



LEE'S SUMMIT MISSOURI

DESIGN AND CONSTRUCTION MANUAL DESIGN MODIFICATION REQUEST

PROJECT NAME: Discovery Park Phase 1

PREMISE ADDRESS: NW Lee's Summit Rd & NE Douglas St.

PERMIT NUMBER: _____

OWNER'S NAME: Discovery Park Lee's Summit, LLC Brian Maenner

TO: The City Engineer

In accordance with the Lee's Summit Design and Construction Manual (DCM) Section 1002.A, I wish to apply for a modification to one or more specification (s). The following articulates my request for your review and action. (NOTE: Cite specific code sections and engineering justification and drawings.)

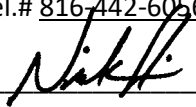
A waiver is being requested for the comprehensive control requirements provided in APWA Section 5608.C.1.a for the "Discovery Park Macro & Phase 1 Micro Stormwater Report". Allowable Release Rates are exceeded in the 2-year storm event at Points of Interest A1, A2, A3, A4, and A6. However, all peak flowrates remain below the calculated existing conditions peak rates at the same Points of Interest with the exception of Point A6.

Point A6 is at the public storm sewer downstream of the Discovery Park Phase 2 development, which crosses Colbern Road and is a minor internal point of interest within the overall stormwater study. The Discovery Park Phase 2 development will be collected in underground storm sewer systems and conveyed into a proposed permanent pool detention basin within the Discovery Park Phase 1 development. All storm sewers, including the Colbern Road crossing, have been adequately sized to convey the 100-year storm event.

An existing low-lying depression within the creek channel (located at Point A5 within the study) provides a small amount of peak flow and channel timing attenuation for approximately 96 acres east of Douglas Street, upstream of the development. This depression is proposed to be removed to allow an amenity space and permanent pool pond to be constructed, and flowrates from upstream offsite areas will bypass the development to a point downstream of the new amenity space. While reviewing various models of the site, it was determined that the existing depression and breached spillway that is being removed were providing enough attenuation of peak flows from upstream, offsite areas A7-A10, offsetting the attenuation via the proposed detention facility is not feasible. As shown in the tables within previous sections of this study, the proposed basin reduces the 2-yr storm event flowrates by 98% for the areas tributary to the basin. To detain the flows to below the calculated ARR, detention would need to be constructed downstream of the site, within the existing creek channel. However, this would require substantial improvements within the existing channel, further impacting the natural creek system. As demonstrated in the attached excerpt from the stormwater study, removal of this depression and construction of the proposed pond will result in all discharges downstream to have peak flowrates that are lower than undeveloped peak flowrates, so no adverse impacts to the downstream system are

anticipated. Further, the proposed development has been reviewed and coordinated with Unity Village, the owner and operator of Unity Lake #2, an existing lake located approximately ½ mile downstream of the proposed Discovery Park Phase 1 development. As indicated in the attached letter from Unity Village, there are no concerns with the proposed development in relation to the existing Unity Lake #2 and this project has their full support.

SUBMITTED BY:

NAME: Nicholas D. Heiser _____ () OWNER (X) OWNER'S AGENT
ADDRESS: 1301 Burlington St. Suite 100 _____ Tel.# 816-442-6056
CITY, STATE, ZIP North Kansas City, MO 64116
Email: nheiser@olsson.com _____ SIGNATURE: 

FORWARDING MANAGER: _____ RECOMMENDATION () APPROVAL () DENIAL

SIGNATURE: _____ DATE: _____

GEORGE BINGER III, P.E. – CITY ENGINEER: () APPROVED () DENIED

SIGNATURE: _____ DATE: _____

COMMENTS _____

A COPY MUST BE ATTACHED TO THE APPROVED PLANS

Letter of Support

January 17, 2023

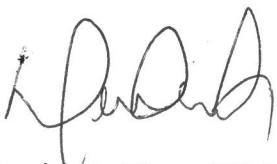
Re: Discovery Park Zones 1 & 2 – Proposed Stormwater Best Management Practices (BMPs)

To whom it may concern,

We have reviewed the proposed stormwater management plan proposed for the Discovery Park Zones 1 & 2 development. Water quality treatment being provided by the proposed development appears to help mitigate any impacts to stormwater being conveyed by the existing creek channel and thus we are in support of the proposed development and removal of the existing creek channel.

We do not believe this will have any negative impacts to Unity Lake #1.

Sincerely,



David R. Vest, CPA
Mayor – Town of Unity Village

Chief Financial Officer – Unity School of Christianity

Stormwater Management Study Excerpt

DISCOVERY PARK MACRO & PHASE 1 MICRO STORMWATER REPORT

Prepared for:

Discovery Park Lee's Summit, LLC
Columbia, Missouri



January 2023

Olsson Project No. A21-04643



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1. INTRODUCTION

This Stormwater Drainage Study has been prepared to evaluate the stormwater hydrology of multiple developments (current and future) that are being proposed within the watershed. Once fully developed, the area treated by improvements proposed within this study will include:

- Aria Apartments: a 22.50-ac apartment development (Zoned RP-4)
- Discovery Park Phase 1: a 39.42-ac commercial development (Zoned CP-2, currently being rezoned to PMIX)
- Discovery Park Phase 2: a 19.82-ac commercial development (Zoned PMIX)
- Discovery Park Future: a 116.26-ac planned mixed use development (Zoned PMIX)
- A future Multi-Family Residential development of approximately 14.96-ac along the west side of NW Lee's Summit Road (Zoned RP-4)

The site is located at the northwest corner of NE Douglas Street and NE Colbern Road, in the NE ¼ of Section 30 & SE ¼ of Section 19, Township 48 North, Range 31 West, entirely within the City of Lee's Summit, Jackson County, Missouri.

Stormwater runoff from the project site is tributary to Unity Lake Number One and Unity Lake Number Two. Unity Lake Number One is approximately 1,000 feet downstream of the Discovery Park Future study area. Unity Lake Number Two is approximately 2,500 feet downstream of the Discovery Park Phase 1 and Aria study areas.

This report, intended to serve as the project Macro Stormwater Drainage Study for the entire Discovery Park development, and the Micro Stormwater Drainage Study for the Aria, Discovery Park Phase 1, and a portion of the Discovery Park Phase 2 Development, has been prepared to evaluate the Existing Conditions stormwater hydrology to establish Allowable Release Rates for the Macro portion of the study, and Proposed Conditions stormwater hydrology of the Micro portion of the study to review impacts the proposed development has on the existing hydrology. Refer to Section 7 for hydrologic model input data and simulation results for Existing- and Proposed Conditions. Refer to Section 8 for maps and exhibits depicting the watersheds evaluated in the analyses.



The FEMA FIRM Panel 29095C-0409G (eff. 20 January, 2017) depict the proposed development areas as “Zone X.” This is the FEMA flood insurance rate zone that “corresponds to areas outside the 0.2-percent-annual-chance floodplain, areas within the 0.2-percent-annual-chance floodplain, areas of 1-percent-annual-chance flooding where average depths are less

than 1 foot, areas of 1-percent-annual-chance flooding where the contributing drainage area is less than 1 square mile, and areas protected from the 1-percent-annual-chance flood by levees. No BFE's or base flood depths are shown for this zone." Refer to the attached FEMA Floodplain Map (Exhibit 8-1.1) for depiction of the established floodplains relative to the project site.

The lower reaches of the modeled sub-watershed that is the subject of this report does include a "Zone AE" boundary along the Little Cedar Creek – Tributary No. 2 channel that forms the main branch of the Unity Lake Number 2 impoundment. No construction proposed for these three areas is proposed to affect the boundaries of the defined floodway for this channel.

1.2. Soil Classification

Soil Maps published in the Soil Survey for Jackson County, Missouri categorizes soils in this watershed as:

Table 1.2-1. Soil Classifications

HSG	Map Symbol	Type	Land-Form
C	10000	Arisburg Silt Loam	1% to 5% Slopes
D	10024	Greenton-Urban Land Complex	5% to 9% Slopes
C	10026	Higginsville Silt Loam	5% to 9% Slopes
C	10082	Arisburg-Urban Land Complex	1% to 5% Slopes
D	10113	Oska Silty Clay Loam	5% to 9% Slopes, E
C/D	10116	Sampsel Silty Clay Loam	2% to 5% Slopes
C/D	10117	Sampsel Silty Clay Loam	5% to 9% Slopes
C	10120	Sharpsburg Silt Loam	2% to 5% Slopes
D	10128	Sharpsburg-Urban Land Complex	2% to 5% Slopes
D	10129	Sharpsburg-Urban Land Complex	5% to 9% Slopes
C	10132	Sibley Silt Loam	2% to 5% Slopes
C	10136	Sibley-Urban Land Complex	2% to 5% Slopes
D	10143	Snead-Urban Land Complex	9% to 30% Slopes
C	10179	Udarents-Urban Land-Oska Complex	5% to 9% Slopes
C	10180	Udarents-Urban Land-Sampsel Complex	2% to 5% Slopes
C/D	30080	Greenton Silty Clay Loam	5% to 9% Slopes
C	30180	Polo Silt Loam	5% to 9% Slopes
C	36083	Kennebec Silt Loam	1% to 4% Slopes, OF
D	40107	Snead-Rock Outcrop Complex, Warm	5% to 14% Slopes
D	40108	Snead-Rock Outcrop Complex, Warm	14% to 30% Slopes
-	99001	Water	-
-	99012	Urban Land, Upland	5% to 9% Slopes

(HSG = Hydrologic Soil Group, E=Eroded, OF=Occasionally Flooded)

NRCS Runoff Curve Numbers (CN's) in this study have been assigned to tributary areas based upon these Hydrologic Soil Groups and associated existing and proposed land use. The majority of land within the modeled sub-watersheds is previously developed, and the CN's are assigned accordingly. Refer to the Soils Map in Section 8 for distribution of soil types throughout the sub-watersheds.

2. METHODOLOGY

The hydrologic analysis provided in this report utilizes methods prescribed by the City of Lee's Summit, Missouri and the Kansas City Metropolitan Chapter of the APWA "Standard Specifications and Design Criteria," Division V, Section 5600 (February 2011) provides the overall framework for stormwater hydrology. The following approved methods were used in this report to model Existing- and Proposed Conditions for stormwater runoff.

- Haestad Methods, Inc. "PondPack" V8i (08.11.01.56).
- NRCS TR-55 Unit Hydrograph Method
- 2-, 10-, and 100-year Return Frequency, 24-hr. Storm Precipitation Depths (TP-40)
- ARC-II Soil Moisture Conditions
- 24-Hour NRCS Type II Rainfall Distribution
- Runoff Curve Numbers per NRCS TR-55 (Tables 2-2a – 2-2c) and APWA Sec.5602.3
- NRCS TR-55 Methods for determination of Time of Concentration and Travel Time.

NOTE: Where detailed information pertaining to channel geometry is unavailable, "length & velocity" estimates for channel-flow Travel Time is utilized per Section 5602.7, Kansas City Metropolitan Chapter- APWA Standard Specifications and Design Criteria.

NOTE: PondPack models utilize "Time of Concentration" rather than "Lag Time" for computing subarea hydrology.

Input data for the Existing- and Proposed Conditions hydrology models and results of computations are included in Section 7. Refer to the attached Drainage Area Maps for Existing- and Proposed Conditions subarea locations, weighted Runoff Curve Numbers, and tributary acreage included in Section 8.

Stormwater runoff models were created for the 2-, 10-, and 100-year design storm events. The precipitation depths used in the analyses have been interpolated from the "Technical Paper No. 40 Rainfall Frequency Atlas of the United States" (TP-40; May 1961) isopleth maps. The following table depicts the rainfall depths used in this analysis:

Table 2-1. Precipitation Depths

Return Period:	24-hour Precipitation Depth (in):
Water Quality Storm ¹	1.37
2-Year (50% Storm)	3.50
10-Year (10% Storm)	5.34
100-Year (1% Storm)	7.71

¹The "Water Quality Storm" is defined in the MARC & APWA "Manual of Best Management Practices for Stormwater Quality" as a 24-hr 1.37" rainfall depth. This particular storm event is utilized for proposed water quality analysis.

Each of the PondPack models constructed for this analysis evaluates multiple rainfall events using these three defined design storms.

The overall hydrology defines 9 modeled sub-watersheds (sub-watersheds “A,” “B,” “C,” “D,” “E,” “F,” “G,” “H,” “I”), and 30 subareas, encompassing approximately 998 acres overall.

Several offsite subareas are included in the models prepared for this report that will remain unaltered as a result of the proposed development areas. Sub-watersheds “B” and “C” are both offsite regions, as are some portions of sub-watershed “A,” “D,” “E,” “F,” “G,” “H,” “I”.

- The Aria site lies within portions of Subareas A3, A4(E), and A5(E).
- The Discovery Park Phase 1 site is located within Subareas A4(E), A4(W), A5(E) and A5(W).
- The Discovery Park Phase 2 site is located within Subareas A6 and D2.
- The Discovery Park Future site is located within Subareas D2, E1, F2, G2, H1 and I1.
- The future Multi-Family Residential development is located within Subarea A1(E).

In accordance with the City-specified criterion and design provision established in the 2011 edition of APWA Section 5600, the proposed stormwater management plan shall “be consistent with the Comprehensive Control Strategy.” This requirement establishes the maximum Allowable Release Rates for the 2-year (0.50 cfs/ac), 10-year (2.0 cfs/ac), and 100-year (3.0 cfs/ac) design storms. In addition to the large storm hydrology design constraints, this strategy requires extended detention (≥ 40 hr.) of the “Water Quality Storm” runoff volume.

Points of Interest

The hydrologic models prepared for this stormwater Drainage Study includes 30 Points of Interest in total. The 17 critical points of interest to be analyzed are briefly described as:

- **Point A1**, located at the southern inlet to the Unity Lake Number 2 impoundment, the downstream point of interest modeled in this report.
- **Point A2** is the confluence of Channels A and B, located north of an existing sanitary holding basin. All stormwater runoff generated by the Aria and Discovery Park projects is conveyed to this point.
- **Point A3** is a culvert in Channel B crossing an access drive for the existing sanitary holding basin.
- **Point A4** is a point within Channel A, upstream of Point A2, Portions of the Aria and Discovery Park projects contribute flow to this point of interest.
- **Point A5** is the downstream end of an existing pond with a breached embankment within Channel A.
- **Point A6** is the downstream end of an existing culvert under NW Colbern Road. A portion of Discovery Park Phase 2 will contribute flow to this point of interest.

- **Point A7** is the downstream end of an existing culvert within Channel A, located at station 25+65 of NE Douglas Street. All stormwater runoff contributed to this point is generated by offsite areas.
- **Point B1** is the downstream end of an existing culvert within Channel B, located at station 45+15 of NE Douglas Street. All stormwater runoff contributed to this point is generated by offsite areas.
- **Point D1** is located at an eastern inlet to the Unity Lake Number 1 impoundment, the downstream point of interest of sub-watershed D. Portions of Discovery Park Phase 1, Discovery Park Phase 2 and Discovery Park Future will contribute flow to this point of interest.
- **Point D2** is the downstream end of an existing culvert under NW Colbern Road. Portions of Discovery Park Phase 2 and Discovery Park Future will contribute flow to this point of interest. This point is where the majority of offsite stormwater, from areas south of Interstate 470, will drain through the future development area.
- **Point D4** is the downstream end of an existing culvert under I-470 highway. All stormwater runoff contributed to this point is generated by offsite areas. The culvert represents a choke point for all the offsite area draining to Discovery Park Future from South of I-470.
- **Point E1** is the downstream end of an existing culvert under NW Colbern Road. A portion of Discovery Park Future will contribute flow to this point of interest.
- **Point F1** is located at a southeastern inlet to the Unity Lake Number 1 impoundment, the downstream point of interest of sub-watershed F. A portion of Discovery Park Future will contribute flow to this point of interest.
- **Point F2** is the downstream end of an existing culvert under NW Colbern Road. A portion of Discovery Park Future will contribute flow to this point of interest.
- **Point G1** is located at a southwestern inlet to the Unity Lake Number 1 impoundment, the downstream point of interest of sub-watershed G. A portion of Discovery Park Future will contribute flow to this point of interest.
- **Point G2** is the downstream end of an existing culvert under NW Colbern Road. A portion of Discovery Park Future will contribute flow to this point of interest.
- **Point H1** is the downstream end of an existing culvert under N Main Street that discharges directly to Little Cedar Creek. A portion of Discovery Park Future will contribute flow to this point of interest.
- **Point I1** is the downstream end of an existing culvert under N Main Street. A small portion of Discovery Park Future will contribute flow to this point of interest.

Several additional points are utilized in the models to assist with the hydrologic analysis for offsite areas. The locations of these points are depicted in the attached Drainage Area Maps,

and the results of the analyses are included in tables provided in this narrative, and in the attached modeling output.

In order to provide a direct comparison between the Existing- and Proposed Conditions (Micro- and Macro) hydrology models, efforts have been made to ensure that the points of interest are consistent between these analyses with the exception of Point A5 as discussed in Section 4 of this report. As noted, additional points to those previously described are included in the hydrologic models, these junctions are of secondary interest to this particular development. Refer to the attached Drainage Area Maps for graphical representation of the modeled subareas and points of interest; refer to Section 7 for schematic view of the PondPack watershed model and connectivity between subareas, channel reaches, and points of interest.

3. EXISTING CONDITIONS ANALYSIS

The purpose of this report is to provide the Macro and Micro Stormwater Drainage analysis to ensure that the proposed site development is compliant with City of Lee's Summit, Missouri requirements. This section of the report will provide the Existing Conditions analysis, representing the current site hydrology. The results of this analysis will be used to establish the "Allowable Release Rates" for the project and will be compared to those of Proposed Conditions in order to determine the hydrologic effects of the development upon the receiving stormwater conveyances and sub-watersheds.

Runoff Curve Numbers have been developed based upon the current land use obtained from survey data, aerial photographs, and site visits. Refer to Section 7 for Existing Conditions weighted NRCS Runoff Curve Number (CN) and Time of Concentration (T_c) calculations. Table 3-1 contains the hydrologic parameters used to characterize subareas the Existing Conditions PondPack model. A graphical representation of the areas is provided on exhibit EX-300 in Section 8.

Table 3-1(a). Existing Conditions Input Data

	Total Area (ac):	Aria Onsite Area (ac):	Discovery Park Phase 1 Onsite Area (ac):	Discovery Park Phase 2 Onsite Area (ac):	Macro-Study Onsite Area (ac):	NRCS Weighted CN:	T_c¹ (hr):
Subarea A1(e):	27.97				12.36	77	0.1736
Subarea A1(w):	43.14					76	0.2511
Subarea A2(e):	3.08					80	0.0875
Subarea A2(w):	23.41					73	0.2133
Subarea A3:	24.42	11.62				80	0.1284
Subarea A4(e):	9.85	5.99	3.35			81	0.1367
Subarea A4(w):	17.41		9.19			73	0.1656
Subarea A5(e):	15.16	4.88	8.00			80	0.1195
Subarea A5(w):	19.77		18.66			76	0.1578
Subarea A6:	13.45			9.21		80	0.1063
Subarea A7:	30.60					76	0.1596
Subarea A8:	21.56					81	0.1299
Subarea A9:	31.84					84	0.1420
Subarea A10:	12.23					94	0.1545
Subtotal:	293.88	22.50	39.20	9.21	12.36		
Subarea B1:	31.94					79	0.1385
Subarea B2:	48.54					78	0.1987
Subarea B3:	40.69					83	0.1447
Subarea B4-DET:	25.05					94	0.1155

	Total Area (ac):	Aria Onsite Area (ac):	Discovery Park Phase 1 Onsite Area (ac):	Discovery Park Phase 2 Onsite Area (ac):	Macro- Study Onsite Area (ac):	NRCS Weighted CN:	T_c¹ (hr):
Subarea B4-BYP:	7.42					94	0.0787
Subtotal:	153.64						
Subarea C1:	45.21					90	0.2860
Subarea C2:	57.13					81	0.3308
Subarea C3:	52.36					83	0.1199
Subtotal:	154.70						
Subarea D1:	36.91		0.22			77	0.1275
Subarea D2:	47.00			10.62	24.74	78	0.1755
Subarea D3:	16.84					88	0.1561
Subarea D4:	112.38					88	0.1792
Subarea D5:	53.24					89	0.2652
Subtotal:	266.37		0.22	10.62	24.74		
Subarea E1:	22.59				20.68	80	0.1062
Subarea E2:	6.56					73	0.1021
Subtotal:	29.15				20.68		
Subarea F1:	7.85					77	0.0938
Subarea F2:	12.76				12.06	81	0.1260
Subtotal:	20.60				12.06		
Subarea G1:	14.04					81	0.1055
Subarea G2:	7.93				7.11	85	0.1175
Subtotal:	21.96				7.11		
Subarea H1:	56.17				51.06	78	0.1348
Subtotal:	56.17				51.06		
Subarea I1:	1.40				0.61	86	0.1019
Subtotal:	1.40				0.61		
Total:	997.89	22.50	39.42	19.83	128.62		

¹ **Note:** Per TR-55 documentation, minimum T_c is 0.10 hours; model is configured with default value of 0.10 hr. minimum T_c.

Table 3-1(b). Existing Conditions Input Sub-Watershed Data

	Total Area (ac):	Aria Onsite Area (ac):	Discovery Park Phase 1 Onsite Area (ac):	Discovery Park Phase 2 Onsite Area (ac):	Macro-Study Onsite Area (ac):
Sub-W'Shed A Subtotal:	293.88	22.50	39.20	9.21	12.36
Sub-W'Shed B Subtotal:	153.64				
Sub-W'Shed C Subtotal:	154.70				
Sub-W'Shed D Subtotal:	266.37		0.22	10.62	24.74
Sub-W'Shed E Subtotal:	29.15				20.68
Sub-W'Shed F Subtotal:	20.60				12.06
Sub-W'Shed G Subtotal:	21.96				7.11
Sub-W'Shed H Subtotal:	56.17				51.06
Sub-W'Shed I Subtotal:	1.40				0.61
Modeled Total:	997.89	22.50	39.42	19.83	128.62

These tributary areas, Runoff Curve Numbers (CN), and Times of Concentration (T_C) for the corresponding subareas were used as input to the Existing Conditions PondPack model to evaluate the stormwater hydrology. The subareas representing Existing Conditions for the development and associated sub-watersheds are utilized in the PondPack model to compute the stormwater runoff for the three design storms. The resultant peak discharge rate (Q_P), peak time (T_P), and runoff volume (V_R) for the computed hydrographs of modeled subareas are included in Table 3-2.

Table 3-2. Hydrologic Information – Existing Conditions Summary (Subareas)

	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Subarea A-1(e):	53.91	12.01	3.334	110.32	11.99	6.783	187.82	11.99	11.675
Subarea A-1(w):	70.15	12.05	4.908	146.86	12.05	10.132	252.55	12.05	17.597
Subarea A-2(e):	7.64	11.94	0.420	14.90	11.93	0.819	24.63	11.93	1.374
Subarea A-2(w):	34.59	12.03	2.301	76.77	12.03	4.975	136.28	12.03	8.884
Subarea A-3:	57.61	11.96	3.330	112.58	11.95	6.495	186.13	11.95	10.895
Subarea A-4(e):	23.97	11.96	1.402	45.97	11.96	2.698	75.15	11.96	4.488
Subarea A-4(w):	27.56	12.01	1.712	60.85	11.99	3.700	108.22	11.99	6.607
Subarea A-5(e):	36.40	11.95	2.067	70.90	11.95	4.031	116.98	11.95	6.762
Subarea A-5(w):	37.02	12.00	2.249	77.10	11.99	4.643	132.61	11.97	8.064
Subarea A-6:	37.98	11.93	2.083	70.54	11.93	3.904	113.19	11.93	6.389
Subarea A-7:	57.26	12.00	3.482	119.03	11.98	7.186	204.90	11.98	12.481
Subarea A-8:	53.05	11.96	3.070	101.89	11.95	5.907	166.82	11.95	9.826
Subarea A-9:	87.15	11.97	5.138	158.95	11.96	9.507	252.78	11.96	15.435
Subarea A-10:	45.61	11.97	2.890	72.57	11.97	4.733	106.86	11.97	7.128

	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Subarea B-1:	70.94	11.97	4.168	140.29	11.96	8.242	234.20	11.95	13.944
Subarea B-2:	94.72	12.03	6.057	190.16	12.01	12.146	320.74	12.00	20.725
Subarea B-3:	106.50	11.97	6.302	197.62	11.96	11.812	317.37	11.96	19.332
Subarea B-4 (BYP):	30.29	11.93	1.753	48.22	11.92	2.871	71.04	11.92	4.324
Subarea B-4 (DET):	99.43	11.94	5.919	158.21	11.94	9.694	232.94	11.94	14.599
Subarea C-1:	124.83	12.05	9.223	209.38	12.05	15.842	317.10	12.05	24.565
Subarea C-2:	104.44	12.09	8.134	200.90	12.08	15.652	329.78	12.08	26.038
Subarea C-3:	142.61	11.95	8.110	264.83	11.94	15.199	425.51	11.94	24.875
Subarea D-1:	75.69	11.97	4.400	155.84	11.95	8.95	265.94	11.95	15.407
Subarea D-2:	99.28	12.00	6.134	195.91	12.00	12.128	326.27	11.98	20.518
Subarea D-3:	52.46	11.97	3.185	90.29	11.97	5.605	138.69	11.97	8.822
Subarea D-4:	338.74	11.99	21.246	582.43	11.99	37.391	894.20	11.99	58.857
Subarea D-5:	146.13	12.05	10.458	247.91	12.05	18.183	377.88	12.03	28.406
Subarea E-1:	55.42	11.94	3.080	107.96	11.94	6.008	178.18	11.93	10.078
Subarea E-2:	11.34	11.94	0.645	25.34	11.94	1.394	45.12	11.94	2.489
Subarea F-1:	16.88	11.94	0.935	34.70	11.93	1.903	59.21	11.93	3.275
Subarea F-2:	31.60	11.96	1.816	60.52	11.95	3.495	99.00	11.95	5.814
Subarea G-1:	36.14	11.94	1.999	69.23	11.93	3.846	113.41	11.93	6.398
Subarea G-2:	23.49	11.94	1.332	42.32	11.94	2.433	66.71	11.94	3.92
Subarea H-1:	119.73	11.97	7.009	240.97	11.96	14.056	406.72	11.95	23.984
Subarea I-1:	4.44	11.93	0.244	7.87	11.93	0.441	12.29	11.93	0.705

The PondPack models created for these analyses include 24 channel reaches modeled using the Muskingum Method. Various modeled points of interest are connected in the model by these channel reaches. The reach segments account for the time-lagging effect on the hydrograph peaks, including a relatively insignificant amount of peak flow rate attenuation as the flood wave travels downstream. Travel time for each reach is based upon the reach length and average bed-slope. The following table contains the input data for the channel reaches used in the Existing- and Proposed Conditions PondPack models.

Table 3-3. Muskingum Channel Reach Routing Information

Reach I.D.	K (hr.):	X_(coeff.):	No. of Sub-reaches	K (ea. sub-reach) (hr.):
Route A2~A1:	0.0826	0.25	9	0.0092
Route A4~A2:	0.0288	0.25	3	0.0097
Route A5~A4:	0.0203	0.25	3	0.0067
Route A3~A2:	0.0192	0.25	2	0.0097
Route A6~A5:	0.0378	0.25	4	0.0094
Route A7~A5:	0.0412	0.25	5	0.0083
Route A8~A7:	0.0340	0.25	4	0.0086
Route A9~A7:	0.0390	0.25	4	0.0097
Route A10~A9:	0.0636	0.25	7	0.0092
Route B1~A3:	0.0331	0.25	4	0.0083
Route B2~B1:	0.0162	0.25	2	0.0081
Route B3~B2:	0.0824	0.25	9	0.0092
Route B4~B3:	0.0587	0.25	6	0.0097
Route C1~B1:	0.0210	0.25	3	0.0069
Route C2~C1:	0.0701	0.25	8	0.0089
Route C3~C2:	0.0148	0.25	2	0.0075
Route D2~D1:	0.0428	0.25	8	0.0053
Route D3~D2:	0.0758	0.25	6	0.0126
Route D4~D2:	0.0598	0.25	6	0.0100
Route D5~D4:	0.0475	0.25	5	0.0095
Route E1~D1:	0.0339	0.25	6	0.0056
Route E2~E1:	0.0336	0.25	3	0.0112
Route F2~F1:	0.0192	0.25	3	0.0064
Route G2~G1:	0.0244	0.25	3	0.0081

The preceding stormwater conveyances have been incorporated into both the Existing and Proposed Conditions PondPack models. The results of the Existing Conditions hydrologic routing are provided in Table 3.2-1, in Section 3.2, which provides the summary data for the points of interest defined in this Stormwater Drainage Study. Refer to the Existing Conditions Drainage Area Map for the location of the points of interest, modeled subareas, channels, and conveyances, in relation to proposed development area.

The defined subareas and stormwater conveyances, with associated hydrograph attenuation effects have been incorporated into the Existing (and Proposed) Conditions PondPack models. There are also several offsite storage areas included in the Existing- and Proposed Conditions models for the project site. The following section of this report provides the geometric configuration and hydrologic routing summary for existing locations. The results of the Existing Conditions hydrologic routing at the designated Points of Interest are provided in the Section 3.2.

3.1. Existing Stormwater Storage Areas & Detention Facilities

The following information is provided to define the geometry of the outfall structure and storage capacity for the existing modeled storage areas and stormwater detention basins included in the Existing Conditions hydrology models. The routing summary tables provided for each location contain the Existing Conditions hydrologic routing summary for each of the design storm events considered in this analysis.

Point A3 – Culverts at Pump Station Drive Culverts:

An access drive extends across the channel that runs along the northern edge of the Aria project site. This restricted-access drive leads to the pump station that is positioned at the confluence of two channels that define the upper regions of the modeled sub-watershed. The Existing- and Proposed Conditions geometry of the culverts positioned across the existing access drive to the pump station are defined as follows:

Primary Outlet Structure:

Dual 84" Diameter (*modeled equivalent circular: 72" x 96" CMPA Culvert*); Length \approx 66.6 L.F.
Flowline In / Out \approx 900.81 / 900.43 (ft; NAVD)

Secondary Outlet Structure:

Roadway (Weir)-

Station (ft):	0	22	43.1	76.73	124.4	157.13	223.83	251	265.18	270.32	277.43
Elevation (ft):	912	910	908.52	907.24	907.44	908.16	908.35	908.6	909	910	912

Culvert flow at the threshold of overtopping/weir-flow is approximately 255 cfs.

The potential storage area upstream from this roadway crossing is defined in the following table based upon storage area for a given elevation. As depicted below, the potential storage area is somewhat modest, relative to the existing conditions flows that are contributed to this location.

Table 3.1-1(a). Existing Storage Volume – Pump Station Drive Storage Area

Elevation (ft; NAVD):	Storage Area (sq. ft.):	Incremental Volume (ft ³):	Cumulative Volume (ac-ft):
900.81	106	0	0.000
901	114	21	0.000
902	691	362	0.009
902.4	908	319	0.016
903	1,252	645	0.031
904	3,270	2,182	0.081
906	7,400	10,393	0.320
908	38,290	41,682	1.276
910	58,812	96,371	3.489
912	84,827	142,847	6.768

Note: The conic method is used to compute the incremental volume between pond contours.

The sum of the computed incremental volumes provides the cumulative pond volume for a given elevation.

Utilizing these geometric parameters, the following table depicts the stormwater routing summary for this location.

**Table 3.1-1(b). Existing Conditions Storage Area Routing –
Point A3 – Pump Station Drive Culverts**

	Peak Q In (cfs):	T _P In (hr.):	Peak Q Out (cfs):	T _P Out (hr.):	V _R (ac-ft):	Peak WSEL (ft; NAVD):	Max. Storage Volume (ac-ft):
WQv:	102.98	12.15	101.73	12.18	9.704	904.61	0.134
2-Year:	632.94	12.13	628.64	12.15	52.997	908.50	1.747
10-Year:	1,109.82	12.13	1,107.67	12.15	97.952	909.07	2.341
100-Year:	1,597.03	12.16	1,595.56	12.17	159.297	909.50	2.849

The preceding routing summary indicates that the culverts are somewhat under-sized for most large-storm events, though the Water Quality Volume storm event is readily conveyed by this system.

Point A5 – Former Pond:

The Existing Conditions geometry of the outfall structure for the former pond site that is located just north of the Discovery Park Phase 1 property is based upon field survey and LIDAR topographic data. This former pond-site has an embankment breach, and therefore provides relatively minimal attenuation for larger precipitation events. The feature does, however affect the hydrograph timing somewhat, and is therefore included in these models. The pond is proposed to be removed with the Discovery Park Phase 1 development. The following information provides the outfall geometry and storage area for the Existing Conditions PondPack model:

Primary Outlet Structure- Weir Cut through Embankment:

Station (ft):	0	23.88	64.85	167.91	238.1	246.58	251.38	253.1	257.52	263.49	268.12	273.3	297.08	333.5
Elevation (ft):	940	938	936	934	932	930	929	929	930	932	934	936	938	940

The potential storage area upstream from this pond embankment is defined in the following table based upon a stage versus storage area for a given elevation.

Table 3.1-2(a). Existing Storage Volume – Boone Creek Embankment

Elevation (ft; NAVD):	Storage Area (sq. ft.):	Incremental Volume (ft³):	Cumulative Volume (ac-ft):
929	20,212	0	0.000
930	34,209	26,905	0.618
932	75,878	107,357	3.082
934	98,561	173,945	7.075
936	130,004	227,841	12.306
938	171,091	300,156	19.197
940	226,998	396,774	28.305

Note: The conic method is used to compute the incremental volume between pond contours.

The sum of the computed incremental volumes provides the cumulative pond volume for a given elevation.

The following table depicts the stormwater routing summary for this location under each of the modeled precipitation events.

Table 3.1-2(b): Existing Conditions Storage Area Routing – Point A5 – Former Pond

	Peak Q In (cfs):	T_P In (hr.):	Peak Q Out (cfs):	T_P Out (hr.):	V_R (ac-ft):	Peak WSEL (ft; NAVD):	Max. Storage Volume (ac-ft):
WQv:	47.52	12.08	22.44	12.24	3.428	930.22	0.800
2-Year:	315.36	12.05	242.32	12.15	20.98	932.48	3.940
10-Year:	552.78	12.05	489.66	12.12	39.91	933.43	5.828
100-Year:	847.10	12.05	799.19	12.10	66.087	934.15	7.413

Data provided in the preceding table indicates that, while larger storm events are not significantly affected by this feature, the more frequent events do exhibit some hydrograph attenuation at this location.

Point B1 – Lee’s Summit Road RCB Culverts (Sta. 45+15):

Lee’s Summit Road has a pair of RCB Culverts that convey stormwater across the roadway into the channel that runs along the northern edge of the Aria project site. This RCB includes stormwater flows generated by defined sub-watersheds “B” and “C.” The following information provides the outfall geometry and storage area for this location in both Existing- and Proposed Conditions PondPack models:

Primary Outlet Structure:

Dual 10’-w x 7’-ht RCB Culvert; Length \approx 174.4 L.F.

Flowline In / Out \approx 916.22 / 911.54 (ft; NAVD)

Secondary Outlet Structure:

Roadway (Weir)-

Station (ft):	0	64.20	155.0	244.3	309.0
Elevation (ft):	930	929	928.68	929	930

Culvert flow at the threshold of overtopping/weir-flow is approximately 1,800 cfs.

Table 3.1-3(a). Existing Storage Volume – Pump Station Drive Storage Area

Elevation (ft; NAVD):	Storage Area (sq. ft.):	Incremental Volume (ft ³):	Cumulative Volume (ac-ft):
916.22	377	0	0
917	2,013	848	0.019
918	8,841	5,024	0.135
919	14,303	11,463	0.398
920	18,052	16,141	0.769
921	21,576	19,788	1.223
922	25,405	23,464	1.761
923	29,240	27,300	2.388
924	39,203	34,100	3.171
925	50,630	44,795	4.199
926	60,928	55,700	5.478
927	71,454	66,121	6.996
928	83,880	77,584	8.777
929	139,252	110,403	11.312
930	178,519	158,480	14.950

Note: The conic method is used to compute the incremental volume between pond contours.

The sum of the computed incremental volumes provides the cumulative pond volume for a given elevation.

The following table depicts the stormwater routing summary for this location under each of the modeled precipitation events.

**Table 3.1-3(b). Existing Conditions Storage Area Routing –
Point B1 – Lee's Summit Drive Culverts (Sta. 45+15)**

	Peak Q In (cfs):	T _P In (hr.):	Peak Q Out (cfs):	T _P Out (hr.):	V _R (ac-ft):	Peak WSEL (ft; NAVD):	Max. Storage Volume (ac-ft):
WQv:	101.70	12.11	100.64	12.12	9.245	917.82	0.101
2-Year:	634.00	12.07	615.14	12.11	49.667	921.57	1.519
10-Year:	1,121.63	12.07	1,076.61	12.11	91.457	923.99	3.164
100-Year:	1,726.54	12.06	1,551.65	12.14	148.401	926.77	6.626

Data provided in the preceding table indicates that each of the modeled events are effectively conveyed by the culverts, and sufficient freeboard appears to be available for the 100-year design storm.

Point A7 – NE Douglas Street RCB Culvert (Sta. 25+65):

NE Douglas Street has another RCB Culvert that conveys stormwater across the roadway into the channel that runs into the eastern edge of the Discovery Park project site. This culvert conveys stormwater into the project area from upper portions of the defined sub-watershed “A.” The following information provides the outfall geometry and storage area for this location in both Existing- and Proposed Conditions PondPack models:

Primary Outlet Structure:

13'-w x 7'-ht RCB Culvert; Length ≈ 174.4 L.F.

Flowline In / Out ≈ 945.3 / 937.5 (ft; NAVD)

Secondary Outlet Structure:

Roadway (Weir)-

Station (ft):	0	46.5	118.2	206.1	276.8	345.4	392.4
Elevation (ft):	958	957	956	955.65	956	957	958

Culvert flow at the threshold of overtopping/weir-flow is approximately 1,213 cfs.

**Table 3.1-4(a): Existing Storage Volume –
Point A7 – Lee’s Summit Road RCB Culvert (Sta. 25+65)**

Elevation (ft; NAVD):	Storage Area (sq. ft.):	Incremental Volume (ft ³):	Cumulative Volume (ac-ft):
945.3	273	0	0.000
946	728	338	0.008
947	3,022	1,744	0.048
948	6,247	4,538	0.152
949	19,586	12,298	0.434
950	36,075	27,414	1.064
951	52,213	43,896	2.071
952	62,652	57,353	3.388
953	78,881	70,611	5.009
954	93,052	85,869	6.980
955	109,368	101,100	9.301
956	141,847	125,256	12.177
957	178,341	159,746	15.844
958	220,369	198,985	20.412

Note: The conic method is used to compute the incremental volume between pond contours.

The sum of the computed incremental volumes provides the cumulative pond volume for a given elevation.

The following table depicts the stormwater routing summary for this location under each of the modeled precipitation events.

**Table 3.1-4(b): Existing Conditions Storage Area Routing –
Point A7 – Lee’s Summit Drive Culverts (Sta.25+65)**

	Peak Q In (cfs):	T _P In (hr.):	Peak Q Out (cfs):	T _P Out (hr.):	V _R (ac-ft):	Peak WSEL (ft; NAVD):	Max. Storage Volume (ac-ft):
WQv:	38.00	12.06	37.95	12.06	2.539	946.33	0.015
2-Year:	236.01	12.02	229.26	12.05	14.58	948.71	0.319
10-Year:	437.94	12.01	397.23	12.07	27.333	950.22	1.251
100-Year:	707.16	12.00	603.32	12.08	44.871	951.80	3.100

Data provided in the preceding table indicates that each of the modeled events is effectively conveyed by this existing culvert, and sufficient freeboard appears to be available for each design storm.

Sub-Basin B4- Stormwater Detention Facility:

The upper reaches of the sub-watershed “B” region includes a relatively large, impervious property that includes an onsite wet-basin stormwater detention facility. The following information provides the modeled outfall geometry and storage area for the Existing- and Proposed Conditions PondPack models:

Primary Outlet Structure:

Inlet Box: 4.2 S.F. Opening; Crest Elev. \approx 992.0

36" Dia. RCP Discharge Culvert: Length = 280 L.F.

Flowline In / Out \approx 981 / 976 (ft; NAVD)

**Table 3.1-5(a): Existing Storage Volume –
Proposed Sub-Basin B4 Detention Facility**

Elevation (ft; NAVD):	Storage Area (sq. ft.):	Incremental Volume (ft ³):	Cumulative Volume (ac-ft):
992	75,323	0	0.000
994	88,777	163,916	3.763
996	101,439	190,076	8.127
998	114,890	216,190	13.090

Note: The conic method is used to compute the incremental volume between pond contours.

The sum of the computed incremental volumes provides the cumulative pond volume for a given elevation.

The following table depicts the stormwater routing summary for this location under each of the modeled precipitation events.

**Table 3.1-5(b). Existing Conditions Storage Area Routing –
Sub-Basin B4 Detention Facility**

	Peak Q In (cfs):	T _P In (hr.):	Peak Q Out (cfs):	T _P Out (hr.):	V _R (ac-ft):	Peak WSEL (ft; NAVD):	Max. Storage Volume (ac-ft):
WQv:	30.27	11.95	8.05	12.15	1.713	992.37	0.642
2-Year:	99.43	11.94	22.46	12.16	5.919	993.25	2.284
10-Year:	158.21	11.94	28.68	12.19	9.694	994.04	3.850
100-Year:	232.94	11.94	34.97	12.24	14.599	995.03	5.948

The preceding routing summary indicates that the existing wet-basin detention facility provides significant attenuation for the modeled events.

Point C2 – Storage Area~Inlet C2:

The Existing- and Proposed Conditions models include a relatively significant storage unit in the defined sub-watershed "C" area. Geometric parameters for this storage area are based upon LIDAR topography and GIS base data. The following information provides the modeled outfall geometry and storage area for the Existing- and Proposed Conditions PondPack models:

Primary Outlet Structure:

Grate Inlet; Crest Elev. \approx 956.85

Opening Area: 27.2 S.F.

66" Dia. RCP Discharge Culvert: Length = 177 L.F.

Flowline In / Out \approx 945.7 / 945.1 (ft; NAVD)

Table 3.1-6(a). Existing Storage Volume – Storage Area C2

Elevation (ft; NAVD):	Storage Area (sq. ft.):	Incremental Volume (ft ³):	Cumulative Volume (ac-ft):
956.85	777	0	0
958	10,771	5,536	0.127
960	29,311	38,567	1.012
962	63,479	90,617	3.093
964	98,380	160,590	6.779
966	135,252	232,656	12.120
968	175,712	310,083	19.239
970	245,445	419,220	28.863
972	310,449	554,622	41.595

Note: The conic method is used to compute the incremental volume between pond contours.

The sum of the computed incremental volumes provides the cumulative pond volume for a given elevation.

The following table depicts the stormwater routing summary for this location under each of the modeled precipitation events.

Table 3.1-6(b). Existing Conditions Storage Area Routing – Storage Area C2

	Peak Q In (cfs):	T _P In (hr.):	Peak Q Out (cfs):	T _P Out (hr.):	V _R (ac-ft):	Peak WSEL (ft; NAVD):	Max. Storage Volume (ac-ft):
WQv:	21.38	12.02	21.36	12.02	1.338	956.90	0.001
2-Year:	142.28	11.96	137.26	12.00	8.110	957.95	0.115
10-Year:	264.20	11.96	219.81	12.05	15.199	959.67	0.804
100-Year:	423.95	11.96	285.13	12.07	24.875	961.59	2.539

The preceding routing summary indicates that, while the more frequent events are minimally affected by this feature, the larger events do undergo some attenuation.

Point D4 – I-470 Highway Culvert:

I-470 Highway has an RCB Culvert that conveys stormwater across the highway into the Discovery Park Future project site. This culvert conveys stormwater into the project area from upper portions of the defined sub-watershed “D.” The following information provides the outfall geometry and storage area for this location in the Existing-Conditions PondPack model:

Primary Outlet Structure:

5'-w x 8'-ht RCB Culvert; Length ≈ 323 L.F.

Flowline In / Out ≈ 952.00 / 948.00 (ft; NAVD)

**Table 3.1-4(a): Existing Storage Volume –
Point D4 – I-470 Highway Culvert**

Elevation (ft; NAVD):	Storage Area (sq. ft.):	Incremental Volume (ft ³):	Cumulative Volume (ac-ft):
956	14,459	0	0
958	38,781	51,280	1.177
960	77,382	113,963	3.793
962	119,729	195,577	8.283
964	164,668	283,206	14.785
966	324,776	480,468	25.815

Note: The conic method is used to compute the incremental volume between pond contours.

The sum of the computed incremental volumes provides the cumulative pond volume for a given elevation.

The following table depicts the stormwater routing summary for this location under each of the modeled precipitation events.

**Table 3.1-4(b): Existing Conditions Storage Area Routing –
Point D4 – I-470 Highway Culvert**

	Peak Q In (cfs):	T _P In (hr.):	Peak Q Out (cfs):	T _P Out (hr.):	V _R (ac-ft):	Peak WSEL (ft; NAVD):	Max. Storage Volume (ac-ft):
WQv:	98.35	12.04	95.20	12.07	6.953	954.62	0.188
2-Year:	447.33	12.02	377.63	12.10	31.704	958.57	2.269
10-Year:	761.81	12.02	526.54	12.14	55.574	960.91	6.129
100-Year:	1157.85	12.02	638.44	12.19	87.262	963.49	13.467

Data provided in the preceding table indicates that each of the modeled events is effectively conveyed by this existing culvert, and sufficient freeboard below I-470 Highway appears to be available for each design storm.

The previously defined modeled stormwater conveyances and the associated hydrograph attenuation effects from the existing storage areas have been incorporated into both Existing- and Proposed Conditions PondPack models. Each of these storage areas is offsite from the proposed development areas contemplated by this Micro- and Macro Stormwater Drainage Study. The results of the Existing Conditions hydrologic routing are provided in the following section. Additional minor storage and conveyance systems are included in the PondPack model to further expand and represent existing conditions for the watersheds. These systems are available within the model output provided in Section 8, for review.

3.2. Existing Conditions Hydrologic Modeling Results

The stormwater storage areas and resultant hydrograph attenuation effects are incorporated into the Existing Conditions PondPack models. The results of this hydrologic routing of the hydrographs generated by the modeled subareas through the sub-watersheds are provided in the following summary tables. This information is provided at the defined Points of Interest.

Refer to the Existing Conditions Drainage Area Map for the location of the points in relation to the proposed development areas, modeled subareas, channels, and conveyances.

Table 3.2-1. Hydrologic Information – Existing Conditions Summary (Junctions)

	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Point A1:	948.96	12.25	88.053	1,784.23	12.22	166.969	2,754.84	12.19	276.008
Point A2:	909.05	12.17	79.810	1,691.73	12.15	150.055	2,566.59	12.13	246.736
Point A3:	628.64	12.15	52.997	1,107.67	12.15	97.952	1,595.56	12.17	159.297
Point A4:	260.92	12.15	24.092	539.49	12.11	46.309	906.56	12.09	77.182
Point A5:	242.32	12.15	20.979	489.66	12.12	39.911	799.19	12.10	66.087
Point A6:	37.98	11.93	2.083	70.54	11.93	3.904	113.19	11.93	6.389
Point A7:	229.26	12.05	14.580	397.23	12.07	27.333	603.32	12.08	44.871
Point A8:	53.05	11.96	3.070	101.89	11.95	5.907	166.82	11.95	9.826
Point A9:	128.79	12.00	8.028	223.48	11.99	14.240	346.07	11.99	22.563
Point A10:	45.61	11.97	2.890	72.57	11.97	4.733	106.86	11.97	7.128
Point B1:	615.14	12.11	49.667	1,076.61	12.11	91.457	1,551.65	12.14	148.401
Point B2:	242.31	12.05	20.032	446.43	12.04	36.522	715.56	12.03	58.980
Point B3:	153.04	11.99	13.975	266.04	11.98	24.377	411.65	11.97	38.255
Point B4:	48.10	11.94	7.673	71.04	11.93	12.565	98.80	11.93	18.924
Point C1:	350.42	12.09	25.466	604.01	12.10	46.692	890.10	12.10	75.478
Point C2:	232.89	12.04	16.244	417.32	12.07	30.851	614.91	12.08	50.912
Point C3:	142.61	11.95	8.110	264.83	11.94	15.199	425.51	11.94	24.875
Point D1:	560.43	12.08	49.147	941.67	12.07	89.659	1,351.30	12.05	144.576
Point D2:	485.06	12.10	41.022	752.83	12.06	73.431	1,052.38	12.06	117.417
Point D3:	52.46	11.97	3.185	90.29	11.97	5.605	138.69	11.97	8.822
Point D4:	377.63	12.10	31.704	526.54	12.14	55.574	638.44	12.19	87.262
Point D5:	142.73	12.07	10.458	239.98	12.08	18.183	357.61	12.09	28.406
Point E1:	65.98	11.95	3.725	106.88	11.99	7.278	143.00	11.98	11.753
Point E2:	11.34	11.94	0.645	25.34	11.94	1.394	45.12	11.94	2.489
Point F1:	47.57	11.97	2.752	74.89	11.96	5.398	106.16	11.94	9.090
Point F2:	31.56	11.96	1.816	45.26	12.06	3.495	65.86	12.07	5.814
Point G1:	58.32	11.95	3.331	102.21	11.94	6.279	163.43	11.97	10.317
Point G2:	23.29	11.96	1.332	36.98	12.02	2.433	62.01	11.99	3.920
Point H1:	119.54	11.97	7.009	240.90	11.96	14.056	406.43	11.96	23.984
Point I1	4.44	11.93	0.244	7.87	11.93	0.441	12.29	11.93	0.705

The information obtained in this Existing Conditions analysis is utilized to establish the “Allowable Release Rates” (ARR) for the project areas based upon percentages of onsite and offsite area draining to the points of interest. A summary of the computation of these ARR’s is provided in the following section.

3.3. Allowable Release Rate Calculations

The purpose of this Stormwater Drainage Study for the proposed development is to ensure compliance with the current specifications and design criteria in effect for the City of Lee’s Summit, Missouri. The Proposed Conditions Micro- and Macro hydrologic analyses are provided in Section 4. Results of that analysis will be compared to the Section 3, Existing Conditions results. The variation in quantity and rate of stormwater discharge between these two models represents the hydrologic effect generated by development of the project site. The Actual-versus Allowable Release Rates for the fully developed, Proposed Conditions will be evaluated in Section 4 for both Micro- and Macro-Study analyses in order to demonstrate compliance with the design objectives for this project.

The following tables are provided to compute the “Allowable Release Rate” (ARR) for the three design storm events in order to demonstrate compliance with the stated design objectives. The Proposed Conditions Micro- and Macro- Models utilize five defined points of discharge that receive stormwater runoff from the project site. The proposed Stormwater Management Plan for the proposed development areas will evaluate the ARR’s at each of these modeled outlet locations.

Allowable Release Rate Calculations:

This section of the Stormwater Drainage Study for the proposed development is provided to establish the project Allowable Release Rates. The ARR is based upon the “Comprehensive Control” strategy provided in the Section 5600 of the APWA “Standard Specifications and Design Criteria” (Feb, 2011). This strategy provides the maximum ARR for the 2-, 10-, and 100-Year Design Storms at 0.50, 2.0, and 3.0 cfs-per-acre, respectively. Additional requirements include Extended Detention of the “Water Quality” design storm (defined as a 1.37” precipitation event).

The following table is provided in order to establish the project ARR for the Discovery Park Micro Stormwater Drainage analysis. The ARR for each of the three modeled events is based upon the sum of the “Allowable Bypass” for offsite flows and the onsite “Allowable Release Rate.” In order to prevent skewing the results, the ARR is based upon the Existing Conditions model. This ensures that the allowable bypass flows are not increased by onsite development, and that any diversions of tributary regions that takes place upon development is not incorporated into the allowable rates.

Information presented in the following table depicts the cumulative onsite and offsite areas for each of the Points, and the Existing Conditions peak discharge rates at these locations. From that information, the ARR is established in the second column from the right. The right-hand column depicts the relative difference in peak discharge rate that the proposed stormwater management plan would meet to achieve compliance with these targeted ARR's.

The Micro-Study section of this report will provide a comparison of these Allowable Release Rates to the computed peak discharge rates at these Points of Interest to demonstrate compliance with the stated design objectives.

Table 3.3-1. Existing Conditions Allowable Release Rate Calculations

Location:	Return Event (Yr):	Existing Conditions Peak Flow Rate (cfs):	Total Area (ac):	Onsite (Offsite) Area (ac):	Onsite (Offsite) Area (%):	Allowable Bypass Flows (cfs):	Allowable Onsite Discharge Rate: (cfs):	Prop. Cond. ARR (cfs):	Difference in Flow Rate: (cfs):
Point A1			604.82	85.87	14%				
	2-Year:	948.96		-----	-----	814.24	42.93	857.17	-91.79
	10-Year:	1,784.23		(518.96)	(86%)	1530.93	171.73	1702.66	-81.57
	100-Year:	2,754.84				2363.74	257.60	2621.34	-133.50
Point A2			531.11	70.91	13%				
	2-Year:	909.05		-----	-----	787.68	35.46	823.14	-85.91
	10-Year:	1,691.73		(460.20)	(87%)	1465.86	141.82	1607.68	-84.05
	100-Year:	2,566.59				2223.92	212.73	2436.65	-129.94
Point A3			332.76	11.62	3%				
	2-Year:	628.64		-----	-----	606.69	5.81	612.50	-16.14
	10-Year:	1,107.67		(321.14)	(97%)	1068.99	23.24	1092.23	-15.44
	100-Year:	1,595.56				1539.85	34.86	1574.71	-20.85
Point A4			171.86	59.29	34%				
	2-Year:	260.92		-----	-----	170.90	29.65	200.55	-60.37
	10-Year:	539.49		(112.57)	(66%)	353.37	118.58	471.95	-67.54
	100-Year:	906.56				593.80	177.87	771.67	-134.89
Point A5			144.60	40.76	28%				
	2-Year:	242.32		-----	-----	174.02	20.38	194.40	-47.92
	10-Year:	489.66		(103.85)	(72%)	351.65	81.51	433.16	-56.50
	100-Year:	799.19				573.94	122.27	696.21	-102.98
Point A6			13.45	9.21	68%				
	2-Year:	37.98		-----	-----	11.97	4.61	16.57	-21.41
	10-Year:	70.54		(4.24)	(32%)	22.23	18.42	40.65	-29.89
	100-Year:	113.19				35.67	27.63	63.30	-49.89
Point A7			96.23	0.00	0%				
	2-Year:	229.26		-----	-----	229.26	0.00	229.26	0.00

Location:	Return Event (Yr):	Existing Conditions Peak Flow Rate (cfs):	Total Area (ac):	Onsite ----- (Offsite) Area (ac):	Onsite ----- (Offsite) Area (%):	Allowable Bypass Flows (cfs):	Allowable Onsite Discharge Rate: (cfs):	Prop. Cond. ARR (cfs):	Difference in Flow Rate: (cfs):
	10-Year:	397.23		(96.23)	(100%)	397.23	0.00	397.23	0.00
	100-Year:	603.32				603.32	0.00	603.32	0.00
Point B1			308.34	0.00	0%				
	2-Year:	615.14		-----	-----	615.14	0.00	615.14	0.00
	10-Year:	1,076.61		(308.34)	(100%)	1076.61	0.00	1076.61	0.00
	100-Year:	1,551.65				1551.65	0.00	1551.65	0.00
Point D1			295.52	56.26	19%				
	2-Year:	560.43		-----	-----	453.75	28.13	481.87	-78.56
	10-Year:	941.67		(239.26)	(81%)	762.41	112.51	874.92	-66.75
	100-Year:	1351.30				1094.07	168.77	1262.83	-88.47
Point D2			229.46	35.36	15%				
	2-Year:	485.06		-----	-----	410.31	17.68	427.99	-57.07
	10-Year:	752.83		(194.10)	(85%)	636.82	70.72	707.54	-45.29
	100-Year:	1052.38				890.21	106.08	996.29	-56.09
Point E1			29.15	20.68	71%				
	2-Year:	65.98		-----	-----	19.17	10.34	29.51	-36.47
	10-Year:	106.88		(8.47)	(29%)	31.06	41.36	72.41	-34.47
	100-Year:	143.00				41.55	62.03	103.59	-39.41
Point F1			20.60	12.06	59%				
	2-Year:	47.57		-----	-----	19.72	6.03	25.75	-21.82
	10-Year:	74.89		(8.54)	(41%)	31.05	24.12	55.17	-19.72
	100-Year:	106.16				44.02	36.18	80.20	-25.96
Point F2			12.76	12.06	95%				
	2-Year:	31.56		-----	-----	1.72	6.03	7.75	-23.81
	10-Year:	45.26		(0.70)	(5%)	2.47	24.12	26.59	-18.67
	100-Year:	65.86				3.59	36.18	39.78	-26.08
Point G1			21.96	7.11	32%				
	2-Year:	58.32		-----	-----	39.43	3.56	42.99	-15.33
	10-Year:	102.21		(14.85)	(68%)	69.10	14.23	83.33	-18.88
	100-Year:	163.43				110.49	21.34	131.84	-31.59
Point G2			7.93	7.11	90%				
	2-Year:	23.29		-----	-----	2.39	3.56	5.94	-17.35
	10-Year:	36.98		(0.82)	(10%)	3.79	14.23	18.02	-18.96
	100-Year:	62.01				6.35	21.34	27.70	-34.31
Point H1			56.17	51.06	91%				
	2-Year:	119.54		-----	-----	10.88	25.53	36.41	-83.13
	10-Year:	240.90		(5.11)	(9%)	21.93	102.12	124.05	-116.85

Location:	Return Event (Yr):	Existing Conditions Peak Flow Rate (cfs):	Total Area (ac):	Onsite ----- (Offsite) Area (ac):	Onsite ----- (Offsite) Area (%):	Allowable Bypass Flows (cfs):	Allowable Onsite Discharge Rate: (cfs):	Prop. Cond. ARR (cfs):	Difference in Flow Rate: (cfs):
	100-Year:	406.43				37.00	153.18	190.18	-216.25
Point I1			1.40	0.61	44%				
	2-Year:	4.44		-----	-----	2.49	0.31	2.80	-1.64
	10-Year:	7.87		(0.78)	(56%)	4.42	1.23	5.64	-2.23
	100-Year:	12.29				6.90	1.84	8.74	-3.55

The greatest required relative reduction in peak discharge rate occurs at the Point A2 location. This is the anticipated location for the greatest required impact, as that point is located at the confluence of the two channels that convey stormwater runoff from the three subareas (Sub's A3, A4(E), & A5(E)) that include portions of the proposed Discovery Park development site.

When the northern Multi-Family Residential, Aria, Discovery Park Phase 2 and Discovery Park Future areas are to be developed it will need to be ensured that these allowable release rates are met. Detention basins will need to be provided to attenuate the peak flows generated from development, as needed. Those detention basins will be discussed in the future Micro-Studies to be prepared when each area is developed. Subareas D through I were analyzed to provide allowable release rates only for future development. These subareas will not be included in the proposed Micro- or Macro- analysis in this report.

4. MICRO ANALYSIS

This section of the Stormwater Drainage Study for the proposed development is provided to evaluate the Proposed Conditions hydrology for the project. As in the Existing Conditions analysis, the overall modeled stormwater drainage area for the Micro Study encompasses approximately 602 acres, including 74 acres within the proposed development site. Due to a diversion of approximately 0.20 acres from adjacent tributary areas, the overall drainage area is slightly increased to approximately 602.42 acres in the Macro Study. The following two sub-sections will provide the Proposed Conditions analysis for the Micro- and Macro- analyses. As previously stated, this section will analyze and review the final micro and macro conditions for subareas A,B, and C which includes the Aria, Discovery Park Phase 1, Discovery Park Phase 2, and the Multi-Family Residential developments. Subareas D through I are part of future Discovery Park developments, which will be further reviewed and analyzed with future Micro stormwater studies as those layouts and designs are further refined and submitted for city review and approval.

The purpose of this report is to provide the Macro and Micro Stormwater Drainage analysis to ensure that the proposed site development is compliant with City of Lee's Summit requirements. This section of the report will provide the Proposed Conditions analyses, representing the fully developed site hydrology for both Micro- and Macro-Study configurations. The results of this analysis will be compared to those of Existing Conditions in order to determine the hydrologic effects of the development upon the receiving stormwater conveyances and sub-watersheds. Compliance with the project ARR's will also be evaluated through comparison of the Actual-versus Allowable Release Rates at the five defined Points of Interest.

4.1. Micro Stormwater Model: Proposed Conditions Analysis

This section of the Stormwater Drainage Study for the Discovery Park Phase 1 commercial development is provided to evaluate the Micro-Study Proposed Conditions hydrology for the project. The micro conditions development totals 74.06-acres including 39.42-acres of commercial development (office buildings, hotels, restaurants, multifamily residential buildings, associated drives, parking, utilities) and mass grading of the Aria and Discovery Park Phase 2 sites totaling 34.64-acres. As previously discussed, the existing pond located at point A5 is being removed with the proposed development, however all other existing flow-attenuating structures and features will remain unchanged.

Proposed Conditions Runoff Curve Numbers have been developed based upon the current Development Plan for this commercial project. Refer to Section 7 for Micro-Study Proposed Conditions weighted NRCS Runoff Curve Number (CN) and Time of Concentration (T_c) calculations.

The tributary areas, Runoff Curve Numbers (CN), and Times of Concentration (T_c) for the corresponding subareas that are provided in Table 4.1-1 are used as input into the Proposed Conditions PondPack model to evaluate the Micro-stormwater hydrology. Subareas included in the Proposed Conditions for the Discovery Park property are utilized in the PondPack model to compute the stormwater runoff for the three design storms. The resultant peak discharge rate (Q_P), peak time (T_P), and runoff volume (V_R) for the computed hydrographs of modeled subareas are included in Table 4.1-2(a). Areas that are unchanged from the Existing Conditions analysis are presented as *grey print* in the following tables.

Table 4.1-1(a). Micro-Study Proposed Conditions Input Data

	Total Area (ac):	Aria Onsite Area (ac):	Discovery Park Phase 1 Onsite Area (ac):	Discovery Park Phase 2 Onsite Area (ac):	Macro- Study Onsite Area (ac):	NRCS Weighted CN:	T_c^1 (hr):
<i>Subarea A1(e):</i>	27.97				12.36	77	0.1736
<i>Subarea A1(w):</i>	43.14					76	0.2511
<i>Subarea A2(e):</i>	3.08					80	0.0875
<i>Subarea A2(w):</i>	23.41					73	0.2133
Subarea A3:	24.06	11.07			11.07	82	0.0714
Subarea A4(e):	1.48	0.14	0.84		0.98	88	0.0184
Subarea A4(w):	8.75		0.59		0.59	72	0.1095
Subarea A5(e):	19.12	11.29	6.92		18.20	86	0.0769
Subarea A5(w):	33.42		31.08		31.08	95	0.0647
Subarea A6:	13.42			9.88	9.88	83	0.0842
<i>Subarea A7:</i>	30.60					76	0.1596
<i>Subarea A8:</i>	21.56					81	0.1299
<i>Subarea A9:</i>	31.84					84	0.1420
<i>Subarea A10:</i>	12.23					94	0.1545
Subtotal:	294.08	22.50	39.42	9.88	84.15		
<i>Subarea B1:</i>	31.94					79	0.1385
<i>Subarea B2:</i>	48.54					78	0.1987
<i>Subarea B3:</i>	40.69					83	0.1447
<i>Subarea B4-DET:</i>	25.05					94	0.1155
<i>Subarea B4-BYP:</i>	7.42					94	0.0787
Subtotal:	153.64						
<i>Subarea C1:</i>	45.21					90	0.2860
<i>Subarea C2:</i>	57.13					81	0.3308
<i>Subarea C3:</i>	52.36					83	0.1199
Subtotal:	154.70						
Total:	602.42	22.50	39.42	9.88			

¹ **Note:** Per TR-55 documentation, minimum T_c is 0.10 hours; model is configured with default value of 0.10 hr. minimum T_c .

Table 4.1-1(b). Micro-Study Proposed Conditions Input Sub-Watershed Data

	Total Area (ac):	Aria Onsite Area (ac):	Discovery Park Phase 1 Onsite Area (ac):	Discovery Park Phase 2 Onsite Area (ac):	Macro- Study Onsite Area (ac):
Sub-W'Shed A Subtotal:	294.08	22.50	39.42	9.88	84.15
<i>Sub-W'Shed B Subtotal:</i>	<i>153.64</i>				
<i>Sub-W'Shed C Subtotal:</i>	<i>154.70</i>				
Modeled Total:	602.42	22.50	39.42	9.88	84.15

These tributary areas, Runoff Curve Numbers (CN), and Times of Concentration (T_C) for the corresponding subareas were used as input to the Proposed Conditions PondPack model to evaluate the stormwater hydrology for this Micro-Study. The subareas representing Existing- (offsite) and Proposed Conditions (onsite) for the Discovery Park site are utilized in the PondPack model to compute the stormwater runoff for the three design storms. The resultant peak discharge rate (Q_P), peak time (T_P), and runoff volume (V_R) for the computed hydrographs of modeled subareas are included in Table 4.1-2.

Table 4.1-2(a). Hydrologic Information – Micro-Study Proposed Conditions Summary (Subareas)

Location:	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
<i>Subarea A-1(e):</i>	53.91	12.01	3.334	110.32	11.99	6.783	187.82	11.99	11.675
<i>Subarea A-1(w):</i>	70.15	12.05	4.908	146.86	12.05	10.132	252.55	12.05	17.597
<i>Subarea A-2(e):</i>	7.64	11.94	0.420	14.90	11.93	0.819	24.63	11.93	1.374
<i>Subarea A-2(w):</i>	34.59	12.03	2.301	76.77	12.03	4.975	136.28	12.03	8.884
Subarea A-3:	65.13	11.93	3.574	122.96	11.93	6.787	199.22	11.93	11.198
Subarea A-4(e):	5.05	11.93	0.279	8.68	11.93	0.491	13.33	11.93	0.773
Subarea A-4(w):	14.08	11.96	0.817	32.12	11.94	1.796	58.07	11.94	3.239
Subarea A-5(e):	60.82	11.93	3.344	107.76	11.93	6.033	168.25	11.93	9.642
Subarea A-5(w):	139.52	11.92	8.184	219.80	11.92	13.246	322.03	11.92	19.806
Subarea A-6:	37.88	11.93	2.077	70.35	11.93	3.893	112.88	11.93	6.372
<i>Subarea A-7:</i>	57.26	12.00	3.482	119.03	11.98	7.186	204.90	11.98	12.481
<i>Subarea A-8:</i>	53.05	11.96	3.070	101.89	11.95	5.907	166.82	11.95	9.826
<i>Subarea A-9:</i>	87.15	11.97	5.138	158.95	11.96	9.507	252.78	11.96	15.435
<i>Subarea A-10:</i>	45.61	11.97	2.890	72.57	11.97	4.733	106.86	11.97	7.128
<i>Subarea B-1:</i>	70.94	11.97	4.168	140.29	11.96	8.242	234.20	11.95	13.944
<i>Subarea B-2:</i>	94.72	12.03	6.057	190.16	12.01	12.146	320.74	12.00	20.725
<i>Subarea B-3:</i>	106.50	11.97	6.302	197.62	11.96	11.812	317.37	11.96	19.332
<i>Subarea B-4 (BYP):</i>	30.29	11.93	1.753	48.22	11.92	2.871	71.04	11.92	4.324

Location:	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Subarea B-4 (DET):	99.43	11.94	5.919	158.21	11.94	9.694	232.94	11.94	14.599
Subarea C-1:	124.83	12.05	9.223	209.38	12.05	15.842	317.10	12.05	24.565
Subarea C-2:	104.44	12.09	8.134	200.90	12.08	15.652	329.78	12.08	26.038
Subarea C-3:	142.61	11.95	8.110	264.83	11.94	15.199	425.51	11.94	24.875

The hydrologic impact resulting from the Micro-Study Proposed Conditions results is partially revealed by the comparison of the preceding subarea data to those established in Section 3 (Existing Conditions) of this report. The hydrologic conditions for offsite subareas remain unchanged between Existing- and Proposed Conditions analyses. However, the onsite subareas do exhibit changes in impervious cover and diversion of tributary area under Proposed Conditions. The hydrologic impact to the overall sub-watershed of these modified areas will become apparent in the summary tables for the Points of Interest.

The variation in quantity and rate of stormwater discharge between these two models represents the hydrologic effect generated by the proposed development project for given subareas. The following table depicts the difference in computed subarea hydrology between the Existing- and Proposed Conditions models.

**Table 4.1-2(b). Hydrologic Information –
Micro-Study Proposed vs. Existing Conditions Summary (Subareas)**

Location:	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Subarea A-1(e):	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea A-1(w):	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea A-2(e):	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea A-2(w):	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea A-3:	7.52	-0.03	0.244	10.38	-0.02	0.292	13.09	-0.02	0.303
Subarea A-4(e):	-18.92	-0.03	-1.123	-37.29	-0.03	-2.207	-61.82	-0.03	-3.715
Subarea A-4(w):	-13.48	-0.05	-0.895	-28.73	-0.05	-1.904	-50.15	-0.05	-3.368
Subarea A-5(e):	24.42	-0.02	1.277	36.86	-0.02	2.002	51.27	-0.02	2.88
Subarea A-5(w):	102.50	-0.08	5.935	142.70	-0.07	8.603	189.42	-0.05	11.742
Subarea A-6:	-0.10	n/c	-0.006	-0.19	n/c	-0.011	-0.31	n/c	-0.017
Subarea A-7:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea A-8:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea A-9:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea A-10:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea B-1:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea B-2:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea B-3:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c

Location:	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Subarea B-4 (BYP):	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea B-4 (DET):	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea C-1:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea C-2:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Subarea C-3:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c

The sign-convention used in the preceding table is: Proposed Conditions minus Existing Conditions. Therefore, negative values indicate a reduction in computed value, whereas positive values reflect an increase. Those locations that do not exhibit any change in computed values are identified by "n/c."

The previously defined stormwater detention basins, outfall structures, and resultant hydrograph attenuation effects have been incorporated into the Proposed Conditions PondPack model. The channel reaches defined in the Existing Conditions analysis are also included in this routing. Results of this Proposed Conditions hydrologic routing are provided in the following summary table. This information is provided at the same "Points of Interest" used in the Existing Conditions hydrologic analysis for this Stormwater Drainage Study. Refer to the Proposed Conditions Drainage Area Map for the location of the points of interest in relation to the development site, modeled subareas, channels and conveyances. As in the preceding tables, locations that are unchanged from the Existing Conditions analysis are presented as grey print in the following tables.

Table 4.1-3(a). Hydrologic Information–Micro-Study Proposed Conditions Summary (Junctions)

Un-Detained Hydrology (for Proposed Discovery Park Basin)

Location:	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Point A1:	1,036.29	12.19	91.367	1,891.07	12.16	171.623	2,890.34	12.14	281.712
Point A2:	979.74	12.11	83.974	1,759.80	12.09	155.560	2,641.51	12.08	253.290
Point A3:	625.11	12.16	52.191	1,100.39	12.15	97.194	1,586.54	12.08	158.549
Point A4:	414.66	12.03	29.282	697.74	12.03	52.792	1,047.68	12.03	84.703
Point A6:	37.88	11.94	2.077	70.35	11.93	3.893	112.88	11.93	6.372
Point A7:	229.26	12.05	14.580	397.23	12.07	27.333	603.32	12.08	44.871
Point A8:	53.05	11.96	3.070	101.89	11.95	5.907	166.82	11.95	9.826
Point A9:	128.79	12.00	8.028	223.48	11.99	14.240	346.07	11.99	22.563
Point A10:	45.61	11.97	2.890	72.57	11.97	4.733	106.86	11.97	7.128
Point B1:	615.14	12.11	49.667	1,076.61	12.11	91.457	1,551.65	12.14	148.401
Point B2:	242.31	12.05	20.032	446.43	12.04	36.522	715.56	12.03	58.980
Point B3:	153.04	11.99	13.975	266.04	11.98	24.377	411.65	11.97	38.255
Point B4:	48.10	11.94	7.673	71.04	11.93	12.565	98.80	11.93	18.924
Point C1:	350.42	12.09	25.466	604.01	12.10	46.692	890.10	12.10	75.478

Location:	Q _{P-2} (cfs):	T _{P-2} (hr.):	V _{R-2} (ac-ft):	Q _{P-10} (cfs):	T _{P-10} (hr.):	V _{R-10} (ac-ft):	Q _{P-100} (cfs):	T _{P-100} (hr.):	V _{R-100} (ac-ft):
Point C2:	232.89	12.04	16.244	417.32	12.07	30.851	614.91	12.08	50.912
Point C3:	142.61	11.95	8.110	264.83	11.94	15.199	425.51	11.94	24.875

As previously evaluated for the modeled Subareas, these Proposed Conditions results for the Points of Interest will be compared to those established in Section 3 (Existing Conditions) of this report. The points represent similar locations in both Existing- and Proposed Conditions analyses. Therefore, the variation in quantity and rate of stormwater discharge between these two models represents the hydrologic effect generated by the proposed development. The following table depicts the difference between the Existing- and Proposed Conditions models.

**Table 4.1-3(b). Hydrologic Information – Comparison of Results:
Micro- Proposed vs. Existing Conditions Summary (Junctions) Un-Detained Hydrology**

Location:	Q _{P-2} (cfs):	T _{P-2} (hr.):	V _{R-2} (ac-ft):	Q _{P-10} (cfs):	T _{P-10} (hr.):	V _{R-10} (ac-ft):	Q _{P-100} (cfs):	T _{P-100} (hr.):	V _{R-100} (ac-ft):
Point A1:	84.85	-0.06	3.32	102.45	-0.06	4.660	130.62	-0.05	5.706
Point A2:	68.10	-0.06	4.173	62.88	-0.06	5.511	66.98	-0.04	6.556
Point A3:	-5.18	0.01	-0.821	-10.28	n/c	-0.777	-12.83	-0.09	-0.796
Point A4:	152.42	-0.13	5.214	156.66	-0.09	6.518	140.21	-0.06	7.572
Point A6:	0.88	-0.03	-0.813	0.78	n/c	0.011	1.10	n/c	0.018
Point A7:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point A8:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point A9:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point A10:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B1:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B2:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B3:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B4:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point C1:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point C2:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point C3:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c

The sign-convention used in the preceding table is: Proposed Conditions minus Existing Conditions. Therefore, negative values indicate a reduction in computed value, whereas positive values reflect an increase. Those locations that do not exhibit any change in computed values are identified by "n/c."

Information presented in the preceding table exhibits a reduction in peak rates, relative to the existing conditions model. This is the result of reduced times of concentration that occur with Discovery Park development. Overall runoff volumes exhibit modest increase.

The following table is provided to examine the difference between the computed peak discharge rates and the established ARR's for the Discovery Park Micro-Study.

**Table 4.1-4. Allowable Release Rate Evaluation –
Proposed Conditions Micro Model – Without Detention**

Location:	Event:	Computed Peak Discharge Rate (cfs):	ARR (cfs):	Difference (cfs):
Point A1	2-Year:	1,036.29	927.14	109.15
	10-Year:	1,891.07	1,766.79	124.28
	100-Year:	2,890.34	2,724.11	166.23
Point A2	2-Year:	979.74	884.27	95.57
	10-Year:	1,759.80	1,670.03	89.77
	100-Year:	2,641.51	2,532.96	108.55
Point A3	2-Year:	625.11	613.94	11.17
	10-Year:	1,100.39	1,094.99	5.40
	100-Year:	1,586.54	1,578.20	8.34
Point A4	2-Year:	414.66	251.18	163.48
	10-Year:	697.74	528.69	169.05
	100-Year:	1,047.68	882.87	164.81
Point A6	2-Year:	37.88	16.57	21.31
	10-Year:	70.35	40.65	29.70
	100-Year:	112.88	63.30	49.58

The sign-convention utilized for the information presented in the preceding table is based upon “Computed Rate minus ARR.” Therefore, positive values indicate an exceedance of the Allowable Release Rate, whereas negative values would indicate a peak rate that is lower than the ARR, thereby meeting the targeted peak discharge rates. The difference in the rates depicted in this table are all in exceedance of the targeted rates, therefore stormwater detention is required for the project.

Proposed Discovery Park Stormwater Detention Facility:

Based upon the Proposed Conditions analysis, it is apparent that some form of stormwater detention will be required for this project. A wet detention basin will be constructed in the northwest corner of Discovery Park Phase 1 that will serve both Discovery Park and Aria. The basin is proposed to be constructed as a Water-Quality facility, with the intent of allowing the Discovery Park and future Aria site to meet their obligations for 40-hour extended detention upon development of both sites. Following is the Proposed Conditions geometry for the outlet structure:

Primary Outlet Structure:

48" Dia. RCP Culvert; Length \approx 200 L.F.

Flowline In / Out \approx 908.00 / 918.00(ft; NAVD)

Water Quality Treatment Orifice: 7" Dia.; Invert Elev. = 929.00

Includes Inlet Protection Grate

Concrete – Riser Pipe: 48" Dia. Inside,

Open top with Inlet Protection Grate

Opening crest elevation = 933.20

Secondary Outlet Structure:

Emergency Spillway (Weir) 250' L x 2.0' H with 3:1 side slopes; Crest Elev. = 936.50.

The proposed basin's storage area is provided in the following table. This proposed storage area is designed as the final configuration for the facility; it is intended to provide the necessary detention and 40-hour extended detention for the three separate development areas.

**Table 4.1-5(a). Proposed Conditions Storage Volume –
Discovery Park Stormwater Detention Facility**

Elevation (ft; NAVD):	Storage Area (sq. ft.):	Incremental Volume (ft³):	Cumulative Volume (ac-ft):	Cumulative Volume (Dry) (ac-ft):
920	89,372.00	0.00	0.00	0.00
921	94,134.00	91,742.70	2.11	0.00
922	98,954.00	96,533.97	4.32	0.00
923	103,829.00	101,381.73	6.65	0.00
924	108,761.00	106,285.46	9.09	0.00
925	113,750.00	111,246.18	11.64	0.00
926	118,795.00	116,263.38	14.31	0.00
927	123,897.00	121,337.06	17.10	0.00
928	129,055.00	126,467.23	20.00	0.00
929	134,270.00	131,653.89	23.02	0.00
930	140,150.00	137,199.50	26.17	3.15
931	145,315.00	142,724.71	29.45	6.43
932	150,465.00	147,882.53	32.84	9.83
933	155,331.00	152,891.55	36.35	13.34
934	160,582.00	157,949.23	39.98	16.96
935	166,505.00	163,534.56	43.73	20.72
936	170,375.00	168,436.30	47.60	24.58
937	189,499.00	179,852.25	51.73	28.71
938	209,384.00	199,358.84	56.31	33.29

Note: The conic method is used to compute the incremental volume between pond contours.

The sum of the computed incremental volumes provides the cumulative pond volume for a given elevation.

Utilizing these geometric parameters in the Proposed Conditions model, the following table depicts the stormwater routing summary for this location.

**Table 4.1-5(b). Micro-Study Proposed Conditions Hydrologic Routing Summary–
Discovery Park Stormwater Detention Facility**

	Peak Q In (cfs):	T _P In (hr.):	Peak Q Out (cfs):	T _P Out (hr.):	V _R (ac-ft):	Peak WSEL (ft; NAVD):	Max. Storage Volume (ac-ft):
WQv:	62.19	11.94	0.97	19.87	2.782	929.86	2.698
2-Year:	256.14	11.93	2.38	24.03	9.927	932.71	12.305
10-Year:	437.13	11.93	24.49	13.00	19.392	933.85	16.394
100-Year:	672.59	11.93	95.76	12.24	33.681	935.54	22.799

The preceding routing summary indicates that the extended detention aspect of this facility provides substantial attenuation of the all storm events. In an effort to retain the 40-hour (min.) draw-down for Water Quality treatment, yet reduce excessive long-term ponding, a secondary weir is provided to handle the 2, 10 and 100 year storms.

The proposed stormwater detention facility is included in the PondPack modeling for the following table output. These Micro-Study Proposed Conditions results will be compared to the Existing Conditions in the following table. Areas that are unchanged from the Existing Conditions analysis are presented as *grey print* in the following tables.

**Table 4.1-6(a). Hydrologic Information – Micro- Proposed Conditions Summary
(Junctions) Including Proposed Discovery Park Detention Basin**

Location:	Q _{P-2} (cfs):	T _{P-2} (hr.):	V _{R-2} (ac-ft):	Q _{P-10} (cfs):	T _{P-10} (hr.):	V _{R-10} (ac-ft):	Q _{P-100} (cfs):	T _{P-100} (hr.):	V _{R-100} (ac-ft):
Point A1:	914.02	12.23	86.576	1,635.15	12.23	165.539	2,502.62	12.22	275.542
Point A2:	871.69	12.16	79.193	1,545.48	12.16	149.486	2,350.92	12.17	247.132
Point A3:	625.11	12.16	52.191	1,100.39	12.15	97.194	1,586.54	12.17	158.549
Point A4:	229.99	12.11	24.504	400.59	12.13	46.722	695.13	12.14	78.549
Point A6:	37.88	11.93	2.077	70.35	11.93	3.893	112.88	11.93	6.372
<i>Point A7:</i>	<i>229.26</i>	<i>12.05</i>	<i>14.580</i>	<i>397.23</i>	<i>12.07</i>	<i>27.333</i>	<i>603.32</i>	<i>12.08</i>	<i>44.871</i>
<i>Point A8:</i>	<i>53.05</i>	<i>11.96</i>	<i>3.070</i>	<i>101.89</i>	<i>11.95</i>	<i>5.907</i>	<i>166.82</i>	<i>11.95</i>	<i>9.826</i>
<i>Point A9:</i>	<i>128.79</i>	<i>12.00</i>	<i>8.028</i>	<i>223.48</i>	<i>11.99</i>	<i>14.240</i>	<i>346.07</i>	<i>11.99</i>	<i>22.563</i>
<i>Point A10:</i>	<i>45.61</i>	<i>11.97</i>	<i>2.890</i>	<i>72.57</i>	<i>11.97</i>	<i>4.733</i>	<i>106.86</i>	<i>11.97</i>	<i>7.128</i>
<i>Point B1:</i>	<i>615.14</i>	<i>12.11</i>	<i>49.667</i>	<i>1,076.61</i>	<i>12.11</i>	<i>91.457</i>	<i>1,551.65</i>	<i>12.14</i>	<i>148.401</i>
<i>Point B2:</i>	<i>242.31</i>	<i>12.05</i>	<i>20.032</i>	<i>446.43</i>	<i>12.04</i>	<i>36.522</i>	<i>715.56</i>	<i>12.03</i>	<i>58.980</i>
<i>Point B3:</i>	<i>153.04</i>	<i>11.99</i>	<i>13.975</i>	<i>266.04</i>	<i>11.98</i>	<i>24.377</i>	<i>411.65</i>	<i>11.97</i>	<i>38.255</i>
<i>Point B4:</i>	<i>48.10</i>	<i>11.94</i>	<i>7.673</i>	<i>71.04</i>	<i>11.93</i>	<i>12.565</i>	<i>98.80</i>	<i>11.93</i>	<i>18.924</i>

Location:	Q _{P-2} (cfs):	T _{P-2} (hr.):	V _{R-2} (ac-ft):	Q _{P-10} (cfs):	T _{P-10} (hr.):	V _{R-10} (ac-ft):	Q _{P-100} (cfs):	T _{P-100} (hr.):	V _{R-100} (ac-ft):
Point C1:	350.42	12.09	25.466	604.01	12.10	46.692	890.10	12.10	75.478
Point C2:	232.89	12.04	16.244	417.32	12.07	30.851	614.91	12.08	50.912
Point C3:	142.61	11.95	8.110	264.83	11.94	15.199	425.51	11.94	24.875

The following table provides a comparison between the Micro-Study Proposed Conditions (with detention) and the Existing Conditions modeled results. As indicated by the grey font, the lowest six modeled points within the sub-watershed are the only locations that exhibit hydrologic impacts.

**Table 4.1-6(b). Hydrologic Information – Comparison of Results:
Micro- Proposed vs. Existing Conditions Summary (Junctions) Detained Hydrology**

Location:	Q _{P-2} (cfs):	T _{P-2} (hr.):	V _{R-2} (ac-ft):	Q _{P-10} (cfs):	T _{P-10} (hr.):	V _{R-10} (ac-ft):	Q _{P-100} (cfs):	T _{P-100} (hr.):	V _{R-100} (ac-ft):
Point A1:	-34.94	-0.02	-1.477	-149.08	0.01	-1.430	-252.22	0.03	-0.466
Point A2:	-37.36	-0.01	-0.617	-146.25	0.01	-0.569	-215.67	0.04	0.396
Point A3:	-3.53	0.01	-0.806	-7.28	n/c	-0.758	-9.02	n/c	-0.748
Point A4:	-30.93	-0.04	0.412	-138.90	0.02	0.413	-211.43	0.05	1.367
Point A6:	-0.10	n/c	-0.006	-0.19	n/c	-0.011	-0.31	n/c	-0.017
Point A7:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point A8:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point A9:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point A10:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B1:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B2:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B3:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B4:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point C1:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point C2:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point C3:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c

The sign-convention used in the preceding table is: Proposed Conditions minus Existing Conditions. Therefore, negative values indicate a reduction in computed value, whereas positive values reflect an increase. Those locations that do not exhibit any change in computed values are identified by "n/c."

Information presented in the preceding table exhibits a reduction in peak rates, relative to the existing conditions model. This is the result of reduced times of concentration that occur with Discovery Park development. Overall runoff volumes exhibit modest increase.

The following table is provided to examine the difference between the computed peak discharge rates and the established ARR's for the Discovery Park Micro-Study.

**Table 4.1-7. Allowable Release Rate Evaluation –
Proposed Conditions Micro Study (With Detention)**

Location:	Event:	Computed Peak Discharge Rate (cfs):	ARR (cfs):	Difference (cfs):
Point A1	2-Year:	914.02	859.38	54.64
	10-Year:	1635.15	1704.07	-68.92
	100-Year:	2502.62	2623.74	-121.12
Point A2	2-Year:	871.69	823.14	48.55
	10-Year:	1545.48	1607.68	-62.20
	100-Year:	2350.92	2436.65	-85.73
Point A3	2-Year:	625.11	612.50	12.61
	10-Year:	1100.39	1092.23	8.16
	100-Year:	1586.54	1574.71	11.83
Point A4	2-Year:	229.99	200.55	29.44
	10-Year:	400.59	471.95	-71.36
	100-Year:	695.13	771.67	-76.54
Point A6	2-Year:	37.88	16.57	21.31
	10-Year:	70.35	40.65	29.70
	100-Year:	112.88	63.30	49.58

Note: The sign-convention utilized for the information presented in the preceding table is based upon "Computed Rate minus ARR." Positive values indicate an exceedance of the ARR; negative values indicate that the peak rate is lower than the ARR.

The difference in the rates depicted in this table are generally in compliance with the targeted rates; the proposed stormwater management facility provides adequate attenuation for the primary Channel "A" tributary. The information provided in the preceding table does demonstrate that there are a few exceptions: 2 year peak rates at all points, and additional peak rates at Point A3 and A6. Point A3 is the terminal end of the Channel "B" tributary, immediately upstream from confluence at Point A2, while point A6 flows directly into the proposed basin.

Although the ARR is exceeded at the Point A3 location, the Proposed Conditions analysis does exhibit a reduction in peak discharge rate relative to Existing Conditions (*flows reductions are: -3.05 cfs, -9.18 cfs, and -13.18 cfs for the 2-, 10-, and 100-year events, respectively*). The ARR is also exceeded at Point A6 during all storm events, however this point is located upstream of the proposed detention basin, and is routed through the basin for attenuation.

4.2. Micro Stormwater Model: Fully-Developed Conditions Analysis

This section of the Stormwater Drainage Study for the Discovery Park commercial development is provided to evaluate the Fully-Developed Proposed Conditions hydrology for the Unity Lake #2 tributary. Proposed conditions include the fully-developed condition of Aria, Discovery Park, and the Multi-Family Residential developments.

Fully-Developed Conditions Runoff Curve Numbers have been developed based upon the current Aria and Discovery Park Phase 1 Development Plans and for the future Multi-Family Residential development and a portion of the Discovery Park Phase 2 development. Refer to Section 7 for Micro-Study Fully-Developed Conditions weighted NRCS Runoff Curve Number (CN) and Time of Concentration (T_C) calculations. These values, summarized in Table 4.2-1(a) are used as input into the Fully-Developed Conditions PondPack model to evaluate the Micro-Study Fully Developed Conditions stormwater runoff for the three design storms. The resultant peak discharge rate (Q_P), peak time (T_P), and runoff volume (V_R) for the computed hydrographs of modeled subareas are included in Tale 4.2-2(a). Areas that are unchanged from the Existing Conditions analysis are presented as *grey print* in the following tables.

Table 4.2-1(a). Micro-Study Fully-Developed Conditions Input Data

	Total Area (ac):	Aria Onsite Area (ac):	Discovery Park Phase 1 Onsite Area (ac):	Discovery Park Phase 2 Onsite Area (ac):	Macro-Study Onsite Area (ac):	NRCS Weighted CN:	T_C ¹ (hr):
Subarea A1(e):	30.57				14.96	85	0.1401
<i>Subarea A1(w):</i>	<i>43.14</i>					76	<i>0.2511</i>
<i>Subarea A2(e):</i>	<i>3.08</i>					80	<i>0.0875</i>
<i>Subarea A2(w):</i>	<i>23.41</i>					73	<i>0.2133</i>
Subarea A3:	24.06	11.07			11.07	87	0.0717
Subarea A4(e):	1.48	0.14	0.84		0.98	89	0.0184
Subarea A4(w):	8.75		0.59		0.59	72	0.1095
Subarea A5(e):	19.12	11.29	6.92		18.20	92	0.0615
Subarea A5(w):	33.42		31.08		31.08	95	0.0647
Subarea A6:	13.42			9.88	9.88	94	0.0842
<i>Subarea A7:</i>	<i>30.60</i>					76	<i>0.1596</i>
<i>Subarea A8:</i>	<i>21.56</i>					81	<i>0.1299</i>
<i>Subarea A9:</i>	<i>31.84</i>					84	<i>0.1420</i>
<i>Subarea A10:</i>	<i>12.23</i>					94	<i>0.1545</i>
Subtotal:	296.68	22.50	39.42	9.88	86.76		
<i>Subarea B1:</i>	<i>31.94</i>					79	<i>0.1385</i>
<i>Subarea B2:</i>	<i>48.54</i>					78	<i>0.1987</i>

	Total Area (ac):	Aria Onsite Area (ac):	Discovery Park Phase 1 Onsite Area (ac):	Discovery Park Phase 2 Onsite Area (ac):	Macro- Study Onsite Area (ac):	NRCS Weighted CN:	T_C¹ (hr):
<i>Subarea B3:</i>	40.69					83	0.1447
<i>Subarea B4-DET:</i>	25.05					94	0.1155
<i>Subarea B4-BYP:</i>	7.42					94	0.0787
Subtotal:	153.64						
<i>Subarea C1:</i>	45.21					90	0.2860
<i>Subarea C2:</i>	57.13					81	0.3308
<i>Subarea C3:</i>	52.36					83	0.1199
Subtotal:	154.70						
Modeled Total:	605.02	22.50	39.42	9.88	86.76		

¹ **Note:** Per TR-55 documentation, minimum T_C is 0.10 hours; model is configured with default value of 0.10 hr. minimum T_C.

Table 4.2-1(b). Micro-Study Fully-Developed Conditions Input Sub-Watershed Data

	Total Area (ac):	Aria Onsite Area (ac):	Discovery Park Phase 1 Onsite Area (ac):	Discovery Park Phase 2 Onsite Area (ac):	Macro- Study Onsite Area (ac):
Sub-W'Shed A Subtotal:	296.68	22.50	39.42	9.88	86.76
<i>Sub-W'Shed B Subtotal:</i>	153.64				
<i>Sub-W'Shed C Subtotal:</i>	154.70				
Modeled Total:	605.02	22.50	39.42	9.88	86.76

As noted, these tributary areas, Runoff Curve Numbers (CN), and Times of Concentration (T_C) for the corresponding subareas were used as input to the Proposed Conditions PondPack model to evaluate the stormwater hydrology for the Fully-Developed section of this report. The subareas representing Existing- (offsite) and Proposed Conditions (onsite) for the development sites are utilized in the PondPack model to compute the stormwater runoff for the three design storms. The resultant peak discharge rate (Q_P), peak time (T_P), and runoff volume (V_R) for the computed hydrographs of modeled subareas are included in Table 4.2-2(a).

Table 4.2-2(a). Hydrologic Information – Micro- Fully-Developed Conditions Summary (Subareas)

Location:	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Subarea A-1(e):	87.32	11.96	5.137	157.07	11.96	9.384	247.45	11.96	15.116
<i>Subarea A-1(w):</i>	<i>70.15</i>	<i>12.05</i>	<i>4.908</i>	<i>146.86</i>	<i>12.05</i>	<i>10.132</i>	<i>252.55</i>	<i>12.05</i>	<i>17.597</i>
<i>Subarea A-2(e):</i>	<i>7.64</i>	<i>11.94</i>	<i>0.420</i>	<i>14.90</i>	<i>11.93</i>	<i>0.819</i>	<i>24.63</i>	<i>11.93</i>	<i>1.374</i>
<i>Subarea A-2(w):</i>	<i>34.59</i>	<i>12.03</i>	<i>2.301</i>	<i>76.77</i>	<i>12.03</i>	<i>4.975</i>	<i>136.28</i>	<i>12.03</i>	<i>8.884</i>
Subarea A-3:	79.39	11.93	4.376	138.56	11.93	7.797	214.51	11.93	12.366
Subarea A-4(e):	5.22	11.93	0.290	8.85	11.93	0.504	13.49	11.92	0.788
Subarea A-4(w):	14.08	11.96	0.817	32.12	11.94	1.796	58.07	11.94	3.239
Subarea A-5(e):	74.12	11.93	4.202	120.76	11.93	7.047	180.16	11.92	10.767
Subarea A-5(w):	139.52	11.92	8.184	219.79	11.92	13.245	322.02	11.92	19.806
Subarea A-6:	54.75	11.93	3.169	87.16	11.92	5.190	128.41	11.92	7.817
<i>Subarea A-7:</i>	<i>57.26</i>	<i>12.00</i>	<i>3.482</i>	<i>119.03</i>	<i>11.98</i>	<i>7.186</i>	<i>204.90</i>	<i>11.98</i>	<i>12.481</i>
<i>Subarea A-8:</i>	<i>53.05</i>	<i>11.96</i>	<i>3.070</i>	<i>101.89</i>	<i>11.95</i>	<i>5.907</i>	<i>166.82</i>	<i>11.95</i>	<i>9.826</i>
<i>Subarea A-9:</i>	<i>87.15</i>	<i>11.97</i>	<i>5.138</i>	<i>158.95</i>	<i>11.96</i>	<i>9.507</i>	<i>252.78</i>	<i>11.96</i>	<i>15.435</i>
<i>Subarea A-10:</i>	<i>45.61</i>	<i>11.97</i>	<i>2.890</i>	<i>72.57</i>	<i>11.97</i>	<i>4.733</i>	<i>106.86</i>	<i>11.97</i>	<i>7.128</i>
<i>Subarea B-1:</i>	<i>70.94</i>	<i>11.97</i>	<i>4.168</i>	<i>140.29</i>	<i>11.96</i>	<i>8.242</i>	<i>234.20</i>	<i>11.95</i>	<i>13.944</i>
<i>Subarea B-2:</i>	<i>94.72</i>	<i>12.03</i>	<i>6.057</i>	<i>190.16</i>	<i>12.01</i>	<i>12.146</i>	<i>320.74</i>	<i>12.00</i>	<i>20.725</i>
<i>Subarea B-3:</i>	<i>106.50</i>	<i>11.97</i>	<i>6.302</i>	<i>197.62</i>	<i>11.96</i>	<i>11.812</i>	<i>317.37</i>	<i>11.96</i>	<i>19.332</i>
<i>Subarea B-4 (BYP):</i>	<i>30.29</i>	<i>11.93</i>	<i>1.753</i>	<i>48.22</i>	<i>11.92</i>	<i>2.871</i>	<i>71.04</i>	<i>11.92</i>	<i>4.324</i>
<i>Subarea B-4 (DET):</i>	<i>99.43</i>	<i>11.94</i>	<i>5.919</i>	<i>158.21</i>	<i>11.94</i>	<i>9.694</i>	<i>232.94</i>	<i>11.94</i>	<i>14.599</i>
<i>Subarea C-1:</i>	<i>124.83</i>	<i>12.05</i>	<i>9.223</i>	<i>209.38</i>	<i>12.05</i>	<i>15.842</i>	<i>317.10</i>	<i>12.05</i>	<i>24.565</i>
<i>Subarea C-2:</i>	<i>104.44</i>	<i>12.09</i>	<i>8.134</i>	<i>200.90</i>	<i>12.08</i>	<i>15.652</i>	<i>329.78</i>	<i>12.08</i>	<i>26.038</i>
<i>Subarea C-3:</i>	<i>142.61</i>	<i>11.95</i>	<i>8.110</i>	<i>264.83</i>	<i>11.94</i>	<i>15.199</i>	<i>425.51</i>	<i>11.94</i>	<i>24.875</i>

As in the previous section, the hydrologic impact resulting from the Fully-Developed Conditions results is partially revealed by the comparison of the preceding subarea data to those established in Section 3 (Existing Conditions) of this report. The hydrologic conditions for offsite subareas remain unchanged between Existing- and Proposed Conditions analyses. Onsite subareas do exhibit changes in impervious cover and diversion of tributary area under Fully-Developed Conditions. The hydrologic impact to the overall sub-watershed of these modified areas will become apparent in the summary tables for the Points of Interest.

The variation in quantity and rate of stormwater discharge between these two models represents the hydrologic effect generated by the proposed development project for given subareas. The following table depicts the difference in computed subarea hydrology between the Existing- and Fully-Developed Conditions models.

Table 4.2-2(b). Hydrologic Information – Micro- Fully-Developed Conditions Summary (Subareas)

Location:	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Subarea A-1(e):	33.41	-0.05	1.803	46.75	-0.03	2.601	59.63	-0.03	3.441
<i>Subarea A-1(w):</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea A-2(e):</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea A-2(w):</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
Subarea A-3:	21.78	-0.03	1.046	25.98	-0.02	1.302	28.38	-0.02	1.471
Subarea A-4(e):	-18.75	-0.03	-1.112	-37.12	-0.03	-2.194	-61.66	-0.04	-3.7
Subarea A-4(w):	-13.48	-0.05	-0.895	-28.73	-0.05	-1.904	-50.15	-0.05	-3.368
Subarea A-5(e):	37.72	-0.02	2.135	49.86	-0.02	3.016	63.18	-0.03	4.005
Subarea A-5(w):	102.50	-0.08	5.935	142.69	-0.07	8.602	189.41	-0.05	11.742
Subarea A-6:	16.77	<i>n/c</i>	1.086	16.62	-0.01	1.286	15.22	-0.01	1.428
<i>Subarea A-7:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea A-8:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea A-9:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea A-10:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea B-1:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea B-2:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea B-3:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea B-4 (BYP):</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea B-4 (DET):</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea C-1:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea C-2:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Subarea C-3:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>

The sign-convention used in the preceding table is: Proposed Conditions minus Existing Conditions. Therefore, negative values indicate a reduction in computed value, whereas positive values reflect an increase. Those locations that do not exhibit any change in computed values are identified by "n/c."

The previously defined stormwater detention basins, outfall structures and resultant hydrograph attenuation effects have been incorporated into the Fully-Developed Conditions PondPack model. The channel reaches defined in the Existing Conditions analysis are also included in this routing. Results of this Fully-Developed Conditions hydrologic routing are provided in the following summary table. This information is provided at the same "Points of Interest" used in the Existing Conditions hydrologic analysis for this Stormwater Drainage Study. Refer to the Micro- Fully-Developed Drainage Area Map for the location of the points of interest in relation to the development sites, modeled subareas, channels and conveyances. As in the preceding tables, locations that are unchanged from the Existing Conditions analysis are presented as *grey print* in the following tables.

Table 4.2-3(a). Hydrologic Information – Micro- Fully-Developed Conditions Summary (Junctions)**Un-Detained Hydrology (for Proposed Discovery Park Basin)**

Location:	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Point A1:	1,074.58	12.17	95.932	1,932.54	12.16	177.559	2,939.19	12.14	288.906
Point A2:	1,005.57	12.10	86.736	1,784.23	12.09	158.893	2,665.36	12.08	257.043
Point A3:	627.78	12.15	52.992	1,103.02	12.15	98.204	1,588.55	12.17	159.717
Point A4:	436.65	12.03	31.243	718.69	12.03	55.115	1,066.26	12.03	87.287
Point A6:	54.75	11.93	3.169	87.16	11.92	5.190	128.41	11.92	7.817
<i>Point A7:</i>	<i>229.26</i>	<i>12.05</i>	<i>14.580</i>	<i>397.23</i>	<i>12.07</i>	<i>27.333</i>	<i>603.32</i>	<i>12.08</i>	<i>44.871</i>
<i>Point A8:</i>	<i>53.05</i>	<i>11.96</i>	<i>3.070</i>	<i>101.89</i>	<i>11.95</i>	<i>5.907</i>	<i>166.82</i>	<i>11.95</i>	<i>9.826</i>
<i>Point A9:</i>	<i>128.79</i>	<i>12.00</i>	<i>8.028</i>	<i>223.48</i>	<i>11.99</i>	<i>14.240</i>	<i>346.07</i>	<i>11.99</i>	<i>22.563</i>
<i>Point A10:</i>	<i>45.61</i>	<i>11.97</i>	<i>2.890</i>	<i>72.57</i>	<i>11.97</i>	<i>4.733</i>	<i>106.86</i>	<i>11.97</i>	<i>7.128</i>
<i>Point B1:</i>	<i>615.14</i>	<i>12.11</i>	<i>49.667</i>	<i>1,076.61</i>	<i>12.11</i>	<i>91.457</i>	<i>1,551.65</i>	<i>12.14</i>	<i>148.401</i>
<i>Point B2:</i>	<i>242.31</i>	<i>12.05</i>	<i>20.032</i>	<i>446.43</i>	<i>12.04</i>	<i>36.522</i>	<i>715.56</i>	<i>12.03</i>	<i>58.980</i>
<i>Point B3:</i>	<i>153.04</i>	<i>11.99</i>	<i>13.975</i>	<i>266.04</i>	<i>11.98</i>	<i>24.377</i>	<i>411.65</i>	<i>11.97</i>	<i>38.255</i>
<i>Point B4:</i>	<i>48.10</i>	<i>11.94</i>	<i>7.673</i>	<i>71.04</i>	<i>11.93</i>	<i>12.565</i>	<i>98.80</i>	<i>11.93</i>	<i>18.924</i>
<i>Point C1:</i>	<i>350.42</i>	<i>12.09</i>	<i>25.466</i>	<i>604.01</i>	<i>12.10</i>	<i>46.692</i>	<i>890.10</i>	<i>12.10</i>	<i>75.478</i>
<i>Point C2:</i>	<i>232.89</i>	<i>12.04</i>	<i>16.244</i>	<i>417.32</i>	<i>12.07</i>	<i>30.851</i>	<i>614.91</i>	<i>12.08</i>	<i>50.912</i>
<i>Point C3:</i>	<i>142.61</i>	<i>11.95</i>	<i>8.110</i>	<i>264.83</i>	<i>11.94</i>	<i>15.199</i>	<i>425.51</i>	<i>11.94</i>	<i>24.875</i>

As previously evaluated for the modeled Subareas, these Fully-Developed Proposed Conditions results for the Points of Interest will be compared to those established in Section 3 (Existing Conditions) of this report. The points represent similar locations in both Existing- and Proposed Conditions analyses. Therefore, the variation in quantity and rate of stormwater discharge between these two models represents the hydrologic effect generated by the proposed development. The following table depicts the difference between the Existing- and Fully-Developed Conditions models without provisions for onsite stormwater detention.

Table 4.2-3(b). Hydrologic Information – Comparison of Results: Micro- Fully-Developed vs. Existing Conditions Summary (Junctions) Un-Detained Hydrology

Location:	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Point A1:	123.14	-0.08	7.888	143.92	-0.06	10.596	179.47	-0.05	12.900
Point A2:	93.93	-0.07	6.935	87.31	-0.06	8.844	90.83	-0.05	10.309
Point A3:	-2.51	n/c	-0.02	-7.65	n/c	0.223	-10.82	n/c	0.372
Point A4:	174.41	-0.12	7.175	177.61	-0.09	8.841	158.79	-0.06	10.156
Point A6:	17.21	-0.01	1.086	17.59	-0.01	1.286	21.55	-0.01	1.427
<i>Point A7:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>

Location:	Q_{P-2} (cfs):	T_{P-2} (hr.):	V_{R-2} (ac-ft):	Q_{P-10} (cfs):	T_{P-10} (hr.):	V_{R-10} (ac-ft):	Q_{P-100} (cfs):	T_{P-100} (hr.):	V_{R-100} (ac-ft):
Point A8:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point A9:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point A10:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B1:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B2:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B3:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point B4:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point C1:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point C2:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c
Point C3:	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c	n/c

The sign-convention used in the preceding table is: Proposed Conditions minus Existing Conditions. Therefore, negative values indicate a reduction in computed value, whereas positive values reflect an increase. Those locations that do not exhibit any change in computed values are identified by "n/c."

Information presented in the preceding table exhibits an increase in peak rates at most Points of Interest, relative to the existing conditions model. This is the result of the substantial increased imperviousness for the onsite areas.

Additional analysis is required to determine whether the performance of the proposed development is able to establish compliance with the proposed stormwater management objectives. The following table is provided to examine the difference between the computed peak discharge rates and the established ARR's for the Discovery Park Micro-Study.

**Table 4.2-4. Allowable Release Rate Evaluation –
Micro- Fully-Developed Conditions Summary Model – Without Detention**

Location:	Event:	Computed Peak Discharge Rate (cfs):	ARR (cfs):	Difference (cfs):
Point A1	2-Year:	1,074.58	881.71	192.87
	10-Year:	1,932.54	1,725.98	206.56
	100-Year:	2,939.19	2,657.53	281.66
Point A2	2-Year:	1,005.57	848.13	157.44
	10-Year:	1,784.23	1,634.53	149.70
	100-Year:	2,665.36	2,478.07	187.29
Point A3	2-Year:	627.78	613.94	13.84
	10-Year:	1,103.02	1,094.99	8.03
	100-Year:	1,588.55	1,578.20	10.35

Point A4	2-Year:	436.65	220.67	215.98
	10-Year:	718.69	494.51	224.18
	100-Year:	1,066.26	815.03	251.23
Point A6	2-Year:	54.75	16.57	38.18
	10-Year:	87.16	40.65	46.51
	100-Year:	128.41	63.30	65.11

The sign-convention utilized for the information presented in the preceding table is based upon "Computed Rate minus ARR." Therefore, positive values indicate an exceedance of the Allowable Release Rate, whereas negative values would indicate a peak rate that is lower than the ARR, thereby meeting the targeted peak discharge rates. The difference in the rates depicted in this table are all in exceedance of the targeted rates. Though the increased peak discharge rates at the defined Points of Interest are relatively moderate, these exceedances are of noticeably greater magnitude.

Proposed Discovery Park Stormwater Detention Facility:

The proposed stormwater management facility that is defined in the Micro Study section of this report is configured for the fully-developed geometry. Therefore, the basin geometry is not repeated in this section. Utilizing the effects of this basin, the following table depicts the stormwater routing summary for the Micro-Study Fully-Developed Conditions.

**Table 4.2-5. Fully-Developed Conditions Hydrologic Routing Summary–
Discovery Park Stormwater Detention Facility**

	Peak Q In (cfs):	T_P In (hr.):	Peak Q Out (cfs):	T_P Out (hr.):	V_R (ac-ft):	Peak WSEL (ft; NAVD):	Max. Storage Volume (ac-ft):
WQv:	82.63	11.93	1.17	19.68	3.665	930.12	3.541
2-Year:	286.84	11.93	2.54	24.03	10.827	933.20	14.042
10-Year:	467.52	11.93	38.80	12.53	21.710	934.10	17.346
100-Year:	700.31	11.93	104.55	12.21	36.262	936.00	24.586

The preceding routing summary indicates that, under Fully-Developed Conditions, this basin provides substantial attenuation of the modeled design storm events. The maximum storage volume for each of the modeled design storms is greater than 50% of the total runoff volume contributed to this facility. Section 8 of this report includes hydrographs for this proposed stormwater management facility.

An emergency spillway is to be constructed on the northwest side of the basin at an elevation of 937.00. The spillway is designed to, should the primary outfall structure be clogged or completely fail, route the 100 year storm into the existing stream. The 100 year storm is

calculated as a peak flow of 700.31 cfs. The spillway length is designed as 470 LF. In the 100 year storm, the water surface elevation in the weir is calculated as 937.58. The top of the basin is set at 938.88, giving the basin 1.30 LF of freeboard.

Results of this Fully-Developed Conditions hydrologic routing, including the proposed stormwater detention facility are provided in the following summary table. This information is provided at the same “Points of Interest” used in the Existing Conditions hydrologic analysis for this Stormwater Drainage Study. Refer to the Micro Fully-Developed Conditions Drainage Area Map for the location of the points of interest in relation to the development sites, modeled subareas, channels and conveyances. As in the preceding tables, locations that are unchanged from the Existing Conditions analysis are presented as *grey print* in the following tables.

Table 4.2-6(a). Hydrologic Information – Micro- Fully-Developed Conditions Summary (Junctions) Including Proposed Discovery Park Detention Basin

Location:	Q _{P-2} (cfs):	T _{P-2} (hr.):	V _{R-2} (ac-ft):	Q _{P-10} (cfs):	T _{P-10} (hr.):	V _{R-10} (ac-ft):	Q _{P-100} (cfs):	T _{P-100} (hr.):	V _{R-100} (ac-ft):
Point A1:	921.19	12.23	90.079	1,657.18	12.23	171.468	2,513.37	12.22	282.734
Point A2:	875.19	12.16	80.894	1,567.41	12.16	152.813	2,362.46	12.17	250.882
Point A3:	627.78	12.15	52.992	1,103.02	12.15	98.204	1,588.55	12.17	159.717
Point A4:	230.17	12.11	25.404	419.99	12.14	49.040	704.29	12.14	81.130
Point A6:	54.75	11.93	3.169	87.16	11.92	5.190	128.41	11.92	7.817
<i>Point A7:</i>	<i>229.26</i>	<i>12.05</i>	<i>14.580</i>	<i>397.23</i>	<i>12.07</i>	<i>27.333</i>	<i>603.32</i>	<i>12.08</i>	<i>44.871</i>
<i>Point A8:</i>	<i>53.05</i>	<i>11.96</i>	<i>3.070</i>	<i>101.89</i>	<i>11.95</i>	<i>5.907</i>	<i>166.82</i>	<i>11.95</i>	<i>9.826</i>
<i>Point A9:</i>	<i>128.79</i>	<i>12.00</i>	<i>8.028</i>	<i>223.48</i>	<i>11.99</i>	<i>14.240</i>	<i>346.07</i>	<i>11.99</i>	<i>22.563</i>
<i>Point A10:</i>	<i>45.61</i>	<i>11.97</i>	<i>2.890</i>	<i>72.57</i>	<i>11.97</i>	<i>4.733</i>	<i>106.86</i>	<i>11.97</i>	<i>7.128</i>
<i>Point B1:</i>	<i>615.14</i>	<i>12.11</i>	<i>49.667</i>	<i>1,076.61</i>	<i>12.11</i>	<i>91.457</i>	<i>1,551.65</i>	<i>12.14</i>	<i>148.401</i>
<i>Point B2:</i>	<i>242.31</i>	<i>12.05</i>	<i>20.032</i>	<i>446.43</i>	<i>12.04</i>	<i>36.522</i>	<i>715.56</i>	<i>12.03</i>	<i>58.980</i>
<i>Point B3:</i>	<i>153.04</i>	<i>11.99</i>	<i>13.975</i>	<i>266.04</i>	<i>11.98</i>	<i>24.377</i>	<i>411.65</i>	<i>11.97</i>	<i>38.255</i>
<i>Point B4:</i>	<i>48.10</i>	<i>11.94</i>	<i>7.673</i>	<i>71.04</i>	<i>11.93</i>	<i>12.565</i>	<i>98.80</i>	<i>11.93</i>	<i>18.924</i>
<i>Point C1:</i>	<i>350.42</i>	<i>12.09</i>	<i>25.466</i>	<i>604.01</i>	<i>12.10</i>	<i>46.692</i>	<i>890.10</i>	<i>12.10</i>	<i>75.478</i>
<i>Point C2:</i>	<i>232.89</i>	<i>12.04</i>	<i>16.244</i>	<i>417.32</i>	<i>12.07</i>	<i>30.851</i>	<i>614.91</i>	<i>12.08</i>	<i>50.912</i>
<i>Point C3:</i>	<i>142.61</i>	<i>11.95</i>	<i>8.110</i>	<i>264.83</i>	<i>11.94</i>	<i>15.199</i>	<i>425.51</i>	<i>11.94</i>	<i>24.875</i>

As with the previous section, these Fully-Developed Conditions results for the Points of Interest will be compared to those established in Section 3 (Existing Conditions) of this report. The points represent similar locations in both Existing- and Proposed Conditions analyses. Therefore, the variation in quantity and rate of stormwater discharge between these two models

represents the hydrologic effect generated by the proposed development. The following table depicts the difference between the Existing- and Fully-Developed Conditions models, including the effects of the proposed onsite stormwater detention facility.

**Table 4.2-6(b). Hydrologic Information – Comparison of Results:
Micro- Fully-Developed Conditions Summary (Junctions) Detained Hydrology**

Location:	Q _{P-2} (cfs):	T _{P-2} (hr.):	V _{R-2} (ac-ft):	Q _{P-10} (cfs):	T _{P-10} (hr.):	V _{R-10} (ac-ft):	Q _{P-100} (cfs):	T _{P-100} (hr.):	V _{R-100} (ac-ft):
Point A1:	-27.77	-0.02	2.03	-91.55	0.01	4.50	-241.47	0.02	6.73
Point A2:	-33.86	-0.01	1.084	-124.32	0.01	2.758	-204.13	0.04	4.146
Point A3:	-0.86	n/c	-0.005	-4.65	n/c	0.252	-7.01	n/c	0.420
Point A4:	-30.75	-0.04	1.312	-119.50	0.03	2.731	-202.27	0.05	3.948
Point A6:	16.77	n/c	1.086	16.62	-0.01	1.286	15.22	-0.01	1.428
<i>Point A7:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Point A8:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Point A9:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Point A10:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Point B1:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Point B2:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Point B3:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Point B4:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Point C1:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Point C2:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>
<i>Point C3:</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>	<i>n/c</i>

The sign-convention used in the preceding table is: Proposed Conditions minus Existing Conditions. Therefore, negative values indicate a reduction in computed value, whereas positive values reflect an increase. Those locations that do not exhibit any change in computed values are identified by "n/c."

Data presented in the preceding table depicts the significant reduction in peak rates relative to the Existing Conditions model along the Channel "A" tributary. There is no change from the Micro-Study Proposed Conditions at the Point A3 location. Although the peak discharge rates are reduced at the modeled Points of Interest, further analysis is required to establish compliance with the project's stormwater management objectives.

The following table is provided to examine the difference between the computed Fully-Developed Conditions peak discharge rates and the established ARR's for the overall development.

Table 4.2-7. Allowable Release Rate Evaluation – Fully-Developed Conditions Micro Study (W/Detention)

Location:	Event:	Computed Peak Discharge Rate (cfs):	ARR (cfs):	Difference (cfs):
Point A1	2-Year:	921.19	857.17	64.02
	10-Year:	1665.01	1702.66	-37.65
	100-Year:	2526.80	2621.34	-94.54
Point A2	2-Year:	875.19	823.14	52.05
	10-Year:	1567.41	1607.68	-40.27
	100-Year:	2362.46	2436.65	-74.19
Point A3	2-Year:	627.78	612.50	15.28
	10-Year:	1103.02	1092.23	10.79
	100-Year:	1588.55	1574.71	13.84
Point A4	2-Year:	230.17	200.55	29.62
	10-Year:	419.99	471.95	-51.96
	100-Year:	704.29	771.67	-67.38
Point A6	2-Year:	54.75	16.57	38.18
	10-Year:	87.16	40.65	46.51
	100-Year:	128.41	63.30	65.11

Note: The sign-convention utilized for the information presented in the preceding table is based upon "Computed Rate minus ARR." Positive values indicate an exceedance of the ARR; negative values indicate that the peak rate is lower than the ARR.

The difference in the rates depicted in this table are generally in compliance with the targeted rates; the proposed stormwater management facility provides adequate attenuation for the primary tributary Channel "A." The information provided in the preceding table does demonstrate that there are a few exceptions 2 year peak rates at all Points, and additional peak rates at Points A3 and A6. This is the terminal end of the Channel "B" tributary, immediately upstream from confluence at Point A2.

Although the ARR is exceeded at the Point A3 location, the Fully-Developed Conditions analysis does exhibit a reduction in peak discharge rate relative to Existing Conditions (*flows are reduced to: -0.86 cfs, -4.65 cfs, and -7.01 cfs for the 2-, 10-, and 100-year events, respectively*). The ARR is also exceeded at Point A6 during all storm events, however this point is located upstream of the

4.3. Proposed Stormwater Best Management Practices

This section of the study will address water quality treatment for the Discovery Park Phase 1 and portions of the Discovery Park Phase 2 and Aria developments. Future Discovery Park sites, the untreated portion of the Aria site, and the future Multi-Family Residential sites will be required to provide an analysis during the development of those areas. While the City of Lee's Summit has not adopted the Mid America Regional Council and American Public Works Association "Manual of Best Management Practices for Stormwater Quality" (Oct. 2012), 40-hr extended detention of the water quality storm volume generated by the development is required. In the sections above, it has been shown that the proposed detention basin does provide 40-hr extended detention for the Discovery Park and Aria land areas tributary to the basin. Below is a table summarizing the water quality volume calculations for the extended wet detention basin orifice sizing.

Table 4.3-1. Water Quality Volume Calculations – Proposed Conditions

Subarea	Tributary Area (ac)	Volumetric Runoff Coefficient	WQv (cf)	Depth of WQv at Outlet (ft)	WQv Orifice Dia. (in)	Release Time (hr)
Micro Study	65.73	0.58	189,364	1.38	7	44
Macro Study	65.73	0.74	241,513	1.75	7	50

5. SUMMARY

See Tables 5-1 and 5-2, below, for a summary comparison of Existing, Allowable, and Proposed peak flowrates during both the Micro and Macro conditions.

Table 5-1. Micro- Proposed Peak Flowrate Comparison

Location:	Event:	Existing Flowrate (cfs):	ARR (cfs):	Proposed Flowrate (cfs):	Difference (Existing vs. Proposed) (cfs):	Difference (ARR vs. Proposed) (cfs):
Point A1	2-Year:	948.96	859.38	914.02	-34.94	54.64
	10-Year:	1,748.23	1704.07	1635.15	-113.08	-68.92
	100-Year:	2,754.84	2623.74	2502.62	-252.22	-121.12
Point A2	2-Year:	909.05	823.14	871.69	-37.36	48.55
	10-Year:	1,691.73	1607.68	1545.48	-146.25	-62.20
	100-Year:	2,566.59	2436.65	2350.92	-215.67	-85.73
Point A3	2-Year:	628.64	612.50	625.11	-3.53	12.61
	10-Year:	1,107.67	1092.23	1100.39	-7.28	8.16
	100-Year:	1,595.56	1574.71	1586.54	-9.02	11.83
Point A4	2-Year:	260.92	200.55	229.99	-30.93	29.44
	10-Year:	539.49	471.95	400.59	-138.90	-71.36
	100-Year:	906.56	771.67	695.13	-211.43	-76.54
Point A6	2-Year:	37.98	16.57	37.88	-0.10	21.31
	10-Year:	70.54	40.65	70.35	-0.19	29.70
	100-Year:	113.19	63.30	112.88	-0.31	49.58

Note: The sign-convention utilized for the information presented in the preceding table is based upon "Proposed Rate minus Existing (or ARR)." Positive values indicate an exceedance of the Existing (or ARR); negative values indicate that the peak rate is lower than the Existing (or ARR).

Table 5-2. Micro-Future- Peak Flowrate Comparison

Location:	Event:	Existing Flowrate (cfs):	ARR (cfs):	Proposed Flowrate (cfs):	Difference (Existing vs. Proposed) (cfs):	Difference (ARR vs. Proposed) (cfs):
Point A1	2-Year:	948.96	857.17	921.19	-27.77	64.02
	10-Year:	1,748.23	1702.66	1657.18	-91.55	-45.48
	100-Year:	2,754.84	2621.34	2513.37	-241.47	-107.97
Point A2	2-Year:	909.05	823.14	875.19	-33.86	52.05
	10-Year:	1,691.73	1607.68	1567.41	-124.32	-40.27
	100-Year:	2,566.59	2436.65	2362.46	-204.13	-74.19
Point A3	2-Year:	628.64	612.5	627.78	-0.86	15.28
	10-Year:	1,107.67	1092.23	1103.02	-4.65	10.79
	100-Year:	1,595.56	1574.71	1588.55	-7.01	13.84
Point A4	2-Year:	260.92	200.55	230.17	-30.75	29.62
	10-Year:	539.49	471.95	419.99	-119.50	-51.96
	100-Year:	906.56	771.67	704.29	-202.27	-67.38
Point A6	2-Year:	37.98	16.57	54.75	16.77	38.18
	10-Year:	70.54	40.65	87.16	16.62	46.51
	100-Year:	113.19	63.3	128.41	15.22	65.11

Note: The sign-convention utilized for the information presented in the preceding table is based upon "Proposed Rate minus Existing (or ARR)." Positive values indicate an exceedance of the Existing (or ARR); negative values indicate that the peak rate is lower than the Existing (or ARR).

Along with peak rate control, 40-hr extended wet detention is being provided for the Discovery Park Phase 1, Discovery Park Phase 2, and Aria development areas tributary to the proposed pond. Future developments will be required to provide extended detention and/or additional water quality treatment as those plans are designed and submitted.

Table 5-3. Macro ARR Comparison

Location:	Return Event (Yr):	Existing Conditions Flow Rate	ARR (cfs):
Point A7			
	2-Year:	229.26	229.26
	10-Year:	397.23	397.23
	100-Year:	603.32	603.32
Point B1			
	2-Year:	615.14	615.14
	10-Year:	1,076.61	1076.61
	100-Year:	1,551.65	1551.65
Point D1			
	2-Year:	560.43	481.87
	10-Year:	941.67	874.92
	100-Year:	1351.30	1262.83
Point D2			
	2-Year:	485.06	427.99
	10-Year:	752.83	707.54
	100-Year:	1052.38	996.29
Point E1			
	2-Year:	65.98	29.51
	10-Year:	106.88	72.41
	100-Year:	143.00	103.59
Point F1			
	2-Year:	47.57	25.75
	10-Year:	74.89	55.17
	100-Year:	106.16	80.20
Point F2			
	2-Year:	31.56	7.75
	10-Year:	45.26	26.59
	100-Year:	65.86	39.78
Point G1			
	2-Year:	58.32	42.99
	10-Year:	102.21	83.33
	100-Year:	163.43	131.84
Point G2			
	2-Year:	23.29	5.94
	10-Year:	36.98	18.02
	100-Year:	62.01	27.70

Location:	Return Event (Yr):	Existing Conditions Flow Rate	ARR (cfs):
Point H1			
	2-Year:	119.54	36.41
	10-Year:	240.90	124.05
	100-Year:	406.43	190.18
Point I1			
	2-Year:	4.44	2.80
	10-Year:	7.87	5.64
	100-Year:	12.29	8.74

The above table establishes ARR rates for locations unaffected in the Phase 1 Fully-Developed conditions. Future developments within the areas will be analyzed by their respective micro studies.

6. CONCLUSION

This study has been prepared to provide an analysis of the impacts that the fully-developed conditions of project site areas tributary to Unity Lake #2, and provide prescriptive release rate requirements for future development areas within the Discovery Park development. Once fully developed, the area treated by the proposed detention basin and water quality systems include:

- Aria Apartments: a 22.50-ac apartment development (Zoned RP-4)
- Discovery Park Phase 1: a 39.42-ac commercial development (Zoned CP-2)
- Discovery Park Phase 2: a 19.82-ac commercial development (currently zoned AG and to be rezoned at a future time)

As shown in the tables presented in the sections above, it has been determined that with the proposed development and detention basin, the peak runoff rates for the study area are reduced from the pre-developed conditions at all points analyzed. Comprehensive Control requirements are also met at all points in the 10 and 100 year storm events. In the 2 year storm event, ARR are unable to be met at all points.

A waiver is being requested to allow all points in the 2 year storm event to exceed allowable release rates, while remaining below existing peak rates. The existing basin located at Point A5 provides a slight attenuation of peak flows and delays in timing for offsite points A7, A8, A9 and A10. With construction of the new basin, the existing basin will be removed and flows from Point A7 will be rerouted through the site by extending the existing box culvert under Douglas Street to a point downstream of the Discovery Park Phase 1 development. While reviewing various models of the site, it was determined that the existing depression and breached spillway that is being removed were providing enough attenuation of peak flows from upstream, offsite area A7-A10, offsetting the attenuation via the proposed detention facility is not feasible. As shown in the tables within the previous sections of this study, the proposed basin reduces the 2-yr storm event flowrates by 98% for the areas tributary to the basin. To detain the flows to below the calculated ARR, detention would need to be constructed within the existing creek channel downstream of the site. However, this would require substantial improvements within the existing channel, further impacting the natural creek system.

With this consideration, it is infeasible to reduce the peak rates to meet the calculated allowable rates. While the “allowable release rates” are not met in the 2 year storm, peak flow rates are reduced from the Existing Conditions rate in all storm events and no adverse impacts are anticipated downstream to Unity Lake #2 or the receiving creek.

The results of this study demonstrate the overall general compliance with the City of Lee’s Summit design criteria. We therefore request approval of this stormwater management report.