the structural set (S-series ob D. Campbell and company, Inc. The aced, or copies in any manner without th	se drawings ne written pe	may not be photographed, rmission of Bob D. Campbell			S1.46D S1.47D	BUILDING D ROOF FRAMING PLAN BUILDING D SHEARWALL PLAN	4	9.28.22 7.20.22
th C	nd Company, Inc. Exception: Original d ne owner, architect, and general contract onstruction. Subcontractors may not rep	or for coord	ination, bidding, and			S1.51E S1.52Ea	BUILDING E FOUNDATION PLAN BUILDING E PODIUM SLAB REINFORCEMENT PLAN	4	9.28.22 7.11.22
B. I, D	r in any manner. Christopher A. Beverlin, P.E., registerec . Campbell and Company, Inc., do herek	by accept pr	ofessional responsibility as			S1.52Eb	BUILDING E PODIUM STUD RAIL & SLAB GEOMETRY PLAN	2	7.11.22
d oʻ	equired by the professional registration la rawings consisting of S-series drawings. ther drawings in the construction docume	I hereby di ent package	sclaim responsibility for all , they being the responsibility			S1.53E S1.54E	BUILDING E THIRD FLOOR FRAMING PLAN         BUILDING E FOURTH FLOOR FRAMING PLAN	4	9.28.22 9.28.22
0	f other design professionals whose seals lsewhere in the construction document p	s and signed	statements may appear			S1.55E S1.56E	BUILDING E FIFTH FLOOR FRAMING PLANBUILDING E ROOF FRAMING PLAN	4	9.28.22 9.28.22
	<u>STRUC</u>	CTUR/	AL ABBREVIATIONS			S1.57E S1.61F	BUILDING E SHEARWALL PLANBUILDING F FOUNDATION PLAN	4	9.28.22 9.28.22
	AT AND ROUND, DIAMETER	FLR FS FTG	FLOOR FAR SIDE FOOTING	PERP PL PLF	PERPENDICULAR PLATE POUNDS PER LINEAR FOOT	S1.62Fa S1.62Fb	BUILDING F PODIUM SLAB REINFORCEMENT PLANBUILDING F PODIUM STUD RAIL & SLAB GEOMETRY PLAN	23	7.11.22 7.20.22
ADTL AFF	ADDITIONAL ABOVE FINISHED FLOOR ALTERNATE	FV GA GALV	FIELD VERIFY GAGE GALVANIZE(D)	PJP PSF PSI	PARTIAL JOINT PENETRATION POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH	S1.63F S1.64F	BUILDING F THIRD FLOOR FRAMING PLAN BUILDING F FOURTH FLOOR FRAMING PLAN	2	7.11.22
ARCH BLDG	ARCHITECTURAL BUILDING BOTTOM OF	GEN GR GRBM	GENERAL GRADE GRADE BEAM	QTY RAD RD-#	QUANTITY RADIUS ROOF DECK TYPE	S1.65F S1.66F	BUILDING F FIFTH FLOOR FRAMING PLAN       BUILDING F ROOF FRAMING PLAN	2	7.11.22
BM BOTT	BEAM BOTTOM BEARING	HORIZ	HORIZONTAL HOLLOW STRUCTURAL SECTION INSIDE FACE	REF REINF REQD	REFERENCE REINFORCEMENT REQUIRED	S1.67F S1.71G	BUILDING F SHEARWALL PLAN GARAGE FOUNDATION PLAN	3	7.20.22 9.28.22
C CD-#	CAMBER CONCRETE DECK TYPE	INFO INT JST	INFORMATION INTERIOR JOIST	REV RLL RTU	REVISION ROOF LIVE LOAD ROOF TOP UNIT	S1.72G	GARAGE SECOND FLOOR FRAMING PLAN	4	6.08.22
CJP CL	COMPLETE JOINT PENETRATION CENTERLINE CONCRETE MASONRY UNIT	JT K KSF	JOINT KIPS (1000 LBS) KIPS PER SQUARE FOOT	SC SCHED SECT	SLIP CRITICAL SCHEDULE(D) SECTION	S1.73G S1.74G	GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN	1	6.08.22 6.08.22
COL CONC	COLUMN CONCRETE CONNECTION	KSF KSI LBS, # Ld	KIPS PER SQUARE FOOT KIPS PER SQUARE INCH POUNDS DEVELOPMENT LENGTH	SHT SIM SJ	SHEET SIMILAR SAW JOINT	S1.75G S1.76G	GARAGE FIFTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN	1	6.08.22
CONT COORD	CONTINUOUS COORDINATE COVER	LL LLH LLV	LIVE LOAD LONG LEG HORIZONTAL LONG LEG VERTICAL	SL SOG SOG-#	SNOW LOAD SLAB-ON-GRADE SLAB-ON-GRADE TYPE	S2.00 S2.01	STAIR FRAMING - BUILDING A STAIR FRAMING - BUILDING B	3	7.20.22 9.28.22
DBL DET	DOUBLE DETAIL DIAMETER	LONG LSLT LTWT	LONGITUDINAL LONG-SLOTTED HOLE TRANSVERSE LIGHTWEIGHT	SPCG SPEC SPRT	SPACING SPECIFICATION SUPPORT	S2.02 S2.03	STAIR & ELEVATOR FRAMING - BUILDING CSTAIR & ELEVATOR FRAMING - BUILDING D	2	7.11.22 7.11.22
DIM DL	DIMENSION DEAD LOAD DRAWING	M MAX MECH	MOMENT FORCE MAXIMUM MECHANICAL	SQ SS SSLT	SQUARE STAINLESS STEEL SHORT-SLOTTED HOLE TRANSVERSE	S2.04 S2.05	STAIR & ELEVATOR FRAMING - BUILDING E STAIR FRAMING - BUILDING F	2	7.11.22 7.11.22
EA EF	EACH EACH FACE EXPANSION JOINT	MFGR MIN MISC	MANUFACTURER MINIMUM MISCELLANEOUS	STD STIFF STIR	STANDARD STIFFENER STIRRUP	S2.10 S2.11	STAIR FRAMING DETAILS ELEVATOR FRAMING DETAILS	2	7.11.22
EL, ELEV EMBED	ELEVATION EMBEDMENT, EMBEDDED ENGINEER	MSRY MTL NF	MASONRY METAL NEAR FACE	STL STRUCT T/	STEEL	S2.20 S2.21	BALCONY FRAMING PLANS BALCONY FRAMING PLANS	3	7.20.22 7.20.22
EOD EOR	EDGE OF DECK ENGINEER OF RECORD EDGE OF SLAB	NS NTS NW	NEAR SIDE NOT TO SCALE NORMAL WEIGHT	THRU TOS TRANS	THROUGH TOP OF STEEL, TOP OF SLAB TRANSVERSE	S2.22 S2.30	BALCONY FRAMING PLANS BALCONY FRAMING DETAILS	3	7.20.22 7.11.22
EQUIP	EQUAL EQUIPMENT EACH WAY	OC OF OPNG	ON CENTER OUTSIDE FACE OPENING	TYP UNO V	TYPICAL UNLESS NOTED OTHERWISE SHEAR FORCE	S3.00 S3.01	TYPICAL FOUNDATION DETAILS GARAGE FOUNDATION DETAILS	2	7.11.22 7.11.22
EXP EXT	EXPANSION EXTERIOR EXISTING	OPP OVS P	OPPOSITE OVERSIZED HOLE AXIAL FORCE	VERT W/ W/0	VERTICAL WITH WITHOUT	S3.02 S3.03	PILE & PODIUM FOUNDATION DETAILS APARTMENT FOUNDATION DETAILS	4	9.28.22
FD-# FDN	FLOOR DECK TYPE FOUNDATION FAR FACE	PAF PC PCF	POWDER ACTUATED FASTENER PRECAST / PILE CAP POUNDS PER CUBIC FOOT	WF WL WP	WIDE FLANGE WIND LOAD WORK POINT	S3.04	FOUNDATION DETAILS	4	9.28.22
FIN	FINISH	PEMB	PRE-ENGINEERED METAL BUILDING	WWF	WELDED WIRE FABRIC	S3.05 S3.06	FOUNDATION DETAILS FOUNDATION DETAILS	3	9.28.22 7.20.22
STRI MARK	JCTURAL DECK & S	SLAB	SCHEDULE			S3.07 S3.08	FOUNDATION DETAILS FOUNDATION DETAILS	3	7.20.22
FD-1	1" TO 1 1/4" GYPCRETE ATOP 23/ EXP 1 SHEATHING. SHEATHING S					S3.11 S3.12	CONCRETE FRAMING DETAILS CONCRETE FRAMING DETAILS	4 3	9.28.22 7.20.22
	3" NORMAL WEIGHT CONC. SLAE	AT 6"o.c. AT 3 (4500psi, /	EDGES & 12"o.c. AT FIELD.			S3.15 S3.30	SHEAR RAIL DETAILSWOOD FLOOR FRAMING DETAILS	2 3	7.11.22 7.20.22
CD-1	REINFORCE WITH CELLULOSE F	IBER AT 1.5 5/32" EXTE	5 LBS/CU. YD. ATOP WATERPROOFING RIOR GRADE PLYWOOD SHEATHING			S3.31 S3.41	WOOD FLOOR FRAMING DETAILS WOOD ROOF FRAMING DETAILS	3	7.20.22
CD-2	3" NORMAL WEIGHT CONC. SLAE REINFORCE WITH CELLULOSE F	3 (4500psi, <i>i</i>				S3.42 S3.45	WOOD ROOF FRAMING DETAILS WOOD FIREWALL DETAILS	3	7.20.22
	3" NORMAL WEIGHT CONC. SLAE	3 (4500psi, <i>i</i>	AIR-ENTRAINED)			S3.50 S3.51	PRECAST GARAGE FRAMING DETAILS PRECAST GARAGE FRAMING DETAILS	4	9.28.22
CD-3	REINFORCE WITH CELLULOSE F	IBER AT 1.	5lb/cu. yd. ATOP 2" RIGID INSULATION PODIUM SLAB. SLOPE TO DRAIN PER		LEGEND:	S3.60	CFMF DETAILS	3	7.20.22
SOG-1			OP 15 MIL VAPOR BARRIER ATOP 4" OF		$\begin{array}{c} & & \\ & & \\ \hline \end{array} \\ \begin{array}{c} & \\ & \\ \hline \end{array} \\ \begin{array}{c} & \\ & \\ & \\ \end{array} \\ \begin{array}{c} & \\ & \\ & \\ \end{array} \\ \begin{array}{c} & \\ & \\ & \\ & \\ \end{array} \\ \begin{array}{c} & \\ & \\ & \\ & \\ & \\ \end{array} \\ \begin{array}{c} & \\ & \\ & \\ & \\ & \\ \end{array} \\ \begin{array}{c} & \\ & \\ & \\ & \\ & \\ & \\ \end{array} \\ \begin{array}{c} & \\ & \\ & \\ & \\ & \\ & \\ & \\ \end{array} \\ \begin{array}{c} & \\ & \\ & \\ & \\ & \\ & \\ & \\ & \\ & \\ & $	K - DECK TYPE	PER SCHEULE ON S0.01		
	PER GEOTECH SPECIFICATIONS	5. T/SLAB EI	ATOP SUITABLE SUBGRADE MATERIAL = PER PLAN, SLOPE TO DRAIN		BASE PLATE MARK - SEE SCHEDU	LE ON SHEET	S0.04		
SOG-2	4" CONC. SLAB (4500psi, AIR-ENT REINFORCE WITH 6x6-W2.9xW2.9 LEVELING COURSE, ATOP SUITA	9 WWF ÁTC BLE SUBG	RADE MATERIAL PER GEOTECH		A#-#        BEAM OR HEADER PER SC         (A#-#u)        UPSET BEAM OR HEADER				
SOG-3	8" CONC. SLAB (4500psi, AIR-ENT BEINEORCE WITH #4 @ 12"cc EA	RAINED)			# BEARING WALL TYPE PER				
	REINFORCE WITH #4 @ 12"oc EA GRANULAR LEVELING COURSE, GEOTECH SPECIFICATIONS. T/SI	ATOP SUIT	ABLE SUBGRADE MATERIAL PER		A SHEARWALL HOLDDOWN T				
SOG-4	6" CONC. SLAB (4500psi, AIR-ENT REINFORCE WITH #4 @ 12"o.c. EA LEVELING COURSE, ATOP SUITA	ACH WAY A	ATOP 4" OF 3/4" CLEAN GRANULAR RADE MATERIAL PER GEOTECH		#/# NUMBER OF JACK STUDS/				
	SPECIFICATIONS T/SLAB EL. = PE 19/32" APA RATED, EXP 1 SHEAT	ER PLAN, S HING ATTA	CHED WITH #10 SCREWS AT		SW# SHEARWALL TYPE PER SC		30.03		
RD-1	6"o.c. AT EDGES & 12"o.c. AT FIEL FIREWALLS - REFER TO ARCH D	.D. (PROVII RAWINGS	DE FRT TREATED PLYWOOD AT FOR LOCATION AND EXTENTS)		CSW# CONCRETE SHEARWALL T				
RD-2	23/32" APA RATED T&G STURD-I SHALL BE GLUED AND NAILED W AT 4"o.c. AT EDGES & 12"o.c. AT F	// 10d RING	SHANK NAILS OR #10 SCREWS		$ \begin{array}{c}     \begin{array}{c}             3/4" \\                                    $				
NOTES:									

								STRUCTURAL SHEET LIST		
A. T d	tement of Structural Special Ir The structural design for this project is ba during construction in accordance with s	ased on cor ection 1704	npletion of special inspections of the International Building				Sheet Number	. Sheet Name	Current Revision	F
C ti B. T	Code. The owner shall employ one or me he required special inspections. The special inspector shall furnish inspe	ore qualified ction report	l special inspectors to provide s to the building official, owner,				S0.01 S0.02	GENERAL NOTES WOOD SCHEDULES & TYPICAL DETAILS	<b>4</b> 3	9 7
a C. A c	architect and structural engineer, and an All discrepancies shall be brought to the correction, then, if uncorrected, to the pro-	y other des immediate	ignated person. attention of the contractor for				S0.03 S0.04	WOOD SCHEDULES & TYPICAL DETAILS STEEL SCHEDULES	3	7
D. T s	structural engineer. The special inspector shall submit a final special inspection was, to the best of the	inspector's	knowledge, in conformance with				S0.05 S0.06	CMU DETAILS WOOD SHRINKAGE & MOVEMENT	2	7
ti E. T	he approved plans and specifications ar he building code. The following inspections and tests are r	equired with	n the frequency (continuous or				S0.10 S1.01	CONCRETE SCHEDULE         FOUNDATION PLAN	2	7
C	periodic) as defined within the referenced General Contractor shall provide notifica Inspection are ready to be inspected and	tion to the i l provide ac	nspector when items requiring cess for those inspections.				S1.02 S1.03	SECOND FLOOR FRAMING PLAN THIRD FLOOR FRAMING PLAN		
1	<ol> <li>Shop Fabrication – structural steel p shop</li> <li>Shop Fabrication – pre-engineered v</li> </ol>						S1.04	FOURTH FLOOR FRAMING PLAN		
3	<ul><li>TPI certified shop</li><li>Shop Fabrication – precast concrete shop</li></ul>	-					S1.05 S1.06	FIFTH FLOOR FRAMING PLAN         ROOF FRAMING PLAN		
4	<ol> <li>Steel Construction per Section 1705 of AISC 341 Chapter J (as reference 5. Concrete Construction per Section 1</li> </ol>	ed by AISC	360)				S1.11A S1.12A	BUILDING A FOUNDATION PLAN         BUILDING A SECOND FLOOR FRAMING PLAN	3	7
	<ul><li>a. Reinforcing Steel Placement</li><li>b. Reinforcing Steel Welding</li><li>c. Cast in Place Anchors</li></ul>						S1.13A S1.14A	BUILDING A THIRD FLOOR FRAMING PLANBUILDING A FOURTH FLOOR FRAMING PLAN	3	7
	<ul> <li>d. Post Installed Anchors</li> <li>e. Design Mix Verification</li> <li>f. Concrete Sampling and Testing</li> </ul>						S1.15A S1.16A	BUILDING A ROOF FRAMING PLANBUILDING A SHEARWALL PLAN	4 3	9 7
	<ul><li>g. Concrete Placement</li><li>h. Concrete Curing</li><li>i. Prestressed Concrete Stressing</li></ul>	and Grouti	ng				S1.21B S1.22B	BUILDING B FOUNDATION PLANBUILDING B SECOND FLOOR FRAMING PLAN	4	9 9
	j. Erection of Precast k. Verification of In-situ Concrete S Concrete	-	-				S1.23B S1.24B	BUILDING B THIRD FLOOR FRAMING PLANBUILDING B FOURTH FLOOR FRAMING PLAN	4	9 9
	I. Formwork Shape, Location and Masonry Construction per Section 1 requirements of TMS 402/ACI530/A	705.4 and t SCE5 and ⊺	he quality assurance				S1.25B S1.26B	BUILDING B ROOF FRAMING PLAN BUILDING B SHEARWALL PLAN	4	9
7 8 9	<ol> <li>Verification of Soils per Table 1705.</li> <li>Inspections and Tests of Cast-In-Pla</li> <li>Wood Lateral System (periodic)</li> </ol>	ice Deep Fo					S1.31C S1.32C	BUILDING C FOUNDATION PLAN BUILDING C SECOND FLOOR FRAMING PLAN	4 4	9
	<ul><li>a. Wood shearwalls (include sheat attachments)</li><li>b. Portal frames</li></ul>	-	bard and bottom plate				S1.33C S1.34C	BUILDING C THIRD FLOOR FRAMING PLAN         BUILDING C FOURTH FLOOR FRAMING PLAN	4	9
1	<ul> <li>c. Shear wall and portal frame hold</li> <li>d. Shear wall tension rod system</li> <li>10. Wood Gravity Framing and Placeme</li> </ul>		requency of random sampling				S1.35C	BUILDING C ROOF FRAMING PLAN	4	9
	where indicated as required) a. Heavy timber/SCL/glulam beam b. Headers and jambs (random sa	mpling)	orts (periodic)				S1.36C S1.41D	BUILDING C SHEARWALL PLAN BUILDING D FOUNDATION PLAN BUILDING D FOUNDATION PLAN	4	9
	<ul><li>c. Bearing walls (random sampling</li><li>d. Connector/hardware installation</li><li>e. Floor and roof trusses (random</li></ul>	(random sa	ampling)				S1.42Da S1.42Db		3	7
15. Cop	pyright and Disclaimer						S1.43D S1.44D		4	9
E	All drawings in the structural set (S-serie Bob D. Campbell and company, Inc. The	ese drawing	s may not be photographed,				S1.45D S1.46D	BUILDING D FIFTH PLDOFFFRANING FLAN       L         BUILDING D ROOF FRAMING PLAN	4	9 9
a ti	raced, or copies in any manner without and Company, Inc. Exception: Original he owner, architect, and general contrac	drawings m ctor for coor	ay be printed for distribution to dination, bidding, and				S1.47D S1.51E	BUILDING D SHEARWALL PLANBUILDING E FOUNDATION PLAN	3 4	7 9
с В. I,	construction. Subcontractors may not re or in any manner. , Christopher A. Beverlin, P.E., registere	ed engineer	and a representative of Bob				S1.52Ea S1.52Eb		2	7
r d	<ol> <li>Campbell and Company, Inc., do here equired by the professional registration lrawings consisting of S-series drawings</li> </ol>	laws of this s. I hereby	state for the structural design disclaim responsibility for all				S1.53E S1.54E	BUILDING E THIRD FLOOR FRAMING PLAN BUILDING E FOURTH FLOOR FRAMING PLAN	4	9
C	other drawings in the construction docun of other design professionals whose sea elsewhere in the construction document	ls and signe					S1.55E S1.56E	BUILDING E FIFTH FLOOR FRAMING PLAN         BUILDING E ROOF FRAMING PLAN	4	9
	STRU	CTUR	AL ABBREVIATIONS				S1.57E S1.61F	BUILDING E SHEARWALL PLAN       BUILDING F FOUNDATION PLAN	4	9
@ &	AT AND	FLR FS	FLOOR FAR SIDE	PERP PL	PERPENDIC PLATE	CULAR	S1.62Fa	BUILDING F PODIUM SLAB REINFORCEMENT PLAN	2	9
Ø ADTL AFF	ROUND, DIAMETER ADDITIONAL ABOVE FINISHED FLOOR	FTG FV GA	FOOTING FIELD VERIFY GAGE	PLF PJP PSF	PARTIAL JO	ER LINEAR FOOT INT PENETRATION ER SQUARE FOOT	S1.62Fb S1.63F	BUILDING F THIRD FLOOR FRAMING PLAN	2	7
ALT ARCH BLDG	ALTERNATE ARCHITECTURAL BUILDING	GALV GEN GR	GALVANIZE(D) GENERAL GRADE	PSI QTY RAD	QUANTITY RADIUS	ER SQUARE INCH	S1.64F S1.65F	BUILDING F FOURTH FLOOR FRAMING PLAN         BUILDING F FIFTH FLOOR FRAMING PLAN	2	7
B/ BM BOTT	BOTTOM OF BEAM BOTTOM	GRBM HORIZ HSS	GRADE BEAM HORIZONTAL HOLLOW STRUCTURAL SECTION	RD-# REF REINF	ROOF DECH REFERENC REINFORCE	E	S1.66F S1.67F	BUILDING F ROOF FRAMING PLAN         BUILDING F SHEARWALL PLAN	3	7
BRG C CD-#	BEARING CAMBER CONCRETE DECK TYPE	IF INFO INT	INSIDE FACE INFORMATION INTERIOR	REQD REV RLL	REQUIRED REVISION ROOF LIVE		S1.71G S1.72G	GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN	4	9 6
CJ CJP CL	CONSTRUCTION/CONTROL JOINT COMPLETE JOINT PENETRATION CENTERLINE		JOIST JOINT KIPS (1000 LBS)	RTU SC SCHED	ROOF TOP SLIP CRITIC SCHEDULE	UNIT AL	S1.73G S1.74G	GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN	1	6 6
CMU COL CONC	CONCRETE MASONRY UNIT COLUMN CONCRETE	KSF KSI LBS, #	KIPS PER SQUARE FOOT KIPS PER SQUARE INCH POUNDS	SECT SHT SIM	SECTION SHEET SIMILAR		S1.75G S1.76G	GARAGE FIFTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN	1	6
CONN CONT COORD	CONNECTION CONTINUOUS COORDINATE	Ld LL LLH	DEVELOPMENT LENGTH LIVE LOAD LONG LEG HORIZONTAL	SJ SL SOG	SAW JOINT SNOW LOA SLAB-ON-G	D	S2.00 S2.01	STAIR FRAMING - BUILDING A STAIR FRAMING - BUILDING B	3	79
COV, CVR DBL DET	COVER DOUBLE DETAIL	LLV LONG LSLT	LONG LEG VERTICAL LONGITUDINAL LONG-SLOTTED HOLE TRANSVERSE	SOG-# SPCG SPEC	SLAB-ON-G SPACING SPECIFICAT		S2.02 S2.03	STAIR & ELEVATOR FRAMING - BUILDING C STAIR & ELEVATOR FRAMING - BUILDING D	2	7
DIA DIM DL	DIAMETER DIMENSION DEAD LOAD	LTWT M MAX	LIGHTWEIGHT MOMENT FORCE MAXIMUM	SPRT SQ SS	SUPPORT SQUARE STAINLESS		S2.04 S2.05	STAIR & ELEVATOR FRAMING - BUILDING E STAIR FRAMING - BUILDING F	2	7 7
DWG EA EF	DRAWING EACH EACH FACE	MECH MFGR MIN	MECHANICAL MANUFACTURER MINIMUM	SSLT STD STIFF	STANDARD STIFFENER		S2.10	STAIR FRAMING DETAILS	2	7
EJ EL, ELEV EMBED	EXPANSION JOINT ELEVATION EMBEDMENT, EMBEDDED	MISC MSRY MTL	MISCELLANEOUS MASONRY METAL	STIR STL STRUCT		E, STRUCTURAL	S2.11 S2.20	ELEVATOR FRAMING DETAILS BALCONY FRAMING PLANS	3	7
ENGR EOD EOR	ENGINEER EDGE OF DECK ENGINEER OF RECORD	NF NS NTS	NEAR FACE NEAR SIDE NOT TO SCALE	T/ THRU TOS		EEL, TOP OF SLAB	S2.21 S2.22	BALCONY FRAMING PLANS BALCONY FRAMING PLANS	3	7
EOS EQ EQUIP	EDGE OF SLAB EQUAL EQUIPMENT	NW OC OF	NORMAL WEIGHT ON CENTER OUTSIDE FACE	TRANS TYP UNO		TED OTHERWISE	S2.30 S3.00	BALCONY FRAMING DETAILS TYPICAL FOUNDATION DETAILS	2 2	7 7
EW EXP EXT	EACH WAY EXPANSION EXTERIOR	OPNG OPP OVS	OPENING OPPOSITE OVERSIZED HOLE	V VERT W/	SHEAR FOR VERTICAL WITH	RCE	S3.01 S3.02	GARAGE FOUNDATION DETAILS PILE & PODIUM FOUNDATION DETAILS	2 4	7 9
FD-# FDN	EXISTING FLOOR DECK TYPE FOUNDATION	P PAF PC	AXIAL FORCE POWDER ACTUATED FASTENER PRECAST / PILE CAP	W/0 WF WL	WITHOUT WIDE FLAN WIND LOAD	)	S3.03 S3.04	APARTMENT FOUNDATION DETAILS FOUNDATION DETAILS	3	7 9
FF FIN	FAR FACE FINISH	PCF PEMB	POUNDS PER CUBIC FOOT PRE-ENGINEERED METAL BUILDING	WP WWF	WORK POIN WELDED W		S3.05 S3.06	FOUNDATION DETAILS FOUNDATION DETAILS	4 3	9 7
STR	UCTURAL DECK &	SLAB	SCHEDULE				S3.07 S3.08	FOUNDATION DETAILS FOUNDATION DETAILS	3 3	7
MARK FD-1	DESCRIPTION 1" TO 1 1/4" GYPCRETE ATOP 23						S3.11 S3.12	CONCRETE FRAMING DETAILS CONCRETE FRAMING DETAILS	4 3	9 7
• • •	SHANK NAILS OR #10 SCREWS	AT 6"o.c. A					S3.15 S3.30	SHEAR RAIL DETAILS WOOD FLOOR FRAMING DETAILS	2	7
CD-1	MEMBRANE (RE: ARCH.) ATOP	FIBER AT 1 15/32" EXT	, AIR-ENTRAINED) .5 LBS/CU. YD. ATOP WATERPROOFING ERIOR GRADE PLYWOOD SHEATHING				S3.31 S3.41	WOOD FLOOR FRAMING DETAILS WOOD ROOF FRAMING DETAILS	3	7
CD-2	(SLOPE PER ARCH.) RE: NOTE S 3" NORMAL WEIGHT CONC. SLA	AB (4500psi					S3.42 S3.45	WOOD ROOF FRAMING DETAILS WOOD FIREWALL DETAILS		7
	REINFORCE WITH CELLULOSE STAIR LANDING 3" NORMAL WEIGHT CONC. SLA						S3.50	PRECAST GARAGE FRAMING DETAILS		
CD-3	REINFORCE WITH CELLULOSE	FIBER AT 1	.5lb/cu. yd. ATOP 2" RIGID INSULATION PODIUM SLAB. SLOPE TO DRAIN PER		LEGEN	).	S3.51 S3.60	PRECAST GARAGE FRAMING DETAILS CFMF DETAILS	4	9 7
SOG-1	4" CONC. SLAB (4000psi)		OP 15 MIL VAPOR BARRIER ATOP 4" OF		<b>→</b>	SPAN DIRECTION OF DECI	K - DECK TYPI	E PER SCHEULE ON S0.01		
	3/4" CLEAN GRANULAR LEVELIN	IG COURS	E ATOP SUITABLE SUBGRADE MATERIAL EL. = PER PLAN, SLOPE TO DRAIN		$\sim$	<sup>3x6x1/4</sup> COLUMN SIZE E PLATE MARK - SEE SCHEDU	JLE ON SHEET	T S0.04		
SOG-2		.9 WWF Á1	OP 4" OF 3/4" CLEAN GRANULAR GRADE MATERIAL PER GEOTECH		A#-#	BEAM OR HEADER PER SC				
. –	8" CONC. SLAB (4500psi, AIR-EN	PER PLAN,			(A#-#u)	UPSET BEAM OR HEADER BEARING WALL TYPE PER				
SOG-3	REINFORCE WITH #4 @ 12"oc E	ACH WAÝ I , ATOP SU	BOTTOM ATOP 4" OF 3/4" CLEAN ITABLE SUBGRADE MATERIAL PER PER PLAN. SLOPE TO DRAIN			SHEARWALL HOLDDOWN	TYPE PER SC	HEDULE ON S0.03		
SOG-4	6" CONC. SLAB (4500psi, AIR-EN REINFORCE WITH #4 @ 12"o.c. I	TRAINED) EACH WAY	ATOP 4" OF 3/4" CLEAN GRANULAR		#	NUMBER OF WALL STUDS				
<b></b>	LEVELING COURSE, ATOP SUIT SPECIFICATIONS T/SLAB EL. = F	ABLE SUB PER PLAN,	GRADE MATERIAL PER GEOTECH SLOPE TO DRAIN		A	PLAN NOTE PER SCHEDUL	LE ON S0.02			
RD-1		LD. (PROV	ACHED WITH #10 SCREWS AT IDE FRT TREATED PLYWOOD AT FOR LOCATION AND EXTENTS)		sw# csw#	SHEARWALL TYPE PER SC CONCRETE SHEARWALL T				
RD-2		N/ 10d RIN	G SHANK NAILS OR #10 SCREWS		3/4"	AMOUNT OF UPWARD POS				
	AT 4"o.c. AT EDGES & 12"o.c. AT FLOOR DECK TYPE.	FIELU WIT	II ALL EUGES BLUGKED		8      A	PILE CAP SIZE PER SCHEE SHEAR RAIL TYPE PER SC				
2. CD = C 3. SOG = S	CONCRETE DECK TYP. SLAB-ON-GRADE TYP. ROOF DECK TYP.									

- SOG = SLAB-ON-GRADE TYP. 4. RD = ROOF DECK TYP.
- 5. PROVIDE 1" DEEP TOOLED CONTROL JOINT (TRANSVERSE DIRECTION ) @ MID-SPAN OF BALCONY (8'-0" MAX SPACING) FILL JOINT w/ SEALANT.

= 20 psf or 14 psf plus Drift = 10 psf = 5 psf combination with wind uplift shall be 5psf for each chord.

= 15 psf (TPO Roof)

= 20 psf (Plus Rooftop Equipment)



SHEET NUMBER

# GENERAL NOTES

SHEET TITLE

BELL & CO. Since 1957	816.531.4144 www.bdc-engrs.com
BOB D. CAMPBELI Structural Engineers S	4338 Belleview Ave. 816.531.4144 Kansas City, MO 64111 www.bdc-engrs.com

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS



PROJECT TEAM

REGISTRATION

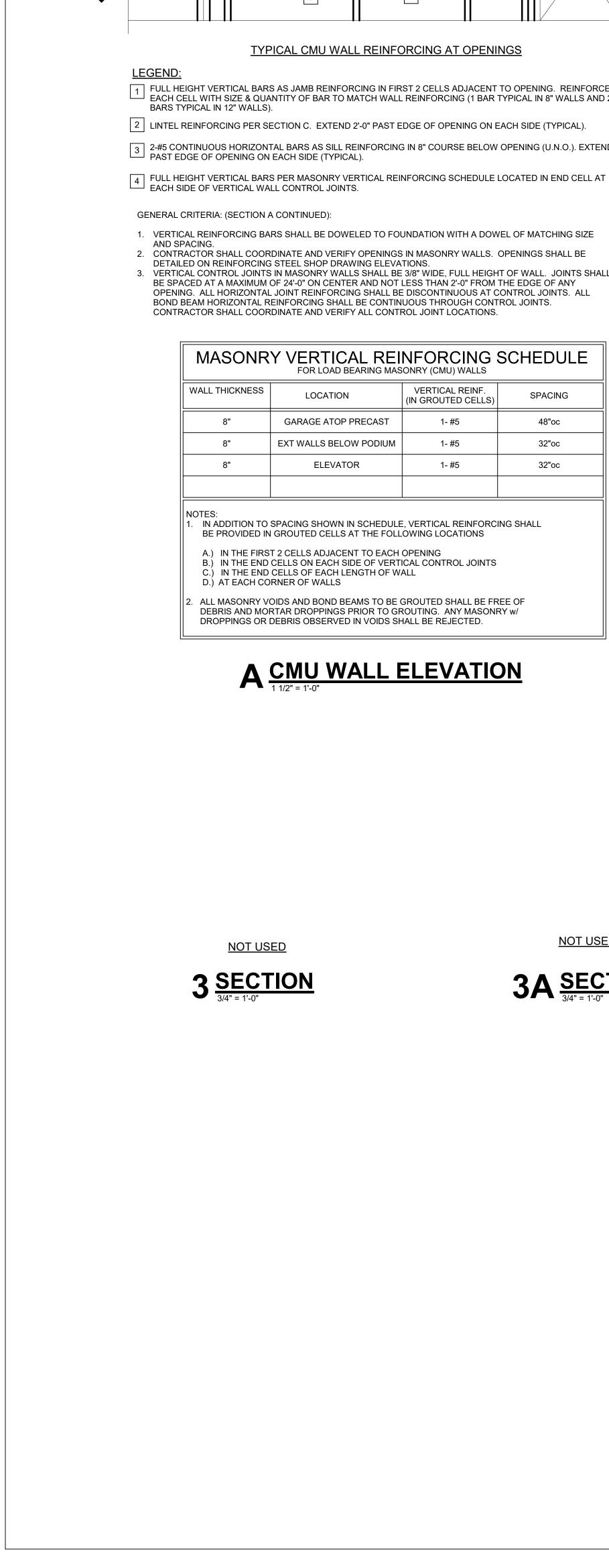
Project No.:		18017,19050.07,19050.08			
Date	e:	10.18.24			
Issu	ed For:	GARAGE PERMIT			
		REVISIONS			
No.	Date	Description			
1	6.08.22	Permit Response			
2	7.11.22	ADDENDUM 1			
$\frac{1}{2}$ $\frac{3}{4}$	7.20.22	ADDENDUM 2			
4	9.28.22	ASI 1			

PARAG	ON	STAR
NORTH	VIL	LAGE

3200 NW PARAGON PKWY

LEE'S SUMMIT, MO 64081

# paragon



 $A \underline{CMU WALL ELEVATION}_{1.1/2" = 1'-0"}$ 

MASONRY VERTICAL REINFORCING SCHEDULE				
WALL THICKNESS	LOCATION	VERTICAL REINF. (IN GROUTED CELLS)	SPACING	
8"	GARAGE ATOP PRECAST	1- #5	48"oc	
8"	EXT WALLS BELOW PODIUM	1- #5	32"oc	

1- #5

BE SPACED AT A MAXIMUM OF 24'-0" ON CENTER AND NOT LESS THAN 2'-0" FROM THE EDGE OF ANY OPENING. ALL HORIZONTAL JOINT REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS. ALL BOND BEAM HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL JOINTS. CONTRACTOR SHALL COORDINATE AND VERIFY ALL CONTROL JOINT LOCATIONS.

DETAILED ON REINFORCING STEEL SHOP DRAWING ELEVATIONS. VERTICAL CONTROL JOINTS IN MASONRY WALLS SHALL BE 3/8" WIDE, FULL HEIGHT OF WALL. JOINTS SHALL

1. VERTICAL REINFORCING BARS SHALL BE DOWELED TO FOUNDATION WITH A DOWEL OF MATCHING SIZE

ELEVATOR

S0.05

3

VERT. WALL -

CONTROL JT.

(FULL HEIGHT)

2'-0" MIN.

4

в

S0.05

AND SPACING.

GENERAL CRITERIA: (SECTION A CONTINUED):

TYPICAL CMU WALL REINFORCING AT OPENINGS

- HORIZONTAL BOND BEAM

**REINFORCING CONTINUOUS** 

THROUGH CONTROL JOINTS

2

TOP OF WALL

S0.05

LINE WHERE

INDICATED

1 FULL HEIGHT VERTICAL BARS AS JAMB REINFORCING IN FIRST 2 CELLS ADJACENT TO OPENING. REINFORCE EACH CELL WITH SIZE & QUANTITY OF BAR TO MATCH WALL REINFORCING (1 BAR TYPICAL IN 8" WALLS AND 2 BARS TYPICAL IN 12" WALLS). 2 LINTEL REINFORCING PER SECTION C. EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL). 3 2-#5 CONTINUOUS HORIZONTAL BARS AS SILL REINFORCING IN 8" COURSE BELOW OPENING (U.N.O.). EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).

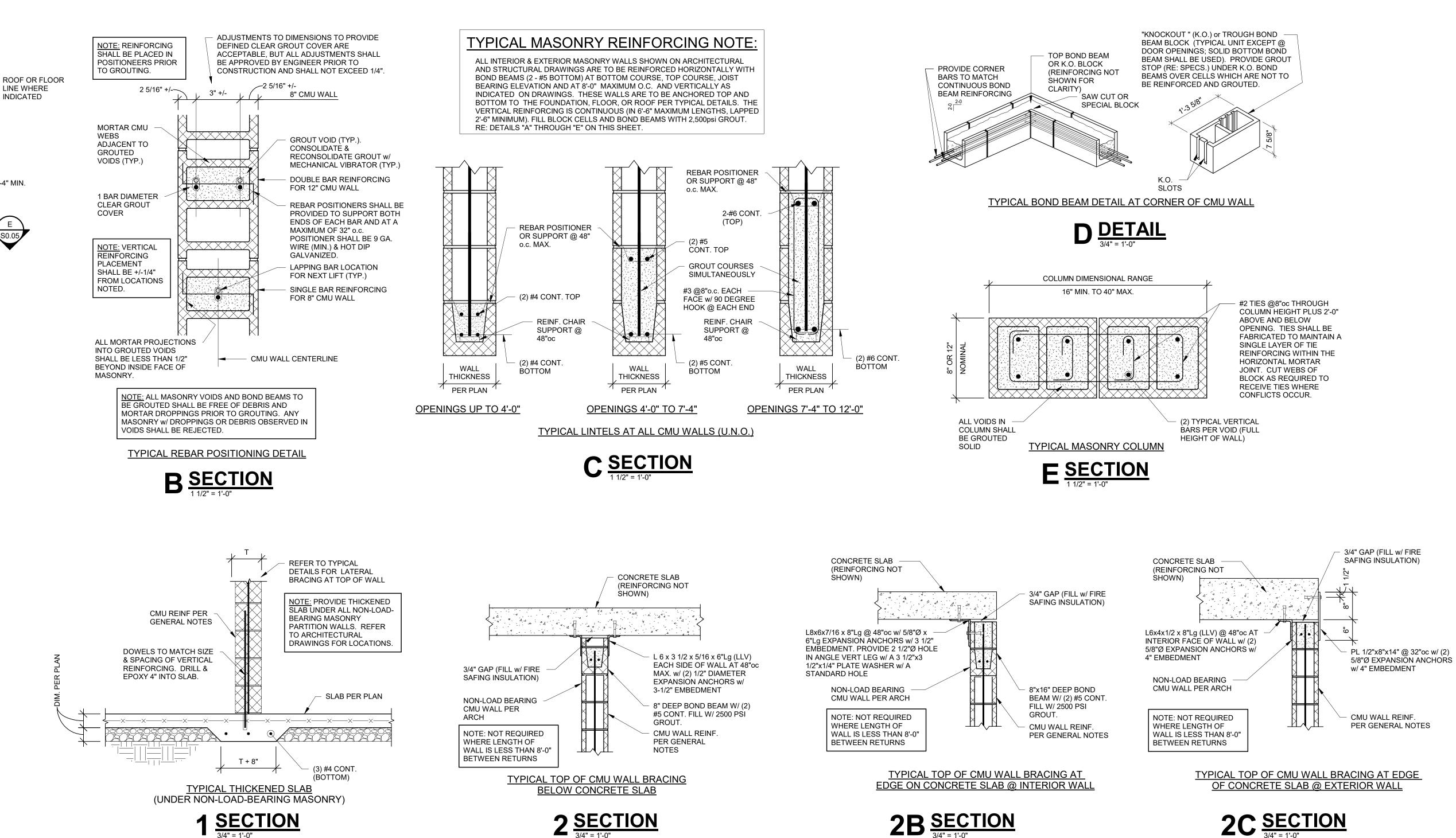


<u>NOT USED</u>

32"oc



NOT USED





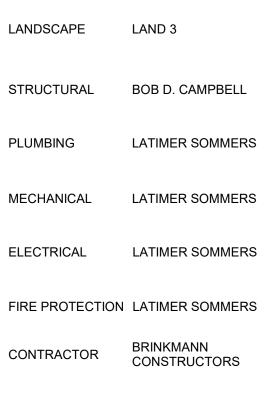






SHEET TITLE





# CHRISTOPHER A BEVERLIN NUMBER PE-2017012583 10.18.24

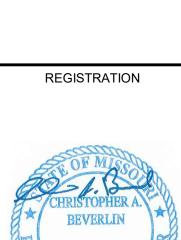
PROJECT TEAM

FINKLE+WILLIAMS ARCHITECTURE

**GBA ENGINEERS** 

ARCHITECT

CIVIL



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081				
Project No.:	18017,19050.07,19050.08			
Date:	10.18.24			
Issued For:	GARAGE PERMIT			
REVISIONS				
No. Date				
2 7.11.22	ADDENDUM 1			

60

paragon star

PARAGON STAR

NORTH VILLAGE

		SLAB NOTES
BEAM SCHEDULES	BEAM SCHEDULES	1. SEE GENERAL NOTES (STRUCTURAL) ON SHEET S001. 2. PODIUM SLAB IS 15" THICK REINFORCED WITH A CONTINUOUS (60" LAP AT COLUMN CENTERLINE OF COLUMN STRIPS AND 24" LAP AT COLUMN
MARK       OR       OR       QUANTITY       SIZE       TOTAL LENGTH       MK       TOP       PLACED       BOT.       REMARKS       NO.       SHAPE       SPACING         Image: Section in the section of the s	MARK       D       QUANTITY       SIZE       TOTAL LENGTH       MK       TOP       PLACED       BOT.       REMARKS       NO.       SHAPE       SPACING	CENTERLINE OF MID-STRIPS) BOTTOM MAT OF #6 @ 12" EACH WAY. SEE PLAN FOR BOTTOM MAT EXTENDING EAST/WEST THAT SHALL BE SUPPORTED ON 1" SLAB BOLSTERS AT 4'-0"o.c. 3. TOP REINFORCING BARS PLACING SEQUENCE:
Image: Section of the section of t	Image: Constraint of the straint o	14A 5 19'-9" TOTAL LENGTH OF BAR IN FEET AND INCHES SIZE OF BAR AND LOCATION IN SLAB AS NOTED BELOW TOTAL NUMBER OF EXTRA BARS IN STRIP DEFINED ON PLAN
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	B32     24"     36"     2     #7     20-0     •       4     #7     22-0     •     •     ALT HOOK DIRECTION     #4     @ 12"oc	"A" #6 EXTRA BOTTOM BARS WITH 1" CLEAR COVER BOTTOM. (PLACE WITH 1" CLEAR COVER BOTTOM MAT BARS.)
B2     36"     36"     6     #8     12-0     •     •     •       6     #8     27-0     •     •     •     •     #4     •     10 @ 4 60; 12 @ 8"oc LEFT END RMDR @ 12"oc	B33 24" 36" 4 #7 20-0 6 e #4 20-0 20-0 20-0 20-0 20-0 20-0 20-0 20-	"B" #6 EXTRA BOTTOM BARS WITH 1 3/4" CLEAR COVER BOTTOM. (PLACE WITH 1 3/4" CLEAR COVER BOTTOM MAT BARS.) PLACE ON TOP OF PERPENDICULAR (1" CLEAR COVER) BOTTOM MAT AND "A" BARS. "C" #7 TOP BARS WITH 1 7/8" CLEAR COVER WHERE TWO LAYERS OF BARS OCCUR AND 1" CLEAR COVER WHERE ONE LAYER OF BARS OCCUR
B3 48" 36" 48" 36" 6 #8 32-0 • • • • • PROJECT 4'-0" PAST GRID F.2 8 #8 36-0 • • • • 6" CLR FROM BOTT (2) #4 [0] @ 10"oc	B34     24"     36"     4     #7     29-0     •     ALT HOOK DIRECTION       4     #8     26-0     •     •     •     #4     #6       4     #8     26-0     •     •     ALT HOOK DIRECTION     #4       6     4     #8     27-0     •     •     ALT HOOK DIRECTION	ON IHC @4'-0" o.c. AND #5 SUPPORT BARS @4'-0"o.c. "D" #7 TOP BARS WITH 1" CLEAR COVER TOP. PLACE ON TOP OF "C" BARS WHERE THEY OCCUR OR OTHERWISE PLACE ON IHC AT 4'-0"o.c.
B4     48"     36"     6     #8     40-0     •     •     •     •     PROJECT 4'-0" PAST 12" WALL       12     #8     24-0     •     •     •     •     •     •     •       8     #8     14-0     •     •     •     •     •     •     •		AND #5 SUPPORT BARS AT 4'-0" o.c. "E" #6 TOP BARS WITH 1" CLEAR COVER WHERE ONE LAYER OF BARS OCCUR ON IHC AT 4'-0" o.c. AND #5 SUPPORT BARS AT 4'-0"o.c. "F" #5 TOP BARS WITH 1 7/8" CLEAR COVER WHERE TWO LAYERS OF BARS
B5 36" 36" 4 #8 32-0 • PROJECT 4'-0" PAST GRID F.2 #4 #8 36-0 • 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	CONCRETE SHEARWALL SCHEDULE         TYPE       THICKNESS       VERTICAL REINFORCEMENT       HORIZONTAL REINFORCEMENT       COLUMN @ END OF WALL FOR PILE DESIGN (kif)       UPLIFT AT EACH END OF WALL FOR PILE DESIGN (kifs)	OCCUR AND 1" CLEAR COVER WHERE ONE LAYER OF BARS OCCUR ON IHC AT 4'-0" o.c. AND #5 SUPPORT BARS AT 4'-0" o.c. "G" #5 TOP BARS WITH 1" CLEAR COVER TOP, PLACE ON TOP OF "F" BARS WHERE THEY OCCUR, OTHERWISE PLACE ON IHC AT 4'-0" o.c. AND #5 SUPPORT BARS AT 4'-0" o.c.
B6       I	THE     THECKNESS     REINFORCEMENT     REINFORCEMENT     END OF WALL     FOR PILE DESIGN (kif)     FOR PILE DESIGN (kips)       CSW1     8"     #5 @ 12"oc CTR IN WALL     #5 @ 12"oc CTR IN WALL     8"x32"     6"x32"     6"x32"	4. REINFORCING SHALL BE SPLAYED AROUND OPENINGS LESS THAN 18" WIDE. REINFORCING SHALL BE CUT AT OPENINGS GREATER THAN 18" WIDE WITH EQUAL CONTINUOUS BARS ADDED 0NE-HALF EACH SIDE OF OPENING.
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	CSW2         8"         #5 @ 12"oc CTR IN WALL         #5 @ 12"oc CTR IN WALL         8"x32"	PROVIDE REINFORCING PER GENERAL NOTE 7F AT ALL OPENINGS LARGER THAN 8". 5. STRIP LINES ARE LOCATED AT 1/4 POINTS BETWEEN COLUMN CENTERLINES UNLESS NOTED ON PLAN OTHERWISE. 6. SEE DETAIL 2/S3.10 FOR PLACING PATTERN FOR TOP REINFORCING BARS
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	CSW3         8"         #5 @ 12"oc CTR IN WALL         #5 @ 12"oc CTR IN WALL         8"x32"	OVER INTERIOR COLUMN AS NOTED. 7. TOP BARS SHOWN STAGGERED ON PLAN SHALL BE STAGGERED WHEN PLACED; THE END OF EVERY OTHER BAR TO BE PLACED AT RELATIVE STRIP LINE, UNLESS NOTED ON PLAN.
B8     36"     32"     2     #8     24-0     •     •       4     #8     36-0     •     •     •     #4     @ 12"oc	CSW4         8"         #5 @ 12"oc CTR IN WALL         #5 @ 12"oc CTR IN WALL         8"x32"	8. BOTTOM BARS ARE SHOWN THUS TOP BARS ARE SHOWN THUS TOP BARS SHOWN ON PLAN THUS
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	CSW6     8"     #5 @ 12"oc CTR IN WALL     #5 @ 12"oc CTR IN WALL     8"x32"	9. UNLESS SHOWN ON "S" SERIES DRAWINGS, NO HOLES LARGER THAN TEN INCH DIAMETER SHALL BE PLACED THROUGH SLAB. NOT MORE THAN ONE, SIX TO EIGHT INCH DIAMETER HOLES, OR TWO FOUR INCH DIAMETER HOLES, OR THREE TWO INCH DIAMETER OR SMALLER HOLES SHALL BE PLACE
B10 36" 32" 4 #8 40-0 •	CSW6       8"       #5 @ 12"oc CTR IN WALL       #5 @ 12"oc CTR IN WALL       8"x32"         CSW7       8"       #5 @ 12"oc CTR IN WALL       #5 @ 12"oc CTR IN WALL       8"x32"	WITHIN 20" OF THE FACE OF THE COLUMNS. 10. CAMBER ALL SPANS BETWEEN 16'-0" AND 24'-0" (CENTERLINE TO CENTERLINE OF SUPPORTS) FOR L/600 MINIMUM AT MIDSPAN (WITH L = SPAN IN INCHES) (I.E., 3/8 AT MIDSPAN FOR 18'-0" SPAN. CAMBER ALL SPANS LONGER
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	CSW8         12"         #5 @ 12" oc EA FACE         #6 @ 12" oc EA FACE         12" x24"	(I.E., 3/8 AT MIDSPAN FOR 18-0 SPAN. CAMBER ALL SPANS LONGER THAN 24'-0" FOR L/480 (I.E., 3/4" AT MIDSPAN FOR 30'-0" SPAN.) 11. AT TERMINATION OF COLUMN STRIP AT COLUMN, WALL, BEAM, PROVIDE 90° STANDARD ACI HOOK EACH END AT (4) BOTTOM BARS NEAREST TO COLUMN CENTERLINE PER 6/S3.10
B11 36" 32" 2 #8 22-0 • • • @ 12"oc	CSW9         8"         #5 @ 12"oc EA FACE         #6 @ 12"oc EA FACE         12"x24"	
B12 36" 32" 2 #8 12-0 6 #8 16-0 4 6 @ 8"oc	CSW10         12"         #5 @ 12"oc EA FACE         #6 @ 12"oc EA FACE         12"x24"	BEAM SCHEDULE PLACING NOTES
B13 36" 32" 6 #8 28-0 • • • • • • • • • • • • • • • • • • •	CSW11         8"         #5 @ 12"oc CTR IN WALL         #5 @ 12"oc CTR IN WALL         8"x32"	<ol> <li>See General Notes (Structural) on sheet S0.01.</li> <li>Orientation of beams in schedule are as seen from the bottom or right of the plan sheet.</li> <li>Center group of top bars indicated thus in "placed" schedule</li> </ol>
B14       48"       32"       12       #8       27-0       •       •       ALT HOOK DIRECTION       • <t< td=""><td>CSW12         8"         #5 @ 12"oc CTR IN WALL         #5 @ 12"oc CTR IN WALL         8"x32"</td><td>on centerline of support. Stagger bars 5% of longer span. Bar length = 55% of longer span. 4. Top bars scheduled thus</td></t<>	CSW12         8"         #5 @ 12"oc CTR IN WALL         #5 @ 12"oc CTR IN WALL         8"x32"	on centerline of support. Stagger bars 5% of longer span. Bar length = 55% of longer span. 4. Top bars scheduled thus
18     #8     32-0     ▲     ▲     ALT HOOK DIRECTION     ▲	CSW13       12"       #5 @ 12" oc EA FACE       #6 @ 12" oc EA FACE       12" x24"         CSW14       12"       #5 @ 12" oc EA FACE       #6 @ 12" oc EA FACE       12" x24"	<ul> <li>5. All lapped top bars shall have a minimum of lap of 2'-6" or 48 bar diameters. Perimeter beams shall have two bars lapped a minimum of 66 bar diamters.</li> <li>6. Bottom bars indicated thus + + + + = in "placed schedule to have</li> </ul>
DIO     14     #8     32-0     Image: Constraint of the second secon	CSW15         8"         #5 @ 12"oc CTR IN WALL         #5 @ 12"oc CTR IN WALL         8"x32"	scheduled bars extend 12" past centerline of support each end and a minimum 2 bars with a 66 bar diameter lap. 7. Bottom bars scheduled thus     ++     extend to within 1/8 point of span.
B16 B16R     40"     4     #8     16-0     •     •     •     Image: Constraint of the second se	CSW16         8"         #5 @ 12"oc CTR IN WALL         #5 @ 12"oc CTR IN WALL         8"x32"	<ul> <li>8. Start stirrups 2" from face of support each end unless noted.</li> <li>9. All bars shown thus to have standard ACI hook. Extend to within 2" of exterior face.</li> <li>10. No holes, sleeves, or conduit larger than 1" diameter round shall be put through hearma without written outherization from the engineer.</li> </ul>
B17     36"     40"     8     #8     36-0     •     •     1/2 AT EA COLUMN       1/2 AT EA COLUMN     4	CSW17         8"         #5 @ 12"oc CTR IN WALL         #5 @ 12"oc CTR IN WALL         8"x32"	through beams without written authorization from the engineer. All conduit shall be PVC (non metalic). 11. Splice length at bars of different sizes shall be based on the larger of the two bar diameters.
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	CSW18         12"         #5 @ 12"oc EA FACE         #6 @ 12"oc EA FACE         12"x24"	BEAM PLACEMENT SCHEDULE NOTE: ALL SIMILAR CONDITIONS TO BE PLACED PER THE PLACING SCHEDULE.
B18 36" 40" 40" 40" 40" 40" 40" 40" 40" 40" 40	CSW19         12"         #5 @ 12"oc EA FACE         #6 @ 12"oc EA FACE         12"x24"	
2     #8     20-0     •       4     #8     24-0     •       4     #8     24-0       4     #8     24-0	CSW20       8"       #5 @ 12"oc CTR IN WALL       #5 @ 12"oc CTR IN WALL       8"x32"         CSW21       12"       #5 @ 12"oc EA FACE       #6 @ 12"oc EA FACE       12"x24"	BM SCHEDULE NOTE #     1/4 POINT ADJ SPAN       #3 TOP BEAM     5% SPAN
B19 36" 40" 40" 44 #5 17-0 1/2 EA FACE @ 10"oc FROM BOTT #4 @ 12"oc @ 10"oc FROM BOTT #4 #8 24-0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	CSW22         12"         #5 @ 12"oc EA FACE         #6 @ 12"oc EA FACE         12"x24"	#4 TOP BEAM     2" /*     5% OF SPAN       #5 TOP BEAM     PER NOTE
B20 24" 40" 4 #8 40-0 • 1/2 EA FACE @ 10"oc FROM BOTT #4 0 @ 12"oc @ 12"oc	CSW23         12"         #5 @ 12" oc EA FACE         #6 @ 12" oc EA FACE         12" x24"	#6     BOTTOM BEAM     PER NOTE       #7     BOTTOM BEAM
6     #8     34-0     •     ALT HOOK DIRECTION       10     #8     36-0     •     •     •	NOTES: 1) PROVIDE (2)#7 CONT TOP AND BOTTOM OF EACH WALL WITHIN 4" OF THE TOP AND BOTTOM. SPLICE BOTTOM BARS 5'-0" AND TOP BARS 6'-0" WITH 90 DEG HOOKS AT DISTONT ENDS	TYPICAL BOTTOM     2" // // // // // // // // // // // // //
B21     36"     40"     4     #5     21-0     1/2 EA FACE @ 10"oc FROM BOTT     #4     @ 12"oc       4     #8     28-0     •     •     •     •     •	<ol> <li>VERTICAL FOUNDATION DOWELS TO MATCH VERTICAL WALL REINFORCEMENT SIZE AND SPACING WITH 48 BAR Ø LAP INTO WALL AND 90 DEG HOOK INTO BOTTOM OF FOOTING BELOW.</li> <li>TERMINATE TOP OF VERTICAL BARS WITH 90 DEG HOOK INTO TOP OF SLAB WITH 3" OF CLEAR COVER.</li> </ol>	AT CANTILEVER
B22 36" 40" 40" 40" 40" 40" 40" 40" 40" 40" 40		2" EDGE 30% OF SPAN OR CANT DIST. OF CANT 1 WHICHEVER IS GREATER
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	CONCRETE COLUMN SCHEDULE 1) PROVIDE (4) SETS OF TIES AT 3"oc TOP & BOTTOM OF EACH COLUMN 2) ALL COLUMNS TO CENTER ON GRIDLINE AND PIER/FOUNDATION U.N.O.	
B23 36" 40" 12 4 4 4 5 12-0 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	COLUMN SIZE       REINFORCEMENT       COLUMN SIZE       REINFORCEMENT       3)       PROVIDE VERTICAL FOUNDATION DOWELS MATCH SIZE AND QUANITY OF VERTICAL REINFORCEMENT WITH 48 BAR Ø LAP INTO COLUMN AND 90         COLUMN SIZE       COLUMN SIZE       REINFORCEMENT       3)       PROVIDE VERTICAL REINFORCEMENT WITH 48 BAR Ø LAP INTO COLUMN AND 90         COLUMN SIZE       COLUMN SIZE       COLUMN SIZE       COLUMN SIZE       COLUMN SIZE         COLUMN SIZE       COLUMN SIZE       REINFORCEMENT       COLUMN SIZE       COLUMN SIZE         COLUMN SIZE       COLUMN SIZE       REINFORCEMENT       COLUMN SIZE       COLUMN SIZE         COLUMN SIZE       COLUMN SIZE       REINFORCEMENT       COLUMN SIZE       COLUMN SIZE         COLUMN SIZE       COLUMN SIZE       REINFORCEMENT       COLUMN SIZE       COLUMN SIZE	
$B24  36"  40"  \boxed{2}{4}  \frac{36}{4}  \frac{2}{4}  \frac{36}{4}  \frac{2}{4}  \frac{36}{4}  \frac{2}{4}  \frac{36}{4}  \frac{2}{4}  \frac{36}{4}  \frac{36}{4$	8X32     (2) #3 TIES @ 8"oc     16X32     (2) #3 TIES @ 16"oc     LAP INTO COLUMN AND 90 DEG HOOK INTO TOP OF SLAB ABOVE.       40Y04     (8) #7 VERTICAL     04Y04     (12) #8 VERTICAL     LAP INTO COLUMN AND 90 DEG HOOK INTO TOP OF SLAB ABOVE.	
DO5 36" 40" 7 6 4 #8 36-0 • 1/2 EA FACE @ 10"oc FROM BOTT #4 0 12"oc	12X24     (2) #3 TIES @ 12"oc     24X24     (3) #3 TIES @ 16"oc       14X34     (10) #8 VERTICAL     24Ø     (10) #8 VERTICAL	
4     #8     24-0     •     •     •       8     #8     38-0     •     •     1/2 AT EA COLUMN		ELOPMENT LENGTH AND LAP SPLICE SCHEDULE
B26     36"     40"     4     #5     40-0     1/2 EA FACE @ 10"oc FROM BOTT       4     #8     40-0     4	CONCRETE STRENGTH = 5000 psi	CONCRETE STRENGTH = 4000 psi CONCRETE STRENGTH = 3500 psi
B27 36" 40"	LENGTH OR CLASS B LAP	CASE DEVELOPMENT LENGTH OR CLASS & LAD CLASS & LAD CLASS & LAD CLASS & LAD CLASS & LAD
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	VERT REINF (2) #3 TIES	CLASS A LAP CLASS A LAP TOP OTHER TOP OTHER RADS DADS DADS DADS DADS DADS DADS DADS
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	16"x32" COLUMN       3/4" CHAMFER       Image: Size of the state of the s	AR ZEBARSBARSBARSBARSBARSBARSBARSBARSBARSBARSBARSBARS#324242424#324242624
B29     36"     40"     4     #8     18-0     •     •     1/2 EA FACE @ 10"oc FROM BOTT     #4     [20]     (20) @ 4"oc LEFT END RMDR @12"oc	Image: West relinf     Image: West r	#4       25       24       33       25       #4       27       24       35       27         #5       31       24       41       31       #5       33       26       43       33         #0       07       07       04       07       04       04       04       04       04
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	VERT REINF       14"x34" COLUMN       #6       34       26       43       34         (2) #3 TIES       (2) #3 TIES       3/4" CHAMFER (TYP)       #7       49       38       63       49         8"x32" & 12"x24" COLUMN       (TYP)       #8       56       43       72       56	#6       37       29       49       37       #6       40       31       52       40         #7       54       42       71       54       #7       58       45       75       58         #0       62       48       62       49       62       66       51       96       66
B30 48" 40" 40" 40" 40" 40" 40" 40" 40" 40" 40	1/2" CLR     1/2" CLR       1 1/2" CLR	#8       62       48       81       62       #8       66       51       86       66         #9       70       54       91       70       #9       75       58       97       75         #10       79       61       102       79       #10       84       65       109       84
$P21 \xrightarrow{48"} 40" \xrightarrow{10} \frac{10}{4} \frac{10}{4$	CVR (TYP) +10 -1	#10       79       61       102       79       #10       84       65       109       84         #11       87       67       113       87       #11       93       72       121       93
DJI     10     #8     32-0     •     6" CLR FROM BOTT     •     •     6" CLR FROM BOTT       16     #8     36-0     •     •     ALT HOOK DIRECTION     • <td>#3 TIE VERT REINF VERT REINF (3) #3 TIES NOTES: 1. UNLESS SPECIFICALLY INDICATED OTHERWISE, USE THE MINIMUM LENGTH FOR A APPLICABLE FACTOR(S) LISTED BELOW.</td> <td>A CLASS B LAP SPLICE OR THE MINIMUM DEVELOPMENT LENGTH INDICATED IN THE TABLES ABOVE MULTIPLIED BY THE</td>	#3 TIE VERT REINF VERT REINF (3) #3 TIES NOTES: 1. UNLESS SPECIFICALLY INDICATED OTHERWISE, USE THE MINIMUM LENGTH FOR A APPLICABLE FACTOR(S) LISTED BELOW.	A CLASS B LAP SPLICE OR THE MINIMUM DEVELOPMENT LENGTH INDICATED IN THE TABLES ABOVE MULTIPLIED BY THE

**24"Ø COLUMN 24"x24" COLUMN 7 CONCRETE COLUMN DETAILS 3**/4" = 1'-0"

APPLICABLE FACTOR(S) LISTED BELOW.
 WHERE THE CLEAR SPACING BETWEEN BARS LAP SPLICED OR EMBEDDED AT ANY SECTION IS LESS THAN 2 BAR DIAMETERS, OR WHERE THE BAR COVER IS LESS THAN OR EQUAL TO THE BAR DIAMETER. INCREASE THE INDICATED BAR SPLICE OR DEVELOPMENT LENGTH BY 50%.
 TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
 MECHANICAL COUPLERS MAY BE SUBSTITUTED FOR TENSION LAP SPLICED BARS PROVIDED THAT THEY MEET THE REQUIREMENTS OF ACI 318-11, 12.14.
 AT LOCATIONS WHERE REINFORCING WITHIN A STRUCTURAL ELEMENT WILL BE SPLICED, ALTERNATING SPLICES SHALL BE STAGGERED A MINIMUM OF THE CLASS B SPLICE LENGTH UNLESS INDICATED OTHERWISE.



CONCRETE SCHEDULE

SHEET TITLE

DBELL & CO.	Ø	www.bdc-engrs.com
BOB D. CAMPBEI	4338 Belleview Ave.	Kansas City, MO 64111

LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

STOF MISS PHER A. BEVERLIN NUMBER 7-201701

PROJECT TEAM

ARCHITECT FINKLE+WILLIAMS ARCHITECTURE

GBA ENGINEERS

CIVIL

REGISTRATION

\_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_ \_\_\_\_\_ \_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ 

PARAGON STAR NORTH VILLAGE 3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08

REVISIONS

Description

Issued For: GARAGE PERMIT

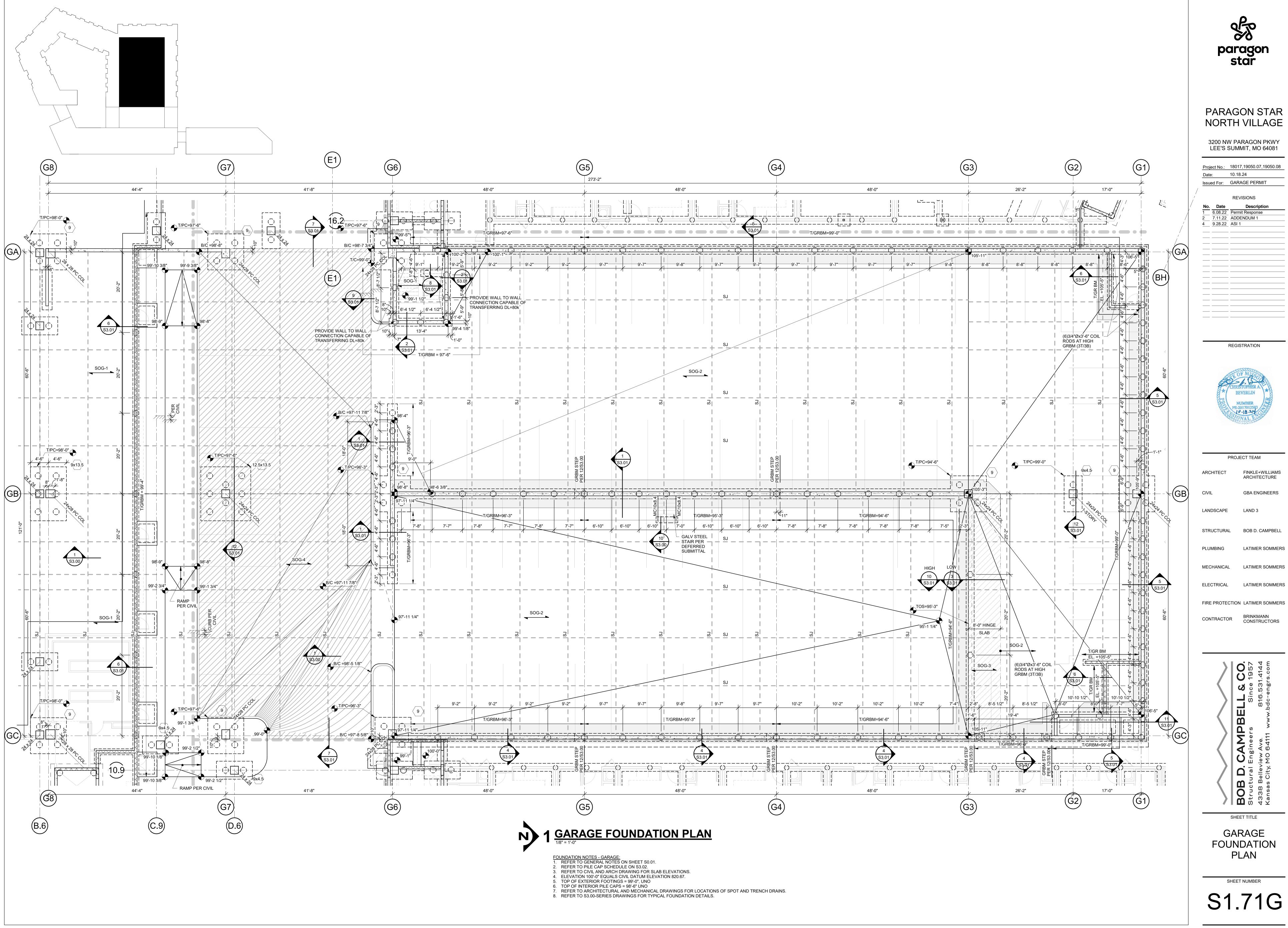
2 7.11.22 ADDENDUM 1

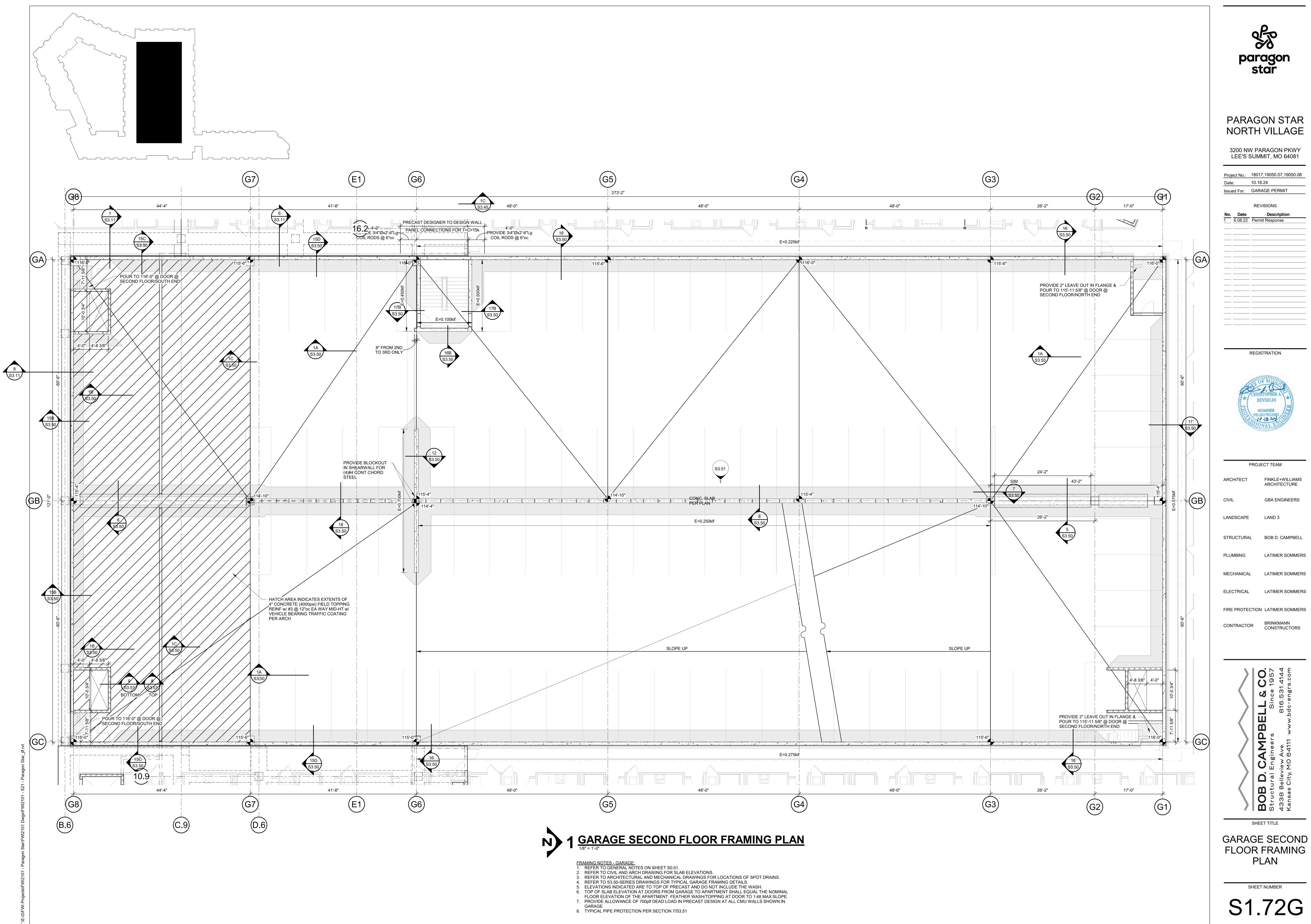
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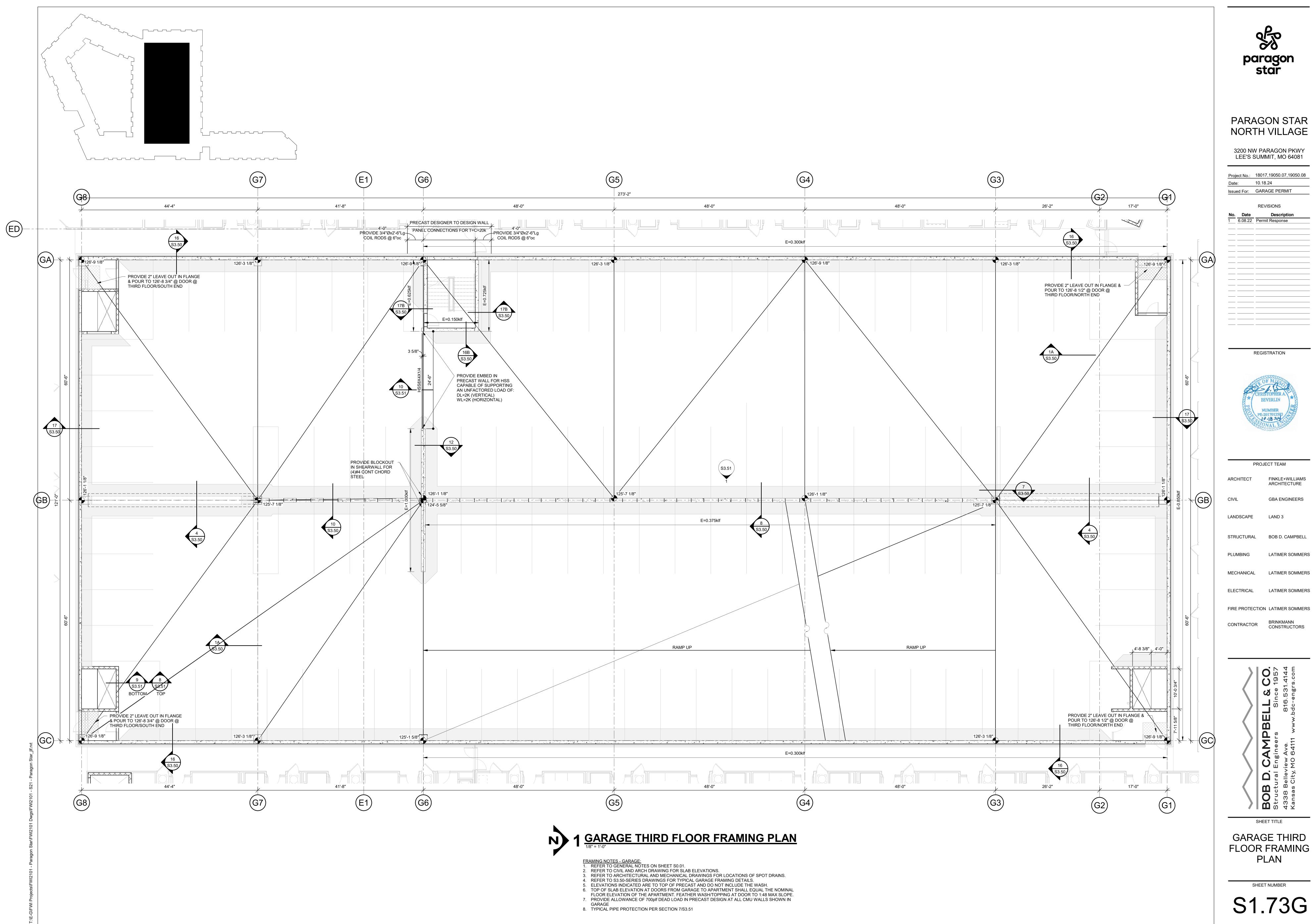
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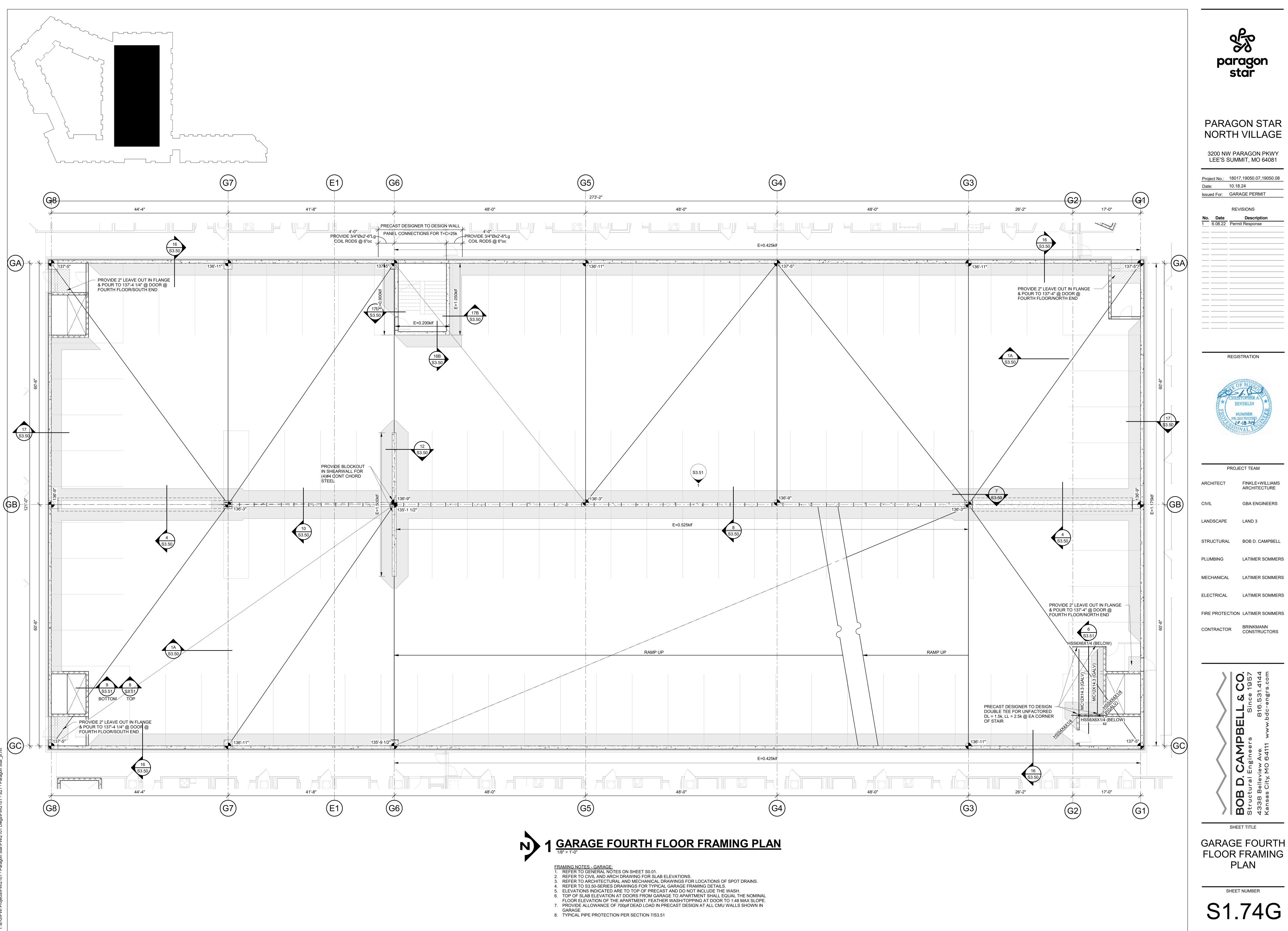
paragon star

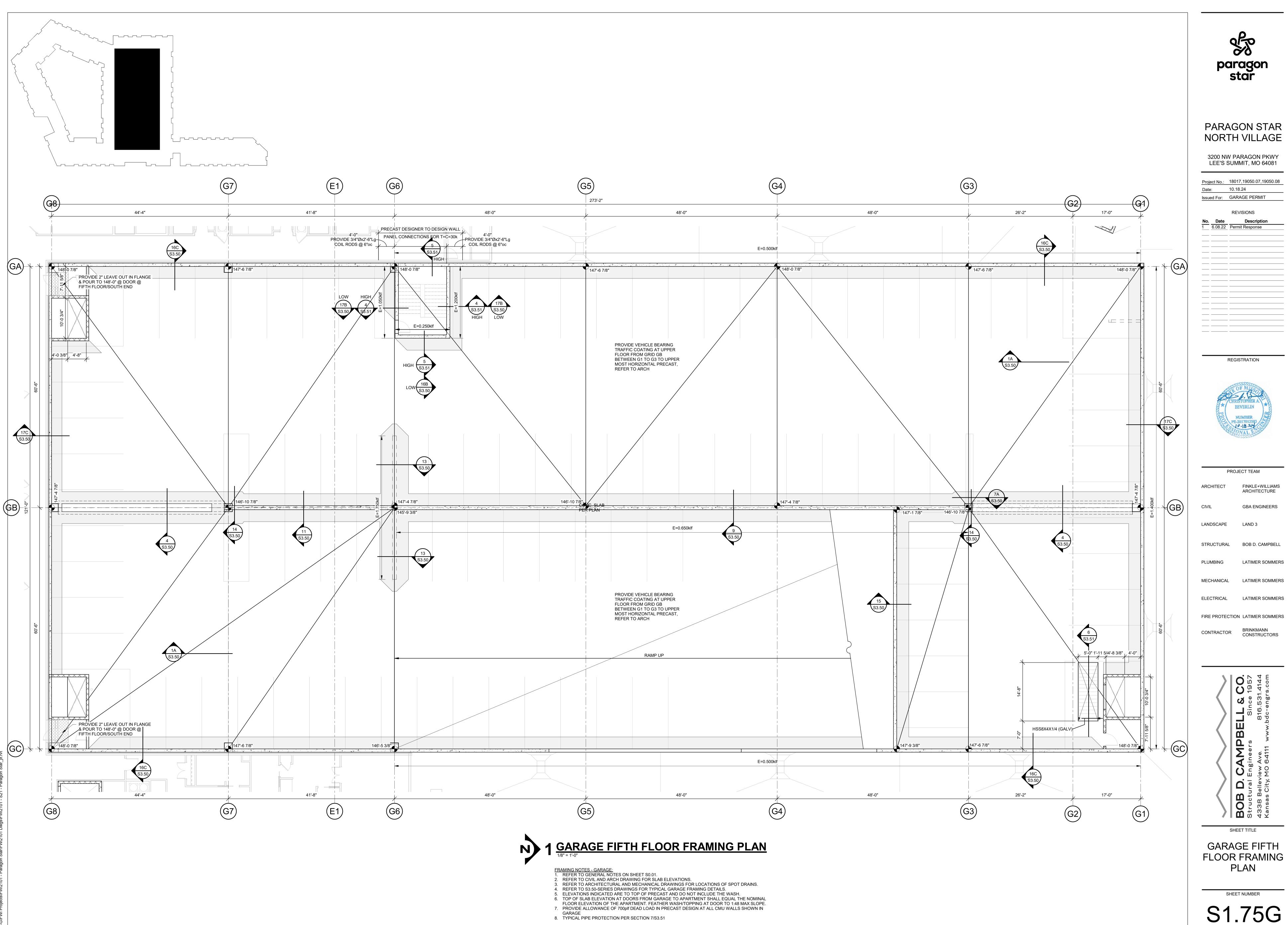
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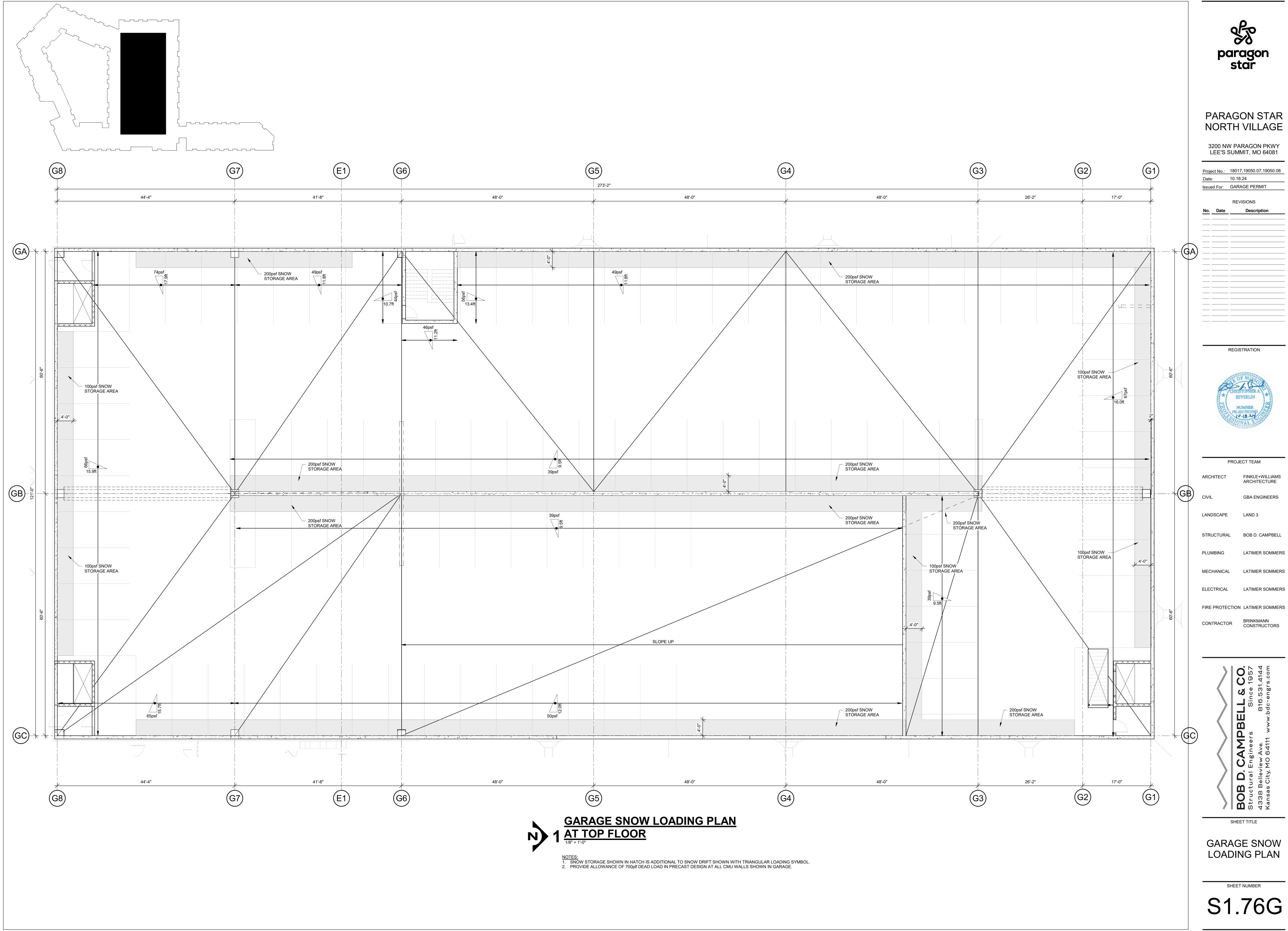




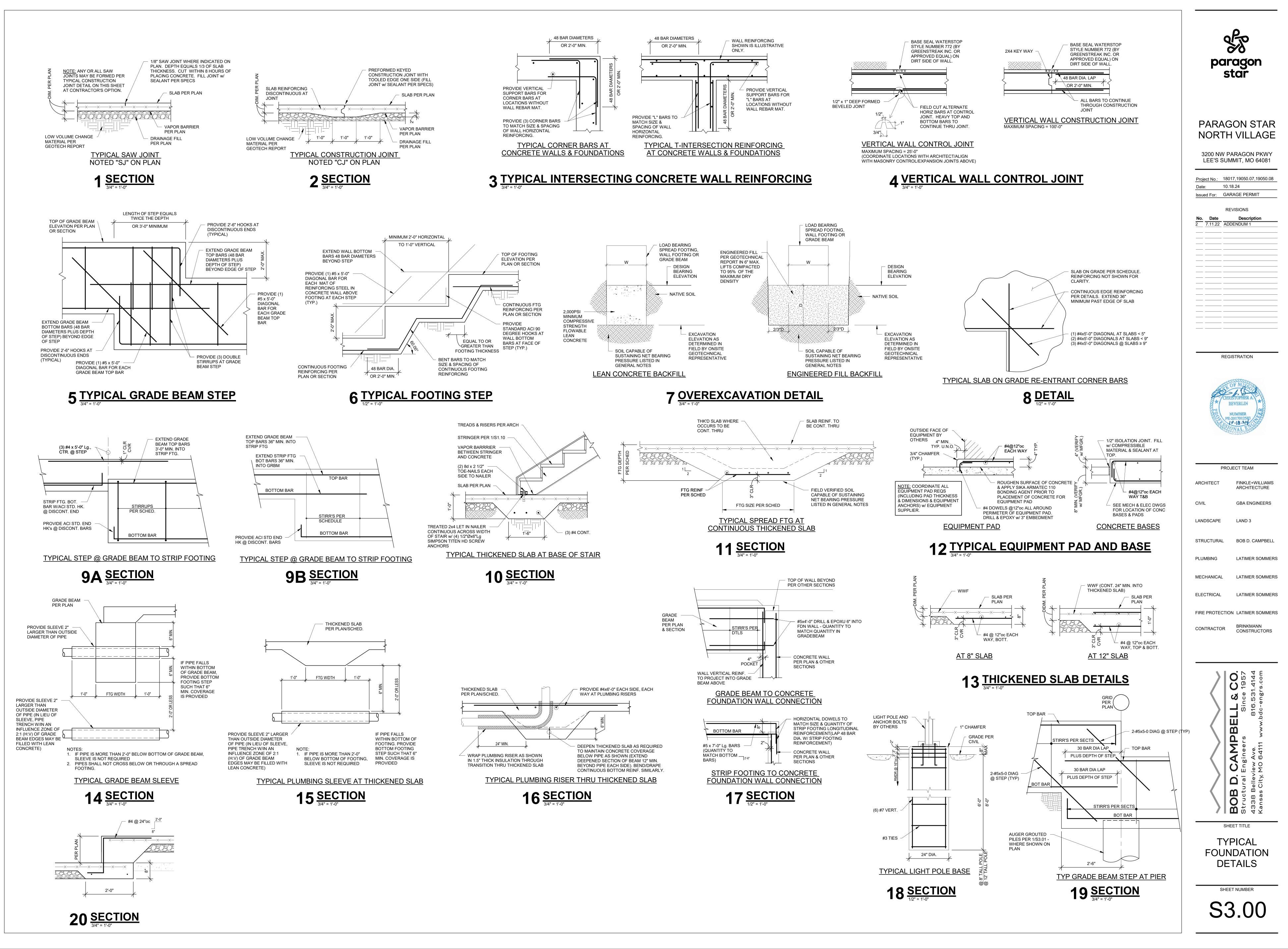


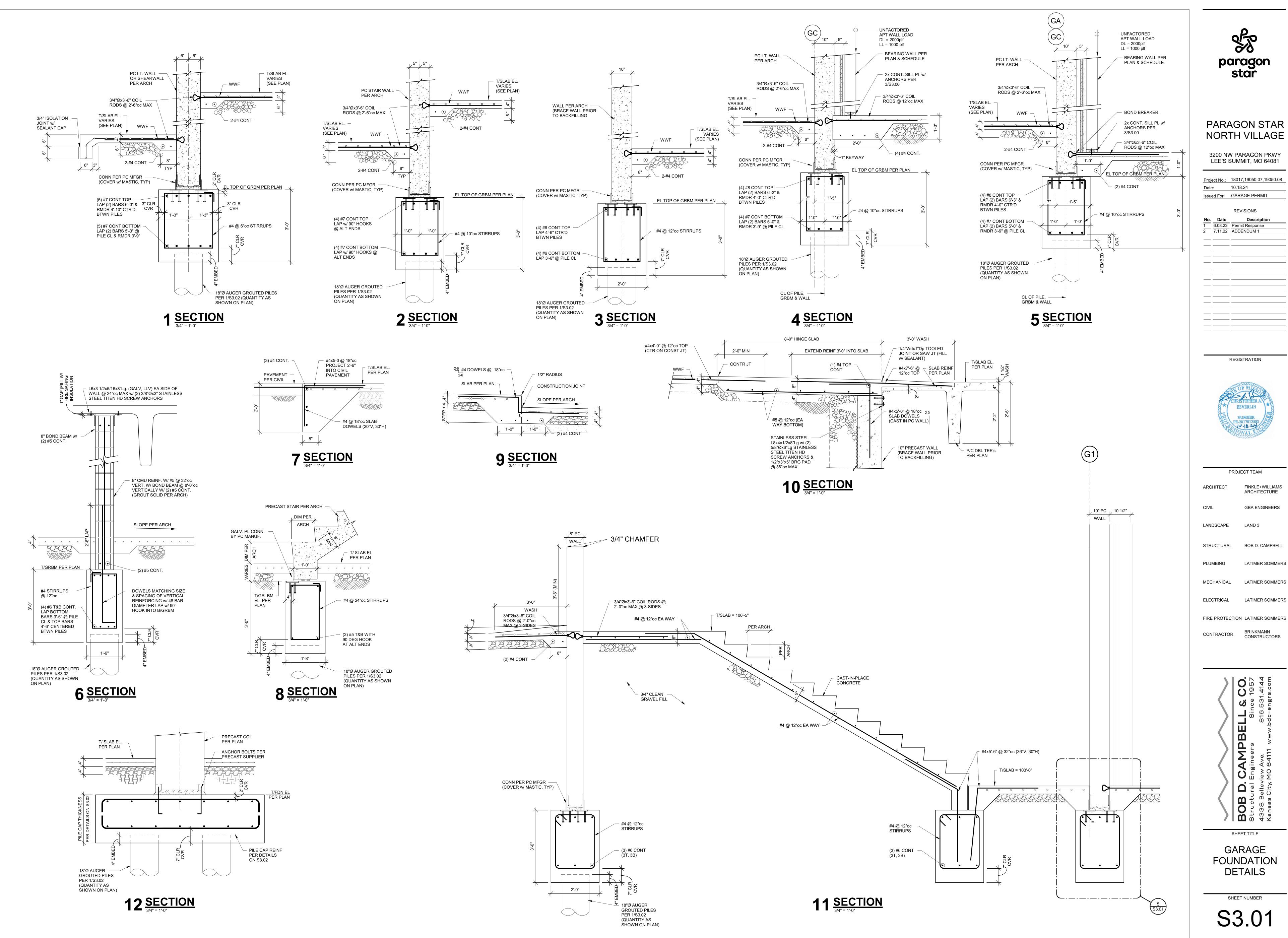


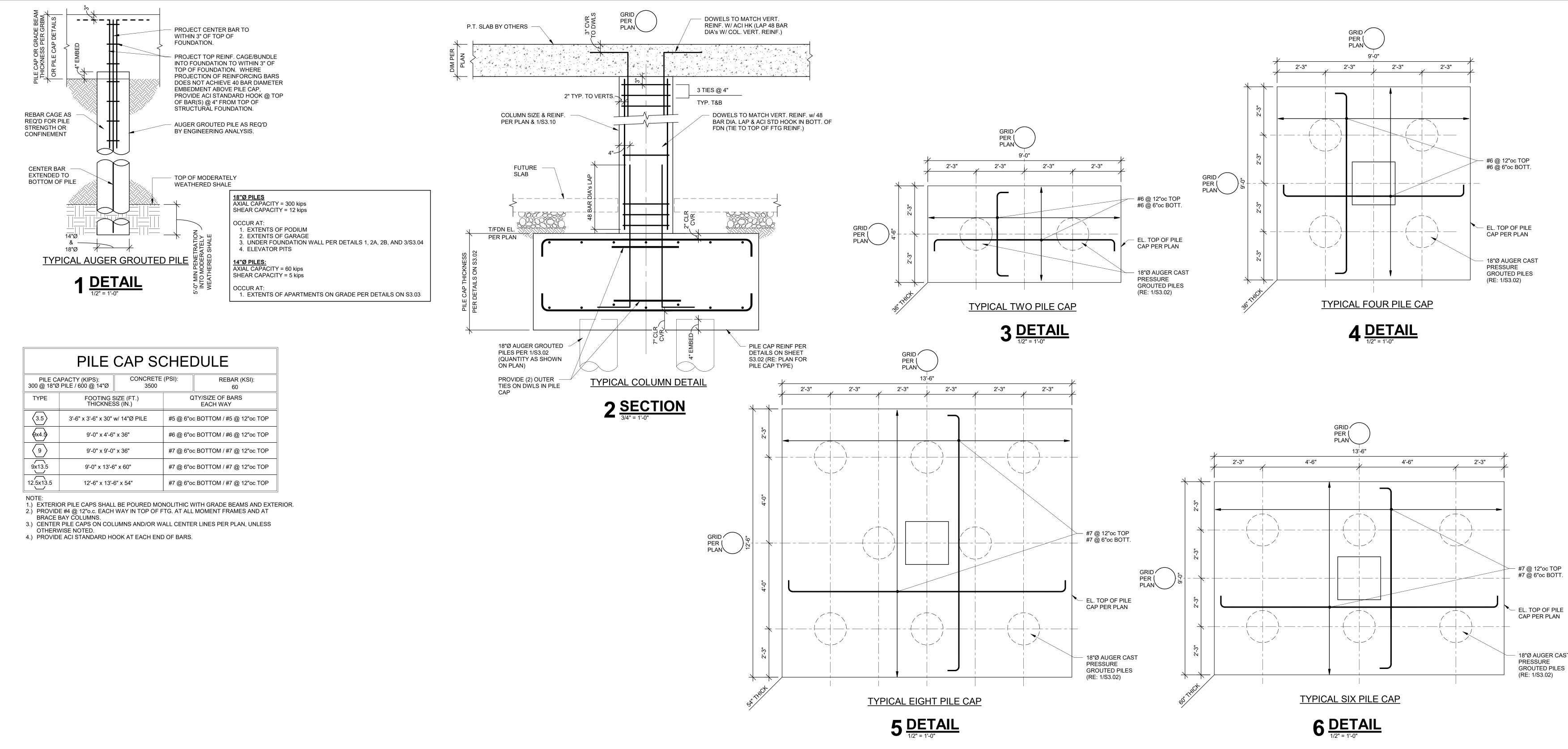




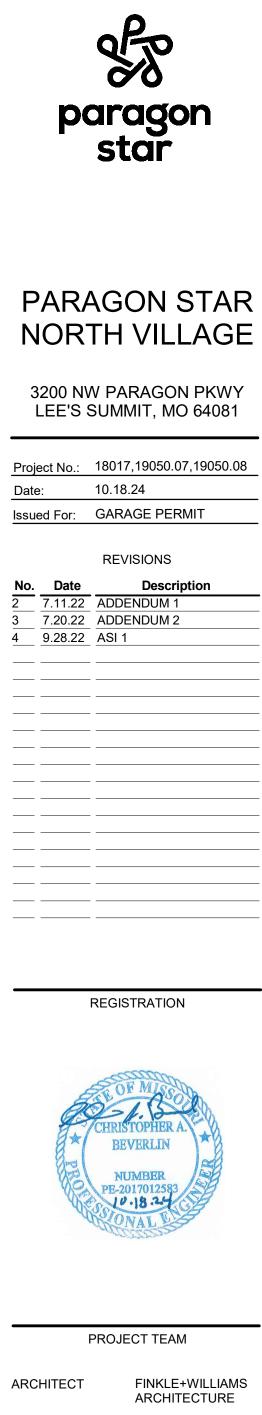






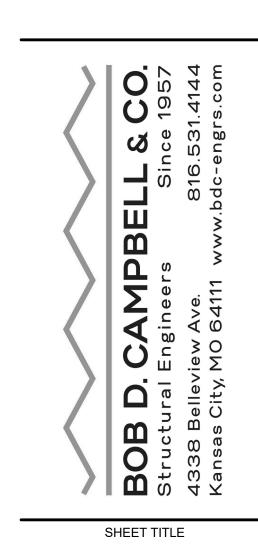


PILE CAP SCHEDULE					
PILE CAPACTY (KIPS): 300 @ 18"Ø PILE / 600 @ 14"Ø		CONCRETE (PSI): 3500		REBAR (KSI): 60	
TYPE	FOOTING SIZE (FT.) THICKNESS (IN.)		QTY/SIZE OF BARS EACH WAY		
3.5	3'-6" x 3'-6" x 30" w/ 14"Ø PILE		#5 @ 6"oc BOTTOM / #5 @ 12"oc TOP		
(x4.5)	9'-0" x 4'-6" x 36"		#6 @ 6"oc BOTTOM / #6 @ 12"oc TOP		
9	9'-0" x 9'-0" x 36"		#7 @ 6"oc BOTTOM / #7 @ 12"oc TOP		
9x13.5	9'-0" x 13'-6" x 60"		#7 @ 6"oc BOTTOM / #7 @ 12"oc TOP		
12.5x13.5	12'-6" x 13'-6" x 54"		#7 @ 6"oc BOTTOM / #7 @ 12"oc TOP		



- 18"Ø AUGER CAST PRESSURE GROUTED PILES (RE: 1/S3.02)

PROJECT TEAM				
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE			
CIVIL	GBA ENGINEERS			
LANDSCAPE	LAND 3			
STRUCTURAL	BOB D. CAMPBELL			
PLUMBING	LATIMER SOMMERS			
MECHANICAL	LATIMER SOMMERS			
ELECTRICAL	LATIMER SOMMERS			
FIRE PROTECTION	LATIMER SOMMERS			
CONTRACTOR	BRINKMANN CONSTRUCTORS			

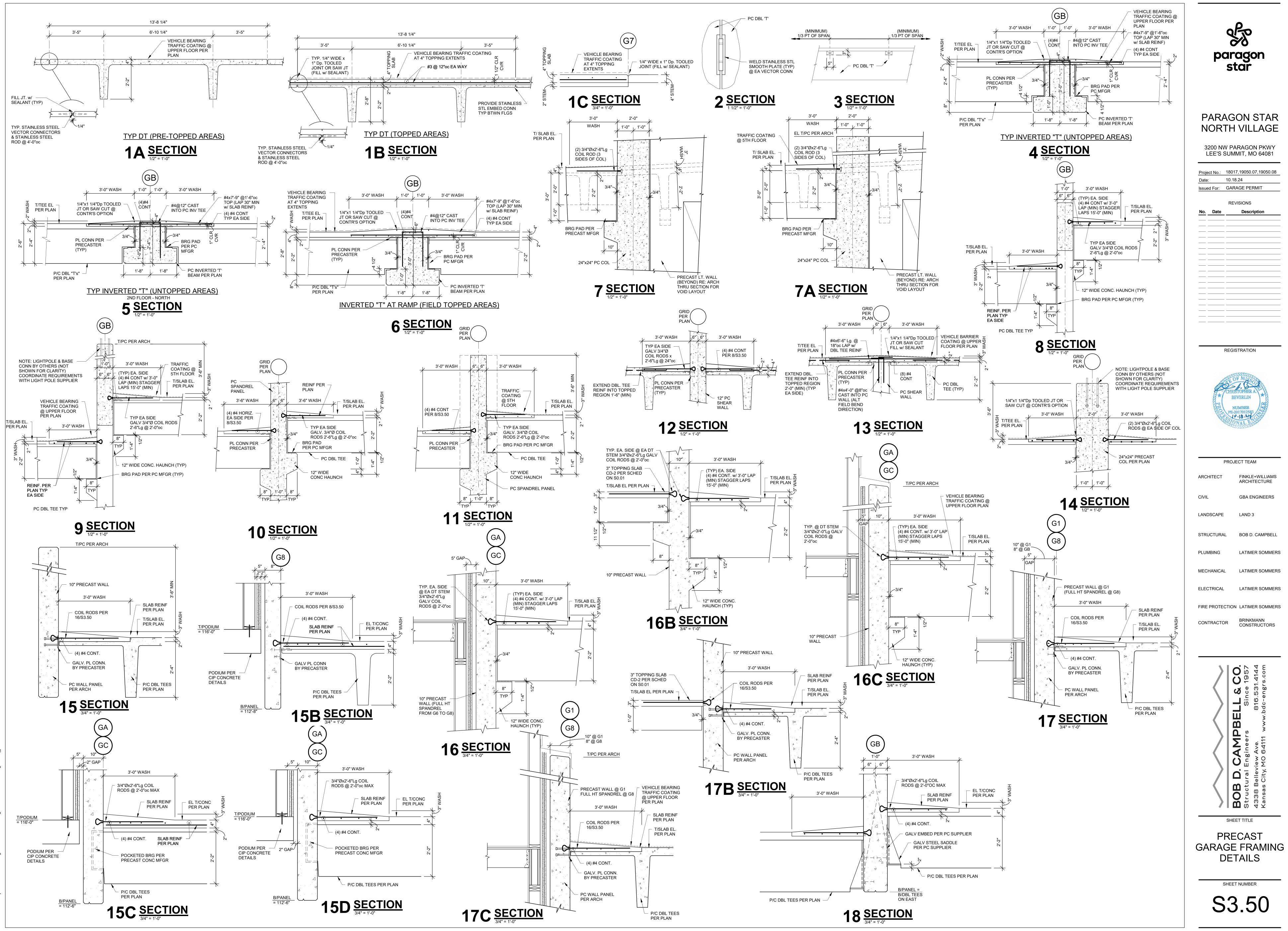


FOUNDATION DETAILS

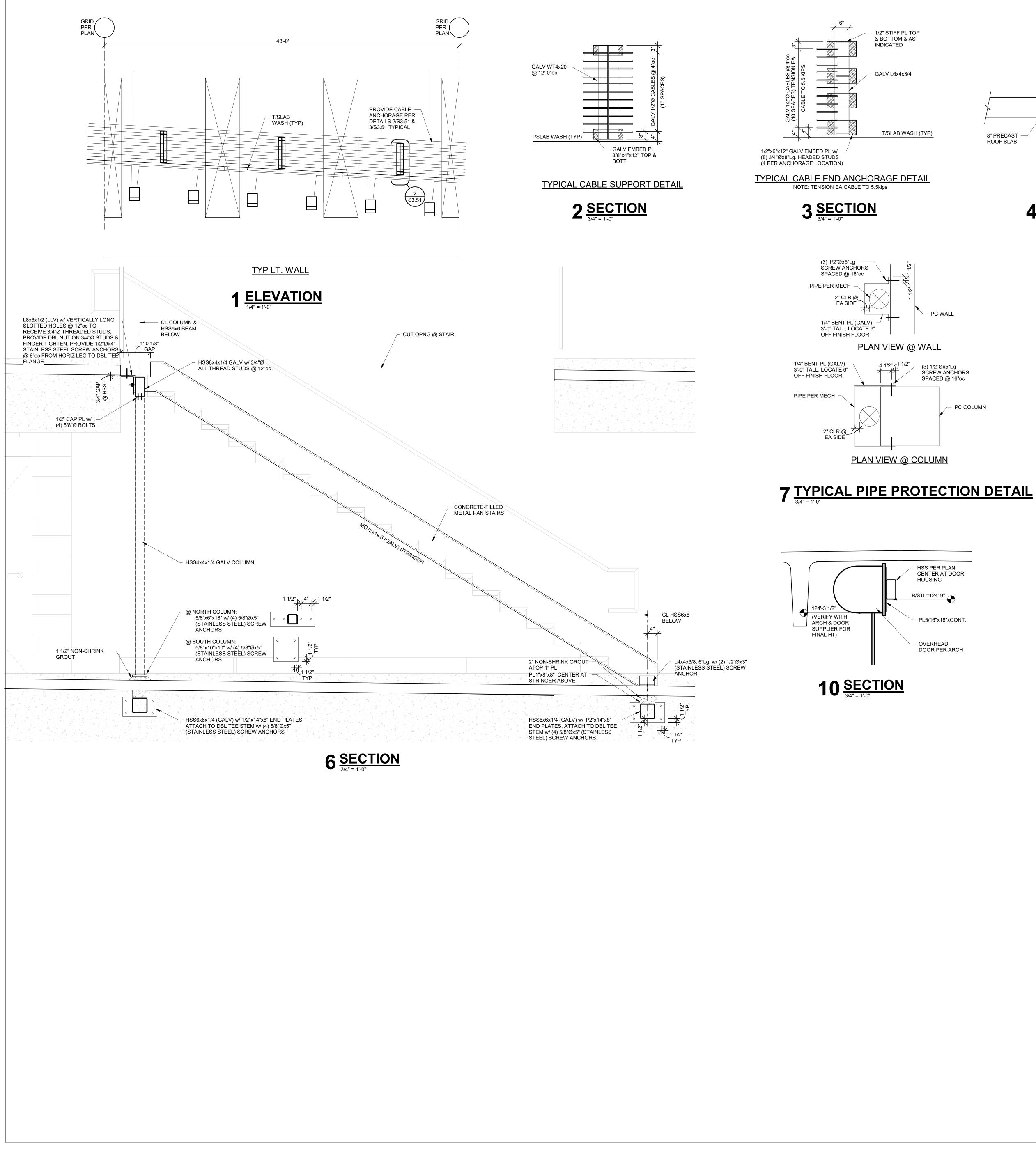
PILE & PODIUM

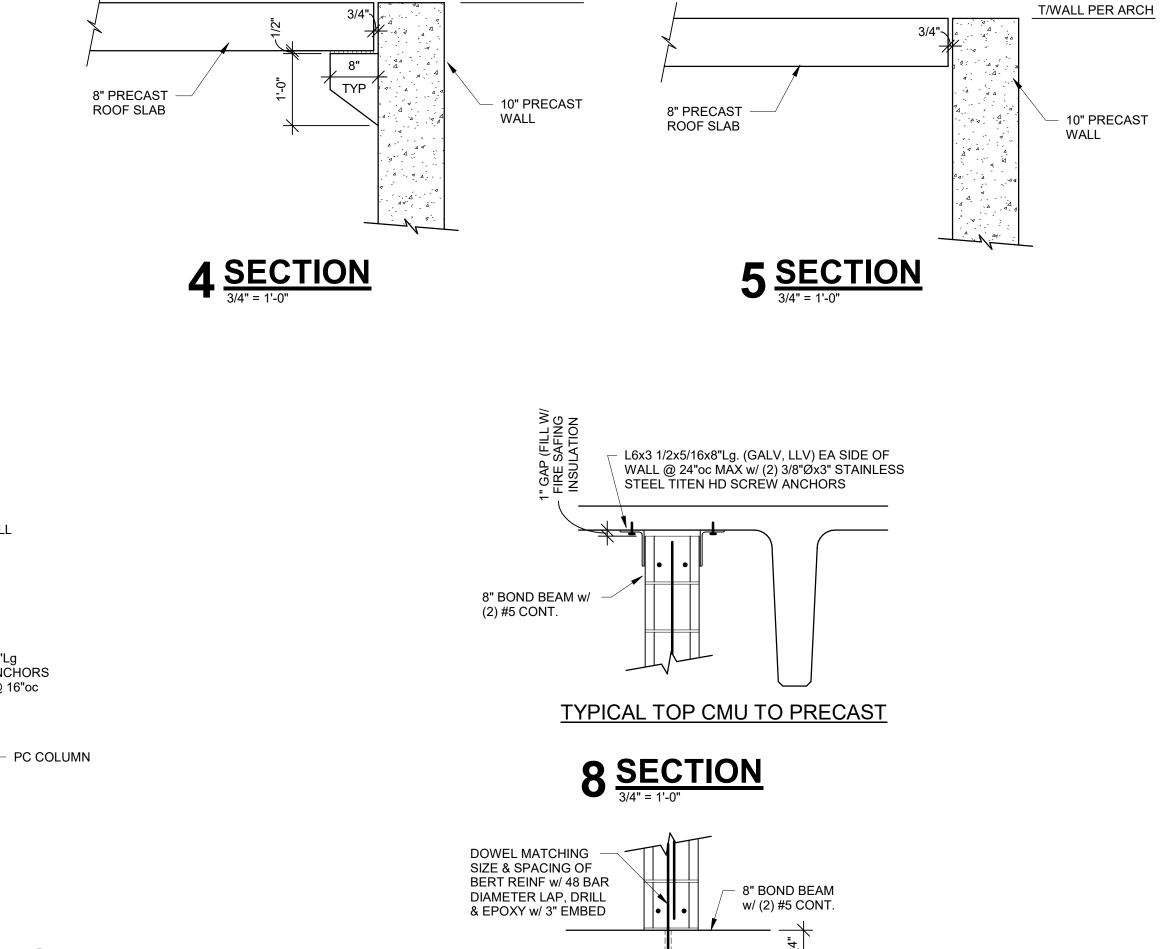
S3.02

SHEET NUMBER



-WI Projects\FWI2101 - Paragon Star\FWI2101 Dwgs\FWI2101 - S21 - Paragon Star\_jlf.rvt





TYPICAL BOTTOM CMU TO PRECAST

9 <u>SECTION</u> 3/4" = 1'-0"

T/WALL PER ARCH

HSS PER PLAN

HOUSING

- PL5/16"x18"xCONT.

- OVERHEAD DOOR PER ARCH



NFPA SYMBOLS LEGEND		
0	SMOKE DETECTOR	
Øs	SMOKE DETECTOR WITH SOUNDER BASE	
() ISO	SMOKE DETECTOR WITH ISOLATOR BASE	
	HEAT DETECTOR	
$\bigcirc$	DUCT DETECTOR	
$\overline{\mathbf{O}}$	ADDRESSABLE MANUAL PULL STATION	
Ŷ	DOOR HOLDER	
	FLOW DETECTOR/SWITCH	
ୁ ଜୁ	TAMPER DETECTOR	
Ξ	TEST STATION	
R]	MR101/C SHUTDOWN RELAY, SPDT W/RED	
⊠⊠	A/V (WALL MOUNTED) 24 VDC	
×	STROBE	
Ŷ	BELL ANNUNCIATOR	
Ŋ	HORN/SPEAKER	
[FCP]	FIRE ALARM CONTROL PANEL	
C	FIREMAN'S PHONE	
[ARA]	AREA RESCUE CALL STATION	
[ARA] <sub>M</sub>	AREA RESCUE MASTER STATION	
[ZAM] S	SIGNAL ZAM	
[ZAM] C	CONTROL ZAM	
[ZAM] DET	DETECTOR ZAM	
	MONITOR MODULE	
	RELAY IAM	
[PC]	GRAPHIC COMMAND CENTER	
[FAA]	REMOTE FIRE ALARM AUDIO	
[FSA]	REMOTE ANNUNCIATOR WITH AUDIO	
[ANN]	ANNUNCIATOR	
-[FS]-	FIRE SMOKE DAMPER	
[NAC]	NAC POWER EXTENDER	
* ALL SYMBOLS SHOWN ABOVE MAY NOT APPEAR ON PLANS		

SYMBOL	DESCRIPTION	REMARKS
D201/A	TELECOMMUNICATIONS OUTLET WITH ROOM AND TYPE IDENTIFIER	1
W	TELECOMMUNICATIONS OUTLET WALL PHONE PLATE	2
<b>V</b> AV	AUDIO/VISUAL OUTLET	3
TV	TELEVISION OUTLET	1
	EMT CONDUIT BY E/C (1 1/4" UNLESS NOTED OTHERWISE)	4
-[]	EMT SLEEVE BY E/C (2" UNLESS NOTED OTHERWISE)	4
AFF	ABOVE FINISHED FLOOR	
T/C	TELECOMMUNICATIONS CONTRACTOR	
E/C	ELECTRICAL CONTRACTOR	
G/C	GENERAL CONTRACTOR	
AC	DEVICE LOCATED ABOVE COUNTER	
TMGB	TELECOMMUNICATIONS MAIN GROUNDING BUSBAR	
TGB	TELECOMMUNICATIONS GROUNDING BUSBAR	
	TELECOMMUNICATIONS CABLING	5
FACP	FIRE ALARM CONTROL PANEL	
SM	SINGLEMODE FIBER	
MM	MULTIMODE FIBER	
WAP	WIRELESS ACCESS POINT	6
TELECON	MUNICATIONS CABLING IDENTIFIER	
T	ELECOM ROOM D201/A - OUTLET TYPE	

AND 1 1/4" CONDUIT TO ABOVE ACCESSIBLE CEILING AS INDICATED ON DRAWINGS BY E/C.
2 - 2x4 BACKBOX WITH 3/4" CONDUIT TO ABOVE ACCESSIBLE CEILING.
3 - 4x4 STEEL CITY BACKBOX, MODEL NUMBER 72171-1-1/4 WITH DOUBLE GANG PLASTER RING BY E/C. CONDUITS AS INDICATED ON PLANS.
4 - E/C TO PROVIDE CONDUIT BUSHING ON CONDUIT PRIOR TO T/C INSTALLING CABLING.
5 - CABLING SHALL BE SUPPORTED WITH J-HOOKS AT 48" O.C. WHERE NOT IN CONDUIT.
6 - 2x4 SURFACE MOUNT BACKBOX LOCATED ABOVE ACCESSIBLE CEILING.

P-1	WATER CLOSET & TYPE (TYP. FOR ALL PLUMBING FIXTURES)	— снз—	CHILLED HOT SUPPLY
	WASTE LINE ABOVE EARTH (W.)	- CHR-	CHILLED HOT RETURN
	WASTE LINE IN EARTH (W.)		
— II co	CLEAN OUT		FLEXIBLE PIPE CONNECTION
FFCO O	FLUSH FLOOR CLEAN OUT		MANUAL DAMPER
FGCO O	FLUSH GRADE CLEAN OUT		BACKDRAFT DAMPER
" <u>(1) FD</u>	FLOOR DRAIN AND TYPE		AUTOMATIC DAMPER
RD	ROOF DRAIN		FIRE DAMPER
-ORD-	OVERFLOW ROOF DRAIN		FIRE/SMOKE DAMPER
" <u>(1) rd</u> 0	ROOF DRAIN AND TYPE		SMOKE DAMPER
	VENT LINE (V.)	6x6 A X	GRILLE, REGISTER OR DIFFUSER, SIZE, TYPE & CFM
	DOMESTIC COLD WATER SUPPLY (DCW)		VOLUME EXTRACTOR AND TURNING VANES
	DOMESTIC HOT WATER SUPPLY (DHW)		RETURN, EXHAUST OR FRESH AIR DUCT SECTION UP & DOWN
	DOMESTIC HOT WATER RETURN (DHWR)		SUPPLY AIR DUCT SECTION UP AND DOWN
+ HB/36"	HOSE BIBB AND MOUNTING HEIGHT		FLEXIBLE DUCT CONNECTION
EI WH	WALL HYDRANT		ROUND OR RECTANGULAR DUCT
— F ——	FIRE LINE/STANDPIPE		FLEXIBLE DUCT
— D —	DRAIN LINE	φ	THERMOSTAT
— G —	NATURAL GAS LINE	— R —	REFRIGERANT LIQUID/SUCTION
<u>-iqi-iz</u>	RISE & DROP IN PIPE WITH CUT-OFF VALVE	AD	ACCESS DOOR
<b></b>	REDUCER	AFF	ABOVE FINISHED FLOOR
	CHECK VALVE	EA	EXHAUST AIR
	STOP VALVE	OA	OUTSIDE AIR
R	BALANCING VALVE/AUTOFLOW VALVE	RA	RETURN AIR
	PLUG VALVE	SA	SUPPLY AIR
	2-WAY CONTROL VALVE OR SOLENOID VALVE	VBS	VENT BELOW SLAB
	3-WAY CONTROL VALVE OR SOLENOID VALVE	VTR	VENT THRU ROOF
_₫	PRESSURE REDUCING VALVE	•	CONNECT NEW TO EXISTING
	STRAINER		LOCKABLE GUARD
—cws—	CHILLED WATER SUPPLY	VFD	VARIABLE FREQUENCY DRIVE
—CWR—	CHILLED WATER RETURN		
—HWS—	HOT WATER SUPPLY		
—HWR—	HOT WATER RETURN		

	ELECTRICAL S		LS LEGEND
$\frown$	CONDUIT CONCEALED IN CEILING OR WALL. 2 HOT + GROUND.	φ	THERMOSTAT
$\sim$	CONDUIT CONCEALED IN FLOOR SLAB		LOCKABLE GUARD
~~~	EXPOSED CONDUIT		JUNCTION BOX
	HOMERUN - ARROW INDICATES CKT., LINES INDICATE WIRES	\$	SWITCH - SINGLE POLE
<u> </u> IP	GROUNDING ROD	\$	SWITCH - 3-WAY
φ	SINGLE RECEPTACLE	\$4	SWITCH - 4-WAY
Þ	DUPLEX RECEPTACLE (20 AMP UNLESS NOTED)		LIGHT FIXTURE AND TYPE
<b>φ</b> υ	DUPLEX RECEPTACLE WITH USB OUTLETS	$\mathbb{X}$	EMERGENCY LIGHT FIXTURE WITH BATTERY PACK
<b>þ</b> sw	SWITCHED DUPLEX RECEPTACLE		FIXTURE ON LIFE SAFETY BRANCH OF EMERGENCY SYSTEM
<b>#</b>	FOURPLEX RECEPTACLE	с С П	LIGHT FIXTURE (WALL MOUNTED)
ф	208 OR 240 VOLT RECEPTACLE (20 AMP UNLESS NOTED)	<u> </u>	EXIT LIGHT (CEILING OR WALL MOUNTED)
<b></b>	GROUND FAULT INTERRUPTER (GFI) DUPLEX RECEPTACLE		FLUSH PANELBOARD (LIGHT & RECEPTACLES)
▼	TELE/DATA OUTLET *		SURFACE PANELBOARD (LIGHT & RECEPTACLES)
Б	PUSHBUTTON		DISTRIBUTION PANEL OR SWITCHBOARD
[VFD]	VARIABLE FREQUENCY DRIVE	AC	DEVICE LOCATED ABOVE COUNTER
ORT	OVERRIDE TIMER	AFF	ABOVE FINISHED FLOOR
[PC]	PHOTOCELL	D	DIMMER
6	MOTOR	E	INDICATES EXISTING DEVICE
\$	FUSIBLE SWITCH (BUSSMAN SSU)	EDF	ELECTRIC DRINKING FOUNTAIN
Ю	DISCONNECT SWITCH (D.S.)	NL	NIGHTLIGHT FIXTURE, WIRED HOT
لکا	COMBINATION MOTOR STARTER (CMS)	WP	WEATHERPROOF
[R]	RELAY	AFCI	ARC FAULT CIRCUIT INTERRUPTER
00	FLOOR BOX	•	CONNECT NEW TO EXISTING

# PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.:		18017,19050.07,19050.08
Date:		10.18.2024
Issue	d For:	GARAGE PERMIT SET
		REVISIONS
No.	Date	Description
<u> </u>	Date	Description

REGISTRATION



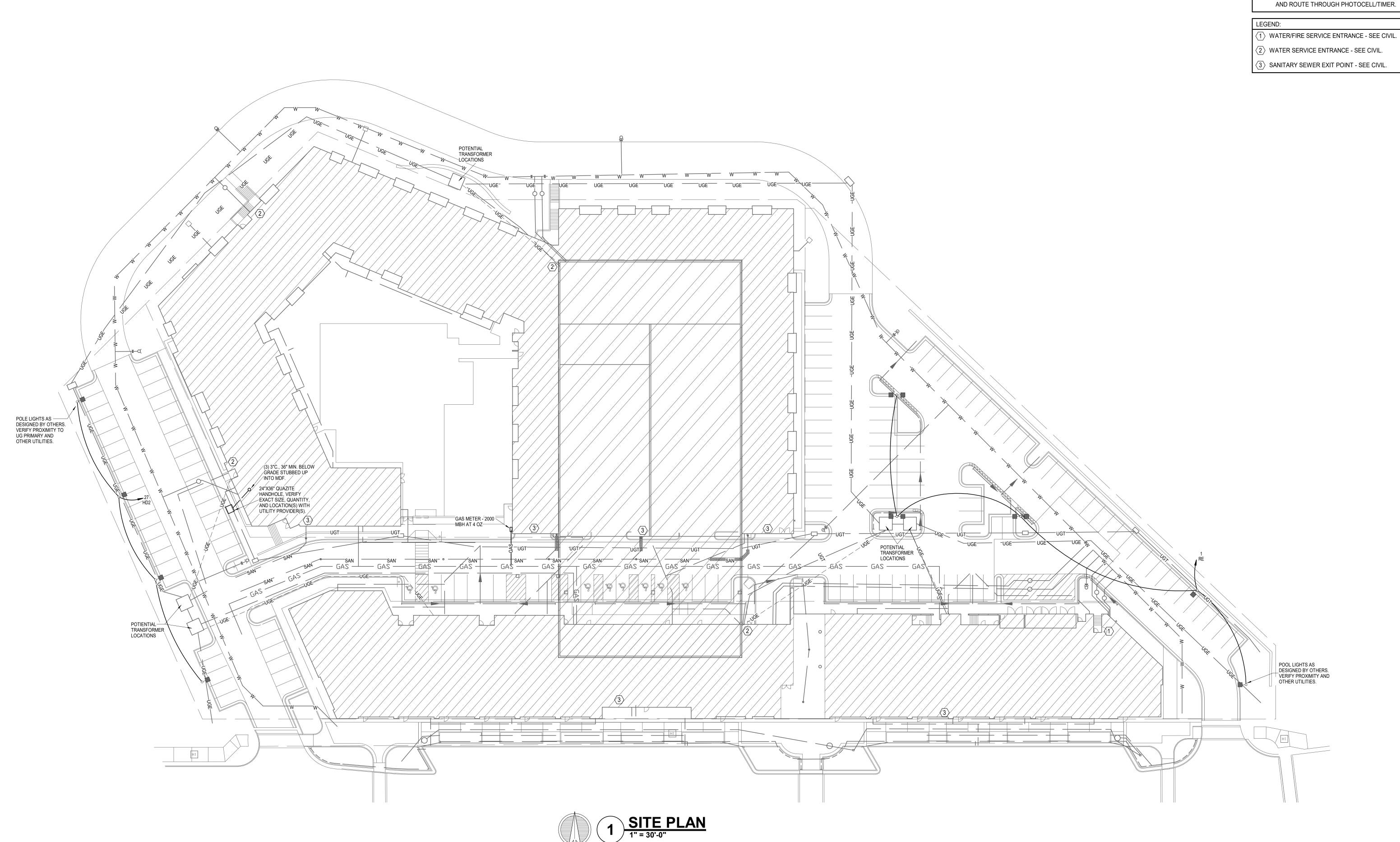
PROJECT TEAM		
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE	
CIVIL	GBA ENGINEERS	
LANDSCAPE	LAND 3	
STRUCTURAL	BOB D. CAMPBELL	
PLUMBING	LATIMER SOMMERS	
MECHANICAL	LATIMER SOMMERS	
ELECTRICAL	LATIMER SOMMERS	
FIRE PROTECTION	LATIMER SOMMERS	
CONTRACTOR	BRINKMANN CONSTRUCTORS	



COVER SHEET

SHEET NUMBER

ME-000



NORTH

NOTES	NOTES:		
1.	COORDINATE WITH CIVIL PLANS FOR EXACT LOCATIONS OF UTILITIES, SITE FEATURES AND GRADE.		
2.	COORDINATE WITH UTILITY PROVIDERS, INCLUDE ALL REQUIREMENTS AND FEES WITHIN THE BID WORK. IF NO FEE IS AVAILABLE, PROVIDE AN ALLOWANCE AND LIST ON BID SUBMISSION.		
3.	TRANSFORMER LOCATIONS ARE AS SUGGESTED AND PREFERRED. POWER COMPANY WILL MAKE FINAL DETERMINATION.		
4.	FOR ANY GAS SERVICES, PROVIDE STEEL RISER, SHUT- OFF AND APPROPRIATE PRV AT APPLIANCES IF APPROPRIATE.		

ALL SITE LIGHTING IS #10 WIRE IN 1" MIN. PVC CONDUIT AND ROUTE THROUGH PHOTOCELL/TIMER.

 $\langle 2 \rangle$  water service entrance - see civil.

 $\langle \overline{3} \rangle$  SANITARY SEWER EXIT POINT - SEE CIVIL.



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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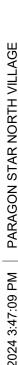
PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

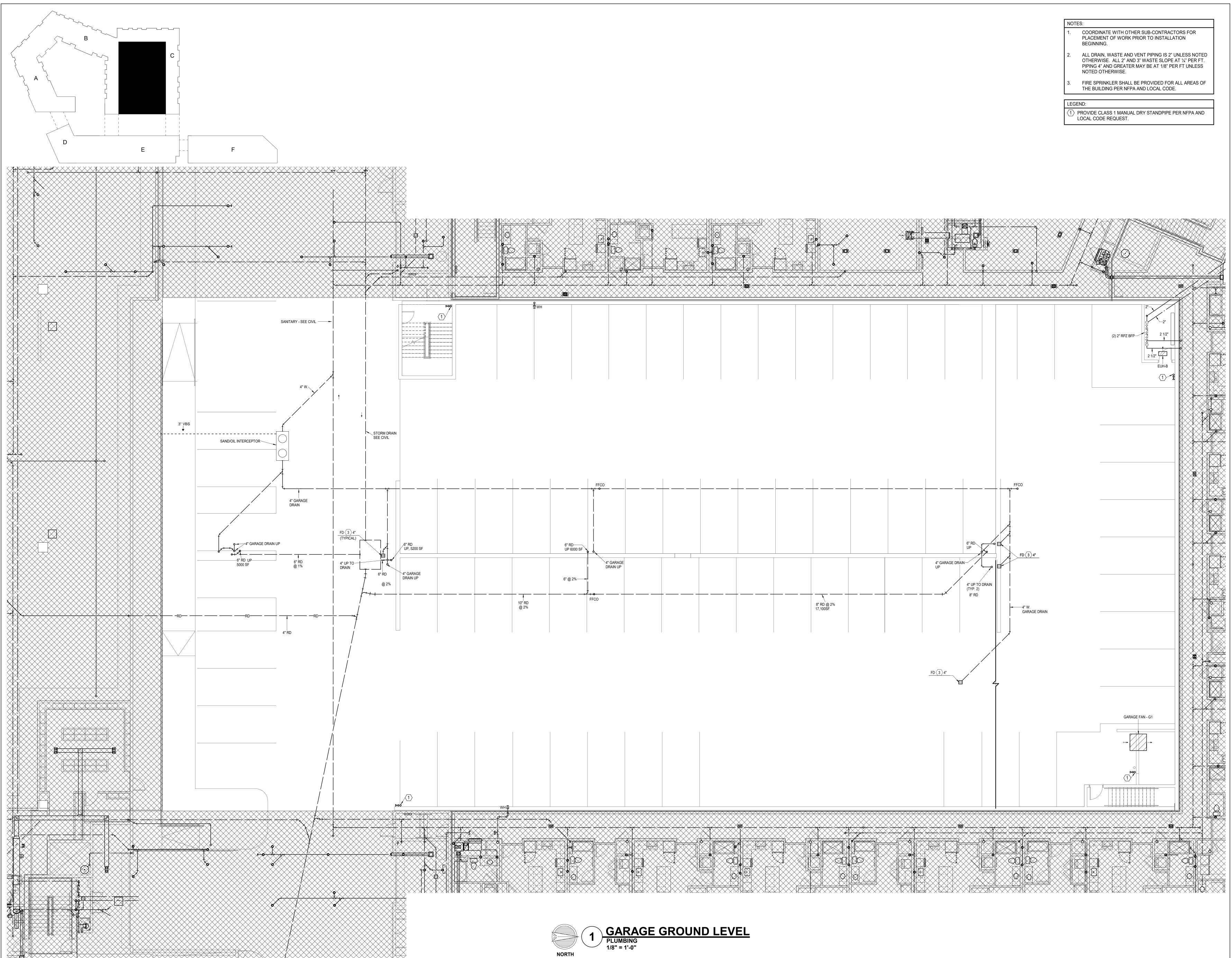


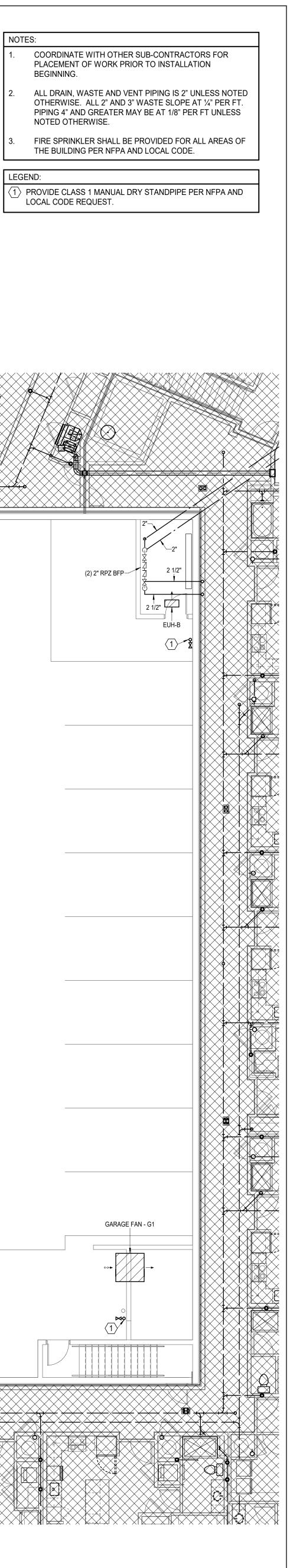
SITE PLAN

SHEET NUMBER

ME-001







3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.:		18017,19050.07,19050.08
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		REVISIONS
No.	Date	Description
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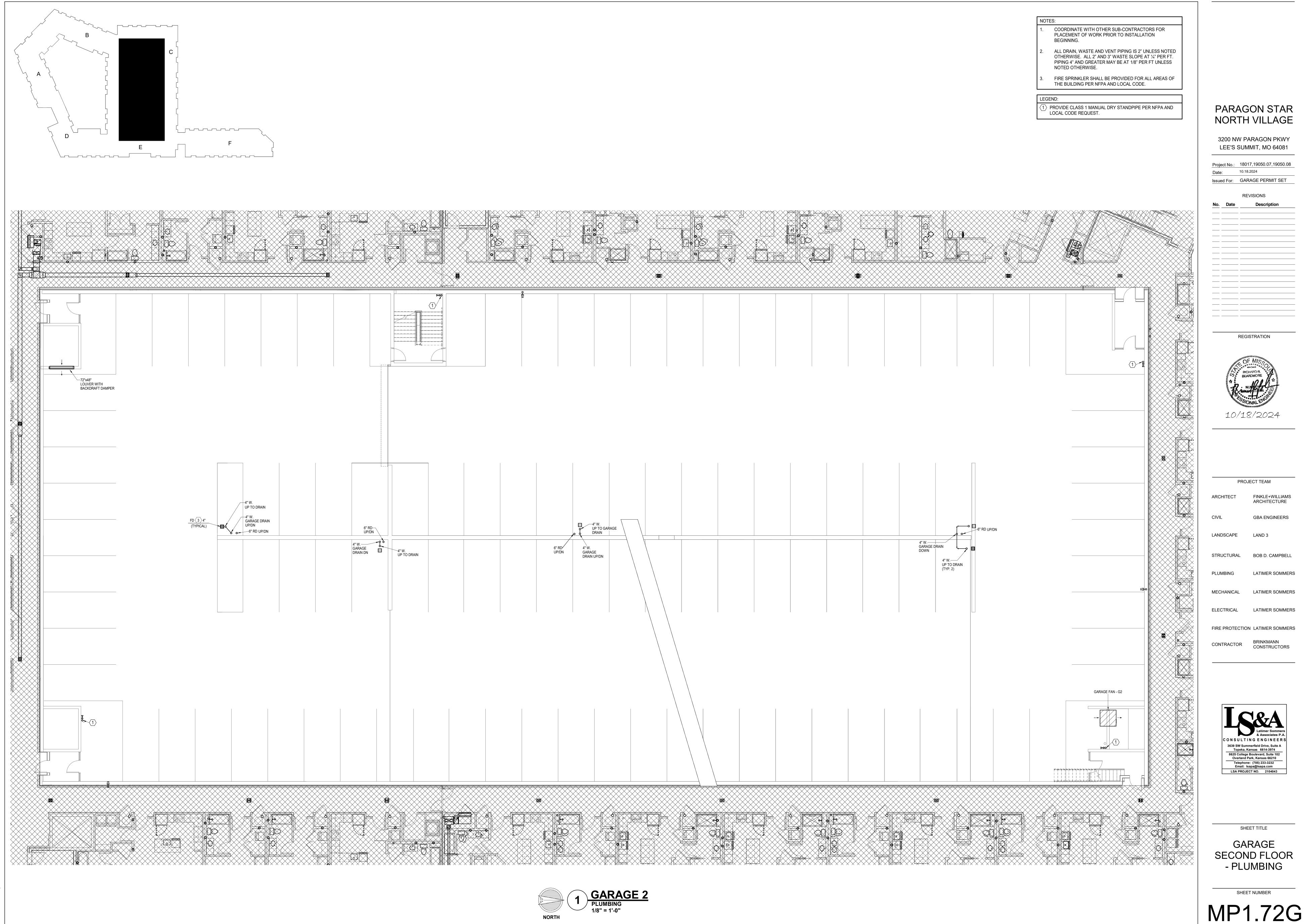
PROJECT TEAM		
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE	
CIVIL	GBA ENGINEERS	
LANDSCAPE	LAND 3	
STRUCTURAL	BOB D. CAMPBELL	
PLUMBING	LATIMER SOMMERS	
MECHANICAL	LATIMER SOMMERS	
ELECTRICAL	LATIMER SOMMERS	
FIRE PROTECTION	LATIMER SOMMERS	
CONTRACTOR	BRINKMANN CONSTRUCTORS	



SHEET TITLE GARAGE FIRST FLOOR -PLUMBING

SHEET NUMBER

MP1.71G



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.:		18017,19050.07,19050.08
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No.	Date	Description

REGISTRATION

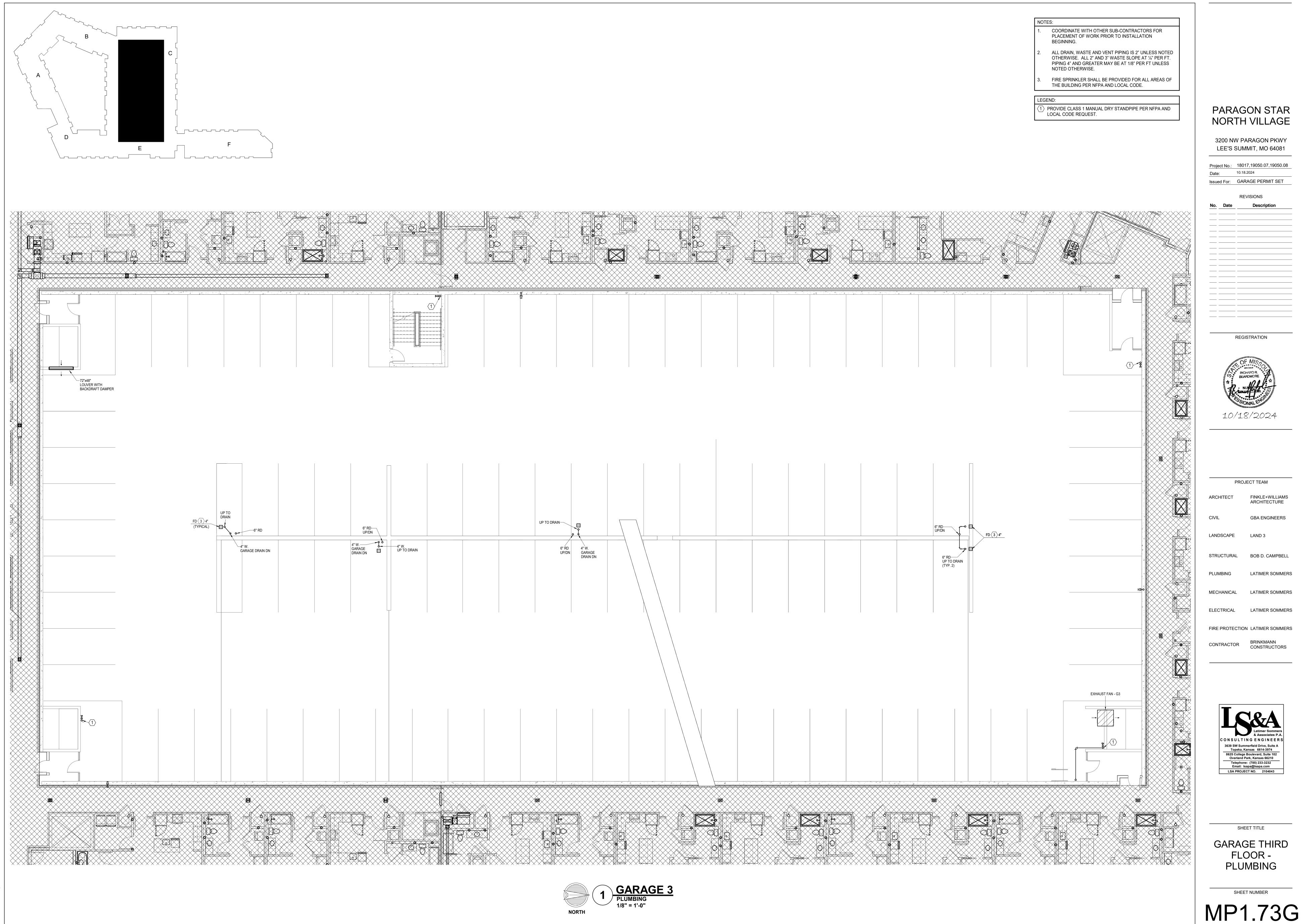


PROJECT TEAM		
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE	
CIVIL	GBA ENGINEERS	
ANDSCAPE	LAND 3	
STRUCTURAL	BOB D. CAMPBELL	
PLUMBING	LATIMER SOMMERS	
MECHANICAL	LATIMER SOMMERS	
ELECTRICAL	LATIMER SOMMERS	
FIRE PROTECTION	LATIMER SOMMERS	
CONTRACTOR	BRINKMANN CONSTRUCTORS	





SHEET NUMBER



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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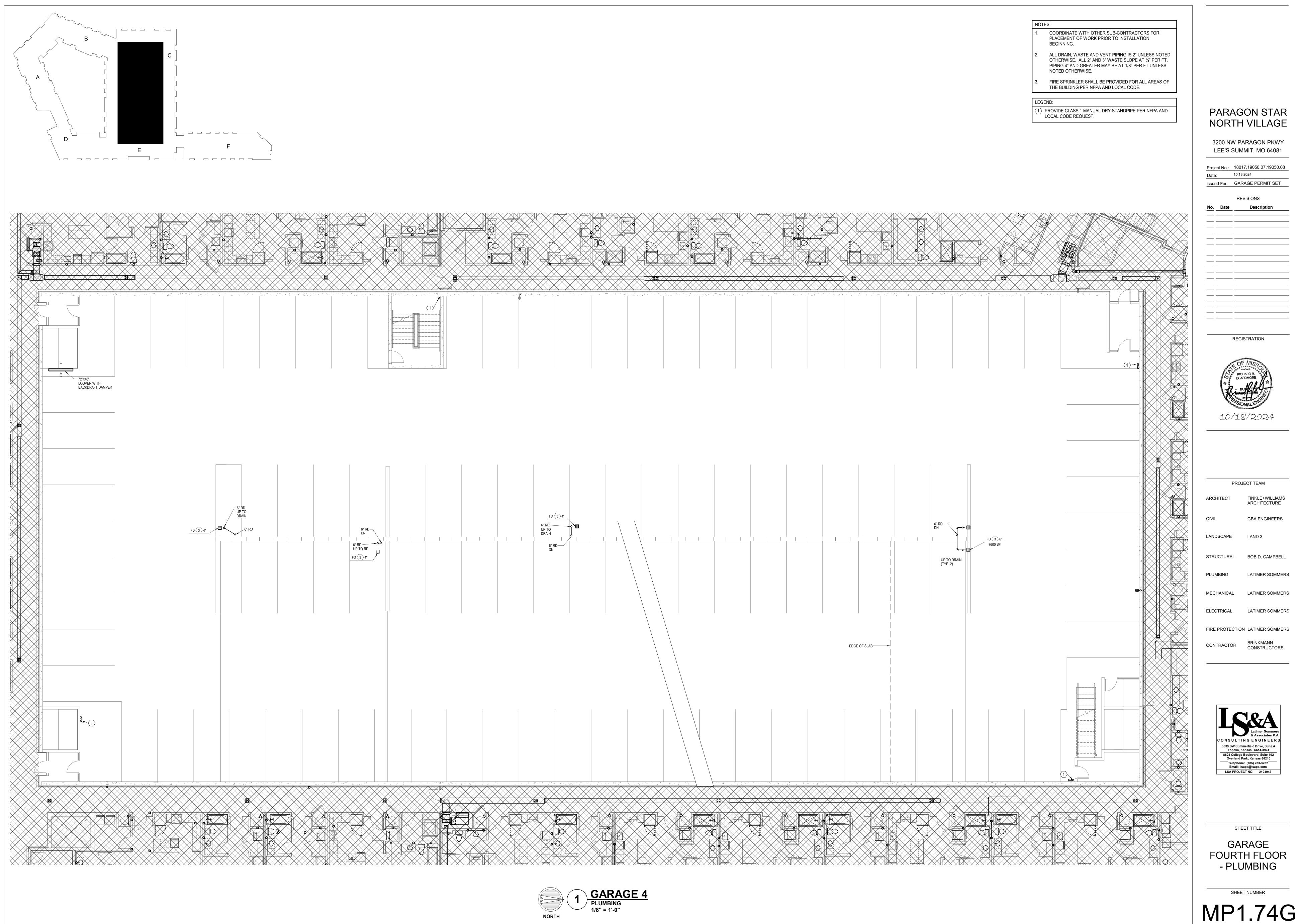


PROJECT TEAM			
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE		
CIVIL	GBA ENGINEERS		
ANDSCAPE	LAND 3		
STRUCTURAL	BOB D. CAMPBELL		
PLUMBING	LATIMER SOMMERS		
MECHANICAL	LATIMER SOMMERS		
ELECTRICAL	LATIMER SOMMERS		
FIRE PROTECTION	LATIMER SOMMERS		
CONTRACTOR	BRINKMANN CONSTRUCTORS		



SHEET TITLE GARAGE THIRD FLOOR -PLUMBING

SHEET NUMBER



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Proje	ct No.:	18017,19050.07,19050.08	
Date:		10.18.2024	
Issue	d For:	GARAGE PERMIT SET	
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No.	Date	Description	

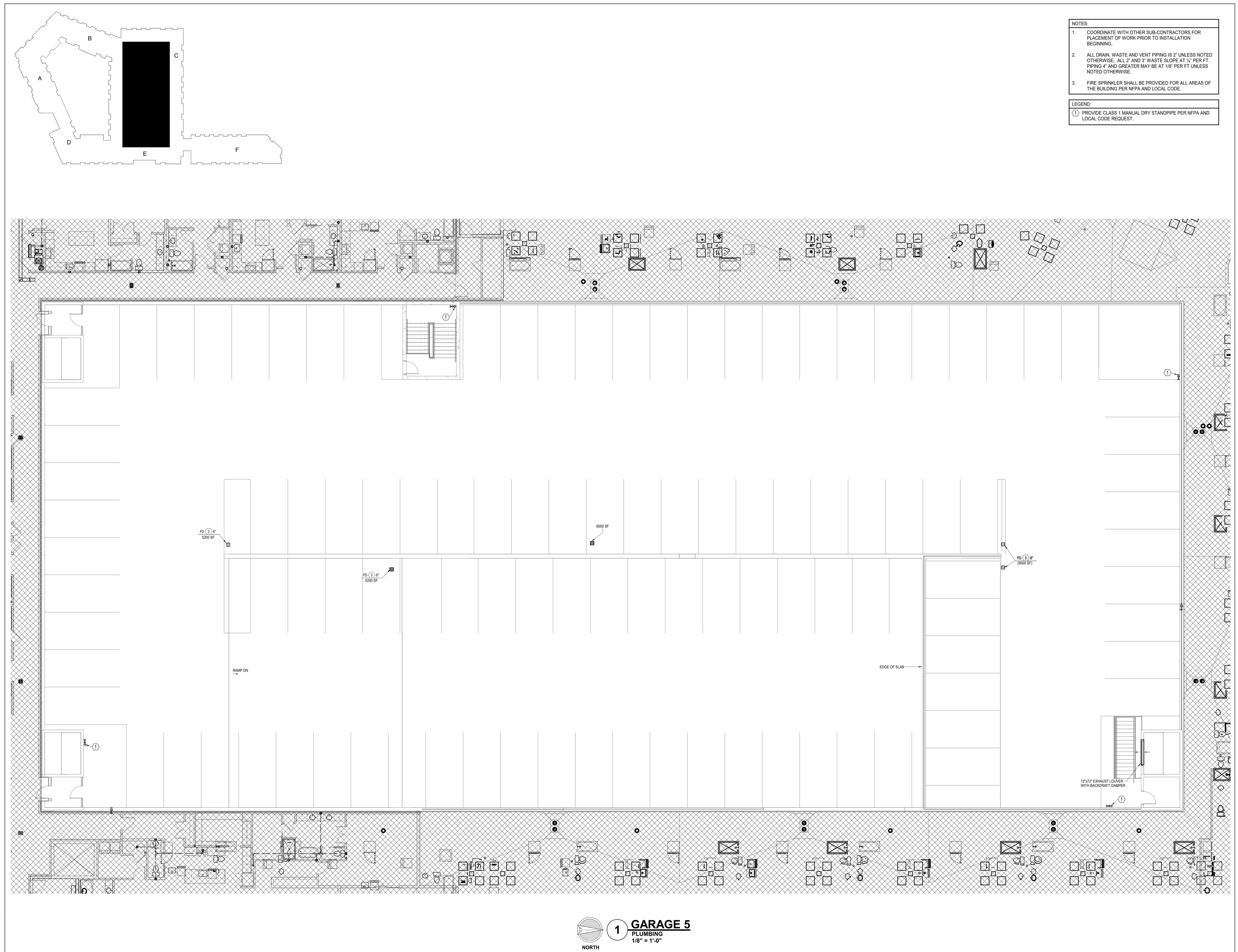
REGISTRATION



PROJECT TEAM		
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE	
CIVIL	GBA ENGINEERS	
ANDSCAPE	LAND 3	
STRUCTURAL	BOB D. CAMPBELL	
PLUMBING	LATIMER SOMMERS	
MECHANICAL	LATIMER SOMMERS	
ELECTRICAL	LATIMER SOMMERS	
FIRE PROTECTION	LATIMER SOMMERS	
CONTRACTOR	BRINKMANN CONSTRUCTORS	







3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: Date: Issued For:	18017,19050.07,19050.08 10.18.2024 GARAGE PERMIT SET
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No. Date	Description

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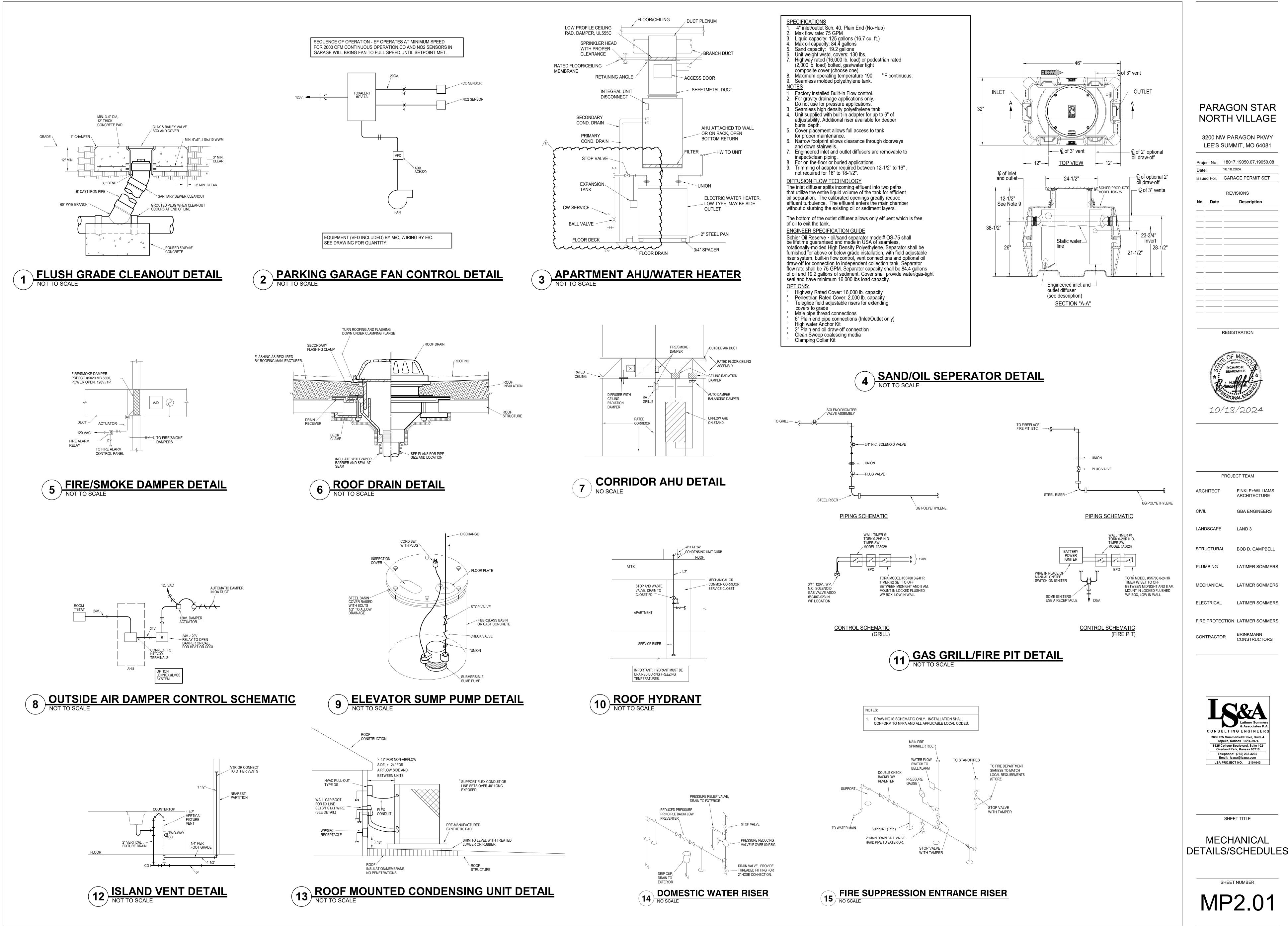
PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

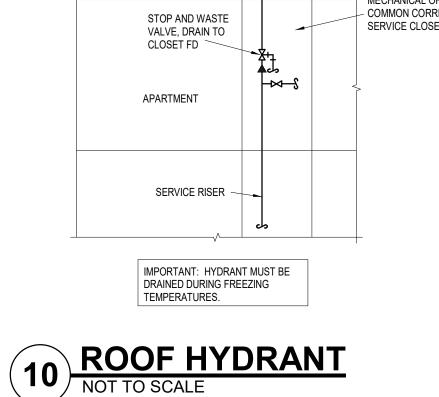


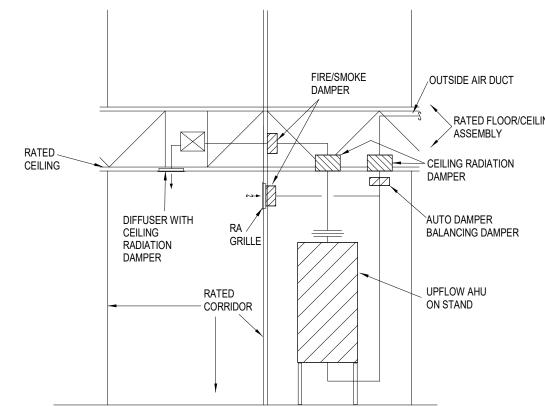


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MP1.75G



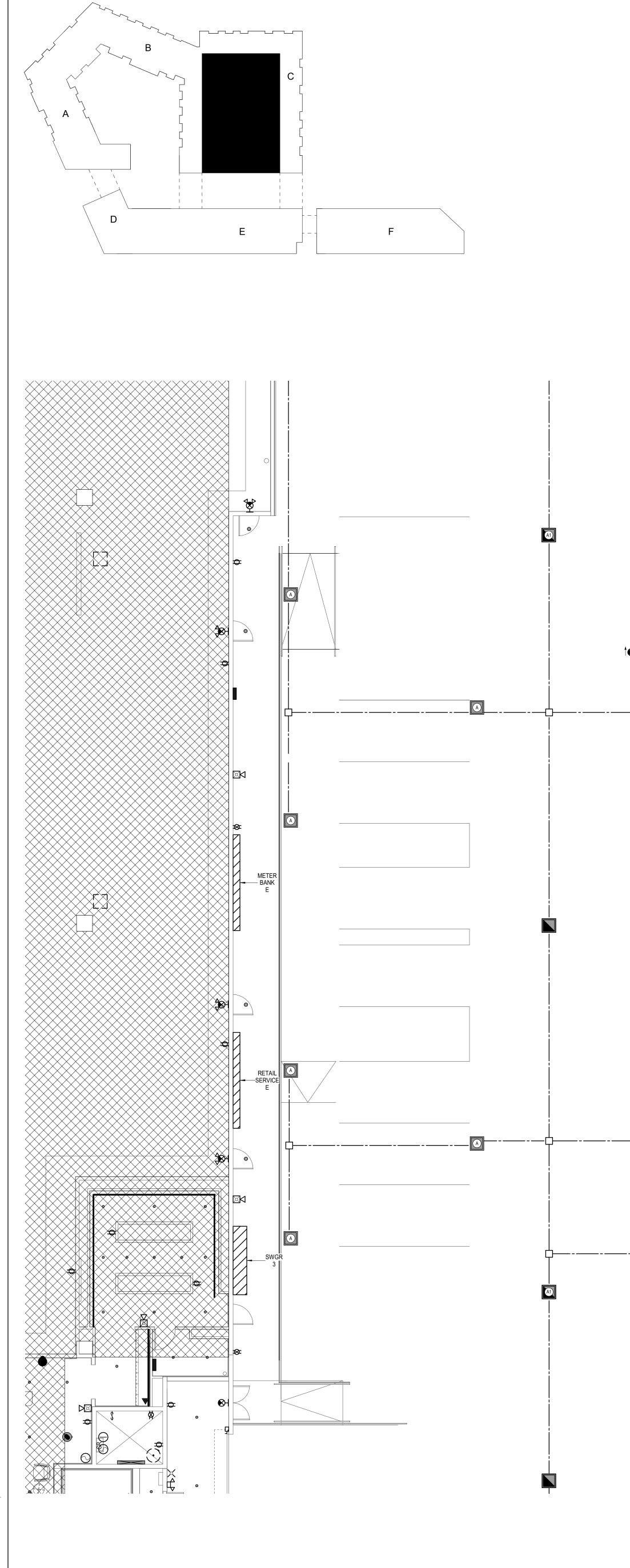




Project No.:	18017,19050.07,19050.08
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Issued For:	GARAGE PERMIT SET
	REVISIONS
No. Date	Description
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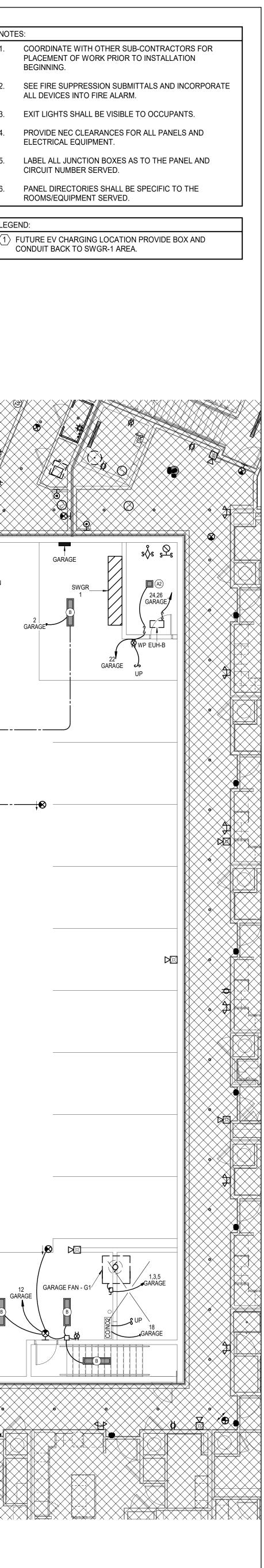
PROJECT TEAM		
RCHITECT	FINKLE+WILLIAMS ARCHITECTURE	
IVIL	GBA ENGINEERS	
ANDSCAPE	LAND 3	
TRUCTURAL	BOB D. CAMPBELL	
LUMBING	LATIMER SOMMERS	
IECHANICAL	LATIMER SOMMERS	
LECTRICAL	LATIMER SOMMERS	
IRE PROTECTION	LATIMER SOMMERS	
ONTRACTOR	BRINKMANN CONSTRUCTORS	





NORTH

NOTE	S:
1.	COORDINATE WITH OTHER SUB-CONTRACTORS FO PLACEMENT OF WORK PRIOR TO INSTALLATION BEGINNING.
2.	SEE FIRE SUPPRESSION SUBMITTALS AND INCORPO ALL DEVICES INTO FIRE ALARM.
3.	EXIT LIGHTS SHALL BE VISIBLE TO OCCUPANTS.
4.	PROVIDE NEC CLEARANCES FOR ALL PANELS AND ELECTRICAL EQUIPMENT.
5.	LABEL ALL JUNCTION BOXES AS TO THE PANEL AND CIRCUIT NUMBER SERVED.
6.	PANEL DIRECTORIES SHALL BE SPECIFIC TO THE ROOMS/EQUIPMENT SERVED.
LEGE	ND:



### PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Proje	ct No.:	18017,19050.07,19050.08
Date:		10.18.2024
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No.	Date	Description
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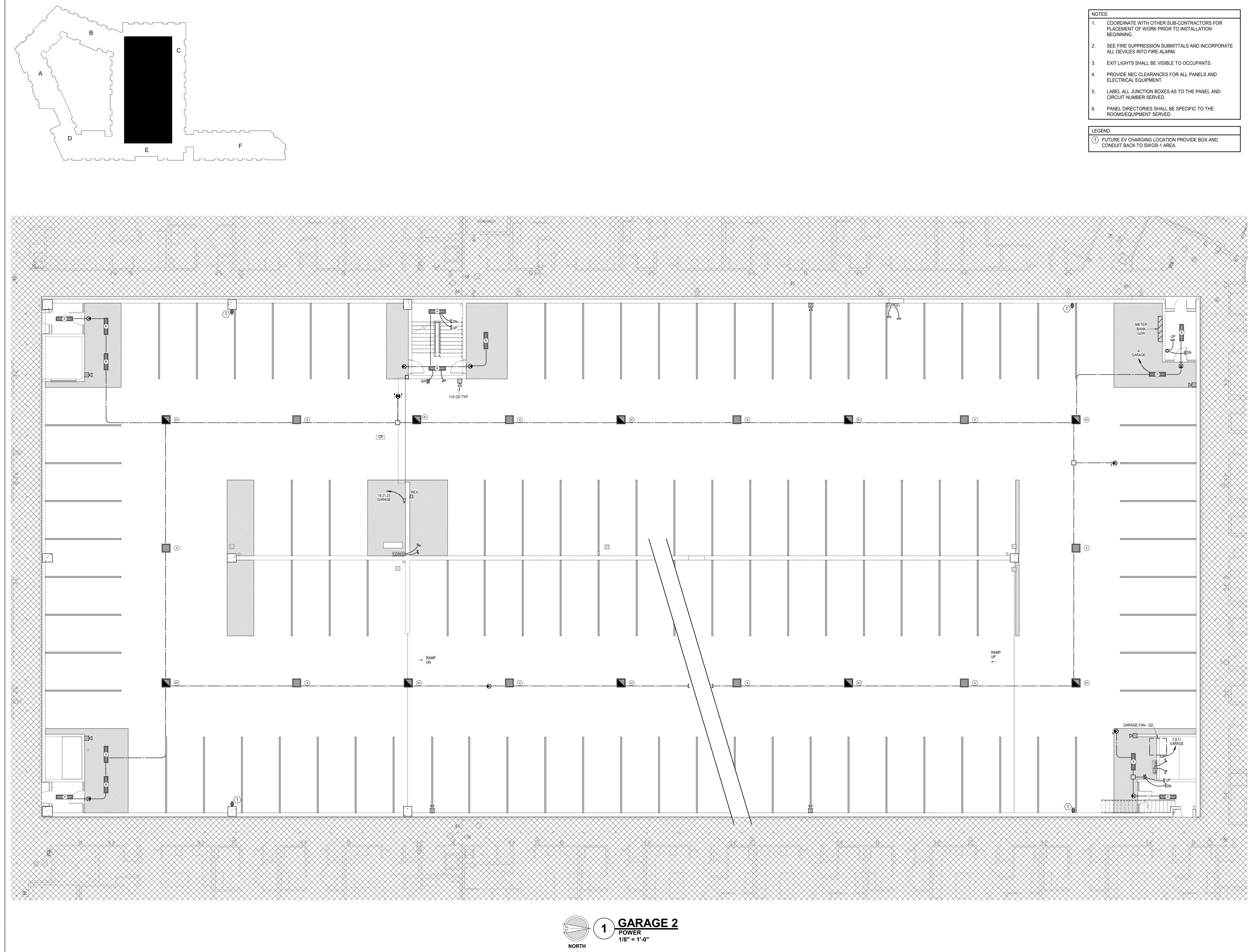
PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

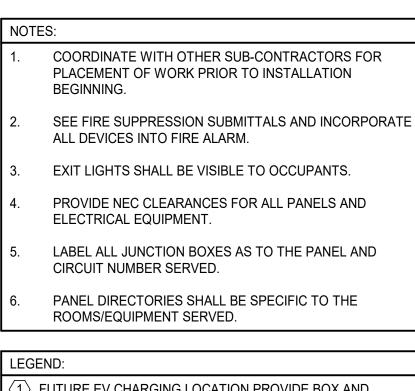


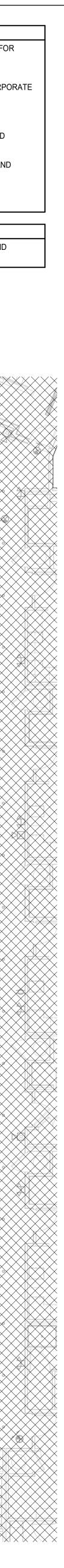


SHEET NUMBER

E1.71G







3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

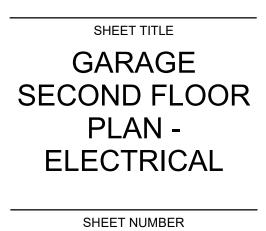
Project No.: Date:	18017,19050.07,19050.08 10.18.2024
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	REVISIONS
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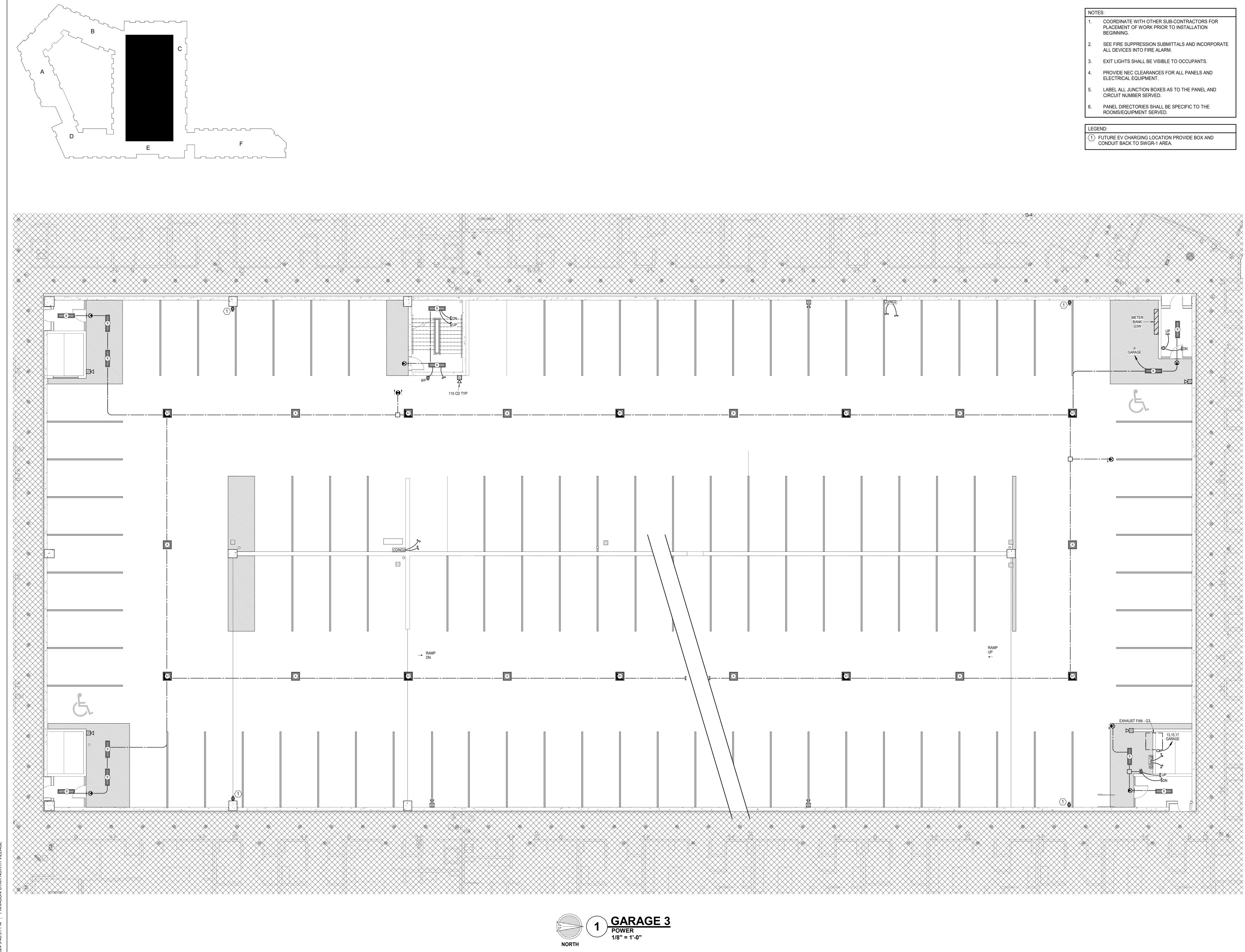


PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

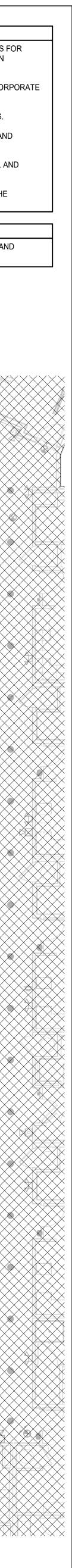




E1.72G



NOT	ES:
1.	COORDINATE WITH OTHER SUB-CONTRACTORS FO PLACEMENT OF WORK PRIOR TO INSTALLATION BEGINNING.
2.	SEE FIRE SUPPRESSION SUBMITTALS AND INCORPO ALL DEVICES INTO FIRE ALARM.
3.	EXIT LIGHTS SHALL BE VISIBLE TO OCCUPANTS.
4.	PROVIDE NEC CLEARANCES FOR ALL PANELS AND ELECTRICAL EQUIPMENT.
5.	LABEL ALL JUNCTION BOXES AS TO THE PANEL AND CIRCUIT NUMBER SERVED.
6.	PANEL DIRECTORIES SHALL BE SPECIFIC TO THE ROOMS/EQUIPMENT SERVED.
LEG	END:
	FUTURE EV CHARGING LOCATION PROVIDE BOX AND



3200 NW PARAGON PKWY \_EE'S SUMMIT, MO 6408<sup>.</sup>

Project No.: Date:		18017,19050.07,19050.08 10.18.2024
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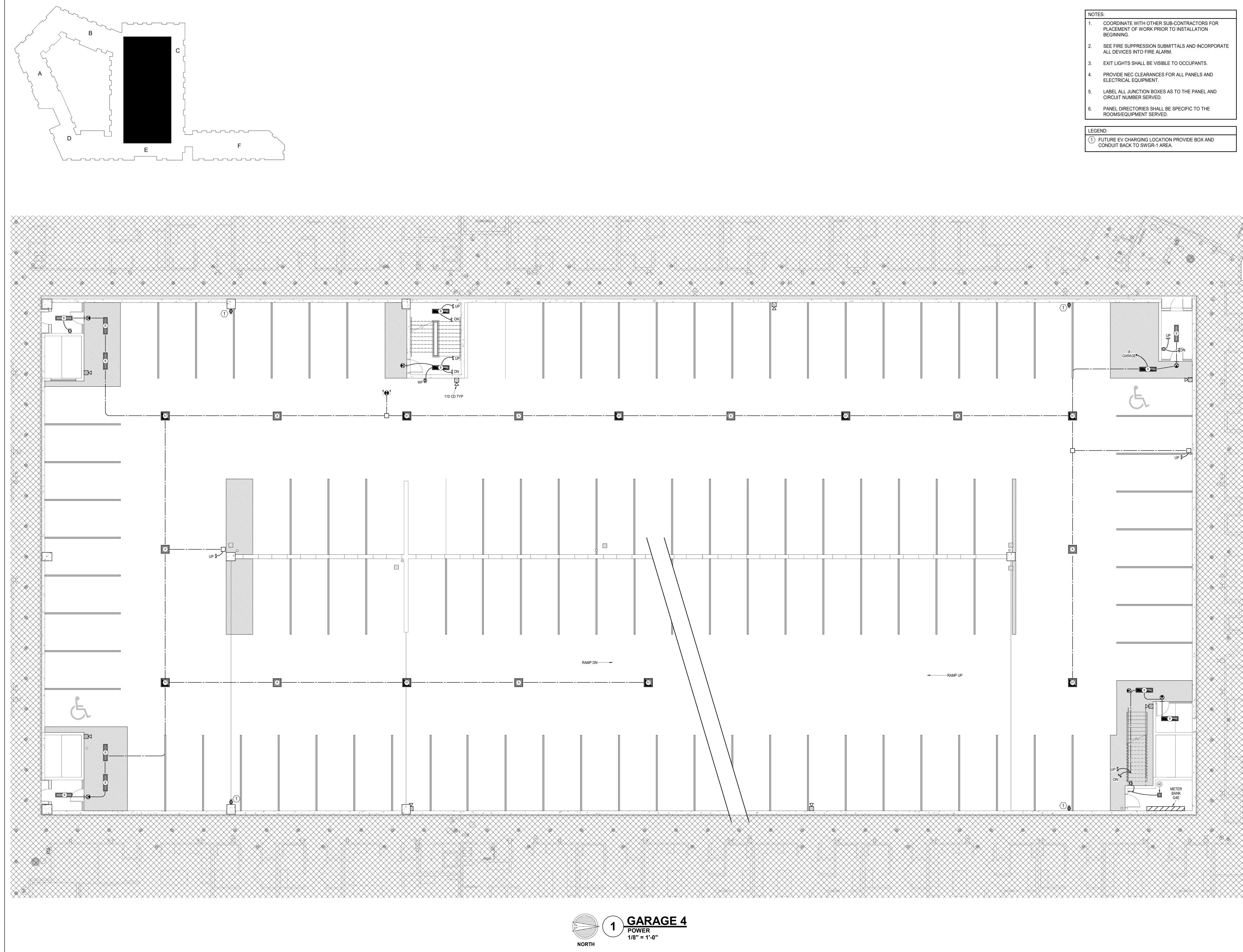
PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

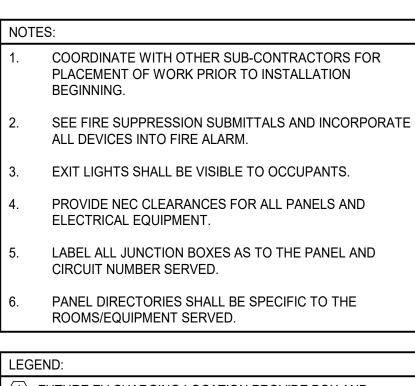


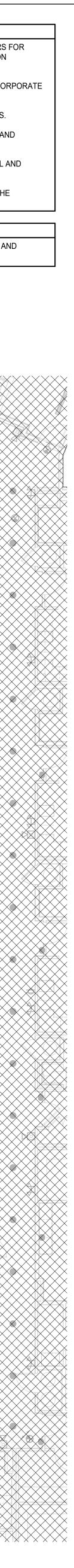


SHEET NUMBER

E1.73G







3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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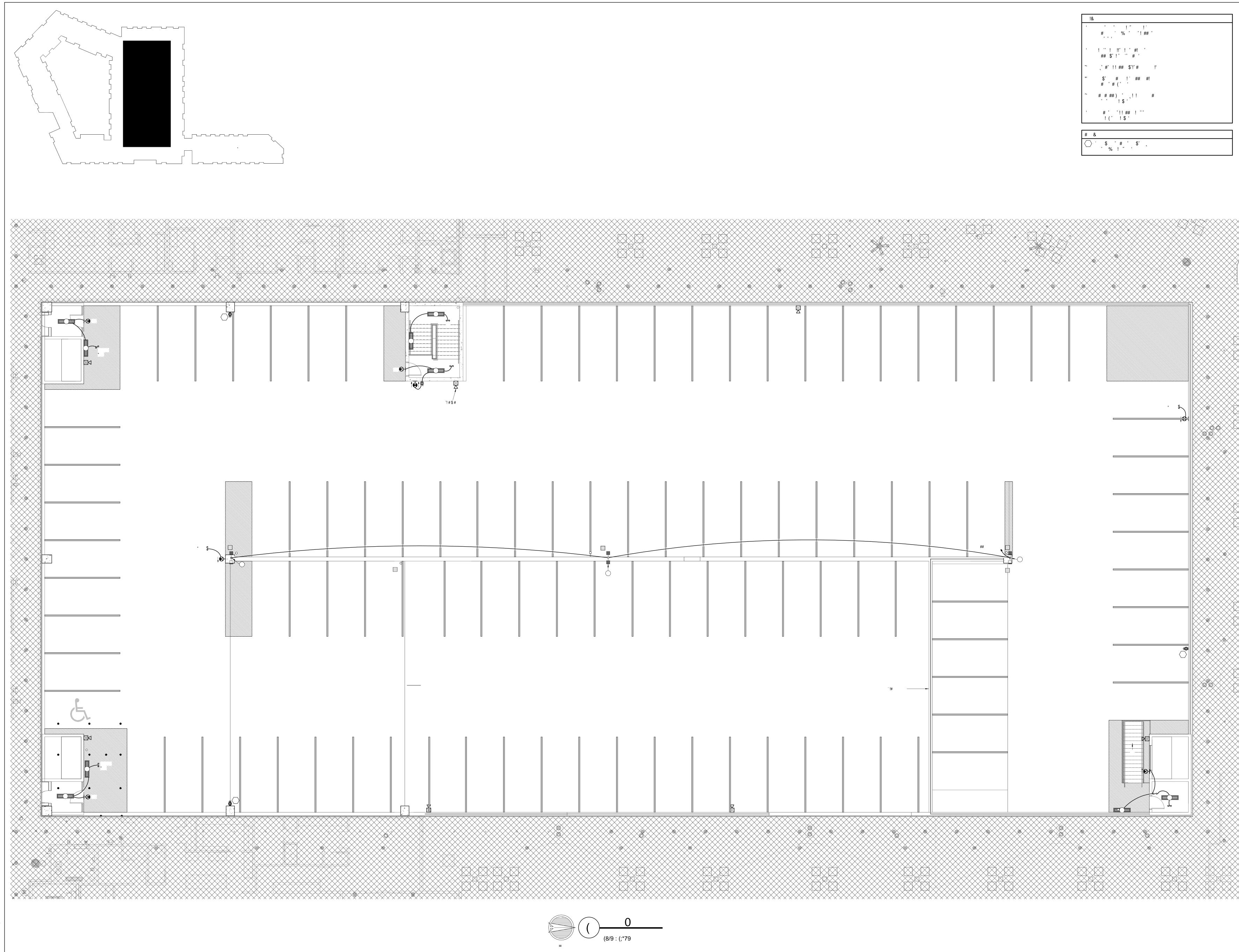
PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

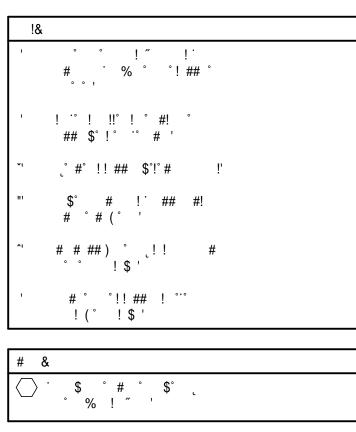


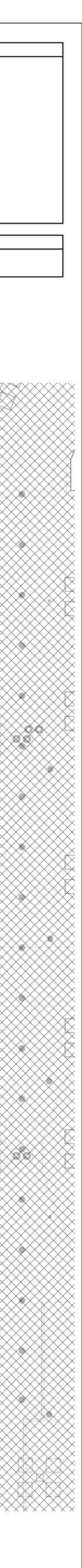


SHEET NUMBER

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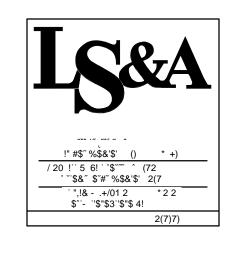


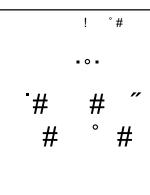


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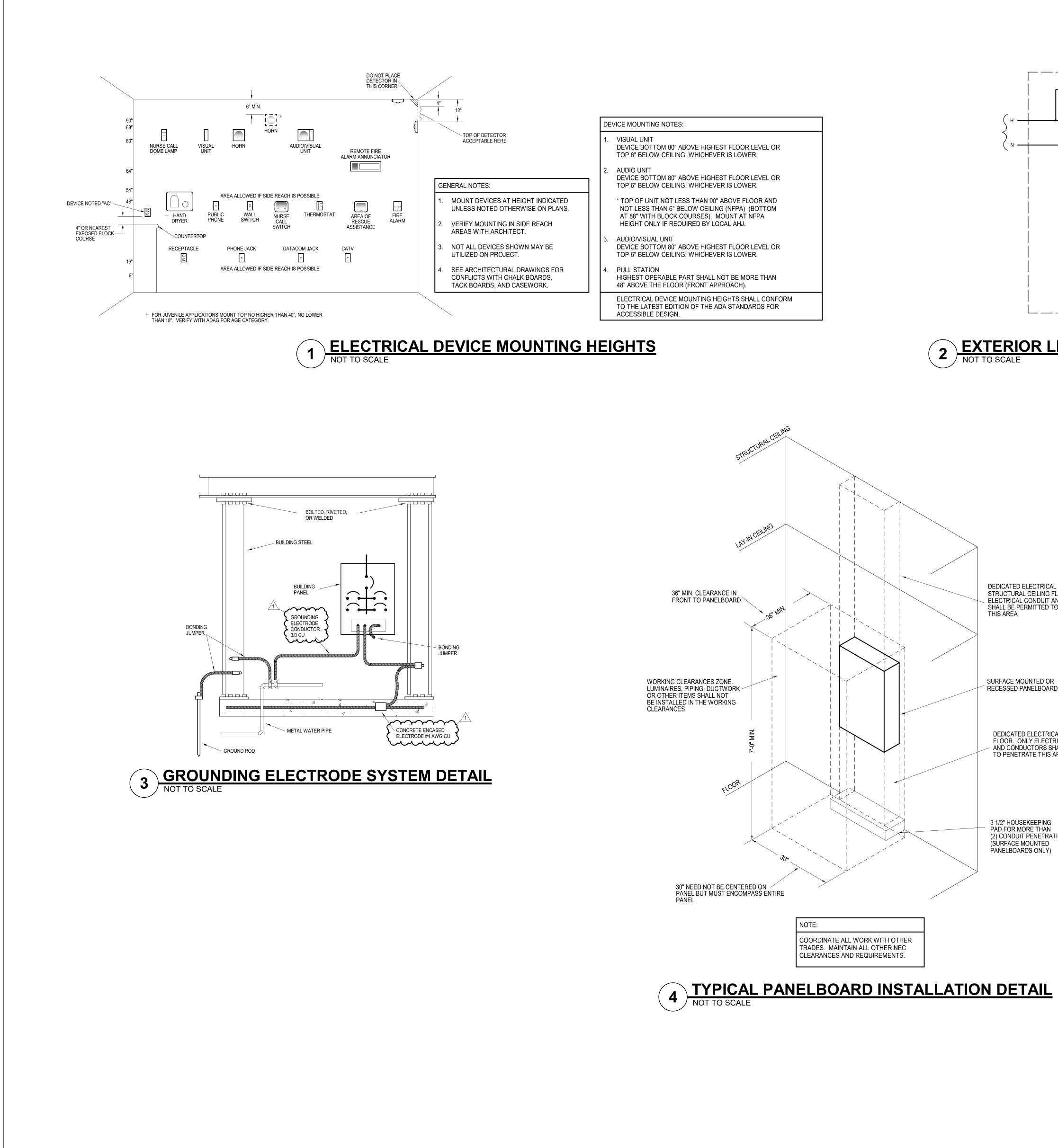
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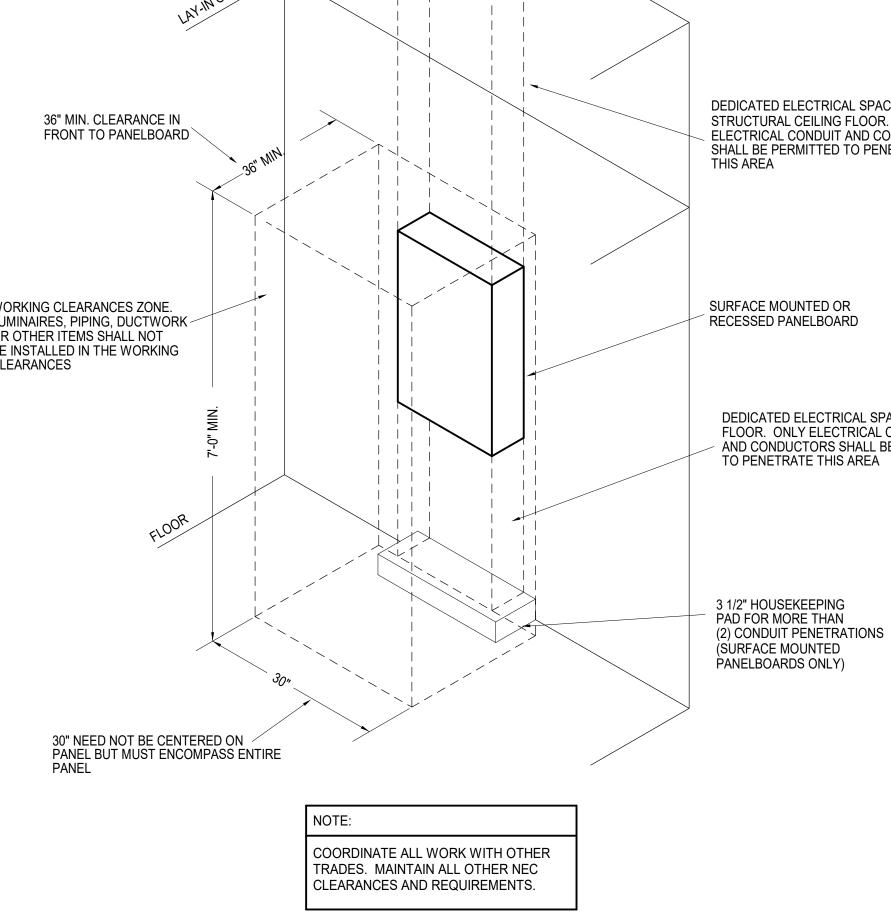
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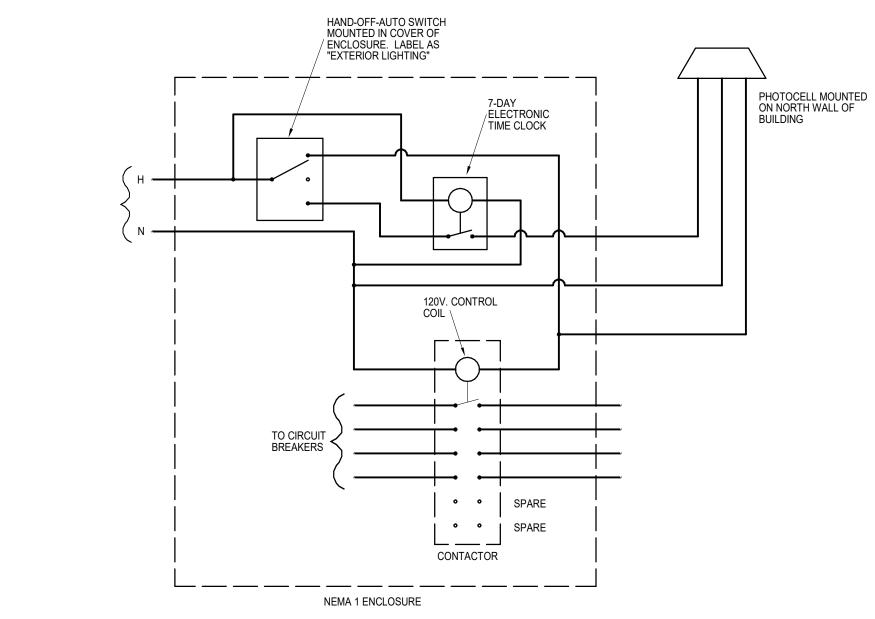


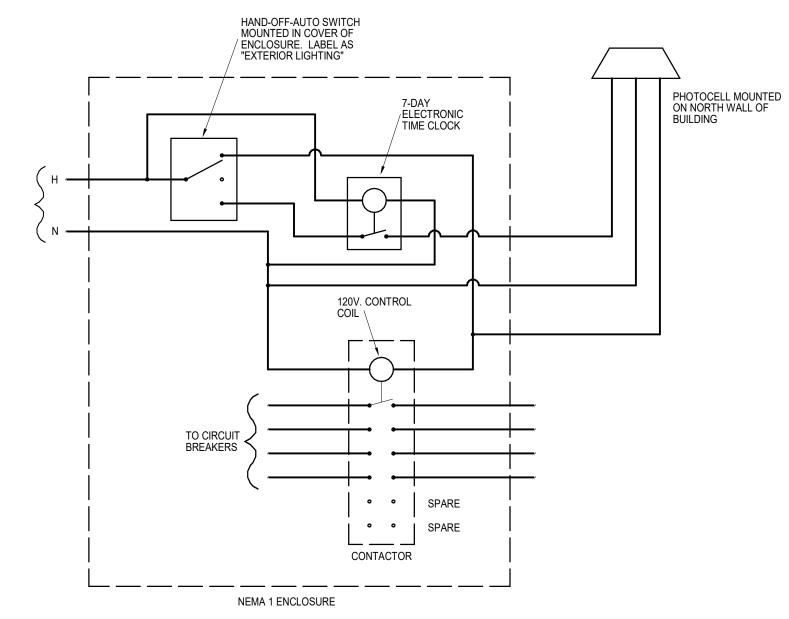




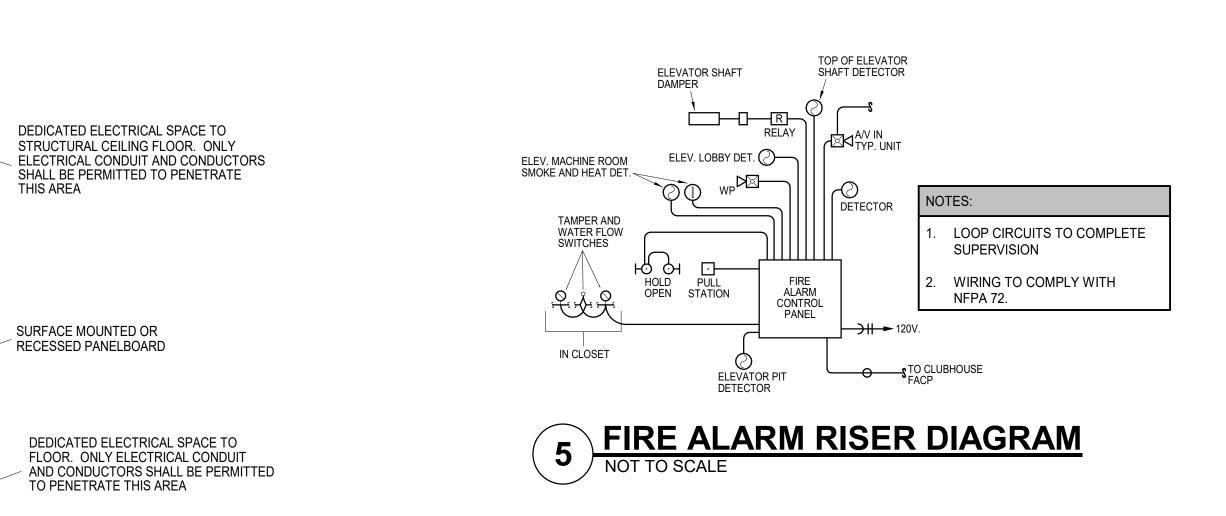








## 2 EXTERIOR LIGHTING CONTROL PANEL SCHEMATIC NOT TO SCALE



#### PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.:	18017,19050.07,19050.08							
Date:	10.18.24							
Issued For:	GARAGE PERMIT							
REVISIONS								
No. Date	Description							
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REGISTRATION



PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

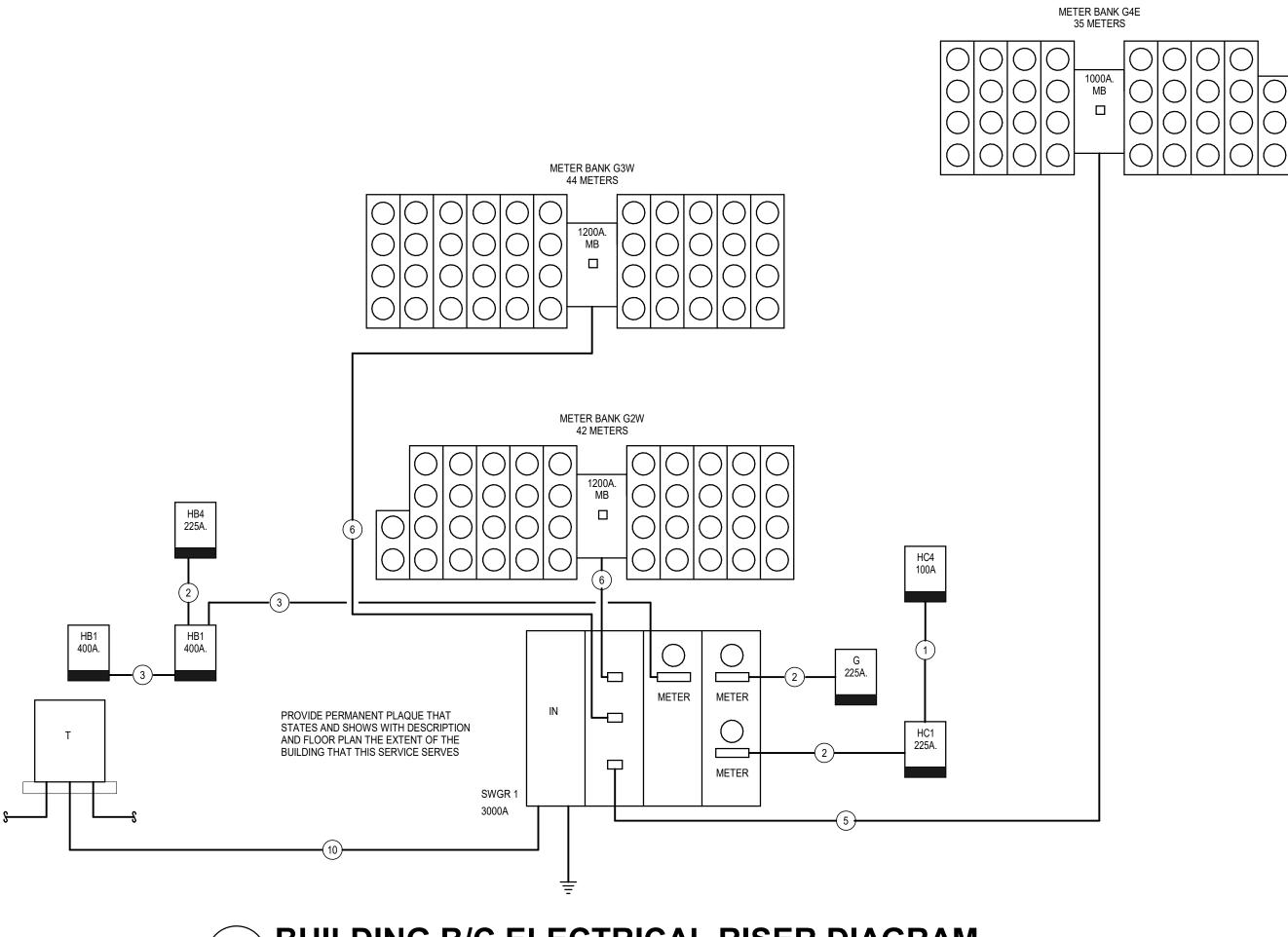




SHEET NUMBER

E3.01G

5/2024 3:47:04 PM PARAGON STAR NORTH VILLAGE



#### 2 BUILDING B/C ELECTRICAL RISER DIAGRAM NOT TO SCALE

SWITCHGEAR LOAD ANALYSIS GARAGE								
Panel Name	Total Units	Total Connected	NEC Building Diversity	Diversified KVA	Amps @ 208V/3PH	House Load	Retail Load	Total Demand (Amps)
SWGR 1	121	3,802,320 VA	0.23	875 VA	2429	560	0	2989

			wuut-ramily i	Building Load Analysis	GARAGE			
Meter Bank	Total Units	Total Connected Load	Total KVA	NEC Building Diversity	Diversified KVA	Amps @ 208V/3PH	House Load	Total Demand - Amp
Unit Type	Unit Quantity	Sum of Units						
METER BANK G2W	42	1,316,085 VA	1316	28%	369	1024	0	1024
A-1 ADA	2	57780 VA			1	1 1		
B-1	6	175140 VA				· · · · ·		
C-1	17	536180 VA						
C-1S	1	31540 VA						
C-3	9	282645 VA						
C-5	2	63530 VA						
D-3	2	65360 VA				· · · · ·		
D-4	1	32830 VA						
D-10	2	71080 VA						
METER BANK G3W	44	1,401,565 VA	1402	27%	378	1051	0	1051
A-1 ADA	2	57780 VA						
B-1	6	175140 VA						
C-1	16	504640 VA						
C-1S	2	63080 VA						
C-3	9	282645 VA						
C-5	2	63530 VA			-	·		
D-3	1	32680 VA						
D-4	2	65660 VA						
D-10	1	35540 VA						
PH-3	1	40290 VA						
PH-4	1	39690 VA						
PH-5	1	40890 VA						
METER BANK G4E	35	1,084,670 VA	1085	30%	325	904	0	904
A-1	12	346680 VA					•	
C-1S	16	504640 VA						
C-12	4	127180 VA						
D-5	3	106170 VA			_			

	FEEDER SCHEDULE										
MARK OCP SETS NO. COND. SIZE ALUM. SIZE GRD. CU CONDUIT											
			·	•							
1	100 A	1	4	#1/0	#4	1 1/2"					
2	225 A	1	4	300 KCMIL	#2	3"					
3	400 A	2	4	#4/0	#1	2"					
4	600 A	2	4	500 KCMIL	#2/0	3"					
5	1000 A	3	4	600 KCMIL	#4/0	4"					
6	1200 A	4	4	500 KCMIL	250 KCMIL	3"					
7	1600 A	5	4	600 KCMIL	350 KCMIL	4"					
9	800 A	3	4	400 KCMIL	#3/0	3"					
10	2000 A	6	4	600 KCMIL	400 KCMIL	4"					

Garage Luminaire Schedule							
MARK	DESCRIPTION	MFGR	MODEL	MOUNTING	FINISH	LAMPS	NOTES
А	Garage Light	Royal Pacific	4431OB-60-MS	surface	standard	60W 4000K	with 50% motion dimming
A1	Garage Light	Royal Pacific	4431OB-60-MS	surface	standard	60W 4000K	with 50% motion dimming/battery
A2	Surface Globe	Lithonia	LDN4CYL-40/LO4/AR/LSS/fcm	surface	white	1000 Lumen 4000K 10W	
В	Strip	Lithonia	MNSL-L46 1LL MVOLt 40k	surface	white	20W 4000K LED	with battery
С	Double head Pole	McGraw Edison	(2) GLEON-SA2C-740-U-5WQ	jb/surface	white	15,200 Lumen 4000K 115W LED	16 ft pole
X1	Exit	Lithonia	LQM SW3R	surface	white/red	LED	
X2	Exit - waterproof	Lithonia	WLTE-W-1R-EL	surface	white/red	LED	

Panel: GARAGE Location: GARAGE Supply From: Mounting: SURFACE Enclosure:				Volts: 120/208 Wye Phases: 3 Wires: 4					A.I.C. Rating: 10000 Mains Type: MLO Mains Rating: 225 A MCB Rating:					
Notes														
скт	Circuit Description	Trip	Wire Size	A	A B		с		Wire Size 12	<b>Trip</b> 20 A	Circuit Description	t Description	СКТ 2	
1		•	1			820								
3	GARAGE EF	15 A	12			1321	1600	-		12	20 A	Lighting		4
5								1321	1680	12	20 A	Lighting		6
7		45.0	10	1321	1752	1	000			12	20 A	Lighting		8
9	GARAGE EF	15 A	12			1321	600	1321	833	12 12	20 A	Lighting		10
11 13				1321				1321	833	12	20 A	Lighting		12 14
15	GARAGE EF	15 A	12	1521		1321	800			12	20 A	CO SENSO	R	14
17		10 7	12			1021	000	1321	800	12	20 A	CO SENSO		18
19		20 A		0	800	)		1021	000	12	20 A	CO SENSO		20
21	Power Space 1305		12			0	296			12	20 A	Receptacle		22
23								0	2704					24
25					2704	4				8	35 A	EUH-B		26
27							1995			12	20 A	Lighting STA	AIR 106	28
29														30
31														32
33														34
35														36
37														38
39														40
41		<b></b>		4000	0.1/4	0.10		004						42
			al Load: Amps:	1092 92			4 VA ' A		2 VA · A			_		
Leger	nd:	TOLA	i Amps.	92	<u>A</u>		<u>A</u>	04	A					
Load	Classification		Connec	ted Loa	d [	Demand Fa	actor	Estim	nated			Panel	Totals	
HVAC			17297 VA			65.00%		11243 VA				i uner		
Lighting			8652 VA			125.00%		10815 VA			Total Conn. Lo		d: 30019 VA	
Other			2400 VA			100.00%		2400 VA			Total Est. Demand:			
Power			500 VA			100.00%		500 VA			Total Conn.:			
Receptacle			1440 VA			100.00%		1440 VA			Total Est. Demand:		72 A	
Notes	:													

#### PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.:		18017,19050.07,19050.08				
Date:		10.18.2024				
Issued For:		GARAGE PERMIT SET				
		REVISIONS				
No.	Date	Description				
<u> </u>	Date	Description				

REGISTRATION



PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS





SHEET NUMBER

E3.02G