

Cook, Flatt & Strobel Engineers 1100 W. Cambridge Circle Drive, Suite 700 Kansas City, Kansas 66103 913.627.9040

September 12, 2024

David N. Olson SWP XIII, LLC P.O. Box 24302 Overland Park, KS 66283

Re: The Streets of West Pryor Lot 13 Pavement Section

Lee's Summit, Missouri

CFS # 23-1128

Mr. Olson,

Cook, Flatt & Strobel (CFS) Engineers, P.A. has reviewed the pavement section for the reference project. The contractor used crushed concrete granular base in lieu of crushed limestone granular base. We have evaluated the crushed concrete base material and in our professional opinion the crushed concrete material submitted by the contractor as indicated in the attached KTI test report number 418162C.025 is an equivalent base aggregate product to MODOT type 5 for substitution as base aggregate for the Project's specified pavement section.

The Florida Department of Transportation recommends the same structural coefficient for crushed concrete base as for crushed limestone base. Pavement design evaluation was performed using the Tensar Plus flexible pavement design software. Below is a breakdown of the current Lee's Summit requirements compared with the proposed pavement sections.

Material	Lee's Summit	Lee's Summit	Proposed	Proposed
	Light Duty	Heavy Duty	Pavement	Pavement
	Pavement (in)	Pavement (in)	Light Duty	Heavy Duty
			(in)	(in)
Surface Asphalt APWA Type	1.5	1.5	1.5	1.5
III				
Base Asphalt APWA Type I	4.0	5.0	4.0	5.0
MoDOT Type 5 Base Rock	6.0	6.0	NA	NA
Crushed Concrete Type 5	NA	NA	6.0	6.0
Geogrid	Yes	Yes	Yes	Yes
Sub-Grade Material	Compacted	Compacted Soil	Soil Rock	Soil Rock
	Soil		Fill	Fill
Structural Number	3.160	3.560	3.160	3.560



A CBR value of 3.3 was utilized for the pavement sub-grade. Based on the input data, the pavement sections are equivalent. This information has been reviewed and approved by the civil engineer of record for the project.

It should be noted that CFS tested the fill material for the site and observed and passed a proof-roll on the sub-grade prior to placement of the geo-grid and base rock. A proof-roll was performed and passed on the base rock prior to asphalt placement as well.

Please contact CFS with further questions. 913-627-4090

Respectfully,

Cook, Flatt & Strobel Engineers, P.A.

McEACHRON

NUMBER

Adam M. McEachron, P.E.

Senior Geotechnical Engineer

Sam Malinowski, P.E. Project Civil Engineer

Attachments: FDOT Flexible Pavement Design Coefficients

MDOT Section 1007 Aggregate Base

Laboratory Testing of Crushed Concrete Utilized

TABLE 5.4

STRUCTURAL COEFFICIENTS FOR DIFFERENT PAVEMENT LAYERS
(New Construction or Reconstruction)

Layer Type	Layer Coeff. per inch	Spec.
FC-5	0.00	337
FC-12.5, FC-9.5	0.44	337
Superpave Type SP (SP-9.5, SP-		
12.5, SP-19.0)	0.44	334
Limerock (LBR 100)	0.18	200
Cemented Coquina (LBR 100)	0.18	200
Shell Rock (LBR 100)	0.18	200
Bank Run Shell (LBR 100)	0.18	200
Graded Aggregate (LBR 100)	0.15	204
Recycled Concrete Aggregate (LBR 120)	0.15	204
Type B-12.5	0.30	234
Limerock Stab. (LBR 70)	0.12	230
Shell Stab. (LBR 70)	0.10	
Sand Clay (LBR 75)	0.12	
Soil Cement (500 psi)	0.20	
Soil Cement (300 psi)	0.15	
Type B Stab. (LBR 40)	0.08	
Type B Stab. (LBR 30)	0.06	
Type C Stab.	0.06	
Cement Treated (300 psi)	0.12	
Lime Treated	0.08	

SECTION 1007 AGGREGATE FOR BASE

1007.1 Scope. This specification covers aggregate to be used for base.

1007.2 Type 1 Aggregate.

1007.2.1 Type 1 aggregate for base shall consist of crushed stone, sand and gravel or reclaimed asphalt or concrete. The aggregate shall not contain more than 15 percent deleterious rock and shale. The fraction passing No. 40 sieve shall have a maximum plasticity index of six. Any sand, silt and clay and any deleterious rock and shale shall be uniformly distributed throughout the material

1007.2.2 The aggregate shall be in accordance with the following gradation requirements:

Sieve	Percent by Weight
Passing 1-inch	100
Passing 1/2-inch	60-90
Passing No. 4	35-60
Passing No. 30	10-35

1007.3. Type 5 Aggregate.

1007.3.1 Type 5 aggregate for base shall consist of crushed stone, sand and gravel or reclaimed asphalt or concrete. The aggregate shall not contain more than 15 percent deleterious rock and shale. The fraction passing the No. 40 sieve shall have a plasticity index not to exceed six. Any sand, silt and clay, and any deleterious rock and shale shall be uniformly distributed throughout the material.

1007.3.2 Type 5 aggregate shall be in accordance with the following gradation requirements:

Sieve	Percent by Weight
Passing 1-inch	100
Passing 1/2-inch	60-90
Passing No. 4	35-60
Passing No. 30	10-35
Passing No. 200	0-15

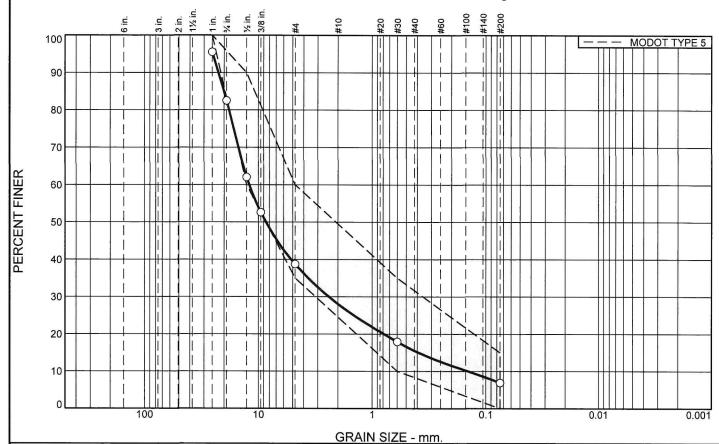
1007.4. Type 7 Aggregate.

1007.4.1 Type 7 aggregate for base shall consist of crushed stone, sand and gravel, or reclaimed asphalt or concrete. The aggregate shall not contain more than 15 percent deleterious rock and shale. The fraction passing the No. 40 sieve shall have a plasticity index not to exceed six. Any sand, silt and clay, and any deleterious rock and shale shall be uniformly distributed throughout the material.

1007.4.2 Type 7 aggregate shall be in accordance with the following gradation requirements:

Sieve	Percent by Weight
Passing 1 1/2-inch	100
Passing 1-inch	70-100
Passing No. 8	15-50
Passing No. 200	0-12

Particle Size Distribution Report



% +3"		% Gravel		% Sand		% Fines	
				Coarse	Fine	Silt	Clay
				12.6	8.6	6.9	
SIEVE SIZE	PERCENT	SPEC.*	PASS?	-	Soil Do	scription	

SIEVE SIZE	PERCENT	SPEC.*	PASS?
OR DIAMETER	FINER	PERCENT	(X=NO)
1"	95.5	100	X
3/4"	82.6		
1/2"	62.0	60-90	
3/8"	52.6		
#4	38.7	35-60	
#30	17.9	10-35	
#200	6.9	0-15	
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Soil Description Client Drop 1" minus Recycled Concrete					
PL= 23	Atterberg Limits LL= 28	PI= 5			
D ₉₀ = 22.4263 D ₅₀ = 8.5750 D ₁₀ = 0.1428	Coefficients D85= 20.0697 D30= 2.4053 Cu= 84.49	D ₆₀ = 12.0633 D ₁₅ = 0.3905 C _c = 3.36			
USCS= GP-GM	Classification AASHTC)= A-1-a			
Remarks KTI Report No.418162C.025					

MODOT TYPE 5

Source of Sample: .025 Bulk Sample

Date: 3/19/24



Client: Metropolitan Concrete Recycle

Project: Metropolitan Concrete Recycle-Testing

Project No: 418162C

Figure

Tested By: DD

Checked By: Otto J. Kruger, Jr., PE

A. Sugs