

DESIGN AND CONSTRUCTION MANUAL DESIGN MODIFICATION REQUEST

PREMISE ADDRESS: 1001 SE Bailey Road, Lee's Summit, MO, 64081 PERMIT NUMBER: PL2020134 OWNER'S NAME: SCHOOL DISTRICT 7 REORGANIZED TO: The City Engineer In accordance with the Lee's Summit Design and Construction Manual (DCM) Section 1002.A, I wish to apply for a modification to one or more specification (s). The following articulates my request for your review and action. (NOTE: Cite specific code sections and engineering justification and drawings.) See Attached SUBMITTED BY: NAME: Olsson (Terry M. Parsons) () OWNER (X) OWNER'S AGENT ADDRESS: 7301 W. 133rd Street, Ste 200 Tel.# 913-634-0903 CITY, STATE, ZIP: Overland Park, KS, 66213 Email: tparsons@olsson.com SIGNATURE: FORWARDING MANAGER. Kent D. Monter, PE RECOMMENDATION () APPROVAL () DENIAL DATE: 03 November 2020 SIGNATURE:_____ GEORGE BINGER III, P.E. – CITY ENGINEER: (X) APPROVED () DENIED SIGNATURE: DATE:_____ COMMENTS

A COPY MUST BE ATTACHED TO THE APPROVED PLANS

PROJECT NAME: LSR7 Middle School #4 – Bailey Road

Narrative for

DESIGN AND CONSTRUCTION MANUAL DESIGN

MODIFICATION REQUEST

In accordance with the Lee's Summit Design and Construction Manual (DCM) Section 1002.A, I wish to apply for a modification to one or more specification (s). The following articulates my request for your review and action. (NOTE: Cite specific code sections and engineering justification and drawings.)

5605.3 Stream Preservation and Buffer Zones – we are requesting a waiver/modification to this section to encroach on the prescribed stream buffer widths in specific areas as identified on the attached drawings. Our encroachment is needed due to the nature of the site being narrow in the east/west direction and meeting the program requirements for all of the components needed on site by the school district. The grade of the site falls from west to east towards the existing streamway. Again, with the narrow site it is difficult grade out the site for buildings, fields, and parking and not have the slope embankment encroach into the buffer.

According to the requirements of Section 5605.3, a total of 2.08 acres should be dedicated to the stream setback. To accommodate the improvements noted above a total of 0.74 acres will have to be removed from the 1.34 acres. Per Exhibit 1 in this submittal. An additional 0.76 acres will be added to the easement. This results in 2.10 acres of permanent stream setback dedication.

There will also be 2 permanent energy dissipation structures (riprap outfall protection and ditch checks) installed at Station 17+66 and Station 22+40 to prevent erosion from the outfalls of Detention Basins EDD-1 and EDD-2. The dissipation structures will be located within the permanent stream setback area.

To accommodate this request Olsson has prepared a design memorandum and Natural Channel Assessment according to Section 5605.4 of the LS DCM to demonstrate the minimal impact the development has on the stream setback.

The memorandum and exhibits are attached to this submittal.





October 9, 2020

City of Lee's Summit, MO 220 SE Green Street Lee's Summit, MO 64063

Re: LSR7 Middle School #4 Stream Buffer Variance - PL2020209

Stream Assessment Design Memorandum

This memorandum for the waiver request on LSR7 Middle School #4 to perform construction activities within the proposed stream buffer for the site.

To support the waiver request Olsson has performed a Stream Assessment to demonstrate that the proposed construction activities within the stream buffer will not adversely affect the natural condition of the existing stream. The stream assessment was performed in accordance with Section 5605 of the APWA KC Metro Design Criteria and Specifications. Information for the steam assessment was based on field survey data and field site visits by Olsson.

Section 5605.4a Plan-Form Analysis

The existing stream information and the plan-form analysis have been shown are the attached Stream Assessment Drawings. The Plan-Form Ratio lies with the typical range with the exception of meander length/full bank width.

This could be explained in the outfall from the enclosed storm system coming under Bailey Road. The north end of the stream where the outfall is located shows a lower sinuosity than the south end of the stream. There two concrete flumes or checks (Sta. 13+05, Sta. 23+80) constructed in the stream. The checks are immediately downstream of locations where natural swales enter the channel. Finally, a structure has been constructed in the stream at the south end of the property. The structure consists of riprap bank protection on each side of the stream and concrete channel walls. These structures would tend to anchor the stream in its current location.

Based on the plan-form analysis, general steam corridor limits have been shown on the Stream Assessment Plans. The stream corridor limits show that the proposed construction will not interfere with the natural meandering of the stream.

Section 5605.4b - Energy Management

In regard to the flow in the stream, an analysis was made of the pre and post construction peak runoff values for the site. The existing peak flow rates were compared to post construction flow rates for the areas that were flowing to the proposed detention basins on the site. Both undetained and detained peak flow rates were included. See Table 1 below.

TABLE 1	L - PRE AND POST		IIDDLE SCHOOL #4 EAK RUNOFF FOR	DETENTION AREAS	ON SITE
	Area	Stom Event	Existing Runoff	Proposed Peak Runoff (Undetained)	Proposed Peak Runof (Detained)
			cfs	cfs	cfs
		2-Yr	3.7	5.7	1.6
EDD-1	3.5	10-Yr	5.6	8.6	4.2
		25-Yr	6.2	9.5	6.3
		100-Yr	8.1	12.5	9.9
EDD-2		2-Yr	7.6	10.9	3.7
	8.2	10-Yr	17.9	25.5	14.0
		25-Yr	19.8	28.4	17.1
		100-Yr	26.3	37.5	24.1
		2-Yr	26.9	38.4	11.1
EDD-3	22.6	10-Yr	63.5	90.7	35.9
		25-Yr	70.7	101.0	44.5
		100-Yr	93.7	133.8	63.0

An analysis was then performed on the pre and post construction flows in the stream itself. The area north of Bailey Road is a fully developed residential area. The proposed middle school development will affect the flow quantities because of the detention basins on site. The net change in these flow rates were used to calculate the post construction flow rates for the stream. As for the area to east of the stream it is assumed that detention will be implemented per city standards when that property is developed.

To evaluate the hydraulic grade line (HGL) and energy grade line (EGL) the existing and proposed flows were evaluated at the six cross sections identified in the Plan Form Analysis. This information was used to calculate the HGL's and EGL's for the indicated storm at each cross section. The analysis indicates minimal variations in the HGL's and EGL's with a maximum 3-inch rise in the 10-Year Storm HGL at Section 22+40. As the stream exits the south property line HGL's are lower for the entire range of storm events.

LEE'S SUMMIT MIDDLE SCHOOL #4 TABLE 2 - PRE AND POST CONSTRUCTION HYDRAULIC AND ENERGY GRADE LINES IN STREAM

															(1)		
			2-Y	ear			10-	Year			25-1	/ear			100-	Year	
Sta	Cond	Q	HGL	V	EGL	Q	HGL	V	EGL	Q	HGL	V	EGL	Q	HGL	V	EGL
Sta	Cond	cfs	ft	fps	ft	cfs	ft	fps	ft	cfs	ft	fps	ft	cfs	ft	fps	ft
	Pre	129.06	1010.14	3.01	1010.28	183.68	1010.42	3.03	1010.56	215.58	1010.53	3.12	1010.68	332.59	1010.83	3.46	1011.02
26+75	Post	129.06	1010.14	3.01	1010.28	183.68	1010.42	3.03	1010.56	215.58	1010.53	3.12	1010.68	332.59	1010.83	3.46	1011.02
	Delta (ft)		0.00		0.00	The state of	0.00		0.00		0.00		0.00	1111	0.00		0.00
	Pre	146.47	1002.50	3.99	1002.75	210.34	1002.86	4.36	1002.86	247.63	1003.04	4.54	1003.36	382.66	1003.58	5.05	1003.98
22+40	Post	144.37	1002.49	3.95	1002.73	208.94	1002.85	4.35	1003.14	247.83	1003.05	4.52	1003.37	384.46	1003.59	5.06	1003.99
	Delta (ft)		-0.01		-0.02		-0.01		0.28		0.01		0.01		0.01		0.01
	Pre	186.16	998.15	5.40	998.60	267.90	998.99	5.24	999.42	315.62	999.42	4.99	999.81	487.92	1000.20	5.04	1000.59
17+66	Post	190.06	998.24	5.33	998.68	271.80	999.02	5.26	998.68	312.92	999.39	5.01	999.78	485.72	1000.19	5.04	1000.58
	Delta		0.09		0.08		0.03		-0.74		-0.03		-0.03		-0.01		-0.01
	Pre	198.81	988.24	4.15	988.51	288.08	988.63	4.55	988.95	340.20	988.83	4.73	989.18	526.59	989.40	5.29	989.83
9+10	Post	202.71	988.27	4.14	988.54	291.98	988.65	4.55	988.97	337.50	988.81	4.73	989.16	524.39	989.38	5.29	989.81
	Delta (ft)		0.03		0.03		0.02		0.02		-0.02		-0.02		-0.02		-0.02
	Pre	216.20	982.43	4.82	982.78	314.53	982.92	5.31	983.36	371.95	983.17	5.52	983.65	576.15	983.91	6.16	984.50
3+35	Post	200.40	982.32	4.74	982.67	298.73	982.85	5.23	983.27	345.75	982.87	5.23	983.27	545.45	983.81	6.08	984.38
	Delta (ft)		-0.11		-0.11		-0.07		-0.09		-0.30		-0.38		-0.10		-0.12
	Pre	276.54	982.18	5.27	982.61	403.76	982.79	5.81	983.31	478.08	983.08	6.07	983.65	741.06	983.95	6.77	984.66
1+61	Post	260.74	982.09	5.22	982.51	387.96	982.71	5.75	983.22	451.88	982.98	5.97	983.53	710.36	983.86	6.70	984.57
	Delta (ft)		-0.09		-0.10		-0.08		-0.09		-0.10		-0.12		-0.09		-0.09

Section 5605.4c – Sediment Transport Continuity

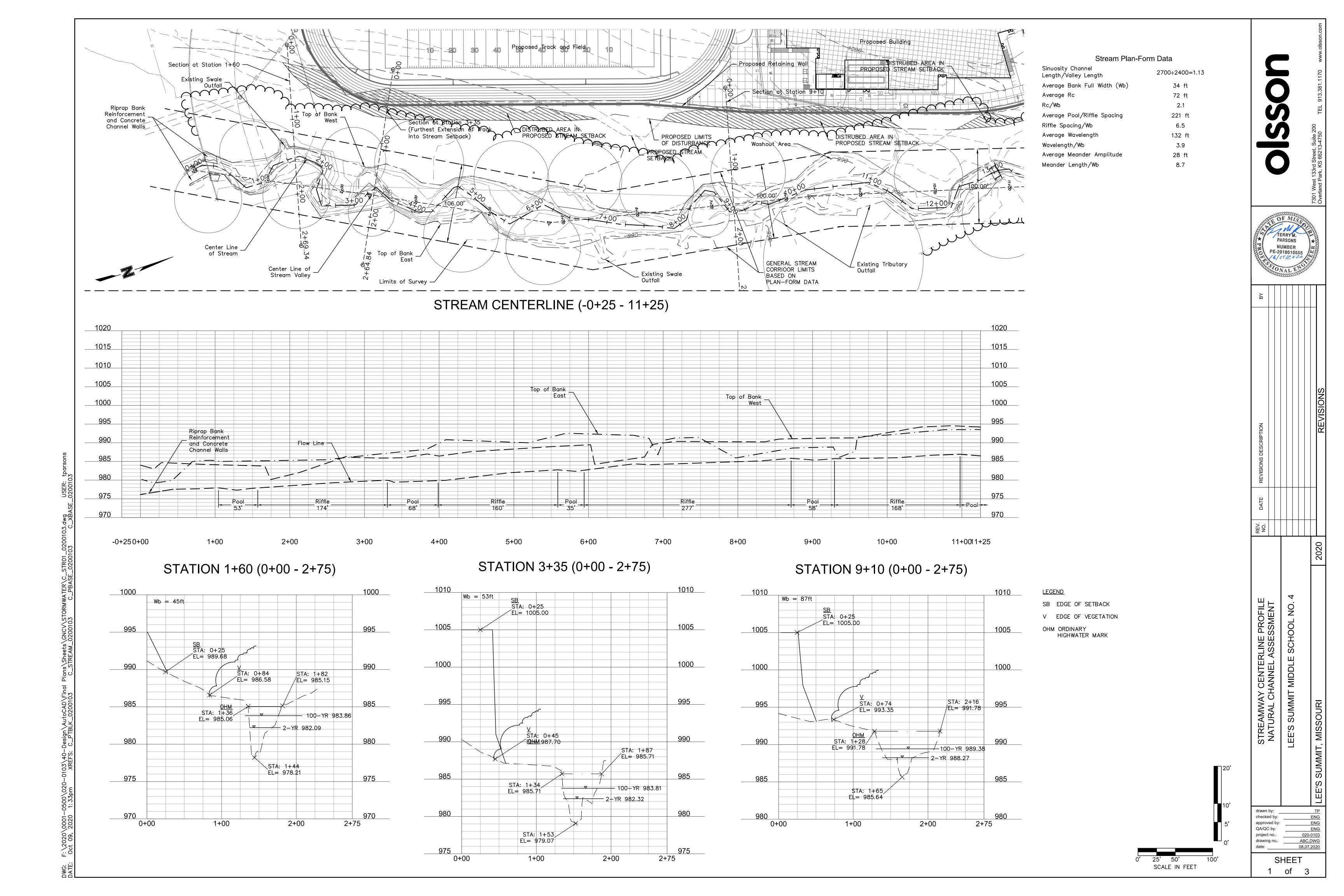
An analysis was performed to calculate the applied shear stress on the channel surface of the stream, based on the pre and post construction peak flows calculated previously. The results are shown in Table3.

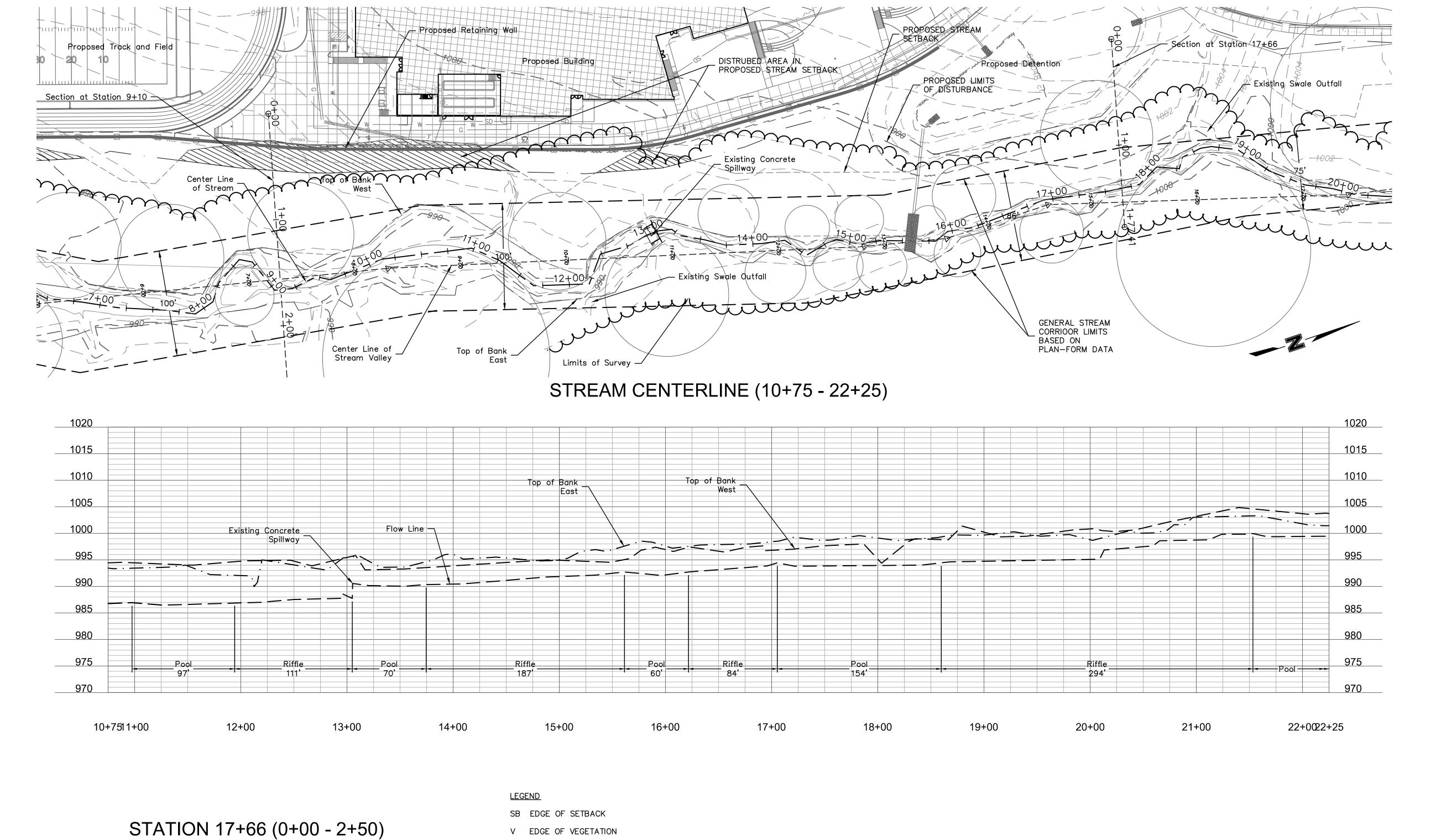
As with the HGL analysis, the variance in applied shear stress is minimal. Pre construction and post construction shear stress values were compared. The percentage increase or decrease is shown in Table 4.

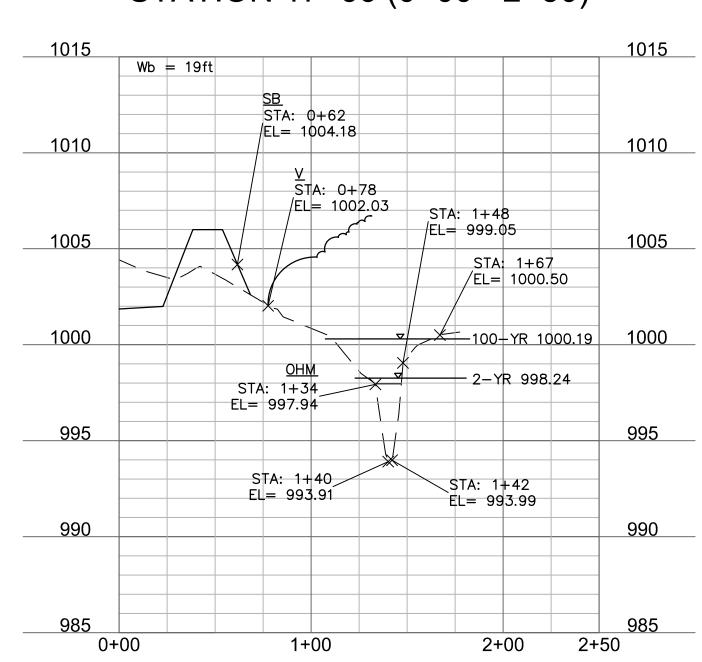
The post construction applied shear stress was also compared to critical shear stress for the bed material. This ratio is shown in Table 5. It indicates the bed material is stable, with the exception of Station 22+40 in the 100-year storm.

LEE'S SUMMIT MIDDLE SCHOOL #4 PRE AND POST CONSTRUCTION SHEAR STRESS ANALYSIS IN STREAM (SEDIMENT TRANSPORT) **TABLE 3 - AVERAGE APPLIED SHEAR STRESS** 10-Year 100-Year Storm Event 25-Year Q R To Q R S To Q R To Sta Cond ft/ft ft2/ft ft/ft ft2/ft psf cfs psf ft2/ft ft/ft cfs cfs psf Pre 183.68 0.82 0.007 0.36 215.58 0.84 0.007 332.59 0.99 0.007 0.37 0.43 26+75 Post 183.68 0.82 0.007 0.36 215.58 0.84 0.007 0.37 332.59 0.99 0.007 0.43 Pre 210.34 1.39 0.007 0.61 247.63 1.48 0.007 0.65 382.66 1.74 0.007 0.76 22+40 208.94 247.83 1.48 0.007 384.46 1.75 Post 1.38 0.60 0.76 Pre 267.90 1.84 0.005 0.57 315.62 1.71 0.005 0.53 487.92 1.73 17+66 Post 271.80 1.84 0.005 312.92 1.71 0.005 485.72 1.73 0.006 9+10 314.53 1.86 0.005 0.58 371.95 1.99 0.005 0.62 576.15 2.34 0.005 0.73 3+35 Post 298.73 1.83 0.005 0.57 345.75 1.94 0.005 0.60 545.45 2.29 0.005 0.72 403.76 0.004 0.53 2.28 0.004 0.57 741.06 2.68 0.004 Pre 2.14 478.08 0.67 1+61 Post 387.96 2.10 0.004 0.53 451.88 2.23 0.004 0.56 710.36 0.004 0.66 TABLE 4 - PRE VS POST APPLIED STRESS TABLE 5 - RATIO To vs Tc (POST CONSTRUCTUION) **Channel Material** 10-Yr 25-Yr 100-Yr Sta Tc 10-Yr 100-Yr 0.0% 0.0% 0.0% 26+75 Course Gravel 0.670 0.53 0.55 0.65 26+75 Course Gravel 22+40 -0.4% 0.3% 0.3% 22+40 0.670 0.90 0.97 1.14 17+66 0.2% 0.1% -0.1% 17+66 Course Gravel 0.670 0.86 0.80 0.81 -0.3% Silts to Cobbles 0.800 9+10 -5.3% -0.3% 9+10 0.70 0.73 0.87 3+35 -1.8% -2.7% -2.1% 3+35 Silts to Cobbles 0.800 0.71 0.75 0.89 1+61 -1.6% -2.1% -1.6% 1+61 Silts to Cobbles 0.800 0.66 0.70 0.82

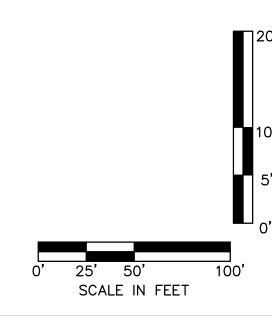
In conclusion, the purpose of the stream setback is to ensure there is sufficient space for the stream to meander in a natural manner. The Plan-Form Analysis demonstrates that the proposed construction will not interfere with the natural movement of the stream channel. The HGL and shear stress analysis also demonstrate minimal impact. In addition, the proposed construction will also not cause any channel constrictions within the flow level of the 100-year storm.

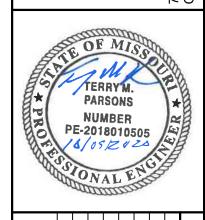






OHM ORDINARY HIGHWATER MARK



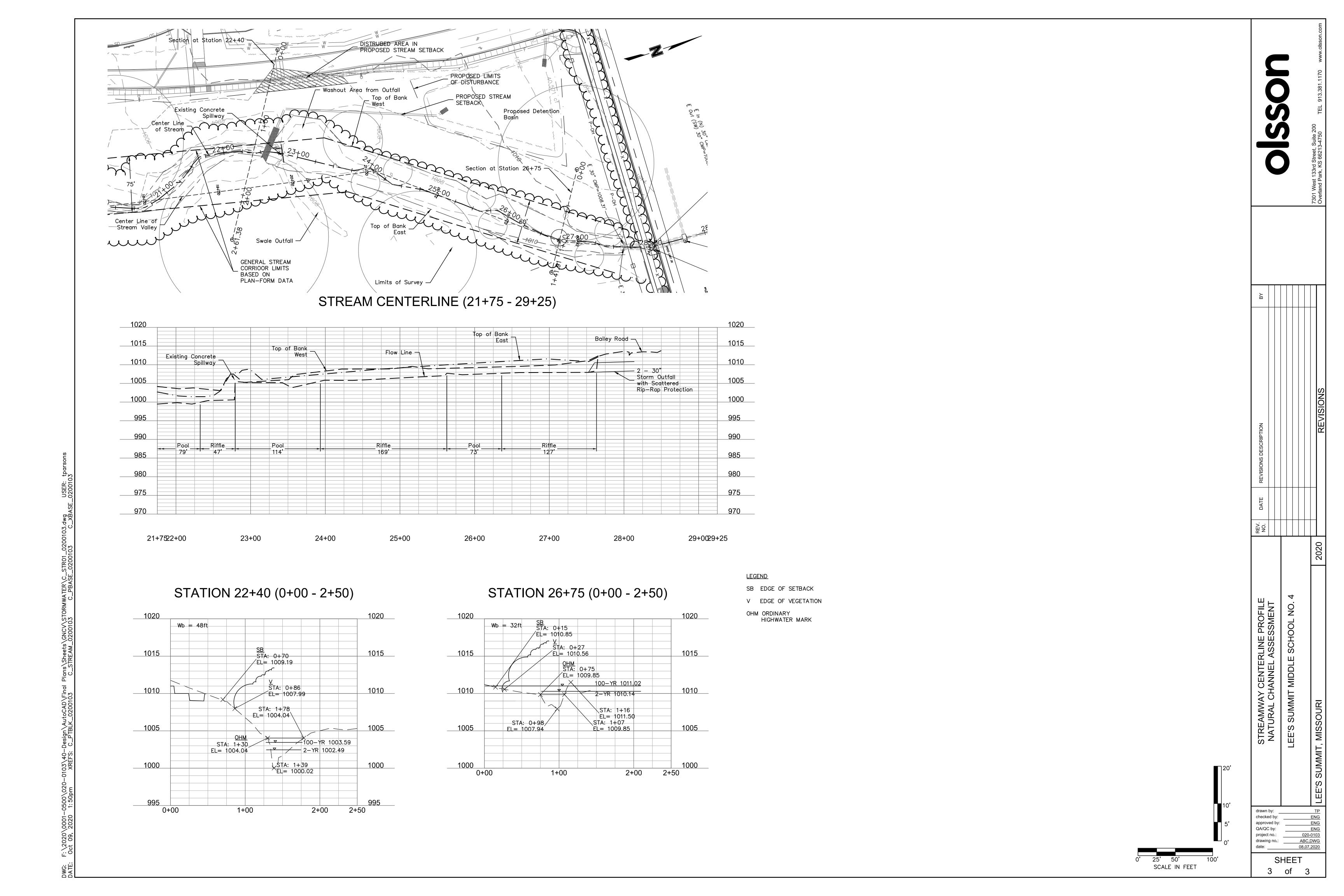


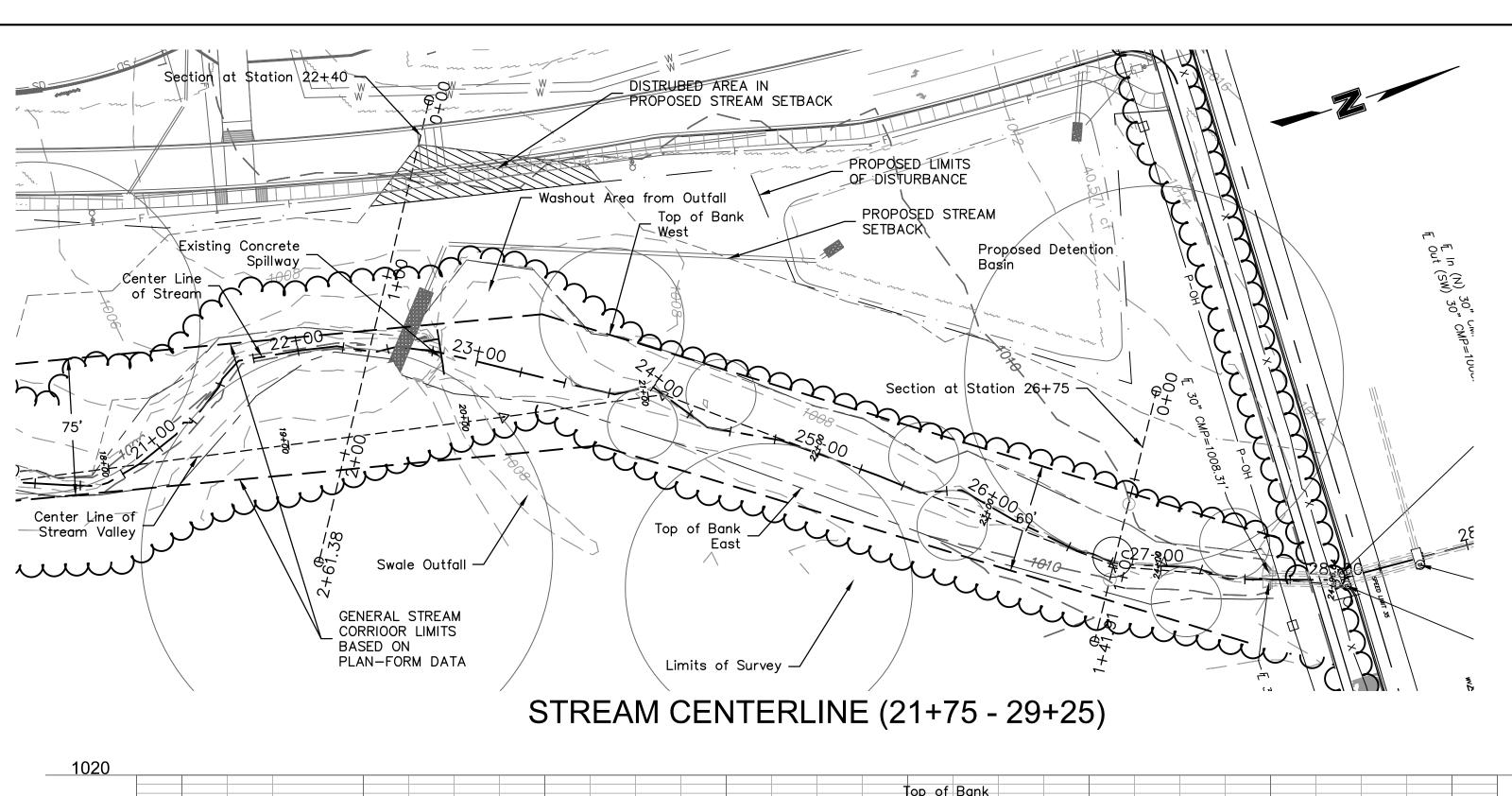
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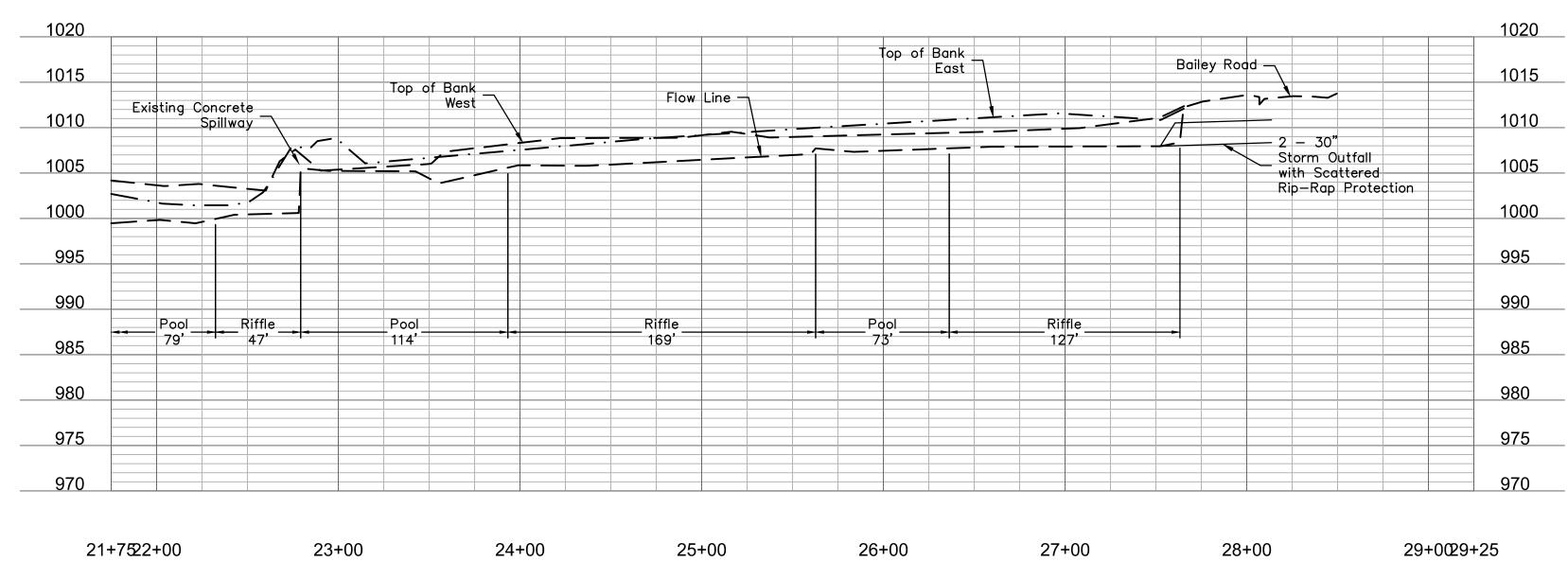
LEE'S SUMMIT MIDDLE SCHO
STREAMWAY CENTERLINE NATURAL CHANNEL ASSES

checked by: ENG
approved by: ENG
QA/QC by: ENG
project no.: 020-0103
drawing no.: ABC.DWG
date: 08.07.2020

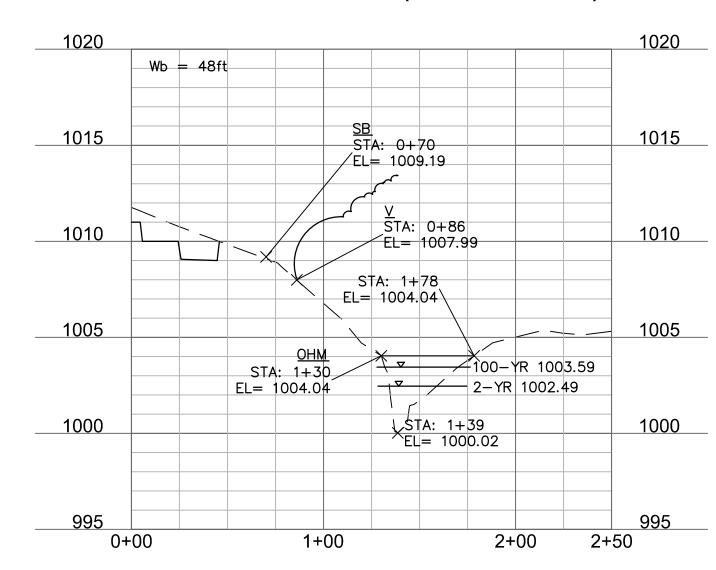
SHEET 2 of 3



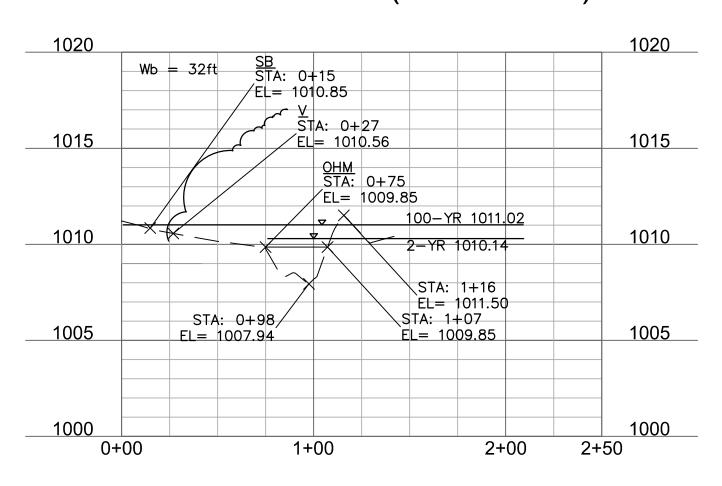








STATION 26+75 (0+00 - 2+50)

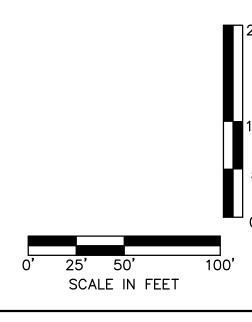


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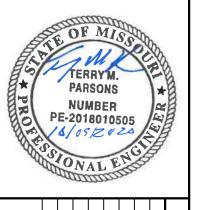
SB EDGE OF SETBACK

V EDGE OF VEGETATION

OHM ORDINARY HIGHWATER MARK



SSS1 West 133rd Street Suite 200



STREAMWAY CENTERLINE PROFILE	REV. NO.	DATE	REVISIONS DESCRIPTION BY	
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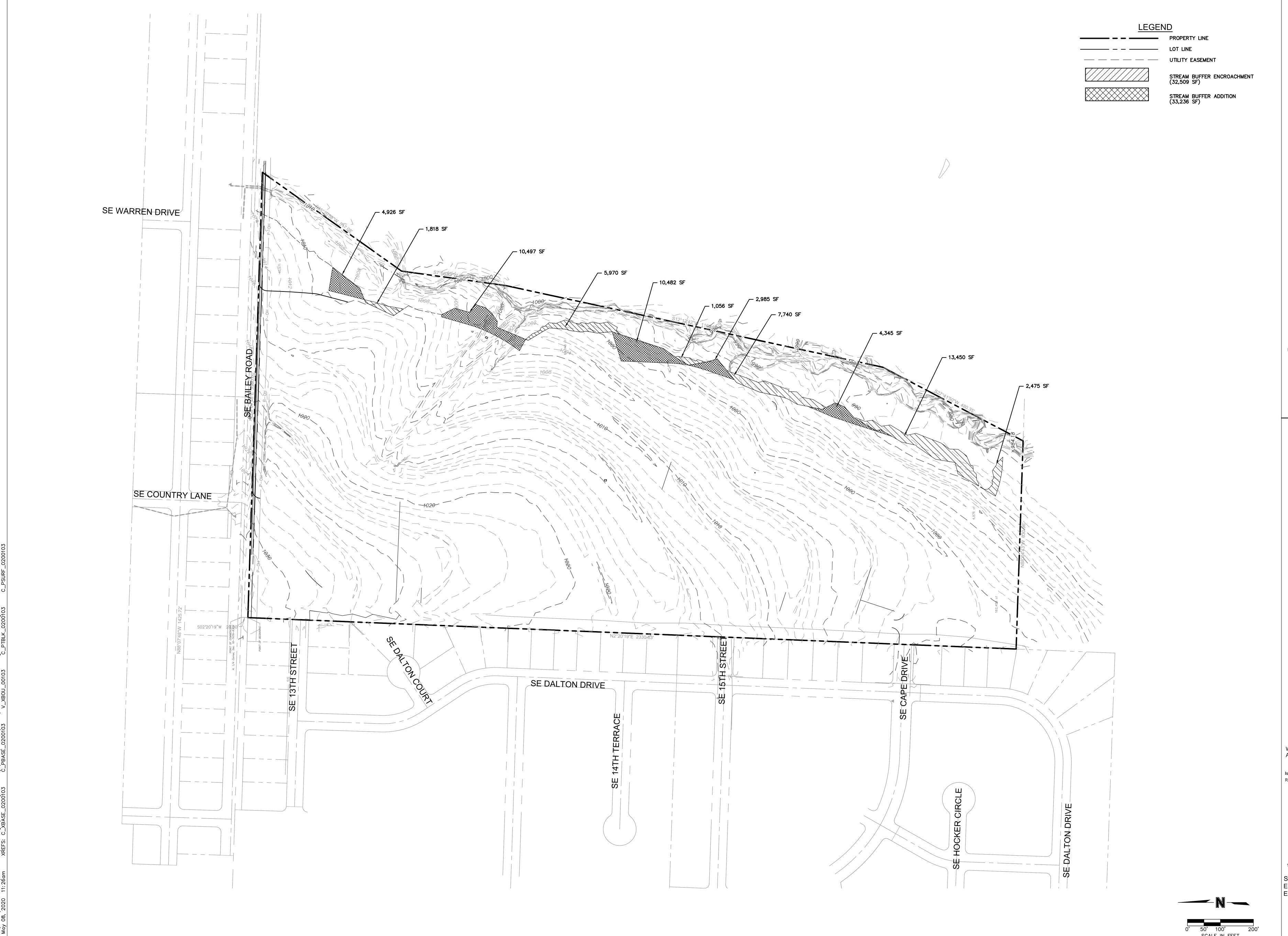
SHEET 3 of 3

LEE'S SUMMIT MIDDLE SCHOOL #4 TABLE 1 - PRE AND POST CONSTRUCTION PEAK RUNOFF FOR DETENTION AREAS ON SITE

	Area	Stom Event	Existing Runoff	Proposed Peak Runoff (Undetained)	Proposed Peak Runoff (Detained)
			cfs	cfs	cfs
		2-Yr	3.7	5.7	1.6
EDD-1	3.5	10-Yr	5.6	8.6	4.2
EDD-1	3.5	25-Yr	6.2	9.5	6.3
		100-Yr	8.1	12.5	9.9
EDD-2	8.2	2-Yr	7.6	10.9	3.7
		10-Yr	17.9	25.5	14.0
		25-Yr	19.8	28.4	17.1
		100-Yr	26.3	37.5	24.1
		2-Yr	26.9	38.4	11.1
EDD-3	22.6	10-Yr	63.5	90.7	35.9
	22.0	25-Yr	70.7	101.0	44.5
		100-Yr	93.7	133.8	63.0

LEE'S SUMMIT MIDDLE SCHOOL #4 TABLE 2 - PRE AND POST CONSTRUCTION HYDRAULIC AND ENERGY GRADE LINES IN STREAM

			2-Y	ear			10-	Y ear			25-\	'ear			100-	Year	
Sta	Cond	Q	HGL	V	EGL	Q	HGL	V	EGL	Q	HGL	V	EGL	Q	HGL	V	EGL
Sta	Cond	cfs	ft	fps	ft	cfs	ft	fps	ft	cfs	ft	fps	ft	cfs	ft	fps	ft
	Pre	129.06	1010.14	3.01	1010.28	183.68	1010.42	3.03	1010.56	215.58	1010.53	3.12	1010.68	332.59	1010.83	3.46	1011.02
26+75	Post	129.06	1010.14	3.01	1010.28	183.68	1010.42	3.03	1010.56	215.58	1010.53	3.12	1010.68	332.59	1010.83	3.46	1011.02
	Delta (ft)		0.00		0.00		0.00		0.00		0.00		0.00		0.00		0.00
	Pre	146.47	1002.50	3.99	1002.75	210.34	1002.86	4.36	1002.86	247.63	1003.04	4.54	1003.36	382.66	1003.58	5.05	1003.98
22+40	Post	144.37	1002.49	3.95	1002.73	208.94	1002.85	4.35	1003.14	247.83	1003.05	4.52	1003.37	384.46	1003.59	5.06	1003.99
	Delta (ft)		-0.01		-0.02		-0.01		0.28		0.01		0.01		0.01		0.01
	Pre	186.16	998.15	5.40	998.60	267.90	998.99	5.24	999.42	315.62	999.42	4.99	999.81	487.92	1000.20	5.04	1000.59
17+66	Post	190.06	998.24	5.33	998.68	271.80	999.02	5.26	998.68	312.92	999.39	5.01	999.78	485.72	1000.19	5.04	1000.58
	Delta		0.09		0.08		0.03		-0.74		-0.03		-0.03		-0.01		-0.01
	Pre	198.81	988.24	4.15	988.51	288.08	988.63	4.55	988.95	340.20	988.83	4.73	989.18	526.59	989.40	5.29	989.83
9+10	Post	202.71	988.27	4.14	988.54	291.98	988.65	4.55	988.97	337.50	988.81	4.73	989.16	524.39	989.38	5.29	989.81
	Delta (ft)		0.03		0.03		0.02		0.02		-0.02		-0.02		-0.02		-0.02
	Pre	216.20	982.43	4.82	982.78	314.53	982.92	5.31	983.36	371.95	983.17	5.52	983.65	576.15	983.91	6.16	984.50
3+35	Post	200.40	982.32	4.74	982.67	298.73	982.85	5.23	983.27	345.75	982.87	5.23	983.27	545.45	983.81	6.08	984.38
	Delta (ft)		-0.11		-0.11		-0.07		-0.09		-0.30		-0.38		-0.10		-0.12
	Pre	276.54	982.18	5.27	982.61	403.76	982.79	5.81	983.31	478.08	983.08	6.07	983.65	741.06	983.95	6.77	984.66
1+61	Post	260.74	982.09	5.22	982.51	387.96	982.71	5.75	983.22	451.88	982.98	5.97	983.53	710.36	983.86	6.70	984.57
	Delta (ft)		-0.09		-0.10		-0.08		-0.09		-0.10		-0.12		-0.09		-0.09



DLR Group

reet, Suite 200 36213-4750 TEL 913.381.1170 www.olss

LEE'S SUMMIT MIDDLE SCHOOL #4

WAIVER
APPLICATION

Issue Date 05.08.20

Issue Date 05.08.20

20102-00 REAM BUFFE

STREAM BUFFER ENCROACHMENT EXHIBIT

EX-1

LEE'S SUMMIT MIDDLE SCHOOL #4 PRE AND POST CONSTRUCTION SHEAR STRESS ANALYSIS IN STREAM (SEDIMENT TRANSPORT)

					TABLE 3 -	AVERAGE A	PPLIED SHE	AR STRESS					
Storm	Event		10-	Year			25-	Year			100-	-Year	
Sta	Cond	Q	R	S	То	Q	R	S	То	Q	R	S	То
Sta	Cond	cfs	ft2/ft	ft/ft	psf	cfs	ft2/ft	ft/ft	psf	cfs	ft2/ft	ft/ft	psf
26+75	Pre	183.68	0.82	0.007	0.36	215.58	0.84	0.007	0.37	332.59	0.99	0.007	0.43
20+73	Post	183.68	0.82	0.007	0.36	215.58	0.84	0.007	0.37	332.59	0.99	0.007	0.43
22+40	Pre	210.34	1.39	0.007	0.61	247.63	1.48	0.007	0.65	382.66	1.74	0.007	0.76
22+40	Post	208.94	1.38	0.007	0.60	247.83	1.48	0.007	0.65	384.46	1.75	0.007	0.76
17+66	Pre	267.90	1.84	0.005	0.57	315.62	1.71	0.005	0.53	487.92	1.73	0.005	0.54
17+00	Post	271.80	1.84	0.005	0.57	312.92	1.71	0.005	0.53	485.72	1.73	0.005	0.54
9+10	Pre	288.08	1.57	0.006	0.59	340.20	1.57	0.006	0.59	526.59	1.86	0.006	0.70
9+10	Post	291.98	1.49	0.006	0.56	337.50	1.57	0.006	0.59	524.39	1.85	0.006	0.69
3+35	Pre	314.53	1.86	0.005	0.58	371.95	1.99	0.005	0.62	576.15	2.34	0.005	0.73
3+33	Post	298.73	1.83	0.005	0.57	345.75	1.94	0.005	0.60	545.45	2.29	0.005	0.72
1+61	Pre	403.76	2.14	0.004	0.53	478.08	2.28	0.004	0.57	741.06	2.68	0.004	0.67
1+01	Post	387.96	2.10	0.004	0.53	451.88	2.23	0.004	0.56	710.36	2.64	0.004	0.66

TABLE 4	- PRE VS PC	OST APPLIE	STRESS
Sta	10-Yr	25-Yr	100-Yr
26+75	0.0%	0.0%	0.0%
22+40	-0.4%	0.3%	0.3%
17+66	0.2%	0.1%	-0.1%
9+10	-5.3%	-0.3%	-0.3%
3+35	-1.8%	-2.7%	-2.1%
1+61	-1.6%	-2.1%	-1.6%

	TABLE 5 - RATIO To	vs Tc (POS	T CONSTRU	CTUION)	
Sta	Channel Material	Tc	10-Yr	25-Yr	100-Yr
26+75	Course Gravel	0.670	0.53	0.55	0.65
22+40	Course Gravel	0.670	0.90	0.97	1.14
17+66	Course Gravel	0.670	0.86	0.80	0.81
9+10	Silts to Cobbles	0.800	0.70	0.73	0.87
3+35	Silts to Cobbles	0.800	0.71	0.75	0.89
1+61	Silts to Cobbles	0.800	0.66	0.70	0.82