### **MACRO STORM WATER DRAINAGE STUDY**

**Oldham Village** 

SW Quadrant SW Oldham Parkway & MO 291 South

Site Acreage: 49.68 Acres (Future Buildout)

Lee's Summit, MO

PREPARED BY:



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Schlicht, PE

Submittal Date: July 14, 2023

Revision

Date	Comment	By
		15
		1.16

### **TABLE OF CONTENTS**

- **1. REPORT COVER SHEET**
- 2. TABLE OF CONTENTS
- 3. GENERAL INFORMATION
  - **3.1 FEMA FLOODPLAIN DETERMINATION**
  - 3.2 NRCS SOIL CLASSIFICATION
- 4. METHODOLOGY
- 5. EXISTING CONDITIONS ANALYSIS
- 6. PROPOSED CONDITIONS ANALYSIS 6.1 DETENTION
- 7. 40 HOUR EXTENDED DETENTION
- 8. CONCLUSIONS & RECOMMENDATIONS
- 9. EXHIBITS

### **3. GENERAL INFORMATION**

The macro storm study has been prepared to evaluate potential negative downstream hydraulic impacts and propose potential mitigation measures associated with the redevelopment of the proposed Planned Mixed Use Development, Oldham Village. The proposed redevelopment will consist of three multi-family communities in addition to multiple commercial establishments. The site is located at the southwest corner of SW Oldham Parkway and MO 291 Highway. The proposed master development contains 45.41 acres with future potential up to 49.68 acres. The existing site is developed and contains primarily hard surface. An existing earthen detention basin is located on the southwest corner of the site. The detention basin drains into the upper most reach of Cedar Creek. The site consists of land located in Section 7, Township 47 North, and Range 31 West. See Exhibit A for an aerial view of the site along with a historical aerial of the site and surrounding area. A proposed site plan is also included in Exhibit A.

### **3.1 FEMA FLOODPLAIN DETERMINATION**

The property is located in an Area of Minimal Flood Hazard, Zone X, according to FEMA Firm Map Number 29095C0419G, effective January 20, 2017.

See Exhibit B for a FEMA FIRMette which includes the proposed project area.

### **3.2 NRCS SOIL CLASSIFICATION**

Soil classifications published by the United States Department of Agriculture/National Resources Conservation Service (USDA/NRCS) website for Jackson County, Missouri, Version 24, August 31, 2022. The existing site contains three major soil types:

10082	Arisburg-Urban Land Complex, 1 to 5 Percent Slopes Hydrologic Soils Group (HSG): Type C
10181	Udarents-Urban Land-Sampsel Complex, 5 to 9 Percent Slopes (HSG): Type C
99033	Udarents-Urban Land Complex, 2 to 9 Percent Slopes (HSG): Type C

See Exhibit C for a detailed soils report of the proposed project site.

### 4. METHODOLOGY

This Macro Storm Drainage Study has been prepared to evaluate potential hydrologic impacts from the proposed redevelopment and recommend improvements to eliminate any potential negative impacts. The study utilized existing contours to create the Existing Drainage Area Map. The study conforms to the requirements of the City of Lee's Summit, Missouri "Design and Construction Manual" and all applicable codes and criteria referred to therein.

Using the above criteria, the proposed site was evaluated using SCS Methods to calculate storm runoff volumes, peak rates of discharge, existing and proposed hydrographs and required storage volumes for detention facilities. The analysis contains results for the 2, 10 and 100-year design storms.

### 5. EXISTING CONDITIONS ANALYSIS

The site consists of one (1) drainage basin. The basin includes a substantial amount of offsite area to the north, south and east. The basin drains to Point of Interest A located approximately 260 feet west of the sites western property boundary. POI A is the termination point for the FEMA FIS on Cedar Creek. The tributary area to the

POI is 150.52+/- acres. The basin consists of 49.68 acres of proposed master development which includes potential future parcels and 100.84 acres of right-of-way and offsite area. The drainage basin has been essentially built out in the past and therefore currently has both open and enclosed storm sewer systems active throughout the basin. The basin generally drains to Cedar Creek located along the west central portion of the property. The basin is fairly symmetric with nearly equal portions draining from the north, east and south. The existing onsite detention basin area (Tract B) adjacent to Cedar Creek will continue to serve as detention for the proposed redevelopment.

An Existing Drainage Map may be found in Exhibit D. Hydraflow Hydrograph software was utilized to calculate SCS Method peak discharge rates. A complete breakdown of Existing and Proposed hydrographs may be found in Exhibit E. The following tables summarize the results of the Existing Conditions analysis.

### **Table 5.1 Existing Conditions Subarea**

Subarea	Area (ac.)	Curve Number	Tc (min)
А	150.52	88	24.9

### Table 5.2 Existing Conditions Runoff Data: Peak Discharge Rates

Subarea	Q2 (cfs)	Q10 (cfs)	Q100 (cfs)
А	347.19	580.94	923.43

Per APWA Section 5608.4 and City of Lee's Summit criteria, the performance criteria for detention is to provide detention to limit peak flow rates at downstream points of interest to maximum release rates:

- 50% storm peak rate less than or equal to 0.5 cfs per site acre
- 10% storm peak rate less than or equal to 2.0 cfs per site acre
- 1% storm peak rate less than or equal to 3.0 cfs per site acre

The allowable offsite contribution was determined by the area ratio method. The allowable offsite peak discharge is the product of the ratio of offsite area to onsite area multiplied by a given storms existing peak discharge rate. The allowable peak discharge rate is the sum of the offsite allowable plus the onsite allowable per APWA Section 5608.4. See allowable 100-year peak discharge rate calculation below.

Example (100-YR):  $[(100.84 / 150.52 \times 923.43) + (49.68 \times 3.0)] = 449.50 \text{ cfs}$ 

### Table 5.3 Existing Conditions APWA Allowable Peak Discharge Release Rates

POI	Onsite Area (ac.)	Offsite Area (ac.)	Q2 (cfs)	Q10 (cfs)	Q100 (cfs)
А	49.68	100.84	257.44	488.56	767.69

There are a few very minor subareas that are peripheral (free release) areas on the site consisting mainly of turf with no hard infrastructure improvements. These areas are not being negatively impacted by the proposed improvements. Subarea A will be the focus of this report.

### 6. PROPOSED CONDITIONS ANALYSIS

The proposed conditions analysis will include potential future Oldham Village lots located along MO 291 Highway south of SW Oldham Parkway. The proposed onsite area including potential future lots is 49.68 acres. Tributary runoff will be conveyed via both open and enclosed storm sewer systems to POI "A". A new retention system shall be constructed to attenuate post development runoff from Subarea A1 which includes 41.80 acres of

right-of-way and offsite area. Detained peak discharge rates from Subarea A1 will be combined with peak discharge rates from Subarea B to determine the overall peak discharge rates at POI "A". The proposed peak discharge rates will be compared to allowable discharge rates to determine if they meet or exceed the City's Comprehensive Control Storm Water Management criteria. The Proposed Drainage Map may be found in Exhibit F.

### **Proposed Flow Rates**

Existing Subarea A has been divided into two subareas to account for proposed retention. Subarea A1 is located generally in the north and accounts for all proposed and potential future improvements for Oldham Marketplace. Subarea 1 will drain via open and enclosed storm sewer systems to a new earthen retention basin located adjacent to Cedar Creek on a parcel of property labeled Tract B. The remainder of the property which will not be detained is generally located in the southern portion of the basin and will be referred to as Subarea B. The composite curve numbers utilized for Subareas A and B consist of the following classifications and land usage specific curve numbers; right-of-way and single family residential 82, multi-family residential 88 and commercial 94.

### **Table 6.1 Proposed Conditions Subarea Data**

Subarea	Area (ac.)	Composite CN	Tc (min)
A1	91.48	89	18.2
В	59.06	85	24.9

Subarea	Q2 (cfs)	Q10 (cfs)	Q100 (cfs)
A1	258.11	424.44	667.95
В	121.55	212.36	347.29
Combined (A1 + B)	379.66	636.80	1015.24

#### Table 6.2 Proposed Conditions Runoff Data: Subarea Peak Discharge Rates

As shown in Table 6.2 above Subarea A1 will require detention to attenuate storm runoff peak discharge rates at or below regulatory levels at POI A.

### **6.1. DETENTION**

A new single stage earthen retention basin A is being proposed in Subarea A1 to attenuate proposed peak discharge rates. Following are a list of design parameters for the retention system.

Designation: Retention Basin A Type: Earthen Basin Side Slopes: 3:1 Max. Bottom Slope: 2% Min., Turf Lined Basin Bottom Elevation: 996.00 Basin Top Berm Elevation: 1018.75 Basin Volume: 911,465 cf @ 1018.00 to normal pool 1004.00 Control Structure: 8'x6' Precast Concrete Box with Interior 6" Baffle/Weir Wall Baffle Wall Orifices: (12) 1" Diameter on 4" Centers, FL=1004.00 (Bottom Orifice) Baffle Wall Crest Elevation: 1008.25 Control Structure Top Elevation: 1016.85 Control Structure Overflow Weir Openings: N/A – NO Field Inlet Openings Control Structure Influent Pipe: 72" RCP, FL (In) = 1000.00, FL (Out) = 1004.00, L=63.6', S= 6.29%

5 | Page

Control Structure Effluent Pipe: 72" RCP, FL (In) = 1003.60, FL (Out) = 998.00, L=62.4', S=8.97% Emergency Spillway: Earthen Broad Crested Weir, Crest Elevation=1016.85, Crest Length=200' Consecutive 100-YR Q=667.95 cfs, Emergency Spillway HGL=1017.75, Freeboard=1.00' Normal Pool Elevation: 1004.00 Pond Bottom: 996.00 - 8' of water volume includes required siltation allowance

The Detention Basin Plan for the Development may be found in Exhibit G. Basin A emergency spillway calculations may be found in Exhibit H. See Table 6.3 for a summary of detention basin data.

#### Peak Q In Peak Q Out Tp Out Tp In Peak Max. Storage Vol. (cf) (cfs) (min.) (cfs) (min) W.S.E. Basin E 724 737 363,450 2-Year 258.11 105.72 1011.23 10-Year 424.44 724 235.46 734 1013.41 516,840

321.15

### **Table 6.3 Proposed Conditions Detention Basin A Data**

723

667.95

100-Year

As shown in the table above all proposed peak discharge rates from Subarea 1A have been attenuated. See Table 6.4 below for a summary of proposed peak discharge rates at POI "A" which consists of combined subareas post detained A1 and B.

### Table 6.4 Proposed Conditions Post Detention Point of Interest Peak Discharge Rates

Point of Interest	Q2 (cfs)	Q10 (cfs)	Q100 (cfs)
А	208.22	437.28	653.97

735

1016.35

758,297

As shown in the above table all peak discharge rates attributable to Proposed POI "A" have been attenuated below both Existing and Allowable Peak Discharge rates as outlined in Tables 5.2 and 5.3, respectively.

Table 6.5 below provides a comparison of runoff data between Proposed, Existing and Allowable Conditions for the Proposed Development.

### **Table 6.5 Point of Interest Discharge Comparison**

POI	Condition	Q2 (cfs)	Q10 (cfs)	Q100 (cfs)
	Proposed	208.22	437.28	653.97
	Existing	347.19	580.94	923.43
А	Difference	-138.97	-143.66	-269.46
	Allowable	257.44	488.56	767.69
	Difference	-49.22	-51.28	-113.72

All proposed peak discharge rates as shown in Table 6.5 will be significantly below allowable.

### 7.40 HOUR EXTENDED DETENTION

In addition to mitigation of peak flow rates, APWA Section 5608.4 also requires 40 hour extended detention of runoff from the local 90% mean annual event (1.37"/24-hour rainfall). The proposed detention facility will release the water quality event over a period of 40-72 hours. See Exhibit I for 40 hour extended detention calculations for the basin.

### 8. CONCLUSIONS & RECOMMENDATIONS

Runoff from the Development will be reduced below both existing and allowable for the Subarea. A retention basin is being proposed in Subarea A1 to attenuate peak discharge rates. Retention Basin A1 will attenuate all proposed peak discharge rates below both Existing and Allowable. It is the opinion of the Engineer that the proposed storm water management improvements outlined in the report will mitigate any negative hydraulic impacts onsite and downstream and therefore recommends approval of said improvements and the storm study.

### 9. EXHIBITS

- Exhibit A
  - Aerial View of Site
  - Historical Aerial View of Site & Surrounding Area
  - Overall Site Plan
- Exhibit B
  - FEMA FIRMette
- Exhibit C
  - NRCS Soils Report
- Exhibit D
  - Existing Drainage Area Map
- Exhibit E
  - Hydraflow Hydrograph Report
- Exhibit F
  - Proposed Drainage Area Map
- Exhibit G
  - Detention Plan
- Exhibit H
  - Emergency Spillway Calculations
- o Exhibit I
  - 40 Hour Extended Detention Calculations

# Exhibit A

### **Aerial View of Site**

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# **Aerial View of Surrounding Area**

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**Overall Site Plan** 

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INDEX OF SHEETS: C.100 ~ PRELIMINARY DEVEL C.101 ~ SITE PLAN NORTH C.102 ~ SITE PLAN SOUTH C.200 ~ GRADING PLAN NOR C.201 ~ GRADING PLAN SOU C.300 ~ STORM SEWER PLAN C.301 ~ STORM SEWER PLAN C.400 ~ SANITARY PLAN NOR C.401 ~ SANITARY PLAN SOU C.500 ~ WATER PLAN NORTH C.501 ~ WATER PLAN SOUTH L.100 ~ LANDSCAPE PLAN SOUTH L.102 ~ LANDSCAPE PLAN SOUTH L.101 ~ LANDSCAPE PLAN SOUTH L.101 ~ LANDSCAPE PLAN SOUTH L.102 ~ LANDSCAPE PLAN SOUTH L.101 ~ LANDSCAPE PLAN SOUTH L	OPMENT PLAN TH TH NORTH NORTH NORTH WTH H NORTH SOUTH DETAILS				ENGINEERING & ENGINEERING & ENGINEERING & ENGINEERING & ENGINEERING & ES 30TH S0 SE 30TH LEE'S SUMMIT P:(816) 623-98881
					Professional Registration Missouri
Site Improvement Notes					Engineering 2005002186-D Surveying 2005008319-D Kansas
Sanitary Sewer Improvements					Engineering E-1695 Surveying LS-218 Oklahoma
– The site will utilize the existing Nater Main Improvements	sanitary sewer on the	east side of SE M 291 .	HWY.		Engineering 6254 Nebraska Engineering CA2821
– The existing 16" water main loc Storm Sewer	cated on the east side	of SE M 291 HWY.			
-Enclosed pipe systems and inlead toward the existing public storm	ts will collected and co sewer system.	onvey the onsite storm w	rater runoff and direct it		$\overline{\kappa}$
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Proposed Storm Sewer					
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Proposed 8" D.I.P. Water	W·				
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IE FOLLOWING LIST OF UTILITY COMPANI JARANTEE OR WARRANTY THAT THIS LIS SPONSIBLE FOR CONTACTING ALL UTILI ONSTRUCTION AND VERIFYING THE ACTU IGINEERING SOLUTIONS AT 816.623.9888 EVERGY ~ 298-1196 MISSOURI GAS ENERGY ~ 756-5261 SOUTHWESTERN BELL TELEPHONE ~ 76 COMCAST CABLE ~ 795-1100 WILLIAMS PIPELINE ~ 422-6300 CITY OF LEE'S SUMMIT PUBLIC WORKS ~ CITY OF LEE'S SUMMIT PUBLIC WORKS II CITY OF LEE'S SUMMIT PUBLIC WORKS II CITY OF LEE'S SUMMIT WATER UTILITIES MISSOURI ONE CALL (DIG RITE) ~ 1-800-33	IES IS PROVIDED FOR INF T IS COMPLETE OR ACCU TY COMPANIES THAT MA' JAL LOCATION OF EACH L OF ANY CONFLICT WITH i1-5011 ~ 969-1800 NSPECTIONS ~ 969-1800 S ~ 969-1900 344-7483	ORMATION ONLY. WE DO N JRATE. THE CONTRACTOR S Y BE AFFECTED BY THE PRO JTILITY LINE. THE CONTRAC PROPOSED IMPROVEMENTS	OT OFFER ANY SHALL BE DPOSED TOR SHALL NOTIFY S.		Motthew J. Schlicht MO PE 2006019708 KS PF 19071
<b>SENERAL NOTES:</b> 1 ~ ALL CONSTRUCTION SHALL CONFORM 1813. 2 ~ ALL REQUIRED EASEMENTS WITHIN T	M TO THE CITY OF LEE'S &	SUMMIT DESIGN AND CONS <sup>-</sup> PROJECT SHALL BE PROVID	TRUCTION MANUAL AS ADOPTED BY O	ORDINANCE	ОК РЕ 25226 REVISIONS

3 ~ ANY REQUIRED EASEMENT LOCATED OUTSIDE OF THE BOUNDARY OF THIS PROJECT SHALL BE PROVIDED FOR BY SEPARATE INSTRUMENT PRIOR TO ISSUANCE OF CONSTRUCTION PERMITS. 4 ~ THE CONTRACTOR SHALL CONTACT THE CITY'S DEVELOPMENT SERVICES ENGINEERING INSPECTORS 48 HOURS PRIOR TO ANY LAND DISTURBANCE WORK AT (816) 969-1200. 5 ~ THE CONTRACTOR SHALL NOTIFY ENGINEERING SOLUTIONS AT 816.623.9888 OF ANY CONFLICT WITH THE IMPROVEMENTS PROPOSED BY

THESE PLANS AND SITE CONDITIONS. 6 ~ THE CONTRACTOR SHALL NOTIFY THE CITY ENGINEER AND OBTAIN THE APPROPRIATE BLASTING PERMITS FOR A REQUIRED BLASTING. IF BLASTING IS ALLOWED, ALL BLASTING SHALL CONFORM TO STATE REGULATIONS AND LOCAL ORDINANCES.

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# Exhibit B

### **FEMA FIRMette**

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# National Flood Hazard Layer FIRMette

0

250

500

1,000

1,500

2,000



#### Legend

regulatory purposes.

#### 94°23'5"W 38°54'17"N SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT ίω Without Base Flood Elevation (BFE) ZoiHAE Zone A. V. A9 With BFE or Depth Zone AE, AO, AH, VE, AR SPECIAL FLOOD HAZARD AREAS **Regulatory Floodway** m 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X Future Conditions 1% Annual Chance Flood Hazard Zone X Area with Reduced Flood Risk due to Levee. See Notes. Zone X OTHER AREAS OF FLOOD HAZARD Area with Flood Risk due to Levee Zone D NO SCREEN Area of Minimal Flood Hazard Zone X Effective LOMRs OTHER AREAS Area of Undetermined Flood Hazard Zone D - — – – Channel, Culvert, or Storm Sewer GENERAL STRUCTURES LIIII Levee, Dike, or Floodwall 20.2 Cross Sections with 1% Annual Chance 17.5 Water Surface Elevation T47N R31W S7 AREA OF MINIMAL FLOOD HAZARD **Coastal Transect** Mase Flood Elevation Line (BFE) Zone X FLCODWA Limit of Study Jurisdiction Boundary 95.5.FEET **Coastal Transect Baseline** T47N R31W S8 OTHER **Profile Baseline** co419Goummit FEATURES Hydrographic Feature 290174 **Digital Data Available** No Digital Data Available MAP PANELS Unmapped The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location. This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 6/9/2023 at 12:56 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for 94°22'28"W 38°53'49"N Feet 1:6,000 unmapped and unmodernized areas cannot be used for

Basemap Imagery Source: USGS National Map 2023

# Exhibit C

# **NRCS Soils Report**

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United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

# Custom Soil Resource Report for Jackson County, Missouri

**Oldham Marketplace** 



# Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2\_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

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# Contents

Preface	2
How Soil Surveys Are Made	5
Soil Map	8
Soil Map	9
Legend	10
Map Unit Legend	. 11
Map Unit Descriptions	11
Jackson County, Missouri	. 13
10082—Arisburg-Urban land complex, 1 to 5 percent slopes	.13
10181—Udarents-Urban land-Sampsel complex, 5 to 9 percent slopes	. 14
99033—Udarents-Urban land complex, 2 to 9 percent slopes	16
References	.19

# **How Soil Surveys Are Made**

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

# Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

#### Custom Soil Resource Report Soil Map



	MAP LEGEND			MAP INFORMATION
Area of Int	<b>terest (AOI)</b> Area of Interest (AOI)	8	Spoil Area Stony Spot	The soil surveys that comprise your AOI were mapped at 1:24,000.
Soils	Soil Map Unit Polygons Soil Map Unit Lines	Ø V	Very Stony Spot Wet Spot	Warning: Soil Map may not be valid at this scale.
Special	Soil Map Unit Points Point Features Playcout	∆  Water Fea	Other Special Line Features	misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed
0 2	Borrow Pit	Transport	Streams and Canals	Please rely on the bar scale on each map sheet for map
*	Closed Depression Gravel Pit	HH Rail Rail US I Majo	Rails Interstate Highways	measurements. Source of Map: Natural Resources Conservation Service
: •	Gravelly Spot Landfill		Major Roads	Coordinate System: Web Mercator (EPSG:3857)
بة ۲	Lava Flow Marsh or swamp	Backgrou	nd Aerial Photography	projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
0	Miscellaneous Water Perennial Water			This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
~ +	Rock Outcrop Saline Spot			Soil Survey Area: Jackson County, Missouri Survey Area Data: Version 24, Aug 31, 2022
**	Sandy Spot Severely Eroded Spot			Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.
\$ \$ @	Slide or Slip Sodic Spot			Date(s) aerial images were photographed: Aug 30, 2022—Sep 8, 2022
Circle Ci				The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
10082	Arisburg-Urban land complex, 1 to 5 percent slopes	2.1	5.8%
10181	Udarents-Urban land-Sampsel complex, 5 to 9 percent slopes	10.2	28.2%
99033	Udarents-Urban land complex, 2 to 9 percent slopes	23.8	65.9%
Totals for Area of Interest	·	36.1	100.0%

### **Map Unit Legend**

### Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or

landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

### Jackson County, Missouri

#### 10082—Arisburg-Urban land complex, 1 to 5 percent slopes

#### **Map Unit Setting**

National map unit symbol: 2w7ld Elevation: 750 to 1,130 feet Mean annual precipitation: 39 to 45 inches Mean annual air temperature: 50 to 55 degrees F Frost-free period: 177 to 220 days Farmland classification: All areas are prime farmland

#### **Map Unit Composition**

Arisburg and similar soils: 61 percent Urban land: 30 percent Minor components: 9 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Arisburg**

#### Setting

Landform: Interfluves Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Convex Across-slope shape: Convex Parent material: Loess

#### **Typical profile**

Ap - 0 to 6 inches: silt loam A - 6 to 13 inches: silt loam Bt - 13 to 19 inches: silty clay loam Btg - 19 to 56 inches: silty clay loam BCg - 56 to 79 inches: silty clay loam

#### **Properties and qualities**

Slope: 1 to 5 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat poorly drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: About 18 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: High (about 11.5 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: C Ecological site: R107XB007MO - Loess Upland Prairie Hydric soil rating: No

#### **Description of Urban Land**

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydric soil rating: No

#### Minor Components

#### Sampsel

Percent of map unit: 3 percent Landform: Hills Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Concave Ecological site: R109XY010MO - Interbedded Sedimentary Upland Savanna Hydric soil rating: Yes

#### Greenton

Percent of map unit: 3 percent Landform: Hillslopes Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Convex Ecological site: R109XY002MO - Loess Upland Prairie Hydric soil rating: No

#### Sharpsburg

Percent of map unit: 3 percent Landform: Ridges Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Linear Across-slope shape: Linear Ecological site: R109XY002MO - Loess Upland Prairie Hydric soil rating: No

#### 10181—Udarents-Urban land-Sampsel complex, 5 to 9 percent slopes

#### Map Unit Setting

National map unit symbol: 1n85g Elevation: 600 to 900 feet Mean annual precipitation: 33 to 43 inches Mean annual air temperature: 50 to 57 degrees F Frost-free period: 175 to 220 days Farmland classification: Farmland of statewide importance

#### Map Unit Composition

Udarents and similar soils: 46 percent Urban land: 39 percent Sampsel and similar soils: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Udarents**

#### Setting

Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Convex Parent material: Mine spoil or earthy fill

#### **Typical profile**

C1 - 0 to 5 inches: silt loam C2 - 5 to 80 inches: silty clay loam

#### **Properties and qualities**

Slope: 5 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.14 to 0.57 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 9.0 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6e Hydrologic Soil Group: C Ecological site: R107XB002MO - Deep Loess Upland Prairie Other vegetative classification: Mixed/Transitional (Mixed Native Vegetation) Hydric soil rating: No

#### **Description of Urban Land**

#### Setting

Landform: Hillslopes Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Across-slope shape: Convex

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydric soil rating: No

#### **Description of Sampsel**

#### Setting

Landform: Hillslopes

Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Convex Parent material: Residuum weathered from shale

#### **Typical profile**

*Ap - 0 to 13 inches:* silty clay loam *Bt - 13 to 80 inches:* silty clay

#### **Properties and qualities**

Slope: 5 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 0 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 8.6 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3e Hydrologic Soil Group: C/D Ecological site: R109XY010MO - Interbedded Sedimentary Upland Savanna Other vegetative classification: Grass/Prairie (Herbaceous Vegetation) Hydric soil rating: No

#### 99033—Udarents-Urban land complex, 2 to 9 percent slopes

#### Map Unit Setting

National map unit symbol: 1n85n Elevation: 710 to 1,470 feet Mean annual precipitation: 31 to 47 inches Mean annual air temperature: 46 to 57 degrees F Frost-free period: 170 to 220 days Farmland classification: Not prime farmland

#### Map Unit Composition

Udarents and similar soils: 50 percent Urban land: 45 percent Minor components: 5 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Udarents**

#### Setting

Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Side slope, crest Down-slope shape: Convex Across-slope shape: Convex Parent material: Mine spoil or earthy fill

#### **Typical profile**

*C1 - 0 to 5 inches:* silt loam *C2 - 5 to 80 inches:* silty clay loam

#### **Properties and qualities**

Slope: 2 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.14 to 0.57 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 9.0 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6e Hydrologic Soil Group: C Other vegetative classification: Mixed/Transitional (Mixed Native Vegetation) Hydric soil rating: No

#### **Description of Urban Land**

#### Setting

Landform position (two-dimensional): Backslope Across-slope shape: Convex

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydric soil rating: No

#### Minor Components

#### Knox

Percent of map unit: 3 percent Landform: Hillslopes Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Convex Ecological site: R107XB003MO - Deep Loess Exposed Backslope Savanna, F107XB004MO - Deep Loess Protected Backslope Woodland Hydric soil rating: No

#### Sibley

Percent of map unit: 2 percent Landform: Hillslopes Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Convex Across-slope shape: Convex Ecological site: R109XY002MO - Loess Upland Prairie Hydric soil rating: No

# References

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.

Federal Register. July 13, 1994. Changes in hydric soils of the United States.

Federal Register. September 18, 2002. Hydric soils of the United States.

Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.

National Research Council. 1995. Wetlands: Characteristics and boundaries.

Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/ nrcs/detail/national/soils/?cid=nrcs142p2\_054262

Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2\_053577

Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2\_053580

Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.

United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.

United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/ home/?cid=nrcs142p2 053374

United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. http://www.nrcs.usda.gov/wps/portal/nrcs/ detail/national/landuse/rangepasture/?cid=stelprdb1043084

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/ nrcs/detail/soils/scientists/?cid=nrcs142p2\_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/? cid=nrcs142p2\_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE\_DOCUMENTS/nrcs142p2\_052290.pdf

# Exhibit D

**Existing Drainage Area Map** 





0 100' 400'

EXISTING DRAINAGE MAP



# Exhibit E

# Hydraflow Hydrograph Report

13 | P a g e

### Watershed Model Schematic

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023



# Hydrograph Return Period Recap Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd.	d. Hydrograph Inflow Peak Outflow (cfs)								Hydrograph		
NO.	type (origin)	nya(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff			347.19			580.94			923.43	EX POI A
2	SCS Runoff			258.11			424.44			667.95	PROP POI A1
3	SCS Runoff			121.55			212.36			347.29	PROP SUBAREA B
4	Reservoir	2		105.72			235.46			321.15	DETAINED SUBAREA A1
5	Combine	3, 4		208.22			437.28			653.97	PROP POLA
		•		•	•	•	•	•			

# Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	347.19	1	728	1,239,743				EX POI A
2	SCS Runoff	258.11	1	724	773,855				PROP POL A1
3	SCS Runoff	121.55	1	728	432,287				PROP SUBAREA B
4	Reservoir	105.72	1	737	614,877	2	1011.23	363,450	DETAINED SUBAREA A1
5	Combine	208.22	1	733	1,047,164	3, 4			PROP POLA
OLDHAM VILLAGE 230714.gpw					Return P		a	Fliday, 07 /	14 / 2023

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 1

EX POI A

Hydrograph type	= SCS Runoff	Peak discharge	= 347.19 cfs
Storm frequency	= 2 yrs	Time to peak	= 728 min
Time interval	= 1 min	Hyd. volume	= 1,239,743 cuft
Drainage area	= 150.540 ac	Curve number	= 88
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 24.90 min
Total precip.	= 3.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 2

PROP POI A1

Hydrograph type	= SCS Runoff	Peak discharge	= 258.11 cfs
Storm frequency	= 2 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 773,855 cuft
Drainage area	= 91.480 ac	Curve number	= 89
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 18.20 min
Total precip.	= 3.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



5

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 3

### PROP SUBAREA B

Hydrograph type	= SCS Runoff	Peak discharge	= 121.55 cfs
Storm frequency	= 2 yrs	Time to peak	= 728 min
Time interval	= 1 min	Hyd. volume	= 432,287 cuft
Drainage area	= 59.060 ac	Curve number	= 85
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 24.90 min
Total precip.	= 3.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 4

DETAINED SUBAREA A1

Hydrograph type	= Reservoir	Peak discharge	= 105.72 cfs
Storm frequency	= 2 yrs	Time to peak	= 737 min
Time interval	= 1 min	Hyd. volume	= 614,877 cuft
Inflow hyd. No.	= 2 - PROP POI A1	Max. Elevation	= 1011.23 ft
Reservoir name	= Retention Basin A	Max. Storage	= 363,450 cuft

Storage Indication method used.



7

### **Pond Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

#### Pond No. 1 - Retention Basin A

#### Pond Data

Contours -User-defined contour areas. Average end area method used for volume calculation. Begining Elevation = 1004.00 ft

#### Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	1004.00	35,167	0	0
2.00	1006.00	43,133	78,300	78,300
4.00	1008.00	51,417	94,550	172,850
6.00	1010.00	60,030	111,447	284,297
8.00	1012.00	68,963	128,993	413,290
10.00	1014.00	78,206	147,169	560,459
12.00	1016.00	87,695	165,901	726,360
14.00	1018.00	97,410	185,105	911,465

#### **Culvert / Orifice Structures**

#### **Weir Structures**

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 72.00	60.00	0.00	1.00	Crest Len (ft)	= 0.00	0.00	0.00	0.00
Span (in)	= 72.00	96.00	0.00	1.00	Crest El. (ft)	= 0.00	0.00	0.00	0.00
No. Barrels	= 1	1	0	12	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 1004.00	1008.25	0.00	1004.00	Weir Type	=			
Length (ft)	= 100.00	0.00	0.00	4.04	Multi-Stage	= No	No	No	No
Slope (%)	= 0.01	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	/ Wet area)		
Multi-Stage	= n/a	Yes	No	Yes	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s). Stage / Storage / Discharge Table

_	_	_											
Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	1004.00	0.00	0.00		0.00							0.000
2.00	78,300	1006.00	0.14 oc	0.00		0.12							0.121
4.00	172,850	1008.00	0.37 oc	0.00		0.35							0.352
6.00	284,297	1010.00	17.06 oc	17.06 ic		0.00							17.06
8.00	413,290	1012.00	156.01 oc	155.94 ic		0.07							156.01
10.00	560,459	1014.00	255.08 oc	254.96 ic		0.12							255.08
12.00	726,360	1016.00	312.32 oc	312.09 ic		0.22							312.31
14.00	911,465	1018.00	360.59 oc	360.24 ic		0.34							360.58

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### Hyd. No. 5

PROP POI A

Hydrograph type= CombineStorm frequency= 2 yrsTime interval= 1 minInflow hyds.= 3, 4	Peak discharge Time to peak Hyd. volume Contrib. drain. area	= 208.22 cfs = 733 min = 1,047,164 cuft = 59.060 ac
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# Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	580.94	1	728	2,108,913				EX POI A
2	SCS Runoff	424.44	1	724	1,301,274				PROP POL A1
3	SCS Runoff	212.36	1	728	761,797				PROP SUBAREA B
4	Reservoir	235.46	1	734	1,141,907	2	1013.41	516,840	DETAINED SUBAREA A1
5	Combine	437.28	1	729	1,903,705	3, 4			PROP POLA
		SE 23071			Return P	eriod: 10 V	ear	Friday 07 /	14 / 2023
OLDHAM VILLAGE 230/14.gpw					enou. IV f	cai	1 11uay, 07 /	17/2023	

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 1

EX POI A

Hydrograph type	= SCS Runoff	Peak discharge	= 580.94 cfs
Storm frequency	= 10 yrs	Time to peak	= 728 min
Time interval	= 1 min	Hyd. volume	= 2,108,913 cuft
Drainage area	= 150.540 ac	Curve number	= 88
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 24.90 min
Total precip.	= 5.20 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 2

PROP POI A1

Hydrograph type	= SCS Runoff	Peak discharge	= 424.44 cfs
Storm frequency	= 10 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 1,301,274 cuft
Drainage area	= 91.480 ac	Curve number	= 89
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 18.20 min
Total precip.	= 5.20 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



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12

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 3

PROP SUBAREA B

Hydrograph type	= SCS Runoff	Peak discharge	= 212.36 cfs
Storm frequency	= 10 yrs	Time to peak	= 728 min
Time interval	= 1 min	Hyd. volume	= 761,797 cuft
Drainage area	= 59.060 ac	Curve number	= 85
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 24.90 min
Total precip.	= 5.20 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



13

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 4

DETAINED SUBAREA A1

Hydrograph type	= Reservoir	Peak discharge	= 235.46 cfs
Storm frequency	= 10 yrs	Time to peak	= 734 min
Time interval	= 1 min	Hyd. volume	= 1,141,907 cuft
Inflow hyd. No.	= 2 - PROP POI A1	Max. Elevation	= 1013.41 ft
Reservoir name	= Retention Basin A	Max. Storage	= 516,840 cuft

Storage Indication method used.



14

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 5

PROP POI A

Hydrograph type= CombineStorm frequency= 10 yrsTime interval= 1 minInflow hyds.= 3, 4	Peak discharge Time to peak Hyd. volume Contrib. drain. area	= 437.28 cfs = 729 min = 1,903,705 cuft = 59.060 ac
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# Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	923.43	1	727	3,429,129				EX POI A
2	SCS Runoff	667.95	1	723	2,098,705				PROP POI A1
3	SCS Runoff	347.29	1	728	1,270,061				PROP SUBAREA B
4	Reservoir	321.15	1	735	1,939,044	2	1016.35	758,297	DETAINED SUBAREA A1
5	Combine	653.97	1	729	3,209,106	3, 4			PROP POLA
OLI	OHAM VILLAC	GE 23071	4.gpw		Return P	eriod: 100	Year	Friday, 07 /	14 / 2023

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 1

EX POI A

Hydrograph type	= SCS Runoff	Peak discharge	= 923.43 cfs
Storm frequency	= 100 yrs	Time to peak	= 727 min
Time interval	= 1 min	Hyd. volume	= 3,429,129 cuft
Drainage area	= 150.540 ac	Curve number	= 88
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 24.90 min
Total precip.	= 7.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



17

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 2

PROP POI A1

Hydrograph type	= SCS Runoff	Peak discharge	= 667.95 cfs
Storm frequency	= 100 yrs	Time to peak	= 723 min
Time interval	= 1 min	Hyd. volume	= 2,098,705 cuft
Drainage area	= 91.480 ac	Curve number	= 89
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 18.20 min
Total precip.	= 7.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 3

**PROP SUBAREA B** 

Hydrograph type	= SCS Runoff	Peak discharge	= 347.29 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 1 min	Hyd. volume	= 1,270,061 cuft
Drainage area	= 59.060 ac	Curve number	= 85
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 24.90 min
Total precip.	= 7.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Friday, 07 / 14 / 2023

19

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 4

DETAINED SUBAREA A1

Hydrograph type	= Reservoir	Peak discharge	= 321.15 cfs
Storm frequency	= 100 yrs	Time to peak	= 735 min
Time interval	= 1 min	Hyd. volume	= 1,939,044 cuft
Inflow hyd. No.	= 2 - PROP POI A1	Max. Elevation	= 1016.35 ft
Reservoir name	= Retention Basin A	Max. Storage	= 758,297 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

### Hyd. No. 5

PROP POI A

Hydrograph type= CombinStorm frequency= 100 yrsTime interval= 1 minInflow hyds.= 3, 4	e Peak discharge Time to peak Hyd. volume Contrib. drain. area	= 653.97 cfs = 729 min = 3,209,106 cuft = 59.060 ac
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# **Hydraflow Rainfall Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Return Period	Intensity-Du	uration-Frequency E	quation Coefficients	(FHA)
(Yrs)	В	D	E	(N/A)
1	64.1474	17.7000	0.8922	
2	95.7859	19.2000	0.9317	
3	0.0000	0.0000	0.0000	
5	118.7799	19.1000	0.9266	
10	125.1300	18.2000	0.9051	
25	158.9867	18.7000	0.9180	
50	171.2459	18.3000	0.9078	
100	187.3624	18.1000	0.9031	

File name: KCMO.IDF

#### Intensity = B / (Tc + D)^E

Return Period (Yrs)	Intensity Values (in/hr)											
	5 min	10	15	20	25	30	35	40	45	50	55	60
1	3.96	3.31	2.86	2.52	2.25	2.04	1.87	1.72	1.60	1.49	1.40	1.32
2	4.92	4.13	3.56	3.14	2.81	2.54	2.32	2.14	1.98	1.85	1.73	1.63
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.23	5.23	4.51	3.98	3.56	3.22	2.94	2.71	2.52	2.35	2.20	2.07
10	7.27	6.09	5.26	4.63	4.14	3.75	3.43	3.16	2.93	2.74	2.57	2.42
25	8.70	7.30	6.30	5.54	4.96	4.49	4.10	3.78	3.51	3.27	3.07	2.89
50	9.83	8.24	7.11	6.26	5.60	5.07	4.64	4.27	3.97	3.70	3.47	3.27
100	11.00	9.21	7.95	7.00	6.26	5.67	5.19	4.78	4.44	4.14	3.89	3.66
1	1	1	1	1	1	1		1	1	1	1	1

Tc = time in minutes. Values may exceed 60.

					Pre	cip. file nar	ne: Z:\acao	d\KCMO.pc	כ
	Rainfall Precipitation Table (in)								
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
SCS 24-hour	1.37	3.50	0.00	3.30	5.20	6.00	6.80	7.70	
SCS 6-Hr	0.00	1.80	0.00	0.00	2.60	0.00	0.00	4.00	
Huff-1st	0.00	1.55	0.00	2.75	4.00	5.38	6.50	8.00	
Huff-2nd	2.49	3.10	0.00	4.01	4.64	5.52	6.21	6.90	
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-Indy	0.00	1.55	0.00	2.75	4.00	5.38	6.50	8.00	
Custom	0.00	1.75	0.00	2.80	3.90	5.25	6.00	7.10	

# Hydraflow Table of Contents

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Watershed Model Schematic	1
Hydrograph Return Period Recap	2

### 2 - Year

Summary Report	3
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, EX POI A	4
Hydrograph No. 2, SCS Runoff, PROP POI A1	. 5
Hydrograph No. 3, SCS Runoff, PROP SUBAREA B	. 6
Hydrograph No. 4, Reservoir, DETAINED SUBAREA A1	7
Pond Report - Retention Basin A	8
Hydrograph No. 5, Combine, PROP POI A	. 9

### 10 - Year

Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, EX POI A	11
Hydrograph No. 2, SCS Runoff, PROP POI A1	12
Hydrograph No. 3, SCS Runoff, PROP SUBAREA B	13
Hydrograph No. 4, Reservoir, DETAINED SUBAREA A1	. 14
Hydrograph No. 5, Combine, PROP POI A	15

### 100 - Year

Summary Report	16
Hydrograph Reports	17
Hydrograph No. 1, SCS Runoff, EX POI A	17
Hydrograph No. 2, SCS Runoff, PROP POI A1	18
Hydrograph No. 3, SCS Runoff, PROP SUBAREA B	19
Hydrograph No. 4, Reservoir, DETAINED SUBAREA A1	20
Hydrograph No. 5, Combine, PROP POLA	21
DF Report	22

# Exhibit F

**Proposed Drainage Area Map** 

14 | P a g e





200'

PROPOSED DRAINAGE MAP



# Exhibit G

# **Retention Plan**

15 | P a g e







# Exhibit H

**Emergency Spillway Calculations** 

16 | P a g e

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Thursday, Jul 13 2023

### **Emergency Spillway Detention Basin A**

Rectangular Weir		Highlighted	
Crest	= Broad	Depth (ft)	= 0.90
Bottom Length (ft)	= 300.00	Q (cfs)	= 667.95
Total Depth (ft)	= 2.00	Area (sqft)	= 270.52
		Velocity (ft/s)	= 2.47
Calculations		Top Width (ft)	= 300.00
Weir Coeff. Cw	= 2.60		
Compute by:	Known Q		
Known Q (cfs)	= 667.95		



# Exhibit I

**40 Hour Extended Detention Calculations** 

17 | P a g e

### Calculate Water Quality for Storm Study

#### Date: 7-14-23

Project: Oldham Village To Calculate: WQv = P \* Rv \* A

P (in) =	1.37
P (ft) =	0.11
Impervious Area (sq. ft.) =	1,341,652.77
Total Area (sq. ft.) =	2,164,060.80
Impervious Area (ac) =	30.80
Total Area (acre) =	49.68
Rv = (0.05 * 0.009(I)) =	0.62
Percent Impervious (I) =	63.33
WQ <sub>v</sub> (cu. ft.) =	153,172
$WQ_v$ (ac. ft.) =	3.516

Enter data in these Fields
Unit Conversions
1 Acre = 43,560 Sq. Ft.

CCN = 89

	Pond Volume			
Elevation	Area (Sq. Ft.)	Volume (Cu. Ft.)		
1,004	35167	0		
1,006	43133	78,300		
1,008	51,467	172,900		
1,010	60,030	284,397		
1,012	68,963	413,390		
1,014	78,206	560,559		
1,016	87,695	726,460		
1,018	97,410	911,565		

### To Calculate $Z_{WQ}$ (ft) interpolate from Storm Study (Sheet 13)

Elevation 1 =	1006.00	Storage 1 =	78,300.00
Elevation X =		Storage X =	153,172.03
Elevation 2 =	1008.00	Storage 2 =	172,900.00
		Elevation X =	1007.77
Lowest Elevation of Pond =	1004.00		
Elevation X =	1007.77		
Z <sub>WQ</sub> (ft) =	3.77		

#### IIc. Water Quality Outlet, Perforated Riser

ne. Water Quality Outlet, i chorateu hiser					
Step 1) Depth at outlet above lowest perforation:	Z <sub>WQ</sub> (ft) =	3.77			
Step 2) Recommended maximum outlet area per row:	A <sub>0</sub> (in <sup>2</sup> ) =	3.846			
Step 3) Circular perforation diameter per row assuming a single column:	D <sub>1</sub> (in) =	0.658	Calculates the diameter of each hole given the depth of water and the area per row. Assuming 4" spacing. If less than 1" use 1" as D <sub>perf</sub> .		
1	n <sub>c</sub> (unitless) =	1			
Step 5) Design circular perforation diameter (should be between 1 and 2 inches):	D <sub>perf</sub> (in) =	1.000			
Step 6) Horizontal perforation column spacing when $n_c > 1$ , center to center:	S <sub>c</sub> (in) =	4			
Note: If $D_{perf} \ge 1.0$ inch, $S_c = 4$					
Step 7) Number of rows (4" vertical spacing between perforations, center to center):	n <sub>r</sub> (unitless) =	12			

Recommended Method:

Perforated Riser