ENGINEER OLSSON

1301 BURLINGTON ST. SUITE 100

NORTH KANSAS CITY, MO 64116 CONTACT: JASON ROUDEBUSH

EMAIL: JROUDEBUSH@OLSSON.COM

NOT FOR CONSTRUCTION

REVIEWED FOR CONSTRUCTION

PHONE: 816.361.1177

# NW 1/4 **VICINITY MAP** S35, T47N, R32W N.T.S PROJECT TEAM & UTILITY CONTACT LIST OWNER / DEVELOPER CLAYTON PROPERTIES GROUP, INC. D.B.A. SUMMIT HOMES 120 SE 30TH STREET CONTACT: VINCENT WALKER LEE'S SUMMIT, MO 64082 PHONE: 816.246.6700 EMAIL: VINCENT@SUMMITHOMESKC.COM 1301 BURLINGTON ST. SUITE 100 NORTH KANSAS CITY, MO 64116 CONTACT: JULIE E. SELLERS, P.E. PHONE: 816.361.1177 EMAIL: JSELLERS@OLSSON.COM

OSAGE 3RD PLAT

MO HWY 150

UTILITY SERVICE NUMBERS

PHONE: 816-969-1940

NAME: SPIRE (MGE) PHONE: 314-342-0500

PHONE: 800-286-8313

PHONE: 816-471-5275

NAME: SPECTRUM (TWC) PHONE: 877-772-2253

PHONE: 877-454-6959

NAME: GOOGLE FIBER

DEPARTMENT

NAME: AT&T

NAME: EVERGY

NAME: LEE'S SUMMIT PUBLIC WORKS PHONE: 816-969-1800

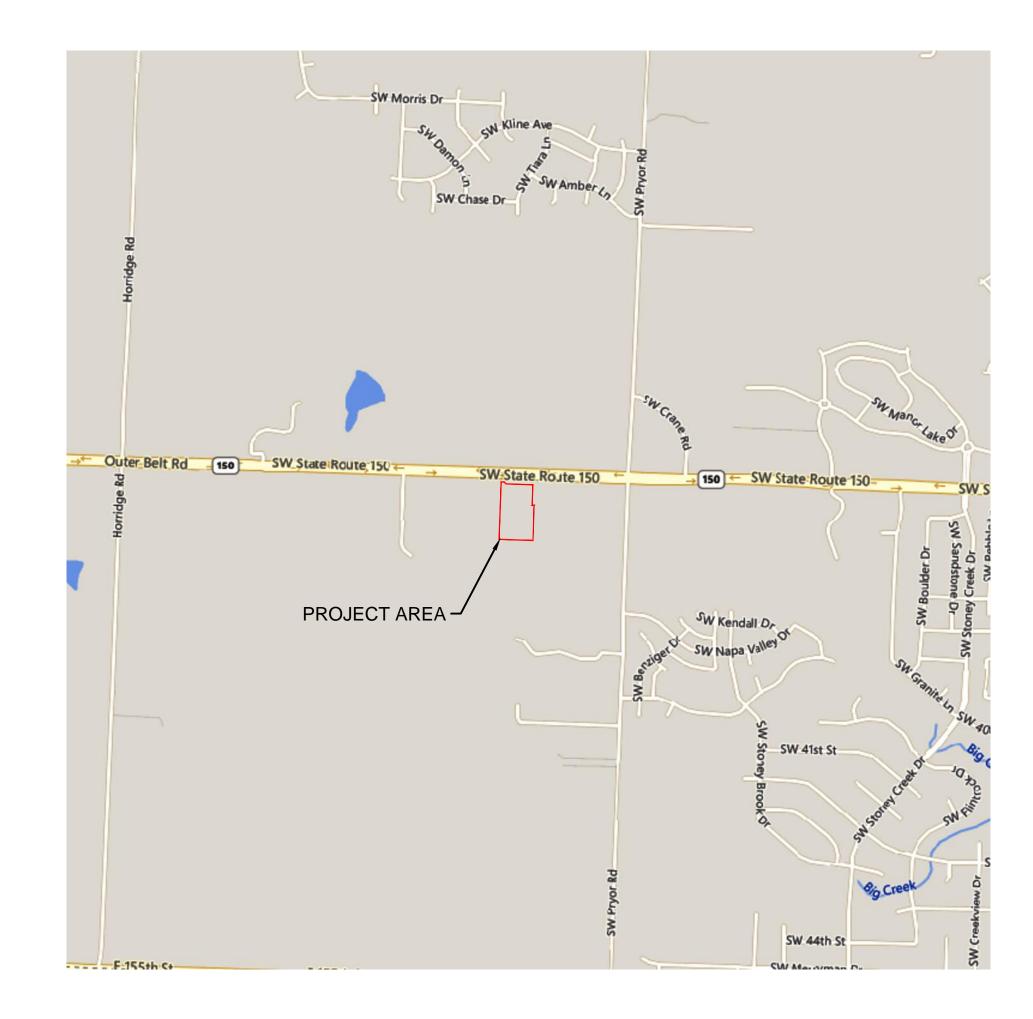
NAME: LEE'S SUMMIT WATER & SERVICES

# OSAGE THIRD PLAT STREET & STORM SEWER PLANS

NE 1/4 SECTION 35, TOWNSHIP 47 N, RANGE 32 W. IN LEE'S SUMMIT, JACKSON COUNTY, MISSOURI

**DRAWING** hese plans have been reviewed for accuracy by the Development Services Staff

PPROVED RECORD



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# AS BUILT

5/9/2023

DATE

### PROPERTY DESCRIPTION:

ALL OF THE WEST HALF OF THE NORTHWEST QUARTER OF THE NORTHEAST QUARTER OF THE NORTHEAST QUARTER OF SECTION 35, TOWNSHIP 47 NORTH, RANGE 32 WEST OF THE FIFTH PRINCIPAL MERIDIAN IN LEE'S SUMMIT, JACKSON COUNTY MISSOURI EXCEPT THAT PART IN MISSOURI STATE HIGHWAY NO. 150

A TRACT OF LAND IN THE WEST HALF OF THE SOUTHWEST QUARTER AND EAST HALF OF THE NORTHWEST QUARTER OF THE NORTHEAST QUARTER OF THE NORTHEAST QUARTER OF SECTION 35, TOWNSHIP 47 NORTH, RANGE 32 WEST OF THE 5TH PRINCIPAL MERIDIAN IN LEE'S SUMMIT, JACKSON COUNTY, MISSOURI BEING BOUNDED AND DESCRIBED BY OR UNDER THE DIRECT SUPERVISION OF JASON S. ROUDEBUSH, P.L.S. 2002014092, AS FOLLOWS: COMMENCING AT THE NORTHEAST CORNER OF SAID NORTHEAST QUARTER OF SAID NORTHEAST QUARTER; THENCE NORTH 88°07'14" WEST, ON THE NORTH LINE OF SAID NORTHEAST QUARTER OF SAID NORTHEAST QUARTER, 1,319.40 FEET TO THE NORTHWEST CORNER OF SAID NORTHEAST QUARTER OF SAID NORTHEAST QUARTER; THENCE SOUTH 02"0'22" WEST, ON THE WEST LINE OF SAID NORTHEAST QUARTER OF SAID NORTHEAST QUARTER, 659.27 FEET TO THE NORTHWEST CORNER OF SAID SOUTHWEST QUARTER OF SAID NORTHEAST QUARTER OF SAID NORTHEAST QUARTER ALSO BEING A POINT ON THE WESTERLY LINE OF PROPOSED OSAGE 2ND PLAT; THENCE SOUTH 88°08'29" EAST ON THE NORTH LINE OF SAID SOUTHWEST QUARTER OF SAID NORTHEAST QUARTER, AND ALONG SAID WESTERLY LINE, 326.96 FEET TO THE NORTHEAST CORNER OF SAID WEST HALF OF SAID SOUTHWEST QUARTER OF SAID NORTHEAST QUARTER OF SAID NORTHEAST QUARTER, ALSO BEING THE SOUTHWEST CORNER OF SAID EAST HALF OF SAID NORTHWEST QUARTER OF SAID NORTHEAST QUARTER OF SAID NORTHEAST QUARTER; THENCE NORTH 02°09'46" EAST ON THE WEST LINE OF SAID EAST HALF OF SAID NORTHWEST QUARTER OF SAID NORTHEAST QUARTER OF SAID NORTHEAST QUARTER, AND ALONG SAID WESTERLY LINE 346.14 FEET; THENCE LEAVING SAID EAST LINE AND SAID WESTERLY LINE, SOUTH 8811'07" EAST, ALONG THE NORTH LINE OF PROPOSED TRACT I OF SAID PROPOSED OSAGE 2ND PLAT, 21.17 FEET; THENCE SOUTH 01°48'53" WEST ALONG THE EASTERLY LINE OF SAID PROPOSED TRACT I, A DISTANCE OF 366.13 FEET; THENCE NORTH 8811'07" WEST, ALONG SAID EASTERLY LINE AND ITS WESTERLY EXTENSION 353.36 FEET TO A POINT ON SAID WEST LINE OF SAID NORTHEAST QUARTER OF SAID NORTHEAST QUARTER; THENCE NORTH 02\*10'22" EAST ON SAID WEST LINE, 20.25 FEET TO THE POINT OF BEGINNING. CONTAINING 14,798 SQUARE FEET OR 0.34 ACRES, MORE OR LESS.

### **BENCHMARK**

ELEVATION = 1014.830

CHISELED PLUS ON THE EAST FLANGED BOLT OF THE FIRE HYDRANT ON THE WEST SIDE OF SW PRYOR ROAD ON ADJOINING PROPERTY SOUTH OF THE SOUTHWEST CORNER OF SUBJECT PROPERTY.

RAILROAD SPIKE IN THE NORTH FACE OF POWER POLE LOCATED ON THE SOUTH SIDE MISSOURI STATE HIGHWAY 150 AT THE WEST SIDE OF THE DRIVEWAY TO 2025 MISSOURI STATE HIGHWAY 150, LEE'S SUMMIT, MO. ELEVATION = 1031.313



OLSSON HAS BEEN RETAINED TO PROVIDE AS-BUILT DRAWINGS FOR THIS PROJECT.

JULIE E. SELLERS, P.E. **CIVIL ENGINEER** MO# PE-2019000237

> checked by: QA/QC by: project no.:

> > SHEET

drawing no.: <u>C\_TTL01\_D192339</u>

## GENERAL NOTES

- 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT THE PLANS IN THEIR POSSESSION ARE THE MOST CURRENT VERSION ISSUED, ARE FULLY COORDINATED WITH ALL SUBCONTRACTORS. AND PRESENT ON SITE AT ALL TIMES. CURRENT PLANS PREPARED BY OLSSON MAY BE OBTAINED AT THE DIRECTION OF OLSSON'S CLIENT. DIRECT REQUESTS TO OLSSON MAY REQUIRE ADDITIONAL AUTHORIZATIONS, AGREEMENTS, AND/OR FEES. PLEASE CONTACT THE ENGINEER FOR INFORMATION.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DEVIATIONS FROM THESE PLANS UNLESS WRITTEN APPROVAL FROM ENGINEER, OWNER, AND DEVELOPER.
- 3. ALL WORK AND MATERIALS SHALL BE SUBJECT TO INSPECTION AND APPROVAL BY THE OWNER OR THE OWNER'S REPRESENTATIVE.
- 4. ALL ESTIMATES OF QUANTITIES ARE FOR INFORMATIONAL PURPOSES ONLY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING QUANTITIES AND ITEMS OF WORK.
- 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL LABOR, MATERIALS, AND EQUIPMENT REQUIRED TO COMPLETE THE WORK SHOWN
- 6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL REQUIRED PERMITS, PAYING ALL FEES, AND FOR OTHERWISE COMPLYING WITH ALL APPLICABLE REGULATIONS GOVERNING THE WORK.
- 7. THE CONTRACTOR SHALL NOT ENGAGE IN ACTIVITIES THAT MAY ENCROACH ON WATERS OF THE U.S., INCLUDING WETLANDS, UNTIL ANY NECESSARY PERMITS MAY BE OBTAINED. THE CONTRACTOR SHALL REVIEW AND COMPLY WITH ALL CONDITIONS DESCRIBED IN THE PERMIT.
- 8. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, THE SAFETY OF ALL PERSONS INCLUDING VISITORS AND THE GENERAL PUBLIC, AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY THROUGHOUT THE PROJECT AND NOT BE LIMITED BY WORKING HOURS. ANY CONSTRUCTION OBSERVATION BY THE ENGINEER OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES.
- 9. PRIOR TO COMMENCEMENT OF WORK THE CONTRACTOR SHALL NOTIFY AND COORDINATE WITH ALL UTILITY COMPANIES AND OBTAIN ANY RELEVANT INFORMATION. NOTIFY ENGINEER OF ANY DISCREPANCIES.
- 10. THE CONTRACTOR IS RESPONSIBLE FOR THE PROTECTION OF ALL BOUNDARY CORNERS AND SECTION CORNERS. ANY BOUNDARY CORNER AND/OR SECTION CORNER DISTURBED OR DAMAGED BY CONSTRUCTION ACTIVITIES SHALL BE RESET BY A LAND SURVEYOR LICENSED IN THE STATE OF MISSOURI, AT THE CONTRACTOR'S EXPENSE.
- 11. THE CONTRACTOR IS RESPONSIBLE FOR THE PROTECTION OF ADJACENT PROPERTIES AND SHALL TAKE ALL PRECAUTIONS NECESSARY TO PREVENT DAMAGE DURING CONSTRUCTION. THE CONTRACTOR IS ALSO RESPONSIBLE FOR REPAIRING ANY DAMAGE RESULTING FROM CONSTRUCTION ACTIVITIES.
- 12. PRIOR TO MOVING OFF THE JOB THE CONTRACTOR SHALL NOTIFY THE OWNER AND ENGINEER TO PERFORM A FINAL WALK-THROUGH OF THE CONSTRUCTION SITE.

#### REFERENCES

- 1. UNLESS EXPLICITLY DESCRIBED OTHERWISE WITHIN THESE PLANS THE FOLLOWING SHALL APPLY:
- A. ALL CONSTRUCTION, INCLUDING THOSE LISTED BELOW, SHALL CONFORM TO THE LATEST CODES AND ORDINANCES OF LEE'S SUMMIT, MISSOURI.
- B. ALL CONSTRUCTION IN MODOT RIGHT-OF-WAY SHALL CONFORM TO THE LATEST SPECIFICATIONS ADOPTED BY U.S. DEPARTMENT OF TRANSPORTATION AND MODOT.
- C. ALL TRAFFIC CONTROL SIGNAGE SHALL CONFORM WITH THE CURRENT EDITION OF THE MANUAL FOR UNIFORM TRAFFIC CONTROL DEVICES
- D. ALL UTILITY EXTENSIONS AND CONSTRUCTION SHALL CONFORM TO THE STANDARDS AND SPECIFICATIONS OF THE UTILITY COMPANIES..
- E. ALL EXTERIOR PAVEMENT (PCC, ASPHALT, ETC.) SHALL BE IN CONFORMANCE WITH THE SPECIFICATIONS OF LEE'S SUMMIT, MISSOURI AND THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT.
- 4. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING THE DELIVERY MANAGER AND COORDINATING ANY MAILBOXES THAT MAY BE DISTURBED. FAILURE TO DO SO MAY SUBJECT THE CONTRACTOR TO PROSECUTION BY THE FEDERAL GOVERNMENT.

#### EXISTING CONDITIONS

- 1. THE CONTRACTOR SHALL VISIT THE SITE AND BECOME FAMILIAR WITH THE EXISTING CONDITIONS OF THE PROJECT AREA.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PERFORMING THEIR OWN INVESTIGATIONS AND MAKING THEIR OWN ASSUMPTIONS REGARDING SITE SURFACE AND SUBSURFACE CONDITIONS. THIS INCLUDES THE LOCATION AND CONSISTENCY OF ANY EXISTING ROCK LAYERS UNDERLYING THE PROJECT SITE. CONTACT THE ENGINEER REGARDING ANY DISCREPANCIES THAT MAY AFFECT THE ABILITY TO CONSTRUCT FROM THESE PLANS AS DESIGNED.
- 3. EXISTING CONDITIONS WERE DETERMINED THROUGH A VARIETY OF METHODS THAT MAY INCLUDE SURVEY, AERIAL IMAGERY, AVAILABLE RECORDS, GIS DATA, ETC. SUBSURFACE CONDITIONS ARE APPROXIMATE AND MAY NOT INCLUDE ALL UTILITIES AND OTHER SITE IMPROVEMENTS PRESENT ON SITE. THE CONTRACTOR SHALL MAKE EXPLORATION EXCAVATIONS AND LOCATE EXISTING UNDERGROUND UTILITIES SUFFICIENTLY AHEAD OF CONSTRUCTION TO PERMIT REVISIONS TO PLANS WHEN CONFLICTS AND DISCREPANCIES ARE FOUND.

### CONSTRUCTION

- 1. THE CONTRACTOR SHALL INSTALL TRAFFIC CONTROL WHILE WORKING IN THE PUBLIC RIGHT-OF-WAY AS SHOWN IN THESE PLANS. IF PLANS ARE NOT PROVIDED, CONTRACTOR SHALL COORDINATE AND PROVIDE CONTROLS TO THE SATISFACTION OF THE RIGHT-OF-WAY OWNER.
- 2. THE CONTRACTOR SHALL PROTECT ALL TREES OVER 3" CALIPER FROM DAMAGE. NO TREE SHALL BE REMOVED WITHOUT PERMISSION OF THE OWNER, UNLESS SHOWN OTHERWISE ON THESE PLANS.
- 3. THE CONTRACTOR SHALL DISPOSE ALL WASTE MATERIAL RESULTING FROM THE PROJECT OFF-SITE AND IN STRICT CONFORMANCE WITH ALL LOCAL CODES AND ORDINANCES.
- 4. ALL MANHOLES, CATCH BASINS, UTILITY VALVES AND METER PITS ARE TO BE ADJUSTED OR REBUILT TO GRADE AS REQUIRED. NOT ALL ADJUSTMENTS ARE INDICATED IN THE PLANS.
- 5. THE CONTRACTOR SHALL STREET SWEEP OR OTHERWISE CLEAN ALL ACCESS ROUTES TO THE SITE AT CONCLUSION OF THE PROJECT. SHOP DRAWINGS
- 1. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS A MINIMUM OF 7 DAYS PRIOR TO THE REQUESTED DATE OF APPROVAL. ENGINEER SHALL REVIEW SHOP DRAWINGS OR SAMPLES IN CONFORMANCE WITH THE DESIGN FOR THIS PROJECT AS DESCRIBED IN THE PLANS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS. THE ENGINEER'S REVIEW SHALL NOT EXTEND TO MEANS OR METHODS OF CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY VARIATION FROM THE REQUIREMENTS OF THE CONTRACT DOCUMENTS UNLESS CONTRACTOR HAS NOTIFIED ENGINEER OF EACH SUCH VARIATION AT THE TIME OF SUBMISSION, AND OBTAINED ENGINEER'S WRITTEN APPROVAL OF EACH SUCH VARIATION. PRIOR TO SUBMITTING EACH SHOP DRAWING OR SAMPLE, CONTRACTOR SHALL HAVE REVIEWED AND VERIFIED:
- A. ALL FIELD MEASUREMENTS, QUANTITIES, DIMENSIONS, SPECIFIED PERFORMANCE CRITERIA, INSTALLATION REQUIREMENTS, MATERIALS, CATALOG NUMBERS AND SIMILAR INFORMATION WITH RESPECT
- B. ALL MATERIALS WITH RESPECT TO INTENDED USE, FABRICATION, SHIPPING, HANDLING, STORAGE, ASSEMBLY AND INSTALLATION PERTAINING TO THE PERFORMANCE OF THE WORK;
- C. ALL INFORMATION RELATIVE TO MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OF CONSTRUCTION AND SAFETY PRECAUTIONS AND PROGRAMS INCIDENT THERETO;
- D. CONTRACTOR SHALL ALSO HAVE REVIEWED AND COORDINATED EACH SHOP DRAWING OR SAMPLE WITH OTHER SHOP DRAWINGS AND SAMPLES, AND WITH THE REQUIREMENTS OF THE WORK AND THE CONTRACT DOCUMENTS.
- E. ALL SUBMITTED SHOP DRAWINGS SHALL BEAR A STAMP OR SPECIFIC WRITTEN INDICATION AND SIGNATURE THAT CONTRACTOR HAS FULLY COMPLETED THE ABOVE TASKS.
- 2. SHOP DRAWINGS AS DESCRIBED ABOVE ARE REQUIRED FOR, BUT NOT LIMITED TO, THE FOLLOWING: A. ALL STORM SEWER STRUCTURES TO BE INSTALLED WITH THIS PROJECT.
- B. ANY ITEMS IN THESE PLANS THAT ALLOW FOR AN "APPROVED EQUAL" ALTERNATIVE.

- 1. STORM STRUCTURES SHALL BE PER CURRENT CITY DETAILS. IF CITY DOES NOT HAVE PUBLISHED DETAILS STRUCTURES SHALL BE PER CURRENT APWA SPECIFICATIONS.
- 2. PRIOR TO COMMENCEMENT OF WORK THE CONTRACTOR SHALL NOTIFY AND COORDINATE CONSTRUCTION WITH CITY OF LEE'S SUMMIT, MISSOURI.
- 3. ALL PIPE LENGTHS AND ELEVATIONS ARE CALCULATED LINEARLY FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE.
- 4. ALL STRUCTURE DIMENSIONS ARE TO INSIDE FACE OF STRUCTURE.
- WHEN ORIENTING THE BOX DURING INSTALLATION.
- 6. THE CONTRACTOR SHALL EXPOSE EXISTING UTILITIES AT LOCATIONS OF POSSIBLE CONFLICT AND POINTS OF CONNECTION PRIOR TO ANY CONSTRUCTION OF STORM
- 7. STORM SEWER TRENCHES SHALL BE CONSTRUCTED SUCH THAT UNDISTURBED EXISTING SOIL OR FILL COMPACTED TO 95% PROCTOR DENSITY IS AT A DEPTH THAT IS 18" ABOVE TOP OF PROPOSED PIPE.
- 8. STRUCTURE INVERT CHANNELS SHALL BE SMOOTH, CIRCULAR, AND CONFORMING TO 1/2 THE ADJACENT PIPE SECTION (INVERT TO CENTER). CHANGES IN DIRECTION OF FLOW SHALL BE MADE WITH A SMOOTH CURVE AND MAINTAIN SHAPE THROUGHOUT. CHANGES IN GRADE OF ADJACENT PIPES SHALL BE TRANSITIONED SMOOTHLY AND EVENLY THROUGH THE STRUCTURE.
- 9. PIPE PENETRATIONS SHALL BE GROUTED TO ENSURE WATERTIGHT SEALS.
- 10. MAINTAIN MINIMUM DEPTH OF COVER PER APWA 5606.06

#### STORM SEWER GENERAL NOTES:

- 5. COORDINATES ARE PROVIDED AT THE CENTER OF STRUCTURE. ADDITIONAL COORDINATES PROVIDED ARE PER LOCAL CODES AND ORDINANCES OR AS AN AID

	• • • • • • • • • • • • • • • • • • • •	<b>40</b> 7	7.0 20.21
STREET			
EXCAVATION	C.Y.	1901	
EMBANKMENT	C.Y.	147	
SUBGRADE STABILIZATION (6" FLYASH TREATMENT)	S.Y.	2571	
6" ASPHALT PAVEMENT	S.Y.	2122	
CONCRETE CURB & GUTTER (CG-2)	L.F.	1347.64	
5' CONCRETE SIDEWALK	L.F.	181.6	
MILL & OVERLAY	S.Y.	13.4	
ADA RAMP	EA.	3	
STOP SIGNS	EA.	1	
STREET NAME SIGNS	EA.	2	
END OF ROAD TREATMENT	EA.	1	

**ESTIMATE OF QUANTITIES** 

DESCRIPTION

ITEM NO.

STOP SIGNS       EA.       1         STREET NAME SIGNS       EA.       2         END OF ROAD TREATMENT       EA.       1         STORM         STD. CURB INLET (5'x3' INSIDE)       EA.       — 7 7         STD. CURB INLET (6'x3' INSIDE)       EA.       — 1 1         STD. STORM MANHOLE (4' DIA. INSIDE)       EA.       — 1 1         STD. FIELD INLET (4'x4' INSIDE)       EA.       — 2 2         BASIN OUTLET STRUCTURE       EA.       — 1 1         WATER QUALITY OUTLET STRUCTURE       EA.       — 1 1         15" HDPE       L.F.       476.95 476.45         18" HDPE       L.F.       272.06 270.81         24" HDPE       L.F.       186.06 187.62         6" PVC       L.F.       - 8 7.42
STORM   STD. CURB INLET (5'x3' INSIDE)   EA.   — 7
STORM         STD. CURB INLET (5'x3' INSIDE)       EA.       — 7 7         STD. CURB INLET (6'x3' INSIDE)       EA.       — 1 1         STD. STORM MANHOLE (4' DIA. INSIDE)       EA.       — 1 1         STD. FIELD INLET (4'x4' INSIDE)       EA.       — 2 2         BASIN OUTLET STRUCTURE       EA.       — 1 1         WATER QUALITY OUTLET STRUCTURE       EA.       — 1 1         15" HDPE       L.F.       476.95 476.45         18" HDPE       L.F.       272.06 270.81         24" HDPE       L.F.       186.06 187.62
STD. CURB INLET (5'x3' INSIDE)       EA.       7       7         STD. CURB INLET (6'x3' INSIDE)       EA.       1       1         STD. STORM MANHOLE (4' DIA. INSIDE)       EA.       1       1         STD. FIELD INLET (4'x4' INSIDE)       EA.       2       2         BASIN OUTLET STRUCTURE       EA.       1       1         WATER QUALITY OUTLET STRUCTURE       EA.       1       1         15" HDPE       L.F.       476.95       476.45         18" HDPE       L.F.       272.96       270.81         24" HDPE       L.F.       186.06       187.62
STD. CURB INLET (6'x3' INSIDE)       EA.       1         STD. STORM MANHOLE (4' DIA. INSIDE)       EA.       1         STD. FIELD INLET (4'x4' INSIDE)       EA.       2         BASIN OUTLET STRUCTURE       EA.       1         WATER QUALITY OUTLET STRUCTURE       EA.       1         15" HDPE       L.F.       476.95       476.45         18" HDPE       L.F.       272.06       270.81         24" HDPE       L.F.       186.06       187.62
STD. STORM MANHOLE (4' DIA. INSIDE)       EA.       —1       1         STD. FIELD INLET (4'x4' INSIDE)       EA.       —2       2         BASIN OUTLET STRUCTURE       EA.       —1       1         WATER QUALITY OUTLET STRUCTURE       EA.       —1       1         15" HDPE       L.F.       476.95       476.45         18" HDPE       L.F.       272.06       270.81         24" HDPE       L.F.       186.06       187.62
STD. FIELD INLET (4'x4' INSIDE)       EA.       2       2         BASIN OUTLET STRUCTURE       EA.       1       1         WATER QUALITY OUTLET STRUCTURE       EA.       1       1         15" HDPE       L.F.       476.95       476.45         18" HDPE       L.F.       272.06       270.81         24" HDPE       L.F.       186.06       187.62
BASIN OUTLET STRUCTURE       EA.       —1       1         WATER QUALITY OUTLET STRUCTURE       EA.       —1       1         15" HDPE       L.F.       476.95       476.45         18" HDPE       L.F.       272.06       270.81         24" HDPE       L.F.       186.06       187.62
WATER QUALITY OUTLET STRUCTURE       EA.       —1—       1         15" HDPE       L.F.       476.95       476.45         18" HDPE       L.F.       272.06       270.81         24" HDPE       L.F.       186.06       187.62
15" HDPE L.F. 476.95 476.45 18" HDPE L.F. 272.06 270.81 24" HDPE L.F. 186.06 187.62
18" HDPE       L.F.       272.06       270.81         24" HDPE       L.F.       186.06       187.62
24" HDPE L.F. <del>186.06</del> 187.62
6" DVC
C.F.     7.42
24" HDPE END SECTION EA. —1— 1
CONNECTION TO EXISTING STRUCTURE EA. —1— 1
RIPRAP S.Y10.4*
TURF REINFORCEMENT MATTING S.Y. <del>880.1</del> 880.10*

AS PROVIDED BY CONTRACTOR\*

APPROVED RECORD DRAWING

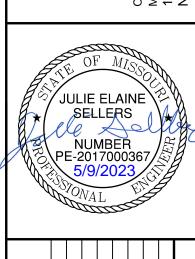
These plans have been reviewed for accuracy by the Development

**Services Staff** 

UNIT QUANTITY AS-BUILT

SUMMARY OF QUANTITIES AS INDICATED ABOVE AND ANY QUANTITIES AS SHOWN WITHIN THE PLANS HAVE BEEN PROVIDED FOR PERMITTING PURPOSES ONLY AND ARE NOT INTENDED FOR USE IN PREPARATION OF CONTRACT DOCUMENTS. QUANTITIES INTENDED FOR, BUT NOT LIMITED TO, THE PREPARATION OF PROPOSALS AND BID DOCUMENTS SHALL BE INDEPENDENTLY EVALUATED BY THE ESTIMATING PARTY BASED UPON THE CONTENTS OF THESE PLANS.

> AS BUILT 08-22-2022



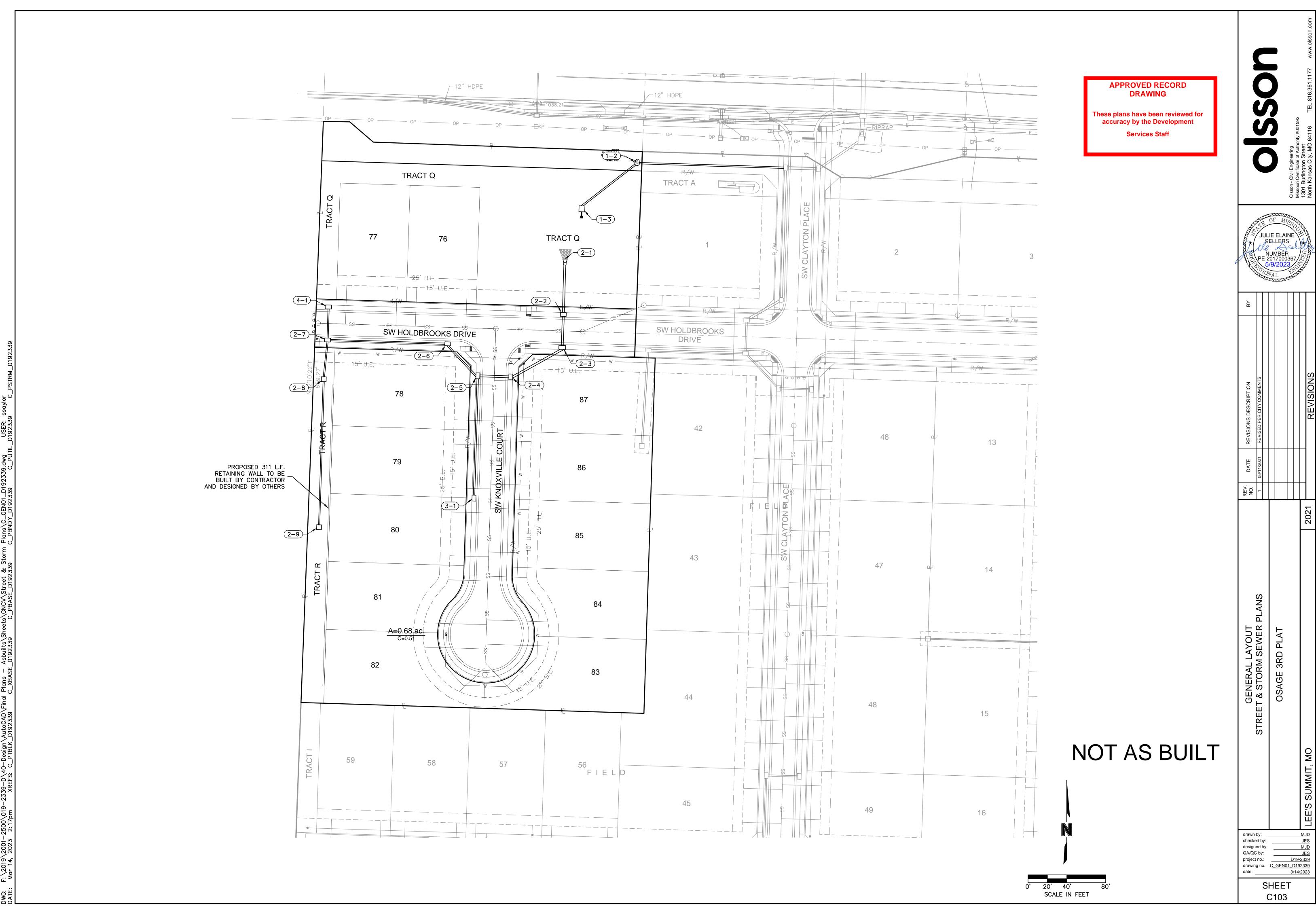
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drawn by: checked by designed by: QA/QC by: project no.: D19-2339

drawing no.: <u>C\_TTL01\_D192339</u>

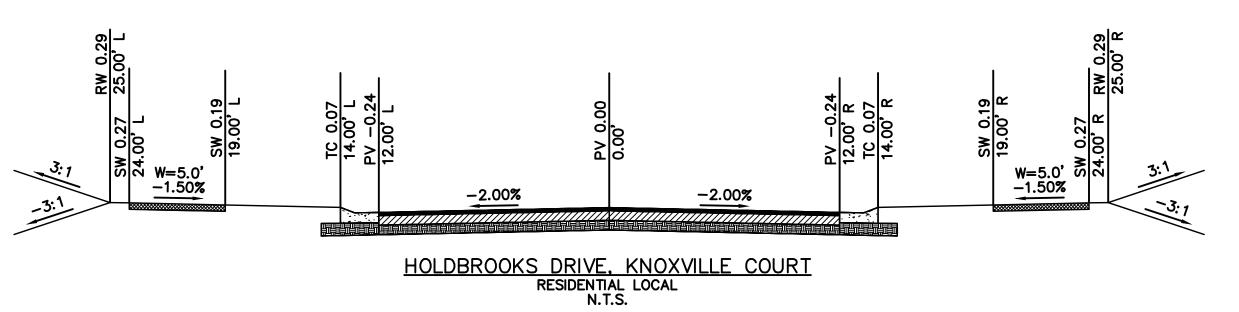
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3/14/2023

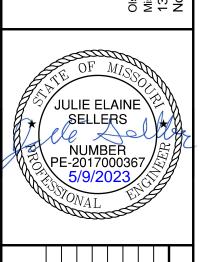


**Services Staff** 

APPROVED RECORD **DRAWING** These plans have been reviewed for accuracy by the Development

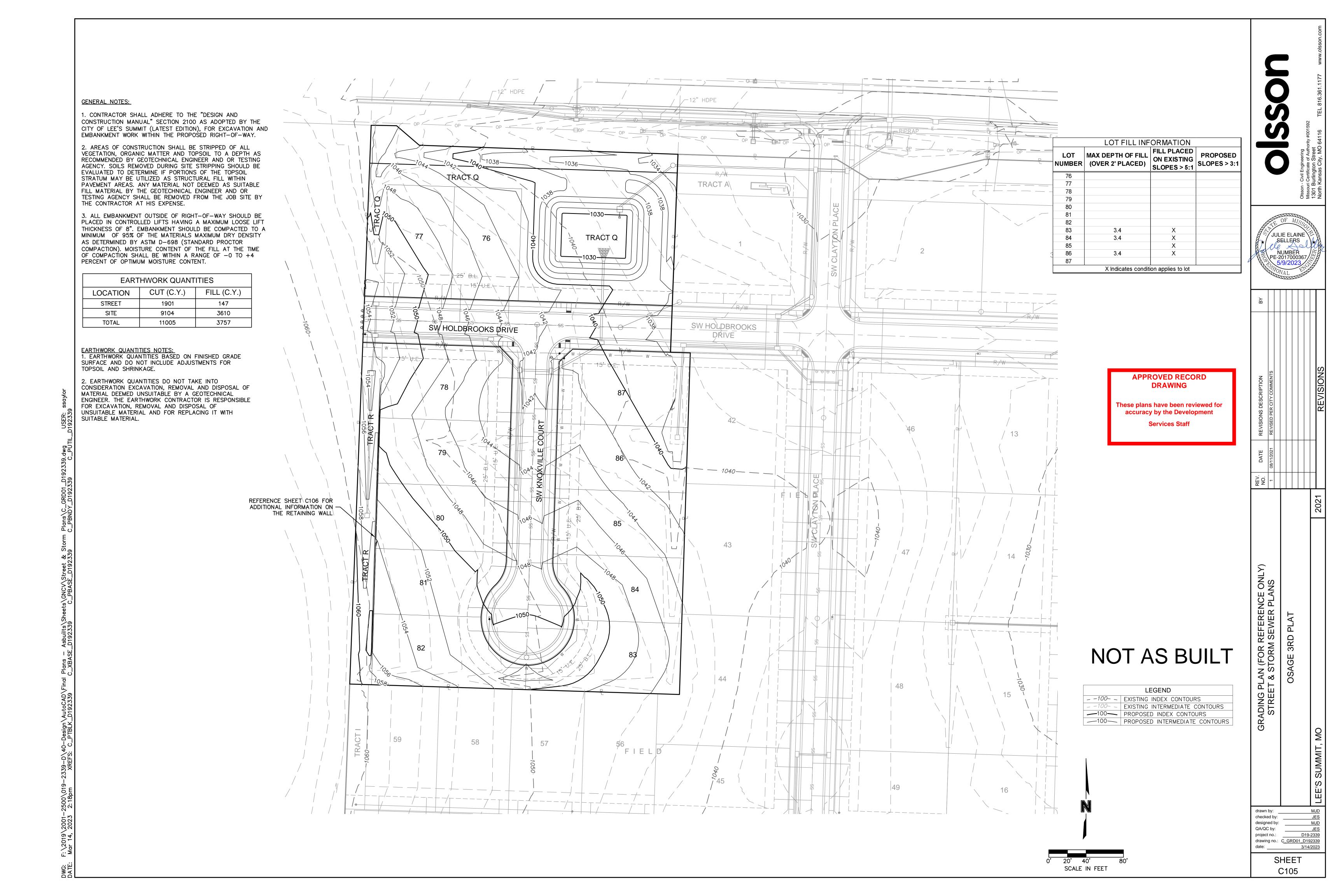


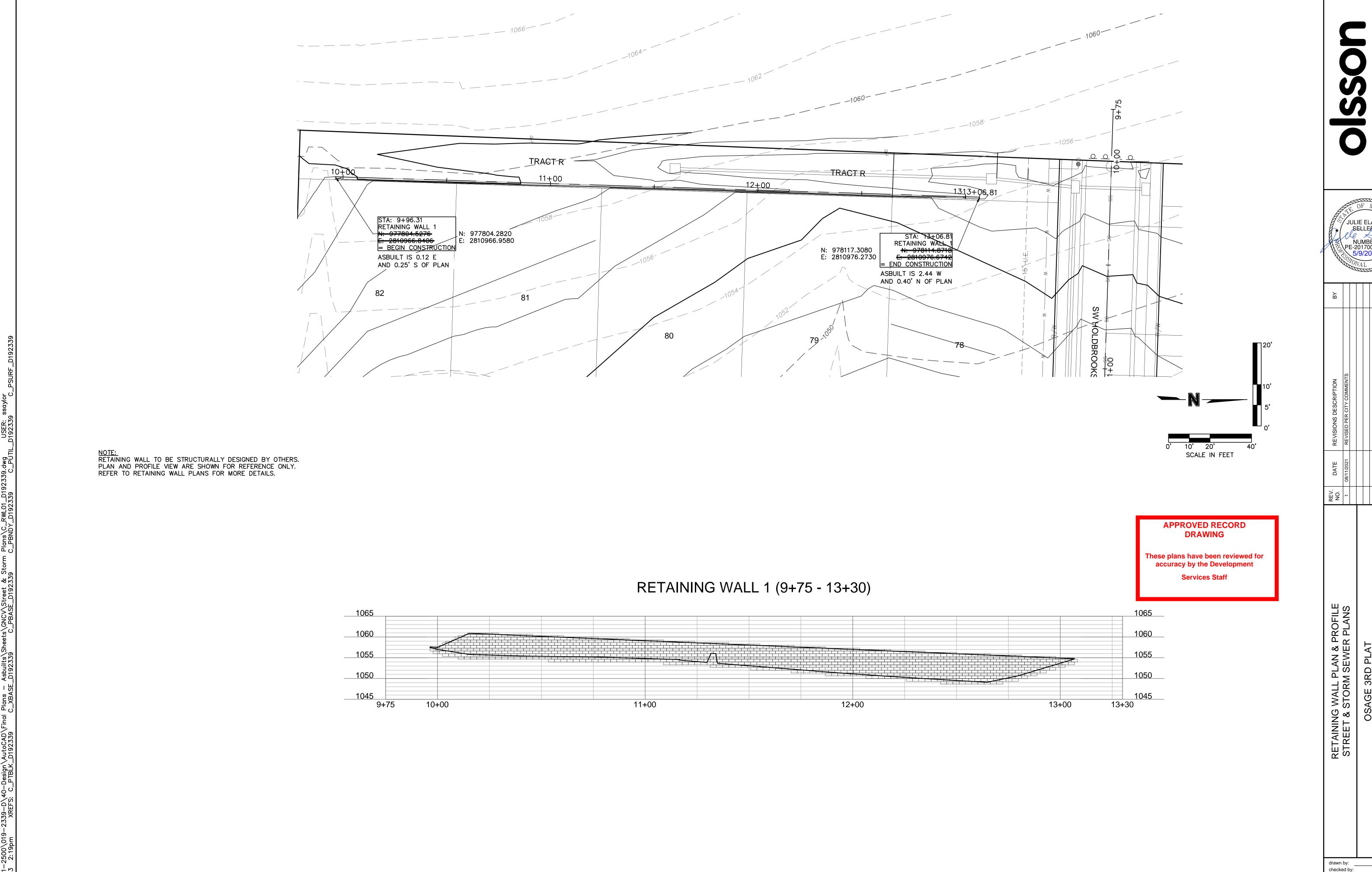
- 2" TYPE 6 ASPHALTIC CONCRETE SURFACE
- 4" TYPE 5 ASPHALTIC CONCRETE BASE COURSE
- 6" MIN. MODOT TYPE 5 STONE BASE
- 6" FLYASH STABILIZED SUBGRADE
- CG-2 CURB & GUTTER



NO.	KEVISIONS DESCRIPTION	ш
1 08/11/2021	REVISED PER CITY COMMENTS	
	REVISIONS	
		REVISIONS

drawn by: MJD checked by: JES designed by: MJD QA/QC by: JES project no.: D19-2339 drawing no.: C\_TYP01\_D192339 date: 3/14/2023





JULIE ELAINE SELLERS

 drawn by:
 MJD

 checked by:
 JES

 designed by:
 MJD

 QA/QC by:
 JES

 project no.:
 D19-2339

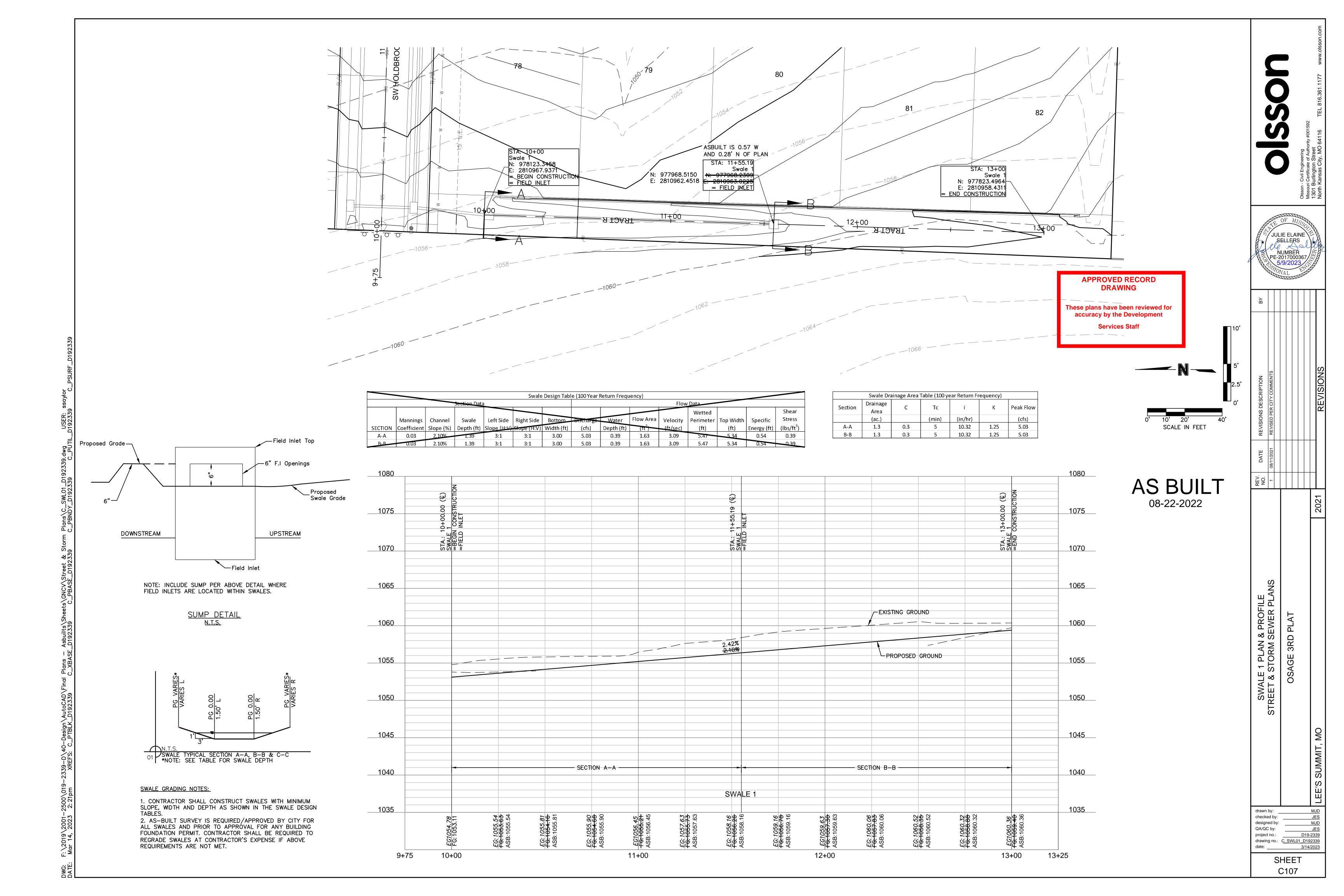
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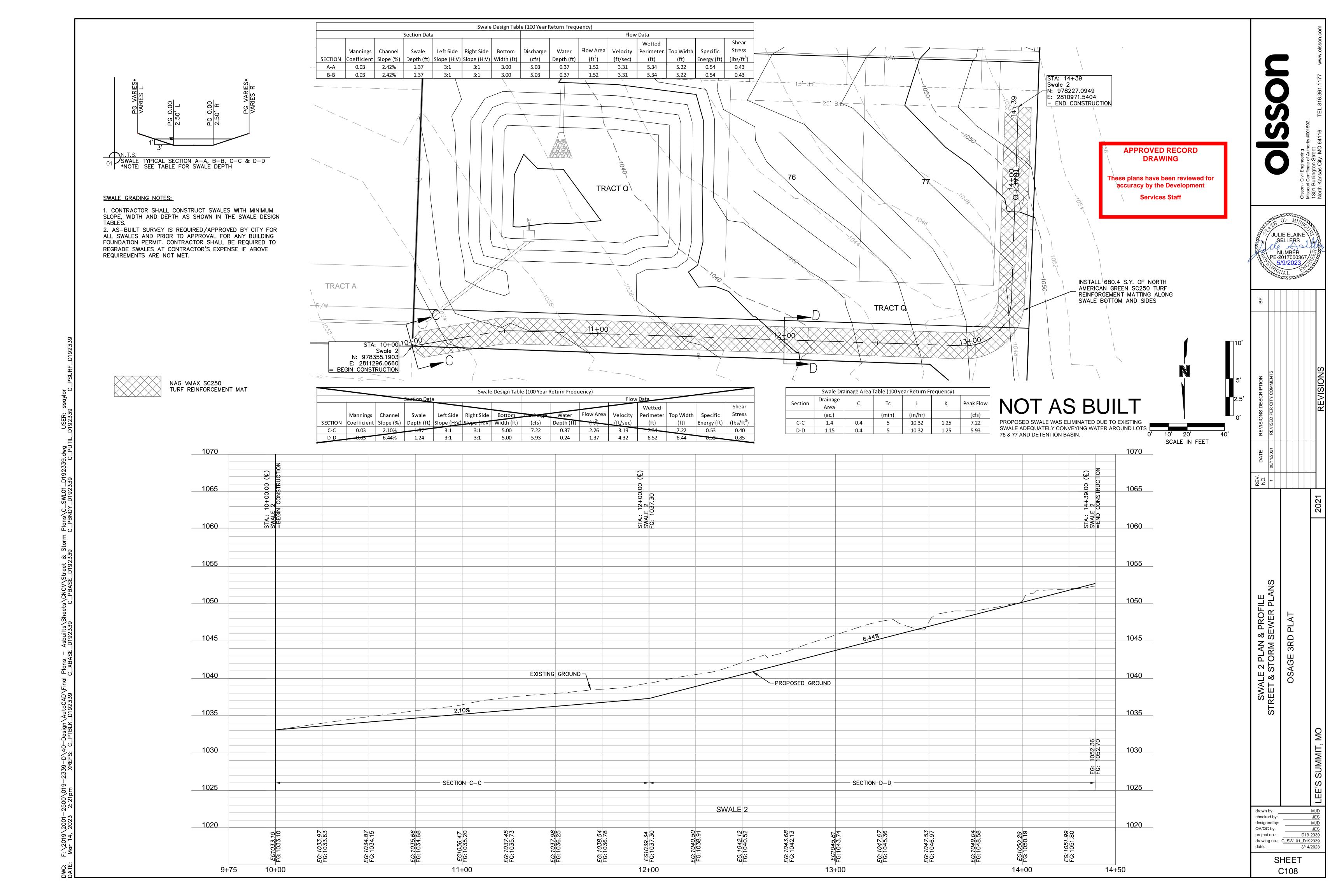
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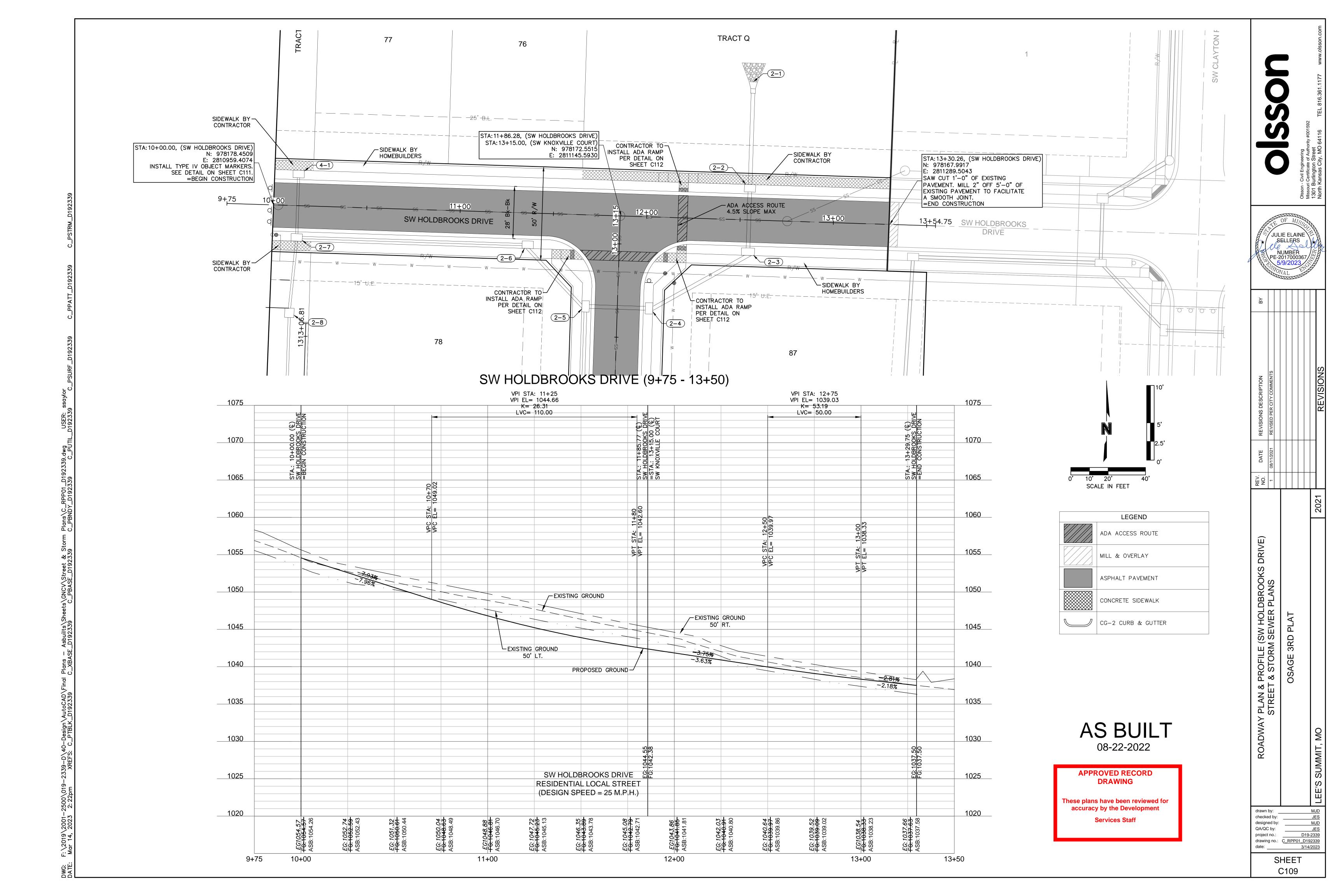
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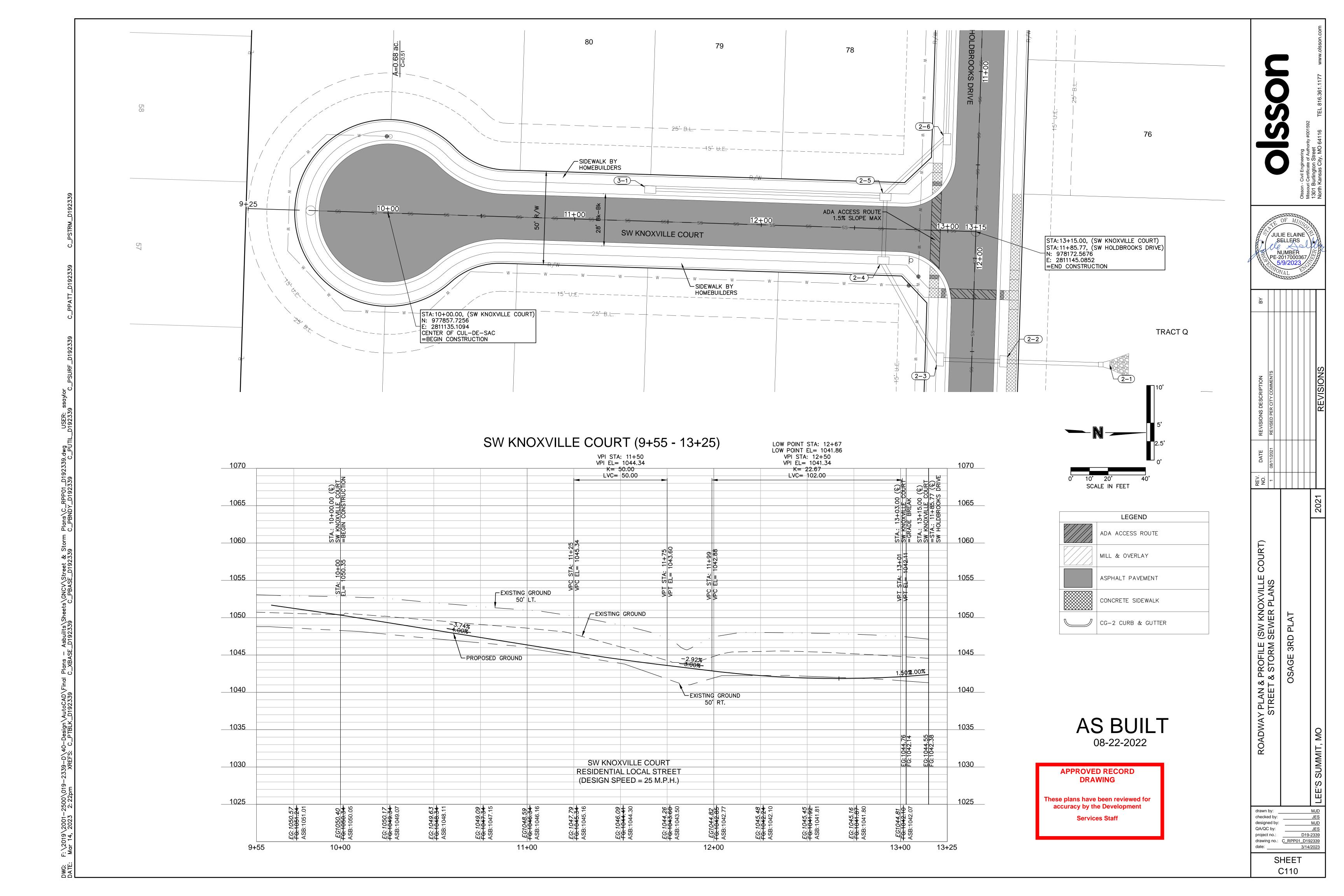
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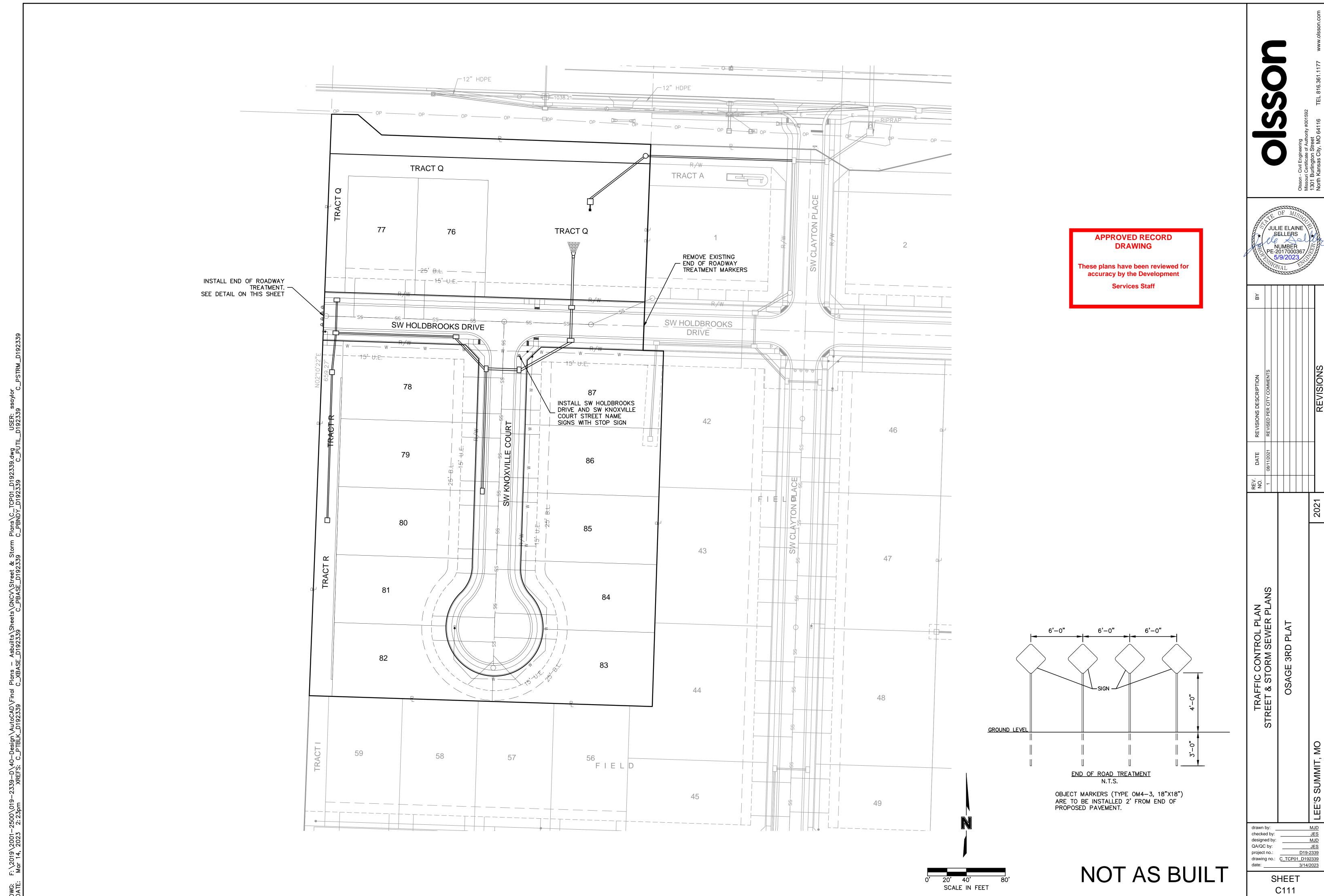
SHEET C106

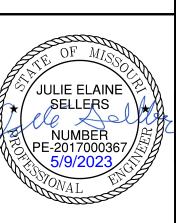












INTERSECTION AND ADA DETAIL NOTES:

1. ALL ADA CURB RAMPS SHALL BE BUILT PER CURRENT MUNICIPALITY ADOPTED ADA STANDARDS.

2. CURB RAMP FLARES SHALL NOT BE STEEPER THAN 1:10 MAX SLOPES.

3. LANDING SHALL BE PROVIDED WHERE INDICATED ON PLAN SHEET

OR BY PROWAG STANDARDS. LANDING SHALL BE 4'X4' MINIMUM.

4. RAMP RUNS SHALL HAVE A MAXIMUM RUNNING SLOPE OF 1:12
UNLESS THE RAMP LENGTH IS OVER 15 FEET, THEN THE SLOPE
CAN BE GREATER AS INDICATED IN DETAILS TO REACH STREET
GRADES.

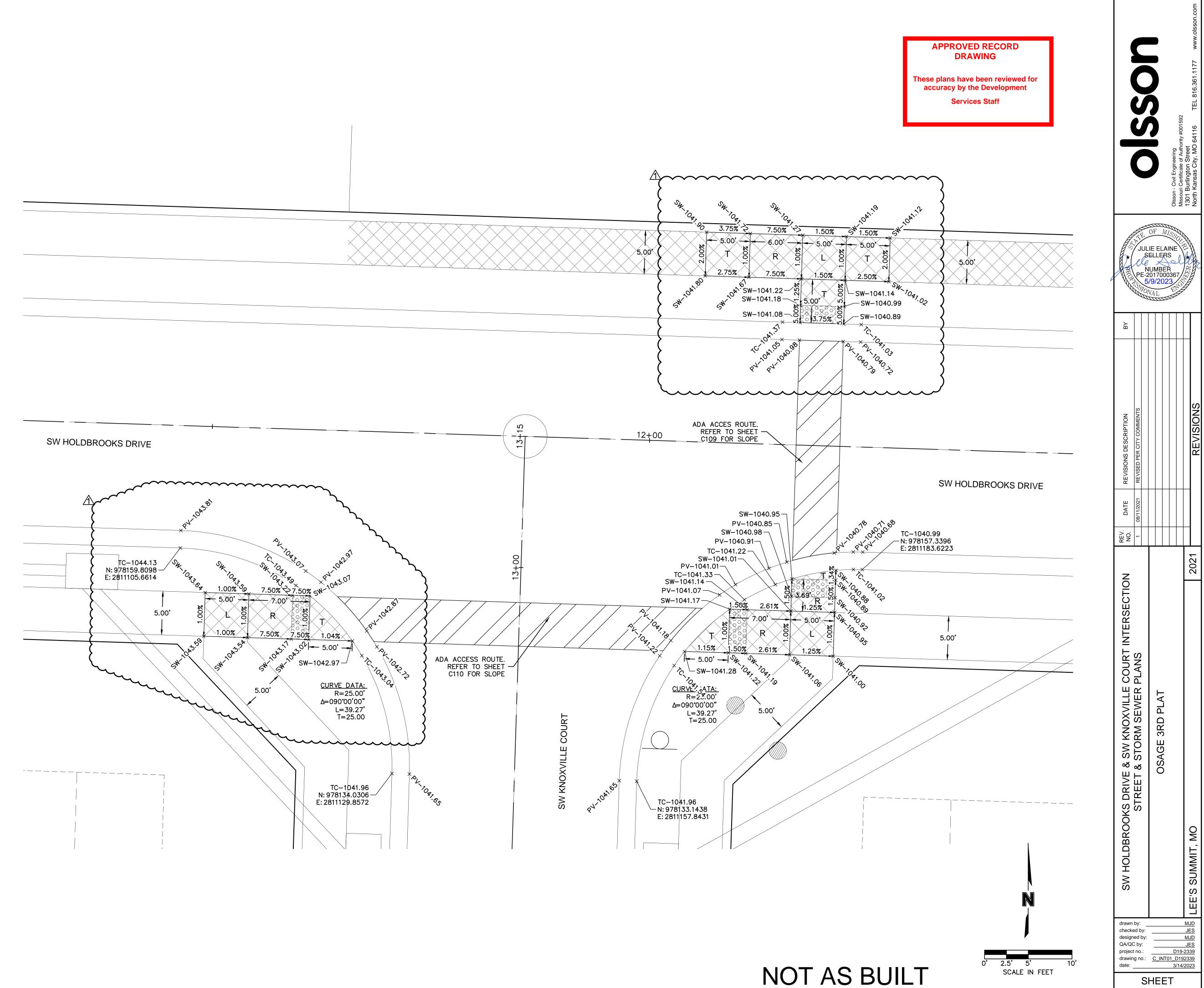
LANDINGS SHALL HAVE A MAXIMUM SLOPE OF 2% IN ANY DIRECTION.

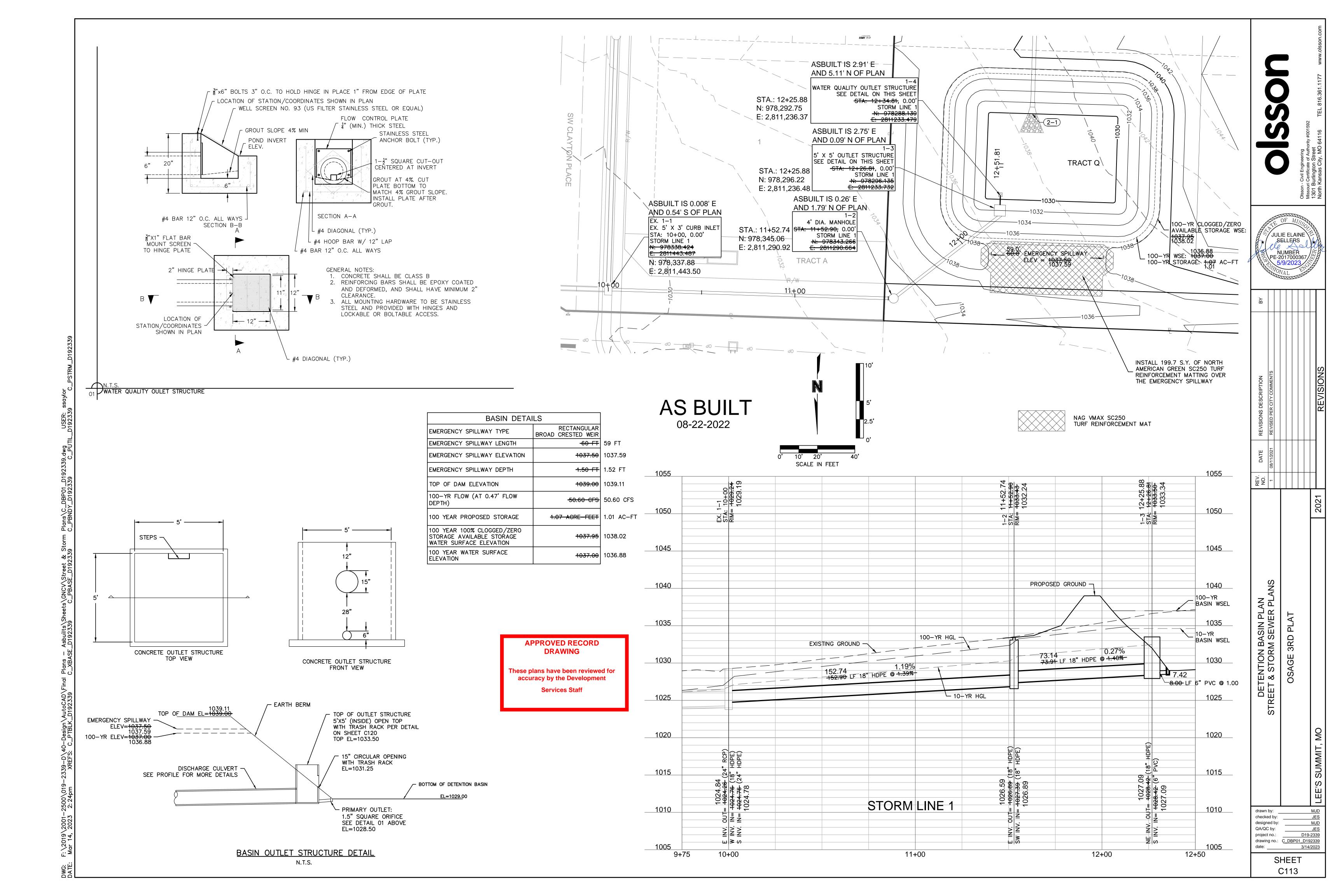
6. CROSS SLOPE FOR RAMPS AND SIDEWALK SHALL NOT EXCEED 2%.
7. AFTER CURBS HAVE BEEN CONSTRUCTED, AND BEFORE ASPHALT OR CONCRETE PAVEMENT IS POURED, CURBS SHOULD BE MEASURED WITH A LEVEL TO ENSURE CURB ALONG ADA RAMPS AND LANDINGS WILL MEET ADA REQUIREMENTS.

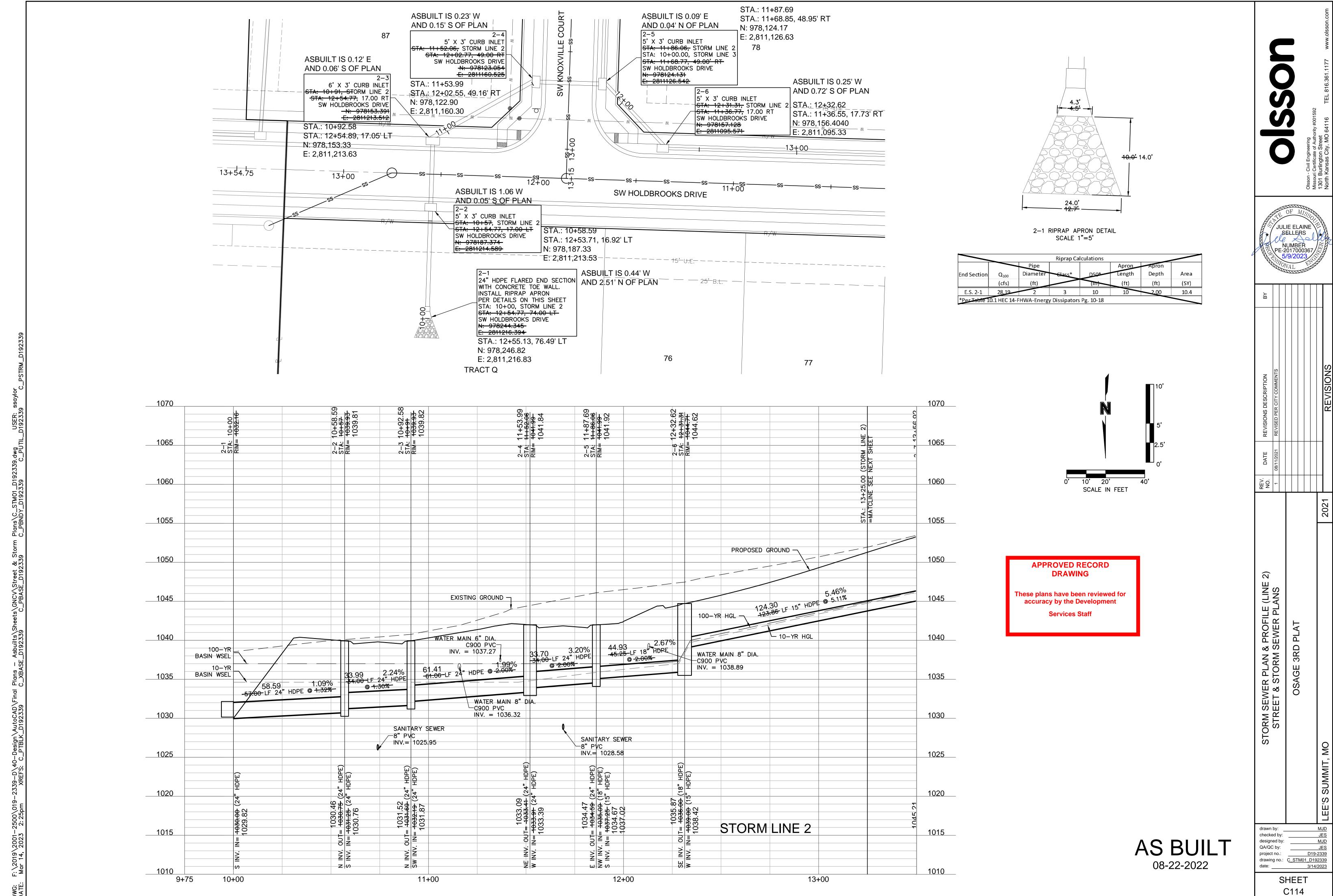
8. ADA RAMP CONSTRUCTION WILL BE INSPECTED THOROUGHLY BY THE CITY INSPECTOR. CONTRACTOR SHALL BE REQUIRED TO RECONSTRUCT RAMPS, CURBS AND/OR PAVEMENT AT CONTRACTOR'S EXPENSE IF ADA RAMPS AND LANDINGS CANNOT MEET THE ADA REQUIREMENTS, PER APPROVED PLAN OR APPROVED ALTERNATIVE.

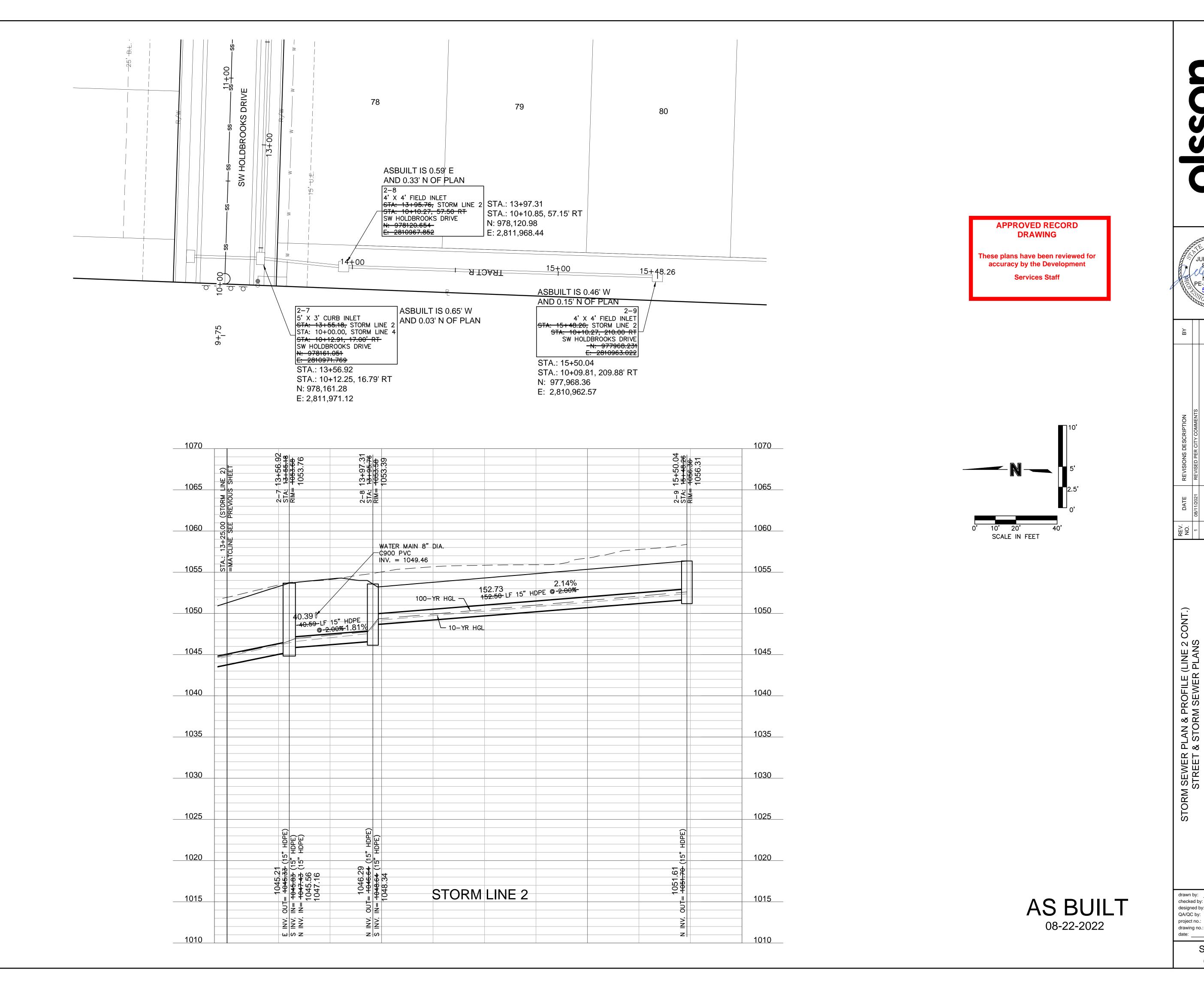
9. CURVE DATA IS FOR BACK OF CURB.

	LEGEND								
TC-	TOP OF CURB								
PV-	TOP OF PAVEMENT								
SW-	SIDEWALK								
L	LANDING AREA								
R	RAMP AREA								
Т	TRANSITION AREA								
	ADA ACCESS ROUTE								
	CG-2 CURB & GUTTER								
	CONCRETE SIDEWALK (CONSTRUCTED BY CONTRACTOR)								

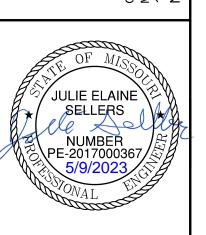








1 - Civil Engineering
Aurlington Street
Kansas City, MO 64116 TEL 816.361.1177 www.ols



NO. DATE REVISED PER CITY COMMENTS

1 08/11/2021 REVISED PER CITY COMMENTS

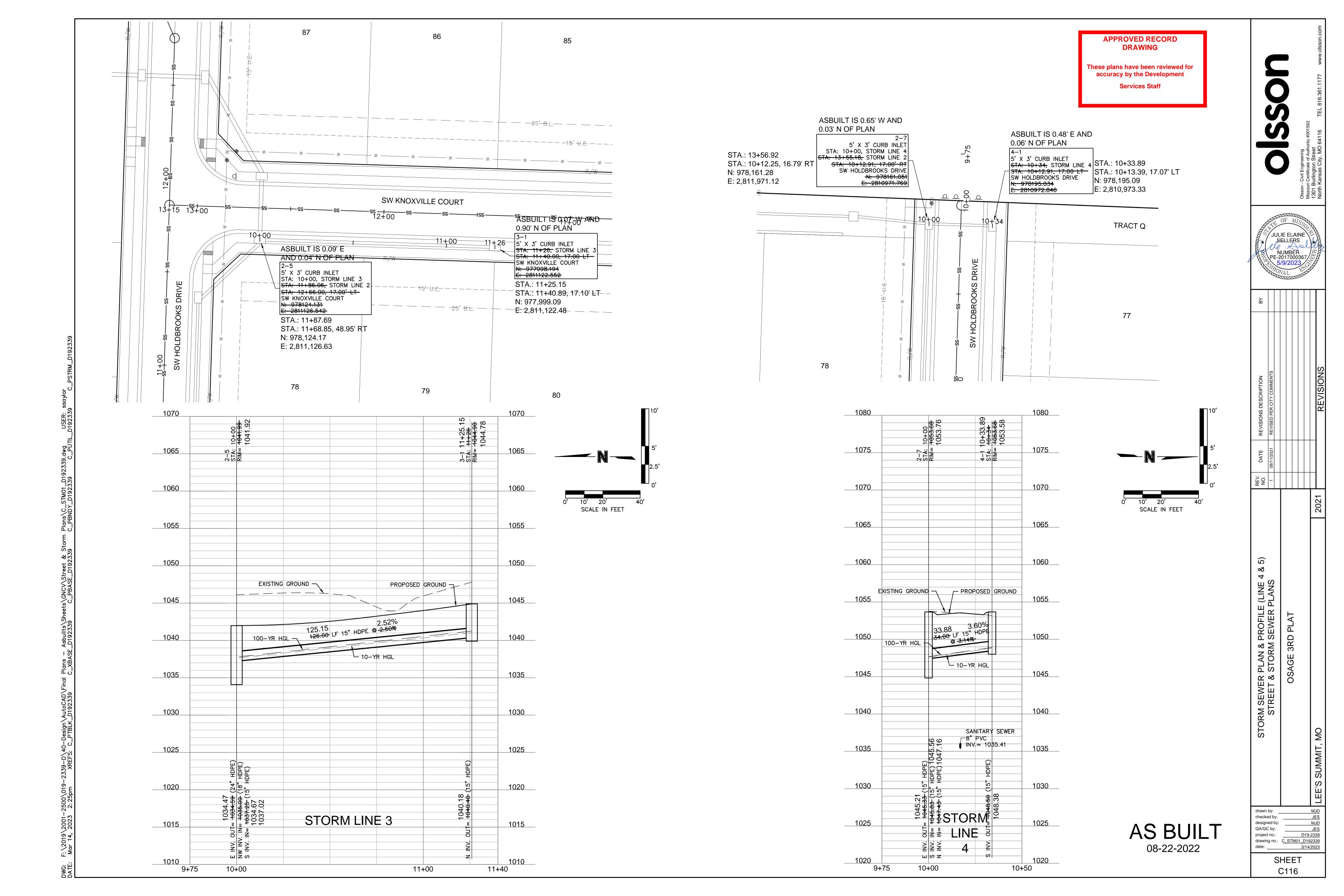
2021 REVISED PER CITY COMMENTS

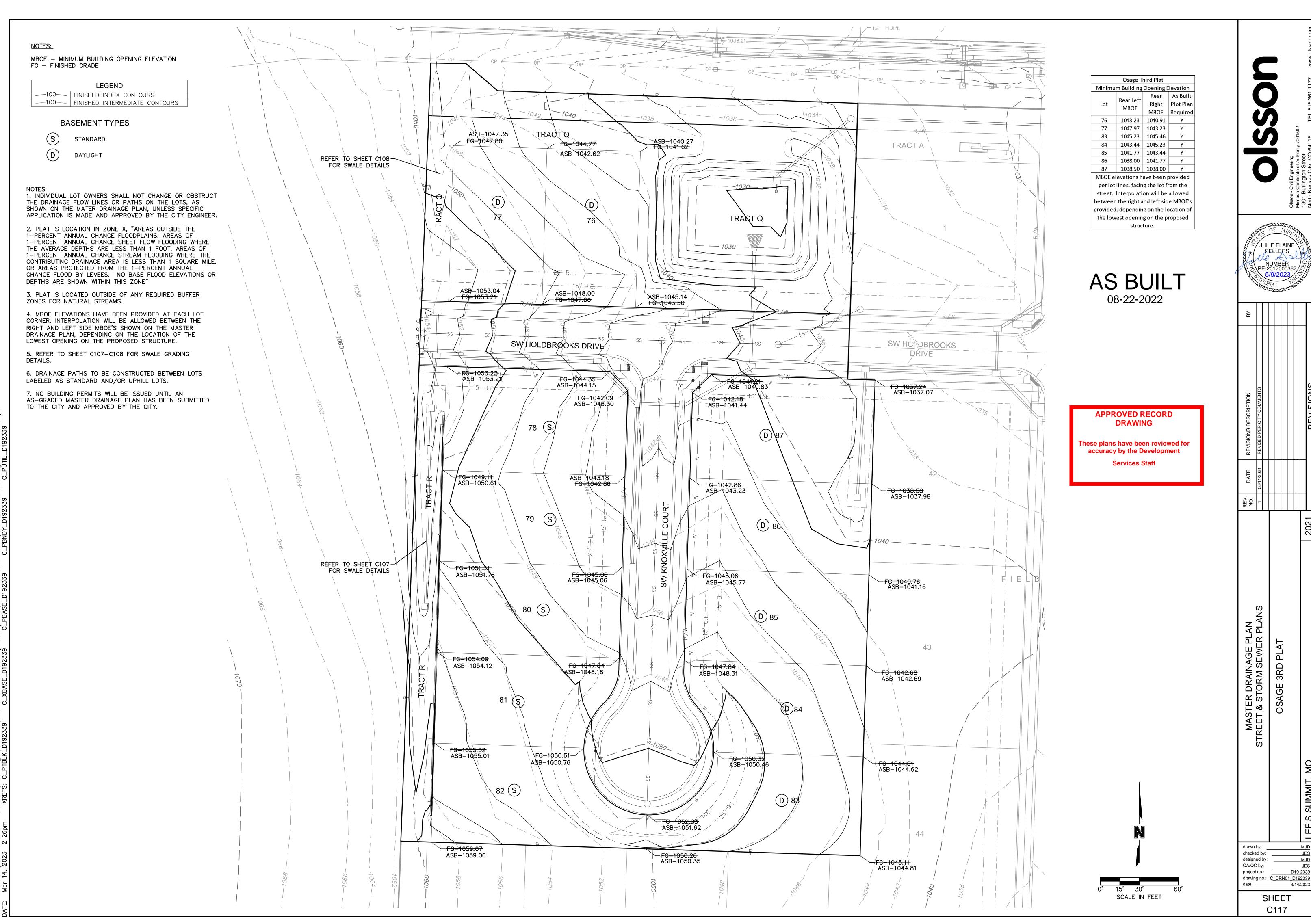
REVISIONS

REVISIONS

drawn by:		MJD
checked by:		<u>JES</u>
designed by:		MJD
QA/QC by:		<b>JES</b>
project no.:	D19-	2339
drawing no.:	C_STM01_D19	2339
date:	3/14/	2023
SI	HEET	

C115



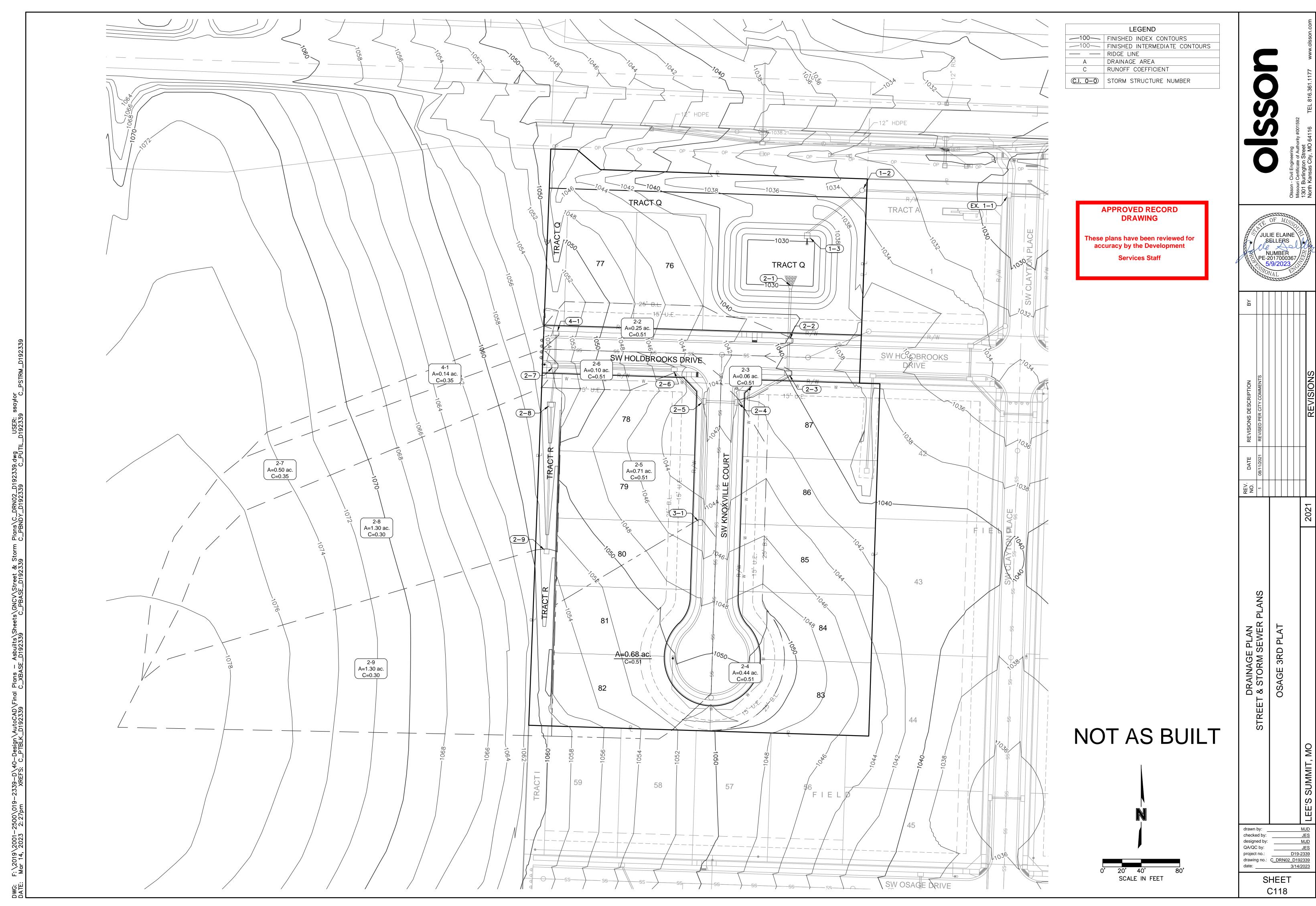


JULIE ELAINE SELLERS NUMBER PE-2017000367

MASTER DRAINAGE PLAN REET & STORM SEWER PLANS

checked by: designed by: QA/QC by: project no.: D19-2339

SHEET



Drainage Are	a Design Tabl	e				
10	Year Return I	requency				
	Drainage					
Inlet ID	Area	С	Tc	i	K	Peak Flow
	(ac)		(min)	(in/hr)		(cfs)
C.I. 2-2	0.25	0.51	5.00	7.35	1.00	0.94
C.I. 2-3	0.06	0.51	5.00	7.35	1.00	0.23
C.I. 2-4	0.44	0.51	5.00	7.35	1.00	1.65
C.I. 2-5	0.71	0.51	5.00	7.35	1.00	2.66
C.I. 2-6	0.10	0.51	5.00	7.35	1.00	0.38
C.I. 2-7	0.50	0.35	5.00	7.35	1.00	1.29
F.I. 2-8	1.30	0.30	5.00	7.35	1.00	2.87
F.I. 2-9	1.30	0.30	5.00	7.35	1.00	2.87
C.I. 3-1	0.68	0.51	5.00	7.35	1.00	2.55
C.I. 4-1	0.25	0.35	5.00	7.35	1.00	0.64

ainage Are	ea Design Tabl	e				
100	Year Return F	requency				
Inlet ID	Drainage Area	С	Тс	i	К	Peak Flow
	(ac)		(min)	(in/hr)		(cfs)
C.I. 2-2	0.25	0.51	5.00	10.32	1.25	1.65
C.I. 2-3	0.06	0.51	5.00	10.32	1.25	0.39
C.I. 2-4	0.44	0.51	5.00	10.32	1.25	2.90
C.I. 2-5	0.71	0.51	5.00	10.32	1.25	4.67
C.I. 2-6	0.10	0.51	5.00	10.32	1.25	0.66
C.I. 2-7	0.50	0.35	5.00	10.32	1.25	2.26
F.I. 2-8	1.30	0.30	5.00	10.32	1.25	5.03
F.I. 2-9	1.30	0.30	5.00	10.32	1.25	5.03
C.I. 3-1	0.68	0.51	5.00	10.32	1.25	4.47
C.I. 4-1	0.25	0.35	5.00	10.32	1.25	1.13

APPROVED RECORD

**DRAWING** These plans have been reviewed for accuracy by the Development **Services Staff** 

Inlet Design	Table												
10	Year Return	Frequency											
	Inlet		Upstream		Clogging	Inlet	Sag Inlet	Captured		Inlet	Gutter	Gutter	Ponding
Inlet ID	Location	Peak Flow	Bypass	Total Flow	Factor	Capacity	Capacity	Flow	Bypass Flow	Efficiency	Depth	Spread	Depth
							(Note 1)			(Note 2)			
		(cfs)	(cfs)	(cfs)		(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
C.I. 2-2	GRADE	0.94	0.10	1.04	1.00	0.89	0.89	0.89	0.15	85.63%	0.11	5.30	
C.I. 2-3	GRADE	0.23	0.10	0.33	1.00	0.31	0.31	0.31	0.01	95.94%	0.07	3.44	
C.I. 2-4(L)	SAG	0.00	•••			•••			•••	•••	0.00	0.00	•••
C.I. 2-4(R)	SAG	1.28	•••	•••	•••	•••		•••	•••	•••	0.11	5.74	•••
C.I. 2-4(B)	SAG	0.38	•••			•••			•••	•••		•••	***
C.I. 2-4	SAG	1.65	0.00	1.65	0.80	19.40	15.52	1.65	0.00	100.00%		•••	•••
C.I. 2-5(L)	SAG	0.94	•••	•••		•••		•••	•••	•••	0.14	7.03	***
C.I. 2-5(R)	SAG	0.00	•••	•••	•••	•••	•••	•••	•••	•••	0.00	0.00	***
C.I. 2-5(B)	SAG	1.73	•••		•••	•••			•••	•••		•••	•••
C.I. 2-5	SAG	2.66	0.00	2.66	0.80	19.40	15.52	2.66	0.00	100.00%	•••	***	***
C.I. 2-6	GRADE	0.38	0.37	0.74	1.00	0.64	0.64	0.64	0.10	86.35%	0.09	4.34	***
C.I. 2-7	GRADE	1.29	0.00	1.29	1.00	0.92	0.92	0.92	0.37	71.44%	0.10	4.96	•••
F.I. 2-8	SAG	2.87	0.00	2.87	0.80	14.00	11.20	2.87	0.00	100.00%		•••	0.17
F.I. 2-9	SAG	2.87	0.00	2.87	0.80	14.00	11.20	2.87	0.00	100.00%	•••	***	0.17
C.I. 3-1	GRADE	2.55	0.00	2.55	1.00	1.86	1.86	1.86	0.69	72.90%	0.16	7.83	•••
C.I. 4-1	GRADE	0.64	0.00	0.64	1.00	0.54	0.54	0.54	0.10	84.59%	0.08	3.83	

1. Inlet capacity at sag location has been reduced by a clogging factor of 0.80, reducing theoretical capacity to 80% capacity, as required per APWA Section 5600.

Both theoretical capacity and reduced capacity are shown.

2. Inlet efficiency shown in the tables is Captured Flow/Total Flow, denoting the actual percentage of flow captured after the capacity has been reduced to 80% of theoretical capacity.

100	Year Return I	requency											
	Inlet		Upstream		Clogging	Inlet	Sag Inlet	Captured		Inlet	Gutter	Gutter	Ponding
Inlet ID	Location	Peak Flow	Bypass	Total Flow	Factor	Capacity	Capacity	Flow	Bypass Flow	Efficiency	Depth	Spread	Depth
							(Note 1)			(Note 2)			
		(cfs)	(cfs)	(cfs)		(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
C.I. 2-2	GRADE	1.65	0.29	1.93	1.00	1.44	1.44	1.44	0.49	74.62%	0.13	6.70	
C.I. 2-3	GRADE	0.39	0.46	0.85	1.00	0.75	0.75	0.75	0.10	88.15%	0.10	4.93	
C.I. 2-4(L)	SAG	0.00	•••		•••			•••			0.00	0.00	
C.I. 2-4(R)	SAG	2.24	•••		•••		•••	•••		•••	0.14	7.09	•••
C.I. 2-4(B)	SAG	0.66	•••		•••		•••	•••		•••	•••	•••	•••
C.I. 2-4	SAG	2.90	0.00	2.90	0.80	19.40	15.52	2.90	0.00	100.00%	•••	•••	•••
C.I. 2-5(L)	SAG	1.65	•••		•••		•••	•••		•••	0.19	9.37	•••
C.I. 2-5(R)	SAG	0.00	•••	•••	•••		•••	•••		•••	0.00	0.00	•••
C.I. 2-5(B)	SAG	3.03	•••		•••			•••			•••		•••
C.I. 2-5	SAG	4.67	0.00	4.67	0.80	19.40	15.52	4.67	0.00	100.00%	•••	***	•••
C.I. 2-6	GRADE	0.66	0.98	1.64	1.00	1.18	1.18	1.18	0.46	72.05%	0.12	5.84	•••
C.I. 2-7	GRADE	2.26	0.00	2.26	1.00	1.28	1.28	1.28	0.98	56.53%	0.12	6.13	
F.I. 2-8	SAG	5.03	0.00	5.03	0.80	14.00	11.20	5.03	0.00	100.00%	•••		0.25
F.I. 2-9	SAG	5.03	0.00	5.03	0.80	14.00	11.20	5.03	0.00	100.00%	•••	***	0.25
C.I. 3-1	GRADE	4.47	0.00	4.47	1.00	2.61	2.61	2.61	1.86	58.37%	0.19	9.67	•
C.I. 4-1	GRADE	1.13	0.00	1.13	1.00	0.84	0.84	0.84	0.29	74.38%	0.09	4.72	

1. Inlet capacity at sag location has been reduced by a clogging factor of 0.80, reducing theoretical capacity to 80% capacity, as required per APWA Section 5600.

Both theoretical capacity and reduced capacity are shown.

2. Inlet efficiency shown in the tables is Captured Flow/Total Flow, denoting the actual percentage of flow captured after the capacity has been reduced to 80% of theoretical capacity.

10	Year Return Freq	uency											
Upstream	Downstream		Upstream	Downstream			Manning's					Upstream	Upstream
Structure	Structure	Length	Invert	Invert	Slope	Diameter	n	Total Flow	Velocity	Capacity	Flow Depth	Struct. HGL	Top Elev.
		(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(ft/s)	(cfs)	(ft)	(ft)	(ft)
1-2	EX. 1-1	152.90	1026.89	1024.76	1.39	18	0.012	16.66	9.43	13.43	1.50	1030.34	1033.41
1-3	1-2	73.91	1028.42	1027.39	1.39	18	0.012	16.66	9.43	13.43	1.50	1032.48	1033.50
2-2	2-1	57.00	1030.75	1030.00	1.32	24	0.012	16.08	6.10	28.11	1.72	1032.20 j	1039.93
2-3	2-2	34.00	1031.69	1031.25	1.29	24	0.012	15.14	7.75	27.87	1.05	1033.09	1039.93
2-4	2-3	61.06	1033.41	1032.19	2.00	24	0.012	14.91	8.50	34.64	0.92	1034.80	1041.90
2-5	2-4	34.00	1034.59	1033.91	2.00	24	0.012	13.26	7.94	34.65	0.89	1035.90	1041.90
2-6	2-5	45.26	1036.00	1035.10	1.99	18	0.012	8.05	7.11	16.04	0.80	1037.10	1044.71
2-7	2-6	123.87	1045.33	1039.01	5.10	15	0.012	7.67	9.75	15.80	0.61	1046.43	1053.79
2-8	2-7	40.59	1046.64	1045.83	2.00	15	0.012	5.74	6.99	9.88	0.68	1047.61	1053.72
2-9	2-8	152.50	1051.70	1048.65	2.00	15	0.012	2.87	5.59	9.89	0.46	1052.38	1056.36
3-1	2-5	126.00	1040.40	1037.25	2.50	15	0.012	2.55	5.68	11.06	0.41	1041.04	1045.44
4-1	2-7	34.00	1048.50	1047.43	3.15	15	0.012	0.64	3.99	12.41	0.19	1048.81	1053.44

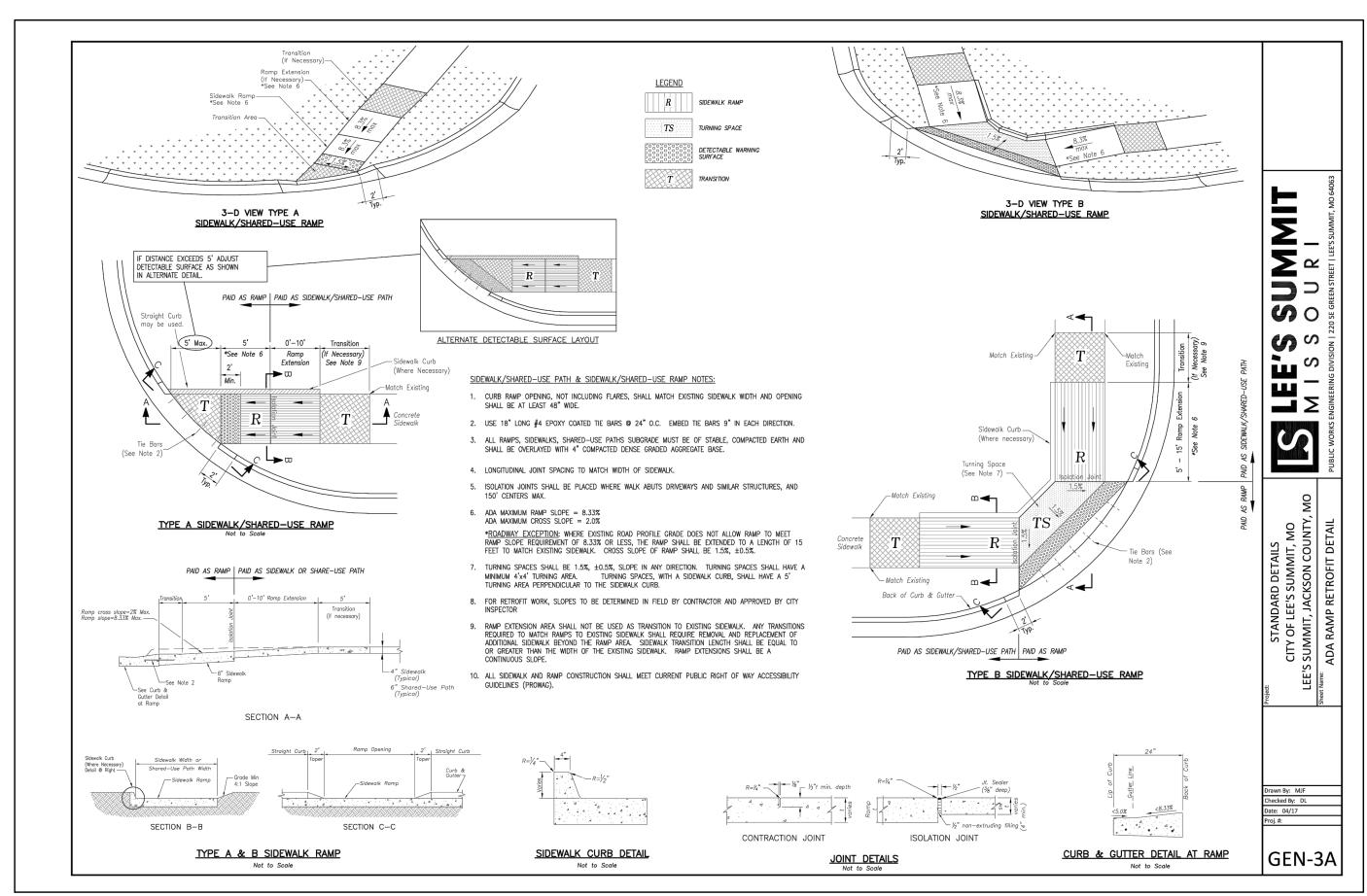
100	Year Return Freq	luency											
Upstream	Downstream		Upstream	Downstream			Manning's					Upstream	Upstream
Structure	Structure	Length	Invert	Invert	Slope	Diameter	n	Total Flow	Velocity	Capacity	Flow Depth	Struct. HGL	Top Elev.
		(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(ft/s)	(cfs)	(ft)	(ft)	(ft)
1-2	EX. 1-1	152.90	1026.89	1024.76	1.39	18	0.012	19.11	10.82	13.43	1.50	1032.44	1033.41
1-3	1-2	73.91	1028.42	1027.39	1.39	18	0.012	19.11	10.82	13.43	1.50	1035.26	1033.50
2-2	2-1	57.00	1030.75	1030.00	1.32	24	0.012	28.19	9.23	28.11	1.92	1032.58	1039.93
2-3	2-2	34.00	1031.69	1031.25	1.29	24	0.012	26.54	9.51	27.87	1.56	1033.49	1039.93
2-4	2-3	61.06	1033.41	1032.19	2.00	24	0.012	26.15	10.47	34.64	1.30	1035.20	1041.90
2-5	2-4	34.00	1034.59	1033.91	2.00	24	0.012	23.25	9.49	34.65	1.29	1036.30	1041.90
2-6	2-5	45.26	1036.00	1035.10	1.99	18	0.012	14.11	8.79	16.04	1.20	1037.38	1044.71
2-7	2-6	123.87	1045.33	1039.01	5.10	15	0.012	13.45	12.73	15.80	0.89	1046.56	1053.79
2-8	2-7	40.59	1046.64	1045.83	2.00	15	0.012	10.06	8.77	9.88	1.05	1047.83	1053.72
2-9	2-8	152.50	1051.70	1048.65	2.00	15	0.012	5.03	6.68	9.89	0.63	1052.61	1056.36
3-1	2-5	126.00	1040.40	1037.25	2.50	15	0.012	4.47	6.76	11.06	0.55	1041.26	1045.44
4-1	2-7	34.00	1048.50	1047.43	3.15	15	0.012	1.13	4.71	12.41	0.26	1048.92	1053.44

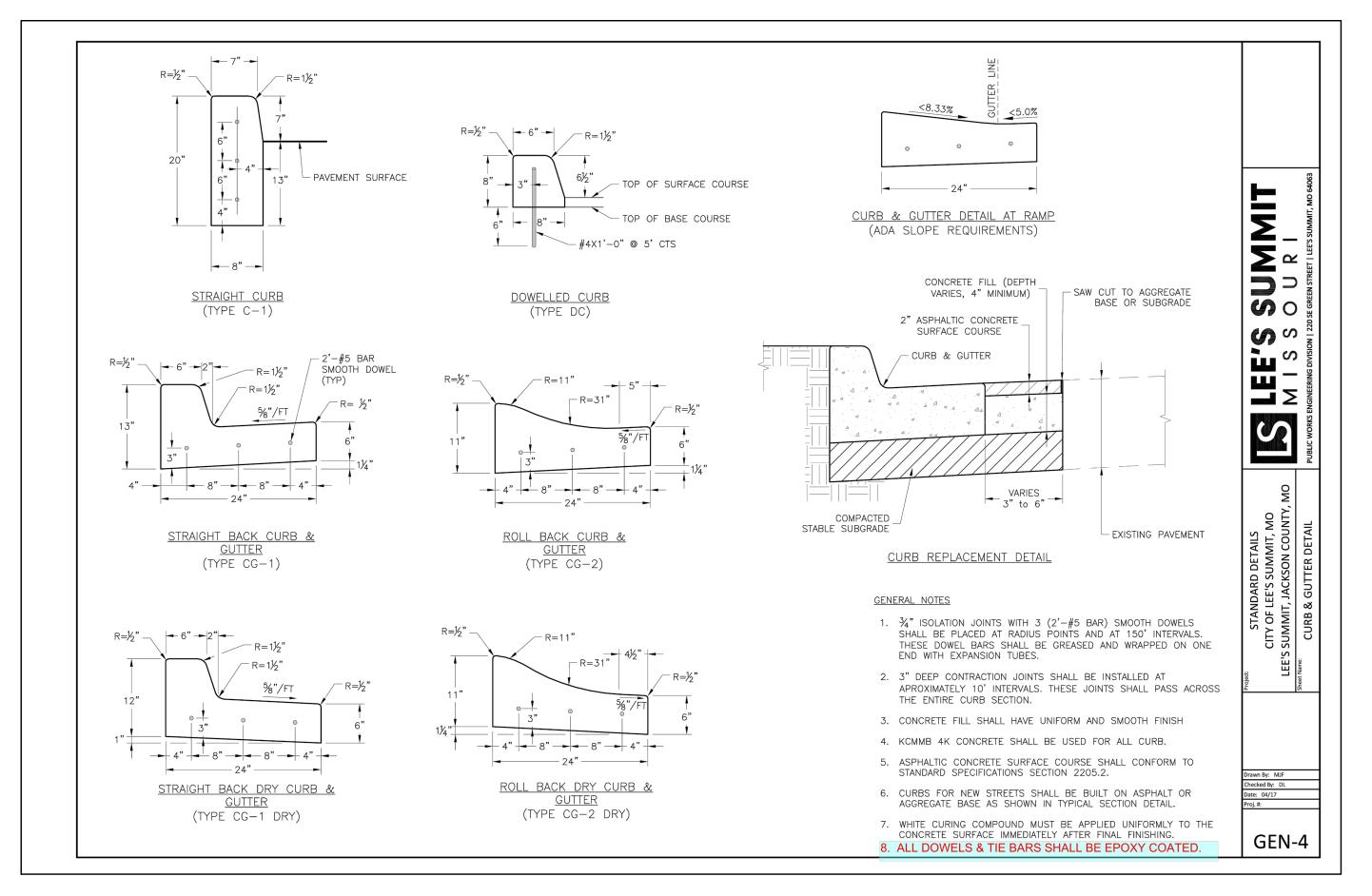
checked by: JES
designed by: MJD
QA/QC by: JES
project no.: D19-2339
drawing no.: C\_DRN02\_D192339
date: 3/14/2023 SHEET

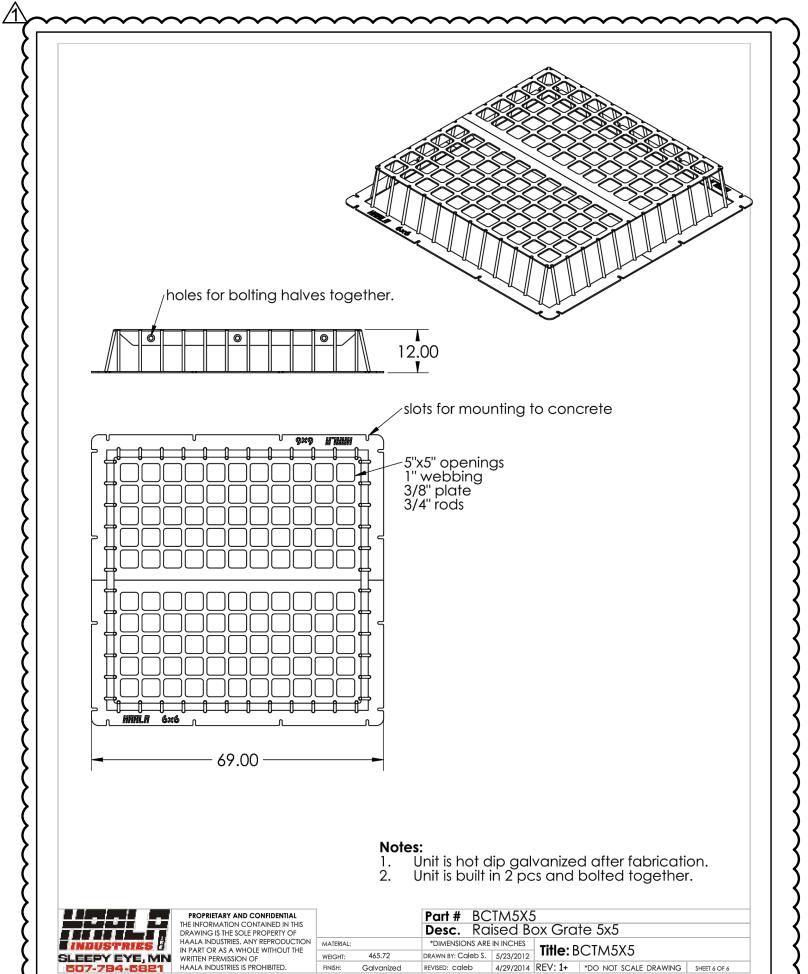
JULIE ELAINE SELLERS

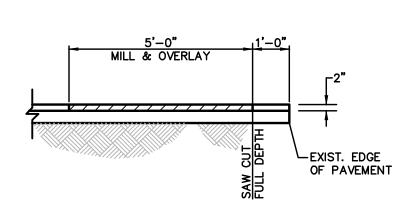
These plans have been reviewed for accuracy by the Development

Services Staff

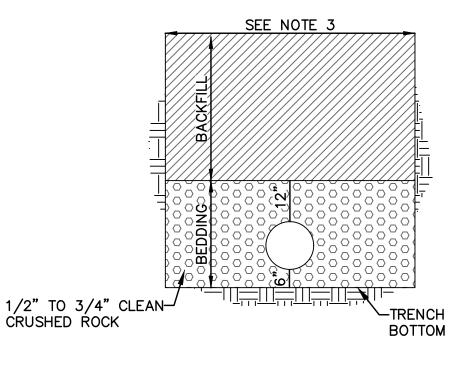








MILL & OVERLAY DETAIL N.T.S.



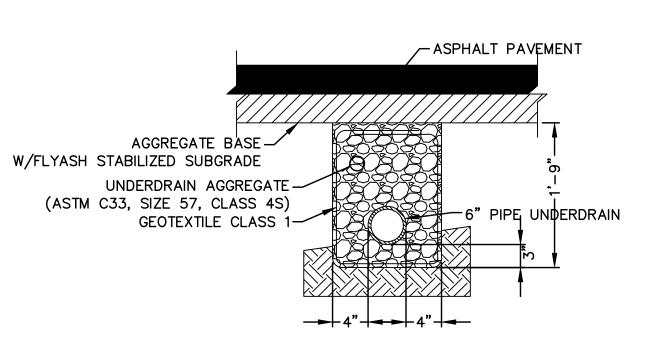
TYPICAL SECTION FOR PLASTIC PIPE (IN ROCK OR SOIL)

## UNDERGROUND PIPE INSTALLATION FOR STORM SEWER LINES

1. BACKFILL SHALL BE JOB EXECAVATED MATERIAL FREE FROM DEBRIS AND STONES COMPACTED TO 90% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT AS DETERMINED BY ASTM D698. BACKFILL UNDER PAVEMENT (EXISTING OR PROPOSED), SHALL BE FLOWABLE FILL.

2. TRENCH BANKS MAY BE CUT BACK ON SLOPES IN ACCORDANCE WITH CURRENT OSHA REGULATIONS, BUT ONLY IN AREAS WHERE THE INCREASED TRENCH WIDTH WILL NOT INTERFERE WITH SURFACE FEATURES. SLOPES MUST NOT EXTEND BELOW TOP OF BEDDING.

3. MINIMUM AND MAXIMUM WIDTHS SHALL BE IN ACCORDANCE WITH PIPE MANUFACTURER'S RECOMMENDATION AS APPROVED ON ENGINEERING PLANS.



PIPE UNDERDRAIN LATERAL N.T.S.

NOTES:

1. Where Pipe Underdrains are used, all Underdrain Outlet Pipes shall be solid wall with watertight joints. All Outlets Pipes shall be tied into the nearest storm sewer inlets at roadway sag locations as indicated in the street profile.

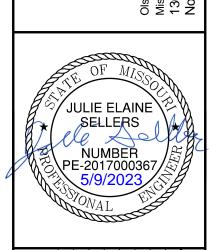
All Underdrain Pipes shall be installed at a minimum slope of 1%.
 Underdrain Pipe shall be installed with the perforations placed down.

4. Blanket Underdrain Aggregate, Pipe Underdrain Aggregate, Pipe Underdrain, Edge Underdrain and Outlet Pipe shall conform to City of

Lee's Summit Specifications.
5. Overlap geotextile at top of trench a minimum of 12".

NOT AS BUILT

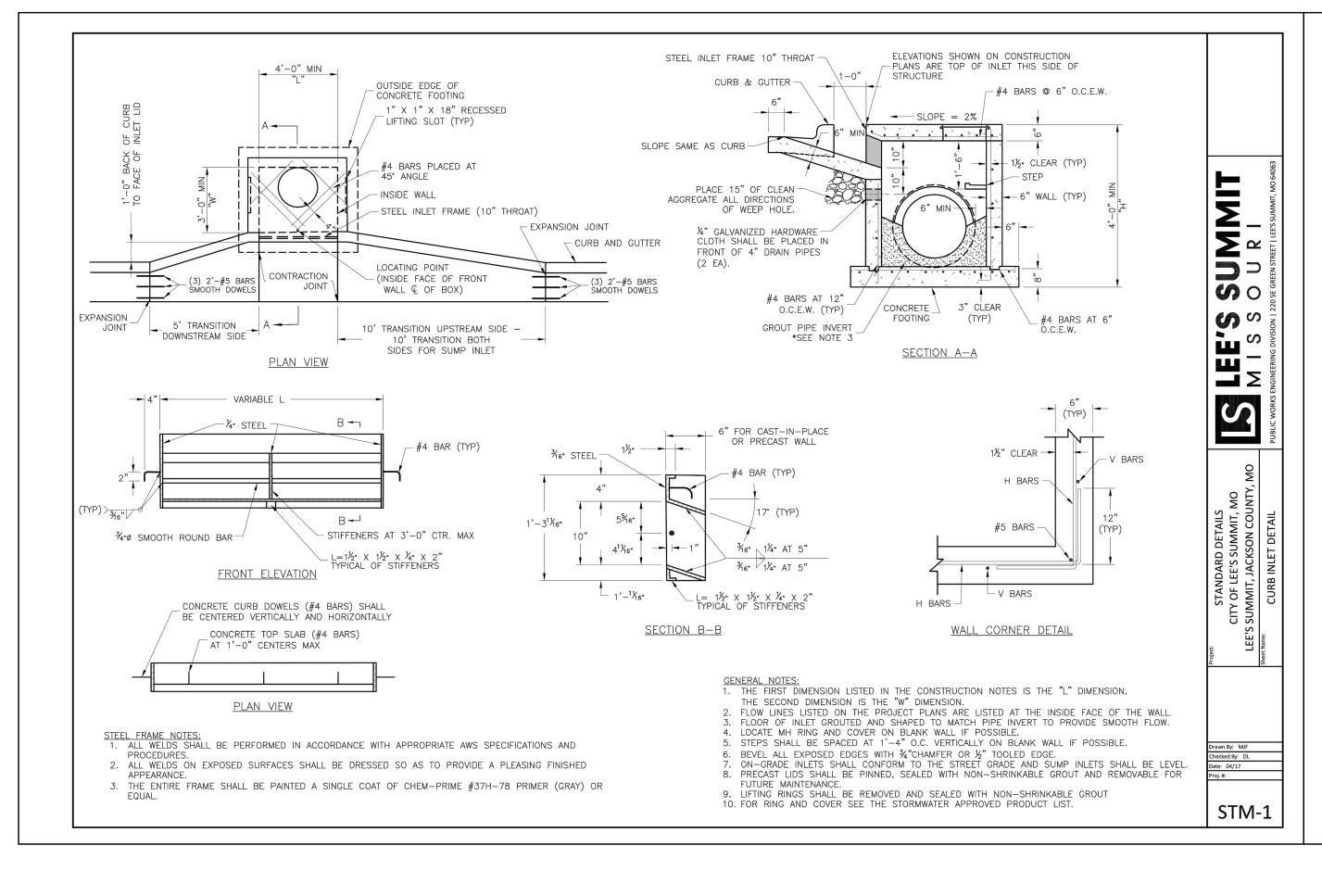
n - Civil Engineering

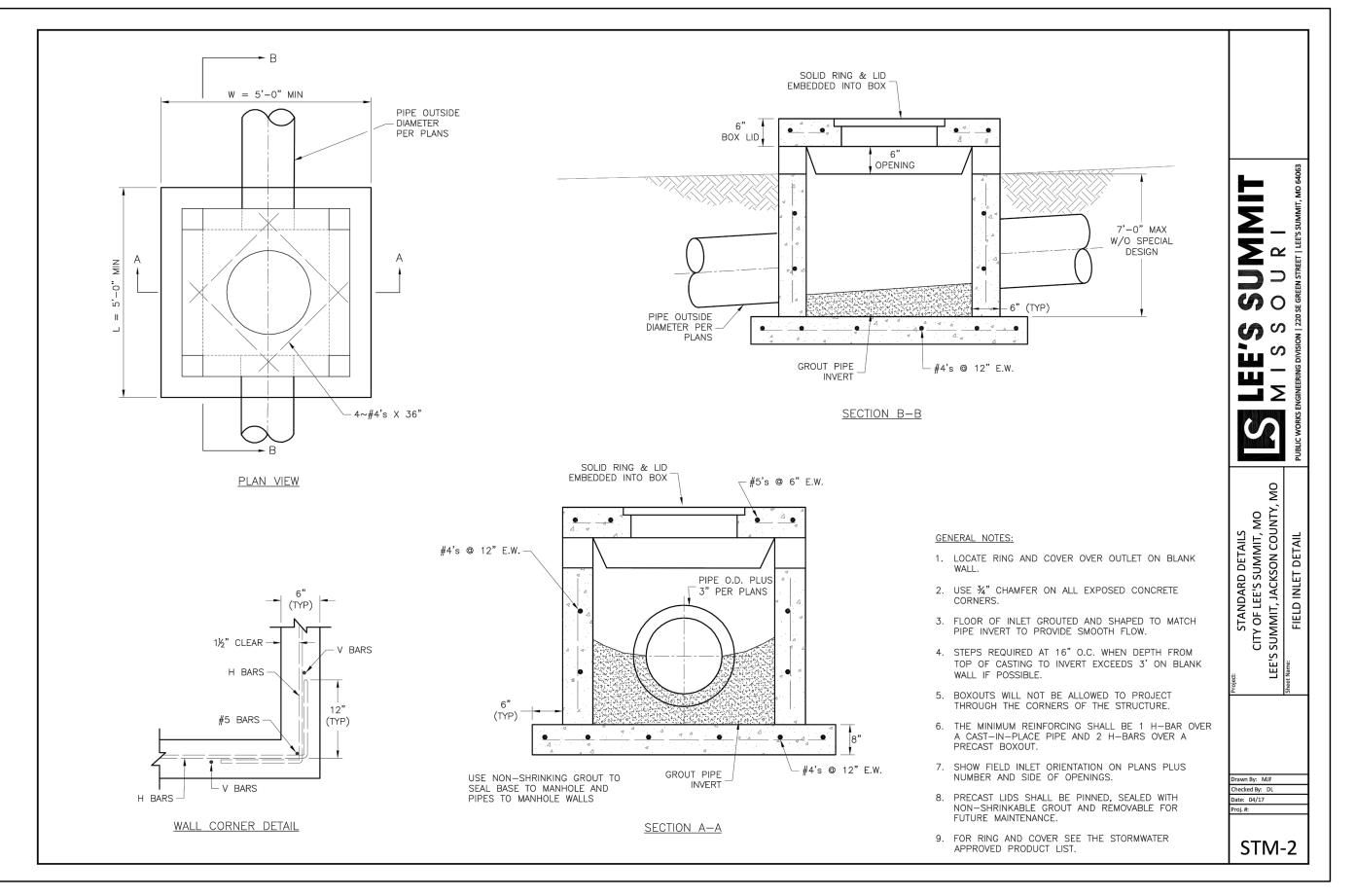


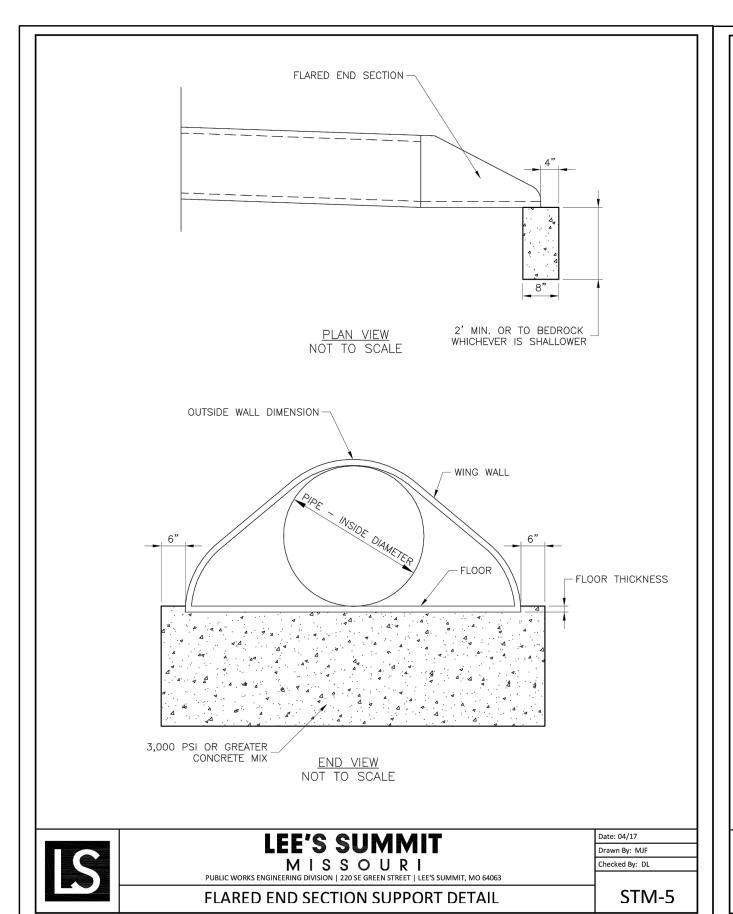
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DETAIL SHEET	REV. NO.	DATE	REVISIONS DESCRIPTION	BY	A CO
STREET & STORM SEWER PLANS	<b>~</b>	08/11/2021	REVISED PER CITY COMMENTS		A LA
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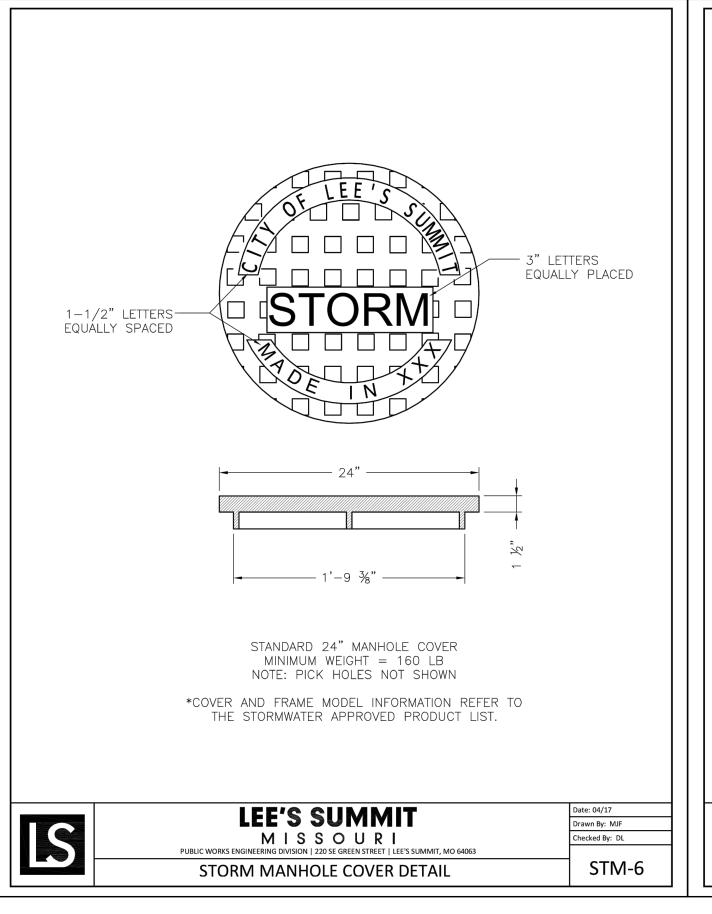
drawn by: MJD
checked by: JES
designed by: MJD
QA/QC by: JES
project no.: D19-2339
drawing no.: C\_DTL01\_D192339

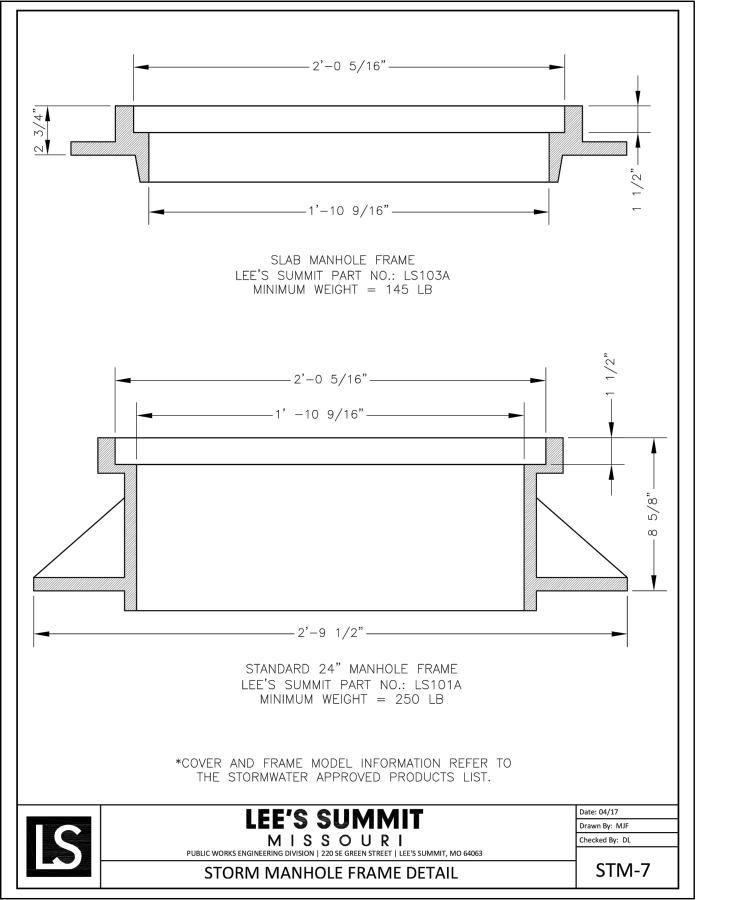
SHEET C120











APPROVED RECORD **DRAWING** These plans have been reviewed for accuracy by the Development Services Staff

NOT AS BUILT

DETAIL SHEET

8 STORM SEWER checked by: QA/QC by: project no.: D19-2339 drawing no.: C\_DTL01\_D192339 date: 3/14/2023 SHEET

C121

SELLERS

NUMBER

PE-2017000367/

SIGN MOUNTING DETAILS

\* The height to the bottom of a sign when it is located in a pedestrian walkway or

NOTE:
1. Generally, the sign mounting height should not be more than 1' greater than

extends into a walkway shall be a minimum of 80 inches above the walkway

TYPICAL MEDIAN SIGN LOCATION

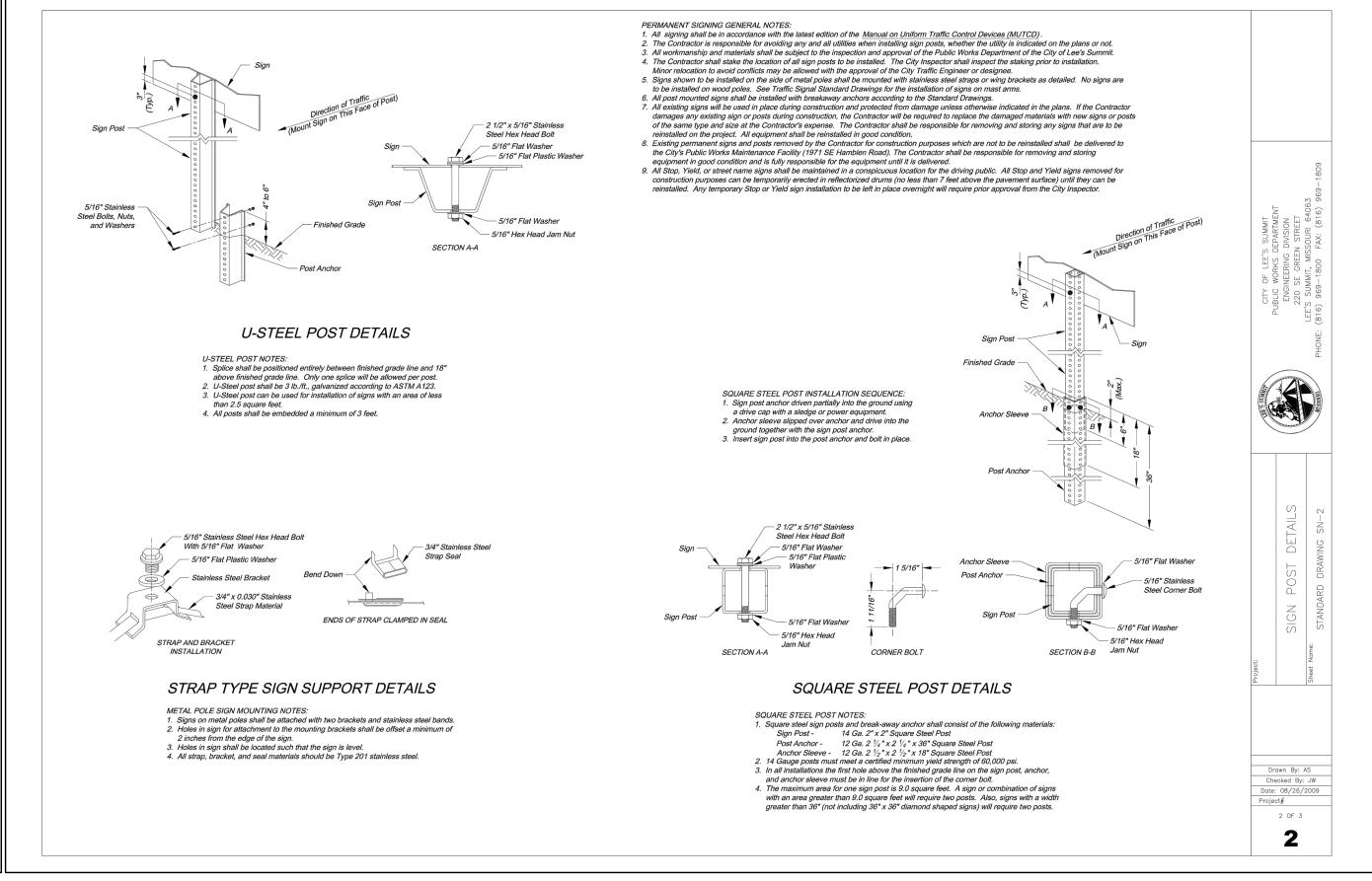
MEDIAN SIGN LOCATION

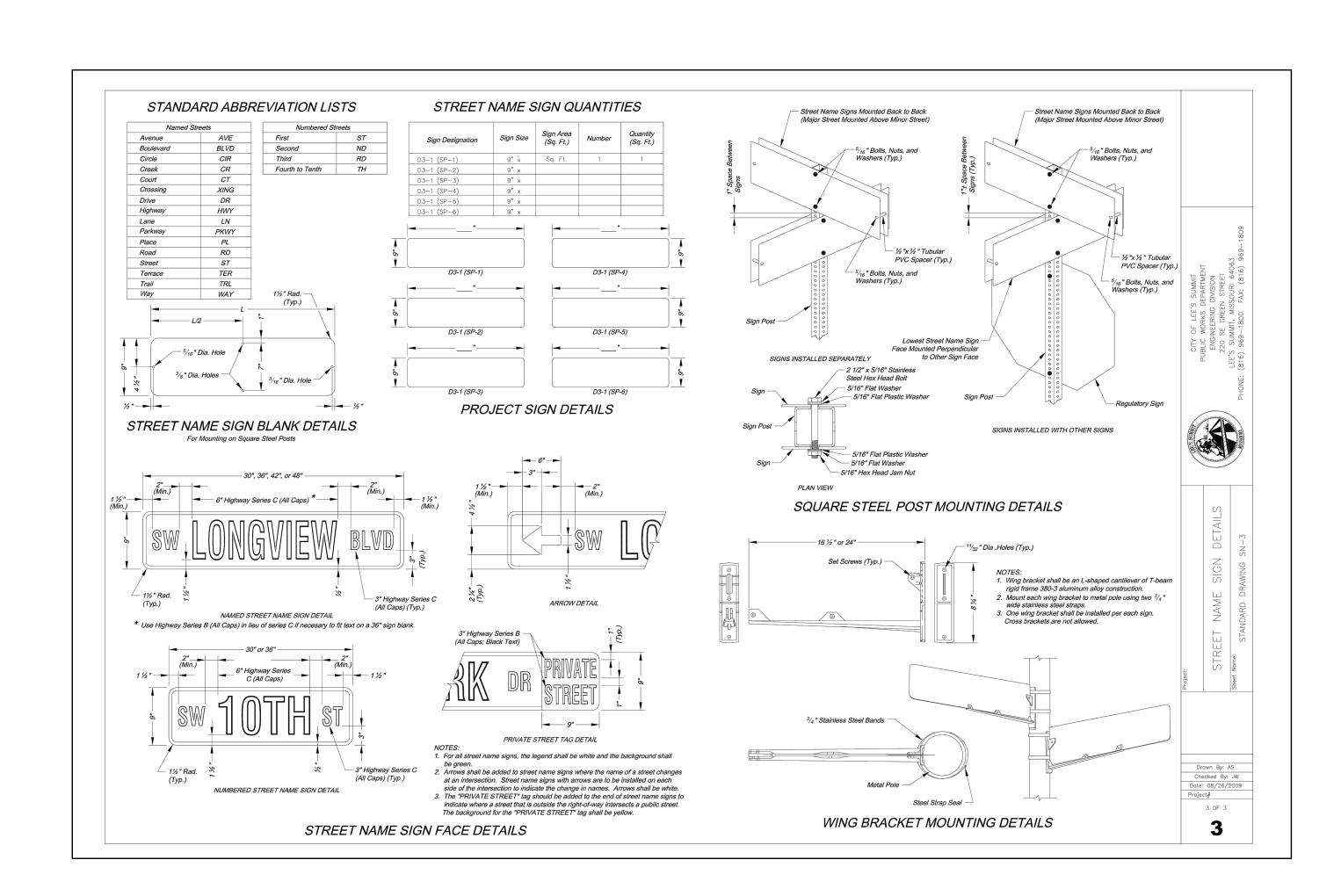
1. A 4" P.V.C. sleeve shall be installed in new concrete medians at each

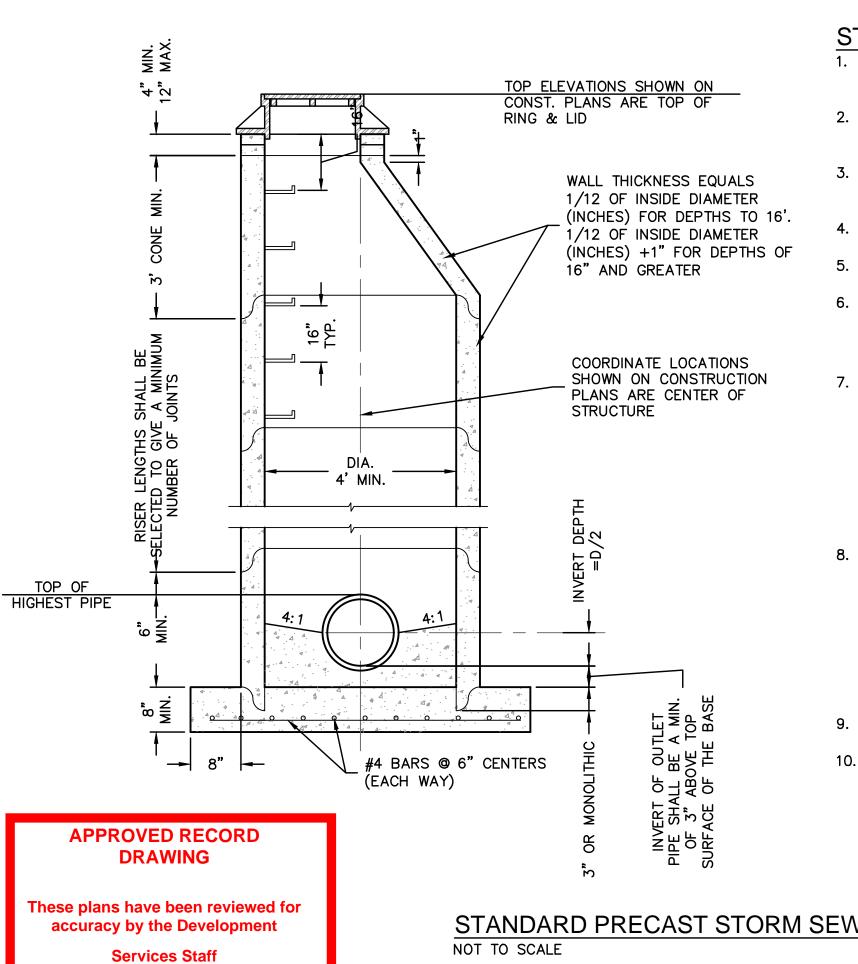
2. For existing concrete medians, a 4" hole shall be cored into the concrete.

location where a sign is to be installed.

Checked By: JW Date: 08/26/2009







## STORM MANHOLE NOTES

- ALL MANHOLES ARE TO BE PRECAST CONCRETE AND OF ECCENTRIC CONE TYPE UNLESS OTHERWISE SPECIFIED.
- MANHOLE TOP ADJUSTMENTS SHALL BE ACCOMPLISHED BY THE USE OF CONCRETE ADJUSTMENT RINGS.
- TOP OF MANHOLE CASTING SHALL BE SET FLUSH AND ON SAME SLOPE AS FINISHED SURFACE OR AS DIRECTED BY THE ENGINEER.
- REINFORCEMENT IN ALL SECTIONS SHALL EQUAL OR
- EXCEED A.S.T.M. C-478 SPECIFICATIONS. THE ENGINEER SHALL DESIGNATE MODIFICATIONS
- FOR MANHOLES WITH SPECIAL DESIGNS.
- THE INSIDE DIAMETER OF THE MANHOLE SHALL BE 4'-0" FOR PIPE DIAMETERS FROM 12" THRU 24", 5'-0" FOR PIPE DIAMETERS FROM 27" THRU 36",
- AND 6'-0" FOR PIPE DIAMETERS 42" THRU 48". CLEARANCE TOLERANCE OF PIPE OPENINGS: THE MAXIMUM ALLOWABLE PIPE OPENING ON A HORIZONTAL AXIS SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 12". THE MAXIMUM ALLOWABLE PIPE OPENING ON VERTICAL AXIS SHALL BE THE OUTSIDE DIAMETER PLUS 8". THE MAXIMUM CLEARANCE BETWEEN THE OUTSIDE SURFACE OF AN INSTALLED PIPE AND THE
- CONCRETE OF THE MANHOLE SHALL BE 2". INSTALLATION OF PIPE OPENINGS: ALL REQUIRED PIPE OPENINGS SHALL BE PLANT CAST IN MANHOLE UNITS. FIELD ALTERATIONS OF OPENINGS WILL BE PERMITTED PROVIDED WALLS ARE SCORED WITH A MASONRY SAW TO A DEPTH SUFFICIENT TO SEVER REINFOECING STEEL. A CHIPPING HAMMER MAY THEN BE USED TO REMOVE THE CONCRETE. MINIMUM DISTANCE BETWEEN ANY TWO ADJACENT
- NO DIRECT PAYMENT FOR SHAPING FLOOR OR CONNECTING PIPES AS SHOWN ON PLANS.

PIPES SHALL BE 2".

10. RING AND COVER TO BE NEENAH R-1736, CLAY & BAILEY #2008, DEETER # 1316, OR APPROVED EQUAL. (CASTING MAY VARY BY MUNICIPALITY, REFER TO PLANS & CONTRACT DOCUMENTS.)

STANDARD PRECAST STORM SEWER MANHOLE NOT TO SCALE

NOT AS BUILT

Ols Mis 13
JULIE ELAINE SELLERS NUMBER PE-2017000367 5/9/2023

REVISIONS DESCRIPTION	REVISED PER CITY COMMENTS						REVISIONS
REV. DATE	1 08/11/2021						
~ ~ ~						, 000	2021
DETAIL SHEET	STREET & STORM SEWER PLANS	OSAGE 3RD PLAT					LEE'S SUMMIT, MO
checke design QA/QC project drawin date:	ed by: by: no.:	 _DT	-LO	1_[	19- 019 /14/	233	JD S 39 39

SHEET