

Final Stormwater Management Plan

prepared for

Manor at Bailey Farms, 1st Plat

1300 SE Ranson Road Lee's Summit, MO 64081

> November 5, 2021 April 18, 2022

> > prepared by

SCHLAGEL & ASSOCIATES, P.A.

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for

Clayton Properties Group Inc., DBA Summit Homes 120 SE 30th Street Lee's Summit, Missouri



Executive Summary

April 18, 2022

Gene Williams, P.E. 220 SE Green Street Lee's Summit, MO 64063

RE: Manor at Bailey Farms, 1st Plat 1300 SE Ranson Road Lee's Summit, MO 64081

Dear Gene Williams,

We are submitting the enclosed final stormwater management study in support of the final development plan for the Manor at Bailey Farms 1st Plat. This report has been prepared to address permitting requirements and provides final design calculations for the required storm water detention and BMP facilities. We have modeled the existing site conditions as they existed at the time this report was prepared.

The proposed site is an 88.70 acre single-family proposed parcel located in Lee's Summit, MO at the intersection of SE Bailey Road and SE Ranson Road. The proposed development has been analyzed and designed to meet the APWA Comprehensive Control Strategy, which entails limiting post-development peak discharge rates from the site for the 2-Year, 10-Year, and 100-Year design storm events. Two Extended Dry Detention Basins (EDDB) have been designed to detain the mentioned events as well as provided 40-hour detention of runoff from the local 90% mean annual event. All elements of the enclosed drainage system will be designed and constructed in accordance with the City of Lee's Summit, Missouri requirements.

This project includes requesting a waiver for a small area located in the southeast corner of the site to allow free-release off-site without additional detention facilities to be implemented.

Sincerely,

Schlagel & Associates, P.A.

Nick Augustine, E.I.T. Design Engineer

Jim Long, P.E. Project Engineer

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1.0 GENERAL INFORMATION

Summit Homes is proposing to develop the 888.64 acres of land located in Section 16, Township 47 North, Range 31 West, Jackson County, Missouri. This analysis includes the 0.75 ac. Bailey Farms, Homestead located in the northeast corner of the site. The property is located at the intersection of SE Bailey Road and SE Ranson Road. The proposed development consists of single-family lots along with associated infrastructure.

1.1 OBJECTIVE

The intent of this report is to provide information pertaining to the existing and proposed watersheds, identifying and addressing any downstream drainage issues, determine and address any detention requirements, provide 40-hour extended detention of runoff from the local 90% mean annual event, and address permitting requirements. This study provides the final design calculations for the development of the facility and associated infrastructure. Detailed designs are provided with permit documents.

1.2 METHODOLOGY

The following were utilized in the assessment, preparation and analysis of watersheds in this design concept plan: Section 5600, 2011, Storm Drainage Systems & Facilities of the Standard Specifications & Design Criteria of the Kansas City Metropolitan Chapter of the American Public Works Association; City of Lee's Summit, Missouri Design Criteria (2011 Revision), Storm Drainage Systems & Facilities, prepared by the City of Lee's Summit, Missouri, Public Works Department.

Watersheds for the site were defined according to soil cover and type, tributary area, and runoff times of concentration. Soil cover was determined from inspection of the site and aerial photography. A soil survey for the project area was obtained from the United States Department of Agriculture, Natural Resources Conservation Service (NRCS), website and was utilized in determining soil type. The entire NRCS Soil Resource Report can be found in Appendix B. Watershed size was determined from both aerial topography and topographical survey, and by the proposed grading plan.

Times of concentration were compiled according to *NRCS TR-55 Urban Hydrology for Small Watersheds (1986)* methodology for sheet flow, shallow concentrated flow, and channel flow. For this report, sheet flow travel lengths were modeled at a total distance of 100 feet. Travel times for channel flows were determined using the length and velocity of the open channel. *HydroCAD-10* was utilized to model the runoff. All storm events were modeled using *SCS 24-hour Type II* distributions and were modeled for 2-Year, 10-Year, and 100-Year storm events.

2.0 EXISTING CONDITIONS ANALYSIS

The site lies within the Big Creek Watershed. The existing site contains one watershed which has a release point located at the southwest corner of the site.

Offsite stormwater comes into the site from the east and drains to the same release point previously mentioned. See Existing Offsite #1 on Sheet A.1 – Existing Drainage Map included in Appendix A.

2.1 TRIBUTARY AREAS

The existing drainage tributary areas are shown in Appendix A, Sheet A.1. The site release points have been identified as:

Release Point 1 (RP-1) - The full site release point.

Release Point 2 (RP-2) – The location where Bailey Farms site releases directly to the drainage channel that parallels the southern property line. Includes Exist. On-Site #2 area

Release Point 3 (RP-3) – The point at the southwest corner of the site. Includes Exist. On-Site #3

Release Point 4 (RP-4) – The centrally located point where the majority of the site drains too. Includes Exist. On-Site #1 and Exist. Off-Site #1 (Area east of Ranson Road) drainage areas.

The tributary areas are delineated according to the existing topography.

2.2 CURVE NUMBER AND TIME OF CONCENTRATION

The existing curve numbers and time of concentrations for each area have been established based on the procedures outlined in *NRCS TR-55 Urban Hydrology for Small Watersheds (1986)*. Existing curve numbers were based upon aerial photography, site inspection, and the soil types present on site.

Cover types for existing conditions were considered to be "pasture/grassland" in good condition for the on-site area, and "Woods/grass combo" in fair condition for the off-site area. Procedures outlined in *NRCS TR-55 Urban Hydrology for Small Watersheds* recommends utilizing curve numbers 74 and 80 for HSG C and D for pasture/grassland, and 76 and 82 for the Woods/Grass combination.

Time of concentration flow paths were based upon sheet flow and shallow concentrated flow for the existing conditions. Sheet flow lengths were limited to where a grade break occurred. Flow was then considered shallow concentrated flow until a channel was visible either from the USGS topographic map or the aerial photograph, and then from that point was considered channel flow determined by the length of the channel and the velocity of flow.

2.3 EXISTING FLOW RATES

Following TR-55 through HydroCAD the existing flow rates were determined for the 2-Year, 10-Year, and 100-Year design storms. Offsite runoff is included in the calculations for Table 2-1 below. Appropriate runoff coefficient curve numbers were based upon aerial photography, site inspection, and the soil types present on site. See NRCS Soils Report included in Appendix B. Detailed calculations with composite curve numbers and time of concentration can be found in the HydroCAD Model Output in Appendix B. Note these flow rates are for informational purposes and not used in the final analysis as the site will be designed to meet the Comprehensive Control rates as defined in APWA 5600.

Table 2-1 Existing Flow Rates

Drainage Sub- Basin	Area, ac.	CN	Storm Event	Runoff, cfs
			2-YR	56.78
Existing On-Site #1	64.82	77	10-YR	118.07
			100-YR	206.11
		76	2-YR	10.50
Existing On-Site #2	6.42		10-YR	21.81
			100-YR	38.02
		77	2-YR	18.79
Existing On-Site #3	17.54		10-YR	38.90
			100-YR	67.74
			2-YR	31.32
Existing Off-Site #1	18.43	77	10-YR	63.84
			100-YR	110.11

Referring to Sheet A.1 – Existing Drainage Map.

- RP-1 Represents the total outfall for the site which is located in the southwest corner of the site.
- RP-2 Represents the release point fir the run-off for Exist. On-Site #2 which is located in the southeast corner of the site. This area discharges into the existing drainage channel that parallels the southern property line.
- RP-3 Represents the run-off for Exist. On-Site #3 which is the point of convergence for this are just before exiting the site in the southwest corner.
- RP-4 Represents the point of convergence between the main tributary running north/south along the west property line and the minor tributary running east/west in the middle of the site. It includes the Off-Site #1.

2.4 DOWNSTREAM DRAINAGE ISSUES

The existing downstream drainage system has been reviewed with the preliminary development plan. FEMA flood maps have been checked and, currently, no immediate downstream issues appear to be present. A FEMA FIRMette is included in Appendix A. The project lies outside of the identified FEMA floodplain per map number 29095C0438G.

2.4.1 Agency Review

Permitting requirements of the following agencies were reviewed as part of the existing conditions analysis.

2.4.2 Corps of Engineers Review

The Approved jurisdictional determination map is included in Appendix A. The assessment indicates there are two wetlands and two intermittent streams on the site. Grading activites will be restricted in these areas.

2.4.3 FEMA Requirements

No FEMA identified floodplain is located on the proposed property per Flood Insurance Rate Map Panel No. 29095C0430G. There is currently no work proposed in the regulated floodplain. Please see the attached FEMA FIRMette in Appendix A, Figure A.4.

2.4.4 Missouri Department of Natural Resources

All land disturbance activities will be permitted in accordance with the City of Lee's Summit, MO specifications as well as the Missouri Department of Water Pollution Control general permit under the National Pollution Discharge Elimination System (NPDES) and an authorized Notice of Intent (NOI) application form. The disturbance of the site is greater than one acre; therefore, NPDES and NOI applications are required with the future permitting of the site in compliance with local, state and federal guidelines.

3.0 PROPOSED CONDITIONS ANALYSIS

With the proposed development, the site watershed will be divided into four sub-basins for analysis. These sub-basins are similar to the existing condition sub-basins. And are broken down accordingly, refer to Sheet A.3 – Proposed Drainage Map included in Appendix A:

- On-site #1 Which is the northern portion of the site draining to EDDB-1 which will be constructed with Cornerstone at Bailey Farms, 1st Plat.
- On-site #2 Which is the southern portion of the site draining to EDDB-2 which will be constructed with Manor at Bailey Farms,1st Plat.
- On-site #3 The southeastern portion of the site that discharges to the drainage channel that flows parallel with our southern property line.
- On-site #4 Which is the central portion of the site. It includes the drainage channel located within a stream buffer. This will be direct release, no detention for this drainage area. Off-site #1 drains through On-Site #4.

The proposed detention basins will both be dry and used for water quality compliance. Both proposed dry detention basins have been sized to detain the 2, 10, and 100-year storm events for on-site drainage. Discharge rates are based on Comprehensive Control per APWA 5608.4.C.1.a.

All components of the overland and enclosed storm sewer systems will meet or exceed the specifications provided in *Section 5600 – Storm Drainage Systems & Facilities* of the *Standard Specifications and Design Criteria* compiled by the Kansas City Metropolitan Chapter of the American Public Works Association.

A stream buffer protection zone will be located on the west side of the property and through a portion of the centrally located drainage channel. The proposed buffer zones will meet the requirements provided in APWA Section 5600. Drainage areas have been delineated to determine the required width of the stream buffer zones.

3.1 STREAM BUFFERS

Referring to Sheet A.4 – Stream Buffer Map there will be two stream buffers on the site.

- Drainage Area 1 125.2 ac. to Point 1 will require a 60' stream buffer. This buffer will extend from Bailey Farms northwest property line to Point 1.
- Sub-Drainage Area 1 37.8 ac. to Point 3 will not require a stream buffer. The drainage that is carried from the east side of Ranson Road will be transported through a storm pipe system and discharged at approximately Point 3.
- Sub-Drainage Area 2 55.1 ac. to Point 1 includes Sub-Drainage Area 1. This will require a 60' stream buffer from Point 3 to Point 1.
- Drainage Area 2 199.3 ac. to Point 2 includes all of Drainage Area 1 and the subdrainage area of 55.1 ac. This will require a 100' buffer which will extend from Point 1 to Point 2.

The buffer widths stated above are to be on each side of the channel measured from the ordinary high-water mark or surveyed top of bank.

3.2 TRIBUTARY AREAS

As stated previously the developed site will be divided into four sub-basins. Please refer to Sheet A.3 – Proposed Drainage Map included in Appendix A.

3.3 CURVE NUMBER AND TIME OF CONCENTRATION

Curve numbers for the proposed development were developed in a similar manner as the existing conditions. Hydrologic Soil Group (HSG) of D was utilized for all post-development conditions. Cover types for the proposed conditions were considered to be 1/8 acre lots, 1/4 acre lots, ½ acre lots, open space, and urban commercial (used for the amenity site) all in good condition.

Time of concentration was established in a similar manner as the existing conditions. Shallow concentrated flow lengths were shortened. Most of the shallow concentrated flow is now along paved drainage ways. Detailed calculations with composite curve

numbers and time of concentration can be found in the HydroCAD Model Output in Appendix B.

3.4 DEVELOPED RELEASE RATES

Developed release rates will follow the Comprehensive Control per APWA 5608.4.C.1.a for the 2-Year, 10-Year, and 100-Year design storms.

- 50% (2 yr) storm peak rate less than or equal to 0.5 cfs per site acre
- 10% (10 yr) storm peak rate less than or equal to 2.0 cfs per site acre
- 1% (100 yr) storm peak rate less than or equal to 3.0 cfs per site acre

The offsite release rates for Off-Site #1 will be calculated using HydroCAD and added to the site developed release rates under Comprehensive Control.

Table 3-1 Allowable Release Rate Calculations

	Allowable Release Rate Calculations				
Release Point	Area (Acres)	Storm Event	Allowable On- Site Release Rate, cfs (A)	Allowable Off- Site Release Rate, cfs (B)	Allowable Release Rate, cfs (A+B)
Total Site		2-YR	44.35	30.84	75.19
RP-1	88.64	10-YR	177.4	62.92	240.32
See Note 1		100-YR	266.1	108.57	374.67
South Bypass		2-YR	8.14	0	8.14
RP-2	2.38	10-YR	11.39	0	11.39
See Note 2		100-YR	20.48	0	20.48
Manor		2-YR	14.67	0	14.67
RP-3	29.34	10-YR	58.68	0	58.68
EDDB-2 (5P- HydroCAD)		100-YR	88.02	0	88.02
Cornerstone and Retreat		2-YR	15.86	0	15.86
RP-4	31.71	10-YR	63.42	0	63.42
EDDB-1 (4P in HydroCAD)		100-YR	95.13	0	95.13

- Note 1. Release rate for the offsite area of 18.43 ac. is generated from HydroCAD. See HydroCAD output included with this study. (5S OFF-SITE #1 in HydroCAD).
- Note 2. Release rate based on the existing drainage area of 6.42 ac. using the rational method (Q=kCiA). Calculations are in Section 3.4. Note that in the evaluation of the total run-off from the site this drainage area was evaluated under the comprehensive control rates.

3.5 DETENTION AND WATER QUAILTY ANALYSIS

The runoff hydrographs utilized to determine the peak flow volumes for each tributary area were determined using *TR-55* methodology and *HydroCAD-10*. For the 2-Year, 10-Year, and 100-Year storm events, the complete hydrograph routing and model output can be found in the HydroCAD Model Output Report in Appendix B.

Detention will be provided in two locations:

- EDDB 1 Will provide detention for the northern drainage areas and will be constructed with Cornerstone at Bailey Farms, 1st Plat.
- EDDB 2 Will provide detention for the southern drainage areas and will be constructed with Manor at Bailey Farms, 1st Plat.

Both detention basins will be sized to comply with the Comprehensive Control Rates from the total development of the Bailey Farms subdivision.

3.5.1 EDDB-1 for Cornerstone at Bailey Farms, 1st Plat

This analysis of the EDDB 1 is provided for informational purposes. Refer to the stormwater report specific to EDDB 1 "Cornerstone at Bailey Farms, 1st Plat" for the final analysis.

This is the analysis of the detention and water quality to be constructed with Cornerstone at Bailey Farms, 1st Plat.

Referring to Sheet A.3 – Proposed Drainage Map, the proposed drainage area to this detention basin is 31.71 acres. Using HydroCAD, the design storms for the 2, 10 and 100 year storms are routed through the basin to reduce the peak storm rates to meet the comprehensive control requirements. The control structure will be a combination of culverts, weirs associated with a storm inlet and an orifice whose primary function is to control the water quality event. A perforated standpipe will be utilized to help keep the orifice plate from clogging. See the attached Plan Sheet 34 – Detention Basin Details for details on the control structure.

The stage-storage and discharge rates are shown in the table below. Additional information can be found in the HydroCAD output located in Appendix B

Table 3-2 Detention Basin Stage-Storage-Discharge Rate Table

		Surface Area, sf	Incremental Storage, cf	Cummulative Storage, cf	Discharge Rate, cfs
Orifice FL	994.16	0	0	0	0.00
FL at Riser	995.67	0	0	0	0.00
	996.00	700	115	115	0.10
	997.00	7,200	3,950	4,065	0.37
	998.00	12,000	9,600	13,665	0.42
	999.00	17,900	14,950	28,615	0.48
	1000.00	27,000	22,450	51,065	0.53
	1001.00	36,000	31,500	82,565	0.60
	1002.00	40,000	38,000	120,565	7.62
	1003.00	44,800	42,400	162,965	13.20
	1004.00	48,700	46,750	209,715	17.34
	1005.00	53,000	50,850	260,565	75.51
	1006.00	57,000	55,000	315,565	79.13
	1007.00	61,000	59,000	374,565	82.00

Storm water run-off is delivered to the detention via a storm sewer pipe system and overland flow. Below is the inflow-outflow summary table with water surface elevations for each of the design storms.

Table 3-3 Detention Basin Inflow/Outflow Summary

Release Point	Area (Acres)	Storm Event	Inflow to Basin, cfs	Discharge Rate from Basin, cfs	Water Surface Elevation
Cornerstone		2-YR	107.19	13.32	1003.03
RP-4	04.74	10-YR	173.27	60.26	1004.79
EDDB-1	31.71	100-YR	260.19	81.35	1006.64
(4P In HydroCAD)		WQV	29.17	0.54	1000.17

Water Quality

Refer to the MARC-BMP Manual worksheets for extended dry detention basins included in Appendix C for design of the water quality outlet orifice. Per the Worksheet, the water quality outlet calculates to require a 3.53" orifice. This design will use a 2.90" orifice resulting in a maximum discharge rate of 0.54 cfs. Meeting the MARC-BMP requirement shown in the worksheet included in Appendix A.

3.5.2 EDDB-2 for Manor at Bailey Farms, 1st Plat

This is the analysis of the detention and water quality to be constructed with Manor at Bailey Farms, 1st Plat.

Referring to Sheet A.3 – Proposed Drainage Map, the proposed drainage area to this detention basin is 29.69 acres. Using HydroCAD, the design storms for the 2, 10 and 100 year storms are routed through the basin to reduce the peak storm rates to meet the comprehensive control requirements. The control structure will be a combination of culverts, weirs associated with a storm inlet and an orifice whose primary function is to control the water quality event. A perforated standpipe will be utilized to help keep the orifice from clogging. See the attached Plan Sheet 34 – Detention Basin Design for details on the control structure.

The stage-storage and discharge rates are shown in the table below. Additional information can be found in the HydroCAD output located in Appendix B.

Table 3-4 Detention Basin Stage-Storage-Discharge Rate Table

		Surface Area, sf	Incremental Storage, cf	Cummulative Storage, cf	Discharge Rate, cfs
Orifice FL	986.62	0	0	0	0.00
FL at Riser	988.16	0	0	0	0.00
	989.00	800	336	336	0.24
	990.00	4,200	2,500	2,836	0.30
	991.00	10,400	7,300	10,136	0.34
	992.00	21,600	16,000	26,136	0.38
	993.00	37,600	29,600	55,736	0.41
	994.00	41,700	39,650	95,386	5.75
	995.00	46,000	43,850	139,236	11.63
	996.00	49,700	47,850	187,086	53.08
	997.00	53,200	51,450	238,536	73.80
	998.00	56,900	55,050	293,586	77.49
	999.00	60,400	58,650	352,236	82.00

Storm water run-off is delivered to the detention via a storm sewer pipe system and overland flow. Below is the inflow-outflow summary table with water surface elevations for each of the design storms.

Table 3-5 Detention Basin Inflow/Outflow Summary

Release Point	Area (Acres)	Storm Event	Inflow to Basin, cfs	Discharge Rate from Basin, cfs	Water Surface Elevation
Manor		2-YR	86.05	9.97	994.63
RP-3	20.24	10-YR	148.68	58.14	996.06
EDDB2	29.34	100-YR	232.03	76.98	997.86
(5P In HydroCAD)		WQV	17.13	0.38	992.24

Water Quality

Refer to the MARC-BMP Manual worksheets for extended dry detention basins included in Appendix C for design of the water quality outlet orifice. Per the Worksheet, the water quality outlet calculates to require a 3.22" orifice. This design will require a 2.50" orifice resulting in a maximum discharge rate of 0.54 cfs. Meeting the MARC-BMP requirement shown in the worksheet included in Appendix A.

3.6 DRAINAGE AREA FOR ON-SITE #3 AT RELEASE POINT #2

On-site #3 will be released un-detained into the existing drainage channel to the south. Referring to Sheet A1 – Existing Drainage Map and Sheet A2 – Developed Drainage Area the pre-developed drainage area is 6.42 acres. The developed drainage area will reduce this to 2.38 acres. Table 3.6 shows the comprehensive rates compared to the discharge rates from HydroCAD. It can be seen that the 2 year rates have not been reduced to meet the comprehensive requirements.

Table 3-6 Comprehensive Rates v. Developed Discharge Rates

	Area, ac.	2-yr, cfs	10-yr, cfs	100-yr, cfs
Pre-Developed, Comprehensive	6.42	3.21	12.84	19.26
Developed, HydroCAD	2.38	7.15	12.33	19.23
Difference		-3.94	0.51	0.03

Using the rational-method to analyze this drainage area shows a significant reduction in stormwater run-off. See Table 3.8 below. This is expected as there is more than a 60% reduction in the drainage area. Also, note that the developed drainage area will be only the homes themselves. The streets will all be captured by a storm sewer system and conveyed to the EDDB-2 at the southwest corner of the site.

Table 3-7 Pre-Developed and Developed Discharge Rates

Pre-Developed Run-Off Calculations							
	Tc, min. Storm Rainfall Q, cfs Event Intensity, in/hr						
Tc, min.	16.7	2-yr	3.52	8.14			
C-factor ¹	0.36	10-yr	4.93	11.39			
Drainage Area, ac. 6.42 100-yr 7.09 20.40				20.48			
1 Equivalent c-factor based on a CN = 76 from TR-55 and Soil Survey							
	Develope	d Run-Off C	alculations				
Tc, min. Storm Rainfall Q, cfs Event Intensity, in/hr							
Tc, min.	12.7	2-yr	4.01	4.87			
C-factor	0.51	10-yr	5.56	6.75			
Drainage Area,	2.38	100-yr	7.88	11.96			

ac.

3.7 TOTAL SITE DISCHARGE

To meet Comprehensive Control rates as stated in APWA 5600 the two detention basins have been designed to enable the entire site to meet these rates. Referring to Sheet A.3 – Proposed Drainage there are two drainage areas that will be un-detained - On-Site #3 and On-Site #4. Detention has been designed to adequately control and reduce the total run-off from the site to the Comprehensive Control rates.

The Off-Site #1 drainage area collects at a point on the east side of Ranson Road where a culvert conveys the run-off to the Bailey Farms site, through On-Site #4.

Table 3-8 Required & Proposed Runoff Comparison

Release Point	Area (Acres)	Storm Event	Allowable Release Rate, cfs	Proposed Release Rate, cfs
		2-YR	75.19	75.14
Total Site RP-1	88.64	10-YR	240.32	214.00
		100-YR	374.67	369.46
		2-YR	8.14	7.15
South Bypass RP-2	2.38	10-YR	11.39	12.33
1 2		100-YR	20.48	19.23
Manor		2-YR	14.67	9.97
RP-3	29.34	10-YR	58.68	58.14
EDDB-2 (5P- HydroCAD)		100-YR	88.02	76.98
Cornerstone and Retreat		2-YR	15.86	13.32
RP-4	31.71	10-YR	63.42	60.26
EDDB-1 (4P in HydroCAD)		100-YR	95.13	81.35

It can be seen in Table 3.9 that the developed site has been designed to reduce the additional run-off peak flow rates to the Comprehensive Control Rates:

Please note: Site release rates are not a direct addition of sub-basin runoff due to differences in the time peak as well as storage effects within the basins.

Stormwater runoff for On-Site #1 is mitigated and detained by Extended Dry Detention Basin 1 located on the west side of the property and discharges to RP-4. Stormwater runoff for On-Site #2 is mitigated and detained by Extended Dry Detention Basin 2 located in the southwest corner of the site and discharges to RP-3. Stormwater run-off from On-Site #4 free releases from the site at RP-3. RP-2, RP-3, and RP-4 all converge to RP-1 located in the southwest corner of the site.

Proposed stormwater drainage structures will be located throughout the site to capture and convey proposed stormwater runoff to both dry detention basins. The Water Quality volume for both basins will be released over 40 hours. Water quality outlet structures have been provided for each basin and have been designed to meet the allowable release rates provided in Table 3-2 for the 2, 10, and 100 year storm events. The water quality storm event will be controlled with an orifice located in the 4'x5' storm structures. Perforated riser pipe will be utilized to help reduce the clogging at the orifices.

Emergency spillways haven been provided for each basin per Section 5600 of the Design and Construction Manual. Each Emergency Spillway will be set 0.5 feet above the 100-year water surface elevation and designed to carry the 100-year storm event assuming a 100% clogged condition and full basin. An additional 1 foot of freeboard will be provided from the water surface elevation in the spillway to the top of dam. Using HydroCAD, all primary discharge devices have been removed to simulate the clogged situation. The water surface elevation was set at the flowline of the emergency spillway to simulate that the basin is completely full prior to the storm event. Table 3-9 below summarizes the analysis. HydroCAD output is included in Appendix B. Also, refer to the included Sheet 34 – Detention Basin Design of the Manor at Bailey Farms, 1st Plat for detailed calculations.

Table 3-9 Emergency Spillway Analysis

	Storm Event	Inflow to Basin, cfs	Emergency Spillway Elevation	Emergency Spillway Length, ft	Clogged Surface Elevation
Manor	100-YR	232.03	998.36	160.0	998.92
Cornerstone	100-YR	260.19	1007.14	216.0	1007.64

Release Point 2 (RP-2) does not meet the allowable release rate required by the comprehensive control requirements for the 2-year storm event. This area has been reduced from 6.42 acres to 2.38 acres and consists of houses and their backyards only. Given this reduction in drainage area and the entire site is collectively meeting comprehensive control requirements, we will be requesting a waiver for this peripheral drainage area to avoid adding additional detention facilities.

4.0 SUMMARY AND RECOMMENDATIONS

The proposed drainage site is an 88.70-acre single-family parcel of land located in Lee's Summit, MO at the intersection of SE Bailey Road and SE Ranson Road. The proposed development has been analyzed and designed to meet the APWA Comprehensive Control Strategy, which entails limiting post-development peak discharge rates from the site for the 2-Year, 10-Year, and 100-Year design storm events. Two extended dry detention basins have been designed to detain the mentioned events as well as provided 40-hour detention of runoff from the local 90% mean annual event. All elements of the enclosed drainage system will be designed and constructed in accordance with all City of Lee's Summit, Missouri, requirements.

The project includes a request for modification to the stream setback to allow transition grading. There is sufficient space to allow for the existing stream to flow naturally. Any proposed construction activities with this development will not disrupt the natural movement of this stream. This project is also requested a waiver for a small area located in the southeast corner of the site to allow free-release off-site without additional detention facilities to be implemented.

* * * *

Appendix A

Sheet A.1 – Existing Conditions Aerial Map

Sheet A.2 - Existing Drainage Map

Sheet A.3 - Proposed Drainage Map

Sheet A.4 – Stream Corridor Map

EDDB Water Quality Design

Figure A.4 - FEMA FIRMette

Figure A.5 Approved Jurisdictional Determination Map

1300 SE RANSON ROAD LEE'S SUMMIT, MISSOURI

EXISTING CONDTIONS AERIAL MAP

SHEET

A.1



SCALE:1" = 250'





BAILEY FARMS HYDRO MAPS 1300 SE RANSON ROAD LEE'S SUMMIT, MISSOURI

JLL
DATE PREPARED:
4/18/2022
PROJ. NUMBER:

STREAM CORRIDOR MAP

SHEET

A.4

SCALE:1" = 400'

Extended Dry Detention Basin

(EDDB-1 To be reviewed during construction of Cornerstone at Bailey Farms, 1st Plat) **Water Quality Volume Calculation**

WQV = P * Weighted RV

WQV - Water Quality Volume (watershed-inches)
P - Rainfall Event (1.37 inches in Kansas City)

RV - Volumetric Runoff Coefficient

RV = 0.05 + 0.009(I)

I - Percent Site Imperviousness (%)

I. Determine Weighted RV & Weighted Rational C Coefficient

			Total	Rational			
	%	Area	Impervious	Runoff			
Cover Type	Impervious	(Ac.)	Area (Ac.)	Coefficient	RV	C * Area	RV * Area
1/8 Acre Lots	65	11.14	7.24	0.66	0.64	7.35	7.07
1/4 Acre Lots	38	19.33	7.35	0.66	0.39	12.76	7.58
Commercial/Clubhouse Area	85	1.24	1.05	0.81	0.82	1.00	1.01
Total		31.71	15.64			21.11	15.66

Rv = Sum(Rv*A)/Total Area = 15.66 / 31.71 = 0.494

C = Sum(C*A)/Total Area = 21.11 / 31.71 = 0.666

II. Determine Water Quality Volume

WQV = P * Rv = 1.37 * 0.4939 **0.677 in**

III. Determine Total Water Quality Volume

Total Watershed Area (AT) = 31.71 acres WQV = 0.677 in

WQV = (31.71 * 0.676)/12 = **1.79 ac-ft**

	Design Procedure Form:Extended Dry Deten Main Worksheet	tion Basin (EDDB)	
Designer: Checked t Company: Date: Project: Location:	,	EDDB -1 be reviewed with constructions at Bailey Farms,	
I. Basin Water (Quality Storage Volume:		
Step 1) Tributar	y Area to EDDB, A _T (ac.)	A _T (ac.) =	31.71
Step 2) Calculat	e WQv using method in Section 6.1	WQv (ac-ft) =	1.79
Step 3) Add 20	percent to account for silt and sand sediment deposition in the basi	n V _{design} (ac-ft) =	2.15
Туре Туре	e 1 = Single Orifice e 2 = Perforated riser or plate e 3 = v-notch weir I to step 2b, 2c, or 2d based on water quality outlet type		
	ity Outlet, Single Orifice	7 (0)	4.50
Step 1) Depth o	f water quality volume at outlet, Z_{WQ} (ft.)	Z_{WQ} (ft.) =	4.50
Step 1) Depth o		Z_{WQ} (ft.) = H_{WQ} (ft.) =	2.25
Step 1) Depth o Step 2) Average H _{WQ} Step 3) Average	f water quality volume at outlet, Z_{WQ} (ft.) head of Water Quality volume over invert of orifice, H_{WQ} (ft)		
Step 1) Depth of Step 2) Average H_{WQ} Step 3) Average Q_{WQ} Step 4) Set value $C_{Q} = 0$	f water quality volume at outlet, Z_{WQ} (ft.) head of Water Quality volume over invert of orifice, H_{WQ} (ft) $= 0.5 * Z_{WQ}$ water quality outflow rate, Q_{WQ} (cfs)	H _{WQ} (ft.) =	2.25
Step 1) Depth of Step 2) Average H_{WQ} Step 3) Average Q_{WQ} Step 4) Set value $C_{Q} = C_{Q} = 0$ Step 5) Water q	f water quality volume at outlet, Z_{WQ} (ft.) head of Water Quality volume over invert of orifice, H_{WQ} (ft) $= 0.5 * Z_{WQ}$ water quality outflow rate, Q_{WQ} (cfs) $= (WQv * 43,560)/(40 * 3600)$ he of orifice discharge coefficient, C_{Q} 0.66 when thickness of riser/weir plate is = or < orifice diameter	H_{WQ} (ft.) = Q_{WQ} (cfs) =	0.54
Step 1) Depth of Step 2) Average Hwo Step 3) Average Qwo Step 4) Set valu Co = Co = Step 5) Water quantum Do =	f water quality volume at outlet, Z_{WQ} (ft.) he head of Water Quality volume over invert of orifice, H_{WQ} (ft) $= 0.5 * Z_{WQ}$ water quality outflow rate, Q_{WQ} (cfs) $= (WQv * 43,560)/(40 * 3600)$ he of orifice discharge coefficient, C_{Q} 0.66 when thickness of riser/weir plate is = or < orifice diameter 0.80 when thickness of riser/weir plate is > orifice diameter uality outlet orifice diameter (4.0-in, min.), D_{Q} (in)	H_{WQ} (ft.) = Q_{WQ} (cfs) = C_{O} = D_{O} (in) =	2.25 0.54 0.66 3.53 calculated 2.9 used
Step 1) Depth of Step 2) Average Hwa Step 3) Average Qwa Step 4) Set valu Co = Co = Step 5) Water q Do = Step 6) To size	f water quality volume at outlet, Z_{WQ} (ft.) he head of Water Quality volume over invert of orifice, H_{WQ} (ft) water quality outflow rate, Q_{WQ} (cfs) (wQv * 43,560)/(40 * 3600) e of orifice discharge coefficient, C_{Q} 0.66 when thickness of riser/weir plate is = or < orifice diameter 0.80 when thickness of riser/weir plate is > orifice diameter uality outlet orifice diameter (4.0-in, min.), D_{Q} (in) 12 * 2 * $(Q_{WQ}/C_{Q}$ * π * $(2$ * g *H) $^{0.5}$)) $^{0.5}$	H_{WQ} (ft.) = Q_{WQ} (cfs) = C_{O} = D_{O} (in) =	2.25 0.54 0.66 3.53 calculated 2.9 used
Step 1) Depth of Step 2) Average HwQ Step 3) Average QwQ Step 4) Set valu Co = Co = Step 5) Water q Do = Step 6) To size	f water quality volume at outlet, Z_{WQ} (ft.) head of Water Quality volume over invert of orifice, H_{WQ} (ft) water quality outflow rate, Q_{WQ} (cfs) (wQv * 43,560)/(40 * 3600) e of orifice discharge coefficient, C_{Q} 0.66 when thickness of riser/weir plate is = or < orifice diameter 0.80 when thickness of riser/weir plate is > orifice diameter uality outlet orifice diameter (4.0-in, min.), D_{Q} (in) 12 * 2 * $(Q_{WQ}/C_{Q}$ * π * $(2 * g * H)^{0.5}))^{0.5}$ outlet orifice for EDDB with an irregular stage-volume relationship,	H_{WQ} (ft.) = Q_{WQ} (cfs) = C_{O} = D_{O} (in) =	2.25 0.54 0.66 3.53 calculated 2.9 used
Step 1) Depth of Step 2) Average Hwq Step 3) Average Qwq Step 4) Set valu Co = Co = Step 5) Water q Do = Step 6) To size Ilc. Water Quality Step 1) Depth at Step 2) Recomm	f water quality volume at outlet, Z_{WQ} (ft.) head of Water Quality volume over invert of orifice, H_{WQ} (ft) $= 0.5 * Z_{WQ}$ water quality outflow rate, Q_{WQ} (cfs) $= (WQv * 43,560)/(40 * 3600)$ he of orifice discharge coefficient, C_{Q} 0.66 when thickness of riser/weir plate is = or < orifice diameter 0.80 when thickness of riser/weir plate is > orifice diameter uality outlet orifice diameter (4.0-in, min.), D_{Q} (in) $= 12 * 2 * (Q_{WQ}/C_{Q} * \pi * (2 * g * H)^{0.5}))^{0.5}$ outlet orifice for EDDB with an irregular stage-volume relationship,	H_{WQ} (ft.) = Q_{WQ} (cfs) = Q_{Q	2.25 0.54 0.66 3.53 calculated 2.9 used

Step 4) Number of Columns, n _c	n _c =	20.00
Step 5) Design circular perforation diameter (should be between 1 and 2 inches), Derf	(in) P _{perf} (in) =	1.00
Step 6) Horizontal perforation column spacing when η_c > 1, center to center, S_c If D_{perf} >/= 1.0 in, S_c =4	S _c (in)=	4.00
Step 7) Number of rows (4" vertical spacing between perforations, center to center), n	n _r =	13
lb. Water Quality Outlet, V-notch Weir		
Step 1) Depth of water quality volume at outlet, Z_{WQ} (ft.)	Z_{WQ} (ft.) =	4.50
Step 2) Average head of Water Quality volume over invert of V-notch, H_{WQ} (ft) $H_{WQ} = 0.5 * Z_{WQ}$	H _{WQ} (ft.) =	2.25
Step 3) Average water quality outflow rate, Q_{WQ} (cfs) $Q_{WQ} = (WQv * 43,560)/(40 * 3600)$	Q _{WQ} (cfs) =	0.54
Step 4) V-notch weir coefficient, C _V	C _V =	2.69
Step 5) V-notch weir angle, θ (deg) $\theta = 2*(180/\pi)*\arctan(Q_{WQ}/C_V*H_{WQ}^{5/2}))$ V-notch angle should be at least 20 degeres. Set to 20 degrees if calculated angle is smaller.	θ (deg) =	20.0
Step 6) Top width of V-notch weir $W_V = 2 * Z_{WQ} * TAN(\theta/2)$	W _V =	1:59
Step 7) To calculate v-notch angle for EDDB with and irregular stage-volume relations	ship, use the V-notch We	ir Worksheet

III. Flood Control

Refer to APWA Specifications Section 5608

Extended Dry Detention Basin

(EDDB-1 To be reviewed during construction of Manor at Bailey Farms, 1st Plat) **Water Quality Volume Calculation**

WQV = P * Weighted RV

WQV - Water Quality Volume (watershed-inches)
P - Rainfall Event (1.37 inches in Kansas City)

RV - Volumetric Runoff Coefficient

RV = 0.05 + 0.009(I)

I - Percent Site Imperviousness (%)

I. Determine Weighted RV & Weighted Rational C Coefficient

Total		29.69	11.28			19.60	11.64
1/4 Acre Lots	38	29.69	11.28	0.66	0.39	19.60	11.64
Cover Type	Impervious	(Ac.)	Area (Ac.)	Coefficient	RV	C * Area	RV * Area
	%	Area	Impervious	Runoff			
			Total	Rational			

Rv = Sum(Rv*A)/Total Area = 11.63 / 29.69 = 0.392

 $C = Sum(C^*A)/Total Area = 19.59 / 29.69 = 0.660$

II. Determine Water Quality Volume

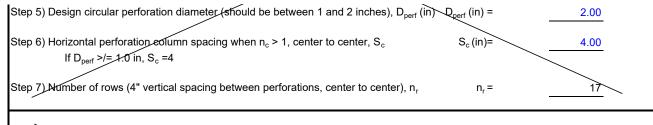
WQV = P * Rv = 1.37 * 0.392 = 0.537 in

III. Determine Total Water Quality Volume

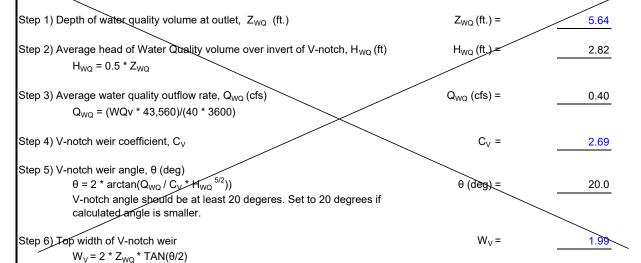
Total Watershed Area (AT) = 29.69 acres WQV = 0.537 in

WQV = (29.69 * 0.537)/12 = **1.33 ac-ft**

	Design Procedure Form:Extended Dry Dete Main Worksheet	ntion Basin (EDDB)	
Designer:	N. AUGUSTINE		
Checked k	J. LONG		
Company:	Schlagel	EDDB -2	
Date:		o be reviewed with constru	ection of
Project:	,	anor at Bailey Farms, 1st F	
Location:	Lee's Summit	anor at balley raims, 19t1	iat)
I. Basin Water Qual	ity Storage Volume:		
Step 1) Tributary Are	a to EDDB, A _T (ac.)	A _T (ac.) =	29.69
Step 2) Calculate W0	Qv using method in Section 6.1	WQv (ac-ft) =	1.33
Step 3) Add 20 perce	ent to account for silt and sand sediment deposition in the bas	sin V _{design} (ac-ft) =	1.59
13.8			
Ila. Water 2	7		
Type 1 =	Single Orifice		
• • •	Perforated riser or plate		
	v-notch weir		
Step 2) Proceed to s	tep 2b, 2c, or 2d based on water quality outlet type		
IIb. Water Quality O	utlet. Single Orifice		
	utlet, Single Orifice		
	utlet, Single Orifice er quality volume at outlet, Z_{WQ} (ft.)	Z_{WQ} (ft.) =	3.62
Step 1) Depth of wat	er quality volume at outlet, Z_{WQ} (ft.)		
Step 1) Depth of wate	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft)	Z_{WQ} (ft.) = H_{WQ} (ft.) =	3.62 1.81
Step 1) Depth of wat	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft)		
Step 1) Depth of wate Step 2) Average hea H _{WQ} = 0.5	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$	H_{WQ} (ft.) =	1.81
Step 1) Depth of wate Step 2) Average hea H _{WQ} = 0.5 Step 3) Average wate	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) 5 * Z_{WQ} er quality outflow rate, Q_{WQ} (cfs)		
Step 1) Depth of wate Step 2) Average hea H _{WQ} = 0.5 Step 3) Average wate	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$	H_{WQ} (ft.) =	1.81
Step 1) Depth of wate Step 2) Average hea $H_{WQ} = 0.5$ Step 3) Average wate $Q_{WQ} = (W_{QQ})$	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $(Q_V * 43,560)/(40 * 3600)$	H_{WQ} (ft.) = Q_{WQ} (cfs) =	0.40
Step 1) Depth of water Step 2) Average hear $H_{WQ} = 0.5$ Step 3) Average water $Q_{WQ} = (W_{Q})$ Step 4) Set value of Q_{WQ}	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $/Qv * 43,560$ / $/(40 * 3600)$ orifice discharge coefficient, C_{Q}	H_{WQ} (ft.) =	1.81
Step 1) Depth of water Step 2) Average hear $H_{WQ} = 0.5$ Step 3) Average water $Q_{WQ} = (W_{Q})$ Step 4) Set value of $C_{Q} = 0.66$	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $/Qv * 43,560$)/($40 * 3600$) orifice discharge coefficient, C_{Q} when thickness of riser/weir plate is = or < orifice diameter	H_{WQ} (ft.) = Q_{WQ} (cfs) =	0.40
Step 1) Depth of water Step 2) Average hear $H_{WQ} = 0.5$ Step 3) Average water $Q_{WQ} = (W_{Q})$ Step 4) Set value of $C_{Q} = 0.66$	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $/Qv * 43,560$ / $/(40 * 3600)$ orifice discharge coefficient, C_{Q}	H_{WQ} (ft.) = Q_{WQ} (cfs) =	0.40
Step 1) Depth of water Step 2) Average hear $H_{WQ} = 0.5$ Step 3) Average water $Q_{WQ} = (W_{Q})$ Step 4) Set value of $C_{Q} = 0.66$ $C_{Q} = 0.80$	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $/Qv * 43,560$)/($40 * 3600$) orifice discharge coefficient, C_{Q} when thickness of riser/weir plate is = or < orifice diameter when thickness of riser/weir plate is > orifice diameter	$H_{WQ} (ft.) =$ $Q_{WQ} (cfs) =$ $C_{O} =$	0.40 0.66
Step 1) Depth of water Step 2) Average hear $H_{WQ} = 0.5$ Step 3) Average water $Q_{WQ} = (W)$ Step 4) Set value of $C_{O} = 0.66$ $C_{O} = 0.80$ Step 5) Water quality	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $/Qv * 43,560$)/($40 * 3600$) orifice discharge coefficient, C_{Q} when thickness of riser/weir plate is = or < orifice diameter when thickness of riser/weir plate is > orifice diameter outlet orifice diameter (4.0-in, min.), D_{Q} (in)	H_{WQ} (ft.) = Q_{WQ} (cfs) =	0.40
Step 1) Depth of water Step 2) Average hear $H_{WQ} = 0.5$ Step 3) Average water $Q_{WQ} = (W)$ Step 4) Set value of $C_{O} = 0.66$ $C_{O} = 0.80$ Step 5) Water quality	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $/Qv * 43,560$)/($40 * 3600$) orifice discharge coefficient, C_{Q} when thickness of riser/weir plate is = or < orifice diameter when thickness of riser/weir plate is > orifice diameter	$H_{WQ} (ft.) =$ $Q_{WQ} (cfs) =$ $C_{O} =$	0.40 0.66
Step 1) Depth of water Step 2) Average hear the HwQ = 0.5 Step 3) Average water QwQ = (W Step 4) Set value of CO = 0.66 CO = 0.80 Step 5) Water quality DO = 12 $^{\circ}$	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $/Qv * 43,560$)/($40 * 3600$) orifice discharge coefficient, C_{Q} when thickness of riser/weir plate is = or < orifice diameter when thickness of riser/weir plate is > orifice diameter outlet orifice diameter (4.0-in, min.), D_{Q} (in)	H_{WQ} (ft.) = Q_{WQ} (cfs) = C_{O} = D_{O} (in) =	1.81 0.40 0.66 3.22 Calculated 2.5 Used
Step 1) Depth of water Step 2) Average hear $H_{WQ} = 0.5$ Step 3) Average water $Q_{WQ} = (W$ Step 4) Set value of $C_O = 0.66$ $C_O = 0.80$ Step 5) Water quality $D_O = 12^{\frac{1}{2}}$ Step 6) To size outlet	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $(Qv * 43,560)/(40 * 3600)$ orifice discharge coefficient, C_{Q} when thickness of riser/weir plate is = or < orifice diameter when thickness of riser/weir plate is > orifice diameter outlet orifice diameter (4.0-in, min.), D_{Q} (in) $(2 * (Q_{WQ}/C_{Q} * \pi * (2 * g * H)^{0.5}))^{0.5}$ et orifice for EDDB with an irregular stage-volume relationship	H_{WQ} (ft.) = Q_{WQ} (cfs) = C_{O} = D_{O} (in) =	1.81 0.40 0.66 3.22 Calculated 2.5 Used
Step 1) Depth of water Step 2) Average hear $H_{WQ} = 0.5$ Step 3) Average water $Q_{WQ} = (W$ Step 4) Set value of $C_O = 0.66$ $C_O = 0.80$ Step 5) Water quality $D_O = 12^{\frac{1}{2}}$ Step 6) To size outlet	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $(Qv * 43,560)/(40 * 3600)$ orifice discharge coefficient, C_{Q} when thickness of riser/weir plate is = or < orifice diameter when thickness of riser/weir plate is > orifice diameter outlet orifice diameter (4.0-in, min.), D_{Q} (in) $(2 * (Q_{WQ}/C_{Q} * \pi * (2 * g * H)^{0.5}))^{0.5}$	H_{WQ} (ft.) = Q_{WQ} (cfs) = C_{O} = D_{O} (in) =	1.81 0.40 0.66 3.22 Calculated 2.5 Used
Step 1) Depth of water S tep 2) Average hear S tep 3) Average water S tep 3) Average water S tep 4) Set value of S tep 4) Set value of S tep 5) Water quality S tep 5) Water quality S tep 6) To size outles S tep 6) To size outles S tep 6) To size outles	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $(Qv * 43,560)/(40 * 3600)$ orifice discharge coefficient, C_{Q} when thickness of riser/weir plate is = or < orifice diameter when thickness of riser/weir plate is > orifice diameter outlet orifice diameter (4.0-in, min.), D_{Q} (in) $(2 * (Q_{WQ}/C_{Q} * \pi * (2 * g * H)^{0.5}))^{0.5}$ et orifice for EDDB with an irregular stage-volume relationship	H_{WQ} (ft.) = Q_{WQ} (cfs) = C_{O} = D_{O} (in) =	1.81 0.40 0.66 3.22 Calculated 2.5 Used
Step 1) Depth of water Step 2) Average hear $H_{WQ} = 0.5$ Step 3) Average water $Q_{WQ} = (W$ Step 4) Set value of $C_0 = 0.66$ $C_0 = 0.80$ Step 5) Water quality $D_0 = 12^{\circ}$ Step 6) To size outlest the step 1 Depth at outlest Step 1 Depth at outlest step 2 Average $C_0 = 0.80$ Step 1 Depth at outlest $C_0 = 0.5$ Step 2 Step 3 Depth at outlest $C_0 = 0.5$ Step 3 Depth at $C_0 = 0.5$ St	er quality volume at outlet, Z_{WQ} (ft.) d of Water Quality volume over invert of orifice, H_{WQ} (ft) $5 * Z_{WQ}$ er quality outflow rate, Q_{WQ} (cfs) $(Q_V * 43,560)/(40 * 3600)$ orifice discharge coefficient, C_Q when thickness of riser/weir plate is = or < orifice diameter when thickness of riser/weir plate is > orifice diameter voutlet orifice diameter (4.0-in, min.), D_Q (in) $(2 * 2 * (Q_{WQ}/C_Q * \pi * (2 * g * H)^{0.5}))^{0.5}$ et orifice for EDDB with an irregular stage-volume relationship utlet, Perforated Riser et above lowest perforation, Z_{WQ} (ft.)	H_{WQ} (ft.) = Q_{WQ} (cfs) = Q_{WQ} (cfs) = Q_{QQ} (in) = Q_{QQ} (in) = Q_{QQ} (ft.) = Q_{QQ}	1.81 0.40 0.66 3.22 Calculated 2.5 Used eet
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Ilb. Water Quality Outlet, V-notch Weir



Step 7) To calculate v-notch angle for EDDB with and irregular stage-volume relationship, use the V-notch Weir Worksheet

III. Flood Control

Refer to APWA Specifications Section 5608

1DWLRODO (DRRG-EDUGIDHU)51WWH







6()65287 XXXS (153.1153 XXX)53281 (\$287 L:WKRXW %DM)ORRGOHMDWLRQ % L:WK%(RUHBWK #QH\$ \$2 \$- 9 \$ 638,\$)22 \$33366 \$HIXODWRU\)ORRGZO \$2000 &000H)ORRG-EDUG \$JHD/ RI DOOMOO FROOTHIORRGIZWKDMUDH G-BWKOHW WKOQRQHIRRW RU ZWKGUDLQ DUHD/RIOHWWKOQRQHVTXOUHEOHROH; XWXUH8RQGLWLRQV\$QQXDO &KOOTH)ORRG-EDUG #QH; \$JHDZWK\$GHGDRG\$WGHWR HYHH 6H RWHV FRCH; 276\$62 1285 \$JHDZWK)ORRG\$LWIGHWRJHMH =RQH \$JHDR QQLBO (ORRG-EDUG (IHFWLYH25) 27-6966 \$JHDR 800HWHUPQHG)DRRG-DDUG #RQHI 8400QHO 8XOYHUW RU 6WRURBHEU 675887856 LILLILL JAMHLINH RUJORRGZDOO &URW 6+FWLRQ/ZWK\$00000 &000H DWHU 6UIDFHOH/DWLRQ &RDVVVDO 7UDQVHEVV %DM)ORRGOH/DWLRQLQH %(LEW RI 6WXG -XULVGLFWLRQ%RXQGDU\ 8RDWVDO 7UDQWFW %DWHOLQH **24**5 3URLOH%DMOLQH)\$856 **YSURUDS/LFHDWXUH** L'TMDO DMD & DTO DEO H RLTMDODWD&DFOH **683**6 802884G 7KHSLQGLVSODHGRQWKHBSLVDQDSSURLBWH

74LVESFREDLH/ZWK)ØVWDQEDJG/IRU WKHXHR G.JWDO IORRGEB/LI LW LV QRW YR GD/GH/RULEHGEHORZ 7KHED/HES WRQETHEDLH/ZWK)ØVED/HES DFXUFXWDQEDJG/

SPLOW VHO HEWHO ENWICH SWHU DOG GRHV ÖRW UHBUH DQDWWRULWDWLYHSURSHUW O REDWLRQ

7KHIORRGKODUGLQRUBWLRQLVG-ULYK-GQ.UHWO\IURRWKH DWKRULWDWLYHJKJEC-VLYLRHVSURYLG-GGE) 7KLVBS 2V.HRUWHGRQ DW 30 UHOHW ROOHVRU DROCHDWV\XEVHIXKQVWRWLVGDWHDQG WLFI 7KHJKOQGHIHWLYHLQRUBWLRQBIROQHRU EHRRI\XSHUWG-GEQ-ZGDWDRXHUWLFI

7KLVESLEJHLVYRLGLI WKHROHRU RUHRI WKHIROORZOJES HOHFOW GROW ESSHUJ, EDHESLEJHUN IORRGJROHODHOV OHHOG VEDOHEUJ ESRUDWLRQGWH FRRQLWNLGHOWLILHUV JSSOCHO QGHU DOGJSHIHFWLYHOWH DSLEJHVIRU XDESGCOGXORO-UQLJGDUHDV FDOORW EHXHGIRU UHVODWRU\SUSRAHV

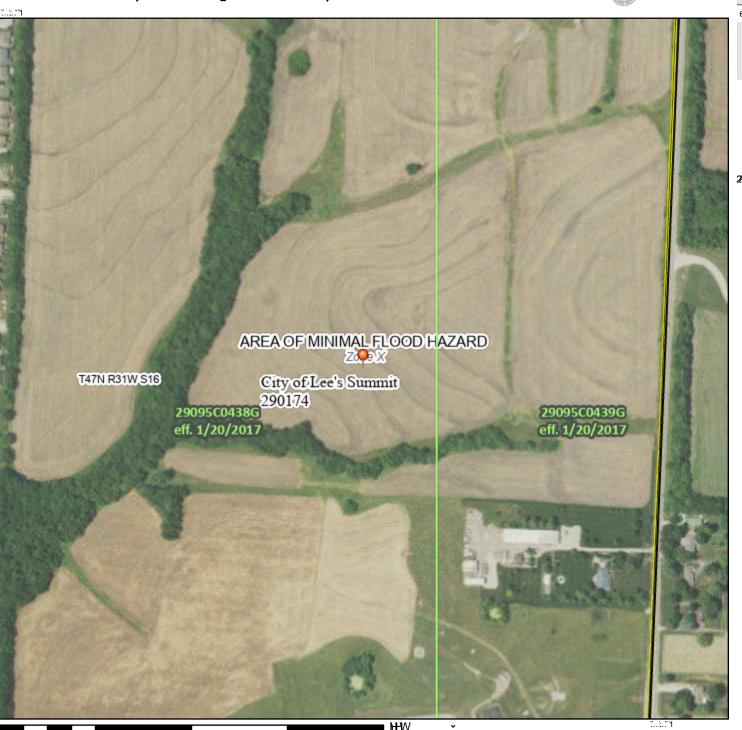




Figure A.5



Appendix B

NRCS Soil Resource Report

HydroCAD Model Output Reports

Existing Conditions

Developed Conditions

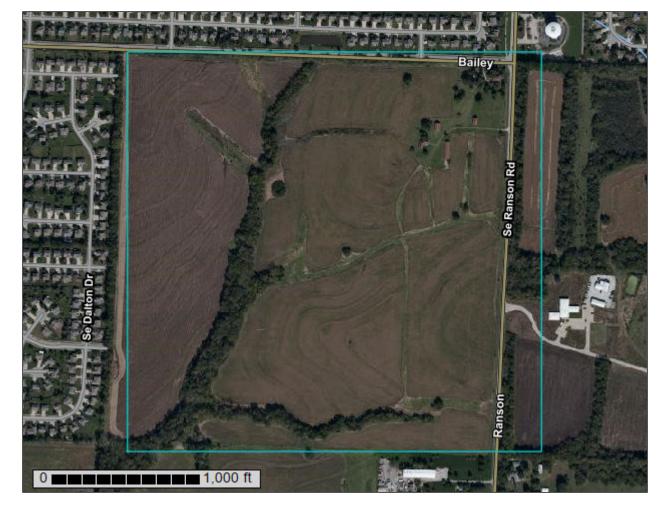
Clogged Conditions



NRCS

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Jackson County, Missouri



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2 053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons



Soil Map Unit Lines



Soil Map Unit Points

Special Point Features

(0)

Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow

Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water
Rock Outcrop



Saline Spot



Sandy Spot

• • •

Severely Eroded Spot



Sinkhole



Sodic Spot

Slide or Slip

8

Spoil Area



Stony Spot



Very Stony Spot



Wet Spot Other



Special Line Features

Water Features

_

Streams and Canals

Transportation

Fransp

Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Jackson County, Missouri Survey Area Data: Version 22, May 29, 2020

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Sep 6, 2019—Nov 16, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
10000	Arisburg silt loam, 1 to 5 percent slopes	62.9	36.4%
10082	Arisburg-Urban land complex, 1 to 5 percent slopes	28.0	16.2%
10117	Sampsel silty clay loam, 5 to 9 percent slopes	82.1	47.5%
Totals for Area of Interest		173.0	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or

landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Jackson County, Missouri

10000—Arisburg silt loam, 1 to 5 percent slopes

Map Unit Setting

National map unit symbol: 2w22b Elevation: 610 to 1,130 feet

Mean annual precipitation: 39 to 43 inches Mean annual air temperature: 50 to 55 degrees F

Frost-free period: 177 to 220 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Arisburg and similar soils: 87 percent Minor components: 13 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Arisburg

Setting

Landform: Interfluves

Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve

Down-slope shape: Convex Across-slope shape: Convex Parent material: Loess

Typical profile

Ap - 0 to 6 inches: silt loam A - 6 to 13 inches: silt loam

Bt - 13 to 19 inches: silty clay loam Btg - 19 to 56 inches: silty clay loam BCg - 56 to 79 inches: silty clay loam

Properties and qualities

Slope: 1 to 5 percent

Depth to restrictive feature: More than 80 inches Drainage class: Somewhat poorly drained

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20

to 0.60 in/hr)

Depth to water table: About 18 to 30 inches

Frequency of flooding: None Frequency of ponding: None

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water capacity: High (about 11.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: C

Ecological site: R107BY007MO - Loess Upland Prairie Amorpha canescens/ Andropogon gerardii-Zizia aurea Leadplant/Big Bluestem-Golden Zizia

Hydric soil rating: No

Minor Components

Greenton

Percent of map unit: 5 percent

Landform: Hillslopes

Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Convex

Ecological site: R109XY002MO - Loess Upland Prairie

Hydric soil rating: No

Sharpsburg

Percent of map unit: 5 percent

Landform: Ridges

Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve

Down-slope shape: Linear Across-slope shape: Linear

Ecological site: R109XY002MO - Loess Upland Prairie

Hydric soil rating: No

Haig

Percent of map unit: 3 percent

Landform: Flats

Landform position (two-dimensional): Summit Landform position (three-dimensional): Talf

Down-slope shape: Linear Across-slope shape: Convex

Ecological site: R109XY001MO - Claypan Summit Prairie

Hydric soil rating: Yes

10082—Arisburg-Urban land complex, 1 to 5 percent slopes

Map Unit Setting

National map unit symbol: 2w7ld Elevation: 750 to 1,130 feet

Mean annual precipitation: 39 to 45 inches
Mean annual air temperature: 50 to 55 degrees F

Frost-free period: 177 to 220 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Arisburg and similar soils: 61 percent

Urban land: 30 percent Minor components: 9 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Arisburg

Setting

Landform: Interfluves

Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve

Down-slope shape: Convex Across-slope shape: Convex Parent material: Loess

Typical profile

Ap - 0 to 6 inches: silt loam A - 6 to 13 inches: silt loam

Bt - 13 to 19 inches: silty clay loam
Btg - 19 to 56 inches: silty clay loam
BCg - 56 to 79 inches: silty clay loam

Properties and qualities

Slope: 1 to 5 percent

Depth to restrictive feature: More than 80 inches Drainage class: Somewhat poorly drained

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20

to 0.60 in/hr)

Depth to water table: About 18 to 30 inches

Frequency of flooding: None Frequency of ponding: None

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water capacity: High (about 11.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: C

Ecological site: R107BY007MO - Loess Upland Prairie Amorpha canescens/ Andropogon gerardii-Zizia aurea Leadplant/Big Bluestem-Golden Zizia

Hydric soil rating: No

Description of Urban Land

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 8

Hydric soil rating: No

Minor Components

Sampsel

Percent of map unit: 3 percent

Landform: Hills

Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Concave

Ecological site: R109XY010MO - Interbedded Sedimentary Upland Savanna

Hydric soil rating: Yes

Greenton

Percent of map unit: 3 percent

Landform: Hillslopes

Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Convex

Ecological site: R109XY002MO - Loess Upland Prairie

Hydric soil rating: No

Sharpsburg

Percent of map unit: 3 percent

Landform: Ridges

Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve

Down-slope shape: Linear Across-slope shape: Linear

Ecological site: R109XY002MO - Loess Upland Prairie

Hydric soil rating: No

10117—Sampsel silty clay loam, 5 to 9 percent slopes

Map Unit Setting

National map unit symbol: 2qkzz

Elevation: 600 to 900 feet

Mean annual precipitation: 33 to 41 inches
Mean annual air temperature: 50 to 55 degrees F

Frost-free period: 177 to 220 days

Farmland classification: Prime farmland if drained

Map Unit Composition

Sampsel and similar soils: 85 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Sampsel

Setting

Landform: Hillslopes

Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope

Down-slope shape: Concave

Across-slope shape: Convex, concave

Parent material: Residuum weathered from shale

Typical profile

Ap - 0 to 13 inches: silty clay loam Bt - 13 to 80 inches: silty clay

Properties and qualities

Slope: 5 to 9 percent

Depth to restrictive feature: More than 80 inches Drainage class: Somewhat poorly drained

Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to

moderately high (0.06 to 0.20 in/hr)

Depth to water table: About 0 to 18 inches

Frequency of flooding: None Frequency of ponding: None

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water capacity: Moderate (about 8.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: C/D

Ecological site: R109XY010MO - Interbedded Sedimentary Upland Savanna

Other vegetative classification: Grass/Prairie (Herbaceous Vegetation)

Hydric soil rating: No

Soil Information for All Uses

Soil Properties and Qualities

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

Soil Qualities and Features

Soil qualities are behavior and performance attributes that are not directly measured, but are inferred from observations of dynamic conditions and from soil properties. Example soil qualities include natural drainage, and frost action. Soil features are attributes that are not directly part of the soil. Example soil features include slope and depth to restrictive layer. These features can greatly impact the use and management of the soil.

Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.



MAP LEGEND MAP INFORMATION Area of Interest (AOI) The soil surveys that comprise your AOI were mapped at С 1:24.000. Area of Interest (AOI) C/D Soils D Warning: Soil Map may not be valid at this scale. Soil Rating Polygons Not rated or not available Α Enlargement of maps beyond the scale of mapping can cause **Water Features** A/D misunderstanding of the detail of mapping and accuracy of soil Streams and Canals line placement. The maps do not show the small areas of В contrasting soils that could have been shown at a more detailed Transportation scale. B/D Rails ---Interstate Highways Please rely on the bar scale on each map sheet for map C/D **US Routes** measurements. Major Roads Source of Map: Natural Resources Conservation Service Not rated or not available Local Roads Web Soil Survey URL: -Coordinate System: Web Mercator (EPSG:3857) Soil Rating Lines Background Aerial Photography Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. Soil Survey Area: Jackson County, Missouri Not rated or not available Survey Area Data: Version 22, May 29, 2020 **Soil Rating Points** Soil map units are labeled (as space allows) for map scales Α 1:50.000 or larger. A/D Date(s) aerial images were photographed: Sep 6, 2019—Nov 16. 2019 B/D The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Table—Hydrologic Soil Group

	_			
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
10000	Arisburg silt loam, 1 to 5 percent slopes	С	62.9	36.4%
10082	Arisburg-Urban land complex, 1 to 5 percent slopes	С	28.0	16.2%
10117	Sampsel silty clay loam, 5 to 9 percent slopes	C/D	82.1	47.5%
Totals for Area of Intere	est	173.0	100.0%	

Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

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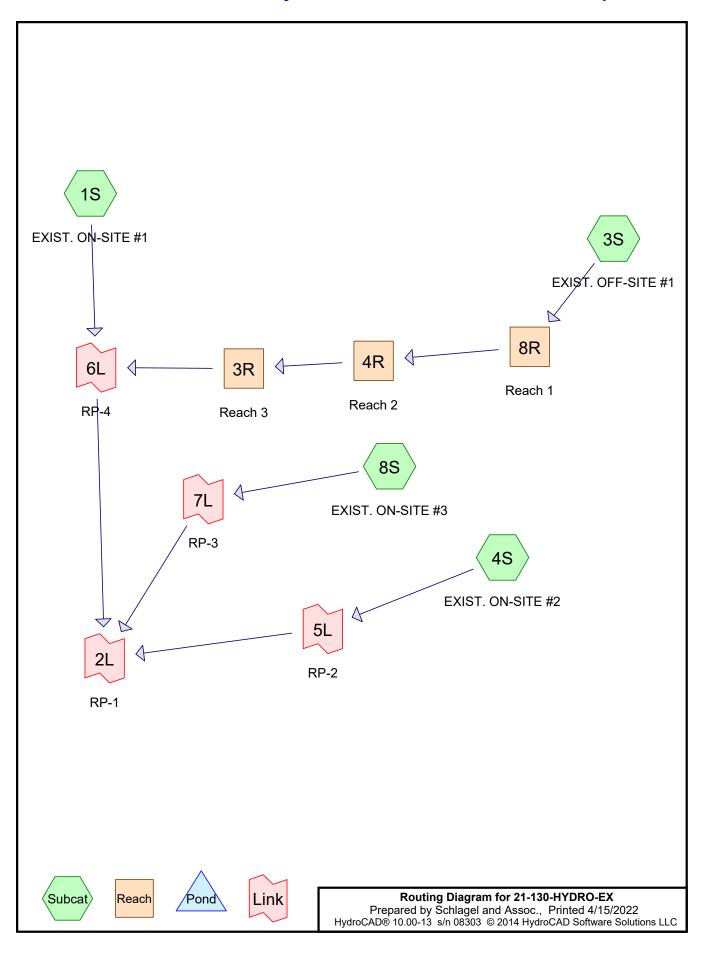
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HydroCAD - Pre-Developed



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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
(40.00)		
48.870	74	Pasture/grassland/range, Good, HSG C (1S, 4S, 8S)
39.910	80	Pasture/grassland/range, Good, HSG D (1S, 4S, 8S)
15.640	76	Woods/grass comb., Fair, HSG C (3S)
2.790	82	Woods/grass comb., Fair, HSG D (3S)

Bailey Farms - PreDeveloped Type II 24-hr 2-Year Rainfall=3.50" Printed 4/15/2022

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Time span=0.00-40.00 hrs, dt=0.05 hrs, 801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: EXIST. ON-SITE#1 Runoff Area=64.820 ac 0.00% Impervious Runoff Depth=1.43" Flow Length=3,450' Tc=47.2 min CN=77 Runoff=56.78 cfs 7.727 af

Subcatchment3S: EXIST. OFF-SITE#1 Runoff Area=18.430 ac 0.00% Impervious Runoff Depth=1.43"

Flow Length=1,147' Slope=0.0210 '/' Tc=17.1 min CN=77 Runoff=31.32 cfs 2.197 af

Subcatchment4S: EXIST. ON-SITE#2 Runoff Area=6.420 ac 0.00% Impervious Runoff Depth=1.37" Flow Length=1,020' Slope=0.0300 '/' Tc=16.7 min CN=76 Runoff=10.50 cfs 0.730 af

Subcatchment8S: EXIST. ON-SITE#3 Runoff Area=17.540 ac 0.00% Impervious Runoff Depth=1.43" Flow Length=1,930' Tc=35.6 min CN=77 Runoff=18.79 cfs 2.091 af

Reach 3R: Reach 3Avg. Flow Depth=0.92' Max Vel=3.48 fps Inflow=27.94 cfs 2.197 af n=0.030 L=1,041.0' S=0.0088 '/' Capacity=2,150.14 cfs Outflow=25.62 cfs 2.197 af

Reach 4R: Reach 2Avg. Flow Depth=0.66' Max Vel=3.55 fps Inflow=30.23 cfs 2.197 af n=0.030 L=853.0' S=0.0225 '/' Capacity=1,999.47 cfs Outflow=27.94 cfs 2.197 af

Reach 8R: Reach 1Avg. Flow Depth=0.27' Max Vel=5.05 fps Inflow=31.32 cfs 2.197 af n=0.012 L=875.0' S=0.0147 '/' Capacity=1,475.63 cfs Outflow=30.23 cfs 2.197 af

Link 2L: RP-1Inflow=101.93 cfs 12.745 af
Primary=101.93 cfs 12.745 af

Link 5L: RP-2Inflow=10.50 cfs 0.730 af
Primary=10.50 cfs 0.730 af

Link 6L: RP-4 Inflow=82.34 cfs 9.924 af Primary=82.34 cfs 9.924 af

Link 7L: RP-3 Inflow=18.79 cfs 2.091 af Primary=18.79 cfs 2.091 af

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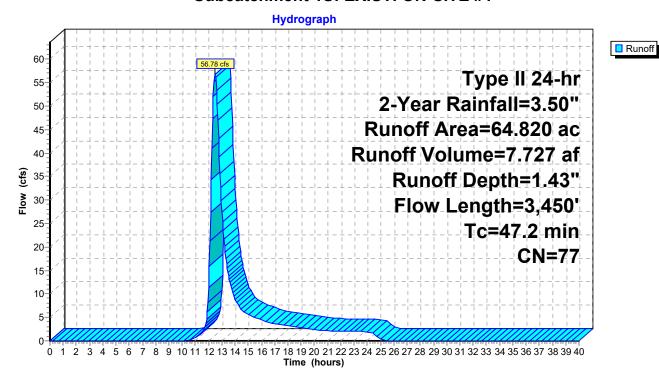
Summary for Subcatchment 1S: EXIST. ON-SITE #1

Runoff = 56.78 cfs @ 12.48 hrs, Volume= 7.727 af, Depth= 1.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

	Area ((ac) C	N Desc	cription		
	34.	840 7	'4 Past	ure/grassla	and/range,	Good, HSG C
_	29.980 80 Pasture/grassland/range, G					Good, HSG D
	64.820 77 Weighted Average					
	64.	820	100.	00% Pervi	ous Area	
		Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.4	100	0.0250	0.20		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.60"
	24.7	2,132	0.0255	1.44		Shallow Concentrated Flow,
						Cultivated Straight Rows Kv= 9.0 fps
	14.1	1,218	0.0092	1.44		Shallow Concentrated Flow,
_						Grassed Waterway Kv= 15.0 fps
	<i>1</i> 7 2	3 450	Total			

Subcatchment 1S: EXIST. ON-SITE #1



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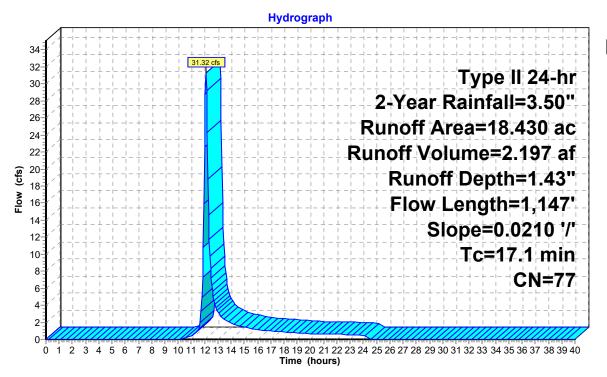
Summary for Subcatchment 3S: EXIST. OFF-SITE #1

Runoff = 31.32 cfs @ 12.10 hrs, Volume= 2.197 af, Depth= 1.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

	Area	(ac) C	N Desc	cription			
					omb., Fair,		
_	2.	790 E	32 Woo	ds/grass c	omb., Fair,	, HSG D	
	18.	430 7	77 Weig	ghted Aver	age		
	18.430 100.00% Pervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	9.1	100	0.0210	0.18		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.60"	
	8.0	1,047	0.0210	2.17		Shallow Concentrated Flow,	
		•				Grassed Waterway Kv= 15.0 fps	
•	17 1	1 147	Total			<u> </u>	

Subcatchment 3S: EXIST. OFF-SITE #1





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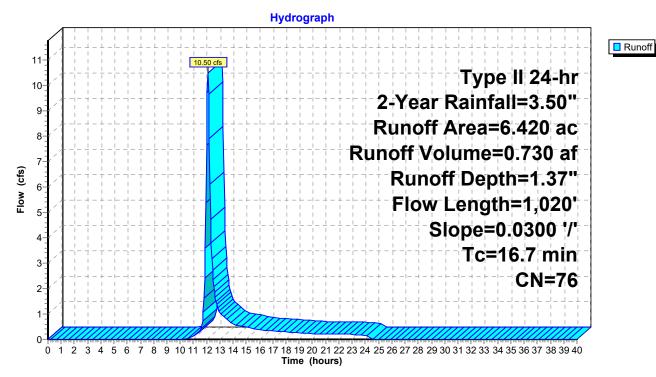
Summary for Subcatchment 4S: EXIST. ON-SITE #2

Runoff = 10.50 cfs @ 12.10 hrs, Volume= 0.730 af, Depth= 1.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

_	Area	(ac) (CN Des	cription		
	4.	200				Good, HSG C
2.220 80 Pasture/grassland/range, Good, HSG D					Good, HSG D	
	6.	420	76 Wei	ghted Aver	age	
	6.	420	100.	00% Pervi	ous Area	
	Tc	Length	•	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.6	80	0.0300	0.20		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.60"
	10.1	940	0.0300	1.56		Shallow Concentrated Flow,
_						Cultivated Straight Rows Kv= 9.0 fps
	16.7	1,020	Total			

Subcatchment 4S: EXIST. ON-SITE #2



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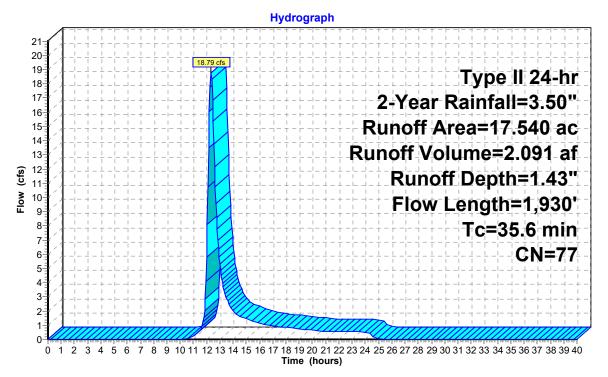
Summary for Subcatchment 8S: EXIST. ON-SITE #3

Runoff = 18.79 cfs @ 12.32 hrs, Volume= 2.091 af, Depth= 1.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

Area	(ac) C	N Desc	cription			
					Good, HSG C	
	710 8	30 Past	ure/grassi	and/range,	Good, HSG D	
17.	540 7	77 Weig	ghted Aver	age		
17.	540		00% Pervi			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
8.3	100	0.0265	0.20		Sheet Flow,	
27.3	1,830	0.0255	1.12		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
35.6	1 930	Total				

Subcatchment 8S: EXIST. ON-SITE #3





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Summary for Reach 3R: Reach 3

18.430 ac, 0.00% Impervious, Inflow Depth = 1.43" for 2-Year event Inflow Area =

27.94 cfs @ 12.30 hrs, Volume= Inflow 2.197 af

2.197 af, Atten= 8%, Lag= 8.8 min 25.62 cfs @ 12.45 hrs, Volume= Outflow

Routing by Stor-Ind+Trans method, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.48 fps, Min. Travel Time= 5.0 min

Avg. Velocity = 0.89 fps, Avg. Travel Time= 19.5 min

Peak Storage= 7,741 cf @ 12.37 hrs

Average Depth at Peak Storage= 0.92'

Bank-Full Depth= 6.40' Flow Area= 252.4 sf, Capacity= 2,150.14 cfs

Custom cross-section, Length= 1,041.0' Slope= 0.0088 '/' (110 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 989.92', Outlet Invert= 980.77'

#

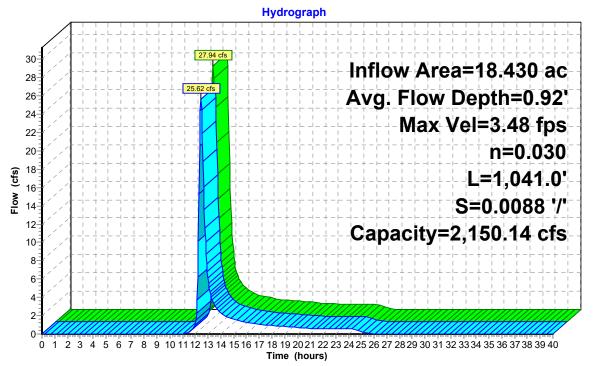
Offset	Offset Elevation	
(feet)	(feet)	(feet)
0.00	996.29	0.00
7.00	996.00	0.29
12.99	994.06	2.23
13.24	994.00	2.29
14.48	993.84	2.45
28.84	992.00	4.29
29.66	991.71	4.58
36.75	990.00	6.29
39.18	989.92	6.37
39.90	989.89	6.40
42.20	989.97	6.32
43.14	990.00	6.29
43.91	990.40	5.89
46.34	992.00	4.29
53.90	993.41	2.88
56.74	994.00	2.29
62.76	994.22	2.07
76.31	994.65	1.64
100.00	996.29	0.00

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P	ag	е	9
	_		

		End Area	Perim.	Storage	Discharge
_	(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
	0.00	0.0	0.0	0	0.00
	0.03	0.0	1.6	25	0.01
	0.08	0.2	4.5	184	0.09
	0.11	0.3	6.4	355	0.22
	0.51	3.4	9.0	3,521	8.20
	1.82	19.8	16.9	20,607	102.00
	2.11	24.7	18.3	25,700	139.72
	3.52	62.5	37.1	65,011	410.25
	3.95	79.1	42.6	82,369	555.08
	4.11	85.9	44.7	89,447	617.31
	4.17	88.6	46.6	92,223	631.78
	4.33	96.2	51.5	100,189	678.51
	4.76	121.1	66.4	126,018	838.92
	6.11	225.0	90.3	234,197	1,919.54
	6.40	252.4	101.5	262,697	2,150.14

Reach 3R: Reach 3





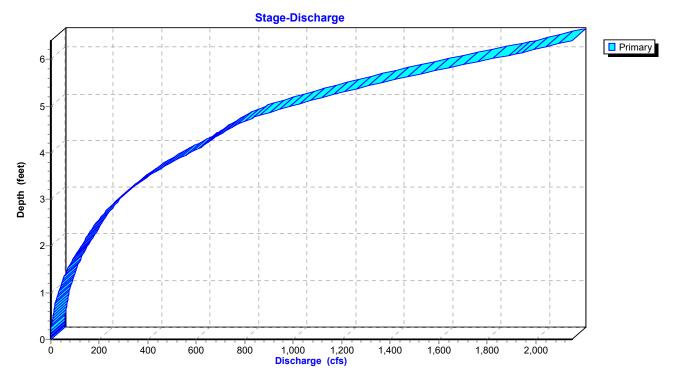
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Reach 3R: Reach 3



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Summary for Reach 4R: Reach 2

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 1.43" for 2-Year event

Inflow = 30.23 cfs @ 12.18 hrs, Volume= 2.197 af

Outflow = 27.94 cfs @ 12.30 hrs, Volume= 2.197 af, Atten= 8%, Lag= 7.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.55 fps, Min. Travel Time= 4.0 min

Avg. Velocity = 1.27 fps, Avg. Travel Time= 11.2 min

Peak Storage= 6,789 cf @ 12.24 hrs

Average Depth at Peak Storage= 0.66'

Bank-Full Depth= 3.24' Flow Area= 204.3 sf, Capacity= 1,999.47 cfs

Custom cross-section, Length= 853.0' Slope= 0.0225 '/' (106 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding

Inlet Invert= 1,009.12', Outlet Invert= 989.90'

\	_	
‡		

Elevation	Chan.Depth	
(feet)	(feet)	
1,009.14	0.00	
1,008.00	1.14	
1,007.80	1.34	
1,007.74	1.40	
1,006.00	3.14	
1,005.90	3.24	
1,005.99	3.15	
1,005.99	3.15	
1,006.00	3.14	
1,007.74	1.40	
1,008.00	1.14	
1,008.27	0.87	
1,008.47	0.67	
1,008.47	0.67	
1,008.89	0.25	
1,009.14	0.00	
	(feet) 1,009.14 1,008.00 1,007.80 1,007.74 1,006.00 1,005.90 1,005.99 1,006.00 1,007.74 1,008.00 1,008.27 1,008.47 1,008.89	

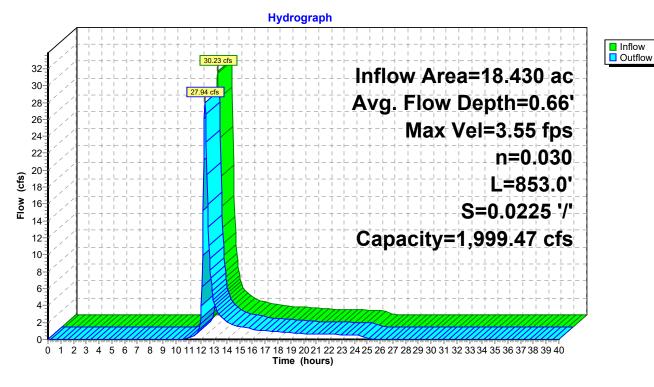
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Depth (feet)	End Area (sq-ft)	Perim. (feet)	Storage (cubic-feet)	Discharge (cfs)
0.00	0.0	0.0	0	0.00
0.09	0.1	2.9	47	0.03
0.10	0.1	4.0	77	0.06
1.84	61.2	66.4	52,179	430.84
1.90	65.2	68.7	55,628	468.55
2.10	79.7	76.6	67,995	608.70
2.37	102.4	92.0	87,382	818.03
2.57	122.0	104.3	104,091	1,007.45
2.99	171.2	130.2	146,042	1,527.76
3.24	204.3	135.4	174,303	1,999.47

Reach 4R: Reach 2



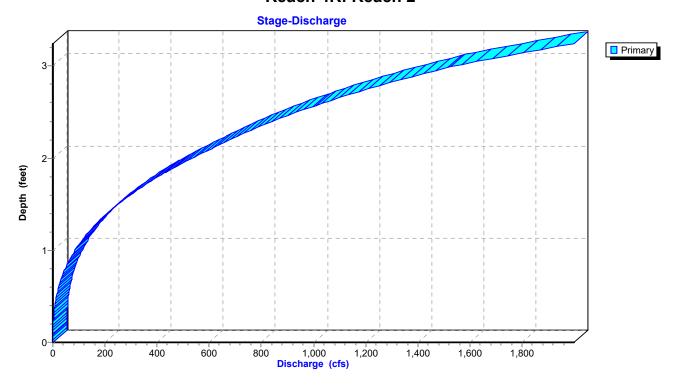
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Reach 4R: Reach 2



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Summary for Reach 8R: Reach 1

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 1.43" for 2-Year event

Inflow = 31.32 cfs @ 12.10 hrs, Volume= 2.197 af

Outflow = 30.23 cfs @ 12.18 hrs, Volume= 2.197 af, Atten= 3%, Lag= 4.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.05 fps, Min. Travel Time= 2.9 min

Avg. Velocity = 1.38 fps, Avg. Travel Time= 10.6 min

Peak Storage= 5,243 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.27'

Bank-Full Depth= 1.86' Flow Area= 89.8 sf, Capacity= 1,475.63 cfs

Custom cross-section, Length= 875.0' Slope= 0.0147 '/' (108 Elevation Intervals)

Constant n= 0.012 Concrete pipe, finished

Inlet Invert= 1,021.96', Outlet Invert= 1,009.12'

<u></u>	
‡	

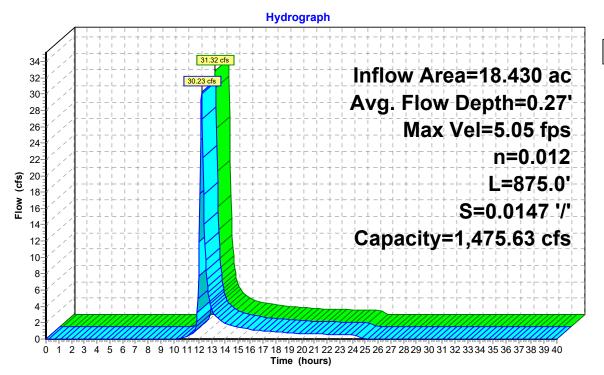
Offset	Elevation	Chan.Depth
(feet)	(feet)	(feet)
6.08	1,023.78	0.00
9.16	1,023.55	0.23
11.33	1,023.58	0.20
11.39	1,023.57	0.21
15.58	1,023.21	0.57
22.68	1,022.85	0.93
26.25	1,022.76	1.02
30.78	1,022.00	1.78
35.77	1,021.93	1.85
36.49	1,021.92	1.86
37.38	1,021.93	1.85
42.61	1,021.97	1.81
45.38	1,021.97	1.81
54.65	1,022.00	1.78
58.89	1,022.15	1.63
59.16	1,022.15	1.63
60.04	1,022.18	1.60
63.66	1,022.37	1.41
70.33	1,022.84	0.94
84.24	1,023.78	0.00

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		End Area	Perim.	Storage (cubic-feet)	Discharge
_	(feet)	(sq-ft)	(feet)	(cubic-leet)	<u>(cfs)</u>
	0.00	0.0	0.0	0	0.00
	0.01	0.0	1.6	7	0.00
	0.05	0.2	12.5	205	0.25
	0.08	8.0	23.9	682	1.19
	0.23	4.7	29.3	4,152	21.15
	0.26	5.6	30.4	4,934	27.55
	0.45	11.9	35.1	10,372	86.23
	0.84	27.1	43.0	23,687	297.75
	0.92	30.7	47.3	26,843	344.59
	0.93	31.2	47.9	27,258	350.85
	1.29	50.6	60.3	44,272	674.94
	1.63	72.6	69.4	63,526	1,122.85
	1.65	74.0	71.6	64,757	1,134.97
	1.66	74.7	72.7	65,388	1,145.01
	1.86	89.8	78.3	78,572	1,475.63

Reach 8R: Reach 1



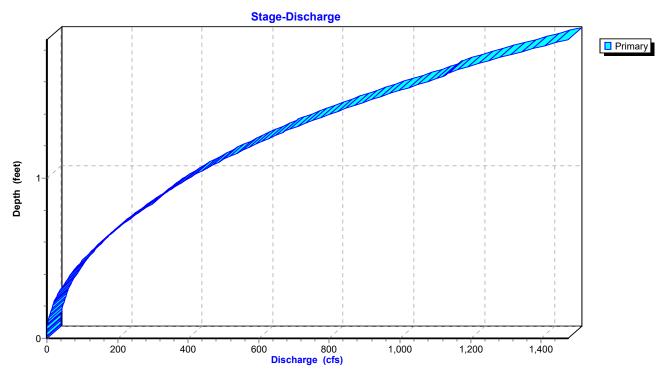


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Reach 8R: Reach 1



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Summary for Link 2L: RP-1

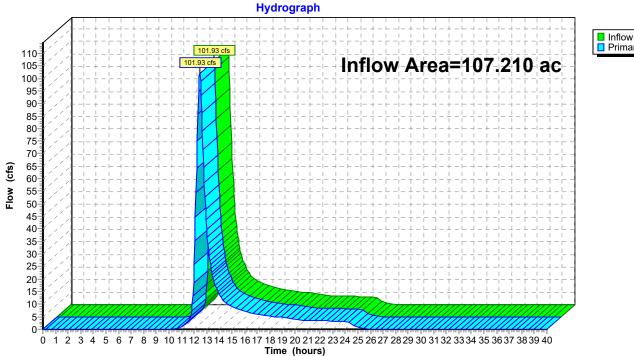
Inflow Area = 107.210 ac, 0.00% Impervious, Inflow Depth = 1.43" for 2-Year event

101.93 cfs @ 12.44 hrs, Volume= Inflow 12.745 af

101.93 cfs @ 12.44 hrs, Volume= 12.745 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 2L: RP-1





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Summary for Link 5L: RP-2

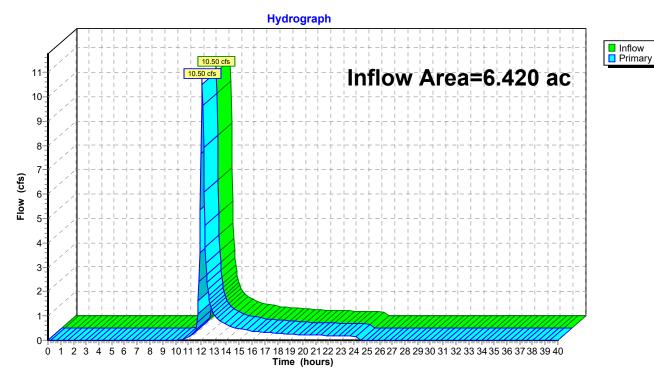
Inflow Area = 6.420 ac, 0.00% Impervious, Inflow Depth = 1.37" for 2-Year event

Inflow 0.730 af

10.50 cfs @ 12.10 hrs, Volume= 10.50 cfs @ 12.10 hrs, Volume= 0.730 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 5L: RP-2



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Summary for Link 6L: RP-4

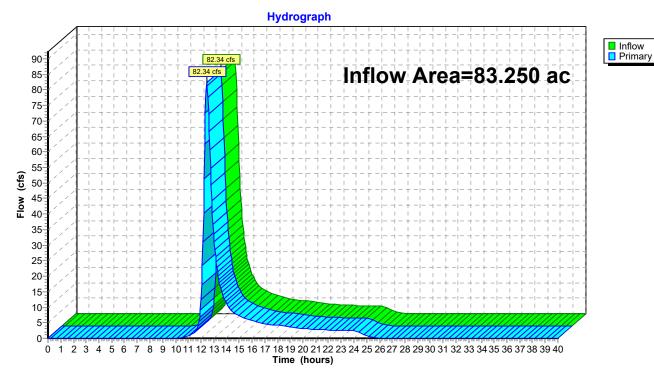
Inflow Area = 83.250 ac, 0.00% Impervious, Inflow Depth = 1.43" for 2-Year event

Inflow = 82.34 cfs @ 12.46 hrs, Volume= 9.924 af

Primary = 82.34 cfs @ 12.46 hrs, Volume= 9.924 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 6L: RP-4



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Summary for Link 7L: RP-3

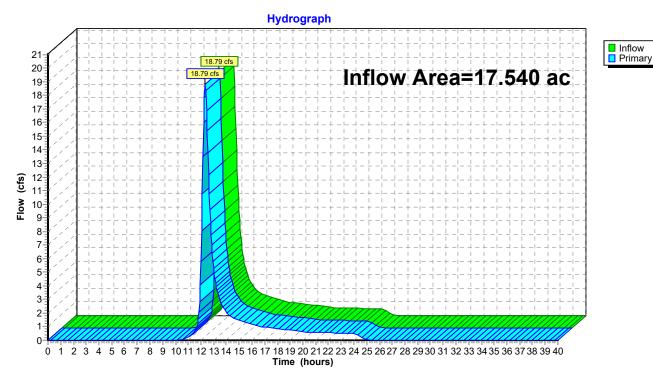
Inflow Area = 17.540 ac, 0.00% Impervious, Inflow Depth = 1.43" for 2-Year event

Inflow = 18.79 cfs @ 12.32 hrs, Volume= 2.091 af

Primary = 18.79 cfs @ 12.32 hrs, Volume= 2.091 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 7L: RP-3



Bailey Farms - PreDeveloped Type II 24-hr 10-Year Rainfall=5.30" Printed 4/15/2022

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Time span=0.00-40.00 hrs, dt=0.05 hrs, 801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: EXIST. ON-SITE#1 Runoff Area=64.820 ac 0.00% Impervious Runoff Depth=2.88" Flow Length=3,450' Tc=47.2 min CN=77 Runoff=118.07 cfs 15.535 af

Subcatchment3S: EXIST. OFF-SITE#1 Runoff Area=18.430 ac 0.00% Impervious Runoff Depth=2.88" Flow Length=1,147' Slope=0.0210 '/' Tc=17.1 min CN=77 Runoff=63.84 cfs 4.417 af

Subcatchment4S: EXIST. ON-SITE#2 Runoff Area=6.420 ac 0.00% Impervious Runoff Depth=2.78" Flow Length=1,020' Slope=0.0300 '/' Tc=16.7 min CN=76 Runoff=21.81 cfs 1.490 af

Subcatchment8S: EXIST. ON-SITE#3 Runoff Area=17.540 ac 0.00% Impervious Runoff Depth=2.88" Flow Length=1,930' Tc=35.6 min CN=77 Runoff=38.90 cfs 4.204 af

Reach 3R: Reach 3Avg. Flow Depth=1.35' Max Vel=4.36 fps Inflow=58.82 cfs 4.417 af n=0.030 L=1,041.0' S=0.0088 '/' Capacity=2,150.14 cfs Outflow=55.15 cfs 4.417 af

Reach 4R: Reach 2Avg. Flow Depth=0.87' Max Vel=4.28 fps Inflow=61.90 cfs 4.417 af n=0.030 L=853.0' S=0.0225'/' Capacity=1,999.47 cfs Outflow=58.82 cfs 4.417 af

Reach 8R: Reach 1Avg. Flow Depth=0.38' Max Vel=6.50 fps Inflow=63.84 cfs 4.417 af n=0.012 L=875.0' S=0.0147 '/' Capacity=1,475.63 cfs Outflow=61.90 cfs 4.417 af

Link 2L: RP-1Inflow=213.45 cfs 25.645 af Primary=213.45 cfs 25.645 af

Link 5L: RP-2Inflow=21.81 cfs 1.490 af
Primary=21.81 cfs 1.490 af

Link 6L: RP-4Inflow=170.41 cfs 19.951 af
Primary=170.41 cfs 19.951 af

Link 7L: RP-3 Inflow=38.90 cfs 4.204 af Primary=38.90 cfs 4.204 af

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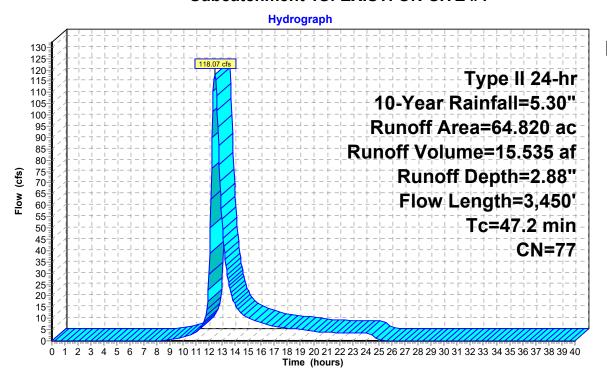
Summary for Subcatchment 1S: EXIST. ON-SITE #1

Runoff 118.07 cfs @ 12.46 hrs, Volume= 15.535 af, Depth= 2.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=5.30"

	Area	(ac) C	N Desc	cription			
	_					Good, HSG C	
_	29.	980 8	0 Past	ure/grassl	<u>and/range,</u>	Good, HSG D	_
	64.	820 7	7 Weig	ghted Aver	age		
	64.	820	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	8.4	100	0.0250	0.20		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.60"	
	24.7	2,132	0.0255	1.44		Shallow Concentrated Flow,	
						Cultivated Straight Rows Kv= 9.0 fps	
	14.1	1,218	0.0092	1.44		Shallow Concentrated Flow,	
						Grassed Waterway Kv= 15.0 fps	
	47.2	3,450	Total	•			_

Subcatchment 1S: EXIST. ON-SITE #1





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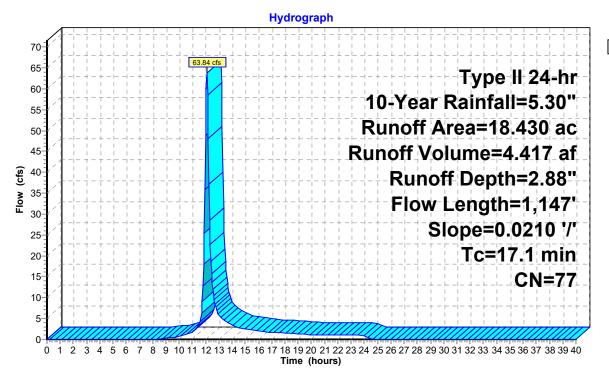
Summary for Subcatchment 3S: EXIST. OFF-SITE #1

Runoff = 63.84 cfs @ 12.10 hrs, Volume= 4.417 af, Depth= 2.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=5.30"

_	Area	(ac) C	N Desc	cription		
15.640 76 Woods/grass comb., Fair, H						
2.790 82 Woods/grass comb., Fair, HSG D						, HSG D
	18.	430 7	77 Weig	ghted Aver	age	
	18.430 100.00% Pervious Area				ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
_	9.1	100	0.0210	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.60"
	8.0	1,047	0.0210	2.17		Shallow Concentrated Flow,
		•				Grassed Waterway Kv= 15.0 fps
•	17 1	1 147	Total			•

Subcatchment 3S: EXIST. OFF-SITE #1





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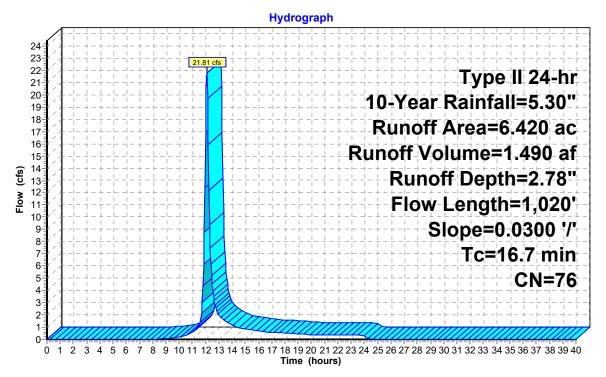
Summary for Subcatchment 4S: EXIST. ON-SITE #2

Runoff = 21.81 cfs @ 12.09 hrs, Volume= 1.490 af, Depth= 2.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=5.30"

	Area	(ac) C	N Desc	cription			
4.200 74 Pasture/grassland/range, Good, HSG C							
2.220 80 Pasture/grassland/range, Good, HSG D							
	6.	420 7	76 Weig	ghted Aver	age		
6.420 100.00% Pervious Area							
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	6.6	80	0.0300	0.20		Sheet Flow,	
	10.1	940	0.0300	1.56		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps	
	16.7	1 020	Total				

Subcatchment 4S: EXIST. ON-SITE #2





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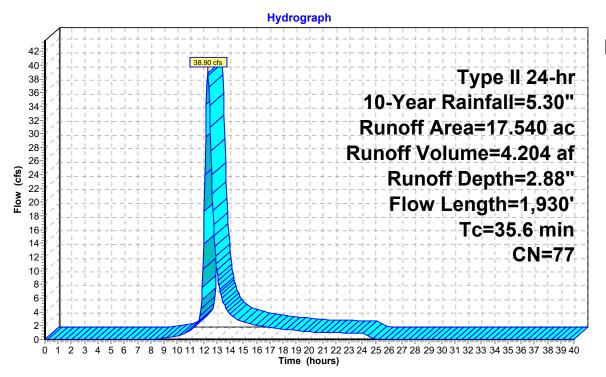
Summary for Subcatchment 8S: EXIST. ON-SITE #3

Runoff = 38.90 cfs @ 12.31 hrs, Volume= 4.204 af, Depth= 2.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=5.30"

_	Area	(ac) C	N Des	cription		
	9.	Good, HSG C				
	7.	710	<u>30 Past</u>	ture/grassl	<u>and/range,</u>	Good, HSG D
	17.	540	77 Weig	ghted Aver	rage	
	17.	540	100.	00% Pervi	ious Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.3	100	0.0265	0.20		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.60"
	27.3	1,830	0.0255	1.12		Shallow Concentrated Flow,
		,				Short Grass Pasture Kv= 7.0 fps
•	35.6	1 930	Total			<u> </u>

Subcatchment 8S: EXIST. ON-SITE #3





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Summary for Reach 3R: Reach 3

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 2.88" for 10-Year event

Inflow = 58.82 cfs @ 12.26 hrs, Volume= 4.417 af

Outflow = 55.15 cfs @ 12.37 hrs, Volume= 4.417 af, Atten= 6%, Lag= 7.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.36 fps, Min. Travel Time= 4.0 min Avg. Velocity = 1.06 fps, Avg. Travel Time= 16.4 min

Peak Storage= 13,333 cf @ 12.31 hrs Average Depth at Peak Storage= 1.35'

Bank-Full Depth= 6.40' Flow Area= 252.4 sf, Capacity= 2,150.14 cfs

Custom cross-section, Length= 1,041.0' Slope= 0.0088 '/' (110 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 989.92', Outlet Invert= 980.77'

‡	

Offset	Elevation	Chan.Depth	
(feet)	(feet)	(feet)	
0.00	996.29	0.00	
7.00	996.00	0.29	
12.99	994.06	2.23	
13.24	994.00	2.29	
14.48	993.84	2.45	
28.84	992.00	4.29	
29.66	991.71	4.58	
36.75	990.00	6.29	
39.18	989.92	6.37	
39.90	989.89	6.40	
42.20	989.97	6.32	
43.14	990.00	6.29	
43.91	990.40	5.89	
46.34	992.00	4.29	
53.90	993.41	2.88	
56.74	994.00	2.29	
62.76	994.22	2.07	
76.31	994.65	1.64	
100.00	996.29	0.00	

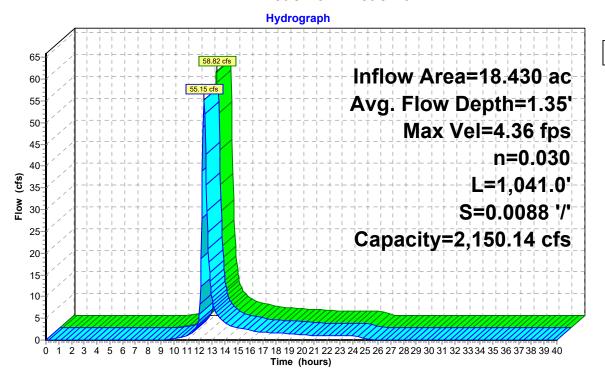
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Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
0.03	0.0	1.6	25	0.01
0.08	0.2	4.5	184	0.09
0.11	0.3	6.4	355	0.22
0.51	3.4	9.0	3,521	8.20
1.82	19.8	16.9	20,607	102.00
2.11	24.7	18.3	25,700	139.72
3.52	62.5	37.1	65,011	410.25
3.95	79.1	42.6	82,369	555.08
4.11	85.9	44.7	89,447	617.31
4.17	88.6	46.6	92,223	631.78
4.33	96.2	51.5	100,189	678.51
4.76	121.1	66.4	126,018	838.92
6.11	225.0	90.3	234,197	1,919.54
6.40	252.4	101.5	262,697	2,150.14

Reach 3R: Reach 3



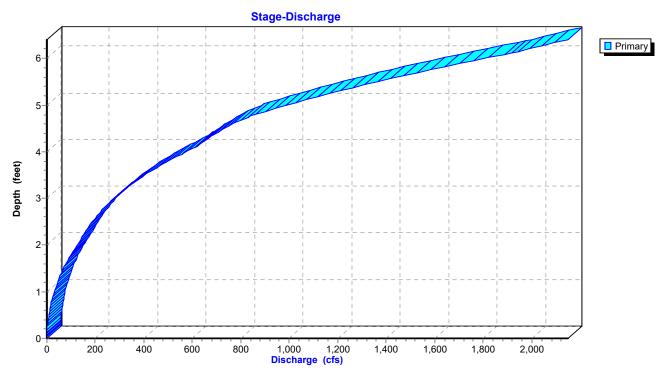


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Reach 3R: Reach 3



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Summary for Reach 4R: Reach 2

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 2.88" for 10-Year event

Inflow = 61.90 cfs @ 12.16 hrs, Volume= 4.417 af

Outflow = 58.82 cfs @ 12.26 hrs, Volume= 4.417 af, Atten= 5%, Lag= 5.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.28 fps, Min. Travel Time= 3.3 min Avg. Velocity = 1.45 fps, Avg. Travel Time= 9.8 min

Peak Storage= 11,793 cf @ 12.20 hrs Average Depth at Peak Storage= 0.87'

Bank-Full Depth= 3.24' Flow Area= 204.3 sf, Capacity= 1,999.47 cfs

Custom cross-section, Length= 853.0' Slope= 0.0225 '/' (106 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 1,009.12', Outlet Invert= 989.90'

‡

Offset (feet)	Elevation (feet)	Chan.Depth (feet)
0.00	1,009.14	0.00
22.15	1,008.00	1.14
24.54	1,007.80	1.34
25.18	1,007.74	1.40
49.26	1,006.00	3.14
49.59	1,005.90	3.24
50.51	1,005.99	3.15
52.13	1,005.99	3.15
53.24	1,006.00	3.14
91.41	1,007.74	1.40
98.60	1,008.00	1.14
108.78	1,008.27	0.87
117.07	1,008.47	0.67
117.16	1,008.47	0.67
134.91	1,008.89	0.25
135.00	1,009.14	0.00

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3.24

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204.3

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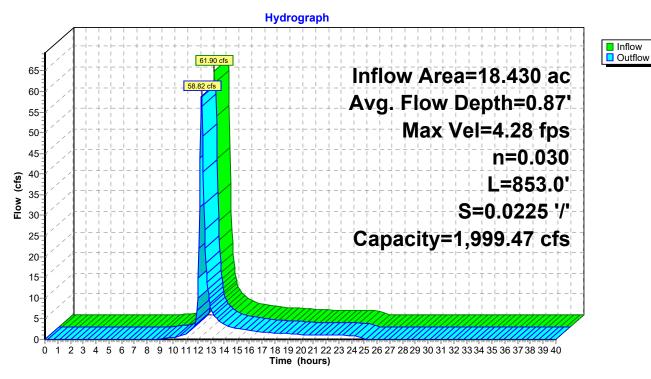
174,303

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
0.09	0.1	2.9	47	0.03
0.10	0.1	4.0	77	0.06
1.84	61.2	66.4	52,179	430.84
1.90	65.2	68.7	55,628	468.55
2.10	79.7	76.6	67,995	608.70
2.37	102.4	92.0	87,382	818.03
2.57	122.0	104.3	104,091	1,007.45
2.99	171.2	130.2	146,042	1,527.76

135.4

Reach 4R: Reach 2

1,999.47

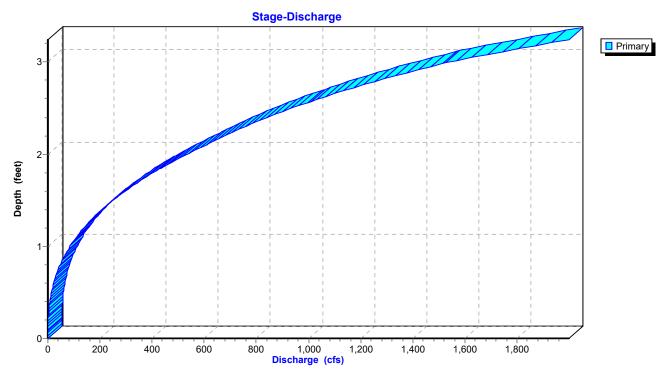


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Reach 4R: Reach 2



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Summary for Reach 8R: Reach 1

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 2.88" for 10-Year event

Inflow = 63.84 cfs @ 12.10 hrs, Volume= 4.417 af

Outflow = 61.90 cfs @ 12.16 hrs, Volume= 4.417 af, Atten= 3%, Lag= 3.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Max. Velocity= 6.50 fps, Min. Travel Time= 2.2 min

Avg. Velocity = 1.61 fps, Avg. Travel Time= 9.1 min

Peak Storage= 8,403 cf @ 12.12 hrs

Average Depth at Peak Storage= 0.38'
Bank-Full Depth= 1.86' Flow Area= 89.8 sf, Capacity= 1,475.63 cfs

Custom cross-section, Length= 875.0' Slope= 0.0147 '/' (108 Elevation Intervals)

Constant n= 0.012 Concrete pipe, finished

Inlet Invert= 1,021.96', Outlet Invert= 1,009.12'

‡	

Offset	Elevation	Chan.Depth	
(feet)	(feet)	(feet)	
6.08	1,023.78	0.00	
9.16	1,023.55	0.23	
11.33	1,023.58	0.20	
11.39	1,023.57	0.21	
15.58	1,023.21	0.57	
22.68	1,022.85	0.93	
26.25	1,022.76	1.02	
30.78	1,022.00	1.78	
35.77	1,021.93	1.85	
36.49	1,021.92	1.86	
37.38	1,021.93	1.85	
42.61	1,021.97	1.81	
45.38	1,021.97	1.81	
54.65	1,022.00	1.78	
58.89	1,022.15	1.63	
59.16	1,022.15	1.63	
60.04	1,022.18	1.60	

1,022.37

1,022.84

1,023.78

1.41

0.94

0.00

63.66

70.33

84.24

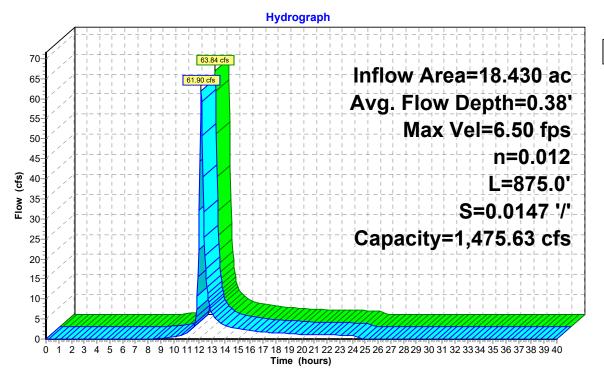
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Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
0.01	0.0	1.6	7	0.00
0.05	0.2	12.5	205	0.25
0.08	0.8	23.9	682	1.19
0.23	4.7	29.3	4,152	21.15
0.26	5.6	30.4	4,934	27.55
0.45	11.9	35.1	10,372	86.23
0.84	27.1	43.0	23,687	297.75
0.92	30.7	47.3	26,843	344.59
0.93	31.2	47.9	27,258	350.85
1.29	50.6	60.3	44,272	674.94
1.63	72.6	69.4	63,526	1,122.85
1.65	74.0	71.6	64,757	1,134.97
1.66	74.7	72.7	65,388	1,145.01
1.86	89.8	78.3	78,572	1,475.63

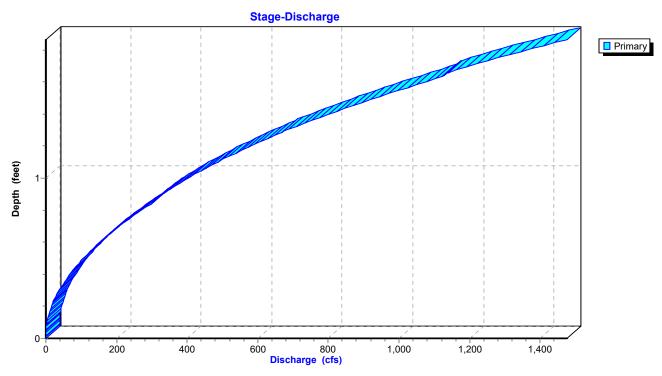
Reach 8R: Reach 1





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Reach 8R: Reach 1



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Summary for Link 2L: RP-1

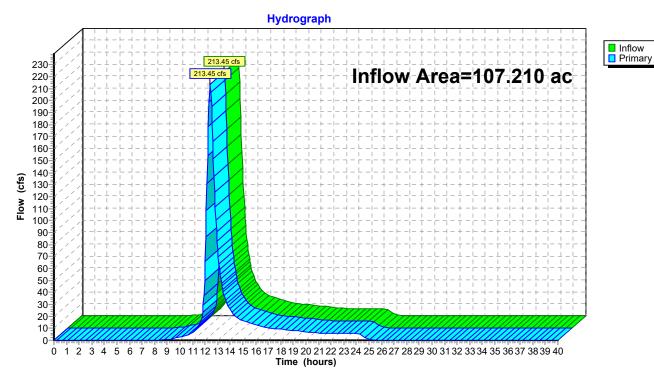
Inflow Area = 107.210 ac, 0.00% Impervious, Inflow Depth = 2.87" for 10-Year event

Inflow = 213.45 cfs @ 12.39 hrs, Volume= 25.645 af

Primary = 213.45 cfs @ 12.39 hrs, Volume= 25.645 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 2L: RP-1



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Summary for Link 5L: RP-2

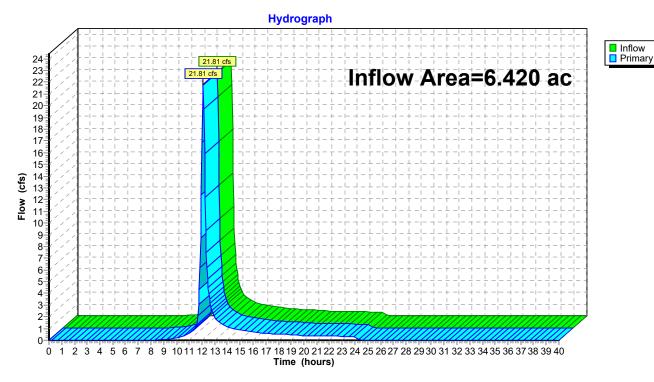
Inflow Area = 6.420 ac, 0.00% Impervious, Inflow Depth = 2.78" for 10-Year event

Inflow = 21.81 cfs @ 12.09 hrs, Volume= 1.490 af

Primary = 21.81 cfs @ 12.09 hrs, Volume= 1.490 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 5L: RP-2



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Summary for Link 6L: RP-4

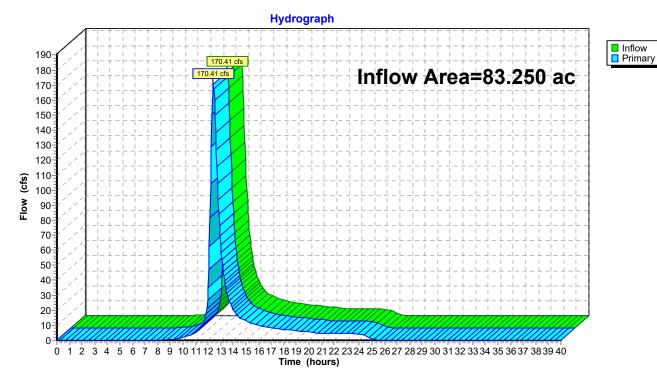
Inflow Area = 83.250 ac, 0.00% Impervious, Inflow Depth = 2.88" for 10-Year event

Inflow = 170.41 cfs @ 12.41 hrs, Volume= 19.951 af

Primary = 170.41 cfs @ 12.41 hrs, Volume= 19.951 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 6L: RP-4



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Summary for Link 7L: RP-3

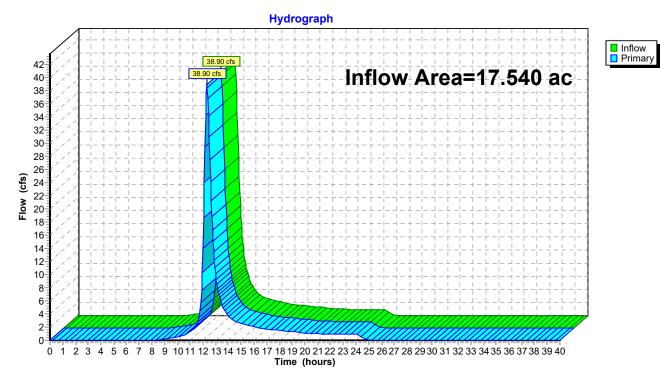
Inflow Area = 17.540 ac, 0.00% Impervious, Inflow Depth = 2.88" for 10-Year event

Inflow = 38.90 cfs @ 12.31 hrs, Volume= 4.204 af

Primary = 38.90 cfs @ 12.31 hrs, Volume= 4.204 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 7L: RP-3



Bailey Farms - PreDeveloped Type II 24-hr 100-Year Rainfall=7.70" Printed 4/15/2022

21-130-HYDRO-EX

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Time span=0.00-40.00 hrs, dt=0.05 hrs, 801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: EXIST. ON-SITE#1 Runoff Area=64.820 ac 0.00% Impervious Runoff Depth=5.00" Flow Length=3,450' Tc=47.2 min CN=77 Runoff=206.11 cfs 27.008 af

Subcatchment3S: EXIST. OFF-SITE#1 Runoff Area=18.430 ac 0.00% Impervious Runoff Depth=5.00" Flow Length=1,147' Slope=0.0210 '/' Tc=17.1 min CN=77 Runoff=110.11 cfs 7.679 af

Subcatchment4S: EXIST. ON-SITE#2 Runoff Area=6.420 ac 0.00% Impervious Runoff Depth=4.89" Flow Length=1,020' Slope=0.0300 '/' Tc=16.7 min CN=76 Runoff=38.02 cfs 2.614 af

Subcatchment8S: EXIST. ON-SITE#3 Runoff Area=17.540 ac 0.00% Impervious Runoff Depth=5.00" Flow Length=1,930' Tc=35.6 min CN=77 Runoff=67.74 cfs 7.308 af

Reach 3R: Reach 3Avg. Flow Depth=1.80' Max Vel=5.10 fps Inflow=103.03 cfs 7.679 af n=0.030 L=1,041.0' S=0.0088 '/' Capacity=2,150.14 cfs Outflow=98.34 cfs 7.679 af

Reach 4R: Reach 2Avg. Flow Depth=1.08' Max Vel=4.91 fps Inflow=106.97 cfs 7.679 af n=0.030 L=853.0' S=0.0225 '/' Capacity=1,999.47 cfs Outflow=103.03 cfs 7.679 af

Reach 8R: Reach 1Avg. Flow Depth=0.50' Max Vel=7.87 fps Inflow=110.11 cfs 7.679 af n=0.012 L=875.0' S=0.0147 '/' Capacity=1,475.63 cfs Outflow=106.97 cfs 7.679 af

Link 2L: RP-1Inflow=372.07 cfs 44.609 af
Primary=372.07 cfs 44.609 af

Link 5L: RP-2 Inflow=38.02 cfs 2.614 af Primary=38.02 cfs 2.614 af

Link 6L: RP-4Inflow=294.00 cfs 34.687 af
Primary=294.00 cfs 34.687 af

Link 7L: RP-3Inflow=67.74 cfs 7.308 af
Primary=67.74 cfs 7.308 af

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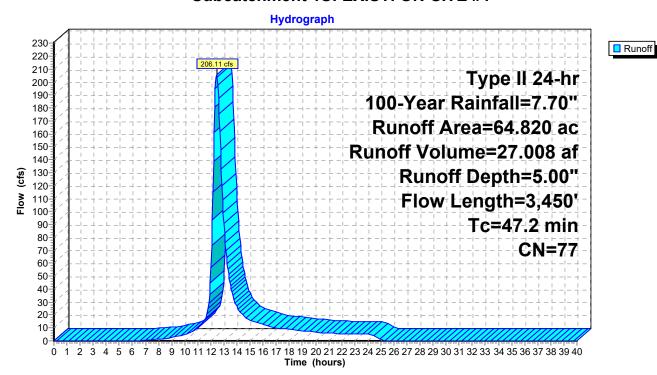
Summary for Subcatchment 1S: EXIST. ON-SITE #1

Runoff = 206.11 cfs @ 12.45 hrs, Volume= 27.008 af, Depth= 5.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 100-Year Rainfall=7.70"

_	Area	(ac) C	N Des	cription				
	34.	840 7	'4 Past	ure/grassl	and/range,	Good, HSG C		
_	29.	980 8	0 Past	:ure/grassl	and/range,	Good, HSG D		
	64.	820 7	7 Weig	Weighted Average				
	64.	820	100.	00% Pervi	ous Area			
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	8.4	100	0.0250	0.20		Sheet Flow,		
						Grass: Short n= 0.150 P2= 3.60"		
	24.7	2,132	0.0255	1.44		Shallow Concentrated Flow,		
						Cultivated Straight Rows Kv= 9.0 fps		
	14.1	1,218	0.0092	1.44		Shallow Concentrated Flow,		
_						Grassed Waterway Kv= 15.0 fps		
	47 2	3 450	Total					

Subcatchment 1S: EXIST. ON-SITE #1



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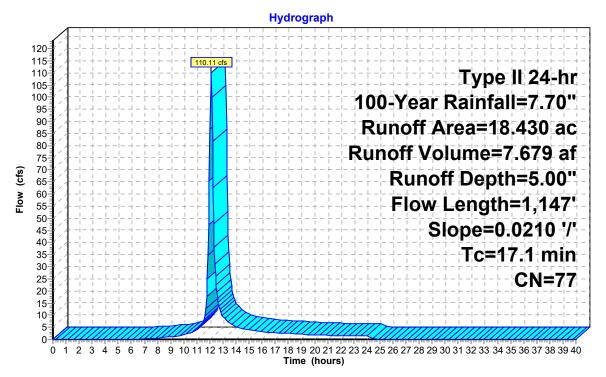
Summary for Subcatchment 3S: EXIST. OFF-SITE #1

Runoff = 110.11 cfs @ 12.09 hrs, Volume= 7.679 af, Depth= 5.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 100-Year Rainfall=7.70"

	Area	(ac) C	N Desc	cription				
	15.	HSG C						
_	2.	, HSG D						
	18.	430 7						
18.430 77 Weighted Average 18.430 100.00% Pervious Area								
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	9.1	100	0.0210	0.18		Sheet Flow,		
						Grass: Short n= 0.150 P2= 3.60"		
	8.0	1,047	0.0210	2.17		Shallow Concentrated Flow,		
		•				Grassed Waterway Kv= 15.0 fps		
•	17 1	1 147	Total			<u> </u>		

Subcatchment 3S: EXIST. OFF-SITE #1





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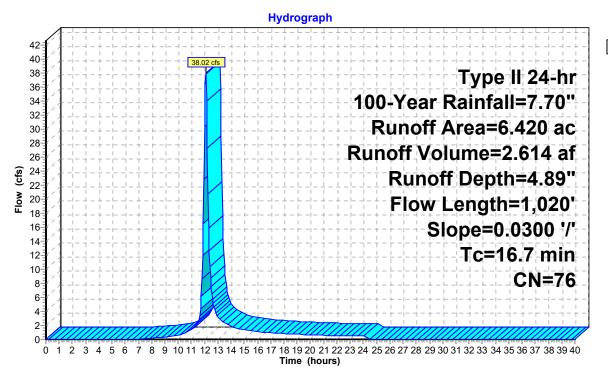
Summary for Subcatchment 4S: EXIST. ON-SITE #2

Runoff = 38.02 cfs @ 12.09 hrs, Volume= 2.614 af, Depth= 4.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 100-Year Rainfall=7.70"

Area	(ac) C	N Desc	cription			
4.200 74 Pasture/grassland/range, Good, HSG C						
2.	220 8	30 Past	ure/grassl	and/range,	Good, HSG D	
6.	420 7	76 Weig	ghted Aver	age		
6.	420		00% Pervi			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
6.6	80	0.0300	0.20		Sheet Flow,	
10.1	940	0.0300	1.56		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps	
16.7	1 020	Total				

Subcatchment 4S: EXIST. ON-SITE #2





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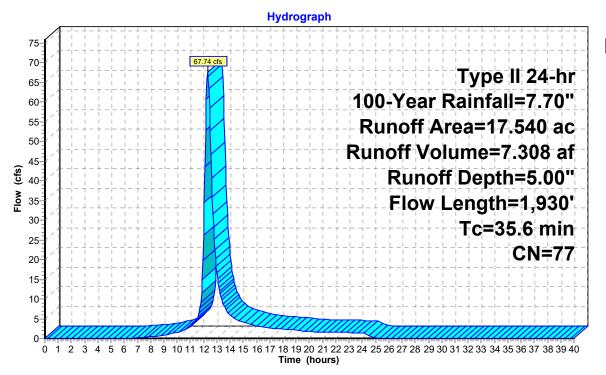
Summary for Subcatchment 8S: EXIST. ON-SITE #3

Runoff = 67.74 cfs @ 12.31 hrs, Volume= 7.308 af, Depth= 5.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs Type II 24-hr 100-Year Rainfall=7.70"

Area	(ac) C	N Desc	Description					
9.830 74 Pasture/grassland/range, Good, HSG C								
1.	710 8	30 Past	ure/grassi	and/range,	Good, HSG D			
17.	540 7	77 Weig	ghted Aver	age				
17.	540		00% Pervi					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
8.3	100	0.0265	0.20		Sheet Flow,			
27.3	1,830	0.0255	1.12		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
35.6	1 930	Total						

Subcatchment 8S: EXIST. ON-SITE #3





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Summary for Reach 3R: Reach 3

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 5.00" for 100-Year event

Inflow = 103.03 cfs @ 12.23 hrs, Volume= 7.679 af

Outflow = 98.34 cfs @ 12.33 hrs, Volume= 7.679 af, Atten= 5%, Lag= 5.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.10 fps, Min. Travel Time= 3.4 min

Avg. Velocity = 1.23 fps, Avg. Travel Time= 14.2 min

Peak Storage= 20,194 cf @ 12.27 hrs

Average Depth at Peak Storage= 1.80'

Bank-Full Depth= 6.40' Flow Area= 252.4 sf, Capacity= 2,150.14 cfs

Custom cross-section, Length= 1,041.0' Slope= 0.0088 '/' (110 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 989.92', Outlet Invert= 980.77'

‡

Offs (fe		levation (feet)	Chan.Depth (feet)	
	00	996.29	0.00	
	00	996.00	0.00	
12.		994.06	2.23	
13.		994.00	2.29	
14.		993.84	2.45	
28.	. •	992.00	4.29	
29.	_	991.71	4.58	
36.		990.00	6.29	
39.	. •	989.92	6.37	
39.	_	989.89	6.40	
42.		989.97	6.32	
43.	14	990.00	6.29	
43.	91	990.40	5.89	
46.	34	992.00	4.29	
53.	90	993.41	2.88	
56.	74	994.00	2.29	
62.	76	994.22	2.07	
76.	31	994.65	1.64	
100.	00	996.29	0.00	

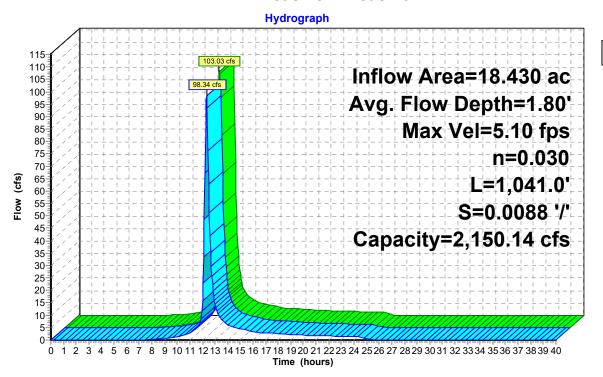
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Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
0.03	0.0	1.6	25	0.01
0.08	0.2	4.5	184	0.09
0.11	0.3	6.4	355	0.22
0.51	3.4	9.0	3,521	8.20
1.82	19.8	16.9	20,607	102.00
2.11	24.7	18.3	25,700	139.72
3.52	62.5	37.1	65,011	410.25
3.95	79.1	42.6	82,369	555.08
4.11	85.9	44.7	89,447	617.31
4.17	88.6	46.6	92,223	631.78
4.33	96.2	51.5	100,189	678.51
4.76	121.1	66.4	126,018	838.92
6.11	225.0	90.3	234,197	1,919.54
6.40	252.4	101.5	262,697	2,150.14

Reach 3R: Reach 3



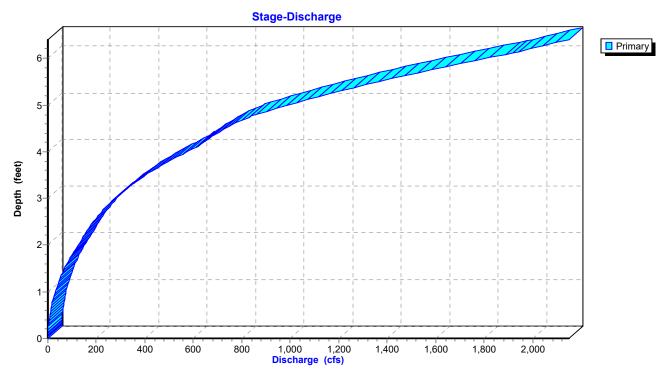


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Reach 3R: Reach 3



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Summary for Reach 4R: Reach 2

18.430 ac, 0.00% Impervious, Inflow Depth = 5.00" for 100-Year event Inflow Area =

106.97 cfs @ 12.15 hrs, Volume= Inflow 7.679 af

7.679 af, Atten= 4%, Lag= 5.1 min Outflow 103.03 cfs @ 12.23 hrs, Volume=

Routing by Stor-Ind+Trans method, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.91 fps, Min. Travel Time= 2.9 min

Avg. Velocity = 1.62 fps, Avg. Travel Time= 8.8 min

Peak Storage= 17,892 cf @ 12.18 hrs Average Depth at Peak Storage= 1.08'

Bank-Full Depth= 3.24' Flow Area= 204.3 sf, Capacity= 1,999.47 cfs

Custom cross-section, Length= 853.0' Slope= 0.0225 '/' (106 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 1,009.12', Outlet Invert= 989.90'

#

Elevation	Chan.Depth
(feet)	(feet)
1,009.14	0.00
1,008.00	1.14
1,007.80	1.34
1,007.74	1.40
1,006.00	3.14
1,005.90	3.24
1,005.99	3.15
1,005.99	3.15
1,006.00	3.14
1,007.74	1.40
1,008.00	1.14
1,008.27	0.87
1,008.47	0.67
1,008.47	0.67
1,008.89	0.25
1,009.14	0.00
	(feet) 1,009.14 1,008.00 1,007.80 1,007.74 1,006.00 1,005.90 1,005.99 1,006.00 1,007.74 1,008.00 1,008.27 1,008.47 1,008.89

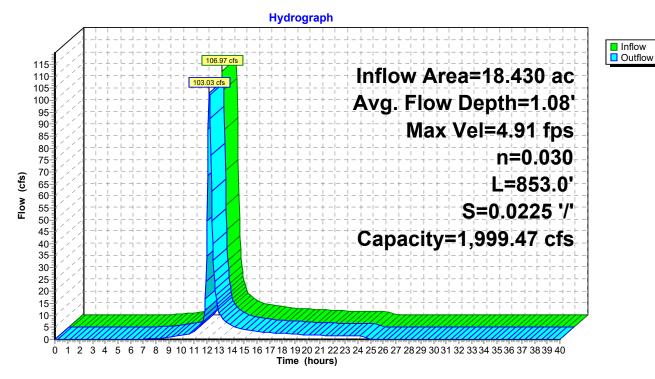
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Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
0.09	0.1	2.9	47	0.03
0.10	0.1	4.0	77	0.06
1.84	61.2	66.4	52,179	430.84
1.90	65.2	68.7	55,628	468.55
2.10	79.7	76.6	67,995	608.70
2.37	102.4	92.0	87,382	818.03
2.57	122.0	104.3	104,091	1,007.45
2.99	171.2	130.2	146,042	1,527.76
3.24	204.3	135.4	174,303	1,999.47

Reach 4R: Reach 2

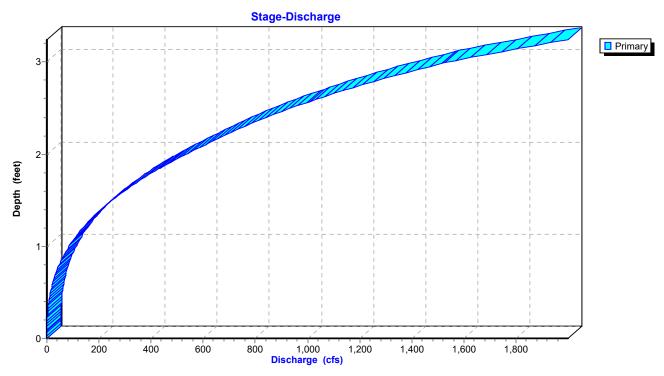


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Reach 4R: Reach 2



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Summary for Reach 8R: Reach 1

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 5.00" for 100-Year event

Inflow = 110.11 cfs @ 12.09 hrs, Volume= 7.679 af

Outflow = 106.97 cfs @ 12.15 hrs, Volume= 7.679 af, Atten= 3%, Lag= 3.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Max. Velocity= 7.87 fps, Min. Travel Time= 1.9 min

Avg. Velocity = 1.86 fps, Avg. Travel Time= 7.8 min

Peak Storage= 12,079 cf @ 12.11 hrs Average Depth at Peak Storage= 0.50'

Bank-Full Depth= 1.86' Flow Area= 89.8 sf, Capacity= 1,475.63 cfs

Custom cross-section, Length= 875.0' Slope= 0.0147 '/' (108 Elevation Intervals)

Constant n= 0.012 Concrete pipe, finished

Inlet Invert= 1,021.96', Outlet Invert= 1,009.12'

‡

Offset (feet)	Elevation (feet)	Chan.Depth (feet)
6.08	1,023.78	0.00
9.16	1,023.55	0.23
11.33	1,023.58	0.20
11.39	1,023.57	0.21
15.58	1,023.21	0.57
22.68	1,022.85	0.93
26.25	1,022.76	1.02
30.78	1,022.00	1.78
35.77	1,021.93	1.85
36.49	1,021.92	1.86
37.38	1,021.93	1.85
42.61	1,021.97	1.81
45.38	1,021.97	1.81
54.65	1,022.00	1.78
58.89	1,022.15	1.63
59.16	1,022.15	1.63
60.04	1,022.18	1.60
63.66	1,022.37	1.41
70.33	1,022.84	0.94
84.24	1,023.78	0.00

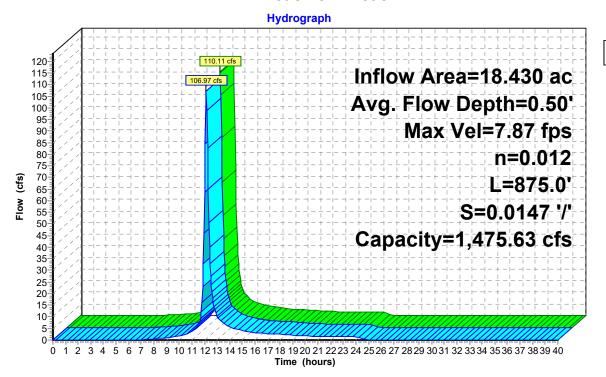
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P	a	g	е	5	•

End Area	Perim.	Storage	Discharge
(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.0	0.0	0	0.00
0.0	1.6	7	0.00
0.2	12.5	205	0.25
0.8	23.9	682	1.19
4.7	29.3	4,152	21.15
5.6	30.4	4,934	27.55
11.9	35.1	10,372	86.23
27.1	43.0	23,687	297.75
30.7	47.3	26,843	344.59
31.2	47.9	27,258	350.85
50.6	60.3	44,272	674.94
72.6	69.4	63,526	1,122.85
74.0	71.6	64,757	1,134.97
74.7	72.7	65,388	1,145.01
89.8	78.3	78,572	1,475.63
	(sq-ft) 0.0 0.0 0.2 0.8 4.7 5.6 11.9 27.1 30.7 31.2 50.6 72.6 74.0 74.7	(sq-ft) (feet) 0.0 0.0 0.0 1.6 0.2 12.5 0.8 23.9 4.7 29.3 5.6 30.4 11.9 35.1 27.1 43.0 30.7 47.3 31.2 47.9 50.6 60.3 72.6 69.4 74.0 71.6 74.7 72.7	(sq-ft) (feet) (cubic-feet) 0.0 0.0 0 0.0 1.6 7 0.2 12.5 205 0.8 23.9 682 4.7 29.3 4,152 5.6 30.4 4,934 11.9 35.1 10,372 27.1 43.0 23,687 30.7 47.3 26,843 31.2 47.9 27,258 50.6 60.3 44,272 72.6 69.4 63,526 74.0 71.6 64,757 74.7 72.7 65,388

Reach 8R: Reach 1



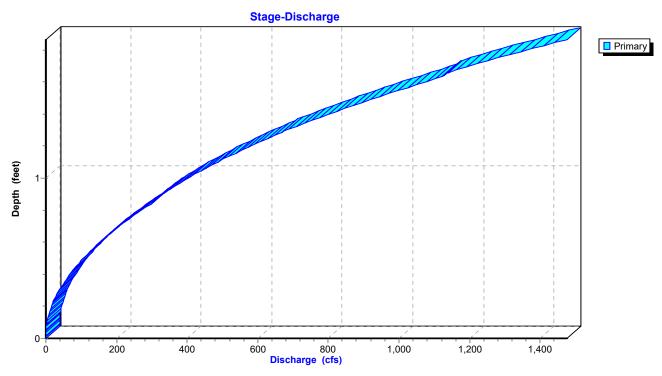


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Reach 8R: Reach 1



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Summary for Link 2L: RP-1

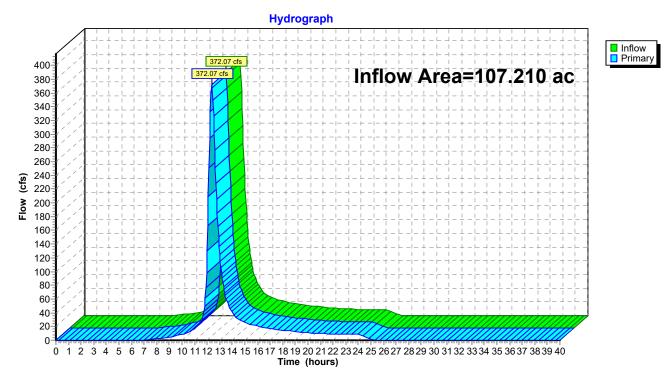
Inflow Area = 107.210 ac, 0.00% Impervious, Inflow Depth = 4.99" for 100-Year event

Inflow = 372.07 cfs @ 12.36 hrs, Volume= 44.609 af

Primary = 372.07 cfs @ 12.36 hrs, Volume= 44.609 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 2L: RP-1



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Summary for Link 5L: RP-2

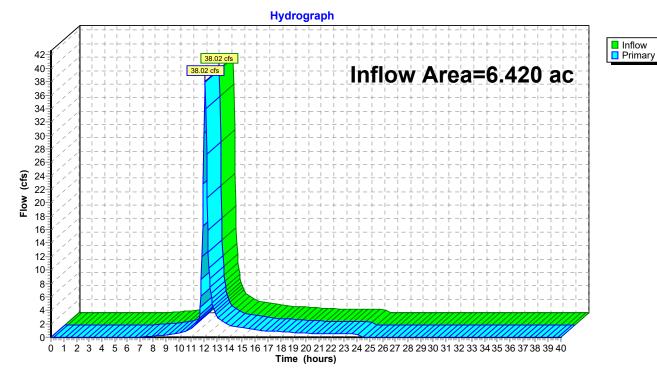
Inflow Area = 6.420 ac, 0.00% Impervious, Inflow Depth = 4.89" for 100-Year event

Inflow = 38.02 cfs @ 12.09 hrs, Volume= 2.614 af

Primary = 38.02 cfs @ 12.09 hrs, Volume= 2.614 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 5L: RP-2



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Summary for Link 6L: RP-4

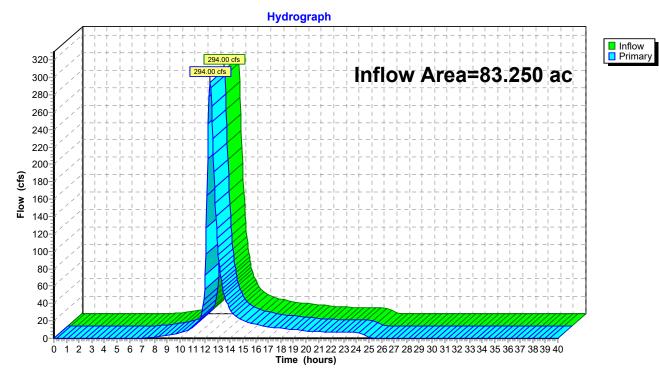
Inflow Area = 83.250 ac, 0.00% Impervious, Inflow Depth = 5.00" for 100-Year event

Inflow = 294.00 cfs @ 12.38 hrs, Volume= 34.687 af

Primary = 294.00 cfs @ 12.38 hrs, Volume= 34.687 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 6L: RP-4



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Summary for Link 7L: RP-3

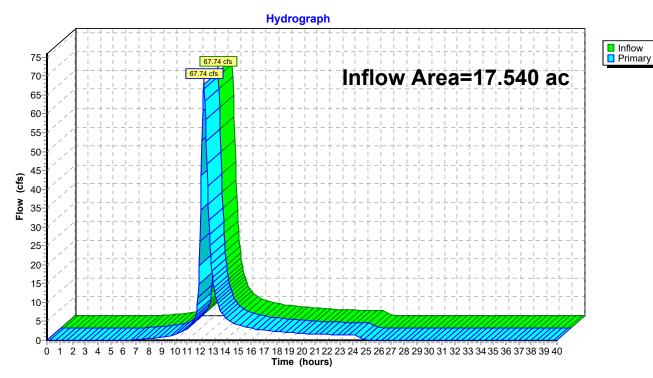
Inflow Area = 17.540 ac, 0.00% Impervious, Inflow Depth = 5.00" for 100-Year event

Inflow = 67.74 cfs @ 12.31 hrs, Volume= 7.308 af

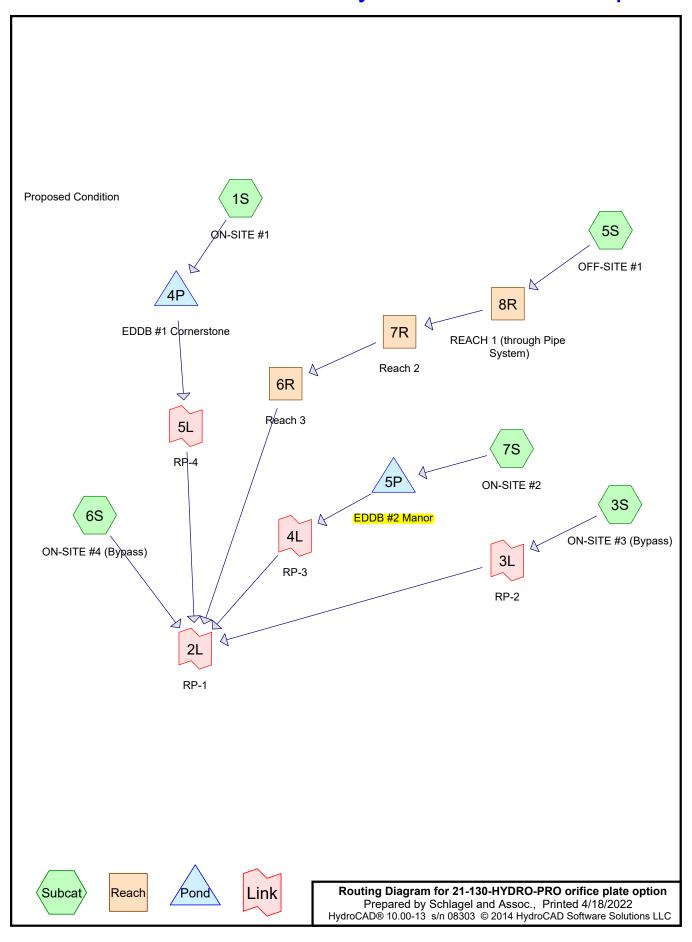
Primary = 67.74 cfs @ 12.31 hrs, Volume= 7.308 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.05 hrs

Link 7L: RP-3



HydroCAD - Developed



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Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
32.070	87	1/4 acre lots, 38% imp, HSG D (3S, 7S)
11.140	92	1/8 acre lots, 65% imp, HSG D (1S)
22.730	80	>75% Grass cover, Good, HSG D (6S)
19.330	92	Duplex/Townhomes (1S)
3.190	95	Urban commercial, 85% imp, HSG D (1S, 6S)
15.640	76	Woods/grass comb., Fair, HSG C (5S)
2.790	82	Woods/grass comb., Fair, HSG D (5S)

Bailey Farms - Developed Type II 24-hr 2-Year Rainfall=3.50" Printed 4/18/2022

21-130-HYDRO-PRO orifice plate option

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: ON-SITE#1Runoff Area=31.710 ac 26.16% Impervious Runoff Depth=2.64"

Flow Length=1,186' Slope=0.0250 '/' Tc=14.0 min CN=92 Runoff=107.19 cfs 6.968 af

Subcatchment3S: ON-SITE#3 (Bypass) Runoff Area=2.380 ac 38.00% Impervious Runoff Depth=2.18" Flow Length=1,110' Tc=12.7 min CN=87 Runoff=7.15 cfs 0.433 af

Subcatchment5S: OFF-SITE#1 Runoff Area=18.430 ac 0.00% Impervious Runoff Depth=1.43" Flow Length=1,247' Tc=17.6 min CN=77 Runoff=30.84 cfs 2.197 af

Subcatchment6S: ON-SITE#4 (Bypass) Runoff Area=24.680 ac 6.72% Impervious Runoff Depth=1.71" Flow Length=1,186' Slope=0.0250 '/' Tc=15.5 min CN=81 Runoff=53.19 cfs 3.514 af

Subcatchment7S: ON-SITE#2Runoff Area=29.690 ac 38.00% Impervious Runoff Depth=2.18"

Flow Length=1,769' Slope=0.0250 '/' Tc=13.8 min CN=87 Runoff=86.05 cfs 5.400 af

Reach 6R: Reach 3Avg. Flow Depth=0.91' Max Vel=3.48 fps Inflow=27.77 cfs 2.197 af n=0.030 L=1,041.0' S=0.0088 '/' Capacity=2,150.14 cfs Outflow=25.59 cfs 2.197 af

Reach 7R: Reach 2Avg. Flow Depth=0.66' Max Vel=3.55 fps Inflow=29.97 cfs 2.197 af n=0.030 L=853.0' S=0.0225 '/' Capacity=1,999.47 cfs Outflow=27.77 cfs 2.197 af

Reach 8R: REACH 1 (through Pipe Avg. Flow Depth=1.19' Max Vel=11.56 fps Inflow=30.84 cfs 2.197 af 36.0" Round Pipe n=0.012 L=1,067.0' S=0.0159 '/' Capacity=91.10 cfs Outflow=29.97 cfs 2.197 af

Pond 4P: EDDB #1 Cornerstone Peak Elev=1,003.03' Storage=164,409 cf Inflow=107.19 cfs 6.968 af Outflow=13.32 cfs 6.025 af

Pond 5P: EDDB #2 Manor Peak Elev=994.63' Storage=122,558 cf Inflow=86.05 cfs 5.400 af

Outflow=9.97 cfs 4.497 af

Link 2L: RP-1Inflow=75.14 cfs 16.666 af

Primary=75.14 cfs 16.666 af

Link 3L: RP-2Inflow=7.15 cfs 0.433 af
Primary=7.15 cfs 0.433 af

Link 4L: RP-3 Inflow=9.97 cfs 4.497 af

Link 5L: RP-4 Inflow=13.32 cfs 6.025 af

Primary=13.32 cfs 6.025 af

Primary=9.97 cfs 4.497 af

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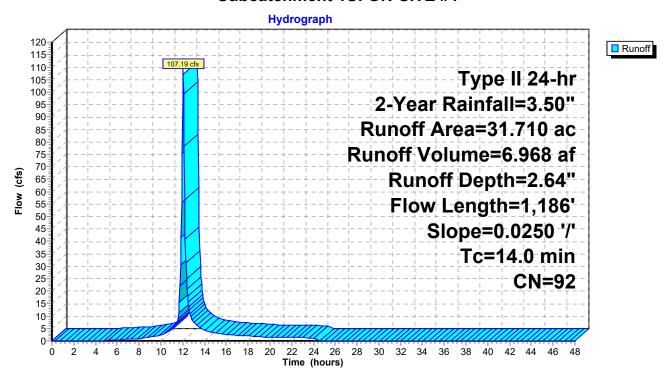
Summary for Subcatchment 1S: ON-SITE #1

Runoff = 107.19 cfs @ 12.05 hrs, Volume= 6.968 af, Depth= 2.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

	Area	(ac)	CN	l Desc	cription						
*	19.	330	92	2 Dupl	uplex/Townhomes						
	11.	140	92	92 1/8 acre lots, 65% imp, HSG D							
_	1.	240	95	5 Urba	ın commer	cial, 85% i	mp, HSG D				
	31.	710	92	2 Weig	hted Aver	age					
	23.	415		73.8	4% Pervio	us Area					
	8.	295		26.1	6% Imper	∕ious Area					
	Тс	Leng	th	Slope	Velocity	Capacity	Description				
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)					
	8.4	10	0	0.0250	0.20		Sheet Flow,				
							Grass: Short n= 0.150 P2= 3.60"				
	5.6	1,08	86	0.0250	3.21		Shallow Concentrated Flow,				
_							Paved Kv= 20.3 fps				
	14.0	1,18	86	Total							

Subcatchment 1S: ON-SITE #1



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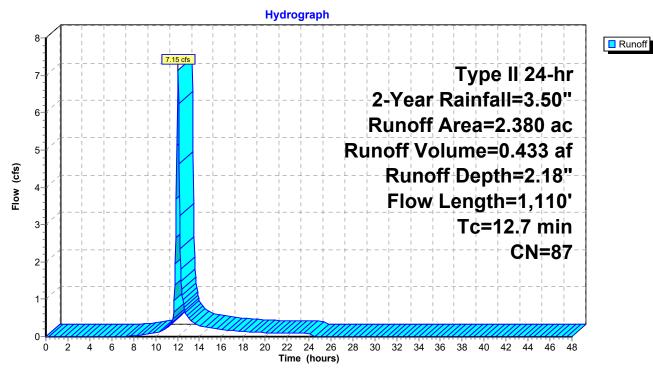
Summary for Subcatchment 3S: ON-SITE #3 (Bypass)

Runoff = 7.15 cfs @ 12.04 hrs, Volume= 0.433 af, Depth= 2.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

_	Area	(ac) C	N Desc	cription			
2.380 87 1/4 acre lots, 38% imp, HSG D							
_	1.476 62.00% Pervious Area						
0.904 38.00% Impervious Area							
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	4.9	50	0.0250	0.17		Sheet Flow,	
	7.8	1,060	0.0200	2.28		n= 0.150 P2= 3.60" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
	12 7	1 110	Total				

Subcatchment 3S: ON-SITE #3 (Bypass)



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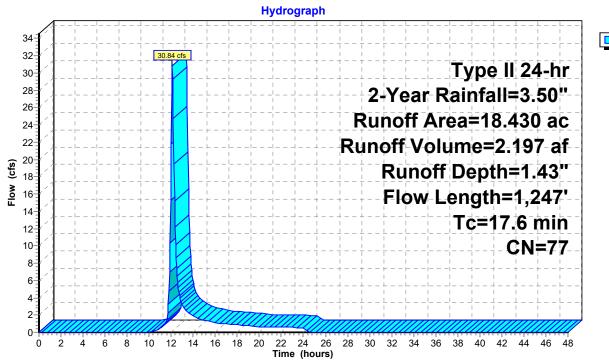
Summary for Subcatchment 5S: OFF-SITE #1

Runoff = 30.84 cfs @ 12.11 hrs, Volume= 2.197 af, Depth= 1.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

_	Area	(ac) C	N Desc	cription			
	15.	640 7	76 Woo	ds/grass d	omb., Fair,	HSG C	
2.790 82 Woods/grass comb., Fair, HSG D							
18.430 77 Weighted Average							
	18.	430	100.	00% Pervi	ous Area		
	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	8.4	100	0.0250	0.20		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.60"	
	9.2	1,147	0.0192	2.08		Shallow Concentrated Flow,	
						Grassed Waterway Kv= 15.0 fps	
_	17.6	1 247	Total				

Subcatchment 5S: OFF-SITE #1





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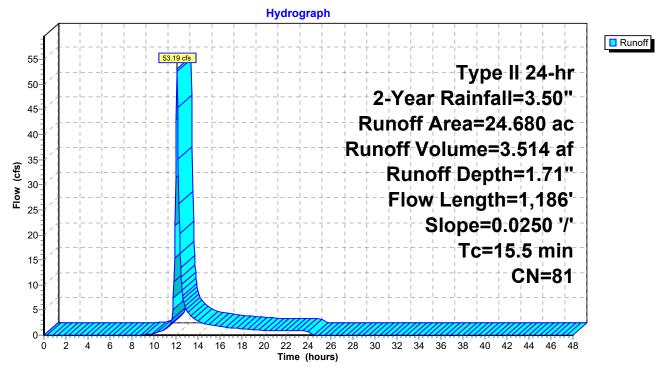
Summary for Subcatchment 6S: ON-SITE #4 (Bypass)

Runoff = 53.19 cfs @ 12.08 hrs, Volume= 3.514 af, Depth= 1.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

_	Area	(ac)	CN Description					
22.730 80 >75% Grass cover, Good, HSG D								
1.950 95 Urban commercial, 85% imp, HSG D							mp, HSG D	
24.680 81 Weighted Average								
	23.	023		93.2	8% Pervio	us Area		
	1.	657		6.72	% Impervi	ous Area		
	Tc Length			lope	Velocity	Capacity	Description	
_	(min)	(feet) ((ft/ft)	(ft/sec)	(cfs)		
	8.4	100	0.0	250	0.20		Sheet Flow,	
							Grass: Short n= 0.150 P2= 3.60"	
	7.1	1,086	0.0)250	2.55		Shallow Concentrated Flow,	
_							Unpaved Kv= 16.1 fps	
	15.5	1,186	oT 6	tal				

Subcatchment 6S: ON-SITE #4 (Bypass)



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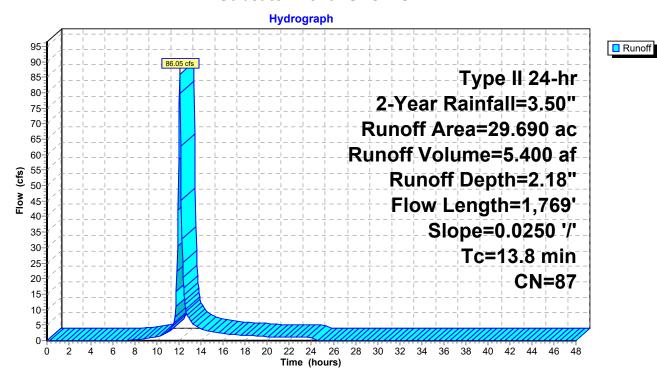
Summary for Subcatchment 7S: ON-SITE #2

Runoff = 86.05 cfs @ 12.06 hrs, Volume= 5.400 af, Depth= 2.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

Area (ac) CN Description							
29.690 87 1/4 acre lots, 38% imp, HSG D							
	18.	408	62.0	0% Pervio	us Area		
11.282 38.00% Impervious Area					ious Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	4.9	50	0.0250	0.17		Sheet Flow,	
	8.9	1,719	0.0250	3.21		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow, Paved Kv= 20.3 fps	
	13.8	1 769	Total				

Subcatchment 7S: ON-SITE #2



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Summary for Reach 6R: Reach 3

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 1.43" for 2-Year event

Inflow = 27.77 cfs @ 12.27 hrs, Volume= 2.197 af

Outflow = 25.59 cfs @ 12.42 hrs, Volume= 2.197 af, Atten= 8%, Lag= 8.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.48 fps, Min. Travel Time= 5.0 min Avg. Velocity = 0.90 fps, Avg. Travel Time= 19.4 min

Peak Storage= 7,714 cf @ 12.33 hrs Average Depth at Peak Storage= 0.91'

Bank-Full Depth= 6.40' Flow Area= 252.4 sf, Capacity= 2,150.14 cfs

Custom cross-section, Length= 1,041.0' Slope= 0.0088 '/' (110 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 989.92', Outlet Invert= 980.77'

‡	

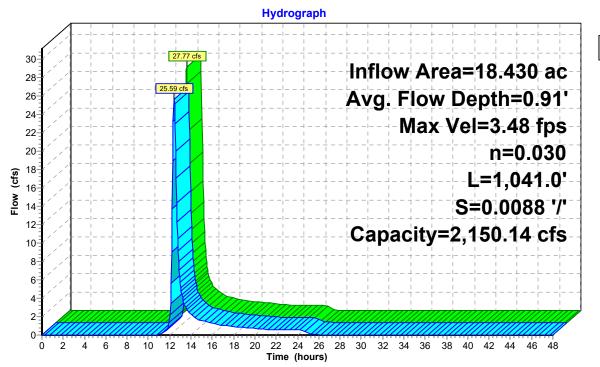
Offset	Elevation	Chan.Depth
(feet)	(feet)	(feet)
0.00	996.29	0.00
7.00	996.00	0.29
12.99	994.06	2.23
13.24	994.00	2.29
14.48	993.84	2.45
28.84	992.00	4.29
29.66	991.71	4.58
36.75	990.00	6.29
39.18	989.92	6.37
39.90	989.89	6.40
42.20	989.97	6.32
43.14	990.00	6.29
43.91	990.40	5.89
46.34	992.00	4.29
53.90	993.41	2.88
56.74	994.00	2.29
62.76	994.22	2.07
76.31	994.65	1.64
100.00	996.29	0.00

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Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
0.03	0.0	1.6	25	0.01
0.08	0.2	4.5	184	0.09
0.11	0.3	6.4	355	0.22
0.51	3.4	9.0	3,521	8.20
1.82	19.8	16.9	20,607	102.00
2.11	24.7	18.3	25,700	139.72
3.52	62.5	37.1	65,011	410.25
3.95	79.1	42.6	82,369	555.08
4.11	85.9	44.7	89,447	617.31
4.17	88.6	46.6	92,223	631.78
4.33	96.2	51.5	100,189	678.51
4.76	121.1	66.4	126,018	838.92
6.11	225.0	90.3	234,197	1,919.54
6.40	252.4	101.5	262,697	2,150.14

Reach 6R: Reach 3





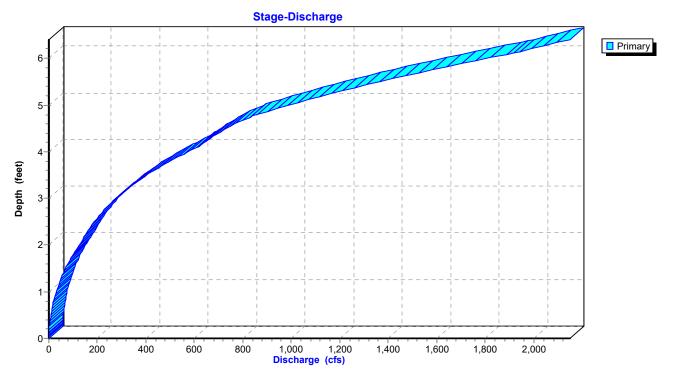
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Reach 6R: Reach 3



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Summary for Reach 7R: Reach 2

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 1.43" for 2-Year event

Inflow = 29.97 cfs @ 12.15 hrs, Volume= 2.197 af

Outflow = 27.77 cfs @ 12.27 hrs, Volume= 2.197 af, Atten= 7%, Lag= 7.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.55 fps, Min. Travel Time= 4.0 min Avg. Velocity = 1.30 fps, Avg. Travel Time= 10.9 min

Peak Storage= 6,767 cf @ 12.21 hrs Average Depth at Peak Storage= 0.66'

Bank-Full Depth= 3.24' Flow Area= 204.3 sf, Capacity= 1,999.47 cfs

Custom cross-section, Length= 853.0' Slope= 0.0225 '/' (106 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 1,009.12', Outlet Invert= 989.90'

‡

Elevation	Chan.Depth	
(feet)	(feet)	
1,009.14	0.00	
1,008.00	1.14	
1,007.80	1.34	
1,007.74	1.40	
1,006.00	3.14	
1,005.90	3.24	
1,005.99	3.15	
1,005.99	3.15	
1,006.00	3.14	
1,007.74	1.40	
1,008.00	1.14	
1,008.27	0.87	
1,008.47	0.67	
1,008.47	0.67	
1,008.89	0.25	
1,009.14	0.00	
	(feet) 1,009.14 1,008.00 1,007.80 1,007.74 1,006.00 1,005.90 1,005.99 1,006.00 1,007.74 1,008.00 1,008.27 1,008.47 1,008.89	

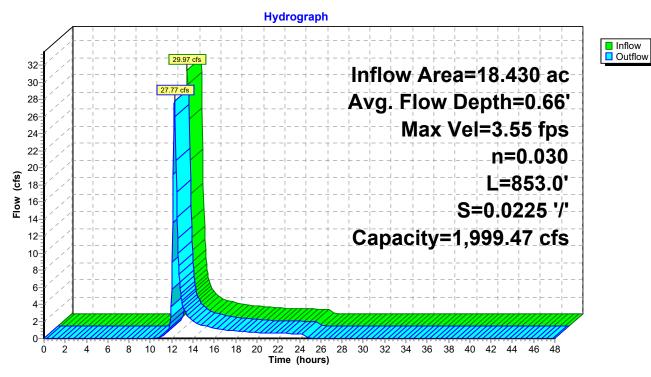
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Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
0.09	0.1	2.9	47	0.03
0.10	0.1	4.0	77	0.06
1.84	61.2	66.4	52,179	430.84
1.90	65.2	68.7	55,628	468.55
2.10	79.7	76.6	67,995	608.70
2.37	102.4	92.0	87,382	818.03
2.57	122.0	104.3	104,091	1,007.45
2.99	171.2	130.2	146,042	1,527.76
3.24	204.3	135.4	174,303	1,999.47

Reach 7R: Reach 2

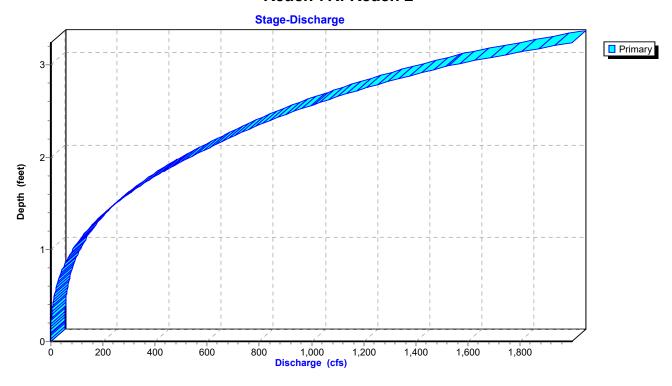


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Reach 7R: Reach 2



Inflow

Outflow

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Summary for Reach 8R: REACH 1 (through Pipe System)

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 1.43" for 2-Year event

Inflow = 30.84 cfs @ 12.11 hrs, Volume= 2.197 af

Outflow = 29.97 cfs @ 12.15 hrs, Volume= 2.197 af, Atten= 3%, Lag= 2.8 min

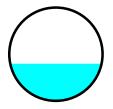
Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Max. Velocity= 11.56 fps, Min. Travel Time= 1.5 min Avg. Velocity = 4.01 fps, Avg. Travel Time= 4.4 min

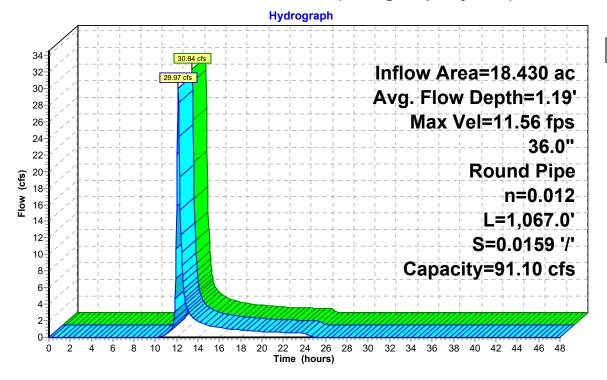
Peak Storage= 2,798 cf @ 12.13 hrs Average Depth at Peak Storage= 1.19'

Bank-Full Depth= 3.00' Flow Area= 7.1 sf, Capacity= 91.10 cfs

36.0" Round Pipe n= 0.012 Concrete pipe, finished Length= 1,067.0' Slope= 0.0159 '/' Inlet Invert= 1,021.96', Outlet Invert= 1,005.00'



Reach 8R: REACH 1 (through Pipe System)

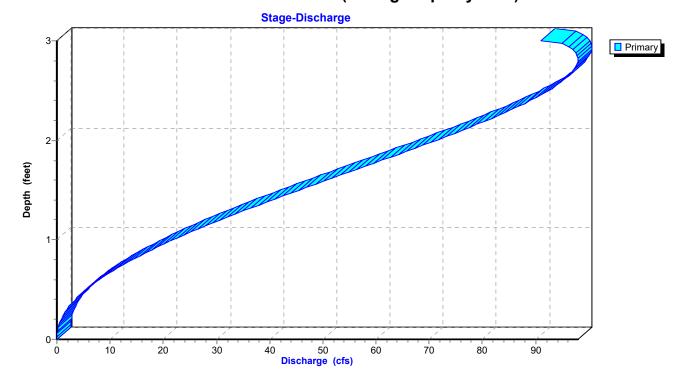


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Reach 8R: REACH 1 (through Pipe System)



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Summary for Pond 4P: EDDB #1 Cornerstone

Inflow Area = 31.710 ac, 26.16% Impervious, Inflow Depth = 2.64" for 2-Year event

Inflow = 107.19 cfs @ 12.05 hrs, Volume= 6.968 af

Outflow = 13.32 cfs @ 12.57 hrs, Volume= 6.025 af, Atten= 88%, Lag= 30.8 min

Primary = 13.32 cfs @ 12.57 hrs, Volume= 6.025 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Peak Elev= 1,003.03' @ 12.57 hrs Surf.Area= 44,926 sf Storage= 164,409 cf

Plug-Flow detention time= 398.2 min calculated for 6.025 af (86% of inflow)

Center-of-Mass det. time= 334.8 min (1,133.6 - 798.8)

Volume	Inve	rt Avail.Sto	rage Storage	Description			
#1	995.67			6 cf Custom Stage Data (Prismatic)Listed below (Recalc)			
Clayatia	n (Surf.Area	Inc.Store	Cum.Store			
Elevation							
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)			
995.6		0	0	0			
996.0		700	116	116			
997.0		7,200	3,950	4,066			
998.0		12,000	9,600	13,666			
999.0		17,900	14,950	28,616			
1,000.0		27,000	22,450	51,066			
1,001.0	00	36,000	31,500	82,566			
1,002.0	00	40,000	38,000	120,566			
1,003.0	00	44,800	42,400	162,966			
1,004.0	00	48,700	46,750	209,716			
1,005.0	00	53,000	50,850	260,566			
1,006.0	00	57,000	55,000	315,566			
1,007.0	00	61,000	59,000	374,566			
,		,	,	,			
Device	Routing	Invert	Outlet Device	es			
#1	Primary	993.54'	30.0" Round	d 30" Culvert			
	-		L= 87.0' RC	P, sq.cut end pro	jecting, Ke= 0.500		
			Inlet / Outlet I	nvert= 993.54' / 9	993.11' S= 0.0049 '/' Cc= 0.900		
			n= 0.012 Cor	ncrete pipe, finish	ned, Flow Area= 4.91 sf		
#2	Device 1	994.16'		Q Orifice C= 0.6			
#3	Device 2	994.36'	8.0" Round	8" PVC Pipe			
				L= 20.0' RCP, groove end projecting, Ke= 0.200			
			Inlet / Outlet Invert= 994.36' / 994.16' S= 0.0100 '/' Cc= 0.900				
			n= 0.012, Flow Area= 0.35 sf				
#4	Device 3	995.67'	,	ser Pipe X 8.00 o	columns		
,, .	_ 000 0	000.01		4.0" cc spacing			
#5	Device 1	1,001.05'			ng in Box C= 0.600		
#5 #6	Device 1	1,004.00'			p of Box C= 0.600		
πΟ	201100 1	1,004.00		ir flow at low hea			
			Littlica to We	ii iiow at iow iiea	uo		

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Primary OutFlow Max=13.31 cfs @ 12.57 hrs HW=1,003.03' (Free Discharge)

1=30" Culvert (Passes 13.31 cfs of 67.85 cfs potential flow)

2=WQ Orifice (Orifice Controls 0.65 cfs @ 14.24 fps)

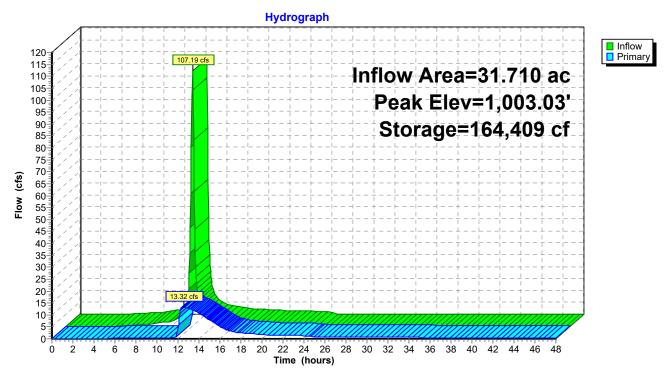
3=8" PVC Pipe (Passes 0.65 cfs of 5.51 cfs potential flow)

4=Riser Pipe (Passes 0.65 cfs of 4.61 cfs potential flow)

5=Opening in Box (Orifice Controls 12.66 cfs @ 6.33 fps)

6=Open Top of Box (Controls 0.00 cfs)

Pond 4P: EDDB #1 Cornerstone

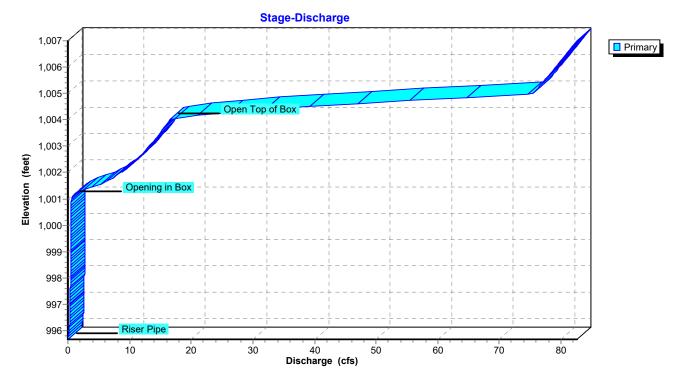


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Pond 4P: EDDB #1 Cornerstone



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#5

#6

Device 1

Device 1

995.25'

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Summary for Pond 5P: EDDB #2 Manor

Inflow Area = 29.690 ac, 38.00% Impervious, Inflow Depth = 2.18" for 2-Year event

Inflow = 86.05 cfs @ 12.06 hrs, Volume= 5.400 af

Outflow = 9.97 cfs @ 12.62 hrs, Volume= 4.497 af, Atten= 88%, Lag= 33.7 min

Primary = 9.97 cfs @ 12.62 hrs, Volume= 4.497 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Peak Elev= 994.63' @ 12.62 hrs Surf.Area= 44,414 sf Storage= 122,558 cf

Plug-Flow detention time= 407.5 min calculated for 4.492 af (83% of inflow)

Center-of-Mass det. time= 335.4 min (1,154.7 - 819.3)

			`	,		
Volume	Inve	rt Avail.Sto	rage Storage	Description		
#1	988.16	6' 352,23	36 cf Custon	n Stage Data (Prismatio	Listed below (Recalc)	
				· ·	,	
Elevation	on S	Surf.Area	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)		
988.	16	0	0	0		
989.0	00	800	336	336		
990.0	00	4,200	2,500	2,836		
991.0	00	10,400	7,300	10,136		
992.0	00	21,600	16,000	26,136		
993.0		37,600	29,600	55,736		
994.0		41,700	39,650	95,386		
995.0		46,000	43,850	139,236		
996.0		49,700	47,850	187,086		
997.0		53,200	51,450	238,536		
998.0		56,900	55,050	293,586		
999.0	00	60,400	58,650	352,236		
Device	Routing	Invert	Outlet Device	es		
#1	Primary	986.00'		d 30" Culvert		
" .	1 milary	000.00			Ke= 0 500	
			L= 77.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 986.00' / 985.62' S= 0.0049 '/' Cc= 0.900			
				ncrete pipe, finished, Flo		
#2	Device 1	986.62'		Q Orifice C= 0.600		
#3	Device 2	986.86'	8.0" Round	8" PVC Pipe		
				P, sq.cut end projecting,	Ke= 0.500	
					S= 0.0120 '/' Cc= 0.900	
			n= 0.012 Co	rrugated PP, smooth inte	erior, Flow Area= 0.35 sf	
#4	Device 3	988.16'		ser Pipe X 8.00 column		
			X 8 rows with	4.0" cc spacing C= 0.60	00	

993.40' **48.0" W x 6.0" H Vert. Opening in Box** C= 0.600

Limited to weir flow at low heads

60.0" x **48.0"** Horiz. Open Top of Box C= 0.600

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Primary OutFlow Max=9.97 cfs @ 12.62 hrs HW=994.63' (Free Discharge)

1=30" Culvert (Passes 9.97 cfs of 64.21 cfs potential flow)

2=WQ Orifice (Orifice Controls 0.46 cfs @ 13.54 fps)

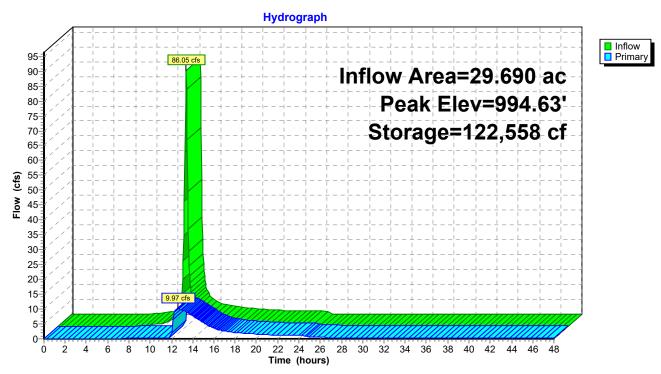
3=8" PVC Pipe (Passes 0.46 cfs of 4.58 cfs potential flow)

4=Riser Pipe (Passes 0.46 cfs of 3.85 cfs potential flow)

5=Opening in Box (Orifice Controls 9.51 cfs @ 4.75 fps)

6=Open Top of Box (Controls 0.00 cfs)

Pond 5P: EDDB #2 Manor

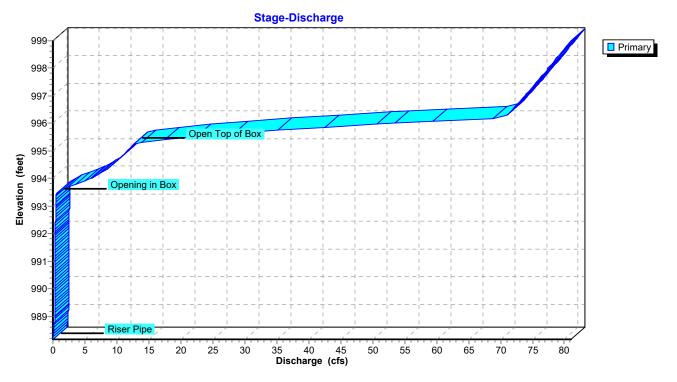


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Pond 5P: EDDB #2 Manor



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Summary for Link 2L: RP-1

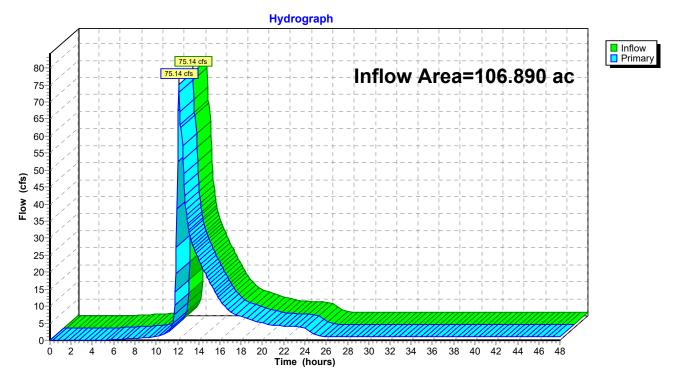
Inflow Area = 106.890 ac, 20.71% Impervious, Inflow Depth > 1.87" for 2-Year event

Inflow = 75.14 cfs @ 12.12 hrs, Volume= 16.666 af

Primary = 75.14 cfs @ 12.12 hrs, Volume= 16.666 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 2L: RP-1



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Summary for Link 3L: RP-2

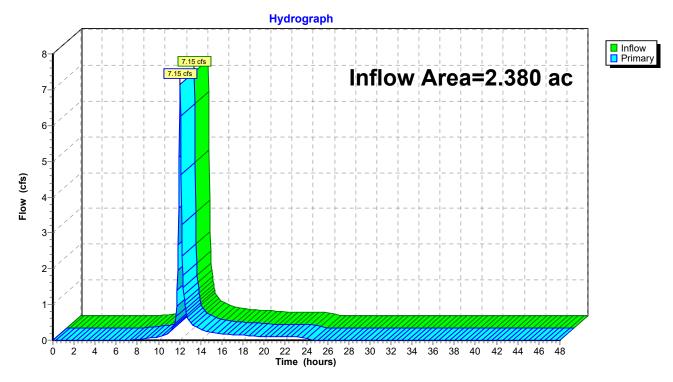
Inflow Area = 2.380 ac, 38.00% Impervious, Inflow Depth = 2.18" for 2-Year event

Inflow

7.15 cfs @ 12.04 hrs, Volume= 0.433 af 7.15 cfs @ 12.04 hrs, Volume= 0.433 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 3L: RP-2



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Summary for Link 4L: RP-3

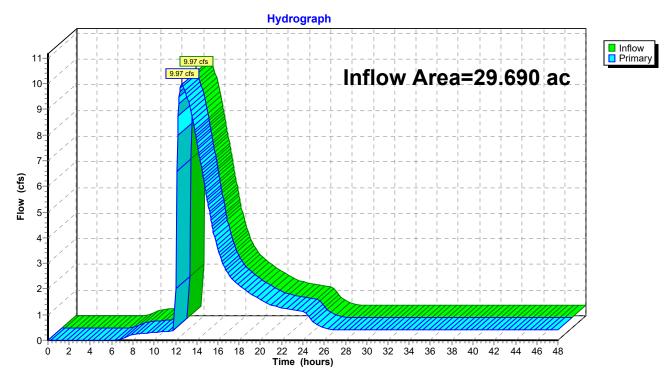
Inflow Area = 29.690 ac, 38.00% Impervious, Inflow Depth > 1.82" for 2-Year event

Inflow = 9.97 cfs @ 12.62 hrs, Volume= 4.497 af

Primary = 9.97 cfs @ 12.62 hrs, Volume= 4.497 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 4L: RP-3



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Summary for Link 5L: RP-4

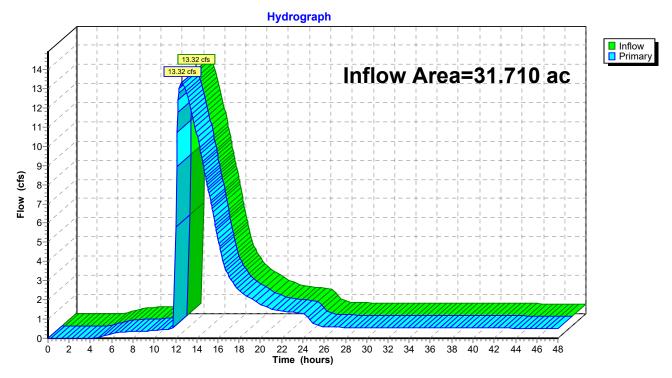
Inflow Area = 31.710 ac, 26.16% Impervious, Inflow Depth > 2.28" for 2-Year event

Inflow = 13.32 cfs @ 12.57 hrs, Volume= 6.025 af

Primary = 13.32 cfs @ 12.57 hrs, Volume= 6.025 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 5L: RP-4



Bailey Farms - Developed Type II 24-hr 10-Year Rainfall=5.30" Printed 4/18/2022

Primary=214.00 cfs 31.088 af

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: ON-SITE#1Runoff Area=31.710 ac 26.16% Impervious Runoff Depth=4.38"

Flow Length=1,186' Slope=0.0250 '/' Tc=14.0 min CN=92 Runoff=173.27 cfs 11.581 af

Subcatchment3S: ON-SITE#3 (Bypass) Runoff Area=2.380 ac 38.00% Impervious Runoff Depth=3.85" Flow Length=1,110' Tc=12.7 min CN=87 Runoff=12.33 cfs 0.764 af

Subcatchment5S: OFF-SITE#1 Runoff Area=18.430 ac 0.00% Impervious Runoff Depth=2.88" Flow Length=1,247' Tc=17.6 min CN=77 Runoff=62.92 cfs 4.417 af

Subcatchment6S: ON-SITE#4 (Bypass) Runoff Area=24.680 ac 6.72% Impervious Runoff Depth=3.25" Flow Length=1,186' Slope=0.0250 '/' Tc=15.5 min CN=81 Runoff=100.98 cfs 6.688 af

Subcatchment7S: ON-SITE#2 Runoff Area=29.690 ac 38.00% Impervious Runoff Depth=3.85" Flow Length=1,769' Slope=0.0250 '/' Tc=13.8 min CN=87 Runoff=148.68 cfs 9.527 af

Reach 6R: Reach 3Avg. Flow Depth=1.35' Max Vel=4.35 fps Inflow=58.33 cfs 4.417 af n=0.030 L=1,041.0' S=0.0088 '/' Capacity=2,150.14 cfs Outflow=54.88 cfs 4.417 af

Reach 7R: Reach 2Avg. Flow Depth=0.87' Max Vel=4.26 fps Inflow=61.28 cfs 4.417 af n=0.030 L=853.0' S=0.0225 '/' Capacity=1,999.47 cfs Outflow=58.33 cfs 4.417 af

Reach 8R: REACH 1 (through Pipe Avg. Flow Depth=1.82' Max Vel=13.85 fps Inflow=62.92 cfs 4.417 af 36.0" Round Pipe n=0.012 L=1,067.0' S=0.0159 '/' Capacity=91.10 cfs Outflow=61.28 cfs 4.417 af

Pond 4P: EDDB #1 Cornerstone Peak Elev=1,004.79' Storage=249,634 cf Inflow=173.27 cfs 11.581 af Outflow=60.26 cfs 10.616 af

Pond 5P: EDDB #2 Manor Peak Elev=996.06' Storage=189,935 cf Inflow=148.68 cfs 9.527 af Outflow=58.14 cfs 8.604 af

Link 2L: RP-1 Inflow=214.00 cfs 31.088 af

Link 3L: RP-2Inflow=12.33 cfs 0.764 af
Primary=12.33 cfs 0.764 af

Link 4L: RP-3Inflow=58.14 cfs 8.604 af
Primary=58.14 cfs 8.604 af

Link 5L: RP-4Inflow=60.26 cfs 10.616 af
Primary=60.26 cfs 10.616 af

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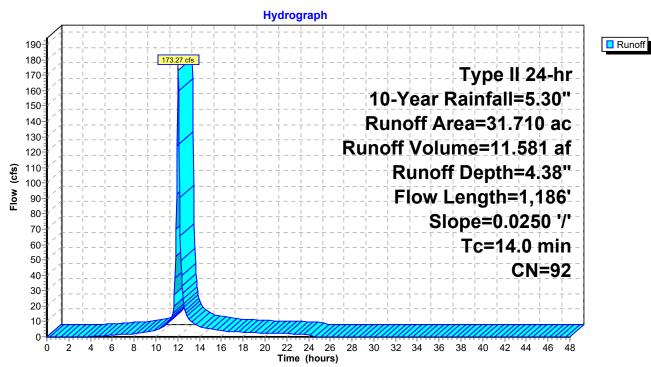
Summary for Subcatchment 1S: ON-SITE #1

Runoff = 173.27 cfs @ 12.05 hrs, Volume= 11.581 af, Depth= 4.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=5.30"

	Area	(ac)	CN	l Desc	cription		
*	19.	330	92	2 Dupl	ex/Townh	omes	
	11.	140	92	2 1/8 a	icre lots, 6	5% imp, H	SG D
	1.	240	95	5 Urba	n commer	cial, 85% i	mp, HSG D
	31.	710	92	2 Weig	hted Aver	age	
	23.	415		73.8	4% Pervio	us Area	
	8.295 26.16% Impervio			6% Imperv	/ious Area		
	Tc	Lengt	th	Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	8.4	10	0	0.0250	0.20		Sheet Flow,
							Grass: Short n= 0.150 P2= 3.60"
	5.6	1,08	6	0.0250	3.21		Shallow Concentrated Flow,
							Paved Kv= 20.3 fps
	14.0	1,18	6	Total			

Subcatchment 1S: ON-SITE #1



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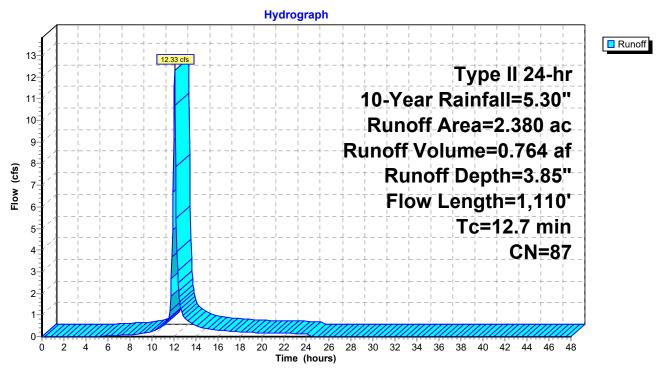
Summary for Subcatchment 3S: ON-SITE #3 (Bypass)

Runoff = 12.33 cfs @ 12.04 hrs, Volume= 0.764 af, Depth= 3.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=5.30"

	Area	(ac) C	N Desc	cription			
Ī	2.380 87 1/4 acre lots, 38% imp, HSG D						
-	1.	476	62.0	0% Pervio	us Area		
	0.	904	38.0	0% Imper	ious Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	4.9	50	0.0250	0.17		Sheet Flow,	
	7.8	1,060	0.0200	2.28		n= 0.150 P2= 3.60" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
	12 7	1 110	Total				

Subcatchment 3S: ON-SITE #3 (Bypass)



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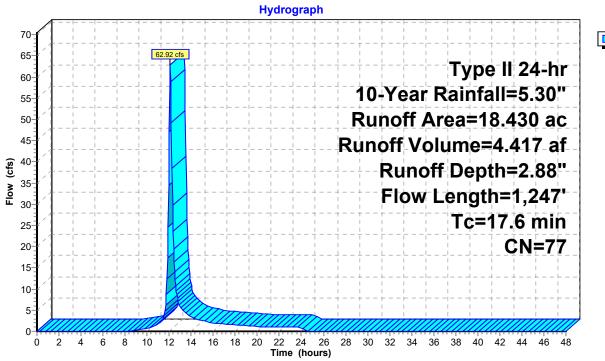
Summary for Subcatchment 5S: OFF-SITE #1

Runoff = 62.92 cfs @ 12.10 hrs, Volume= 4.417 af, Depth= 2.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=5.30"

_	Area	(ac) C	N Desc	cription		
	15.	640 7	76 Woo	ds/grass d	comb., Fair,	HSG C
	2.	790 8	32 Woo	ds/grass o	omb., Fair,	HSG D
	18.	430 7	77 Weig	ghted Aver	age	
	18.	430	100.	00% Pervi	ous Area	
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.4	100	0.0250	0.20		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.60"
	9.2	1,147	0.0192	2.08		Shallow Concentrated Flow,
						Grassed Waterway Kv= 15.0 fps
	17.6	1 247	Total			

Subcatchment 5S: OFF-SITE #1





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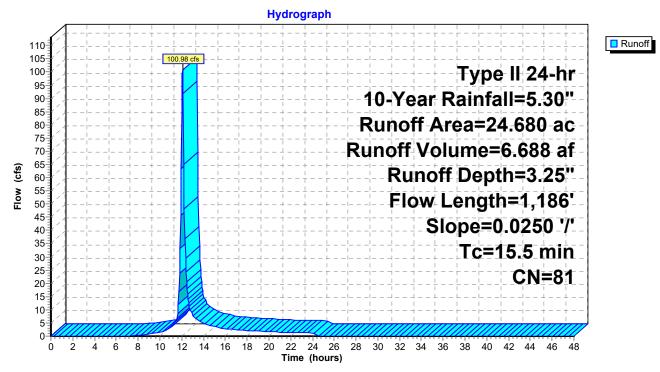
Summary for Subcatchment 6S: ON-SITE #4 (Bypass)

Runoff = 100.98 cfs @ 12.07 hrs, Volume= 6.688 af, Depth= 3.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=5.30"

 Area	(ac)	CN De	scription			
22.	730	80 >7	5% Grass c	over, Good	, HSG D	
 1.	950	95 Ur	oan comme	rcial, 85% i	mp, HSG D	
24.	680	81 W	eighted Ave	rage		
23.	023	93	.28% Pervio	ous Area		
1.	657	6.7	'2% Impervi	ious Area		
Tc (min)	Length (feet)		,	Capacity (cfs)	Description	
8.4	100	0.025	0.20		Sheet Flow,	
7.1	1,086	0.025	2.55		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
15.5	1,186	Total				

Subcatchment 6S: ON-SITE #4 (Bypass)



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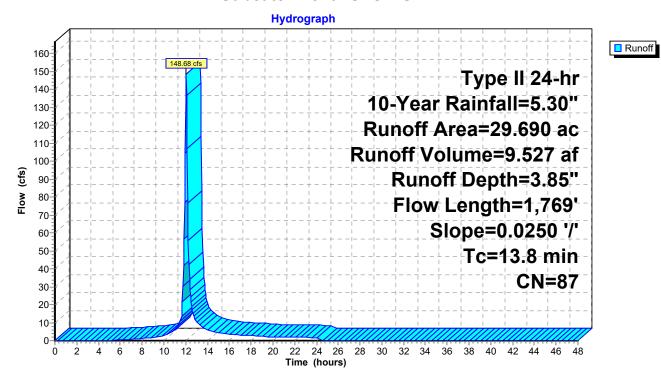
Summary for Subcatchment 7S: ON-SITE #2

Runoff = 148.68 cfs @ 12.05 hrs, Volume= 9.527 af, Depth= 3.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=5.30"

_	Area	(ac) C	N Desc	cription			
_	29.690 87 1/4 acre lots, 38% imp, HSG D						
	18.	408	62.0	0% Pervio	us Area		
	11.	282	38.0	0% Imperv	ious Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	4.9	50	0.0250	0.17		Sheet Flow,	
	8.9	1,719	0.0250	3.21		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow, Paved Kv= 20.3 fps	
	13.8	1 769	Total		•		

Subcatchment 7S: ON-SITE #2



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Summary for Reach 6R: Reach 3

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 2.88" for 10-Year event

Inflow = 58.33 cfs @ 12.24 hrs, Volume= 4.417 af

Outflow = 54.88 cfs @ 12.36 hrs, Volume= 4.417 af, Atten= 6%, Lag= 7.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.35 fps, Min. Travel Time= 4.0 min Avg. Velocity = 1.06 fps, Avg. Travel Time= 16.3 min

Peak Storage= 13,270 cf @ 12.29 hrs Average Depth at Peak Storage= 1.35'

Bank-Full Depth= 6.40' Flow Area= 252.4 sf, Capacity= 2,150.14 cfs

Custom cross-section, Length= 1,041.0' Slope= 0.0088 '/' (110 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 989.92', Outlet Invert= 980.77'

‡	

Offset (feet)	Elevation (feet)	Chan.Depth (feet)
0.00	996.29	0.00
7.00	996.00	0.29
12.99	994.06	2.23
13.24	994.00	2.29
14.48	993.84	2.45
28.84	992.00	4.29
29.66	991.71	4.58
36.75	990.00	6.29
39.18	989.92	6.37
39.90	989.89	6.40
42.20	989.97	6.32
43.14	990.00	6.29
43.91	990.40	5.89
46.34	992.00	4.29
53.90	993.41	2.88
56.74	994.00	2.29
62.76	994.22	2.07
76.31	994.65	1.64
100.00	996.29	0.00

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85.9

88.6

96.2

121.1

225.0

252.4

4.11

4.17

4.33

4.76

6.11

6.40

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89,447

92,223

100,189

126,018

234,197

262,697

Depth (feet)	End Area (sq-ft)	Perim. (feet)	Storage (cubic-feet)	Discharge (cfs)
0.00	0.0	0.0	0	0.00
0.03	0.0	1.6	25	0.01
0.08	0.2	4.5	184	0.09
0.11	0.3	6.4	355	0.22
0.51	3.4	9.0	3,521	8.20
1.82	19.8	16.9	20,607	102.00
2.11	24.7	18.3	25,700	139.72
3.52	62.5	37.1	65,011	410.25
3.95	79.1	42.6	82,369	555.08

44.7

46.6

51.5

66.4

90.3

101.5

Reach 6R: Reach 3

617.31

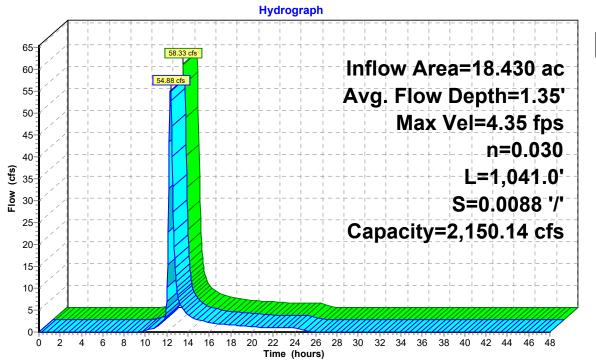
631.78

678.51

838.92

1,919.54

2,150.14





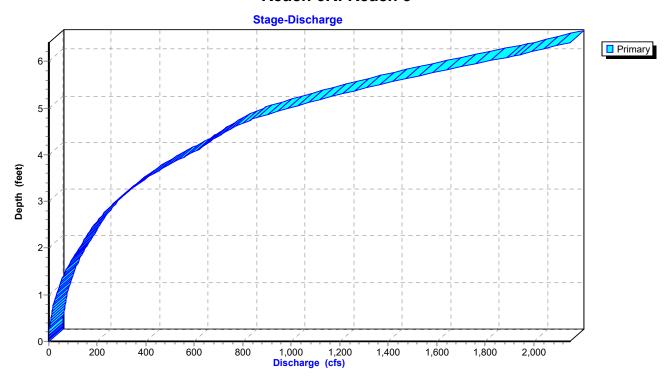
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21-130-HYDRO-PRO orifice plate option

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Reach 6R: Reach 3



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Summary for Reach 7R: Reach 2

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 2.88" for 10-Year event

Inflow = 61.28 cfs @ 12.14 hrs, Volume= 4.417 af

Outflow = 58.33 cfs @ 12.24 hrs, Volume= 4.417 af, Atten= 5%, Lag= 5.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.26 fps, Min. Travel Time= 3.3 min Avg. Velocity = 1.48 fps, Avg. Travel Time= 9.6 min

Peak Storage= 11,715 cf @ 12.18 hrs Average Depth at Peak Storage= 0.87'

Bank-Full Depth= 3.24' Flow Area= 204.3 sf, Capacity= 1,999.47 cfs

Custom cross-section, Length= 853.0' Slope= 0.0225 '/' (106 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 1,009.12', Outlet Invert= 989.90'

‡

Offset (feet)	Elevation (feet)	Chan.Depth (feet)
0.00	1,009.14	0.00
22.15	1,008.00	1.14
24.54	1,007.80	1.34
25.18	1,007.74	1.40
49.26	1,006.00	3.14
49.59	1,005.90	3.24
50.51	1,005.99	3.15
52.13	1,005.99	3.15
53.24	1,006.00	3.14
91.41	1,007.74	1.40
98.60	1,008.00	1.14
108.78	1,008.27	0.87
117.07	1,008.47	0.67
117.16	1,008.47	0.67
134.91	1,008.89	0.25
135.00	1,009.14	0.00

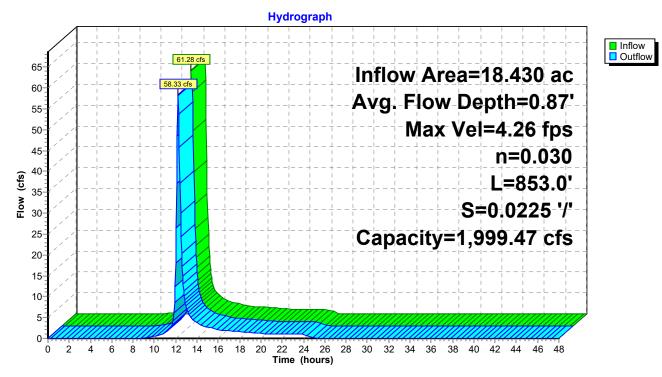
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Pag	е	3	7

Depth (feet)	End Area (sq-ft)	Perim. (feet)	Storage (cubic-feet)	Discharge (cfs)
0.00	0.0	0.0	0	0.00
0.09	0.1	2.9	47	0.03
0.10	0.1	4.0	77	0.06
1.84	61.2	66.4	52,179	430.84
1.90	65.2	68.7	55,628	468.55
2.10	79.7	76.6	67,995	608.70
2.37	102.4	92.0	87,382	818.03
2.57	122.0	104.3	104,091	1,007.45
2.99	171.2	130.2	146,042	1,527.76
3.24	204.3	135.4	174,303	1,999.47

Reach 7R: Reach 2

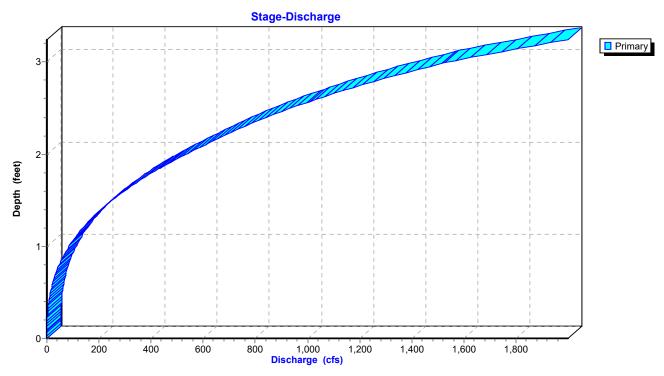


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Reach 7R: Reach 2



21-130-HYDRO-PRO orifice plate option

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Summary for Reach 8R: REACH 1 (through Pipe System)

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 2.88" for 10-Year event

Inflow = 62.92 cfs @ 12.10 hrs, Volume= 4.417 af

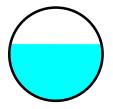
Outflow = 61.28 cfs @ 12.14 hrs, Volume= 4.417 af, Atten= 3%, Lag= 2.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

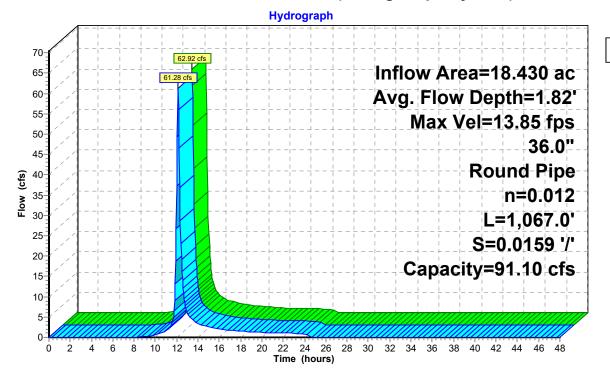
Max. Velocity= 13.85 fps, Min. Travel Time= 1.3 min Avg. Velocity = 4.64 fps, Avg. Travel Time= 3.8 min

Peak Storage= 4,795 cf @ 12.12 hrs Average Depth at Peak Storage= 1.82' Bank-Full Depth= 3.00' Flow Area= 7.1 sf, Capacity= 91.10 cfs

36.0" Round Pipe n= 0.012 Concrete pipe, finished Length= 1,067.0' Slope= 0.0159 '/' Inlet Invert= 1,021.96', Outlet Invert= 1,005.00'



Reach 8R: REACH 1 (through Pipe System)



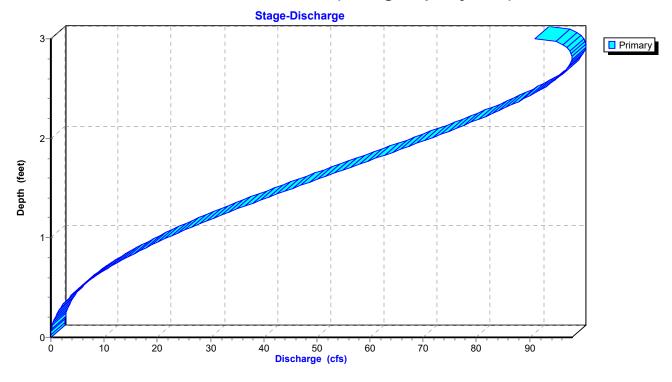


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Reach 8R: REACH 1 (through Pipe System)



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Summary for Pond 4P: EDDB #1 Cornerstone

Inflow Area = 31.710 ac, 26.16% Impervious, Inflow Depth = 4.38" for 10-Year event

Inflow = 173.27 cfs @ 12.05 hrs, Volume= 11.581 af

Outflow = 60.26 cfs @ 12.27 hrs, Volume= 10.616 af, Atten= 65%, Lag= 13.1 min

Primary = 60.26 cfs @ 12.27 hrs, Volume= 10.616 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Peak Elev= 1,004.79' @ 12.27 hrs Surf.Area= 52,106 sf Storage= 249,634 cf

Plug-Flow detention time= 282.2 min calculated for 10.616 af (92% of inflow)

Center-of-Mass det. time= 237.4 min (1,022.3 - 784.9)

Volume	Inver	t Avail.Sto	rage Stora	ge Description			
#1	995.67	374,50	66 cf Custo	om Stage Data (Pr	ismatic)Listed below (Recalc)		
Elevation	9	Surf.Area	Inc.Store	Cum.Store			
(feet)	3	(sq-ft)	(cubic-feet)	(cubic-feet)			
995.67		0	0	0			
996.00		700	116	116			
997.00		7,200	3,950	4,066			
998.00		12,000	9,600	13,666			
999.00		17,900	14,950	28,616			
1,000.00		27,000	22,450	51,066			
1,001.00		36,000	31,500	82,566			
1,002.00		40,000	38,000	120,566			
1,003.00		44,800	42,400	162,966			
1,004.00		48,700	46,750	209,716			
1,005.00		53,000	50,850	260,566			
1,006.00		57,000	55,000	315,566			
1,007.00		61,000	59,000	374,566			
Device R	outing	Invert	Outlet Devi	ces			
	rimary	993.54'		nd 30" Culvert			
<i>TT</i> 1	illiary	330.04		RCP, sq.cut end pro	jecting Ke= 0.500		
					993.11' S= 0.0049 '/' Cc= 0.900		
#2 D	evice 1	994.16'	n= 0.012 Concrete pipe, finished, Flow Area= 4.91 sf 2.9" Vert. WQ Orifice C= 0.600				
	evice 2	994.36'		d 8" PVC Pipe	500		
#0 D	CVICC Z	334.00		•	niecting Ke= 0.200		
			L= 20.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 994.36' / 994.16' S= 0.0100 '/' Cc= 0.900				
				Flow Area= 0.35 sf	754.10 0-0.01007 00-0.000		
#4 D	evice 3	995.67'		Riser Pipe X 8.00 c	columns		
,,, - D	01100 0	000.01		ith 4.0" cc spacing			
#5 D	evice 1	1,001.05'			ng in Box C= 0.600		
	evice 1	1,004.00'			p of Box C= 0.600		
,, 0	5 ¥ 100 1	1,004.00		veir flow at low hea			

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Primary OutFlow Max=59.41 cfs @ 12.27 hrs HW=1,004.78' (Free Discharge)

1=30" Culvert (Passes 59.41 cfs of 74.71 cfs potential flow)

2=WQ Orifice (Orifice Controls 0.72 cfs @ 15.60 fps)

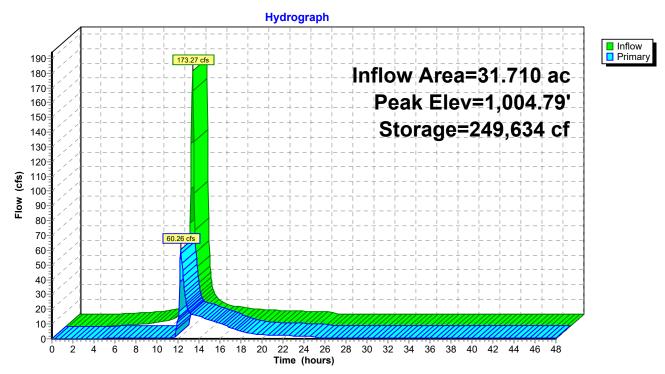
3=8" PVC Pipe (Passes 0.72 cfs of 6.07 cfs potential flow)

4=Riser Pipe (Passes 0.72 cfs of 5.25 cfs potential flow)

5=Opening in Box (Orifice Controls 17.97 cfs @ 8.98 fps)

6=Open Top of Box (Weir Controls 40.73 cfs @ 2.89 fps)

Pond 4P: EDDB #1 Cornerstone

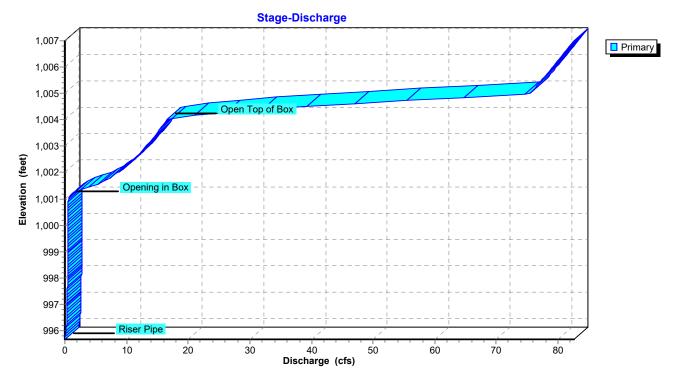


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Pond 4P: EDDB #1 Cornerstone



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Summary for Pond 5P: EDDB #2 Manor

Inflow Area = 29.690 ac, 38.00% Impervious, Inflow Depth = 3.85" for 10-Year event

Inflow = 148.68 cfs @ 12.05 hrs, Volume= 9.527 af

Outflow = 58.14 cfs @ 12.25 hrs, Volume= 8.604 af, Atten= 61%, Lag= 12.0 min

Primary = 58.14 cfs @ 12.25 hrs, Volume= 8.604 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Peak Elev= 996.06' @ 12.25 hrs Surf.Area= 49,900 sf Storage= 189,935 cf

Plug-Flow detention time= 261.1 min calculated for 8.595 af (90% of inflow)

Center-of-Mass det. time= 212.7 min (1,015.9 - 803.2)

995.25'

#6

Device 1

			·	·	
Volume	Inve	ert Avail.Sto	rage Storage	Description	
#1	988.1	6' 352,23	36 cf Custom	Stage Data (Pi	rismatic)Listed below (Recalc)
Elevation		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
988.	16	0	0	0	
989.0		800	336	336	
990.0	00	4,200	2,500	2,836	
991.0		10,400	7,300	10,136	
992.0		21,600	16,000	26,136	
993.0		37,600	29,600	55,736	
994.0		41,700	39,650	95,386	
995.0		46,000	43,850	139,236	
996.0		49,700	47,850	187,086	
997.0		53,200	51,450	238,536	
998.0		56,900	55,050	293,586	
999.0	00	60,400	58,650	352,236	
Device	Routing	Invert	Outlet Device	s	
#1	Primary	986.00'	30.0" Round		
		000.00			ojecting, Ke= 0.500
					985.62' S= 0.0049 '/' Cc= 0.900
					hed, Flow Area= 4.91 sf
#2	Device 1	986.62'		Q Orifice $C = 0$.	
#3	Device 2	986.86'	8.0" Round	8" PVC Pipe	
			L= 20.0' RCI	P, sq.cut end pro	ojecting, Ke= 0.500
					986.62' S= 0.0120 '/' Cc= 0.900
			n= 0.012 Cor	rugated PP, sm	ooth interior, Flow Area= 0.35 sf
#4	Device 3	988.16'		ser Pipe X 8.00	
			X 8 rows with	4.0" cc spacing	C= 0.600
#5	Device 1	993.40'			ing in Box C= 0.600

60.0" x **48.0"** Horiz. Open Top of Box C= 0.600

Limited to weir flow at low heads

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Primary OutFlow Max=58.03 cfs @ 12.25 hrs HW=996.06' (Free Discharge)

1=30" Culvert (Passes 58.03 cfs of 70.14 cfs potential flow)

2=WQ Orifice (Orifice Controls 0.50 cfs @ 14.71 fps)

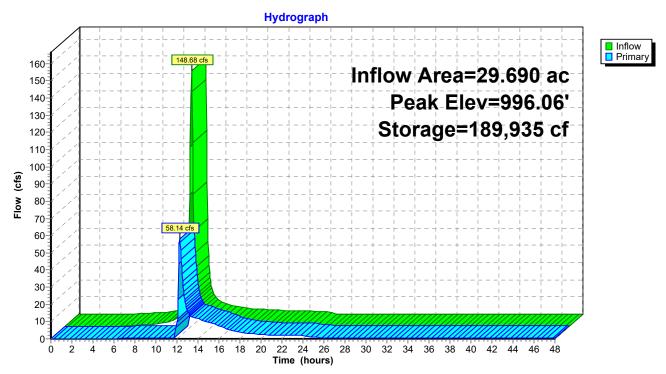
3=8" PVC Pipe (Passes 0.50 cfs of 5.00 cfs potential flow)

4=Riser Pipe (Passes 0.50 cfs of 4.34 cfs potential flow)

5=Opening in Box (Orifice Controls 14.93 cfs @ 7.47 fps)

6=Open Top of Box (Weir Controls 42.60 cfs @ 2.94 fps)

Pond 5P: EDDB #2 Manor

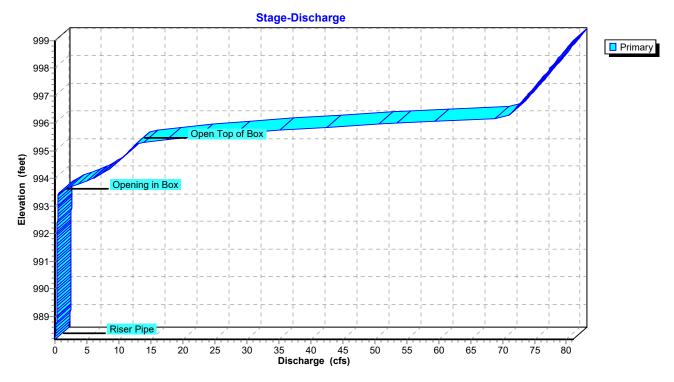


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Pond 5P: EDDB #2 Manor



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Summary for Link 2L: RP-1

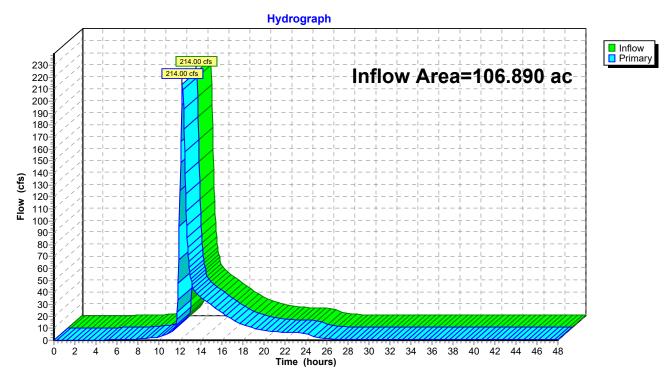
Inflow Area = 106.890 ac, 20.71% Impervious, Inflow Depth > 3.49" for 10-Year event

Inflow = 214.00 cfs @ 12.23 hrs, Volume= 31.088 af

Primary = 214.00 cfs @ 12.23 hrs, Volume= 31.088 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 2L: RP-1



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Summary for Link 3L: RP-2

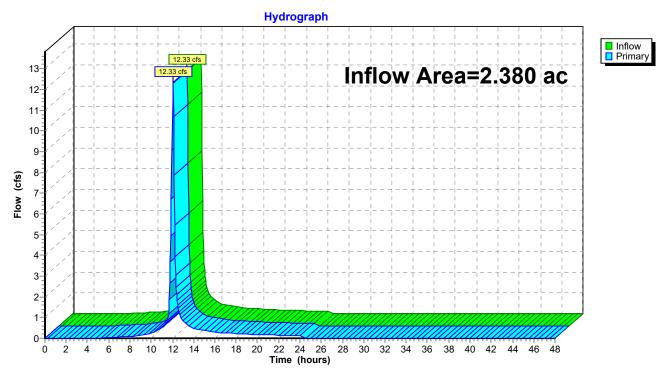
Inflow Area = 2.380 ac, 38.00% Impervious, Inflow Depth = 3.85" for 10-Year event

Inflow 0.764 af

12.33 cfs @ 12.04 hrs, Volume= 12.33 cfs @ 12.04 hrs, Volume= 0.764 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 3L: RP-2



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Summary for Link 4L: RP-3

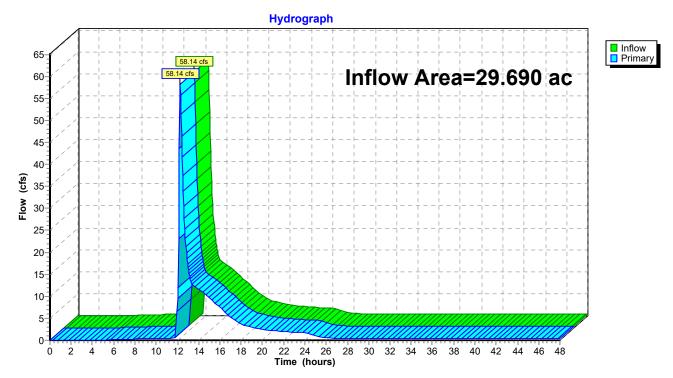
Inflow Area = 29.690 ac, 38.00% Impervious, Inflow Depth > 3.48" for 10-Year event

Inflow = 58.14 cfs @ 12.25 hrs, Volume= 8.604 af

Primary = 58.14 cfs @ 12.25 hrs, Volume= 8.604 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 4L: RP-3



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Summary for Link 5L: RP-4

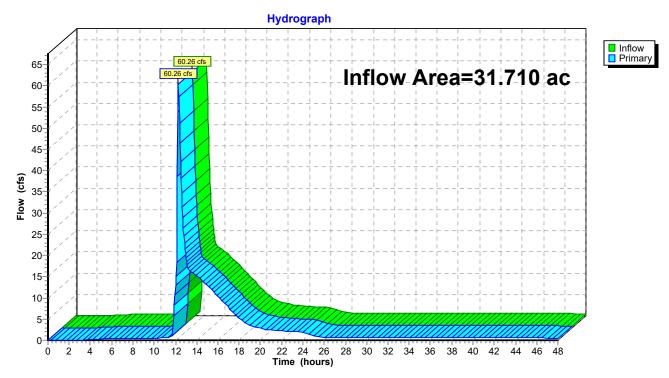
Inflow Area = 31.710 ac, 26.16% Impervious, Inflow Depth > 4.02" for 10-Year event

Inflow = 60.26 cfs @ 12.27 hrs, Volume= 10.616 af

Primary = 60.26 cfs @ 12.27 hrs, Volume= 10.616 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 5L: RP-4



Bailey Farms - Developed Type II 24-hr 100-Year Rainfall=7.70" Printed 4/18/2022

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: ON-SITE#1Runoff Area=31.710 ac 26.16% Impervious Runoff Depth=6.75"

Flow Length=1,186' Slope=0.0250 '/' Tc=14.0 min CN=92 Runoff=260.19 cfs 17.828 af

Subcatchment3S: ON-SITE#3 (Bypass) Runoff Area=2.380 ac 38.00% Impervious Runoff Depth=6.16" Flow Length=1,110' Tc=12.7 min CN=87 Runoff=19.23 cfs 1.221 af

Subcatchment5S: OFF-SITE#1Runoff Area=18.430 ac 0.00% Impervious Runoff Depth=5.00"

Flow Length=1,247' Tc=17.6 min CN=77 Runoff=108.57 cfs 7.679 af

Subcatchment6S: ON-SITE#4 (Bypass) Runoff Area=24.680 ac 6.72% Impervious Runoff Depth=5.46" Flow Length=1,186' Slope=0.0250 '/' Tc=15.5 min CN=81 Runoff=166.84 cfs 11.229 af

Subcatchment7S: ON-SITE#2Runoff Area=29.690 ac 38.00% Impervious Runoff Depth=6.16"

Flow Length=1,769' Slope=0.0250 '/' Tc=13.8 min CN=87 Runoff=232.03 cfs 15.236 af

Reach 6R: Reach 3Avg. Flow Depth=1.72' Max Vel=4.99 fps Inflow=91.47 cfs 7.679 af n=0.030 L=1,041.0' S=0.0088 '/' Capacity=2,150.14 cfs Outflow=90.50 cfs 7.679 af

Reach 7R: Reach 2Avg. Flow Depth=1.03' Max Vel=4.78 fps Inflow=92.93 cfs 7.679 af n=0.030 L=853.0' S=0.0225'/' Capacity=1,999.47 cfs Outflow=91.47 cfs 7.679 af

Reach 8R: REACH 1 (through Pipe Avg. Flow Depth=3.00' Max Vel=14.68 fps Inflow=108.57 cfs 7.679 af 36.0" Round Pipe n=0.012 L=1,067.0' S=0.0159 '/' Capacity=91.10 cfs Outflow=92.93 cfs 7.679 af

Pond 4P: EDDB #1 Cornerstone Peak Elev=1,006.64' Storage=352,702 cf Inflow=260.19 cfs 17.828 af Outflow=81.35 cfs 16.846 af

Pond 5P: EDDB #2 Manor Peak Elev=997.86' Storage=285,540 cf Inflow=232.03 cfs 15.236 af Outflow=76.98 cfs 14.297 af

Link 2L: RP-1 Inflow=369.46 cfs 51.273 af

Primary=369.46 cfs 51.273 af

Link 3L: RP-2 Inflow=19.23 cfs 1.221 af
Primary=19.23 cfs 1.221 af

Link 4L: RP-3 Inflow=76.98 cfs 14.297 af Primary=76.98 cfs 14.297 af

Link 5L: RP-4Inflow=81.35 cfs 16.846 af
Primary=81.35 cfs 16.846 af

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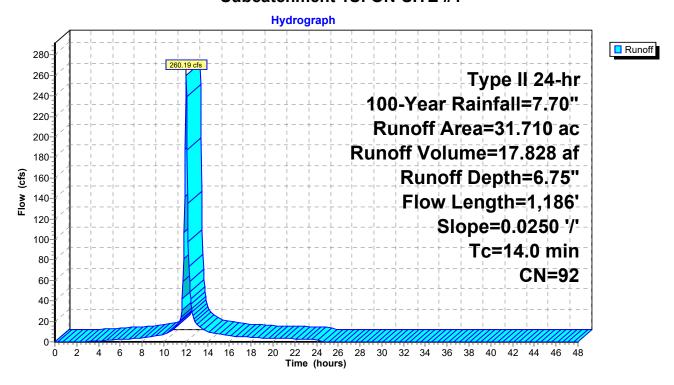
Summary for Subcatchment 1S: ON-SITE #1

Runoff = 260.19 cfs @ 12.05 hrs, Volume= 17.828 af, Depth= 6.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 100-Year Rainfall=7.70"

	Area	(ac)	CN	Desc	cription				
*	19.	330	92	Dupl	ex/Townh	omes			
	11.140 92 1/8 acre lots, 65% imp, HSG D								
	1.240 95 Urban commercial, 85% imp, HSG D								
31.710 92 Weighted Average									
	23.	415		73.8	4% Pervio	us Area			
	8.295 26.16% Impervious Area								
	Tc	Lengt	:h	Slope	Velocity	Capacity	Description		
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)			
	8.4	10	0	0.0250	0.20		Sheet Flow,		
							Grass: Short n= 0.150 P2= 3.60"		
	5.6	1,08	6	0.0250	3.21		Shallow Concentrated Flow,		
							Paved Kv= 20.3 fps		
	14.0	1,18	6	Total					

Subcatchment 1S: ON-SITE #1



21-130-HYDRO-PRO orifice plate option

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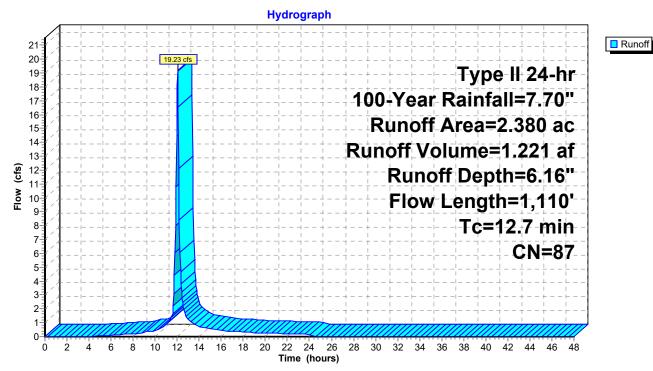
Summary for Subcatchment 3S: ON-SITE #3 (Bypass)

Runoff = 19.23 cfs @ 12.04 hrs, Volume= 1.221 af, Depth= 6.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 100-Year Rainfall=7.70"

	Area	(ac) C	N Desc	cription					
2.380 87 1/4 acre lots, 38% imp, HSG D									
1.476 62.00% Pervious Area									
	0.	904	38.0	0% Imperv	ious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	4.9	50	0.0250	0.17		Sheet Flow,			
	7.8	1,060	0.0200	2.28		n= 0.150 P2= 3.60" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps			
	12 7	1 110	Total						

Subcatchment 3S: ON-SITE #3 (Bypass)



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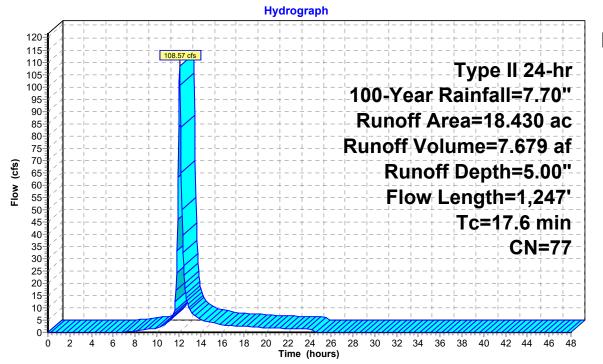
Summary for Subcatchment 5S: OFF-SITE #1

Runoff = 108.57 cfs @ 12.10 hrs, Volume= 7.679 af, Depth= 5.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 100-Year Rainfall=7.70"

	Area	(ac) C	N Des	cription			_	
15.640 76 Woods/grass comb., Fair, HSG C								
	2.	790 8	32 Woo	ds/grass c	omb., Fair,	HSG D		
	18.430 77 Weighted Average							
	18.	430	100.	00% Pervi	ous Area			
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		_	
	8.4	100	0.0250	0.20		Sheet Flow,		
						Grass: Short n= 0.150 P2= 3.60"		
	9.2	1,147	0.0192	2.08		Shallow Concentrated Flow,		
_						Grassed Waterway Kv= 15.0 fps	_	
	17.6	1,247	Total					

Subcatchment 5S: OFF-SITE #1





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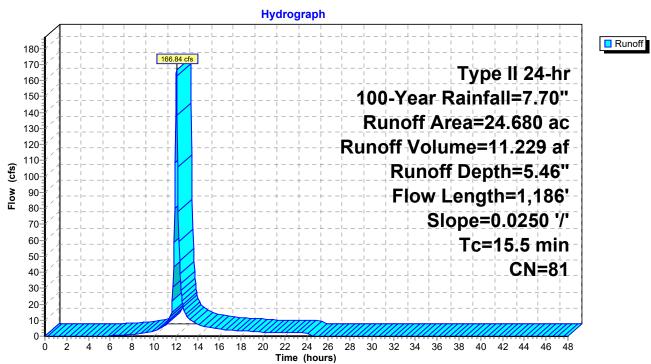
Summary for Subcatchment 6S: ON-SITE #4 (Bypass)

Runoff = 166.84 cfs @ 12.07 hrs, Volume= 11.229 af, Depth= 5.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 100-Year Rainfall=7.70"

	Area	(ac)	CN	Desc	cription			
22.730 80 >75% Grass cover, Good, HSG D								
_	1.950 95 Urban commercial, 85% imp, HSG D							
	24.680 81 Weighted Average							
	23.	023		93.2	8% Pervio	us Area		
	1.	657		6.72°	% Impervi	ous Area		
	Tc (min)	Lengt (feet		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	8.4	10	0.	0250	0.20		Sheet Flow,	
_	7.1	1,08	6 O.	0250	2.55		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
	15.5	1,18	3 To	otal				

Subcatchment 6S: ON-SITE #4 (Bypass)



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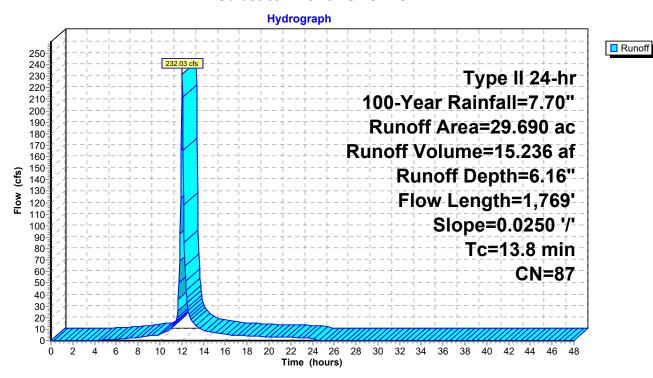
Summary for Subcatchment 7S: ON-SITE #2

Runoff = 232.03 cfs @ 12.05 hrs, Volume= 15.236 af, Depth= 6.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 100-Year Rainfall=7.70"

	Area	(ac) C	N Desc	cription					
29.690 87 1/4 acre lots, 38% imp, HSG D									
18.408 62.00% Pervious Area									
	11.	282	38.0	0% Imperv	ious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	4.9	50	0.0250	0.17		Sheet Flow,			
	8.9	1,719	0.0250	3.21		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow, Paved Kv= 20.3 fps			
_	13.8	1 769	Total	•			•		

Subcatchment 7S: ON-SITE #2



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Summary for Reach 6R: Reach 3

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 5.00" for 100-Year event

Inflow = 91.47 cfs @ 12.30 hrs, Volume= 7.679 af

Outflow = 90.50 cfs @ 12.36 hrs, Volume= 7.679 af, Atten= 1%, Lag= 3.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.99 fps, Min. Travel Time= 3.5 min Avg. Velocity = 1.24 fps, Avg. Travel Time= 14.0 min

Peak Storage= 18,925 cf @ 12.30 hrs Average Depth at Peak Storage= 1.72'

Bank-Full Depth= 6.40' Flow Area= 252.4 sf, Capacity= 2,150.14 cfs

Custom cross-section, Length= 1,041.0' Slope= 0.0088 '/' (110 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 989.92', Outlet Invert= 980.77'

‡	

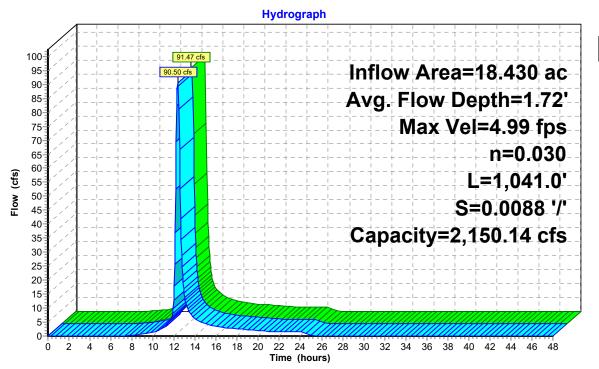
Offset	Elevation	Chan.Depth
(feet)	(feet)	(feet)
0.00	996.29	0.00
7.00	996.00	0.29
12.99	994.06	2.23
13.24	994.00	2.29
14.48	993.84	2.45
28.84	992.00	4.29
29.66	991.71	4.58
36.75	990.00	6.29
39.18	989.92	6.37
39.90	989.89	6.40
42.20	989.97	6.32
43.14	990.00	6.29
43.91	990.40	5.89
46.34	992.00	4.29
53.90	993.41	2.88
56.74	994.00	2.29
62.76	994.22	2.07
76.31	994.65	1.64
100.00	996.29	0.00

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	Depth	End Area	Perim.	Storage	Discharge
_	(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
	0.00	0.0	0.0	0	0.00
	0.03	0.0	1.6	25	0.01
	0.08	0.2	4.5	184	0.09
	0.11	0.3	6.4	355	0.22
	0.51	3.4	9.0	3,521	8.20
	1.82	19.8	16.9	20,607	102.00
	2.11	24.7	18.3	25,700	139.72
	3.52	62.5	37.1	65,011	410.25
	3.95	79.1	42.6	82,369	555.08
	4.11	85.9	44.7	89,447	617.31
	4.17	88.6	46.6	92,223	631.78
	4.33	96.2	51.5	100,189	678.51
	4.76	121.1	66.4	126,018	838.92
	6.11	225.0	90.3	234,197	1,919.54
	6.40	252.4	101.5	262.697	2.150.14

Reach 6R: Reach 3





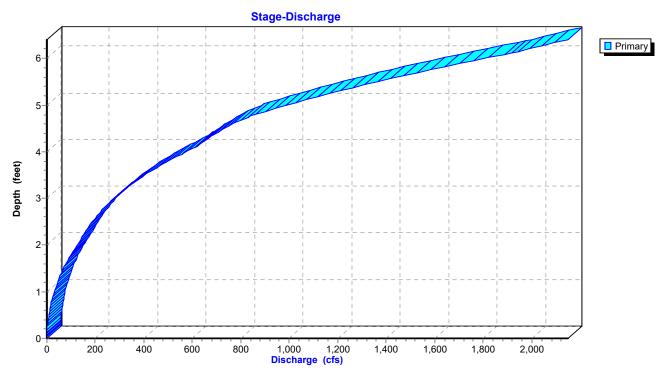
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Reach 6R: Reach 3



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Summary for Reach 7R: Reach 2

18.430 ac, 0.00% Impervious, Inflow Depth = 5.00" for 100-Year event Inflow Area =

92.93 cfs @ 12.11 hrs, Volume= Inflow 7.679 af

7.679 af, Atten= 2%, Lag= 11.2 min Outflow 91.47 cfs @ 12.30 hrs, Volume=

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.78 fps, Min. Travel Time= 3.0 min Avg. Velocity = 1.66 fps, Avg. Travel Time= 8.6 min

Peak Storage= 16,345 cf @ 12.25 hrs Average Depth at Peak Storage= 1.03'

Bank-Full Depth= 3.24' Flow Area= 204.3 sf, Capacity= 1,999.47 cfs

Custom cross-section, Length= 853.0' Slope= 0.0225 '/' (106 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 1,009.12', Outlet Invert= 989.90'

‡

Offset (feet)	Elevation (feet)	Chan.Depth (feet)
0.00	1,009.14	0.00
22.15	1,008.00	1.14
24.54	1,007.80	1.34
25.18	1,007.74	1.40
49.26	1,006.00	3.14
49.59	1,005.90	3.24
50.51	1,005.99	3.15
52.13	1,005.99	3.15
53.24	1,006.00	3.14
91.41	1,007.74	1.40
98.60	1,008.00	1.14
108.78	1,008.27	0.87
117.07	1,008.47	0.67
117.16	1,008.47	0.67
134.91	1,008.89	0.25
135.00	1,009.14	0.00

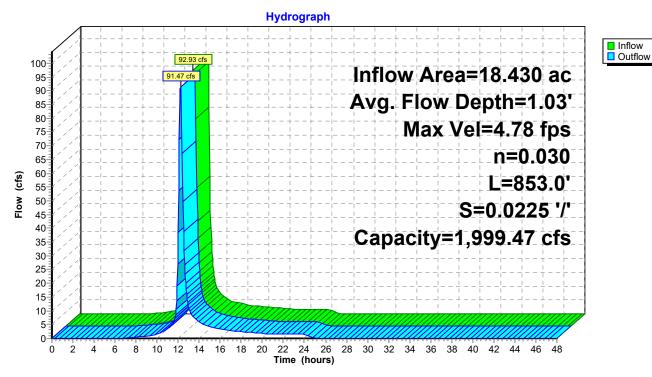
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Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
0.09	0.1	2.9	47	0.03
0.10	0.1	4.0	77	0.06
1.84	61.2	66.4	52,179	430.84
1.90	65.2	68.7	55,628	468.55
2.10	79.7	76.6	67,995	608.70
2.37	102.4	92.0	87,382	818.03
2.57	122.0	104.3	104,091	1,007.45
2.99	171.2	130.2	146,042	1,527.76
3.24	204.3	135.4	174,303	1,999.47

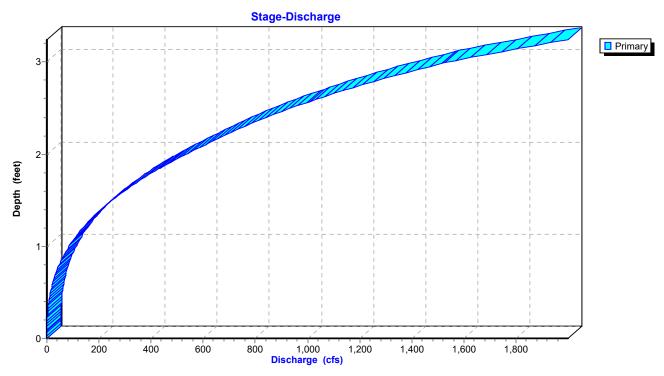
Reach 7R: Reach 2



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Reach 7R: Reach 2



21-130-HYDRO-PRO orifice plate option

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Summary for Reach 8R: REACH 1 (through Pipe System)

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 5.00" for 100-Year event

Inflow = 108.57 cfs @ 12.10 hrs, Volume= 7.679 af

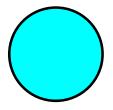
Outflow = 92.93 cfs @ 12.11 hrs, Volume= 7.679 af, Atten= 14%, Lag= 0.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

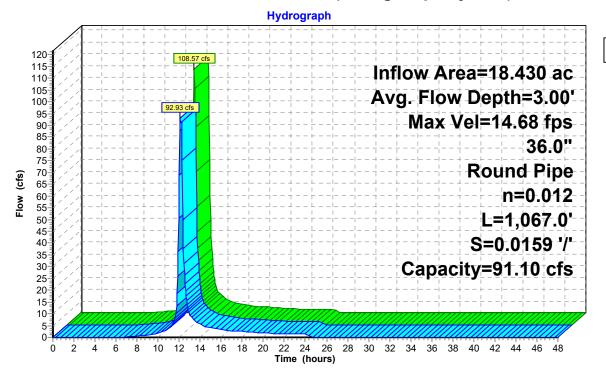
Max. Velocity= 14.68 fps, Min. Travel Time= 1.2 min Avg. Velocity = 5.23 fps, Avg. Travel Time= 3.4 min

Peak Storage= 7,542 cf @ 12.10 hrs Average Depth at Peak Storage= 3.00' Bank-Full Depth= 3.00' Flow Area= 7.1 sf, Capacity= 91.10 cfs

36.0" Round Pipe n= 0.012 Concrete pipe, finished Length= 1,067.0' Slope= 0.0159 '/' Inlet Invert= 1,021.96', Outlet Invert= 1,005.00'



Reach 8R: REACH 1 (through Pipe System)



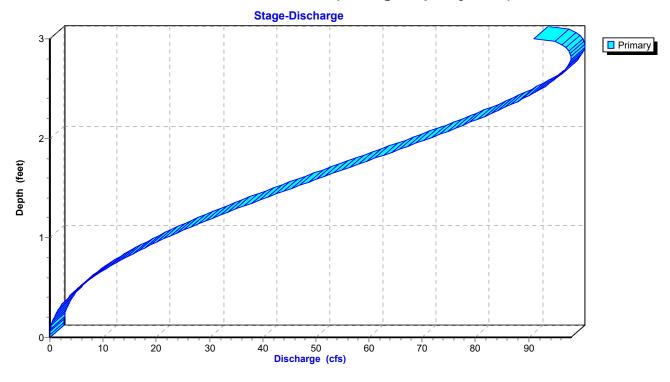


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Reach 8R: REACH 1 (through Pipe System)



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Summary for Pond 4P: EDDB #1 Cornerstone

31.710 ac, 26.16% Impervious, Inflow Depth = 6.75" for 100-Year event Inflow Area =

Inflow 17.828 af

260.19 cfs @ 12.05 hrs, Volume= 81.35 cfs @ 12.29 hrs, Volume= Outflow 16.846 af, Atten= 69%, Lag= 14.1 min

Primary 81.35 cfs @ 12.29 hrs, Volume= 16.846 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Peak Elev= 1,006.64' @ 12.29 hrs Surf.Area= 59,549 sf Storage= 352,702 cf

Plug-Flow detention time= 212.5 min calculated for 16.846 af (94% of inflow)

Center-of-Mass det. time= 180.3 min (954.2 - 773.9)

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	995.67	7' 374,56	66 cf Custom	Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation S		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
995.6	67	0	0	0	
996.0	00	700	116	116	
997.0		7,200	3,950	4,066	
998.0		12,000	9,600	13,666	
999.0	-	17,900	14,950	28,616	
1,000.0		27,000	22,450	51,066	
1,001.0		36,000	31,500	82,566	
1,002.0		40,000	38,000	120,566	
1,003.0		44,800	42,400	162,966	
1,004.0		48,700	46,750	209,716	
1,005.0		53,000	50,850	260,566 245,566	
1,006.0 1,007.0		57,000 61,000	55,000 59,000	315,566 374,566	
1,007.0	,,,	01,000	59,000	374,300	
Device	Routing	Invert	Outlet Device	S	
#1	Primary	993.54'	30.0" Round	30" Culvert	
	_		L= 87.0' RCI	P, sq.cut end pro	jecting, Ke= 0.500
					993.11' S= 0.0049 '/' Cc= 0.900
					ned, Flow Area= 4.91 sf
#2	Device 1	994.16'		Q Orifice C= 0.6	600
#3	Device 2	994.36'	8.0" Round		
					ojecting, Ke= 0.200
					994.16' S= 0.0100 '/' Cc= 0.900
ш.	Davida a 0	005.071	,	w Area= 0.35 sf	
#4	Device 3	995.67'		ser Pipe X 8.00 c	
#5	Device 1	1,001.05'		4.0" cc spacing	C= 0.600 ng in Box C= 0.600
#3 #6	Device 1	1,001.05			p of Box C= 0.600
#0	DEVICE I	1,004.00		ir flow at low hea	•
			Littlica to Wel	ii iiow at iow iica	uo

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```
Primary OutFlow Max=81.33 cfs @ 12.29 hrs HW=1,006.63' (Free Discharge)

1=30" Culvert (Inlet Controls 81.33 cfs @ 16.57 fps)

2=WQ Orifice (Passes < 0.78 cfs potential flow)

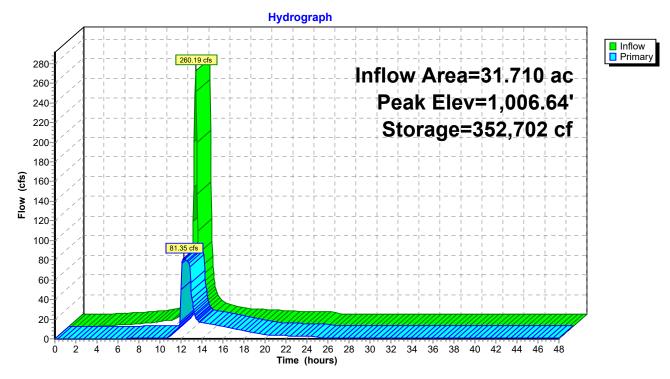
3=8" PVC Pipe (Passes < 6.61 cfs potential flow)

4=Riser Pipe (Passes < 5.85 cfs potential flow)

5=Opening in Box (Passes < 22.23 cfs potential flow)

6=Open Top of Box (Passes < 156.21 cfs potential flow)
```

Pond 4P: EDDB #1 Cornerstone

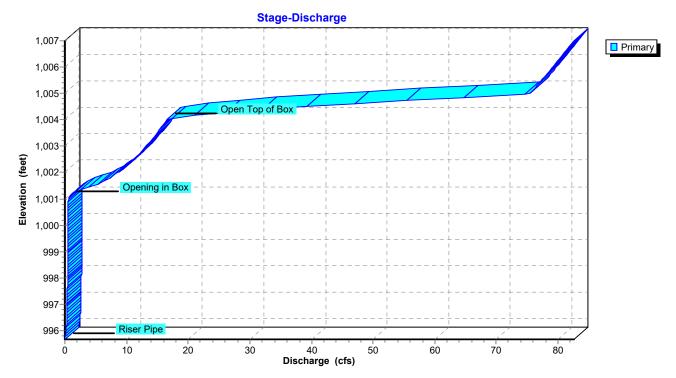


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Pond 4P: EDDB #1 Cornerstone



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#5

#6

Device 1

Device 1

993.40'

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Summary for Pond 5P: EDDB #2 Manor

Inflow Area = 29.690 ac, 38.00% Impervious, Inflow Depth = 6.16" for 100-Year event

Inflow = 232.03 cfs @ 12.05 hrs, Volume= 15.236 af

Outflow = 76.98 cfs @ 12.27 hrs, Volume= 14.297 af, Atten= 67%, Lag= 13.3 min

Primary = 76.98 cfs @ 12.27 hrs, Volume= 14.297 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Peak Elev= 997.86' @ 12.27 hrs Surf.Area= 56,374 sf Storage= 285,540 cf

Plug-Flow detention time= 188.5 min calculated for 14.283 af (94% of inflow)

Center-of-Mass det. time= 155.1 min (945.3 - 790.2)

Volume	Inve	ert Avail Sto	rage Storage I	Description	
#1	988.1				rismatic)Listed below (Recalc)
#1	900.1	0 332,20	oo or Custom	Stage Data (FI	isinatic, Listed below (Necalc)
Elevation	on	Surf.Area	Inc.Store	Cum.Store	
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)	
988.	16	0	0	0	
989.0	00	800	336	336	
990.0	00	4,200	2,500	2,836	
991.0	00	10,400	7,300	10,136	
992.0	00	21,600	16,000	26,136	
993.0	00	37,600	29,600	55,736	
994.0	00	41,700	39,650	95,386	
995.0	00	46,000	43,850	139,236	
996.0	00	49,700	47,850	187,086	
997.0	00	53,200	51,450	238,536	
998.0	00	56,900	55,050	293,586	
999.0	00	60,400	58,650	352,236	
Device	Routing	Invert	Outlet Devices	•	
#1	Primary	986.00'	30.0" Round		
#1	Pilillary	900.00			pjecting, Ke= 0.500
					985.62' S= 0.0049 '/' Cc= 0.900
					963.62
#2	Device 1	986.62'		Orifice C= 0.	
#2 #3	Device 1 Device 2		8.0" Round 8		000
#3	Device 2	900.00		•	pjecting, Ke= 0.500
				, i i	986.62' S= 0.0120 '/' Cc= 0.900
#4	Device 3	988.16'		er Pipe X 8.00	ooth interior, Flow Area= 0.35 sf
#4	Device 3	900.10		4.0" cc spacing	
			A O TOWS WILLI	+.0 CC Spacing	U- 0.000

48.0" W x 6.0" H Vert. Opening in Box C= 0.600

995.25' **60.0" x 48.0" Horiz. Open Top of Box** C= 0.600

Limited to weir flow at low heads

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Primary OutFlow Max=76.94 cfs @ 12.27 hrs HW=997.85' (Free Discharge)

1=30" Culvert (Inlet Controls 76.94 cfs @ 15.67 fps)

2=WQ Orifice (Passes < 0.55 cfs potential flow)

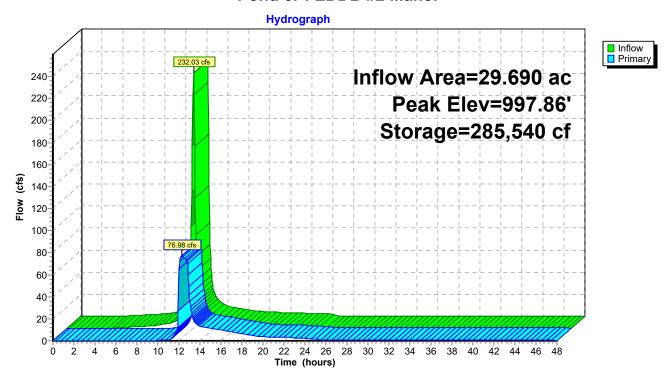
3=8" PVC Pipe (Passes < 5.49 cfs potential flow)

4=Riser Pipe (Passes < 4.89 cfs potential flow)

5=Opening in Box (Passes < 19.73 cfs potential flow)

6=Open Top of Box (Passes < 155.20 cfs potential flow)

Pond 5P: EDDB #2 Manor

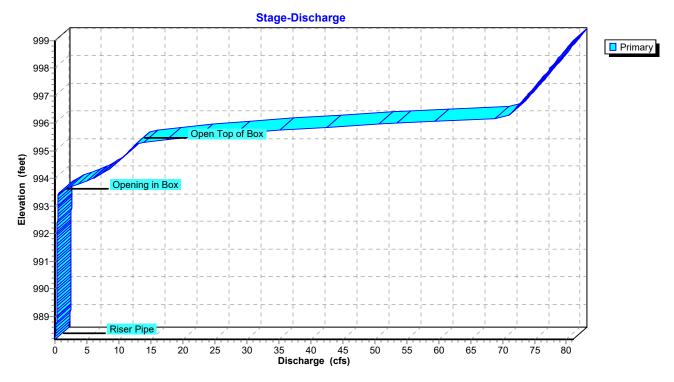


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Pond 5P: EDDB #2 Manor



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Summary for Link 2L: RP-1

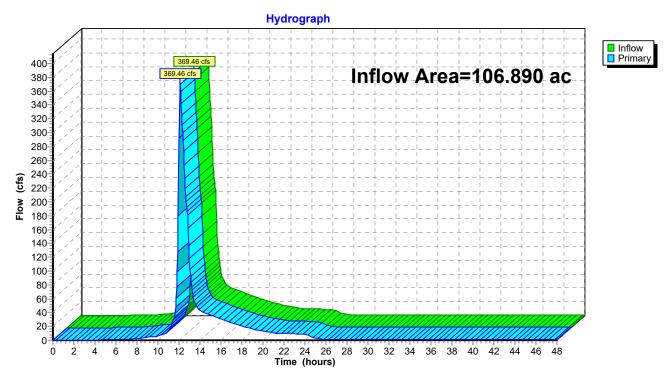
Inflow Area = 106.890 ac, 20.71% Impervious, Inflow Depth > 5.76" for 100-Year event

Inflow = 369.46 cfs @ 12.10 hrs, Volume= 51.273 af

Primary = 369.46 cfs @ 12.10 hrs, Volume= 51.273 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 2L: RP-1



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Summary for Link 3L: RP-2

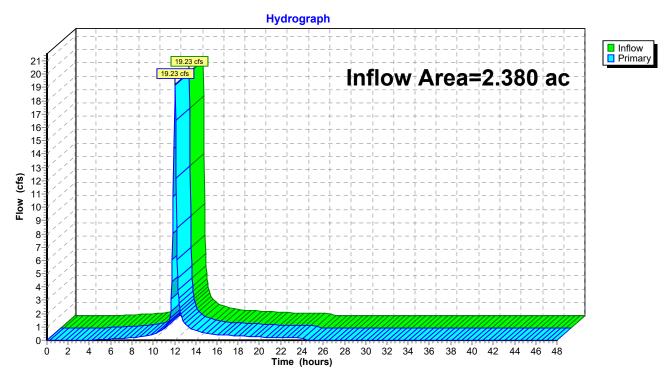
Inflow Area = 2.380 ac, 38.00% Impervious, Inflow Depth = 6.16" for 100-Year event

Inflow = 19.23 cfs @ 12.04 hrs, Volume= 1.221 af

Primary = 19.23 cfs @ 12.04 hrs, Volume= 1.221 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 3L: RP-2



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Summary for Link 4L: RP-3

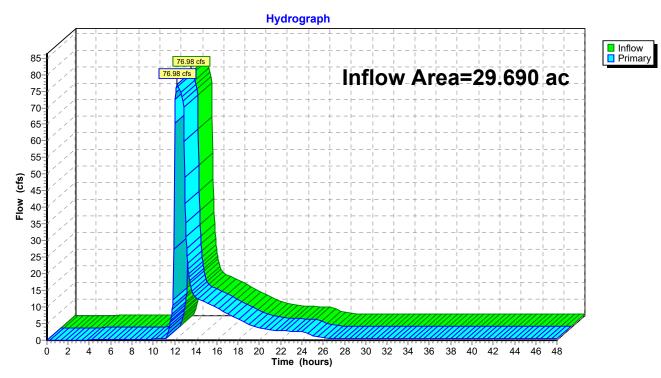
Inflow Area = 29.690 ac, 38.00% Impervious, Inflow Depth > 5.78" for 100-Year event

Inflow

76.98 cfs @ 12.27 hrs, Volume= 14.297 af 76.98 cfs @ 12.27 hrs, Volume= 14.297 af, 14.297 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 4L: RP-3



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Summary for Link 5L: RP-4

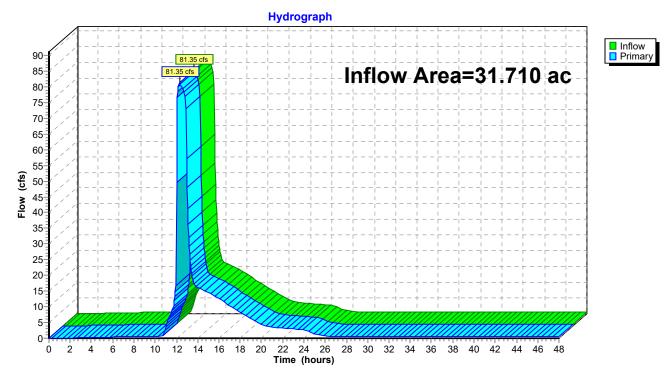
Inflow Area = 31.710 ac, 26.16% Impervious, Inflow Depth > 6.37" for 100-Year event

Inflow = 81.35 cfs @ 12.29 hrs, Volume= 16.846 af

Primary = 81.35 cfs @ 12.29 hrs, Volume= 16.846 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 5L: RP-4



Bailey Farms - Developed Type II 24-hr WQV Rainfall=1.37" Printed 4/18/2022

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: ON-SITE#1Runoff Area=31.710 ac 26.16% Impervious Runoff Depth=0.69"

Flow Length=1,186' Slope=0.0250 '/' Tc=14.0 min CN=92 Runoff=29.17 cfs 1.830 af

Subcatchment3S: ON-SITE#3 (Bypass) Runoff Area=2.380 ac 38.00% Impervious Runoff Depth=0.45" Flow Length=1,110' Tc=12.7 min CN=87 Runoff=1.43 cfs 0.089 af

Subcatchment5S: OFF-SITE#1 Runoff Area=18.430 ac 0.00% Impervious Runoff Depth=0.16" Flow Length=1,247' Tc=17.6 min CN=77 Runoff=2.03 cfs 0.244 af

Subcatchment6S: ON-SITE#4 (Bypass) Runoff Area=24.680 ac 6.72% Impervious Runoff Depth=0.25" Flow Length=1,186' Slope=0.0250 '/' Tc=15.5 min CN=81 Runoff=6.28 cfs 0.514 af

Subcatchment7S: ON-SITE#2Runoff Area=29.690 ac 38.00% Impervious Runoff Depth=0.45"

Flow Length=1,769' Slope=0.0250 '/' Tc=13.8 min CN=87 Runoff=17.13 cfs 1.107 af

Reach 6R: Reach 3Avg. Flow Depth=0.20' Max Vel=1.21 fps Inflow=1.47 cfs 0.244 af n=0.030 L=1,041.0' S=0.0088'/' Capacity=2,150.14 cfs Outflow=1.10 cfs 0.244 af

Reach 7R: Reach 2Avg. Flow Depth=0.23' Max Vel=1.65 fps Inflow=1.90 cfs 0.244 af n=0.030 L=853.0' S=0.0225'/ Capacity=1,999.47 cfs Outflow=1.47 cfs 0.244 af

Reach 8R: REACH 1 (through PipeAvg. Flow Depth=0.30' Max Vel=5.18 fps Inflow=2.03 cfs 0.244 af 36.0" Round Pipe n=0.012 L=1,067.0' S=0.0159 '/' Capacity=91.10 cfs Outflow=1.90 cfs 0.244 af

Pond 4P: EDDB #1 Cornerstone Peak Elev=1,000.17' Storage=55,660 cf Inflow=29.17 cfs 1.830 af Outflow=0.54 cfs 1.544 af

Pond 5P: EDDB #2 Manor Peak Elev=992.24' Storage=31,748 cf Inflow=17.13 cfs 1.107 af

Outflow=0.39 cfs 1.083 af

Link 2L: RP-1Inflow=8.39 cfs 3.473 af
Primary=8.39 cfs 3.473 af

Link 3L: RP-2Inflow=1.43 cfs 0.089 af
Primary=1.43 cfs 0.089 af

Link 4L: RP-3Inflow=0.39 cfs 1.083 af
Primary=0.39 cfs 1.083 af

Link 5L: RP-4 Inflow=0.54 cfs 1.544 af

Primary=0.54 cfs 1.544 af

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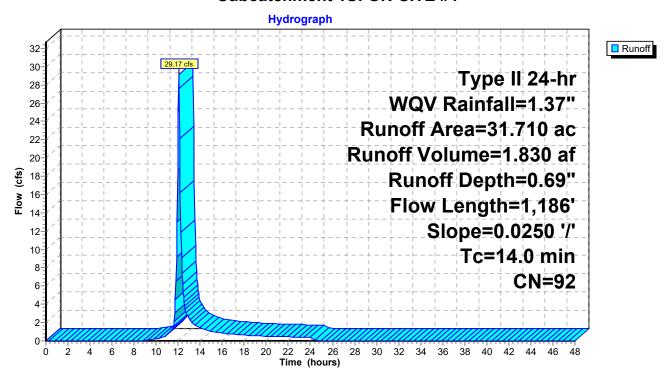
Summary for Subcatchment 1S: ON-SITE #1

Runoff = 29.17 cfs @ 12.06 hrs, Volume= 1.830 af, Depth= 0.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr WQV Rainfall=1.37"

_	Area	(ac)	CN	l Desc	cription								
*	19.	330	92	2 Dupl	ex/Townh	omes							
	11.	140	92	2 1/8 a	1/8 acre lots, 65% imp, HSG D								
1.240 95 Urban commercial, 85% imp, HSG D													
31.710 92 Weighted Average													
	23.	415		73.8	4% Pervio	us Area							
	8.	295		26.1	6% Imperv	/ious Area							
	Tc	Lengt	th	Slope	Velocity	Capacity	Description						
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)							
	8.4	10	0	0.0250	0.20		Sheet Flow,						
							Grass: Short n= 0.150 P2= 3.60"						
	5.6	1,08	6	0.0250	3.21		Shallow Concentrated Flow,						
_							Paved Kv= 20.3 fps						
	14.0	1,18	6	Total									

Subcatchment 1S: ON-SITE #1



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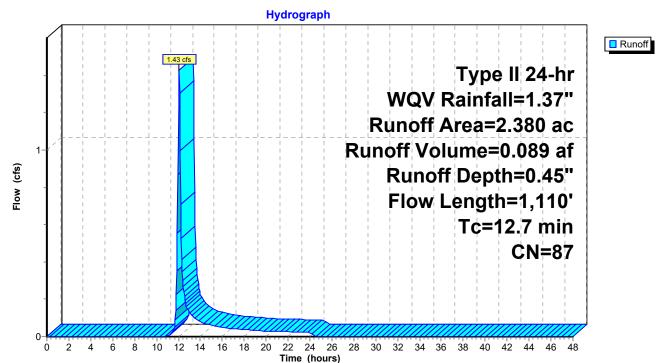
Summary for Subcatchment 3S: ON-SITE #3 (Bypass)

Runoff = 1.43 cfs @ 12.06 hrs, Volume= 0.089 af, Depth= 0.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr WQV Rainfall=1.37"

	Area	(ac) C	N Desc	cription						
2.380 87 1/4 acre lots, 38% imp, HSG D										
_	1.	476								
	0.	904	38.0	0% Imperv	ious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
Ī	4.9	50	0.0250	0.17		Sheet Flow,				
	7.8	1,060	0.0200	2.28		n= 0.150 P2= 3.60" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps				
	12 7	1 110	Total							

Subcatchment 3S: ON-SITE #3 (Bypass)



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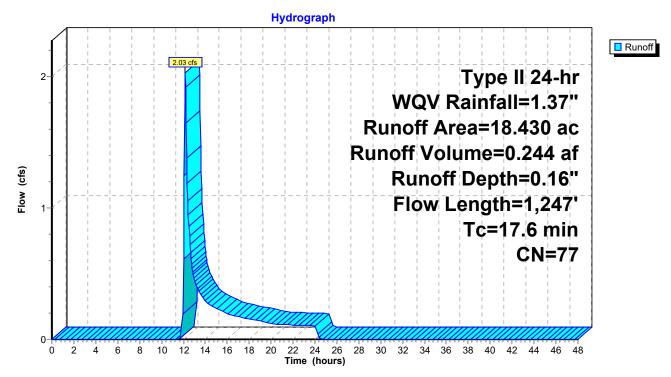
Summary for Subcatchment 5S: OFF-SITE #1

Runoff = 2.03 cfs @ 12.16 hrs, Volume= 0.244 af, Depth= 0.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr WQV Rainfall=1.37"

	Area	(ac) C	N Desc	cription						
	15.640 76 Woods/grass comb., Fair, HSG C									
	2.790 82 Woods/grass comb., Fair, HSG D									
	18.430 77 Weighted Average									
	18.	430	100.	00% Pervi	ous Area					
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	8.4	100	0.0250	0.20		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.60"				
	9.2	1,147	0.0192	2.08		Shallow Concentrated Flow,				
_						Grassed Waterway Kv= 15.0 fps				
	17.6	1 247	Total	•	•					

Subcatchment 5S: OFF-SITE #1



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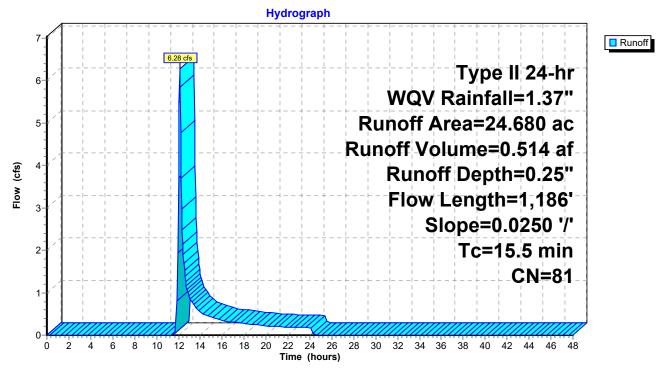
Summary for Subcatchment 6S: ON-SITE #4 (Bypass)

Runoff = 6.28 cfs @ 12.11 hrs, Volume= 0.514 af, Depth= 0.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr WQV Rainfall=1.37"

_	Area	Area (ac) CN Description									
	22.730 80 >75% Grass cover, Good, HSG D										
_	1.950 95 Urban commercial, 85% imp, HSG D										
	24.	680	81	Weig	ghted Aver	age					
	23.	023		93.2	8% Pervio	us Area					
	1.	657		6.72	% Impervi	ous Area					
	Тс	Lengtl		lope	Velocity	Capacity	Description				
_	(min)	(feet) ((ft/ft)	(ft/sec)	(cfs)					
	8.4	100	0.0)250	0.20		Sheet Flow,				
							Grass: Short n= 0.150 P2= 3.60"				
	7.1	1,086	0.0)250	2.55		Shallow Concentrated Flow,				
_							Unpaved Kv= 16.1 fps				
	15.5	1,186	3 То	tal							

Subcatchment 6S: ON-SITE #4 (Bypass)



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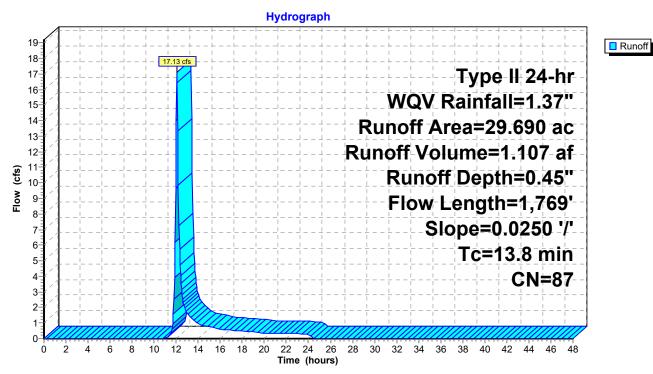
Summary for Subcatchment 7S: ON-SITE #2

Runoff = 17.13 cfs @ 12.07 hrs, Volume= 1.107 af, Depth= 0.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr WQV Rainfall=1.37"

	Area	(ac) C	N Desc	cription						
29.690 87 1/4 acre lots, 38% imp, HSG D										
18.408 62.00% Pervious Area										
	11.	282	38.0	0% Imperv	ious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	4.9	50	0.0250	0.17		Sheet Flow,				
	8.9	1,719	0.0250	3.21		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow, Paved Kv= 20.3 fps				
	13.8	1 769	Total							

Subcatchment 7S: ON-SITE #2



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Summary for Reach 6R: Reach 3

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 0.16" for WQV event

Inflow = 1.47 cfs @ 12.53 hrs, Volume= 0.244 af

Outflow = 1.10 cfs @ 12.99 hrs, Volume= 0.244 af, Atten= 25%, Lag= 27.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.21 fps, Min. Travel Time= 14.4 min Avg. Velocity = 0.53 fps, Avg. Travel Time= 32.8 min

Peak Storage= 950 cf @ 12.75 hrs

Average Depth at Peak Storage= 0.20'

Bank-Full Depth= 6.40' Flow Area= 252.4 sf, Capacity= 2,150.14 cfs

Custom cross-section, Length= 1,041.0' Slope= 0.0088 '/' (110 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 989.92', Outlet Invert= 980.77'

‡

Offset	Elevation	Chan.Depth
(feet)	(feet)	(feet)
0.00	996.29	0.00
7.00	996.00	0.29
12.99	994.06	2.23
13.24	994.00	2.29
14.48	993.84	2.45
28.84	992.00	4.29
29.66	991.71	4.58
36.75	990.00	6.29
39.18	989.92	6.37
39.90	989.89	6.40
42.20	989.97	6.32
43.14	990.00	6.29
43.91	990.40	5.89
46.34	992.00	4.29
53.90	993.41	2.88
56.74	994.00	2.29
62.76	994.22	2.07
76.31	994.65	1.64
100.00	996.29	0.00

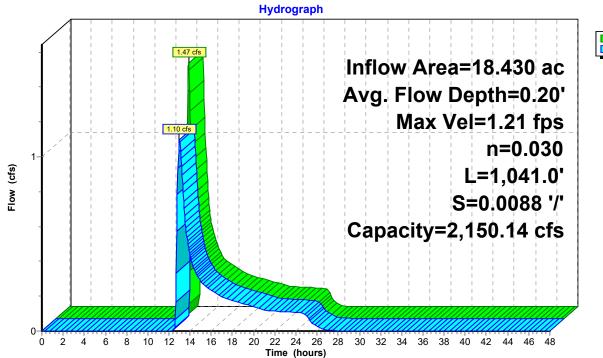
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Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
0.03	0.0	1.6	25	0.01
0.08	0.2	4.5	184	0.09
0.11	0.3	6.4	355	0.22
0.51	3.4	9.0	3,521	8.20
1.82	19.8	16.9	20,607	102.00
2.11	24.7	18.3	25,700	139.72
3.52	62.5	37.1	65,011	410.25
3.95	79.1	42.6	82,369	555.08
4.11	85.9	44.7	89,447	617.31
4.17	88.6	46.6	92,223	631.78
4.33	96.2	51.5	100,189	678.51
4.76	121.1	66.4	126,018	838.92
6.11	225.0	90.3	234,197	1,919.54
6.40	252.4	101.5	262,697	2,150.14

Reach 6R: Reach 3



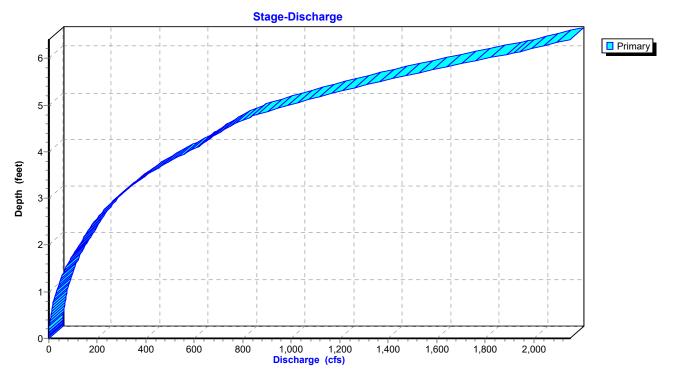


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Reach 6R: Reach 3



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Summary for Reach 7R: Reach 2

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 0.16" for WQV event

Inflow = 1.90 cfs @ 12.27 hrs, Volume= 0.244 af

Outflow = 1.47 cfs @ 12.53 hrs, Volume= 0.244 af, Atten= 23%, Lag= 16.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.65 fps, Min. Travel Time= 8.6 min Avg. Velocity = 0.84 fps, Avg. Travel Time= 16.9 min

Peak Storage= 764 cf @ 12.39 hrs Average Depth at Peak Storage= 0.23'

Bank-Full Depth= 3.24' Flow Area= 204.3 sf, Capacity= 1,999.47 cfs

Custom cross-section, Length= 853.0' Slope= 0.0225 '/' (106 Elevation Intervals)

Constant n= 0.030 Earth, grassed & winding Inlet Invert= 1,009.12', Outlet Invert= 989.90'

‡

Offset	Elevation	Chan.Depth
(feet)	(feet)	(feet)
0.00	1,009.14	0.00
22.15	1,008.00	1.14
24.54	1,007.80	1.34
25.18	1,007.74	1.40
49.26	1,006.00	3.14
49.59	1,005.90	3.24
50.51	1,005.99	3.15
52.13	1,005.99	3.15
53.24	1,006.00	3.14
91.41	1,007.74	1.40
98.60	1,008.00	1.14
108.78	1,008.27	0.87
117.07	1,008.47	0.67
117.16	1,008.47	0.67
134.91	1,008.89	0.25
135.00	1,009.14	0.00

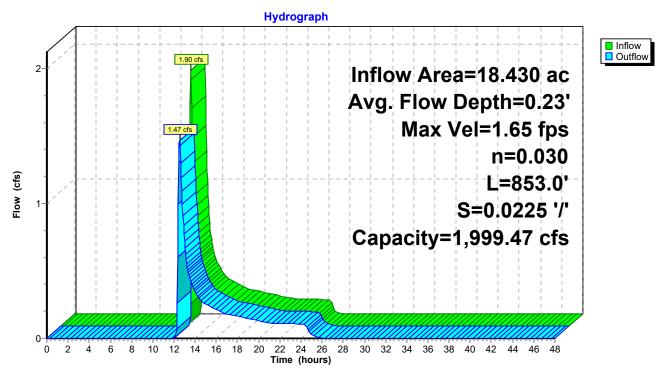
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		End Area	Perim. (feet)	Storage (cubic-feet)	Discharge (efs)
_	(feet)	(sq-ft)	(leet)	(cubic-leet)	(cfs)
	0.00	0.0	0.0	0	0.00
	0.09	0.1	2.9	47	0.03
	0.10	0.1	4.0	77	0.06
	1.84	61.2	66.4	52,179	430.84
	1.90	65.2	68.7	55,628	468.55
	2.10	79.7	76.6	67,995	608.70
	2.37	102.4	92.0	87,382	818.03
	2.57	122.0	104.3	104,091	1,007.45
	2.99	171.2	130.2	146,042	1,527.76
	3 24	204.3	135 4	174 303	1 999 47

Reach 7R: Reach 2

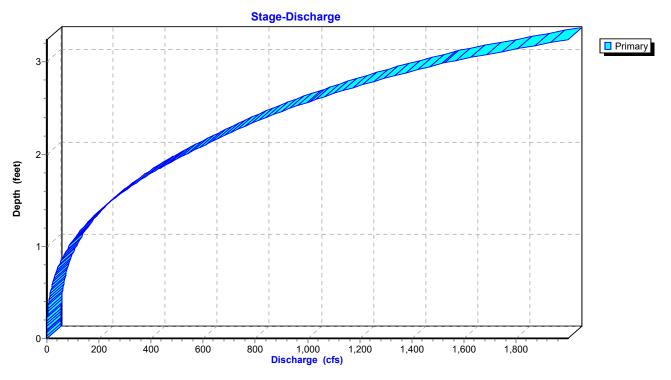


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Reach 7R: Reach 2



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Summary for Reach 8R: REACH 1 (through Pipe System)

Inflow Area = 18.430 ac, 0.00% Impervious, Inflow Depth = 0.16" for WQV event

Inflow = 2.03 cfs @ 12.16 hrs, Volume= 0.244 af

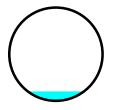
Outflow = 1.90 cfs @ 12.27 hrs, Volume= 0.244 af, Atten= 6%, Lag= 6.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

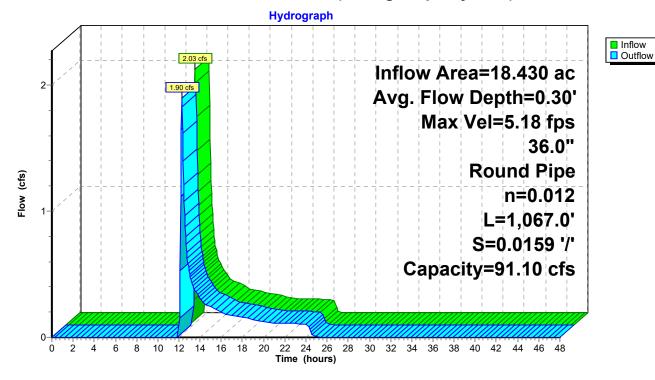
Max. Velocity= 5.18 fps, Min. Travel Time= 3.4 min Avg. Velocity = 2.44 fps, Avg. Travel Time= 7.3 min

Peak Storage= 394 cf @ 12.21 hrs Average Depth at Peak Storage= 0.30' Bank-Full Depth= 3.00' Flow Area= 7.1 sf, Capacity= 91.10 cfs

36.0" Round Pipe n= 0.012 Concrete pipe, finished Length= 1,067.0' Slope= 0.0159 '/' Inlet Invert= 1,021.96', Outlet Invert= 1,005.00'



Reach 8R: REACH 1 (through Pipe System)

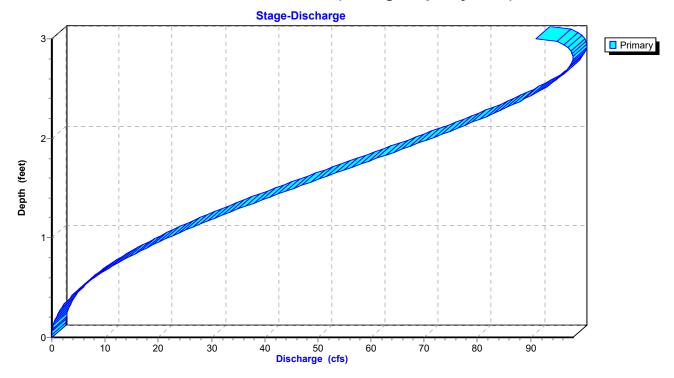


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Reach 8R: REACH 1 (through Pipe System)



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Summary for Pond 4P: EDDB #1 Cornerstone

Inflow Area = 31.710 ac, 26.16% Impervious, Inflow Depth = 0.69" for WQV event

Inflow = 29.17 cfs @ 12.06 hrs, Volume= 1.830 af

Outflow = 0.54 cfs @ 19.36 hrs, Volume= 1.544 af, Atten= 98%, Lag= 437.9 min

Primary = 0.54 cfs @ 19.36 hrs, Volume= 1.544 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Peak Elev= 1,000.17' @ 19.36 hrs Surf.Area= 28,490 sf Storage= 55,660 cf

Plug-Flow detention time= 957.6 min calculated for 1.542 af (84% of inflow)

Center-of-Mass det. time= 886.9 min (1,723.7 - 836.8)

Volume	Inver	t Avail.Sto	rage Storaç	ge Description		
#1	995.67	374,50	66 cf Custo	om Stage Data (Pri	ismatic)Listed below (Recalc)	
Elevation	9	Surf.Area	Inc.Store	Cum.Store		
(feet)	3	(sq-ft)	(cubic-feet)	(cubic-feet)		
995.67		0	0	0		
996.00		700	116	116		
997.00		7,200	3,950	4,066		
998.00		12,000	9,600	13,666		
999.00		17,900	14,950	28,616		
1,000.00		27,000	22,450	51,066		
1,001.00		36,000	31,500	82,566		
1,002.00		40,000	38,000	120,566		
1,003.00		44,800	42,400	162,966		
1,004.00		48,700	46,750	209,716		
1,005.00		53,000	50,850	260,566		
1,006.00		57,000	55,000	315,566		
1,007.00		61,000	59,000	374,566		
Device R	outing	Invert	Outlet Devi	ces		
	rimary	993.54'		nd 30" Culvert		
#1 1	Tillial y	330.04			jecting, Ke= 0.500	
					993.11' S= 0.0049 '/' Cc= 0.900	
					ned, Flow Area= 4.91 sf	
#2 D	evice 1	994.16'		VQ Orifice C= 0.6		
	evice 2	994.36'		d 8" PVC Pipe	,,,,	
#0 D	CVICC Z	334.00		•	ojecting, Ke= 0.200	
					994.16' S= 0.0100 '/' Cc= 0.900	
				Flow Area= 0.35 sf	754.10 0-0.0100 / 00-0.000	
#4 D	evice 3	995.67'			columns	
,, -	0 1100 0	000.07	1.0" Vert. Riser Pipe X 8.00 columns X 9 rows with 4.0" cc spacing C= 0.600			
#5 D	evice 1	1,001.05'			ng in Box C= 0.600	
	evice 1	1,004.00'			p of Box C= 0.600	
,, 0	- VIOC 1	1,004.00		veir flow at low hea		

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```
Primary OutFlow Max=0.54 cfs @ 19.36 hrs HW=1,000.17' (Free Discharge)

1=30" Culvert (Passes 0.54 cfs of 54.80 cfs potential flow)

2=WQ Orifice (Orifice Controls 0.54 cfs @ 11.68 fps)

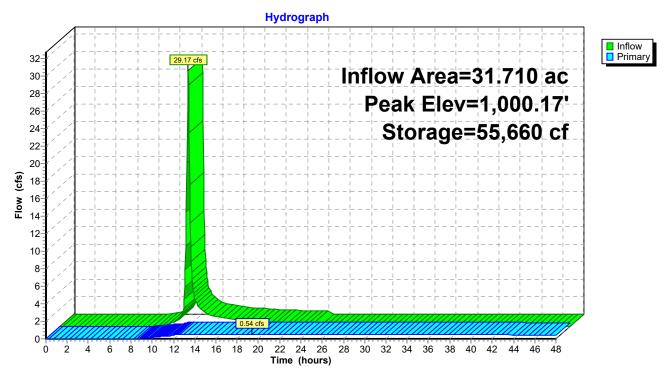
3=8" PVC Pipe (Passes 0.54 cfs of 4.45 cfs potential flow)

4=Riser Pipe (Passes 0.54 cfs of 3.31 cfs potential flow)

5=Opening in Box (Controls 0.00 cfs)

6=Open Top of Box (Controls 0.00 cfs)
```

Pond 4P: EDDB #1 Cornerstone

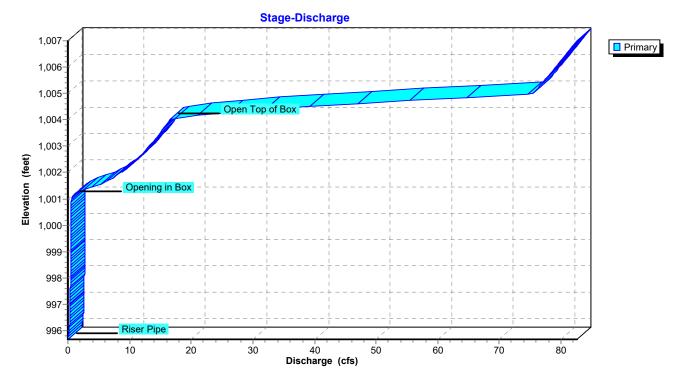


21-130-HYDRO-PRO orifice plate option

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Pond 4P: EDDB #1 Cornerstone



21-130-HYDRO-PRO orifice plate option

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Summary for Pond 5P: EDDB #2 Manor

29.690 ac, 38.00% Impervious, Inflow Depth = 0.45" for WQV event Inflow Area =

1.107 af Inflow

17.13 cfs @ 12.07 hrs, Volume= 0.39 cfs @ 19.53 hrs, Volume= 1.083 af, Atten= 98%, Lag= 447.7 min Outflow

Primary 0.39 cfs @ 19.53 hrs, Volume= 1.083 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 992.24' @ 19.53 hrs Surf.Area= 25,419 sf Storage= 31,748 cf

Plug-Flow detention time= 890.2 min calculated for 1.083 af (98% of inflow)

Center-of-Mass det. time= 878.1 min (1,743.6 - 865.5)

Volume	Invert A	Avail.Stor	age :	Storage	Description	
#1	988.16'	352,23	6 cf	Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevation (feet)	Surf.Ar (sq-		Inc.S -cubic)	Store -feet)	Cum.Store (cubic-feet)	
988.16		0		Ó	0	
989.00	8	00		336	336	
990.00	4,2	00	2	2,500	2,836	
991.00	10,4	00	7	7,300	10,136	
992.00	21,6	00	16	6,000	26,136	
993.00	37,6	00	29	9,600	55,736	
994.00	41,7	00	39	9,650	95,386	
995.00	46,0	00	43	3,850	139,236	
996.00	49,7	00	47	7,850	187,086	
997.00	53,2	00	51	1,450	238,536	
998.00	56,9	00	55	5,050	293,586	
999.00	60,4	00	58	3,650	352,236	
Device Ro	outina	Invert	Outle	t Device:	S	

Device	Routing	Invert	Outlet Devices
#1	Primary	986.00'	30.0" Round 30" Culvert
			L= 77.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 986.00' / 985.62' S= 0.0049 '/' Cc= 0.900
			n= 0.012 Concrete pipe, finished, Flow Area= 4.91 sf
#2	Device 1	986.62'	2.5" Vert. WQ Orifice C= 0.600
#3	Device 2	986.86'	8.0" Round 8" PVC Pipe
			L= 20.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 986.86' / 986.62' S= 0.0120 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.35 sf
#4	Device 3	988.16'	1.0" Vert. Riser Pipe X 8.00 columns
			X 8 rows with 4.0" cc spacing C= 0.600
#5	Device 1	993.40'	48.0" W x 6.0" H Vert. Opening in Box C= 0.600
#6	Device 1	995.25'	60.0" x 48.0" Horiz. Open Top of Box C= 0.600
			Limited to weir flow at low heads

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Primary OutFlow Max=0.39 cfs @ 19.53 hrs HW=992.24' (Free Discharge)

1=30" Culvert (Passes 0.39 cfs of 52.79 cfs potential flow)

2=WQ Orifice (Orifice Controls 0.39 cfs @ 11.31 fps)

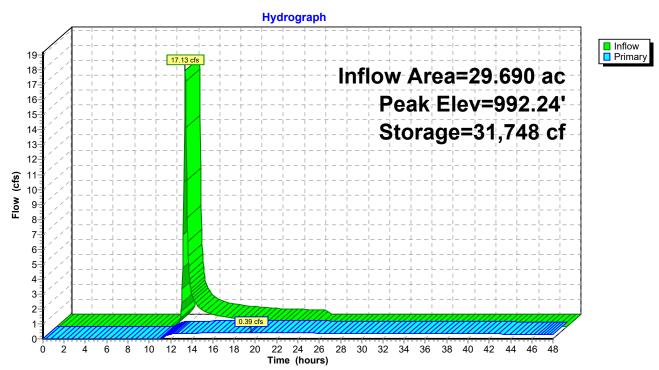
3=8" PVC Pipe (Passes 0.39 cfs of 3.78 cfs potential flow)

4=Riser Pipe (Passes 0.39 cfs of 2.82 cfs potential flow)

5=Opening in Box (Controls 0.00 cfs)

6=Open Top of Box (Controls 0.00 cfs)

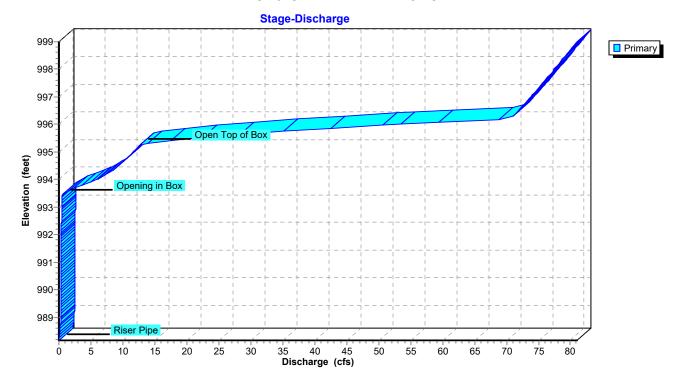
Pond 5P: EDDB #2 Manor



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Pond 5P: EDDB #2 Manor



21-130-HYDRO-PRO orifice plate option

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Summary for Link 2L: RP-1

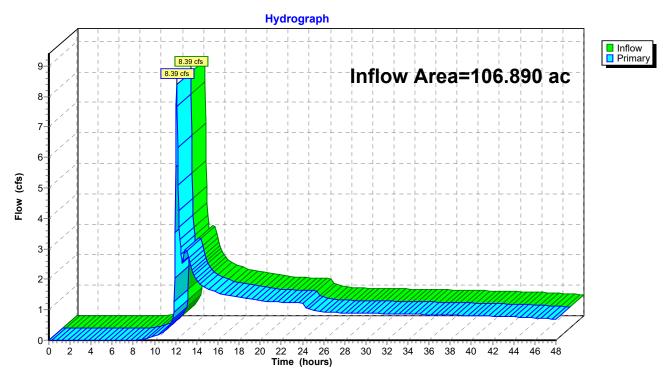
Inflow Area = 106.890 ac, 20.71% Impervious, Inflow Depth > 0.39" for WQV event

Inflow 3.473 af

8.39 cfs @ 12.10 hrs, Volume= 8.39 cfs @ 12.10 hrs, Volume= 3.473 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 2L: RP-1



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nom for Link OL - DD O

Summary for Link 3L: RP-2

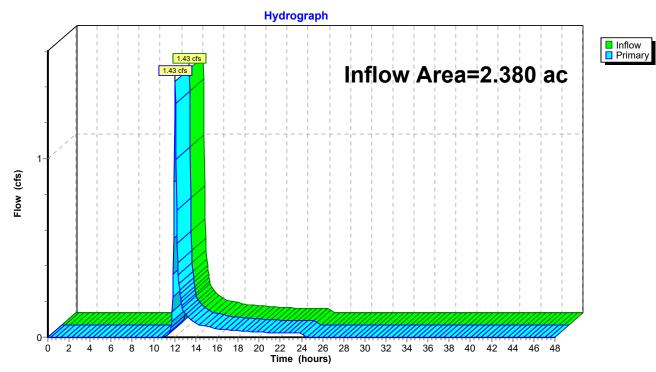
Inflow Area = 2.380 ac, 38.00% Impervious, Inflow Depth = 0.45" for WQV event

Inflow = 1.43 cfs @ 12.06 hrs, Volume= 0.089 af

Primary = 1.43 cfs @ 12.06 hrs, Volume= 0.089 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 3L: RP-2



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Summary for Link 4L: RP-3

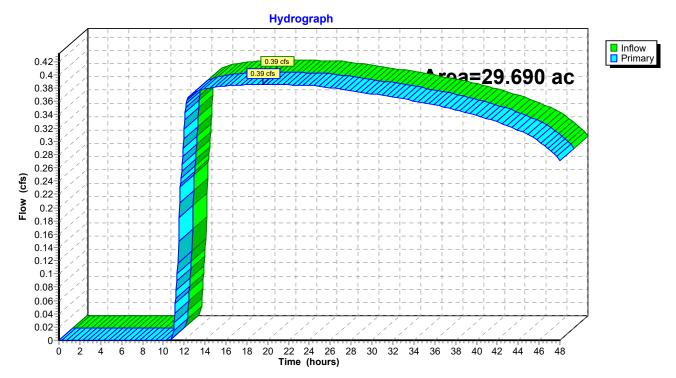
Inflow Area = 29.690 ac, 38.00% Impervious, Inflow Depth > 0.44" for WQV event

Inflow = 0.39 cfs @ 19.53 hrs, Volume= 1.083 af

Primary = 0.39 cfs @ 19.53 hrs, Volume= 1.083 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 4L: RP-3



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Summary for Link 5L: RP-4

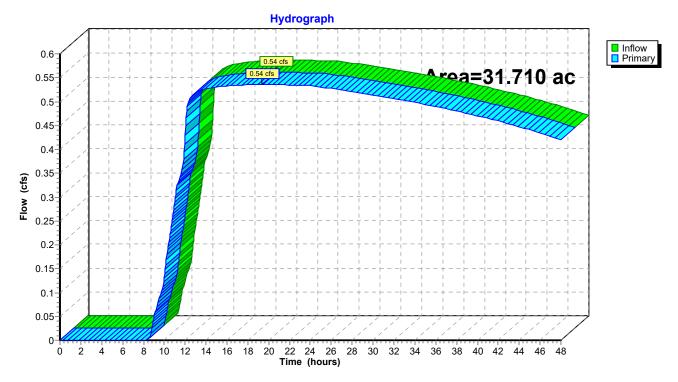
Inflow Area = 31.710 ac, 26.16% Impervious, Inflow Depth > 0.58" for WQV event

Inflow 1.544 af

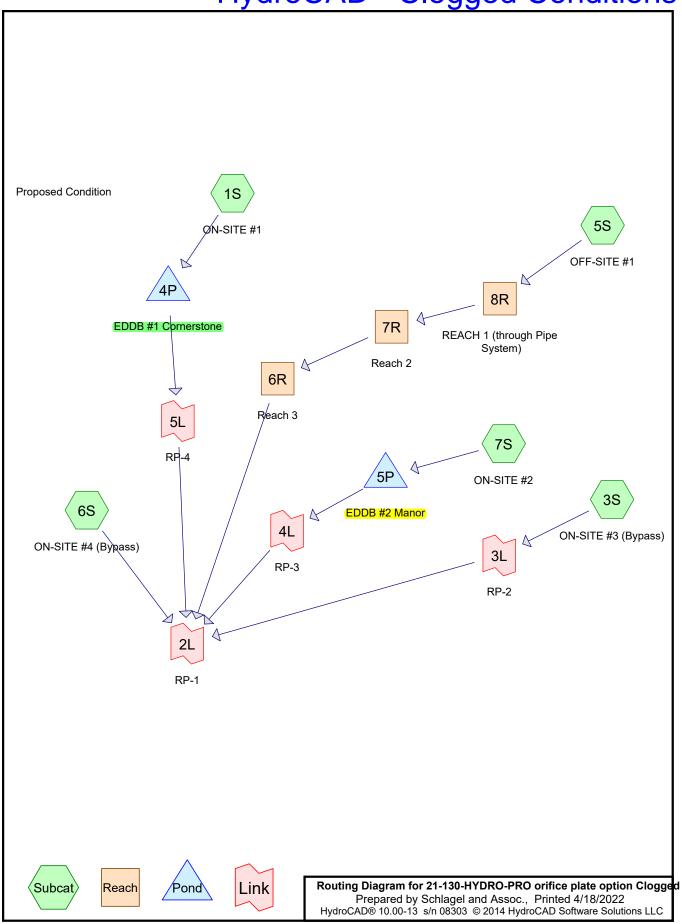
0.54 cfs @ 19.36 hrs, Volume= 0.54 cfs @ 19.36 hrs, Volume= 1.544 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 5L: RP-4



HydroCAD - Clogged Conditions



21-130-HYDRO-PRO orifice plate option Clogged

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Area Listing (all nodes)

Area	CN	Description	
(acres)		(subcatchment-numbers)	
32.070	87	1/4 acre lots, 38% imp, HSG D (3S, 7S)	
11.140	92	1/8 acre lots, 65% imp, HSG D (1S)	
22.730	80	>75% Grass cover, Good, HSG D (6S)	
19.330	92	Duplex/Townhomes (1S)	
3.190	95	Urban commercial, 85% imp, HSG D (1S, 6S)	
15.640	76	Woods/grass comb., Fair, HSG C (5S)	
2.790	82	Woods/grass comb., Fair, HSG D (5S)	

21-130-HYDRO-PRO orifice plate option Clogged

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Soil Listing (all nodes)

Area	Soil	Subcatchment		
(acres)	Group	Numbers		
0.000	HSG A			
0.000	HSG B			
15.640	HSG C	5S		
71.920	HSG D	1S, 3S, 5S, 6S, 7S		
19.330	Other	1S		

21-130-HYDRO-PRO orifice plate option CloggedPrepared by Schlagel and Assoc. HydroCAD® 10.00-13 s/n 08303 © 2014 HydroCAD Software Solutions LLC

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Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
 (acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	0.000	0.000	32.070	0.000	32.070	1/4 acre lots, 38% imp	3S,
							7S
0.000	0.000	0.000	11.140	0.000	11.140	1/8 acre lots, 65% imp	1S
0.000	0.000	0.000	22.730	0.000	22.730	>75% Grass cover, Good	6S
0.000	0.000	0.000	0.000	19.330	19.330	Duplex/Townhomes	1S
0.000	0.000	0.000	3.190	0.000	3.190	Urban commercial, 85% imp	1S,
							6S
0.000	0.000	15.640	2.790	0.000	18.430	Woods/grass comb., Fair	58

Bailey Farms - 100 yr Clogged

21-130-HYDRO-PRO orifice plate option Clogged

Type II 24-hr 100-Year Rainfall=7.70"

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Pond 4P: EDDB #1 Cornerstone Peak Elev=1,007.64' Storage=414,456 cf Inflow=260.19 cfs 17.828 af

Outflow=254.01 cfs 17.828 af

Pond 5P: EDDB #2 Manor Peak Elev=998.91 Storage=347,072 cf Inflow=232.03 cfs 15.236 af

Outflow=224.75 cfs 15.236 af

Type II 24-hr 100-Year Rainfall=7.70" Printed 4/18/2022

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Summary for Pond 4P: EDDB #1 Cornerstone

Inflow Area = 31.710 ac, 26.16% Impervious, Inflow Depth = 6.75" for 100-Year event

Inflow = 260.19 cfs @ 12.05 hrs, Volume= 17.828 af

Outflow = 254.01 cfs @ 12.08 hrs, Volume= 17.828 af, Atten= 2%, Lag= 1.5 min

Primary = 254.01 cfs @ 12.08 hrs, Volume= 17.828 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Starting Elev= 1,007.14' Surf.Area= 61,714 sf Storage= 383,155 cf

Peak Elev= 1,007.64 @ 12.08 hrs Surf.Area= 64,249 sf Storage= 414,456 cf (31,300 cf above start)

Plug-Flow detention time= 239.8 min calculated for 9.032 af (51% of inflow)

Avail Storage Storage Description

Center-of-Mass det. time= 3.8 min (777.7 - 773.9)

Invert

Volume

volume	11	ivert Avaii	.Storage	Storage	Description	
#1	99	95.67' 482,088 cf		Custon	rismatic)Listed below (Recalc)	
Elevation	on	Surf.Area In		:Store	Cum.Store	
(fee		(sq-ft)		c-feet)	(cubic-feet)	
995.6		0	•	0	0	
996.0	00	700		115	115	
997.0	00	7,200		3,950	4,065	
998.0	00	12,000		9,600	13,665	
999.0	00	17,900	•	14,950	28,615	
1,000.0	00	27,000	2	22,450	51,065	
1,001.0	00	36,000	(31,500	82,565	
1,002.0	00	40,000	(38,000	120,565	
1,003.0	00	44,800	4	12,400	162,965	
1,004.0	00	48,700	4	16,750	209,715	
1,005.0		53,000		50,850	260,565	
1,006.0		57,000		55,000	315,566	
1,007.0		61,000		59,000	374,566	
1,008.0		66,100		33,550	438,116	
1,008.6	35	69,200	4	13,972	482,088	
Device	Routin	g Inv	vert Outl	et Device	es	
#1	Drima	v 1.007	1/1' Aev	mmatric	al Wair C= 3 33	

#1 Primary 1,007.14' **Asymmetrical Weir, C= 3.33**

Offset (feet) 0.00 4.50 220.50 225.00 Height (feet) 1.50 0.00 0.00 1.50

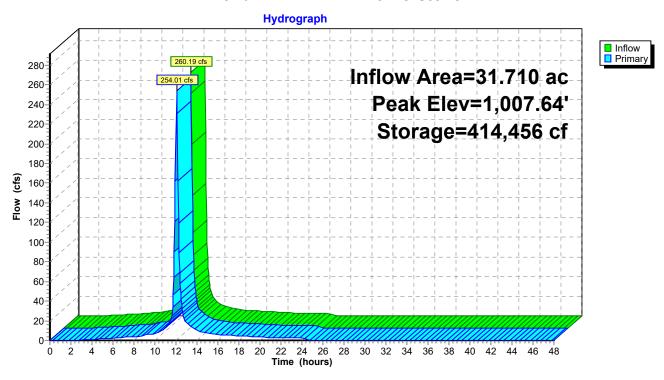
Primary OutFlow Max=248.51 cfs @ 12.08 hrs HW=1,007.63' (Free Discharge) 1=Asymmetrical Weir (Weir Controls 248.51 cfs @ 2.31 fps)

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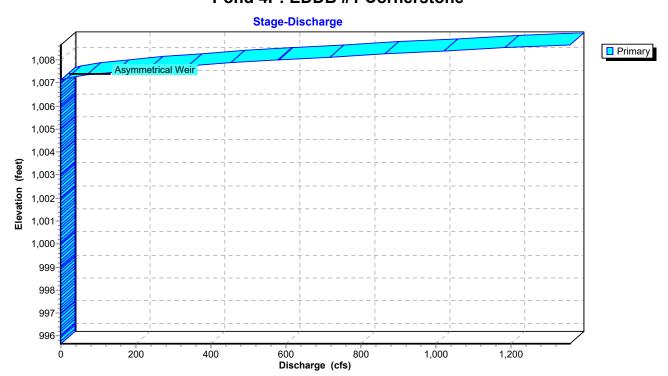
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Pond 4P: EDDB #1 Cornerstone



Pond 4P: EDDB #1 Cornerstone



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Type II 24-hr 100-Year Rainfall=7.70" Printed 4/18/2022

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Summary for Pond 5P: EDDB #2 Manor

Inflow Area = 29.690 ac, 38.00% Impervious, Inflow Depth = 6.16" for 100-Year event

Inflow = 232.03 cfs @ 12.05 hrs, Volume= 15.236 af

Outflow = 224.75 cfs @ 12.08 hrs, Volume= 15.236 af, Atten= 3%, Lag= 1.7 min

Primary = 224.75 cfs @ 12.08 hrs, Volume= 15.236 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Starting Elev= 998.36' Surf.Area= 58,160 sf Storage= 314,297 cf

Peak Elev= 998.91 @ 12.08 hrs Surf.Area= 60,100 sf Storage= 347,072 cf (32,775 cf above start)

Plug-Flow detention time= 222.9 min calculated for 8.020 af (53% of inflow)

Center-of-Mass det. time= 4.4 min (794.5 - 790.2)

Volume	In	vert Ava	il.Storage	Storage	Description	
#1	988	.16'	352,236 cf	Custom	Stage Data (P	rismatic)Listed below (Recalc)
Classation		Court Aman	l.	- Ct	Cura Ctara	
	Elevation Surf.Area			c.Store	Cum.Store	
(fee	et)	(sq-ft)	(cub	ic-feet)	(cubic-feet)	
988.1	16	0		0	0	
989.0	00	800		336	336	
990.0	00	4,200		2,500	2,836	
991.0	00	10,400		7,300	10,136	
992.0	00	21,600		16,000	26,136	
993.0	00	37,600		29,600	55,736	
994.0	00	41,700		39,650	95,386	
995.0	00	46,000		43,850	139,236	
996.0	00	49,700		47,850	187,086	
997.0	00	53,200		51,450	238,536	
998.0	00	56,900		55,050	293,586	
999.0	00	60,400		58,650	352,236	
Device	Routing	g <u>l</u> lr	nvert Ou	tlet Device	es	
#1	Primar	/ 99	8.36' As	ymmetrica	al Weir, C= 3.27	7
			Off	set (feet)	0.00 4.50 169	50 174 00

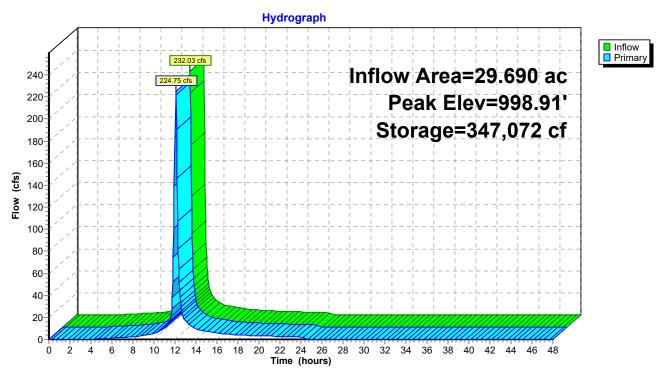
Offset (feet) 0.00 4.50 169.50 174.00 Height (feet) 1.50 0.00 0.00 1.50

Primary OutFlow Max=220.16 cfs @ 12.08 hrs HW=998.91' (Free Discharge) 1=Asymmetrical Weir (Weir Controls 220.16 cfs @ 2.39 fps)

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Pond 5P: EDDB #2 Manor



Pond 5P: EDDB #2 Manor

