

DESIGN & CONSTRUCTION MANUAL DESIGN CRITERIA MODIFICATION REQUEST

PROJECT NAME: Cobey Creek 2nd Plat			
ADDRESS: Intersection of Cobey Creek Drive and Hwy-150			
PERMIT NUMBER: PL2022092		-	
OWNER'S NAME: Clayton Properties Group DBA Summit Home	es KC	-	
TO: Deputy Director of Public Works / City Engineer			
In accordance with the City of Lee's Summit's Design and Const modification to one or more provisions of the code as I feel that the public health, welfare and safety are assured. The following action. (NOTE: Cite specific code sections, justification and all Seeking relief from Section 5608.4(C)(1) for the "peripheral dra subdivisions due to grading changes during construction to less project, and hence the peak runoff from those particular areas versions."	t the spir g articula appropria inage issu sen the dr	it and intent of the tes my request for ate supporting doc ue" that is inherent ainage area in a pa	e DCM is observed and your review and uments.) in most residential articular portion of the
SUBMITTED BY: NAME: Garrett Cates - Anderson Engineering Inc. ADDRESS: 941 W 141st Ter., Suite A CITY, STATE, ZIP: Kansas City, MO 64145 Email: gcates@ae-inc.com	PHONE	IER (X) OWNER #: (913) 284-9362 URE:	2
KENT MONTER, P.E. DEVELOPMENT ENGINEERING MANAGER SIGNATURE:	DATE:	() APPROVAL	
JEFF THORN, P.E. WATER UTILITIES ASSITANT DIRECTOR OF ENGINEERING SERVIOR SIGNATURE:		() APPROVED	
GEORGE M. BINGER III, P.E. DEPUTY DIRECTOR OF PUBLIC WORKS/CITY ENGINEER SIGNATURE:	DATE: _	() APPROVED	() DENIAL
COMMENTS:			
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August 26, 2022

Deputy Director of Public Works/City Engineer **Public Works** 220 SE Green Street Lee's Summit, Missouri 64063

Re: **Cobey Creek - Peripheral Drainage Waiver Request**

Cobey Creek is a multi-phase mixed-use development that is made up of primarily single-family and two-family residential homes. The development was started by JCM Development, LLC in 2018, and hired HG Consultants to complete the Preliminary Development Plan, Master Drainage Study, and 1st Plat design drawings. Due to a change in ownership to Summit Homes KC following the completion of Phase 1 of the development, Anderson Engineering Inc. has been hired to develop the remaining design and construction documents necessary to complete the project. Discussed in the Master Drainage Plan that was completed and approved with the 1st Plat (Phase 1), is the inherent drainage issue referred to as "fringe drainage" or "peripheral drainage". This issue is a result of grading changes during construction to lessen the drainage area in a particular portion of the project, and hence, the peak runoff from those particular areas when compared to the pre-development condition. Due to the challenges meeting Section 5608.4(C)(1) of the City of Lee's Summit Design and Construction Manual, the developer is seeking a waiver relief to allow these peripheral drainage areas that cannot feasibly be captured by the provided wet and dry detention basins, and therefore allow them to be released to adjacent properties at flow rates that are still significantly less than the pre-developed condition. The following paragraphs summarize the results of a micro drainage analysis that was conducted to assess the pre-deevelopment, intermediate, and postdevelopment drainage areas depicted in Exhibit A (pre-construction), Exhibit B (after phase 2 is completed) and Exhibit C (fully developed site), located in Appendix A.

There are two peripheral drainage areas located within the Cobey Creek development, which are referred to as the "North" and "South" peripheral drainage areas in this analysis. The composite curve numbers for each of these areas are set to increase due to an increase in impervious area. Even with this increase in composite curve numbers however, the overall runoff is being reduced because of a decrease in total area from pre-phase 2 construction to a fully developed site. The North peripheral drainage area is reducing in size from 14.7 acres pre-construction, to 3.74 acres after phase 2, and 2.51 acres once the site is fully developed. The South peripheral drainage area is reducing in size from 1.75 acres pre-construction to 1.12 acres once phase 2 is complete as well as once fully developed. Therefore, the total contributing drainage area is reducing by a total of 12.7 acres, or approximately 86%. This decrease in area is more than enough to account for the increase in the composite curve number, which ultimately results to a net decrease in runoff for the peripheral drainage areas.

To model the total runoff for the peripheral drainage areas, Hydraflow Hydragraphs software extension for AutoDesk Civil 3d was utilized. Using the SCS TR-55 method and a Type-Il 24-hour rainfall distribution, hydrographs for a 2-year, 10-year, and 100-year stormwater event were analyzed. **Table 1** and **Table 2** summarize the peak flow values from the Hydraflow model and can be further analyzed in the attached stormwater model output included in **Appendix B**. The analysis confirms that the stormwater runoff for each of the peripheral drainage areas decreases in the intermediate and fully-developed condition, due to the decrease in contributing drainage area.

	Table 1: Pre vs Post North Runoff Summary											
	Pre	Inter	Percent Reduction Inter	Percent Reduction Post								
2-yr	28.5	8.48	7.69	70.2	73.0							
10-yr	60.68	17.52	14.26	71.1	76.5							
100-yr	125.76	35.62	26.57	71.7	78.9							

Table 2: Pre vs Post South Runoff Summary										
	Pre	Inter	Post	Percent Reduction						
2-yr	3.10	2.78	2.78	10.3						
10-yr	6.52	5.57	5.57	14.6						
100-yr	13.35	11.03	11.03	17.4						

Due to the decrease in runoff to the adjacent properties within the peripheral drainage areas identified in this analysis, no downstream impacts are anticipated once phase 2 is completed as well as once the site is fully developed, and a waiver to the comprehensive control measures defined under Section 5608.4(C)(1) is requested for the Cobey Creek Development.

Anderson Engineering, Inc.

Garrett Cates, P.E. GCates@ae-inc.com

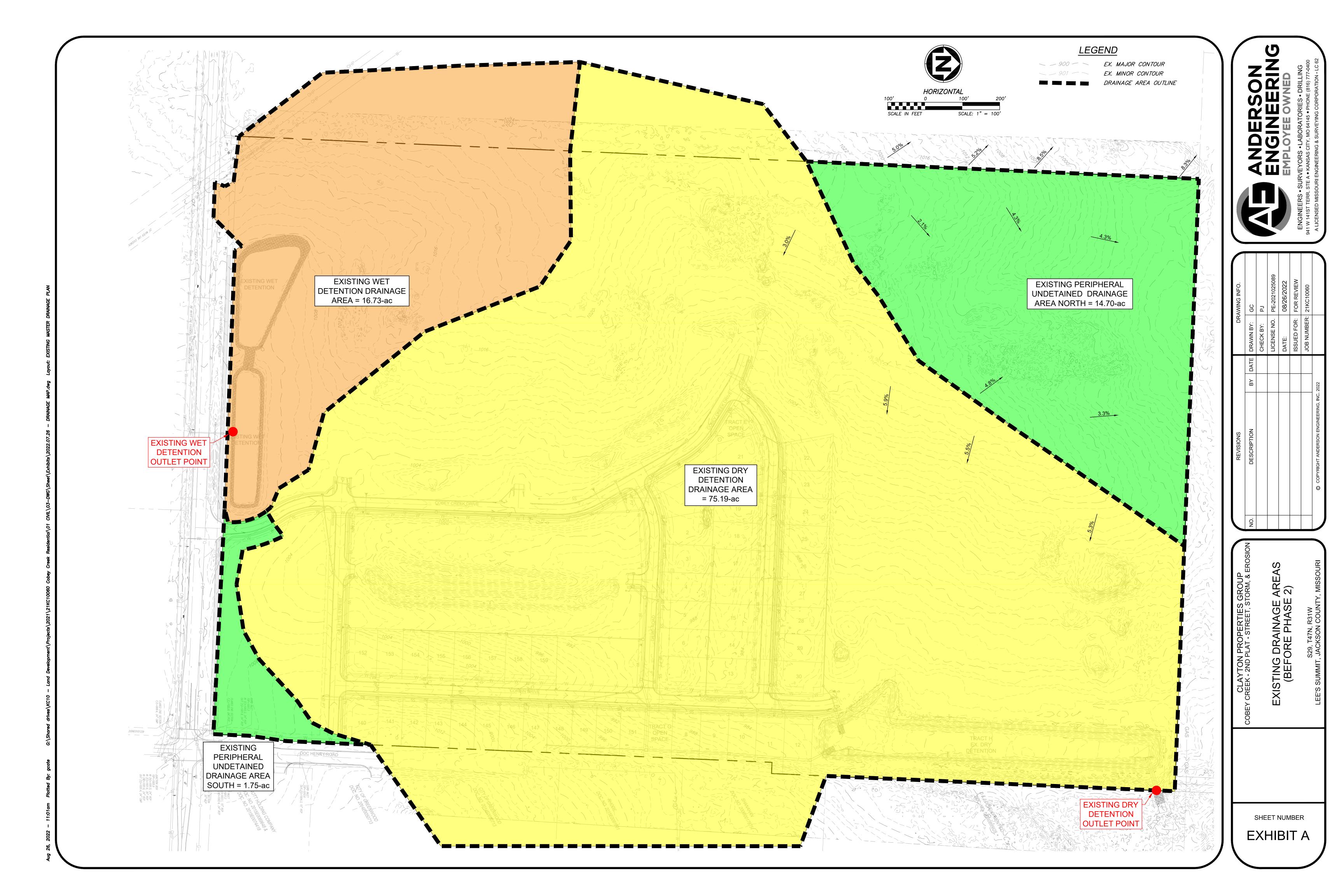
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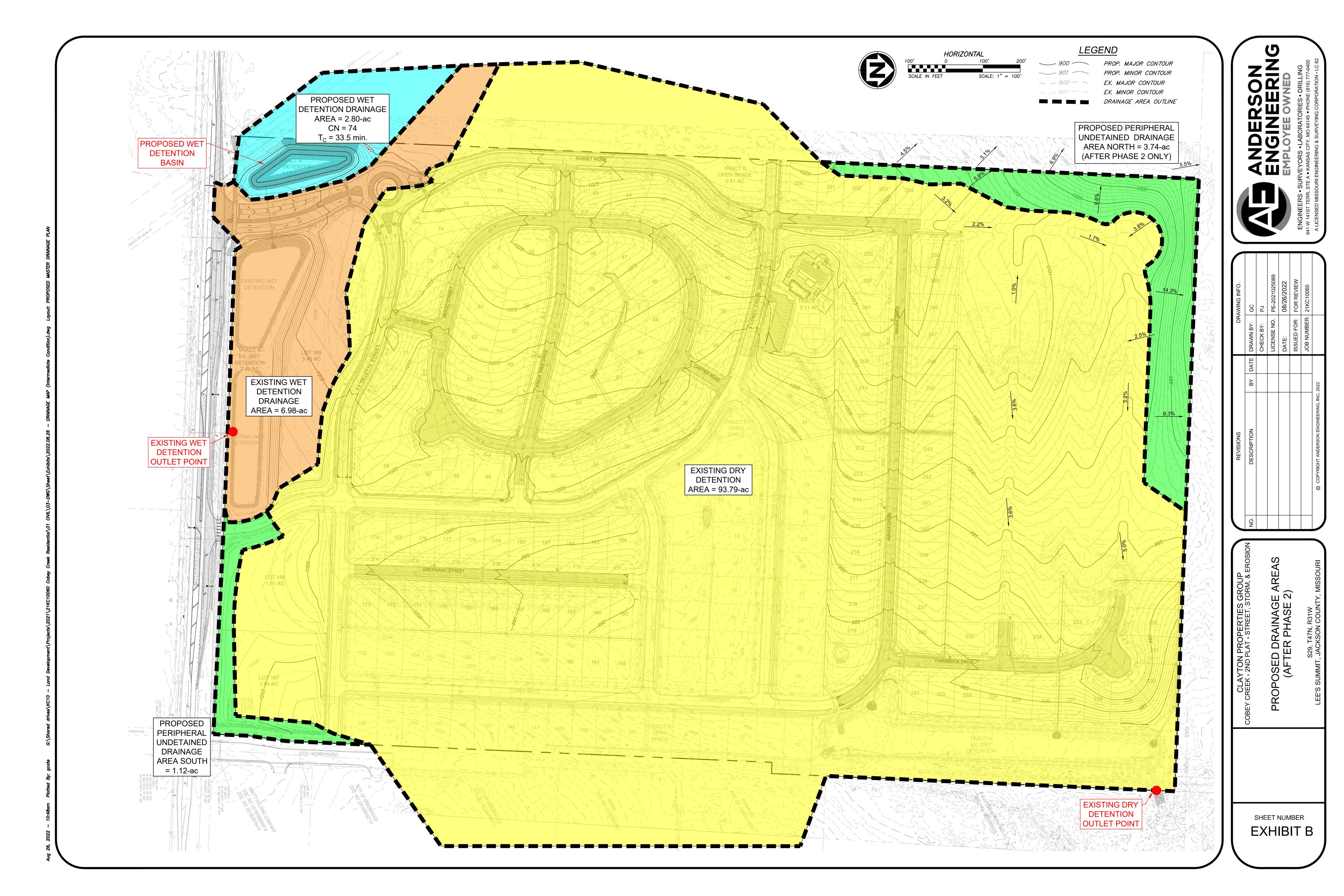
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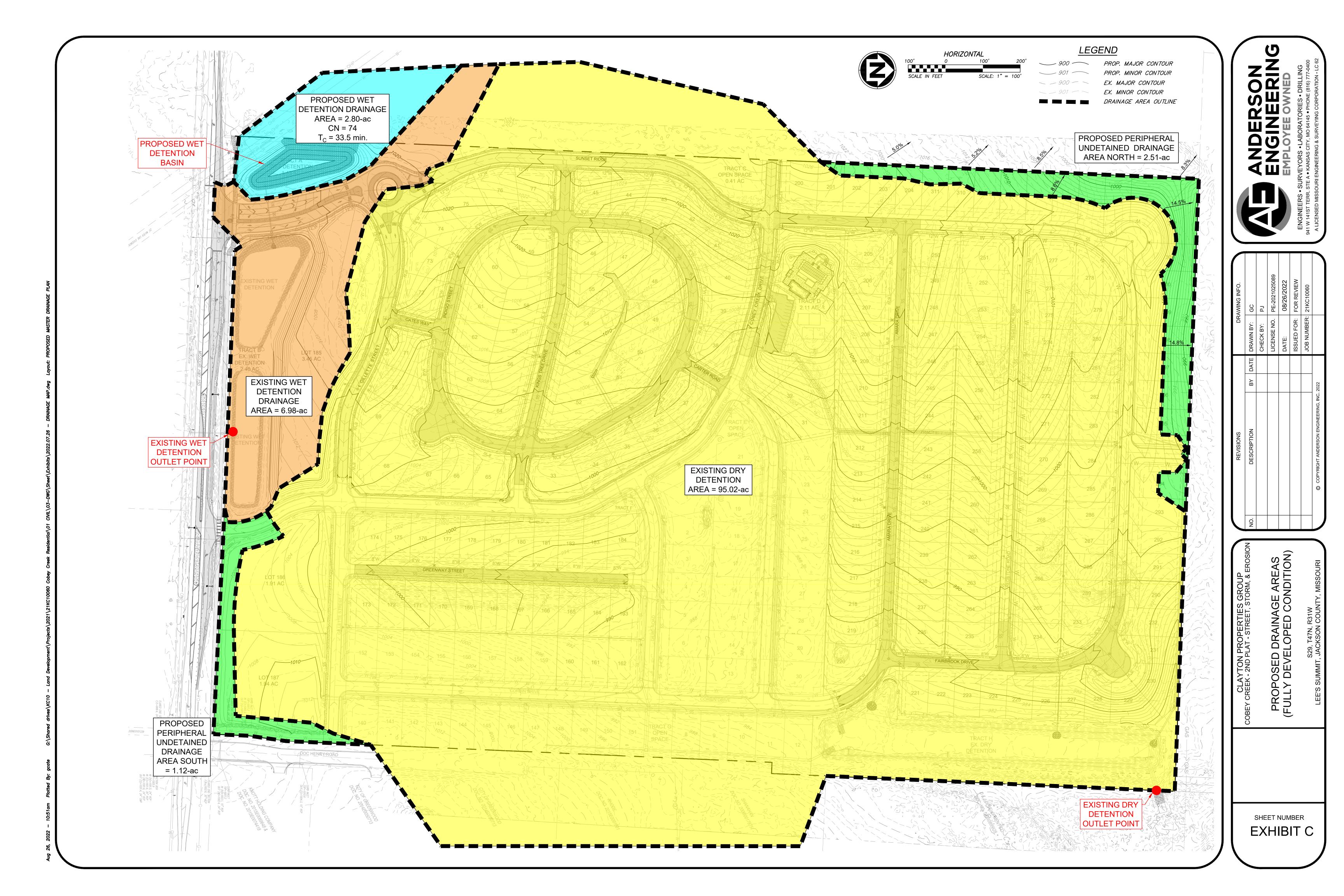


APPENDIX A





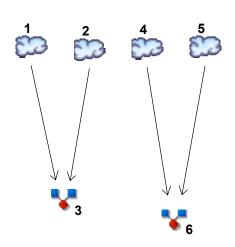


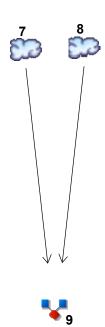


APPENDIX B



Watershed Model Schematic





Legend

<u>Hyd.</u>	<u>Origin</u>	<u>Description</u>
1	SCS Runoff	Exisitng Peripheral Drainage Area North
2	SCS Runoff	Existing Peripheral Drainage Area South
3	Combine	Total Existing Peripheral Drainage Area
4	SCS Runoff	Proposed North Drainage Area
5	SCS Runoff	Proposed South drainage Area
6	Combine	Total Proposed Peripheral Drainage Area
7	SCS Runoff	Intermediate South drainage Area
8	SCS Runoff	Intermediate Drainage Area North
9	Combine	Intermediate Peripheral Area

Hydrograph Return Period Recap Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd.		Inflow				Peak Out	eak Outflow (cfs) Hydrograph				
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		19.79	28.50			60.68			125.76	Exisitng Peripheral Drainage Area No
2	SCS Runoff		2.163	3.095			6.523			13.35	Existing Peripheral Drainage Area So
3	Combine	1, 2	21.94	31.60			66.88			138.62	Total Existing Peripheral Drainage Ar
4	SCS Runoff		5.770	7.694			14.26			26.57	Proposed North Drainage Area
5	SCS Runoff		2.008	2.781			5.571			11.03	Proposed South drainage Area
6	Combine	4, 5	7.771	10.46			19.83			37.60	Total Proposed Peripheral Drainage
7	SCS Runoff		2.008	2.781			5.571			11.03	Intermediate South drainage Area
8	SCS Runoff		5.993	8.477			17.52			35.62	Intermediate Drainage Area North
9	Combine	7, 8	8.001	11.26			23.09			46.65	Intermediate Peripheral Area

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hydrograph No. type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1 SCS Runoff	28.50	2	722	75,151				Exisitng Peripheral Drainage Area No
2 SCS Runoff	3.095	2	722	8,861				Existing Peripheral Drainage Area So
3 Combine	31.60	2	722	84,012	1, 2			Total Existing Peripheral Drainage Ar
4 SCS Runoff	7.694	2	718	17,605				Proposed North Drainage Area
5 SCS Runoff	2.781	2	720	6,365				Proposed South drainage Area
6 Combine	10.46	2	718	23,970	4, 5			Total Proposed Peripheral Drainage
7 SCS Runoff	2.781	2	720	6,365				Intermediate South drainage Area
8 SCS Runoff	8.477	2	720	19,423				Intermediate Drainage Area North
9 Combine	11.26	2	720	25,788	7, 8			Intermediate Peripheral Area

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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

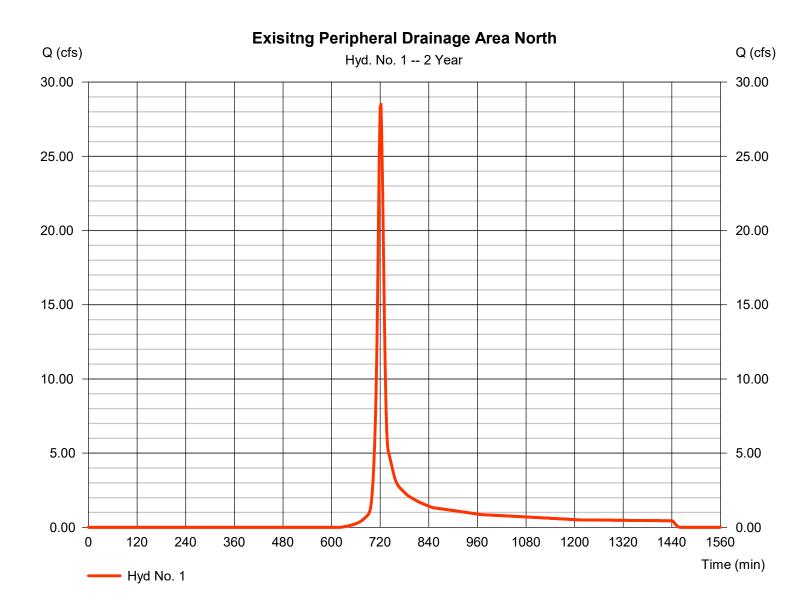
Friday, 08 / 26 / 2022

Hyd. No. 1

Exisitng Peripheral Drainage Area North

Hydrograph type = SCS Runoff Peak discharge = 28.50 cfsStorm frequency = 2 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 75.151 cuft Curve number Drainage area = 14.700 ac= 74* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 10.70 \, \text{min}$ Total precip. Distribution = Type II = 3.68 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(14.700 x 74)] / 14.700



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1Exisitng Peripheral Drainage Area North

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 50.0 = 3.68 = 1.20		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 6.44	+	0.00	+	0.00	=	6.44
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 340.00 = 3.00 = Unpaved =2.79	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.03	+	0.00	+	0.00	=	2.03
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 16.00 = 15.00 = 3.00 = 0.050 =5.39		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value	= 15.00 = 3.00 = 0.050		0.00 0.00 0.015		0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 15.00 = 3.00 = 0.050 =5.39	+	0.00 0.00 0.015 0.00	+	0.00 0.00 0.015 0.00	=	2.26

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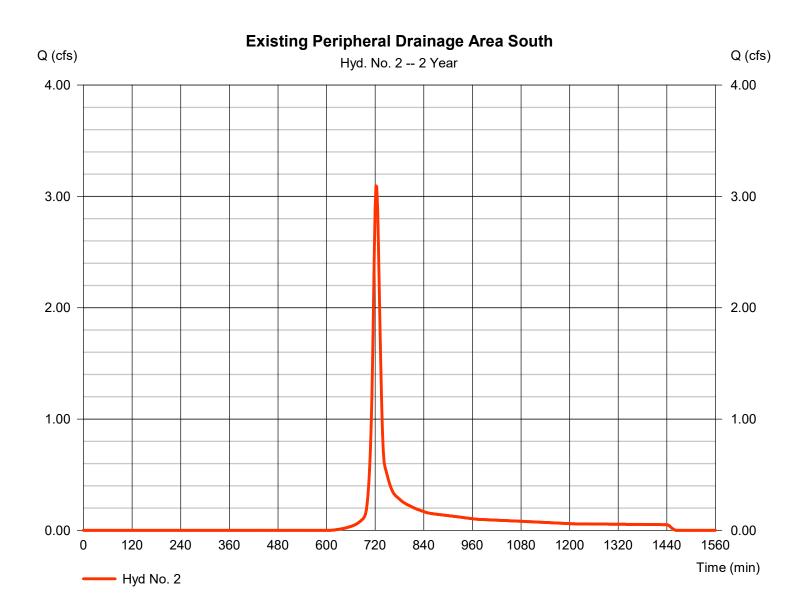
Friday, 08 / 26 / 2022

Hyd. No. 2

Existing Peripheral Drainage Area South

Hydrograph type = SCS Runoff Peak discharge = 3.095 cfsStorm frequency = 2 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 8.861 cuft = 1.750 ac Curve number = 75* Drainage area Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 14.20 \, \text{min}$ Total precip. Distribution = Type II = 3.68 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.087 x 99) + (1.660 x 74)] / 1.750



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 2Existing Peripheral Drainage Area South

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.150 = 150.0 = 3.68 = 1.50	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	14.18
maver mile (mili)	- 14.10	т	0.00	_	0.00	_	14.10
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Unpaved =0.00	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.050 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							14.20 min

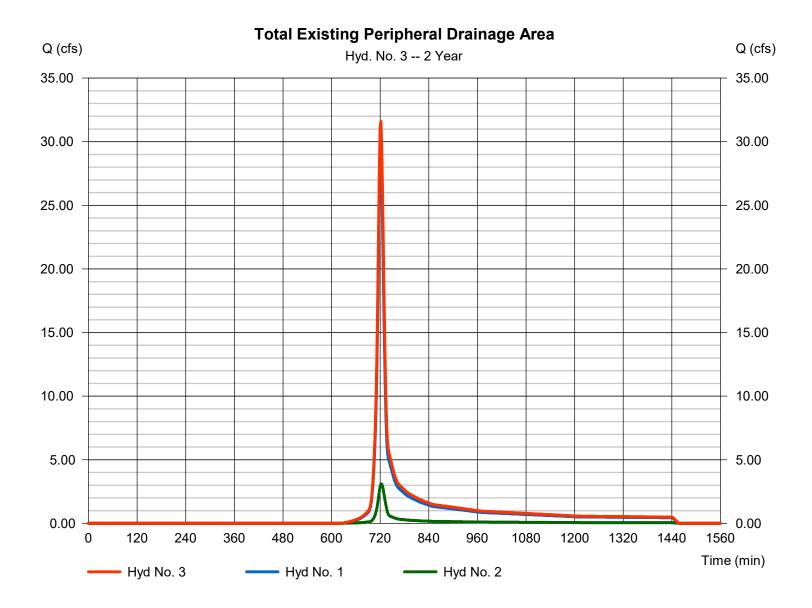
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Hyd. No. 3

Total Existing Peripheral Drainage Area

Hydrograph type = Combine Peak discharge = 31.60 cfsStorm frequency Time to peak = 2 yrs= 722 min Time interval = 2 min Hyd. volume = 84,012 cuft Inflow hyds. = 1, 2 Contrib. drain. area = 16.450 ac



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

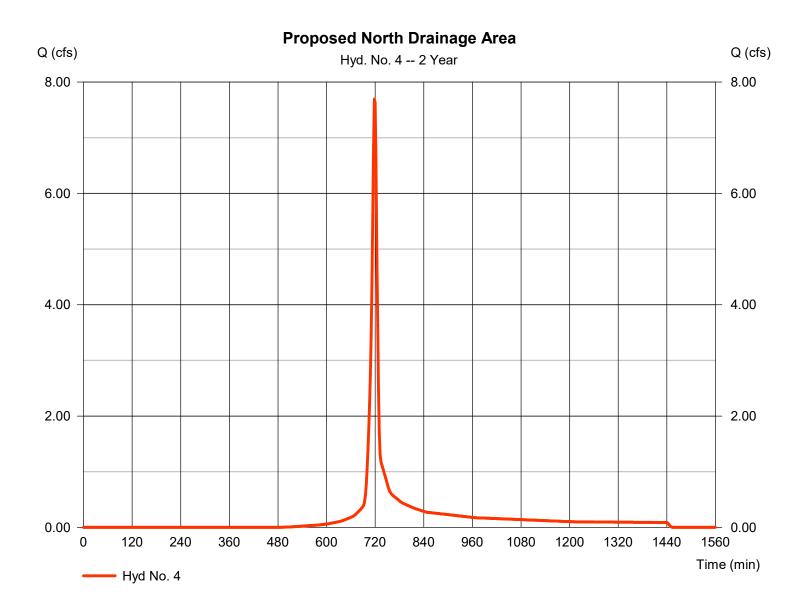
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Hyd. No. 4

Proposed North Drainage Area

Hydrograph type = SCS Runoff Peak discharge = 7.694 cfsStorm frequency = 2 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 17.605 cuft = 2.510 acCurve number Drainage area = 82* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 9.70 \, \text{min}$ = TR55 Total precip. = 3.68 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(2.510 x 82)] / 2.510



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 4Proposed North Drainage Area

<u>Description</u>	<u>A</u>		<u>B</u>		<u>c</u>		<u>Totals</u>		
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 50.0 = 3.68 = 1.20		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00				
Travel Time (min)	= 6.44	+	0.00	+	0.00	=	6.44		
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 100.00 = 3.00 = Unpaved =2.79	l	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00				
Travel Time (min)	= 0.60	+	0.00	+	0.00	=	0.60		
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 16.00 = 15.00 = 3.00 = 0.030 =8.98		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015				
Flow length (ft)	({0})1460.0		0.0		0.0				
Travel Time (min)	= 2.71	+	0.00	+	0.00	=	2.71		
Total Travel Time, Tc									

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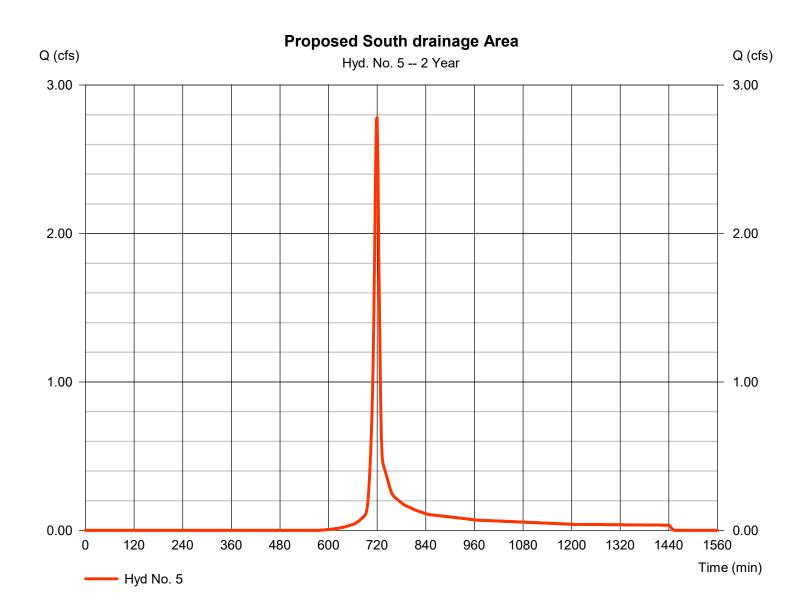
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Hyd. No. 5

Proposed South drainage Area

Hydrograph type = SCS Runoff Peak discharge = 2.781 cfsStorm frequency = 2 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 6,365 cuft= 1.120 ac Drainage area Curve number = 77* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.60 min = TR55 Total precip. Distribution = Type II = 3.68 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.140 x 99) + (0.980 x 74)] / 1.120



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 5Proposed South drainage Area

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 80.0 = 3.68 = 1.50		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.57	+	0.00	+	0.00	=	8.57
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Unpaved =0.00	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.013 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	0.0({0})		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.60 min

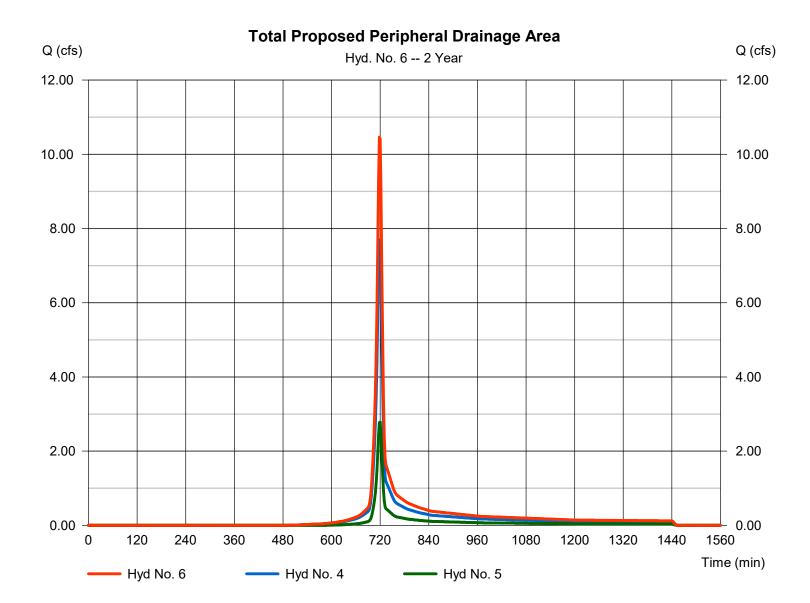
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Hyd. No. 6

Total Proposed Peripheral Drainage Area

Hydrograph type = Combine Peak discharge = 10.46 cfsStorm frequency Time to peak = 2 yrs= 718 min Time interval = 2 min Hyd. volume = 23,970 cuft Inflow hyds. = 4, 5Contrib. drain. area = 3.630 ac



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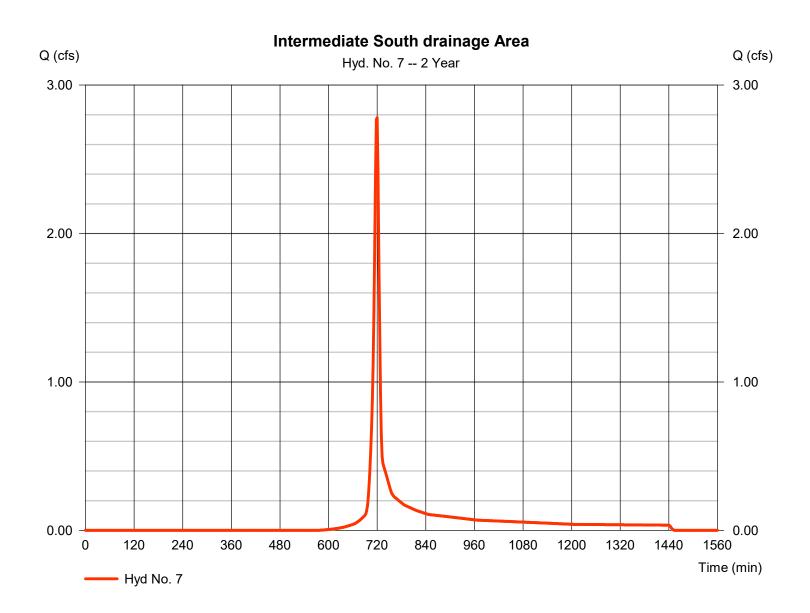
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Hyd. No. 7

Intermediate South drainage Area

Hydrograph type = SCS Runoff Peak discharge = 2.781 cfsStorm frequency = 2 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 6,365 cuftDrainage area = 1.120 acCurve number = 77* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.60 min = TR55 Total precip. Distribution = Type II = 3.68 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.140 x 99) + (0.980 x 74)] / 1.120



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 7Intermediate South drainage Area

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 80.0 = 3.68 = 1.50		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.57	+	0.00	+	0.00	=	8.57
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Unpaved =0.00	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flour							
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.013 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value	= 0.00 = 0.00 = 0.013		0.00 0.00 0.015		0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.013 =0.00	+	0.00 0.00 0.015 0.00	+	0.00 0.00 0.015 0.00	=	0.00

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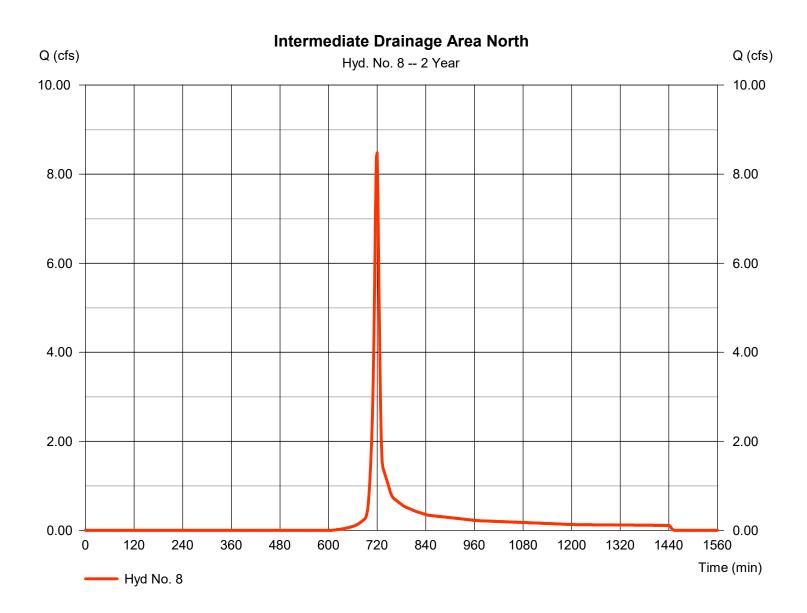
Friday, 08 / 26 / 2022

Hyd. No. 8

Intermediate Drainage Area North

Hydrograph type = SCS Runoff Peak discharge = 8.477 cfsStorm frequency = 2 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 19.423 cuft Curve number Drainage area = 3.740 ac= 75* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 9.80 min = TR55 Total precip. Distribution = Type II = 3.68 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.200 x 82) + (2.310 x 74)] / 3.740



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 8Intermediate Drainage Area North

<u>Description</u>	<u>A</u>		<u>B</u>		<u>c</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 170.0 = 3.68 = 4.90		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 9.76	+	0.00	+	0.00	=	9.76
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Paved =0.00		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							9.80 min

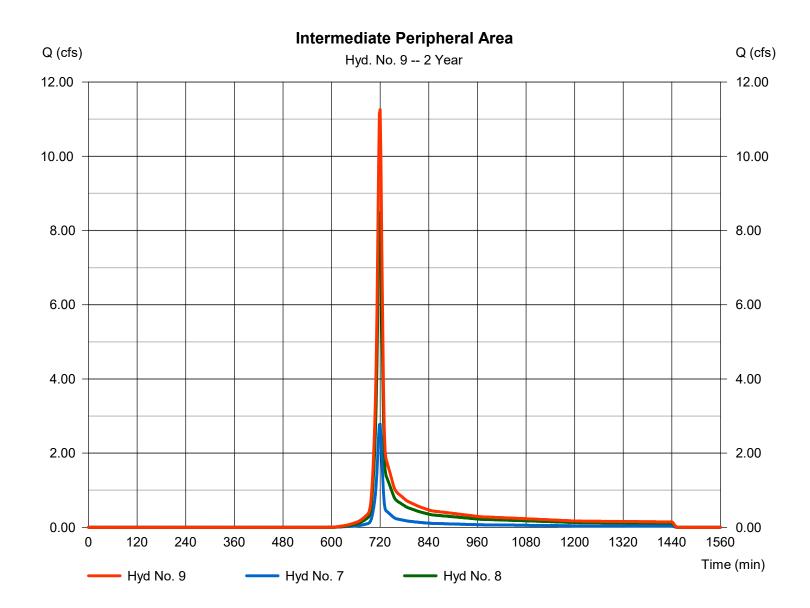
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Hyd. No. 9

Intermediate Peripheral Area

= 11.26 cfsHydrograph type = Combine Peak discharge Storm frequency Time to peak = 2 yrs= 720 min Time interval = 2 min Hyd. volume = 25,788 cuft Inflow hyds. = 7,8 Contrib. drain. area = 4.860 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	60.68	2	720	157,369				Exisitng Peripheral Drainage Area No
2	SCS Runoff	6.523	2	722	18,287				Existing Peripheral Drainage Area So
3	Combine	66.88	2	720	175,656	1, 2			Total Existing Peripheral Drainage Ar
4	SCS Runoff	14.26	2	718	33,074				Proposed North Drainage Area
5	SCS Runoff	5.571	2	718	12,770				Proposed South drainage Area
6	Combine	19.83	2	718	45,844	4, 5			Total Proposed Peripheral Drainage
7	SCS Runoff	5.571	2	718	12,770				Intermediate South drainage Area
8	SCS Runoff	17.52	2	718	40,083				Intermediate Drainage Area North
9	Combine	23.09	2	718	52,853	7, 8			Intermediate Peripheral Area

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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

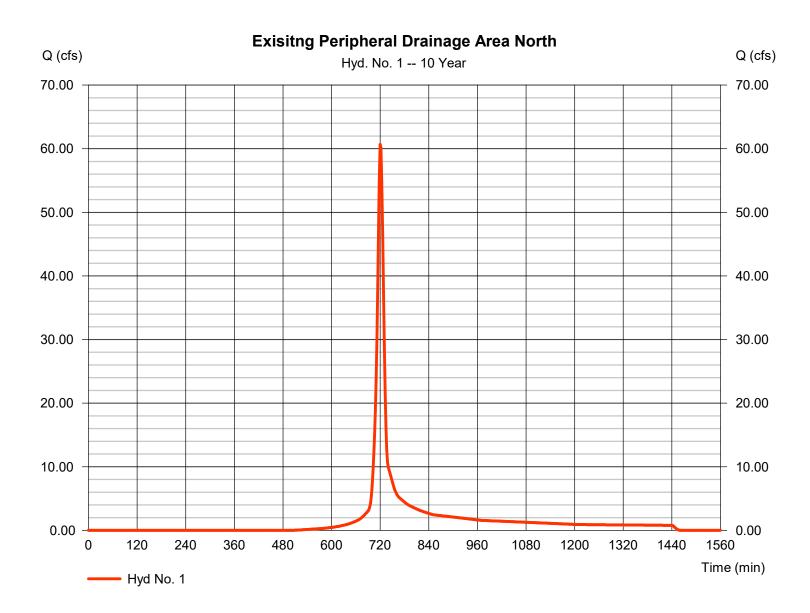
Friday, 08 / 26 / 2022

Hyd. No. 1

Exisitng Peripheral Drainage Area North

Hydrograph type = SCS Runoff Peak discharge = 60.68 cfsStorm frequency = 10 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 157.369 cuft Curve number Drainage area = 14.700 ac= 74* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 10.70 \, \text{min}$ Total precip. Distribution = Type II = 5.61 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(14.700 x 74)] / 14.700



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

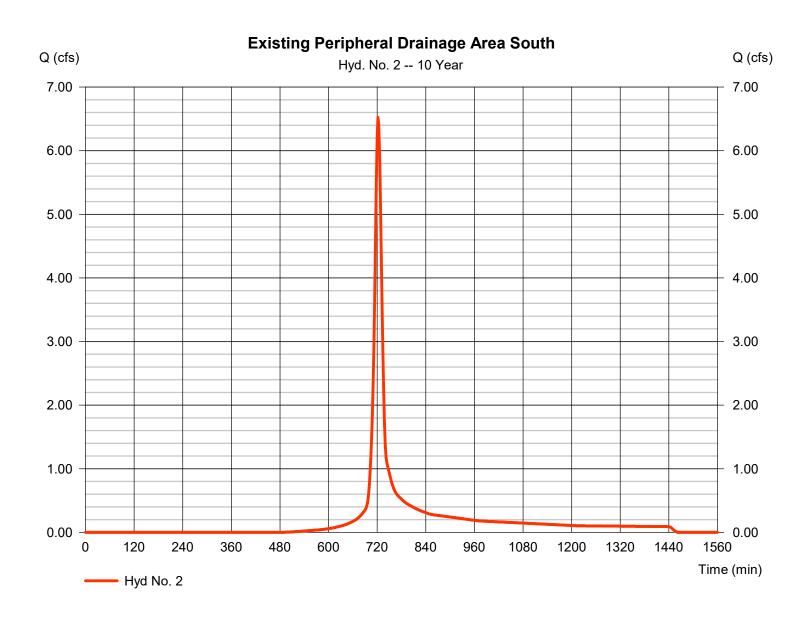
Friday, 08 / 26 / 2022

Hyd. No. 2

Existing Peripheral Drainage Area South

Hydrograph type = SCS Runoff Peak discharge = 6.523 cfsStorm frequency = 10 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 18.287 cuft Curve number Drainage area = 1.750 ac= 75* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 14.20 \, \text{min}$ Total precip. Distribution = Type II = 5.61 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.087 x 99) + (1.660 x 74)] / 1.750



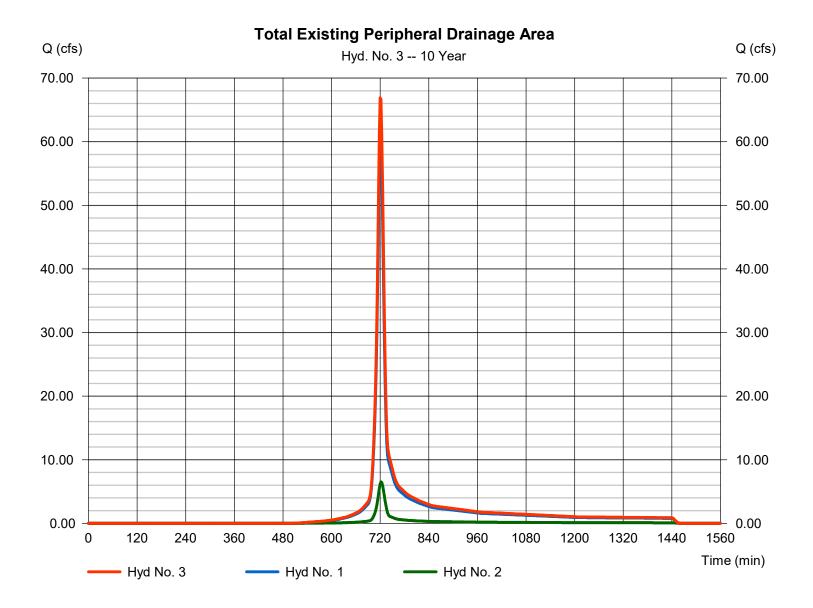
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Friday, 08 / 26 / 2022

Hyd. No. 3

Total Existing Peripheral Drainage Area

Hydrograph type = Combine Peak discharge = 66.88 cfsStorm frequency Time to peak = 10 yrs= 720 min Time interval = 2 min Hyd. volume = 175,656 cuft Inflow hyds. = 1, 2 Contrib. drain. area = 16.450 ac



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

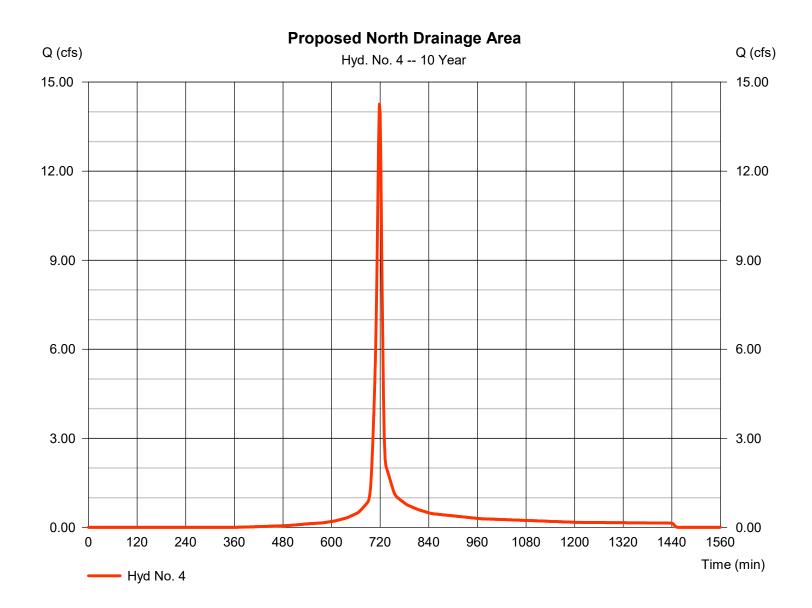
Friday, 08 / 26 / 2022

Hyd. No. 4

Proposed North Drainage Area

Hydrograph type = SCS Runoff Peak discharge = 14.26 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 33.074 cuft Curve number Drainage area = 2.510 ac= 82* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 9.70 \, \text{min}$ = TR55 Total precip. Distribution = Type II = 5.61 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(2.510 x 82)] / 2.510



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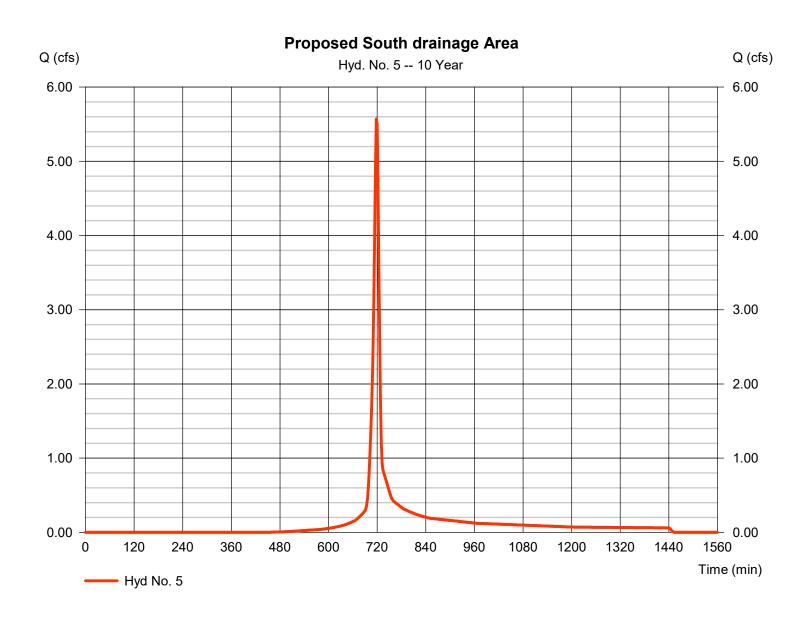
Friday, 08 / 26 / 2022

Hyd. No. 5

Proposed South drainage Area

Hydrograph type = SCS Runoff Peak discharge = 5.571 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 12.770 cuftDrainage area = 1.120 acCurve number = 77* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 8.60 \, \text{min}$ Total precip. Distribution = Type II = 5.61 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.140 x 99) + (0.980 x 74)] / 1.120



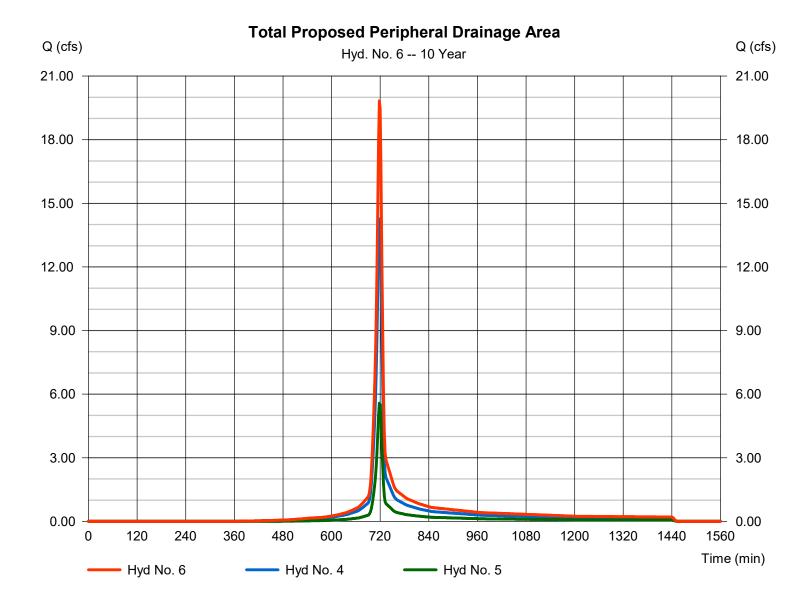
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Friday, 08 / 26 / 2022

Hyd. No. 6

Total Proposed Peripheral Drainage Area

Hydrograph type = Combine Peak discharge = 19.83 cfsStorm frequency Time to peak = 10 yrs= 718 min Time interval = 2 min Hyd. volume = 45,844 cuft Inflow hyds. = 4, 5Contrib. drain. area = 3.630 ac



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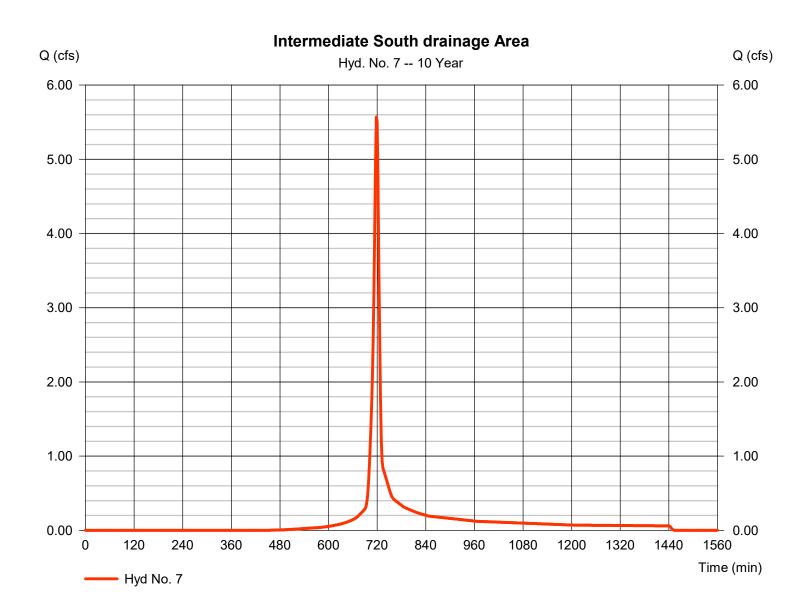
Friday, 08 / 26 / 2022

Hyd. No. 7

Intermediate South drainage Area

Peak discharge = SCS Runoff = 5.571 cfsHydrograph type Storm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 12.770 cuftDrainage area = 1.120 acCurve number = 77* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 8.60 \, \text{min}$ Total precip. Distribution = Type II = 5.61 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.140 x 99) + (0.980 x 74)] / 1.120



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

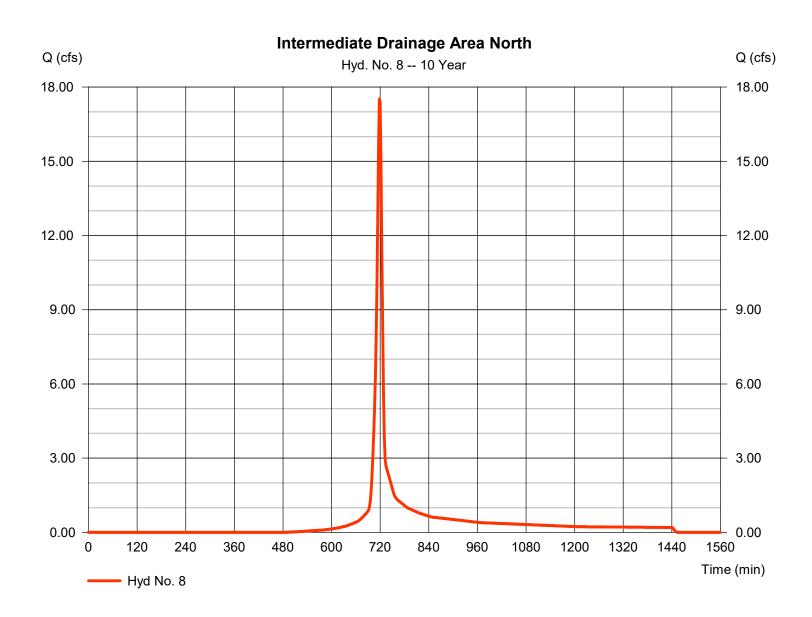
Friday, 08 / 26 / 2022

Hyd. No. 8

Intermediate Drainage Area North

Hydrograph type = SCS Runoff Peak discharge = 17.52 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 40.083 cuftDrainage area Curve number = 75* = 3.740 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 9.80 \, \text{min}$ = TR55 Total precip. Distribution = Type II = 5.61 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.200 x 82) + (2.310 x 74)] / 3.740



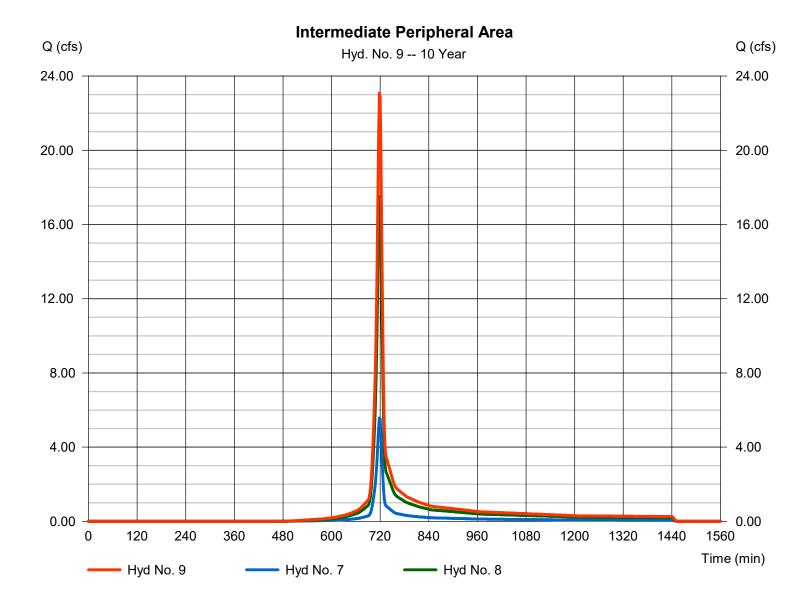
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Friday, 08 / 26 / 2022

Hyd. No. 9

Intermediate Peripheral Area

Hydrograph type = Combine Peak discharge = 23.09 cfsStorm frequency Time to peak = 10 yrs= 718 min Time interval = 2 min Hyd. volume = 52,853 cuftInflow hyds. = 7,8 Contrib. drain. area = 4.860 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hydrograph No. type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1 SCS Runoff	125.76	2	720	329,300				Exisitng Peripheral Drainage Area No
2 SCS Runoff	13.35	2	722	37,835				Existing Peripheral Drainage Area So
3 Combine	138.62	2	720	367,135	1, 2			Total Existing Peripheral Drainage Ar
4 SCS Runoff	26.57	2	718	63,568				Proposed North Drainage Area
5 SCS Runoff	11.03	2	718	25,847				Proposed South drainage Area
6 Combine	37.60	2	718	89,415	4, 5			Total Proposed Peripheral Drainage
7 SCS Runoff	11.03	2	718	25,847				Intermediate South drainage Area
8 SCS Runoff	35.62	2	718	82,933				Intermediate Drainage Area North
9 Combine	46.65	2	718	108,780	7, 8			Intermediate Peripheral Area

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Friday, 08 / 26 / 2022

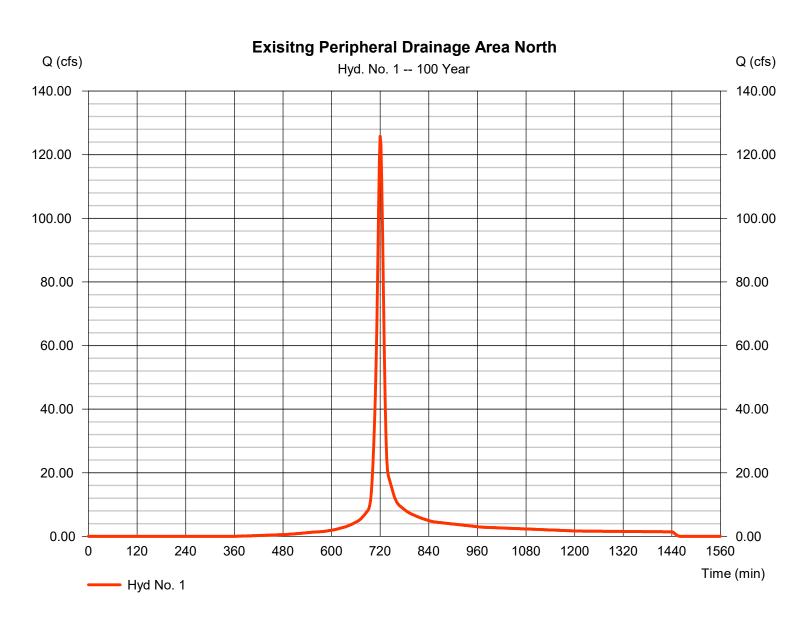
Hyd. No. 1

Exisitng Peripheral Drainage Area North

Hydrograph type = SCS Runoff Peak discharge = 125.76 cfsStorm frequency = 100 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 329.300 cuft Curve number Drainage area = 14.700 ac= 74* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 10.70 \, \text{min}$

Total precip. = 9.17 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(14.700 x 74)] / 14.700



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

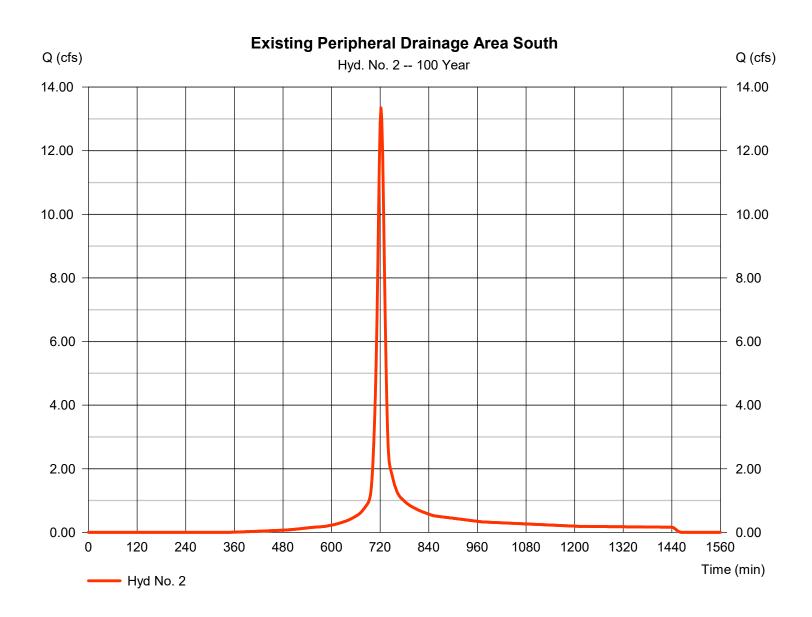
Friday, 08 / 26 / 2022

Hyd. No. 2

Existing Peripheral Drainage Area South

Hydrograph type = SCS Runoff Peak discharge = 13.35 cfsStorm frequency = 100 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 37.835 cuft = 75* = 1.750 ac Curve number Drainage area Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 14.20 \, \text{min}$ Total precip. Distribution = Type II = 9.17 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.087 \times 99) + (1.660 \times 74)] / 1.750$



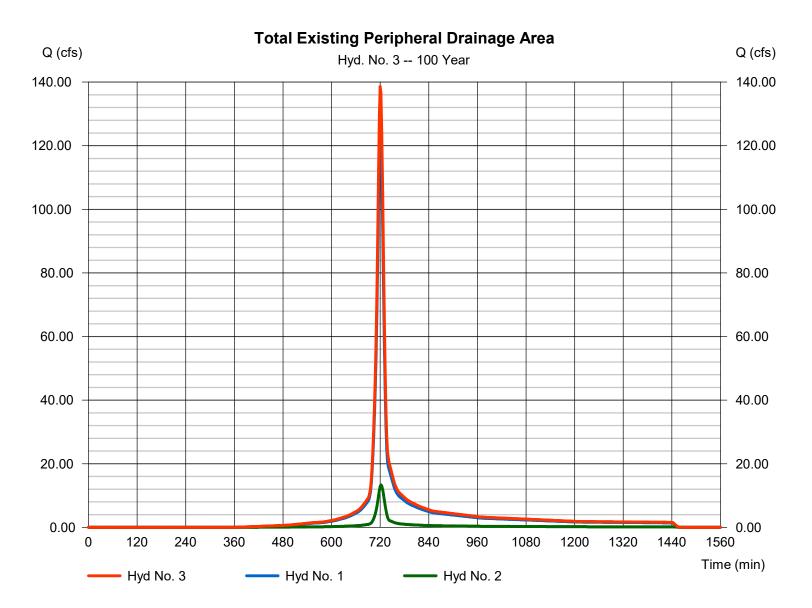
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Friday, 08 / 26 / 2022

Hyd. No. 3

Total Existing Peripheral Drainage Area

Hydrograph type = Combine Peak discharge = 138.62 cfsStorm frequency Time to peak = 100 yrs= 720 min Time interval = 2 min Hyd. volume = 367,135 cuft Inflow hyds. = 1, 2 Contrib. drain. area = 16.450 ac



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

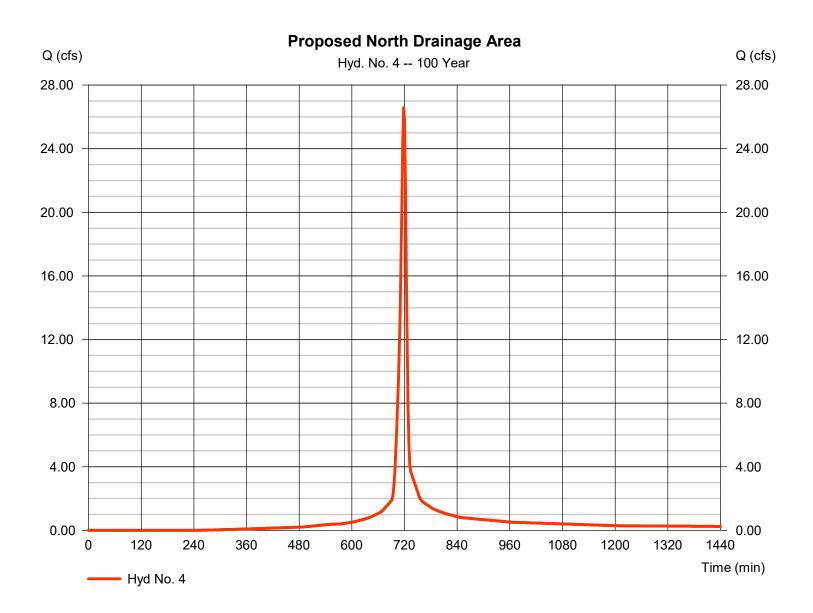
Friday, 08 / 26 / 2022

Hyd. No. 4

Proposed North Drainage Area

Hydrograph type = SCS Runoff Peak discharge = 26.57 cfsStorm frequency = 100 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 63.568 cuft Curve number Drainage area = 2.510 ac= 82* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 9.70 min = TR55 Total precip. Distribution = Type II = 9.17 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(2.510 x 82)] / 2.510



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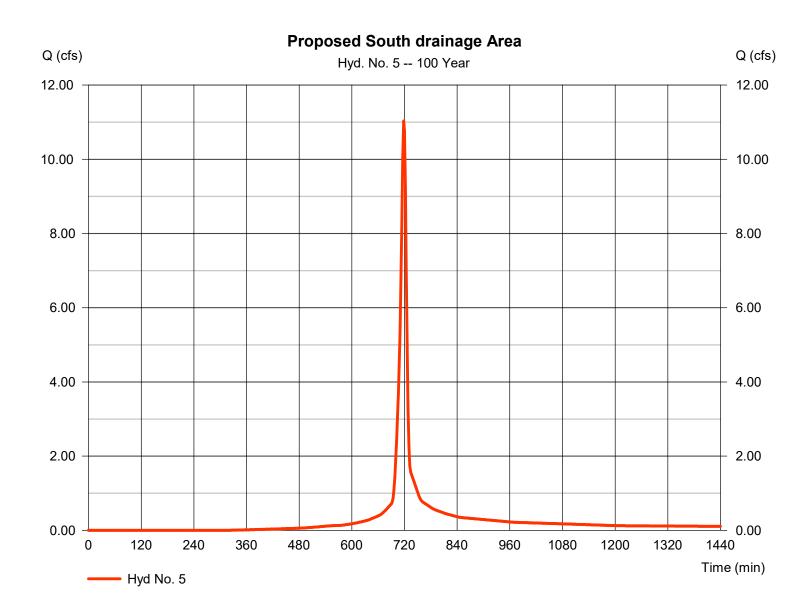
Friday, 08 / 26 / 2022

Hyd. No. 5

Proposed South drainage Area

Hydrograph type = SCS Runoff Peak discharge = 11.03 cfsStorm frequency = 100 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 25.847 cuft Curve number Drainage area = 1.120 ac= 77* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.60 min = TR55 Total precip. Distribution = Type II = 9.17 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.140 x 99) + (0.980 x 74)] / 1.120



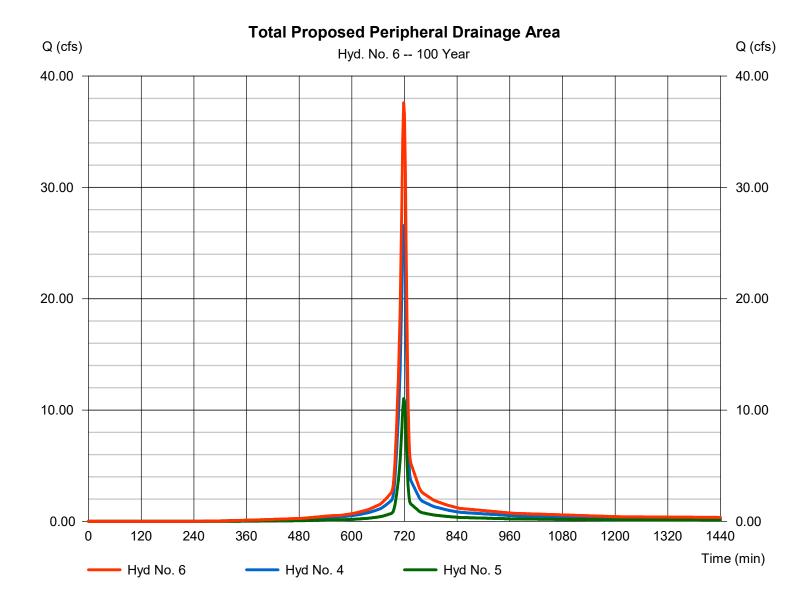
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Friday, 08 / 26 / 2022

Hyd. No. 6

Total Proposed Peripheral Drainage Area

Hydrograph type = Combine Peak discharge = 37.60 cfsStorm frequency Time to peak = 100 yrs= 718 min Time interval = 2 min Hyd. volume = 89,415 cuft Inflow hyds. = 4, 5Contrib. drain. area = 3.630 ac



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

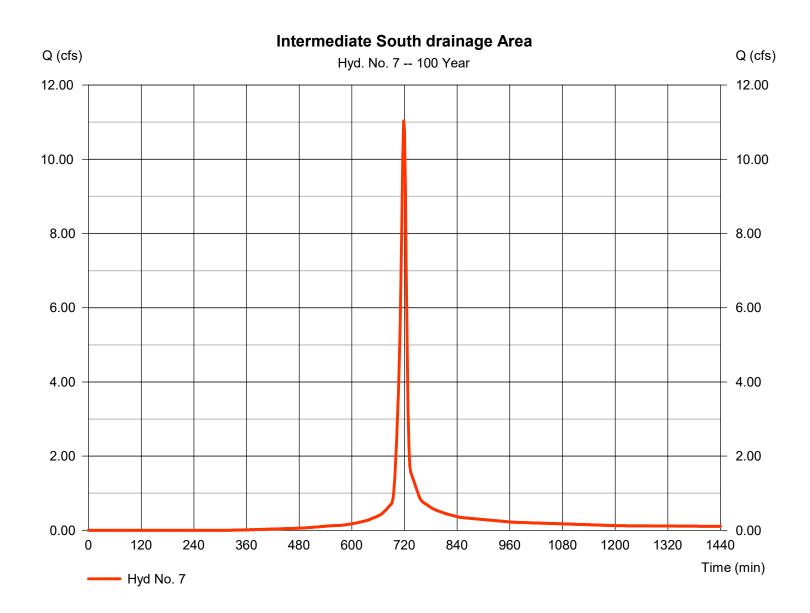
Friday, 08 / 26 / 2022

Hyd. No. 7

Intermediate South drainage Area

Hydrograph type = SCS Runoff Peak discharge = 11.03 cfsStorm frequency = 100 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 25.847 cuft Curve number Drainage area = 1.120 ac= 77* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.60 min = TR55 Total precip. Distribution = Type II = 9.17 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.140 x 99) + (0.980 x 74)] / 1.120



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

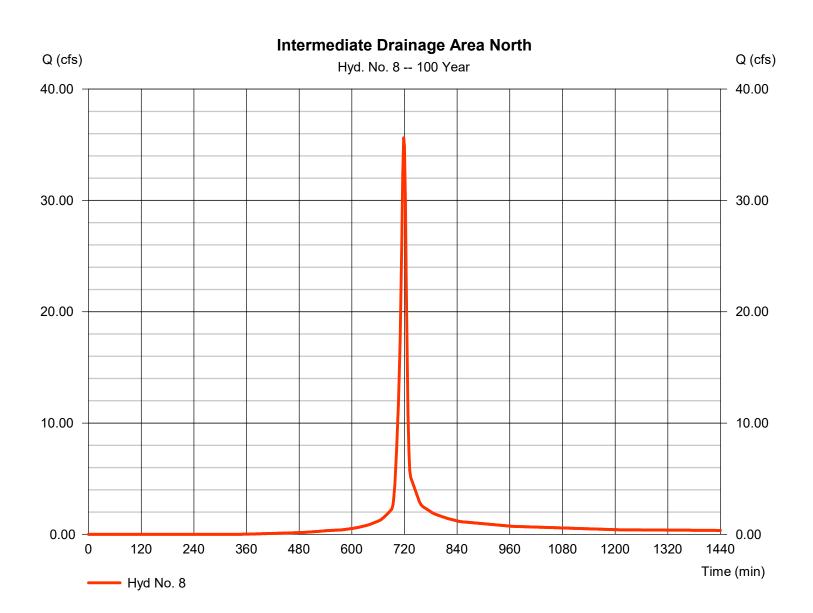
Friday, 08 / 26 / 2022

Hyd. No. 8

Intermediate Drainage Area North

Hydrograph type = SCS Runoff Peak discharge = 35.62 cfsStorm frequency = 100 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 82.933 cuft Drainage area Curve number = 75* = 3.740 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 9.80 min = TR55 Total precip. Distribution = Type II = 9.17 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.200 x 82) + (2.310 x 74)] / 3.740



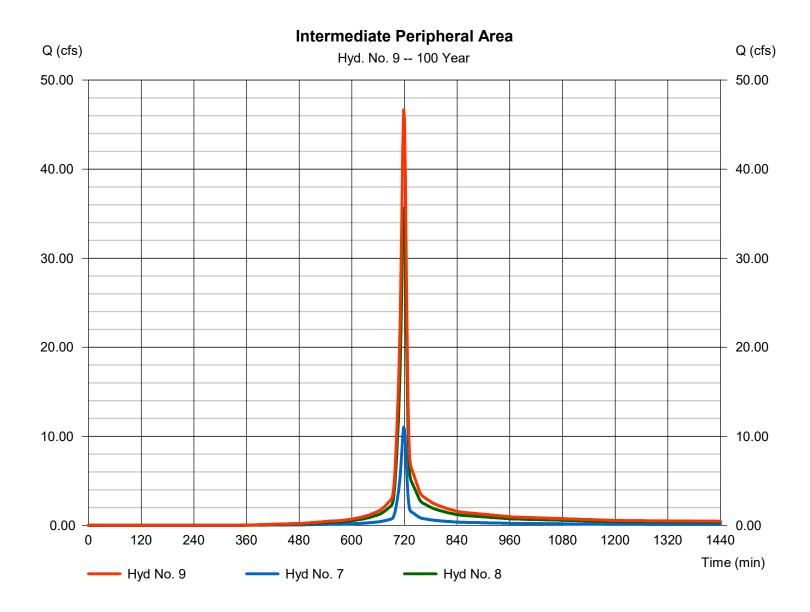
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Friday, 08 / 26 / 2022

Hyd. No. 9

Intermediate Peripheral Area

Hydrograph type = Combine Peak discharge = 46.65 cfsStorm frequency = 100 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 108,780 cuftInflow hyds. = 7,8 Contrib. drain. area = 4.860 ac



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Friday, 08 / 26 / 2022

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)						
1	0.0000	0.0000	0.0000							
2	69.8703	13.1000	0.8658							
3	0.0000	0.0000	0.0000							
5	79.2597	14.6000	0.8369							
10	88.2351	15.5000	0.8279							
25	102.6072	16.5000	0.8217							
50	114.8193	17.2000	0.8199							
100	127.1596	17.8000	0.8186							

File name: Springfield_IDF.IDF

Intensity = B / (Tc + D)^E

Intensity Values (in/hr)											
5 min	10	15	20	25	30	35	40	45	50	55	60
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60
	0.00 5.69 0.00 6.57 7.24 8.25 9.04	0.00 0.00 5.69 4.61 0.00 0.00 6.57 5.43 7.24 6.04 8.25 6.95 9.04 7.65	0.00 0.00 0.00 5.69 4.61 3.89 0.00 0.00 0.00 6.57 5.43 4.65 7.24 6.04 5.21 8.25 6.95 6.03 9.04 7.65 6.66	0.00 0.00 0.00 0.00 5.69 4.61 3.89 3.38 0.00 0.00 0.00 0.00 6.57 5.43 4.65 4.08 7.24 6.04 5.21 4.59 8.25 6.95 6.03 5.34 9.04 7.65 6.66 5.92	5 min 10 15 20 25 0.00 0.00 0.00 0.00 0.00 5.69 4.61 3.89 3.38 2.99 0.00 0.00 0.00 0.00 0.00 6.57 5.43 4.65 4.08 3.65 7.24 6.04 5.21 4.59 4.12 8.25 6.95 6.03 5.34 4.80 9.04 7.65 6.66 5.92 5.34	5 min 10 15 20 25 30 0.00 0.00 0.00 0.00 0.00 0.00 5.69 4.61 3.89 3.38 2.99 2.69 0.00 0.00 0.00 0.00 0.00 0.00 6.57 5.43 4.65 4.08 3.65 3.30 7.24 6.04 5.21 4.59 4.12 3.74 8.25 6.95 6.03 5.34 4.80 4.38 9.04 7.65 6.66 5.92 5.34 4.87	5 min 10 15 20 25 30 35 0.00 0.00 0.00 0.00 0.00 0.00 0.00 5.69 4.61 3.89 3.38 2.99 2.69 2.44 0.00 0.00 0.00 0.00 0.00 0.00 0.00 6.57 5.43 4.65 4.08 3.65 3.30 3.02 7.24 6.04 5.21 4.59 4.12 3.74 3.43 8.25 6.95 6.03 5.34 4.80 4.38 4.02 9.04 7.65 6.66 5.92 5.34 4.87 4.49	5 min 10 15 20 25 30 35 40 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 5.69 4.61 3.89 3.38 2.99 2.69 2.44 2.24 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 6.57 5.43 4.65 4.08 3.65 3.30 3.02 2.79 7.24 6.04 5.21 4.59 4.12 3.74 3.43 3.17 8.25 6.95 6.03 5.34 4.80 4.38 4.02 3.73 9.04 7.65 6.66 5.92 5.34 4.87 4.49 4.16	5 min 10 15 20 25 30 35 40 45 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 5.69 4.61 3.89 3.38 2.99 2.69 2.44 2.24 2.07 0.00 0	5 min 10 15 20 25 30 35 40 45 50 0.00 <	5 min 10 15 20 25 30 35 40 45 50 55 0.00 <td< td=""></td<>

Tc = time in minutes. Values may exceed 60.

Cobey Creek Residential\01 CIVIL\02-R&D\Drainage\SW DETENTION BASIN\COBEY CREEK RAINFALL DATA.pcp

	Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	3.09	3.68	0.00	0.00	5.61	0.00	0.00	9.17		
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		

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TR-55 Tc Worksheet	
Hydrograph No. 6, Combine, Total Proposed Peripheral Drainage Area	
Hydrograph No. 7, SCS Runoff, Intermediate South drainage Area	
TR-55 Tc Worksheet	
Hydrograph No. 8, SCS Runoff, Intermediate Drainage Area North	
TR-55 Tc WorksheetHydrograph No. 9, Combine, Intermediate Peripheral Area	
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Hydrograph No. 4, SCS Runoff, Proposed North Drainage Area	
Hydrograph No. 5, SCS Runoff, Proposed South drainage Area	
Hydrograph No. 6, Combine, Total Proposed Peripheral Drainage Area	
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Hydrograph No. 8, SCS Runoff, Intermediate Drainage Area North	
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jects\20202011101160015660001110116016ntial\01 CIVIL\02-R&D\Drainage\WAIVER REQUEST REPORT\2022.08.26 - PERIPHERAL	DRAINAGE MODEL.gpw