HAWTHORN RIDGE THIRD PLAT STREET & STORM SEWER PLANS

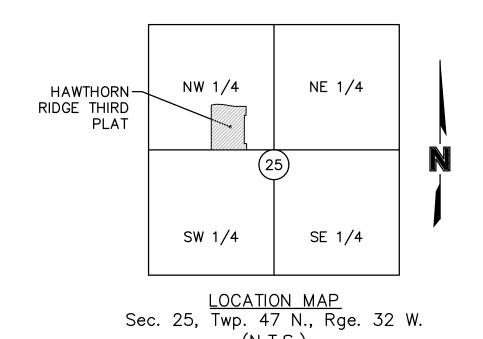
NW 1/4 SECTION 25, TOWNSHIP 47 N, RANGE 32 W IN LEE'S SUMMIT, JACKSON COUNTY, MO

Accepted Record Drawings

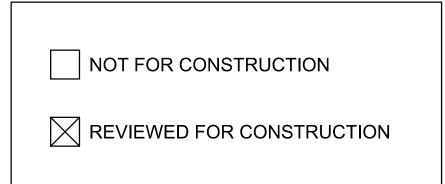
These plans have been reviewed for accuracy and are accepted for basic conformance to the approved

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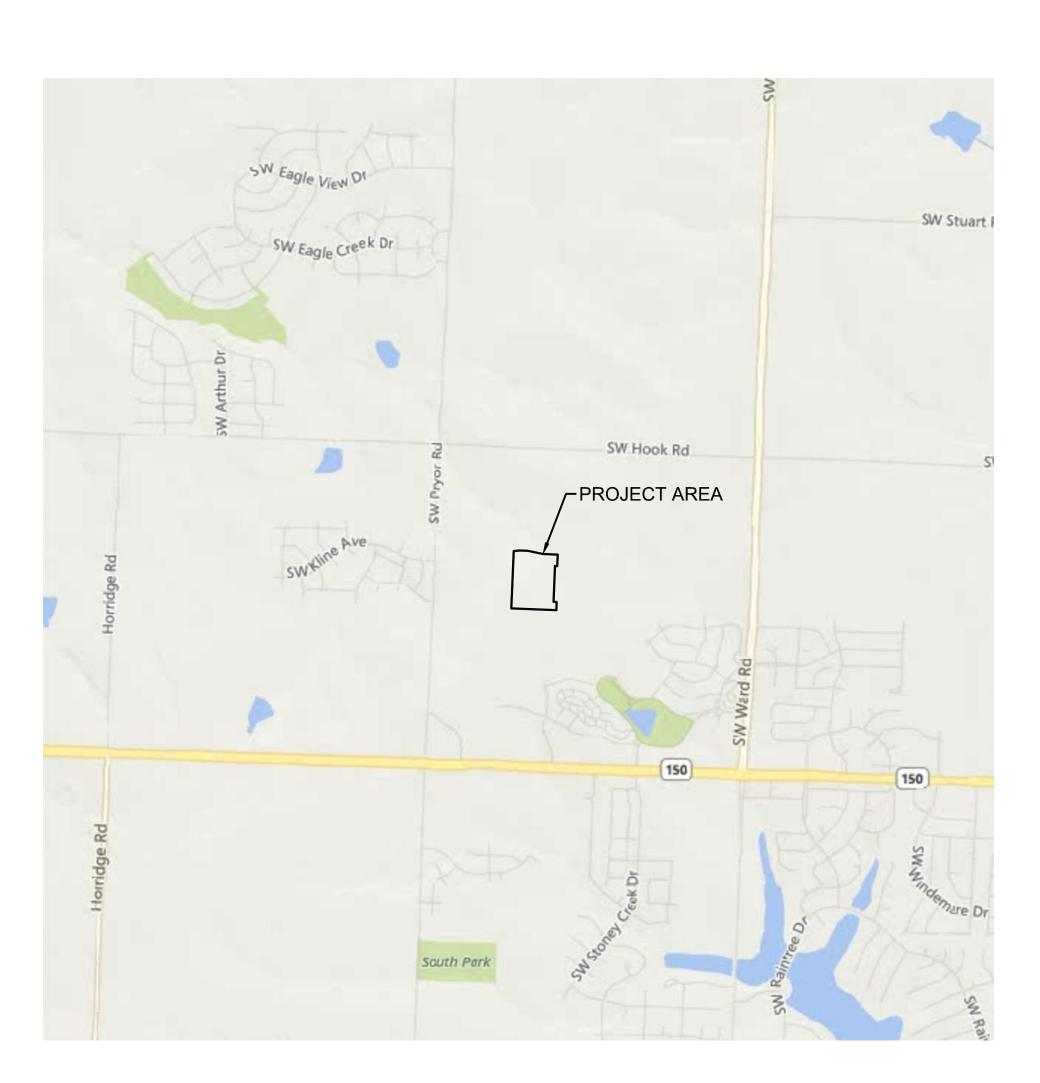
1/18/2022



PROJECT TEAM & UTILITY CONTAC	T LIST
OWNER / DEVELOPER CLAYTON PROPERTIES GROUP, INC. D.B.A. SUMMIT HOMES 120 SE 30TH STREET CONTACT: VINCENT WALKER LEE'S SUMMIT, MO 64082 PHONE: 816.246.6700 EMAIL: VINCENT@SUMMITHOMESKC.COM	UTILITY SERVICE NUMBERS NAME: LEE'S SUMMIT PUBLIC WORKS PHONE: 816-969-1800 NAME: LEE'S SUMMIT WATER & SERVICES DEPARTMENT PHONE: 816-969-1940
ENGINEER OLSSON 1301 BURLINGTON ST. SUITE 100 NORTH KANSAS CITY, MO 64116 CONTACT: BROCK M. WORTHLEY PHONE: 816.361.1177 EMAIL: BWORTHLEY@OLSSON.COM	NAME: SPIRE (MGE) PHONE: 314-342-0500 NAME: AT&T PHONE: 800-286-8313 NAME: KCP&L PHONE: 816-471-5275
SURVEYOR OLSSON 1301 BURLINGTON ST. SUITE 100 NORTH KANSAS CITY, MO 64116 CONTACT: JASON ROUDEBUSH PHONE: 816.361.1177 EMAIL: JROUDEBUSH@OLSSON.COM	NAME: SPECTRUM (TWC) PHONE: 877-772-2253 NAME: GOOGLE FIBER PHONE: 877-454-6959







PROPERTY DESCRIPTION:

A TRACT OF LAND IN THE NORTHWEST QUARTER OF SECTION 25, TOWNSHIP 47 NORTH, RANGE 32 WEST OF THE 5TH PRINCIPAL MERIDIAN IN LEE'S SUMMIT, JACKSON COUNTY, MISSOURI, AND A PORTION OF TRACT E, OF HAWTHORN RIDGE 1ST PLAT, A SUBDIVISION OF LAND RECORDED AS DOCUMENT 2019E0020897 IN BOOK 182 AT PAGE 83, IN THE OFFICE OF RECORDER OF DEEDS FOR JACKSON COUNTY, MISSOURI ALL BEING BOUNDED AND DESCRIBED AS FOLLOWS: COMMENCING AT THE SOUTHWEST CORNER OF SAID NORTHWEST QUARTER OF SECTION 25; THENCE SOUTH 87°46'49" EAST ON THE SOUTH LINE OF SAID NORTHWEST QUARTER, 2,653.29 FEET TO THE SOUTHEAST CORNER OF SAID NORTHWEST QUARTER, (CENTER OF SECTION) AND THE POINT OF BEGINNING OF THE TRACT OF LAND TO BE HEREIN DESCRIBED; THENCE ON SAID SOUTH LINE OF SAID NORTHWEST QUARTER, NORTH 87°46'49" WEST, 577.00 FEET; THENCE LEAVING SAID SOUTH LINE, NORTH 02°13'11" EAST, 135.00 FEET; THENCE NORTH 87'46'49" WEST, 50.79 FEET; THENCE NORTH 02"18'36" EAST, 596.57 FEET; THENCE SOUTH 87°41'24" EAST, 37.00 FEET; THENCE NORTH 02°18'36" EAST, 192.00 FEET; THENCE NORTH 87°41'24" WEST, 75.58 FEET; THENCE NORTH 02'18'36" EAST, 39.05 FEET; THENCE NORTH 33"13'50" WEST, 218.11 FEET TO THE SOUTHWESTERLY CORNER OF LOT 21 OF SAID HAWTHORN RIDGE 1ST PLAT; THENCE ON THE SOUTHERLY LINE OF SAID HAWTHORN RIDGE 1ST PLAT THE FOLLOWING 9 CALLS, NORTH 54°57'38" EAST, 130.00 FEET; THENCE SOUTH 35°02'22" EAST, 58.00 FEET; THENCE NORTH 54°57'38" EAST, 175.90 FEET; THENCE SOUTH 65°30'07" EAST, 95.33 FEET; THENCE SOUTH 87°41'24" EAST, 121.73 FEET; THENCE NORTH 0218'36" EAST, 78.00 FEET; THENCE SOUTH 87'41'24" EAST, 175.00 FEET; THENCE NORTH 0218'36" EAST, 72.00 FEET; THENCE SOUTH 87°41'24" EAST, 130.00 FEET TO THE SOUTHEASTERLY CORNER OF LOT 28 OF SAID HAWTHORN RIDGE 1ST PLAT AND A POINT ON THE EAST LINE OF SAID NORTHWEST QUARTER; THENCE ON SAID EAST LINE, SOUTH 02'18'36" WEST, 1,392.58 FEET TO THE POINT OF BEGINNING. CONTAINING 827,409 SQUARE FEET OR 19.00 ACRES, MORE OR

BENCHMARK

RR SPIKE IN SOUTH FACE OF POWER POLE ON NORTH SIDE OF SW. HOOK ROAD, IMMEDIATELY WEST OF DRIVEWAY FOR HOUSE#1622. ELEVATION= 1024.63'

	Sheet List Table
Sheet Number	Sheet Title
C100	COVER SHEET
C101	GENERAL NOTES
C102	GENERAL LAYOUT
C103	TYPICAL SECTIONS
C104	GRADING PLAN (FOR REFERENCE ONLY)
C105	SWALE PLAN AND PROFILE
C106	SWALE PLAN AND PROFILE (CONT)
C107	ROADWAY PLAN AND PROFILE (BUCKTHORN STREET)
C108	ROADWAY PLAN AND PROFILE (BUCKTHORN STREET CONT)
C109	ROADWAY PLAN AND PROFILE (ARBORWAY TERRACE)
C110	ROADWAY PLAN AND PROFILE (ARBORWAY TERRACE CONT)
C111	ROADWAY PLAN AND PROFILE (ARBORWAY TERRACE CONT)
C112	ROADWAY PLAN AND PROFILE (ARBORIDGE CIRCLE)
C113	TRAFFIC CONTROL PLAN
C114	SPOT ELEVATIONS
C115	SPOT ELEVATIONS
C116	SPOT ELEVATIONS
C117	SPOT ELEVATIONS
C118	STORM SEWER PLAN & PROFILE (LINE 1 & 1A)
C119	STORM SEWER PLAN & PROFILE (LINE 2)
C120	STORM SEWER PLAN & PROFILE (LINE 3)
C121	STORM SEWER PLAN & PROFILE (LINE 5)
C122	STORM SEWER PLAN & PROFILE (LINE 4 & 6)
C123	DRAINAGE PLAN
C124	DRAINAGE TABLES
C125	MASTER DRAINAGE PLAN
C126	SIGN DETAILS
C127	STORM SEWER DETAILS
C128	STORM SEWER DETAILS
C129	ROADWAY MARKING DETAILS

OLSSON HAS BEEN RETAINED TO PROVIDE AS-BUILT DRAWINGS FOR THIS PROJECT.

MO# PE-2019000237

BROCK M. WORTHLEY, P.E. **CIVIL ENGINEER**

1/18/2022 DATE

> QA/QC by: drawing no.: <u>C_TTL01_A191605</u>

> > SHEET

- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DEVIATIONS FROM THESE PLANS UNLESS WRITTEN APPROVAL FROM ENGINEER, OWNER, AND DEVELOPER.
- 3. ALL WORK AND MATERIALS SHALL BE SUBJECT TO INSPECTION AND APPROVAL BY THE OWNER OR THE OWNER'S REPRESENTATIVE.
- 4. ALL ESTIMATES OF QUANTITIES ARE FOR INFORMATIONAL PURPOSES ONLY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING QUANTITIES AND ITEMS OF WORK.
- 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL LABOR, MATERIALS, AND EQUIPMENT REQUIRED TO COMPLETE THE WORK SHOWN IN THE PLANS.
- 6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL REQUIRED PERMITS, PAYING ALL FEES, AND FOR OTHERWISE COMPLYING WITH ALL APPLICABLE REGULATIONS GOVERNING THE WORK.
- 7. THE CONTRACTOR SHALL NOT ENGAGE IN ACTIVITIES THAT MAY ENCROACH ON WATERS OF THE U.S., INCLUDING WETLANDS, UNTIL ANY NECESSARY PERMITS MAY BE OBTAINED. THE CONTRACTOR SHALL REVIEW AND COMPLY WITH ALL CONDITIONS DESCRIBED IN THE PERMIT.
- 8. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, THE SAFETY OF ALL PERSONS INCLUDING VISITORS AND THE GENERAL PUBLIC, AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY THROUGHOUT THE PROJECT AND NOT BE LIMITED BY WORKING HOURS. ANY CONSTRUCTION OBSERVATION BY THE ENGINEER OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES.
- 9. PRIOR TO COMMENCEMENT OF WORK THE CONTRACTOR SHALL NOTIFY AND COORDINATE WITH ALL UTILITY COMPANIES AND OBTAIN ANY RELEVANT INFORMATION. NOTIFY ENGINEER OF ANY DISCREPANCIES.
- 10. THE CONTRACTOR IS RESPONSIBLE FOR THE PROTECTION OF ALL BOUNDARY CORNERS AND SECTION CORNERS. ANY BOUNDARY CORNER AND/OR SECTION CORNER DISTURBED OR DAMAGED BY CONSTRUCTION ACTIVITIES SHALL BE RESET BY A LAND SURVEYOR LICENSED IN THE STATE OF MISSOURI, AT THE CONTRACTOR'S EXPENSE.
- 11. THE CONTRACTOR IS RESPONSIBLE FOR THE PROTECTION OF ADJACENT PROPERTIES AND SHALL TAKE ALL PRECAUTIONS NECESSARY TO PREVENT DAMAGE DURING CONSTRUCTION. THE CONTRACTOR IS ALSO RESPONSIBLE FOR REPAIRING ANY DAMAGE RESULTING FROM CONSTRUCTION ACTIVITIES.
- 12. PRIOR TO MOVING OFF THE JOB THE CONTRACTOR SHALL NOTIFY THE OWNER AND ENGINEER TO PERFORM A FINAL WALK—THROUGH OF THE CONSTRUCTION SITE.

REFERENCES

- 1. UNLESS EXPLICITLY DESCRIBED OTHERWISE WITHIN THESE PLANS THE FOLLOWING SHALL APPLY;

 A ALL CONSTRUCTION INCLUDING THOSE LISTED BELOW SHALL
- A. ALL CONSTRUCTION, INCLUDING THOSE LISTED BELOW, SHALL CONFORM TO THE LATEST CODES AND ORDINANCES OF LEE'S SUMMIT, MISSOURI.
- B. ALL CONSTRUCTION IN MODOT RIGHT-OF-WAY SHALL CONFORM TO THE LATEST SPECIFICATIONS ADOPTED BY U.S. DEPARTMENT OF TRANSPORTATION AND MODOT.
- C. ALL TRAFFIC CONTROL SIGNAGE SHALL CONFORM WITH THE CURRENT EDITION OF THE MANUAL FOR UNIFORM TRAFFIC CONTROL DEVICES
- D. ALL UTILITY EXTENSIONS AND CONSTRUCTION SHALL CONFORM TO
- THE STANDARDS AND SPECIFICATIONS OF THE UTILITY COMPANIES..

 E. ALL EXTERIOR PAVEMENT (PCC, ASPHALT, ETC.) SHALL BE IN CONFORMANCE WITH THE SPECIFICATIONS OF LEE'S SUMMIT, MISSOURI AND THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT.
- 4. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING THE DELIVERY MANAGER AND COORDINATING ANY MAILBOXES THAT MAY BE DISTURBED. FAILURE TO DO SO MAY SUBJECT THE CONTRACTOR TO PROSECUTION BY THE FEDERAL GOVERNMENT.

EXISTING CONDITIONS

- 1. THE CONTRACTOR SHALL VISIT THE SITE AND BECOME FAMILIAR WITH THE EXISTING CONDITIONS OF THE PROJECT AREA.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PERFORMING THEIR OWN INVESTIGATIONS AND MAKING THEIR OWN ASSUMPTIONS REGARDING SITE SURFACE AND SUBSURFACE CONDITIONS. THIS INCLUDES THE LOCATION AND CONSISTENCY OF ANY EXISTING ROCK LAYERS UNDERLYING THE PROJECT SITE. CONTACT THE ENGINEER REGARDING ANY DISCREPANCIES THAT MAY AFFECT THE ABILITY TO CONSTRUCT FROM THESE PLANS AS DESIGNED.
- 3. EXISTING CONDITIONS WERE DETERMINED THROUGH A VARIETY OF METHODS THAT MAY INCLUDE SURVEY, AERIAL IMAGERY, AVAILABLE RECORDS, GIS DATA, ETC. SUBSURFACE CONDITIONS ARE APPROXIMATE AND MAY NOT INCLUDE ALL UTILITIES AND OTHER SITE IMPROVEMENTS PRESENT ON SITE. THE CONTRACTOR SHALL MAKE EXPLORATION EXCAVATIONS AND LOCATE EXISTING UNDERGROUND UTILITIES SUFFICIENTLY AHEAD OF CONSTRUCTION TO PERMIT REVISIONS TO PLANS WHEN CONFLICTS AND DISCREPANCIES ARE FOUND.

CONSTRUCTION

- 1. THE CONTRACTOR SHALL INSTALL TRAFFIC CONTROL WHILE WORKING IN THE PUBLIC RIGHT-OF-WAY AS SHOWN IN THESE PLANS. IF PLANS ARE NOT PROVIDED, CONTRACTOR SHALL COORDINATE AND PROVIDE CONTROLS TO THE SATISFACTION OF THE RIGHT-OF-WAY OWNER.
- 2. THE CONTRACTOR SHALL PROTECT ALL TREES OVER 3" CALIPER FROM DAMAGE. NO TREE SHALL BE REMOVED WITHOUT PERMISSION OF THE OWNER, UNLESS SHOWN OTHERWISE ON THESE PLANS.
- 3. THE CONTRACTOR SHALL DISPOSE ALL WASTE MATERIAL RESULTING FROM THE PROJECT OFF—SITE AND IN STRICT CONFORMANCE WITH ALL LOCAL CODES AND ORDINANCES.
- 4. ALL MANHOLES, CATCH BASINS, UTILITY VALVES AND METER PITS ARE TO BE ADJUSTED OR REBUILT TO GRADE AS REQUIRED. NOT ALL ADJUSTMENTS ARE INDICATED IN THE PLANS.
- 5. THE CONTRACTOR SHALL STREET SWEEP OR OTHERWISE CLEAN ALL ACCESS ROUTES TO THE SITE AT CONCLUSION OF THE PROJECT.

SHOP DRAWINGS

- 1. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS A MINIMUM OF 7 DAYS PRIOR TO THE REQUESTED DATE OF APPROVAL. ENGINEER SHALL REVIEW SHOP DRAWINGS OR SAMPLES IN CONFORMANCE WITH THE DESIGN FOR THIS PROJECT AS DESCRIBED IN THE PLANS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS. THE ENGINEER'S REVIEW SHALL NOT EXTEND TO MEANS OR METHODS OF CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY VARIATION FROM THE REQUIREMENTS OF THE CONTRACT DOCUMENTS UNLESS CONTRACTOR HAS NOTIFIED ENGINEER OF EACH SUCH VARIATION AT THE TIME OF SUBMISSION, AND OBTAINED ENGINEER'S WRITTEN APPROVAL OF EACH SUCH VARIATION. PRIOR TO SUBMITTING EACH SHOP DRAWING OR SAMPLE, CONTRACTOR SHALL HAVE REVIEWED AND VERIFIED:
- A. ALL FIELD MEASUREMENTS, QUANTITIES, DIMENSIONS, SPECIFIED PERFORMANCE CRITERIA, INSTALLATION REQUIREMENTS, MATERIALS, CATALOG NUMBERS AND SIMILAR INFORMATION WITH RESPECT THERETO:
- B. ALL MATERIALS WITH RESPECT TO INTENDED USE, FABRICATION, SHIPPING, HANDLING, STORAGE, ASSEMBLY AND INSTALLATION
- PERTAINING TO THE PERFORMANCE OF THE WORK;

 C. ALL INFORMATION RELATIVE TO MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OF CONSTRUCTION AND SAFETY
- PRECAUTIONS AND PROGRAMS INCIDENT THERETO;

 D. CONTRACTOR SHALL ALSO HAVE REVIEWED AND COORDINATED EACH SHOP DRAWING OR SAMPLE WITH OTHER SHOP DRAWINGS AND SAMPLES, AND WITH THE REQUIREMENTS OF THE WORK AND THE
- CONTRACT DOCUMENTS.

 E. ALL SUBMITTED SHOP DRAWINGS SHALL BEAR A STAMP OR SPECIFIC WRITTEN INDICATION AND SIGNATURE THAT CONTRACTOR HAS FULLY COMPLETED THE ABOVE TASKS.
- 2. SHOP DRAWINGS AS DESCRIBED ABOVE ARE REQUIRED FOR, BUT NOT LIMITED TO, THE FOLLOWING:
- A. ALL STORM SEWER STRUCTURES TO BE INSTALLED WITH THIS PROJECT.
- B. ANY ITEMS IN THESE PLANS THAT ALLOW FOR AN "APPROVED EQUAL" ALTERNATIVE.

STORM SEWER GENERAL NOTES:

- STORM STRUCTURES SHALL BE PER CURRENT CITY DETAILS. IF CITY DOES NOT HAVE PUBLISHED DETAILS STRUCTURES SHALL BE PER CURRENT APWA SPECIFICATIONS.
- 2. PRIOR TO COMMENCEMENT OF WORK THE CONTRACTOR SHALL NOTIFY AND COORDINATE CONSTRUCTION WITH CITY OF LEE'S SUMMIT, MISSOURI.
- 3. ALL PIPE LENGTHS AND ELEVATIONS ARE CALCULATED LINEARLY FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE.
- 4. ALL STRUCTURE DIMENSIONS ARE TO INSIDE FACE OF STRUCTURE.
- 5. COORDINATES ARE PROVIDED AT THE CENTER OF STRUCTURE. ADDITIONAL COORDINATES PROVIDED ARE PER LOCAL CODES AND ORDINANCES OR AS AN AID WHEN ORIENTING THE BOX DURING INSTALLATION.
- THE CONTRACTOR SHALL EXPOSE EXISTING UTILITIES AT LOCATIONS OF POSSIBLE CONFLICT AND POINTS OF CONNECTION PRIOR TO ANY CONSTRUCTION OF STORM SEWER.
- 7. STORM SEWER TRENCHES SHALL BE CONSTRUCTED SUCH THAT UNDISTURBED EXISTING SOIL OR FILL COMPACTED TO 95% PROCTOR DENSITY IS AT A DEPTH THAT IS 18" ABOVE TOP OF PROPOSED PIPE.
- 8. STRUCTURE INVERT CHANNELS SHALL BE SMOOTH, CIRCULAR, AND CONFORMING TO ½ THE ADJACENT PIPE SECTION (INVERT TO CENTER). CHANGES IN DIRECTION OF FLOW SHALL BE MADE WITH A SMOOTH CURVE AND MAINTAIN SHAPE THROUGHOUT. CHANGES IN GRADE OF ADJACENT PIPES SHALL BE TRANSITIONED SMOOTHLY AND EVENLY THROUGH THE STRUCTURE.
- 9. PIPE PENETRATIONS SHALL BE GROUTED TO ENSURE WATERTIGHT SEALS.
- 10. MAINTAIN MINIMUM DEPTH OF COVER PER APWA 5606.06

CONTROL POINT TABLE

HORIZONTAL CONTROL.

Point Number	Northing	Easting	Point Elevation	Raw Description
90009	981383.7330'	2813865.4520'	1064.23'	CP 60D
90012	981431.6120'	2813832.1000'	1062.71'	CP 60D
90033	981440.4750'	2814063.8700'	1047.98'	CP 60D
90044	981710.8560'	2814198.8050'	1027.00'	CP 60D
90052	981859.5430'	2814200.2150'	1017.51'	CP 60D
90056	981975.4580'	2814144.8570'	1011.69'	CP 60D
90080	981971.2190'	2814027.5570'	1016.72'	CP 60D

VERTICAL CONTROL IS BASED ON THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88). THE DEVELOPER IS ADVISED TO USE BENCHMARK INFORMATION FOR VERTICAL CONTROL. HORIZONTAL CONTROL (CONTROL POINT INFORMATION) IS BASED ON THE NORTH AMERICAN DATUM OF 1983 (NAVD83). THE DEVELOPER IS ADVISED TO USE CONTROL POINT INFORMATION FOR

	ESTIMATE OF QUANTITIES									
ITEM NO.	DESCRIPTION	UNIT	QUANTITY	AS-BUILT						
	STREET									
	EXCAVATION	C.Y.	5712							
	EMBANKMENT	C.Y.	12363							
	SUBGRADE STABILIZATION (6" FLYASH TREATMENT)	S.Y.	5481							
	SUBGRADE STABILIZATION (9" FLYASH TREATMENT)	S.Y.	2962							
	6" ASPHALT PAVEMENT	S.Y.	4540							
	7.5" ASPHALT PAVEMENT	S.Y.	2625							
	CONCRETE CURB & GUTTER (CG-1)	L.F.	1371							
	CONCRETE CURB & GUTTER (CG-2)	L.F.	2771							
	MILL & OVERLAY	S.Y.	31							
	ADA RAMP	EA.	4							
	STOP SIGNS	EA.	2							
	STREET NAME SIGNS	EA.	6							
	END OF ROAD TREATMENT	EA.	1							
	STORM									
	STD. CURB INLET (5'x3' INSIDE)	EA.	11	11						
	STD. CURB INLET (5'x4' INSIDE)	EA.	1	1						
	STD. FIELD INLET (4'x4' INSIDE)	EA.	1	1						
	RAISE EXISTING JUNCTION BOX (6'x6' INSIDE)	V.F.	5.06	5.0						
	15" RCP	L.F.	-103.17	103.79						
	15" HDPE	L.F.	-1295.35	1296.05						
	24" HDPE	L.F.	85.23	85.20						
	48" HDPE	L.F.	-122.83	123.79						
	60" HDPE	L.F.	-197.33	186.95						
	15" HDPE END SECTION	EA.	1	1						
	60" CMP END SECTION	EA.	1	1						
	RIPRAP	S.Y.	88.5	147.98						
	CONNECTION TO EXISTING STRUCTURE	EA.	11	1						
	CONNECTION TO EVISTING PIPE	ГΛ		1						

Accepted Record Drawings

EA. | — 2 |

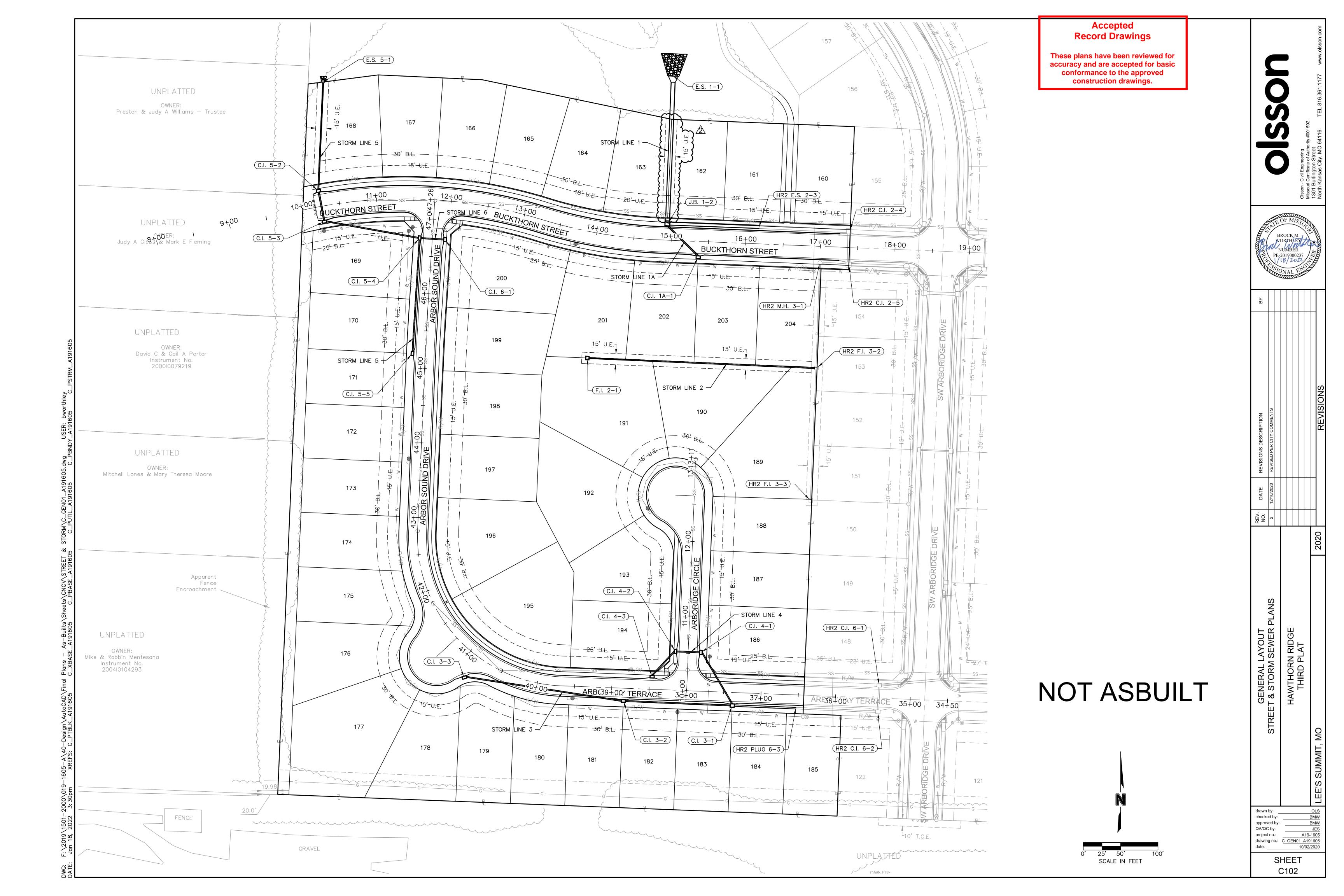
CONNECTION TO EXISTING PIPE

These plans have been reviewed for accuracy and are accepted for basic conformance to the approved construction drawings.

ASBUILT 1/18/2022 **Colorering**

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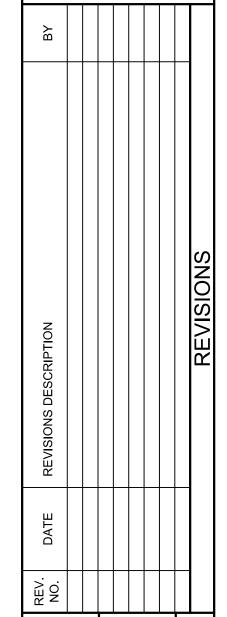
10/02/2020



Accepted Record Drawings

These plans have been reviewed for accuracy and are accepted for basic conformance to the approved construction drawings.





 drawn by:
 OLS

 checked by:
 BMW

 approved by:
 BMW

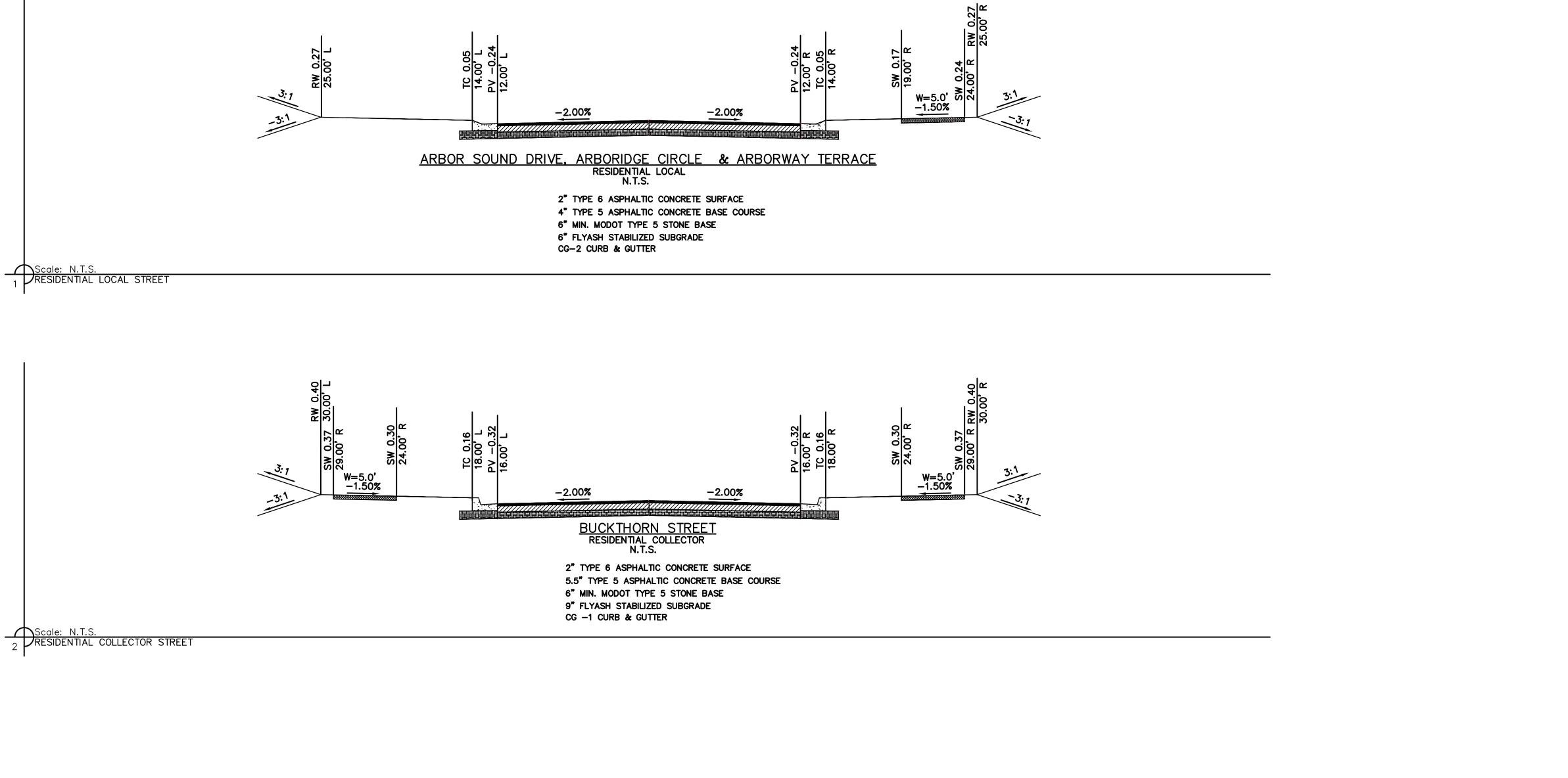
 QA/QC by:
 JES

 project no.:
 A19-1605

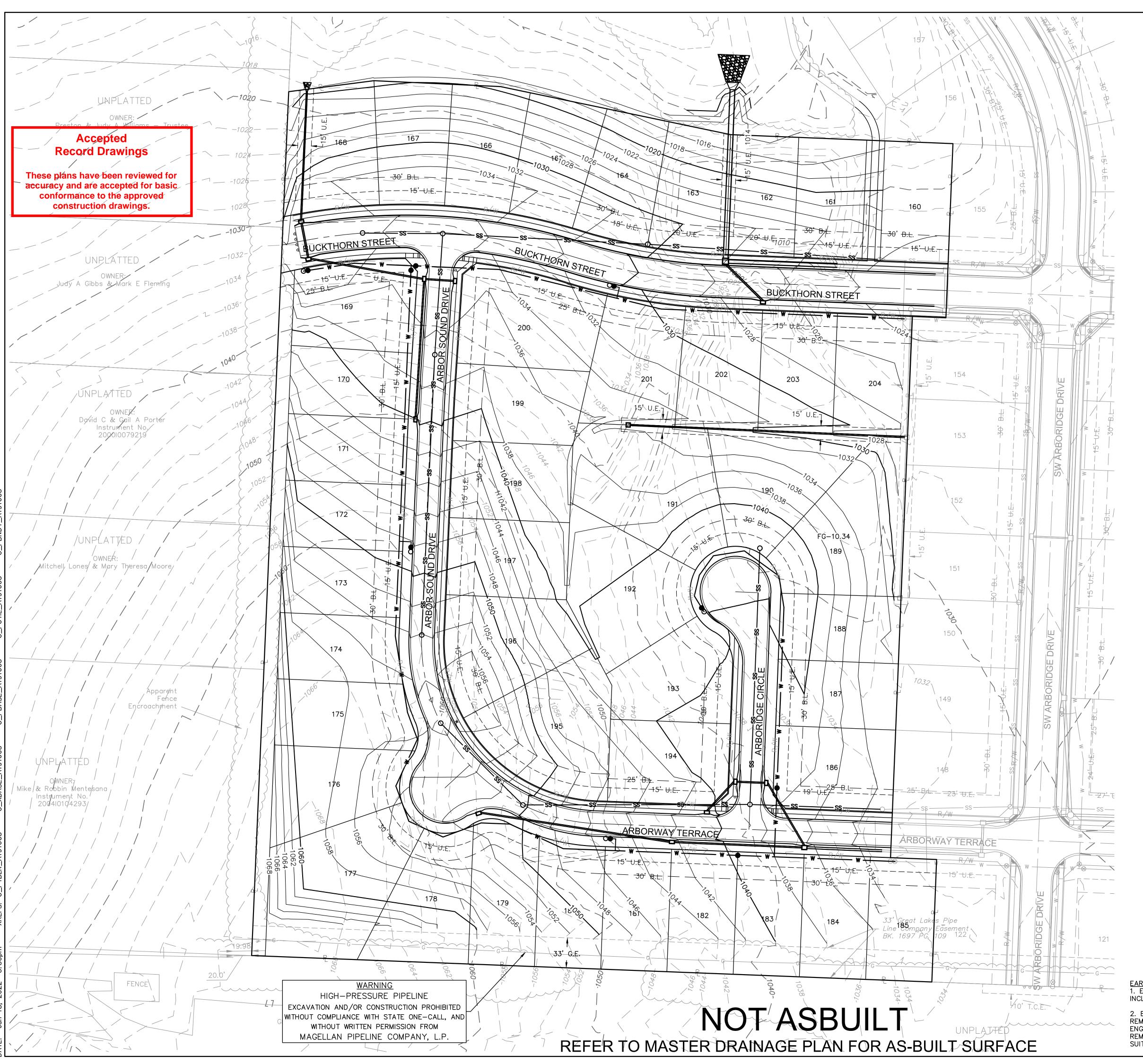
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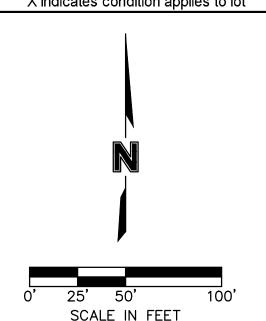
GENERAL NOTES:

1. CONTRACTOR SHALL ADHERE TO THE "DESIGN AND CONSTRUCTION MANUAL" SECTION 2100 AS ADOPTED BY THE CITY OF LEE'S SUMMIT (LATEST EDITION), FOR EXCAVATION AND EMBANKMENT WORK WITHIN THE PROPOSED RIGHT—OF—WAY.

2. AREAS OF CONSTRUCTION SHALL BE STRIPPED OF ALL VEGETATION, ORGANIC MATTER AND TOPSOIL TO A DEPTH AS RECOMMENDED BY GEOTECHNICAL ENGINEER AND OR TESTING AGENCY. SOILS REMOVED DURING SITE STRIPPING SHOULD BE EVALUATED TO DETERMINE IF PORTIONS OF THE TOPSOIL STRATUM MAY BE UTILIZED AS STRUCTURAL FILL WITHIN PAVEMENT AREAS. ANY MATERIAL NOT DEEMED AS SUITABLE FILL MATERIAL BY THE GEOTECHNICAL ENGINEER AND OR TESTING AGENCY SHALL BE REMOVED FROM THE JOB SITE BY THE CONTRACTOR AT HIS EXPENSE.

3. ALL EMBANKMENT OUTSIDE OF RIGHT-OF-WAY SHOULD BE PLACED IN CONTROLLED LIFTS HAVING A MAXIMUM LOOSE LIFT THICKNESS OF 8". EMBANKMENT SHOULD BE COMPACTED TO A MINIMUM OF 95% OF THE MATERIALS MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-698 (STANDARD PROCTOR COMPACTION). MOISTURE CONTENT OF THE FILL AT THE TIME OF COMPACTION SHALL BE WITHIN A RANGE OF -0 TO +4 PERCENT OF OPTIMUM MOISTURE CONTENT.

	LOT FILL INF		T
LOT NUMBER	MAX DEPTH OF FILL (OVER 2' PLACED)	FILL PLACED ON EXISTING SLOPES > 5:1	PROPOSED SLOPES > 3:
160			
161	17.3	X	
162	19.5	X	
163	20.4	X	
164	12.5	Х	
165	10.2		
166	10.3		
167	10.1		
168	9.1		
169			
170			
171			
172			
173			
174			
175			
176			
177			
178			
179			
180			
181			
182			
183			
184			
185			
186	3.9	Х	
187	7.3	Х	
188	10.8	Х	
189	12.9	Х	
190	13.4		
191	11.0		
192	6.1		
193	4.9		
194			
195	3.6		
196	3.6		
197			
198			
199			
200			
201		Х	
202	6.8	X	
203	6.8	X	
204	5.8	Х	
	X Indicates condit		



	30/122 111 1 221
	LEGEND
100	EXISTING INDEX CONTOURS
100	EXISTING INTERMEDIATE CONTOURS
<u>—100—</u>	PROPOSED INDEX CONTOURS
—100—	PROPOSED INTERMEDIATE CONTOURS

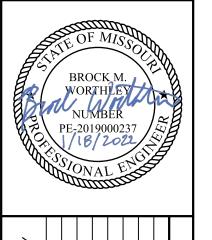
EARTHWORK QUANTITIES								
LOCATION CUT (C.Y.) FILL (C.Y.)								
STREET	10,423	8,630						
SITE	50,641	44,871						
TOTAL	61,064	53,501						

EARTHWORK QUANTITIES NOTES:

1. EARTHWORK QUANTITIES BASED ON FINISHED GRADE SURFACE AND DO NOT INCLUDE ADJUSTMENTS FOR TOPSOIL AND SHRINKAGE.

2. EARTHWORK QUANTITIES DO NOT TAKE INTO CONSIDERATION EXCAVATION, REMOVAL AND DISPOSAL OF MATERIAL DEEMED UNSUITABLE BY A GEOTECHNICAL ENGINEER. THE EARTHWORK CONTRACTOR IS RESPONSIBLE FOR EXCAVATION, REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL AND FOR REPLACING IT WITH SUITABLE MATERIAL.

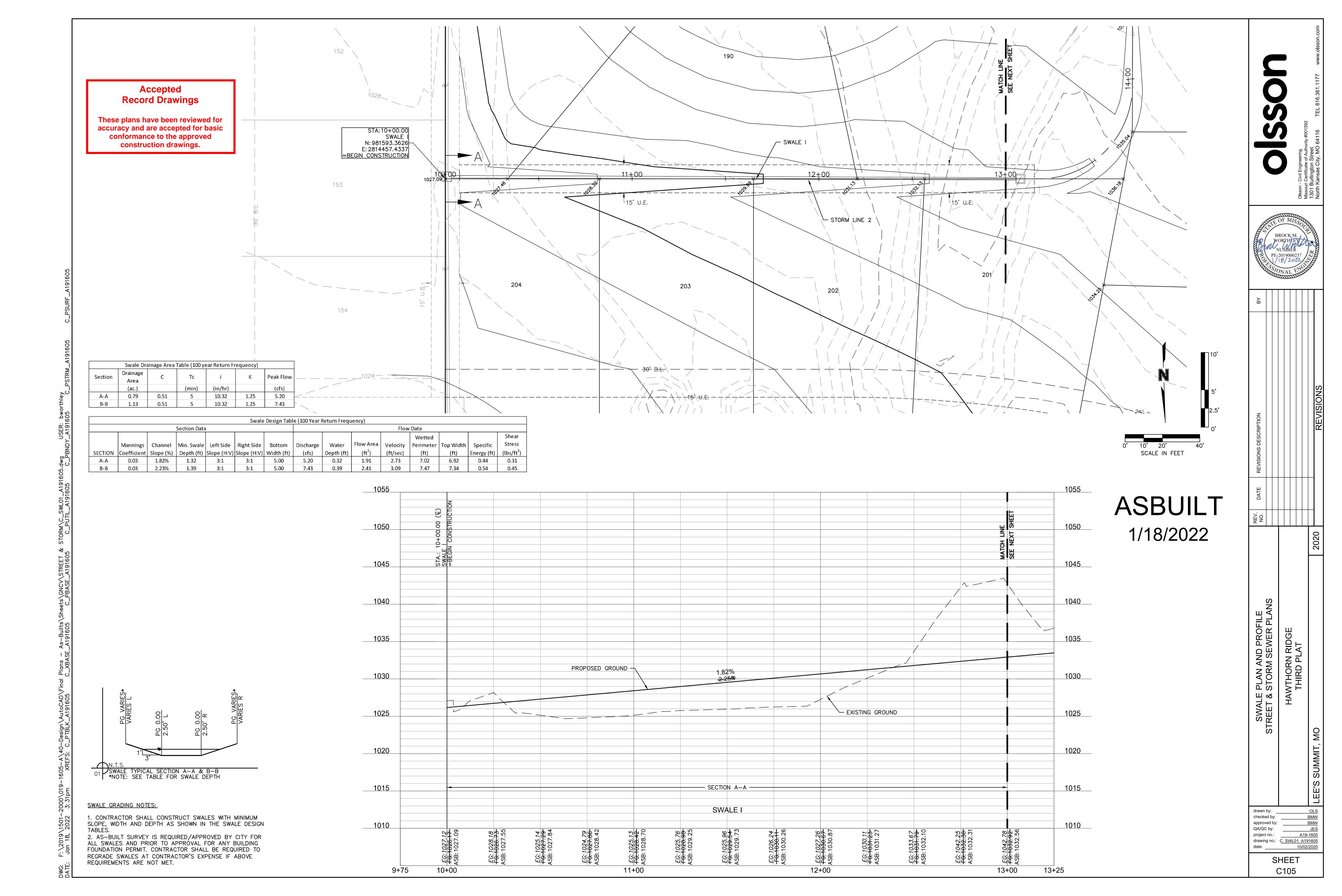
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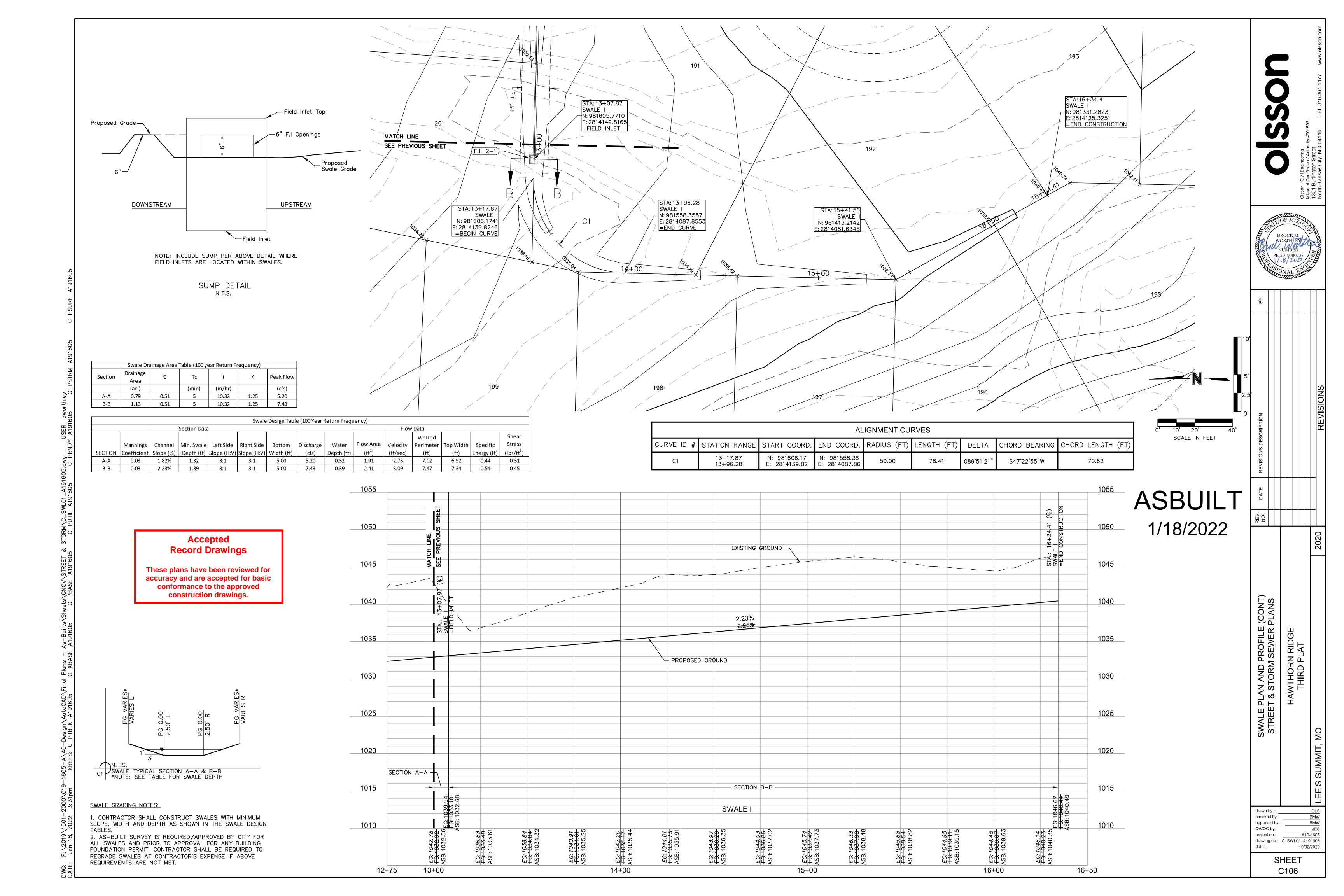


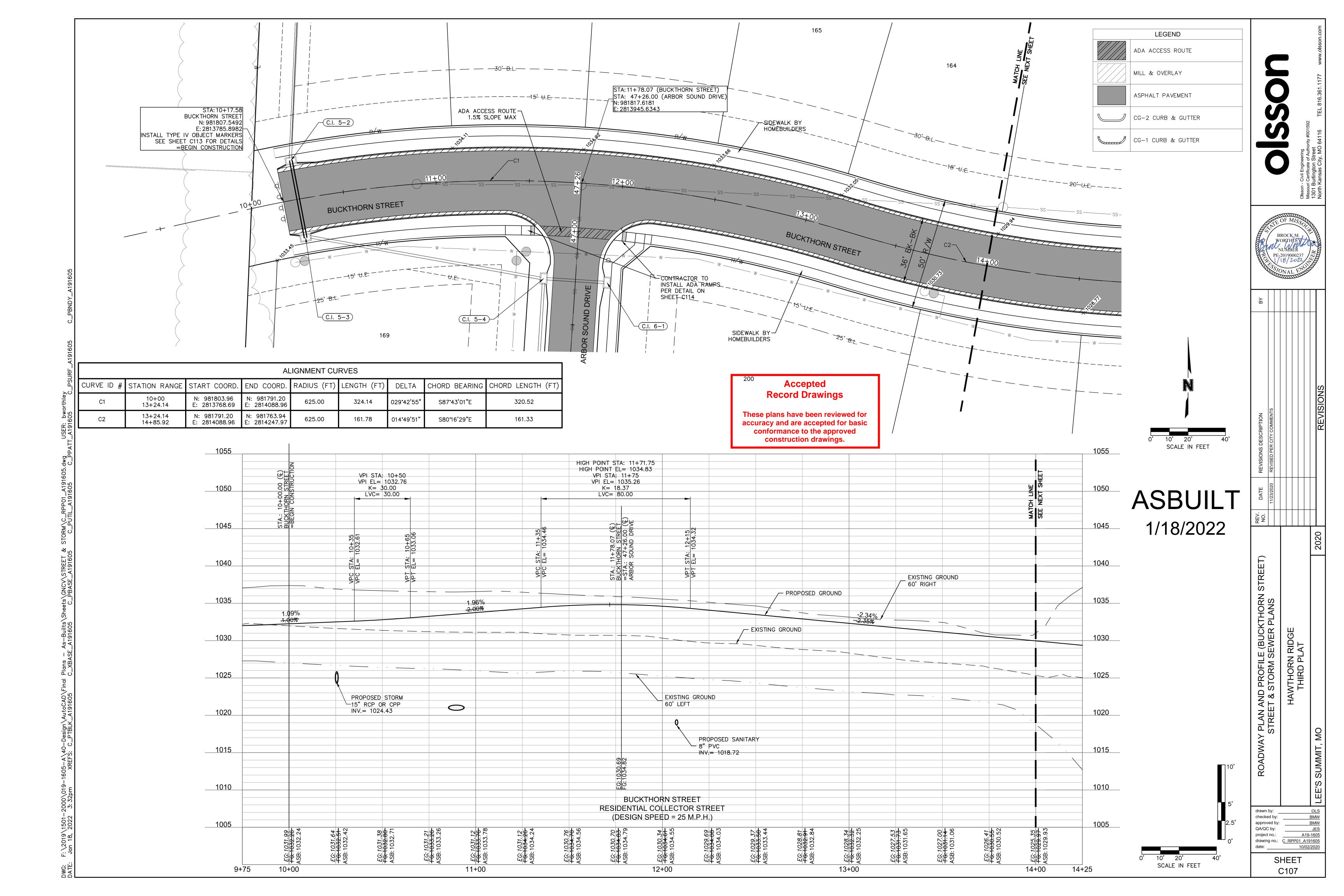
E REVISIONS DESCRIPTION						REVISIONS
DATE						
REV.						
GRADING PLAN (FOR REFERENCE ONLY)	STREET & STORM SEWER PLANS		HAWTHORN RIDGE	THIRD PLAT		III, MO

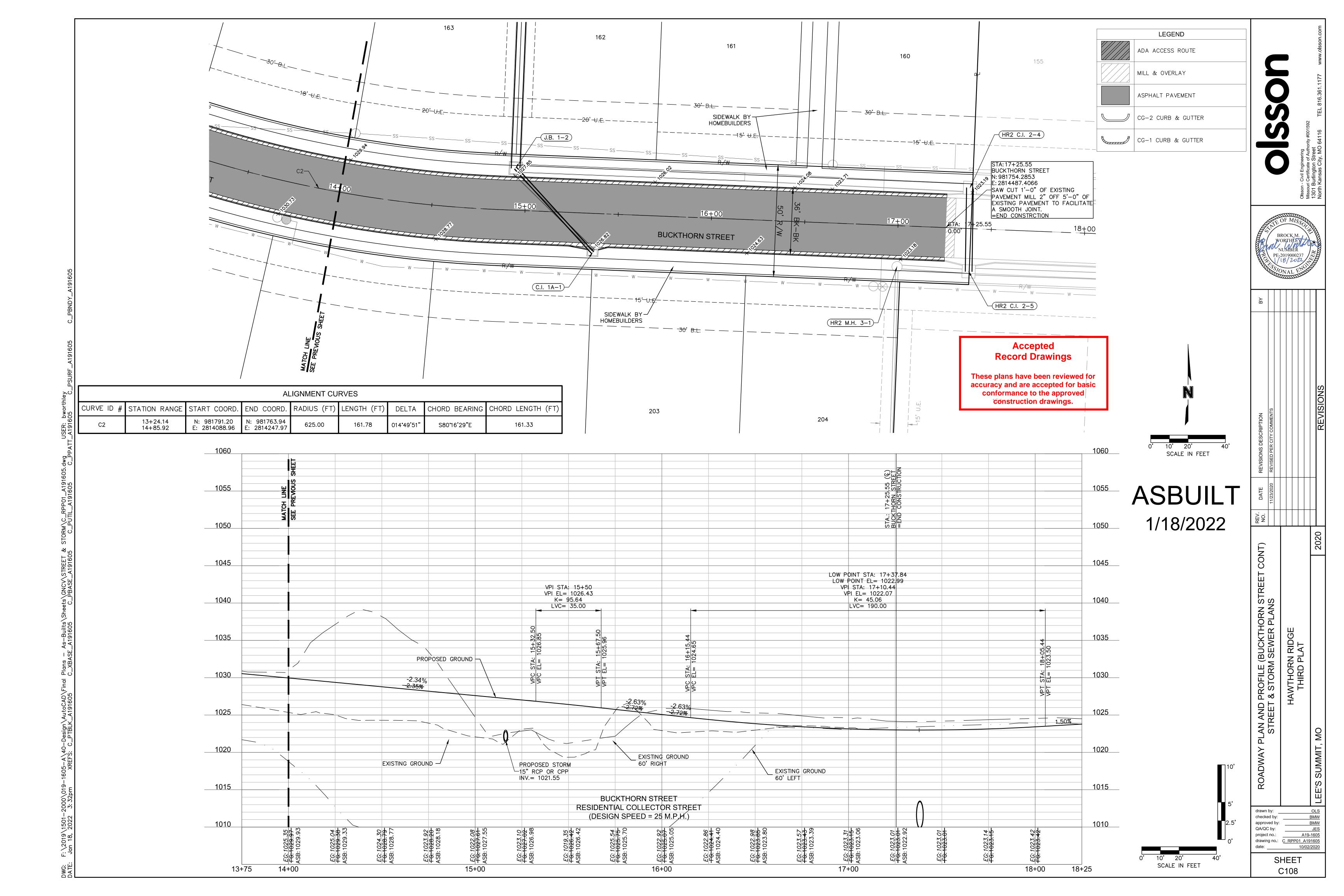
drawn by: OLS
checked by: BMW
approved by: BMW
QA/QC by: JES
project no.: A19-1605
drawing no.: C GRD01 A191605
date: 10/02/2020

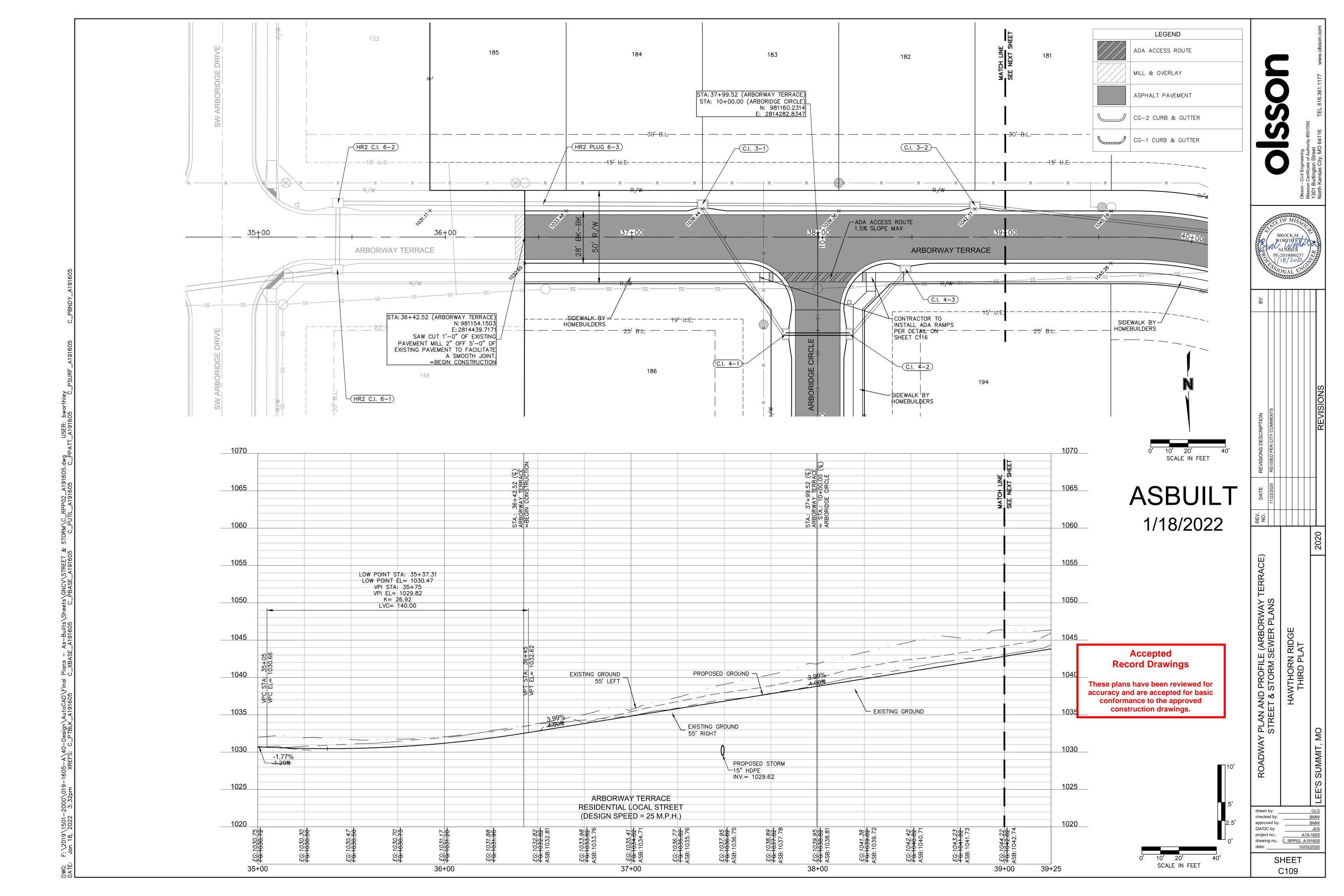
SHEET C104

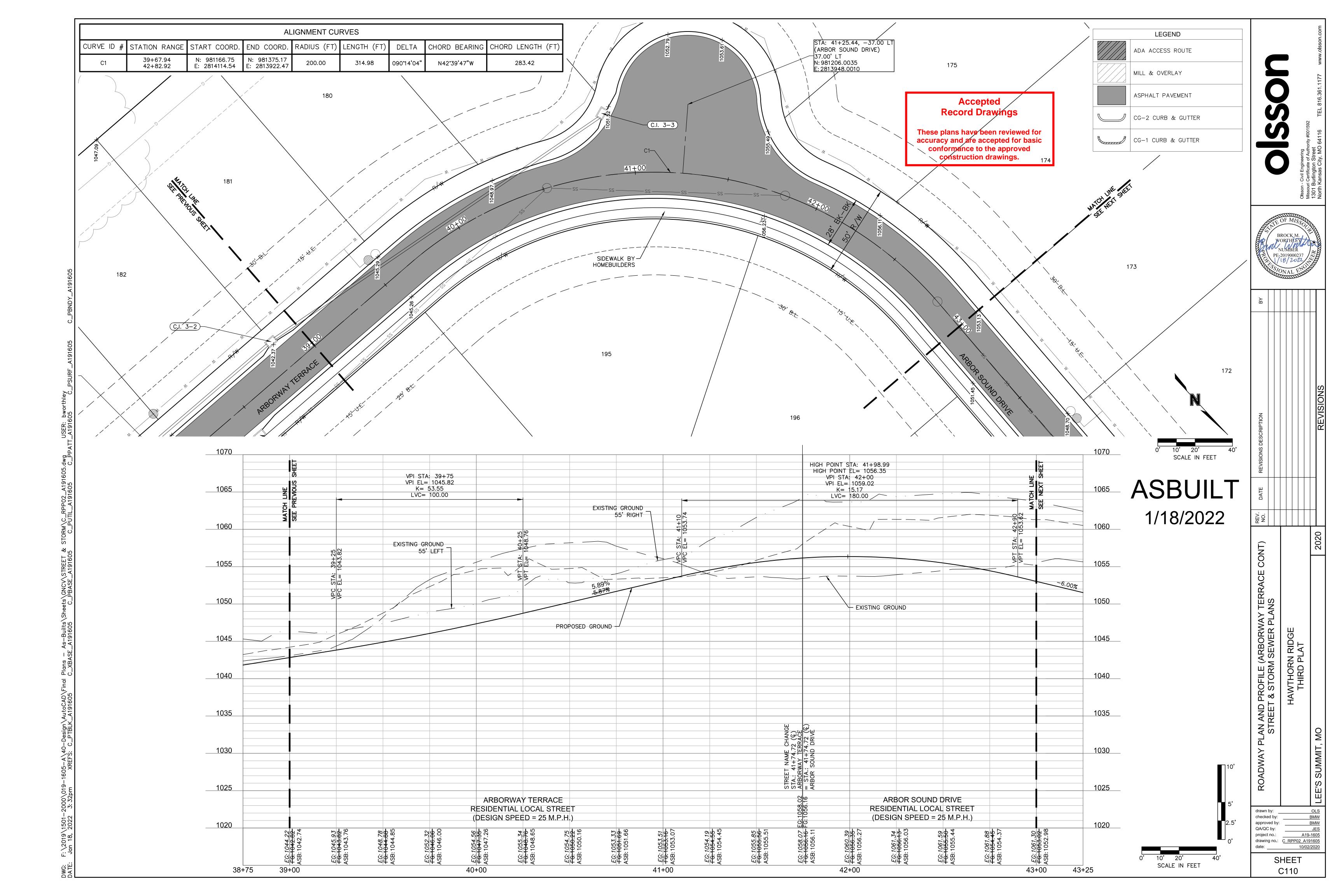


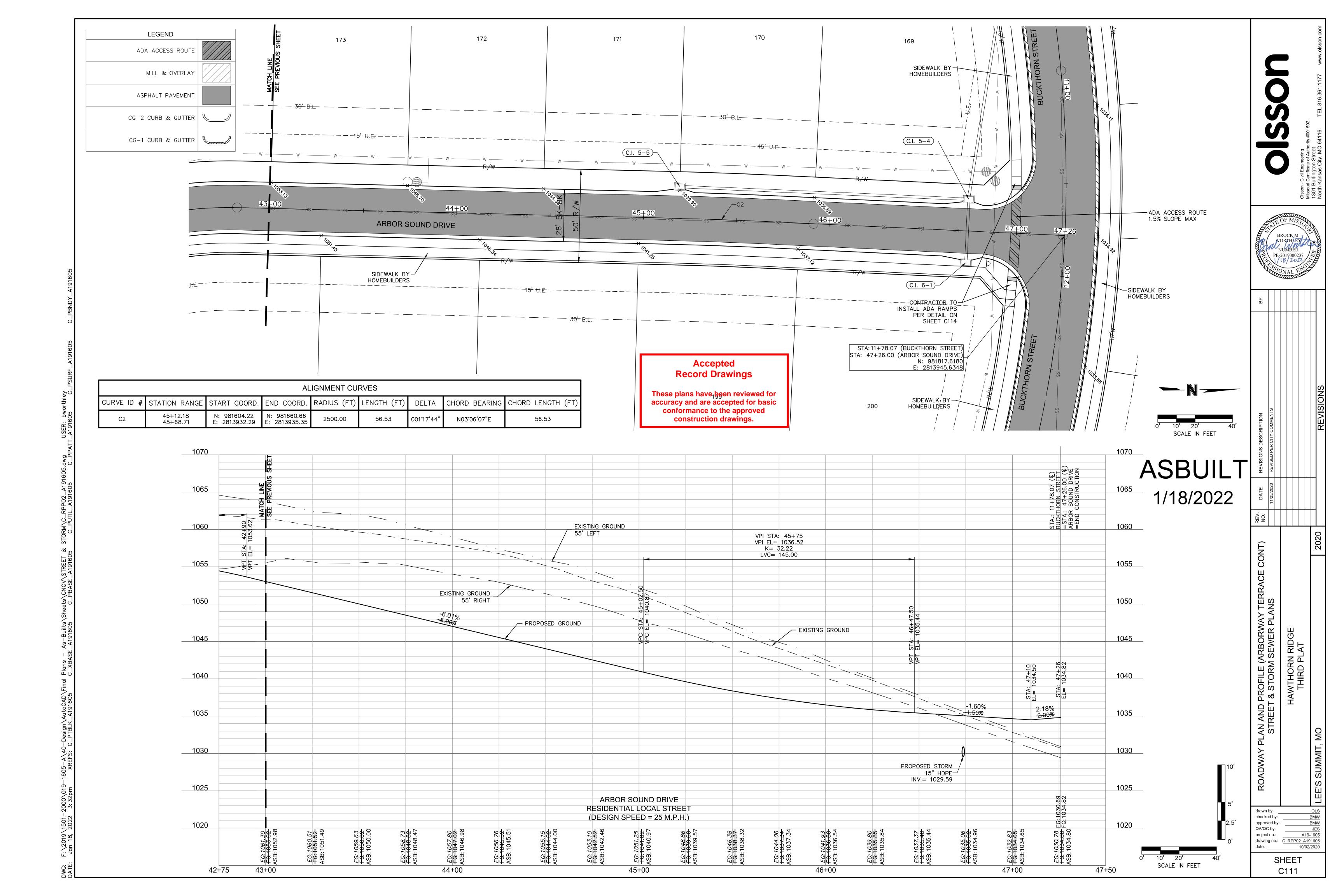


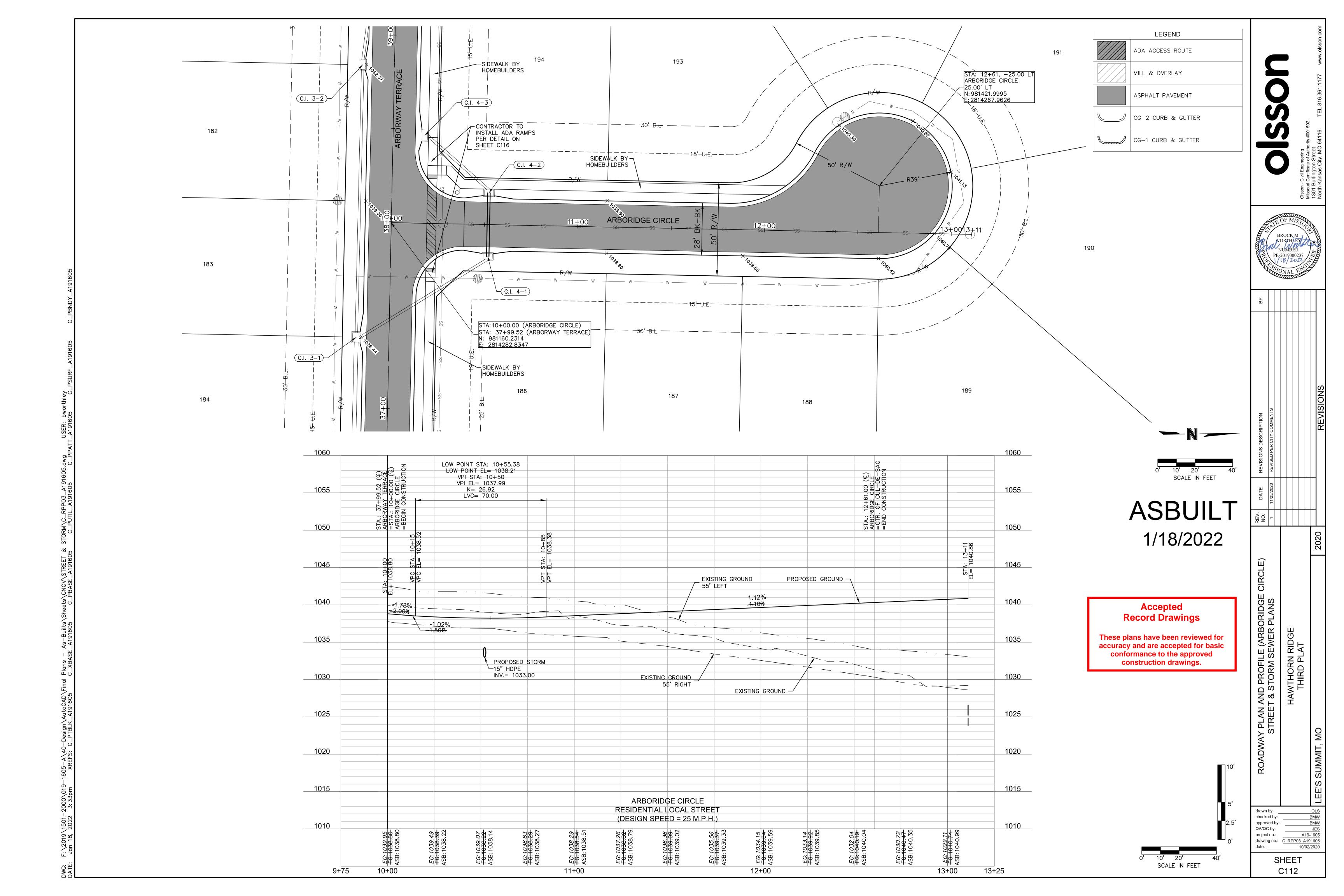


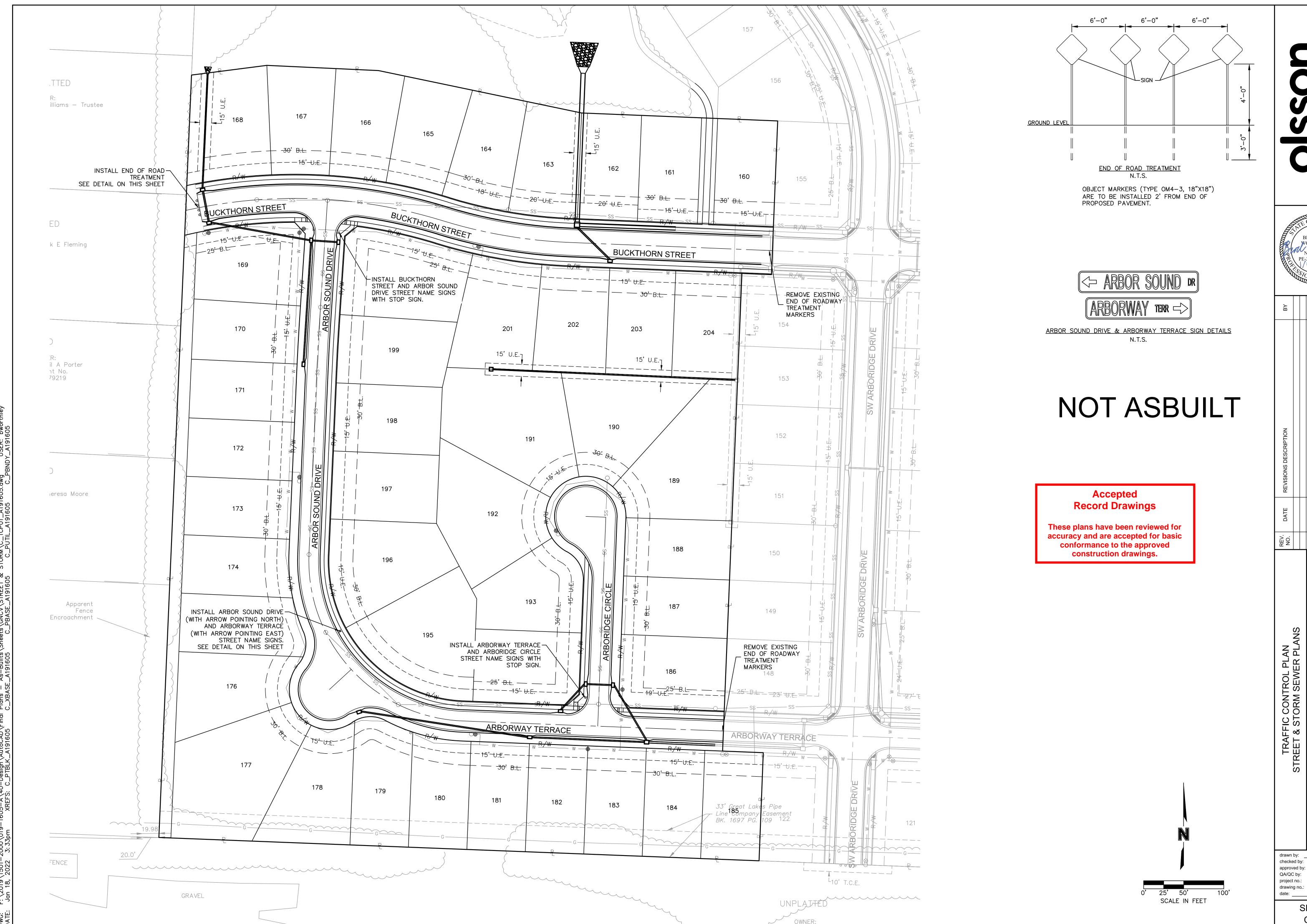












WTHORN RIDGE THIRD PLAT

 QA/QC by:
 JES

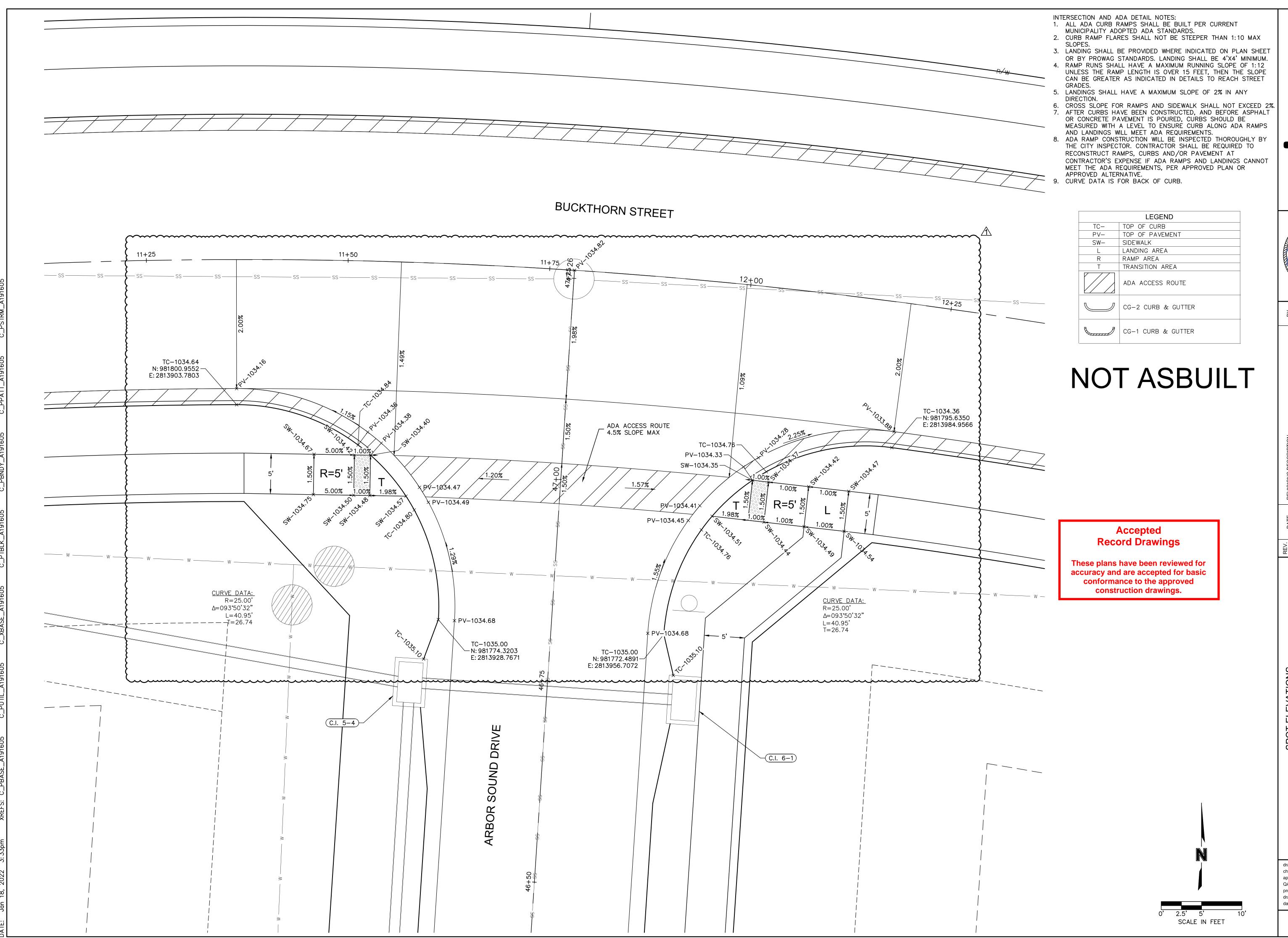
 project no.:
 A19-1605

 drawing no.:
 C TCP01 A191605

 date:
 10/02/2020

SHEET

C113

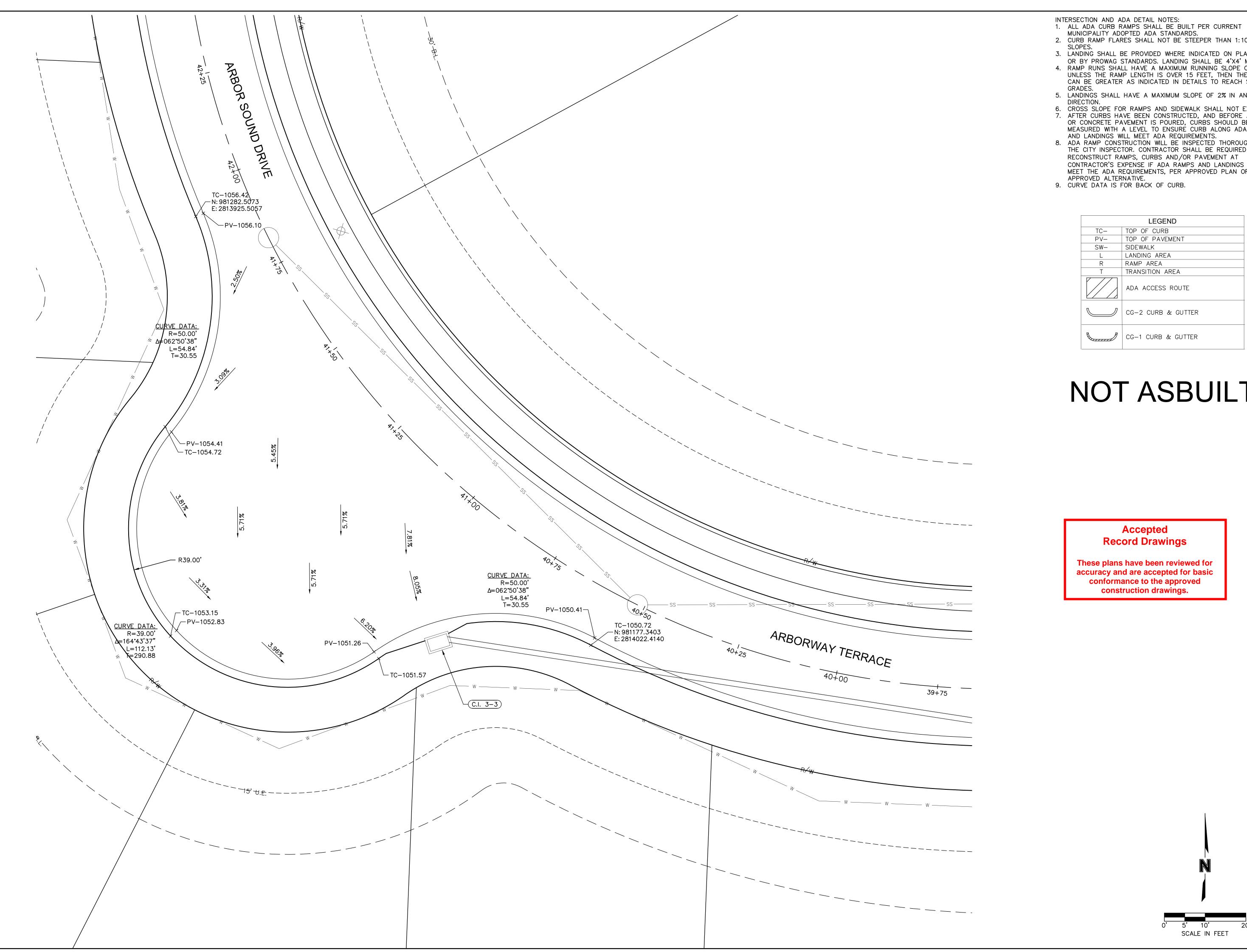


SPOT ELEVATIONS ET & STORM SEWER

drawn by: checked by: QA/QC by: JES
project no.: A19-1605
drawing no.: C SPT01 A191605
date: 10/02/2020 QA/QC by:

SHEET

C114



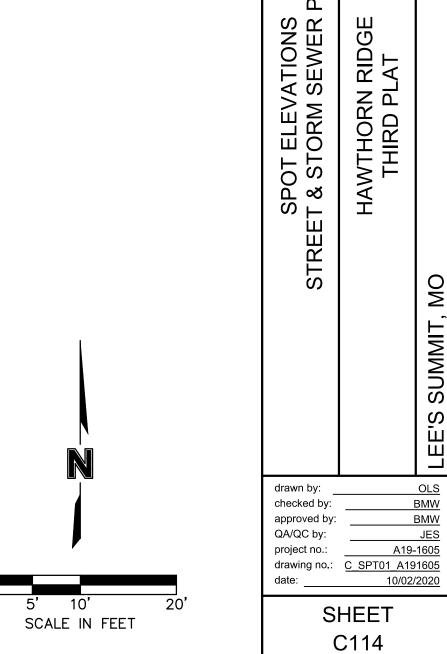
- 2. CURB RAMP FLARES SHALL NOT BE STEEPER THAN 1:10 MAX
- 3. LANDING SHALL BE PROVIDED WHERE INDICATED ON PLAN SHEET OR BY PROWAG STANDARDS. LANDING SHALL BE 4'X4' MINIMUM. 4. RAMP RUNS SHALL HAVE A MAXIMUM RUNNING SLOPE OF 1:12 UNLESS THE RAMP LENGTH IS OVER 15 FEET, THEN THE SLOPE CAN BE GREATER AS INDICATED IN DETAILS TO REACH STREET
- 5. LANDINGS SHALL HAVE A MAXIMUM SLOPE OF 2% IN ANY
- 6. CROSS SLOPE FOR RAMPS AND SIDEWALK SHALL NOT EXCEED 2%. 7. AFTER CURBS HAVE BEEN CONSTRUCTED, AND BEFORE ASPHALT OR CONCRETE PAVEMENT IS POURED, CURBS SHOULD BE MEASURED WITH A LEVEL TO ENSURE CURB ALONG ADA RAMPS
- AND LANDINGS WILL MEET ADA REQUIREMENTS. 8. ADA RAMP CONSTRUCTION WILL BE INSPECTED THOROUGHLY BY THE CITY INSPECTOR. CONTRACTOR SHALL BE REQUIRED TO RECONSTRUCT RAMPS, CURBS AND/OR PAVEMENT AT
- CONTRACTOR'S EXPENSE IF ADA RAMPS AND LANDINGS CANNOT MEET THE ADA REQUIREMENTS, PER APPROVED PLAN OR APPROVED ALTERNATIVE.
- 9. CURVE DATA IS FOR BACK OF CURB.

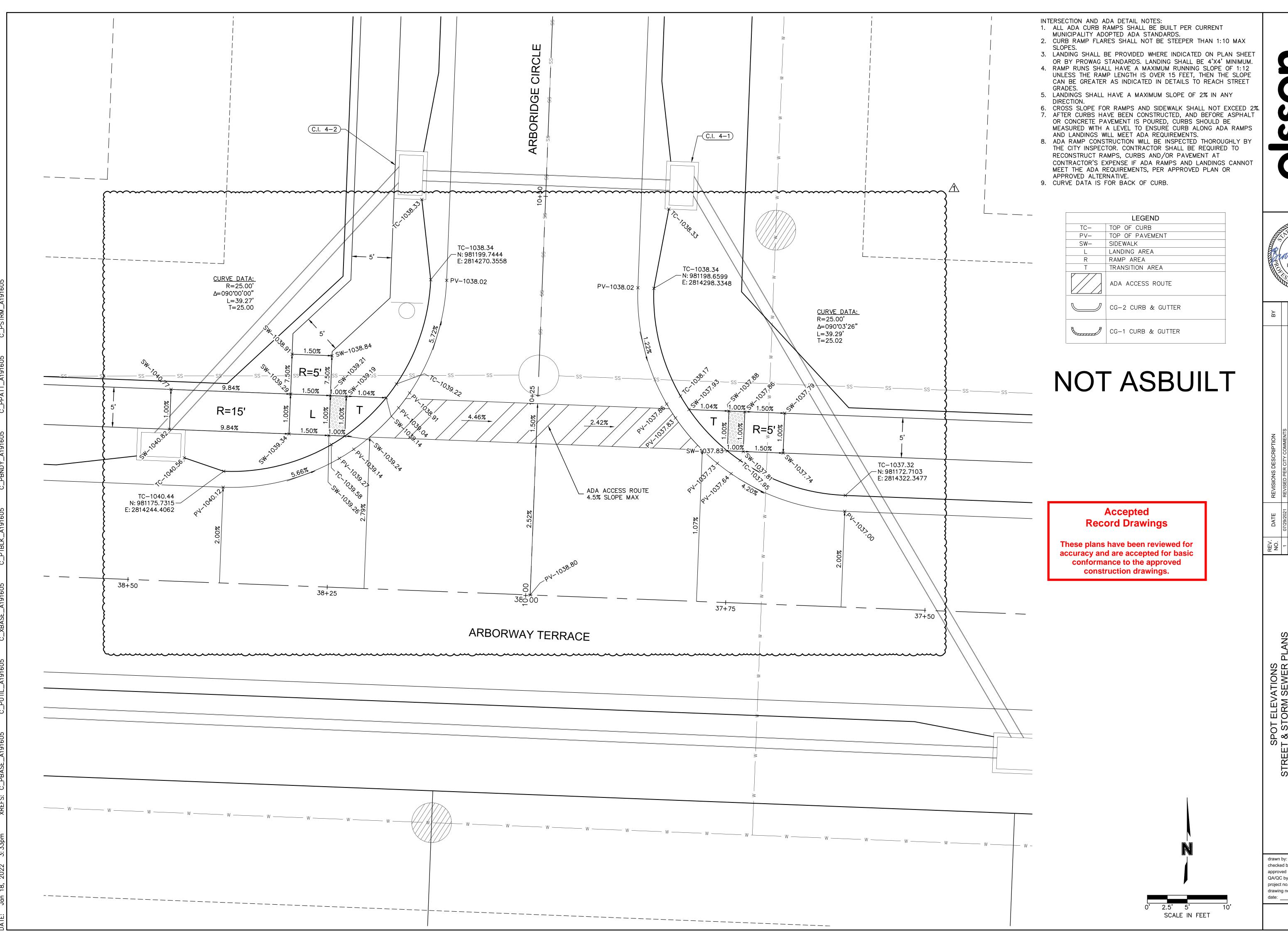
LEGEND							
TC-	TOP OF CURB						
PV-	TOP OF PAVEMENT						
SW-	SIDEWALK						
L	LANDING AREA						
R	RAMP AREA						
Т	TRANSITION AREA						
	ADA ACCESS ROUTE						
	CG-2 CURB & GUTTER						
Burnel .	CG-1 CURB & GUTTER						

NOT ASBUILT

Accepted Record Drawings

These plans have been reviewed for accuracy and are accepted for basic conformance to the approved construction drawings.

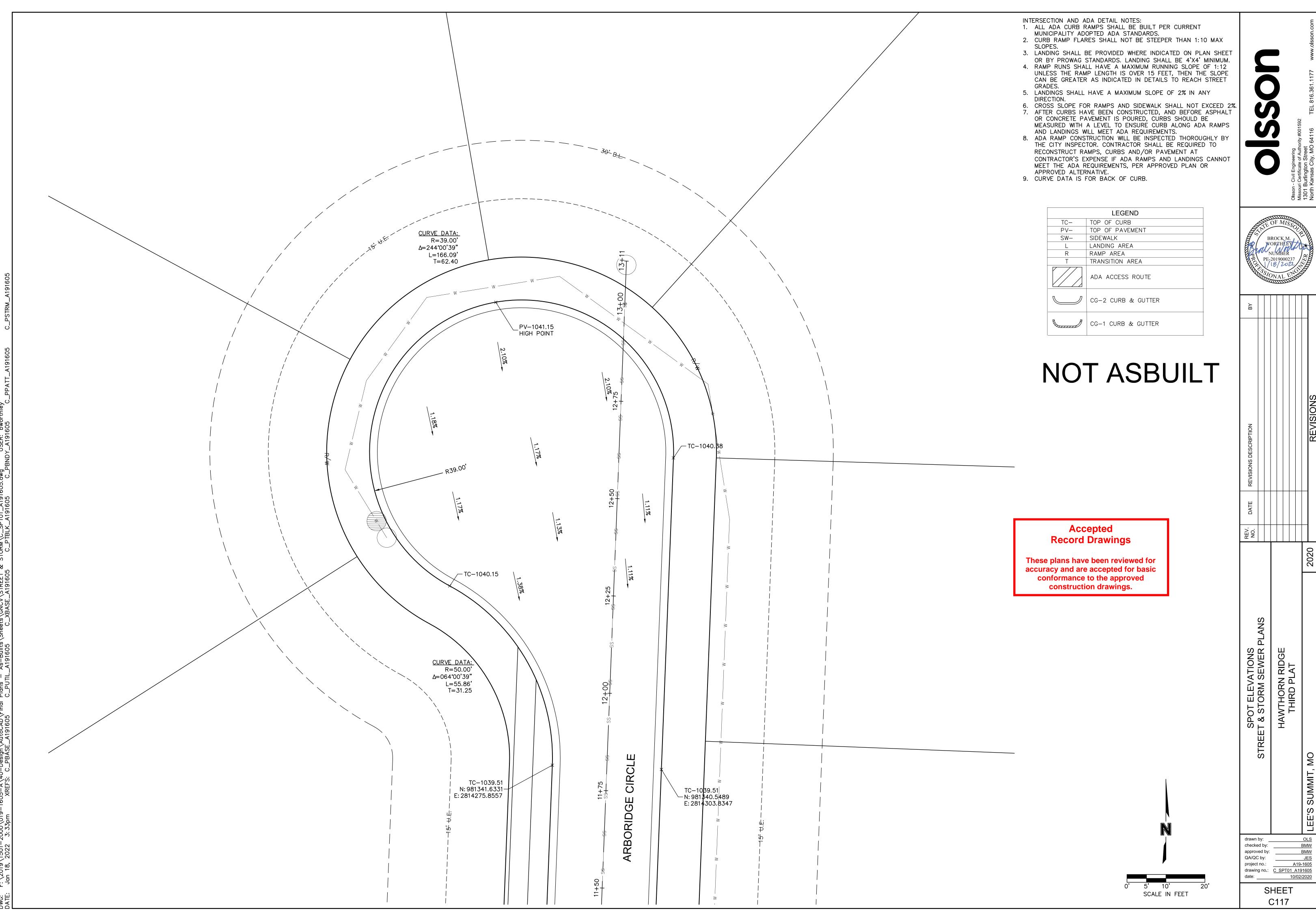


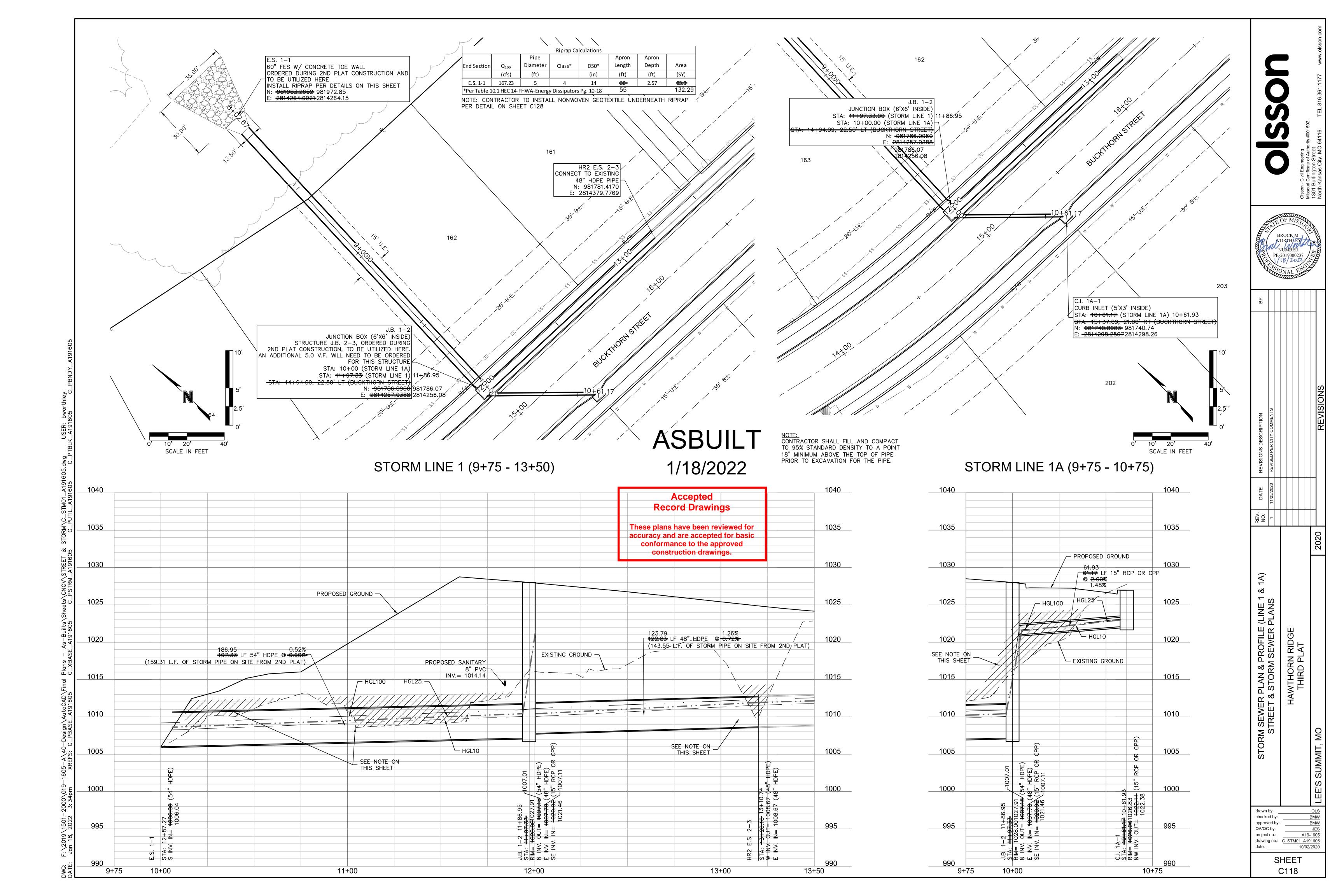


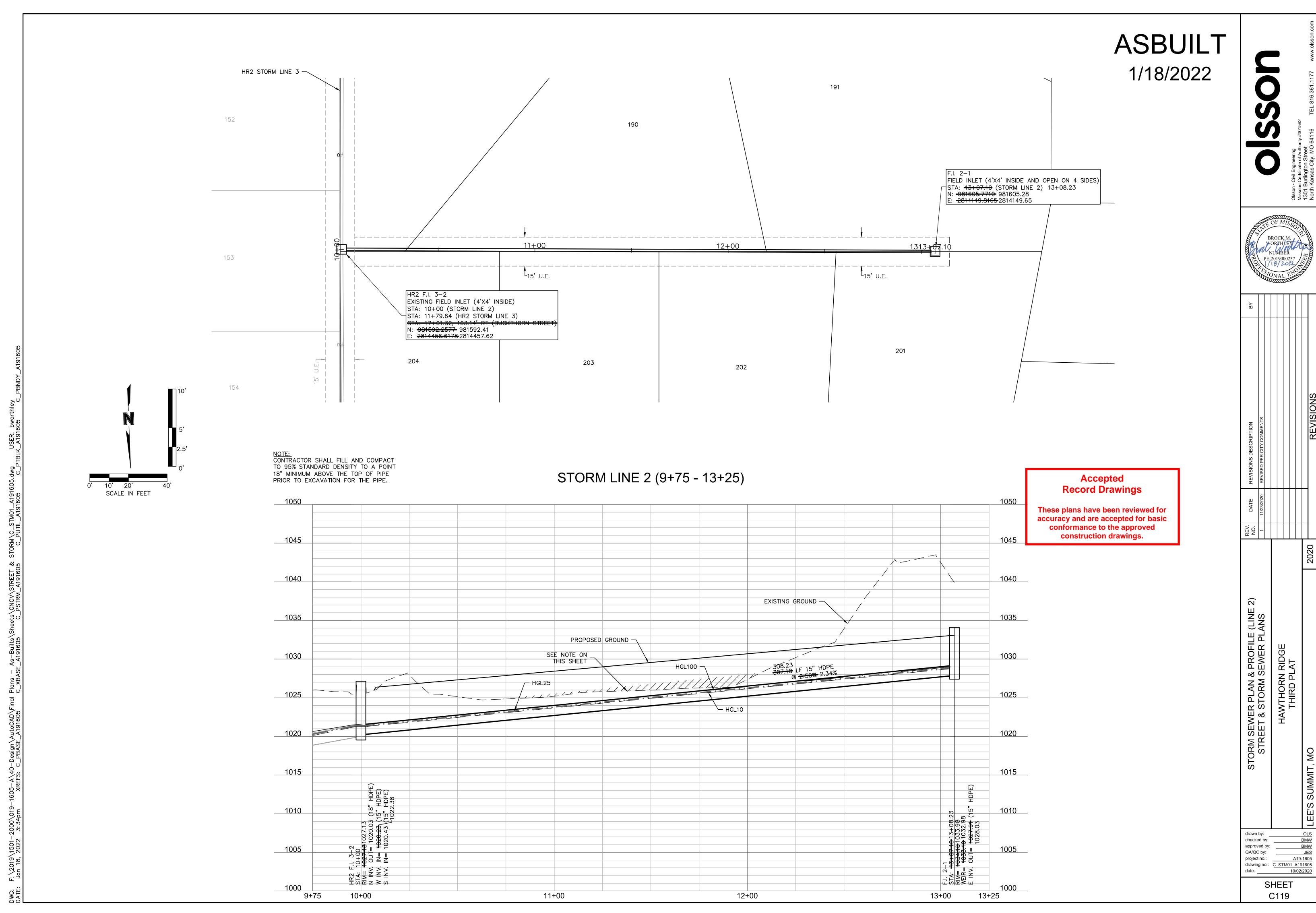
SPOT ELEVATIONS ET & STORM SEWER

checked by: approved by: QA/QC by: project no.: A19-1605
drawing no.: C SPT01 A191605
date: 10/02/2020

SHEET

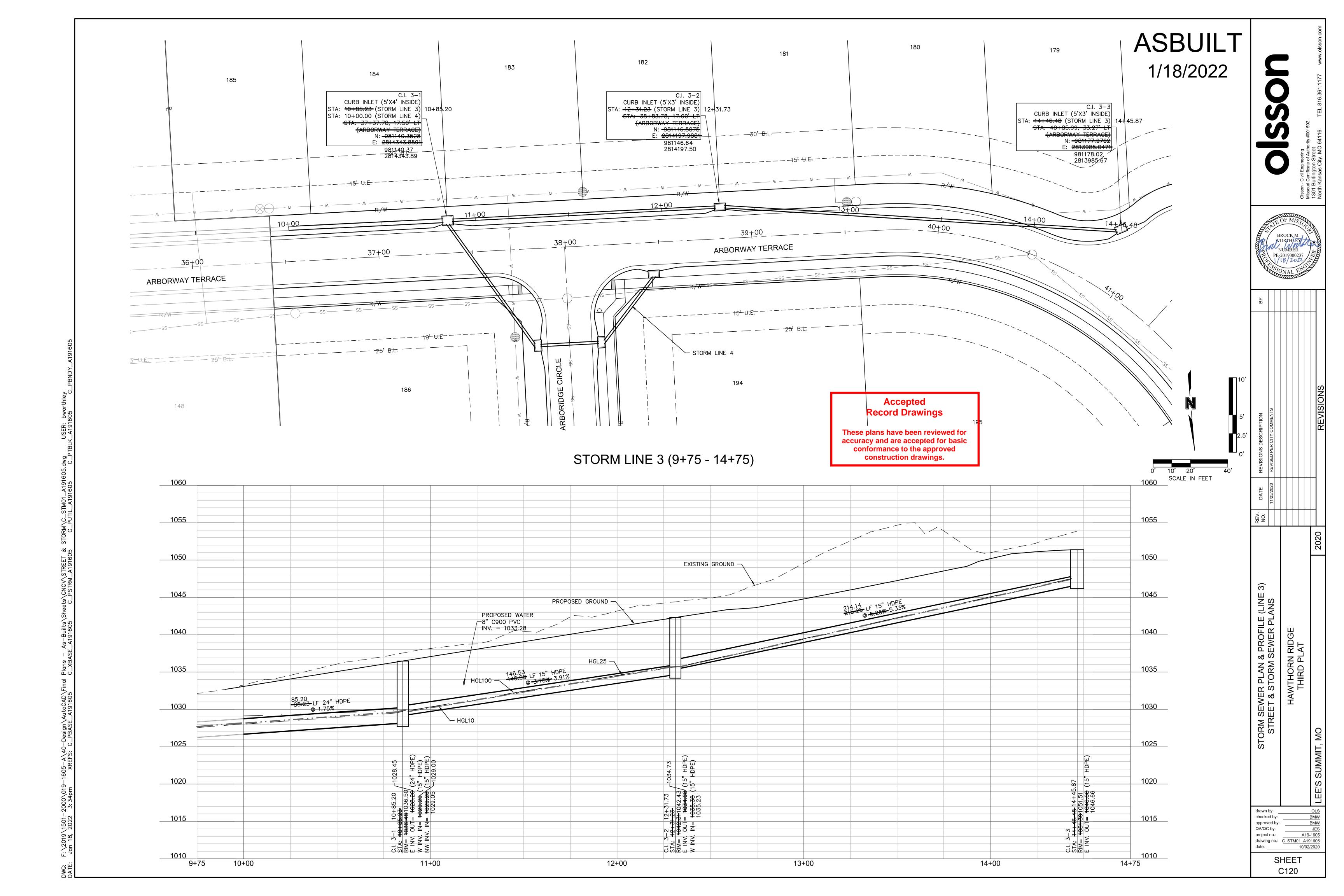


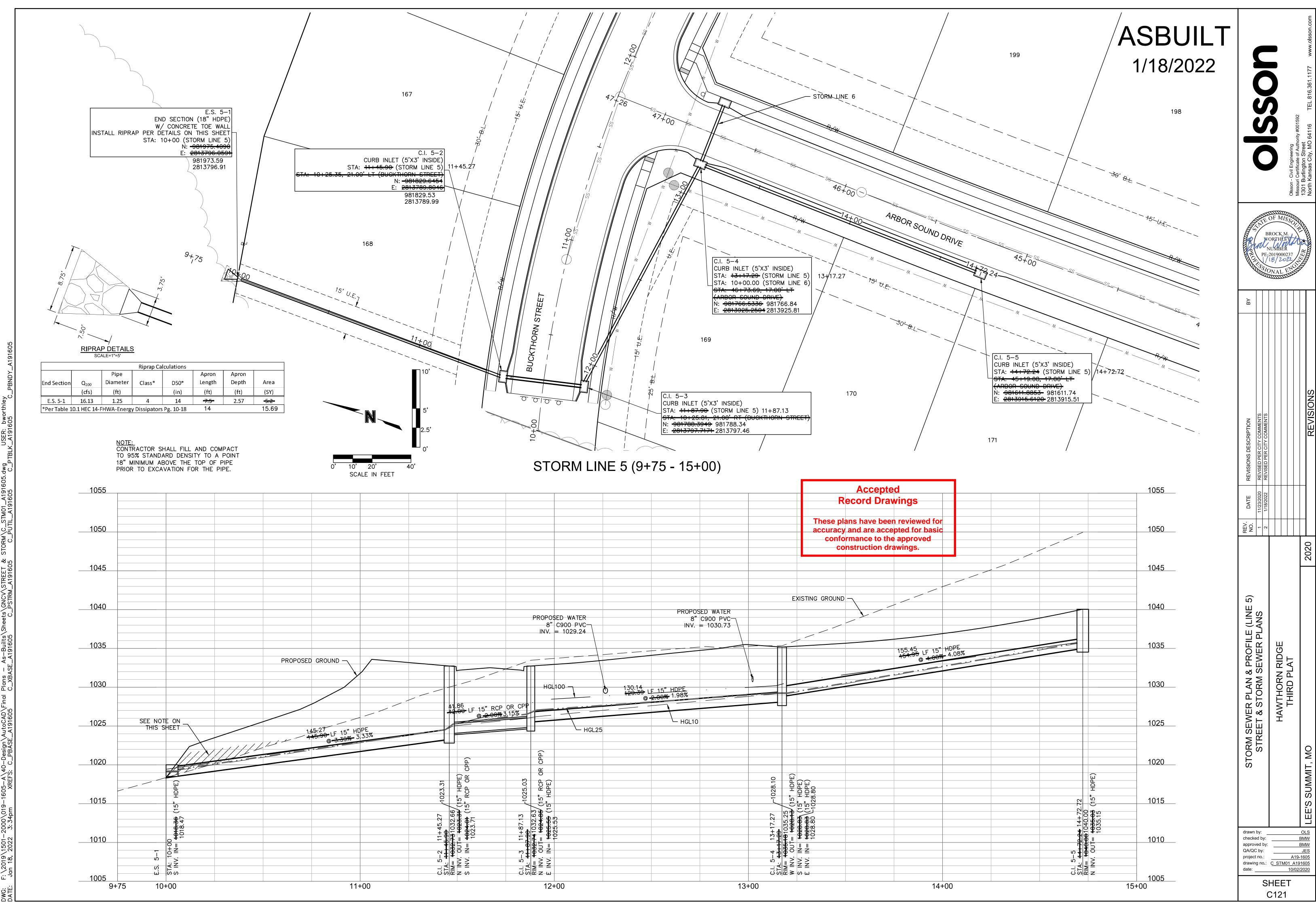




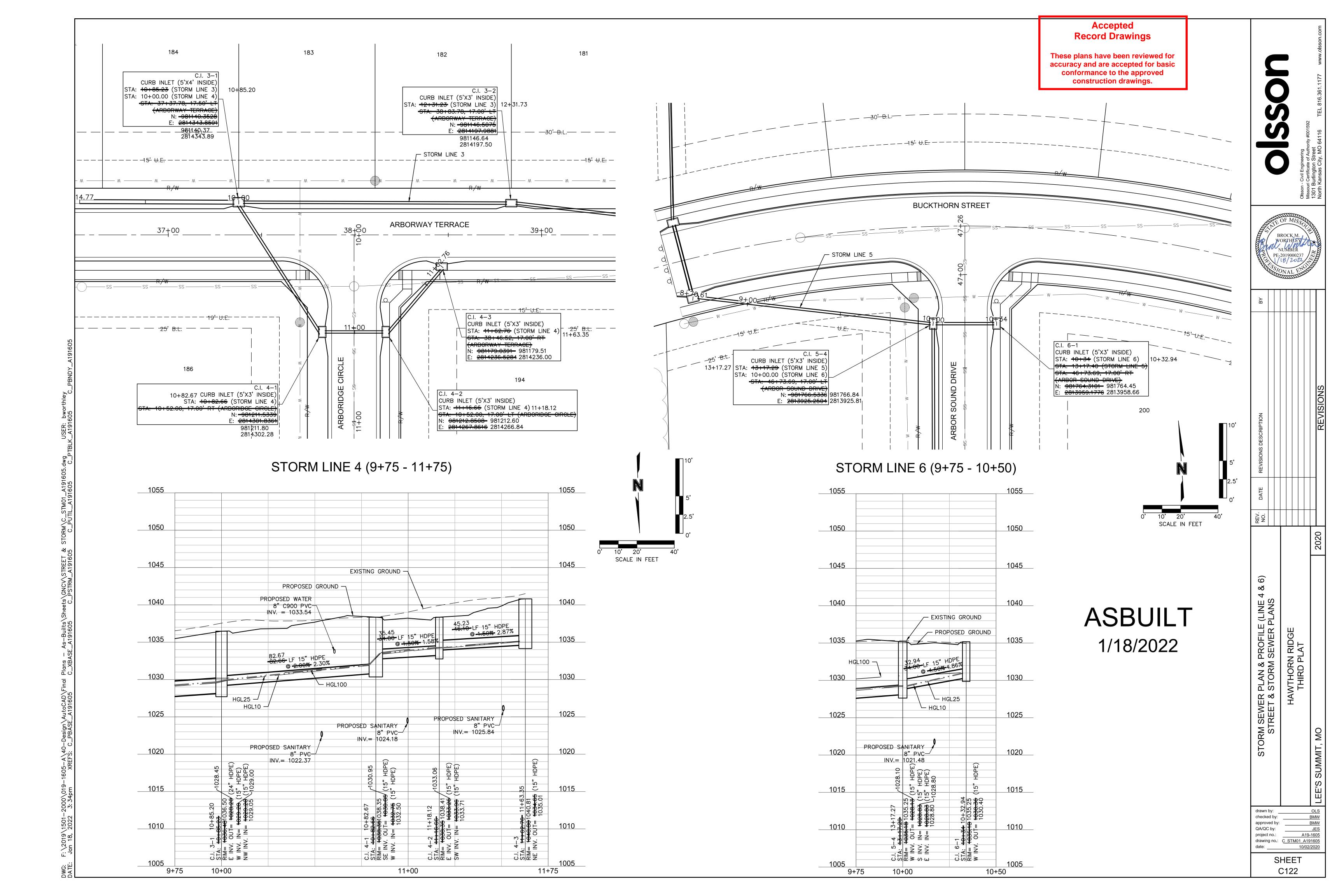


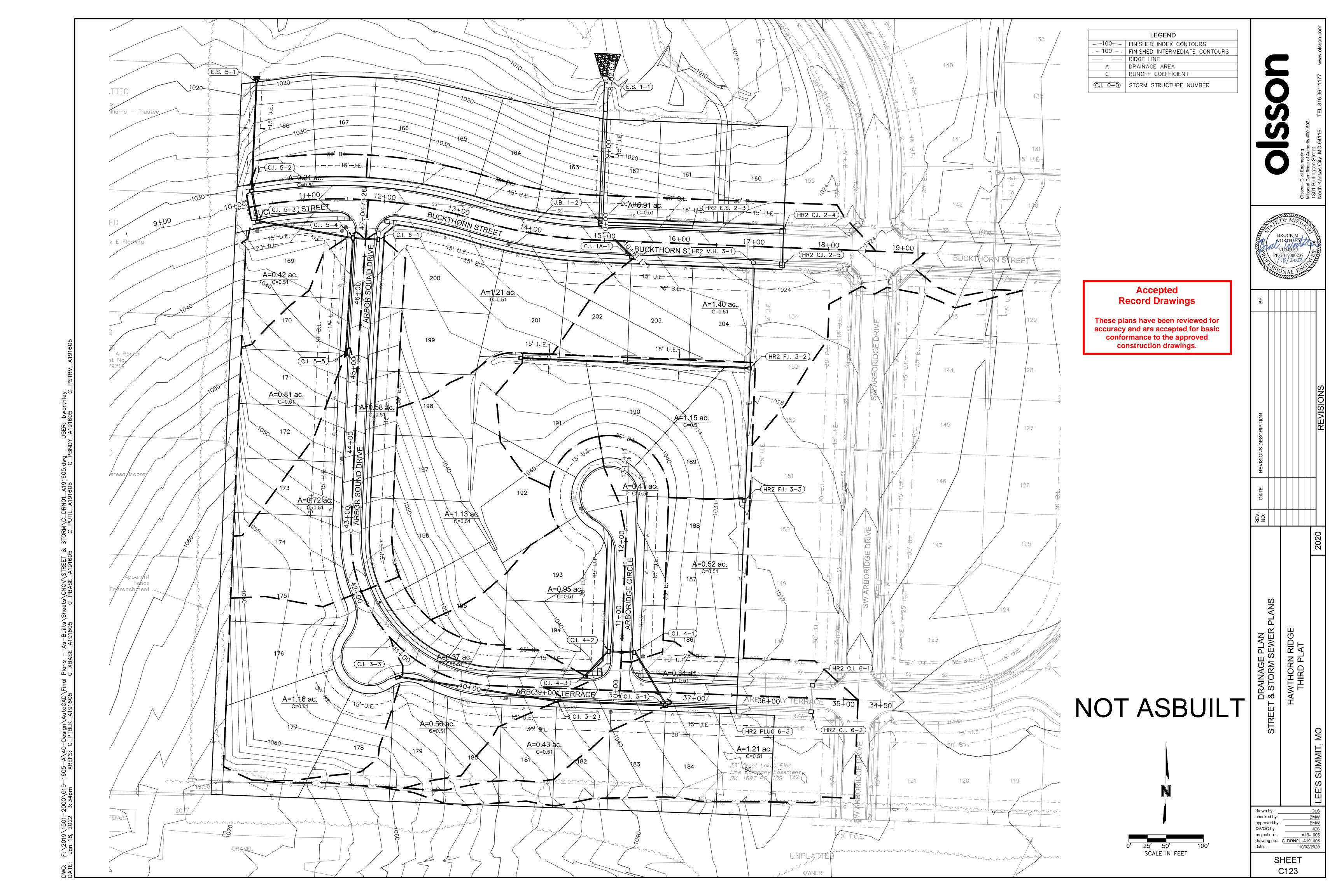
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DATE	REVISIONS DESCRIPTION	
11/23/2020	REVISED PER CITY COMMENTS	
	REVISIONS	
	NO. DATE 1 11/23/2020	DATE REVISIONS DESCI











Orainage Area [-				
10	Year Return F	-requency	Γ	1	I	I
Inlet ID	Drainage Area	С	Тс	i	К	Peak Flow
	(ac)		(min)	(in/hr)		(cfs)
C.I. 1A-1	1.21	0.51	5.00	7.35	1.00	4.54
F.I. 2-1	1.13	0.51	5.00	7.35	1.00	4.24
C.I. 3-1	0.43	0.51	5.00	7.35	1.00	1.61
C.I. 3-2	0.56	0.51	5.00	7.35	1.00	2.10
C.I. 3-3	1.16	0.51	5.00	7.35	1.00	4.35
C.I. 4-1(L)	0.32	0.51	5.00	7.35	1.00	1.20
C.I. 4-1(R)	0.00	0.51	5.00	7.35	1.00	0.00
C.I. 4-1(B)	0.09	0.51	5.00	7.35	1.00	0.34
C.I. 4-1	0.41	0.51	5.00	7.35	1.00	1.54
C.I. 4-2(L)	0.00	0.51	5.00	7.35	1.00	0.00
C.I. 4-2(R)	0.38	0.51	5.00	7.35	1.00	1.43
C.I. 4-2(B)	0.57	0.51	5.00	7.35	1.00	2.14
C.I. 4-2	0.95	0.51	5.00	7.35	1.00	3.56
C.I. 4-3	0.37	0.51	5.00	7.35	1.00	1.39
C.I. 5-2	0.21	0.51	5.00	7.35	1.00	0.79
C.I. 5-3	0.42	0.51	5.00	7.35	1.00	1.58
C.I. 5-4	0.81	0.51	5.00	7.35	1.00	3.04
C.I. 5-5	0.72	0.51	5.00	7.35	1.00	2.70
C.I. 6-1	0.58	0.51	5.00	7.35	1.00	2.18
HR2 F.I. 3-2	1.15	0.51	5.00	7.35	1.00	4.31
HR2 F.I. 3-3	0.52	0.51	5.00	7.35	1.00	1.95

Inlet Design Ta													
10	Year Return	Frequency											
	Inlet		Upstream		Clogging	Inlet	Sag Inlet	Captured		Inlet	Gutter	Gutter	Ponding
Inlet ID	Location	Peak Flow	Bypass	Total Flow	Factor	Capacity	Capacity	Flow	Bypass Flow	Efficiency	Depth	Spread	Depth
							(Note 1)			(Note 2)			
		(cfs)	(cfs)	(cfs)		(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
C.I. 1A-1	GRADE	4.54	0.35	4.89	1.00	3.82	3.82	3.82	1.07	78.16%	0.21	9.77	•••
F.I. 2-1	SAG	4.24	0.00	4.24	0.80	18.67	14.93	4.24	0.00	100.00%	•••		0.19
C.I. 3-1	GRADE	1.61	1.41	3.02	1.00	1.87	1.87	1.87	1.15	62.03%	0.16	7.78	•••
C.I. 3-2	GRADE	2.10	1.31	3.41	1.00	2.00	2.00	2.00	1.41	58.64%	0.16	8.14	•••
C.I. 3-3	GRADE	4.35	0.00	4.35	1.00	3.04	3.04	3.04	1.31	69.90%	0.21	10.67	
C.I. 4-1(L)	SAG	1.20									0.14	7.01	
C.I. 4-1(R)	SAG	0.00	•••			•••	•••		•••	•••	0.00	0.00	•••
C.I. 4-1(B)	SAG	0.34								•••	•••	•••	
C.I. 4-1	SAG	1.54	0.00	1.54	0.80	19.40	15.52	1.54	0.00	100.00%	•••		
C.I. 4-2(L)	SAG	0.00	•••			•••	•••		•••	•••	0.09	4.34	•••
C.I. 4-2(R)	SAG	1.43	•••	•••	•••	•••	•••	•••	•••	•••	0.15	7.48	***
C.I. 4-2(B)	SAG	2.14				•••				•••	•••	•••	•••
C.I. 4-2	SAG	3.56	0.33	3.90	0.80	19.40	15.52	3.90	0.00	100.00%	•••		
C.I. 4-3	GRADE	1.39	0.00	1.39	1.00	1.05	1.05	1.05	0.33	76.01%	0.11	5.51	
C.I. 5-2	GRADE	0.79	0.00	0.79	1.00	0.79	0.79	0.79	0.00	99.69%	0.13	5.83	***
C.I. 5-3	GRADE	1.58	0.90	2.47	1.00	2.37	2.37	2.37	0.10	95.95%	0.19	8.86	•••
C.I. 5-4	GRADE	3.04	0.53	3.57	1.00	2.67	2.67	2.67	0.90	74.83%	0.20	9.96	
C.I. 5-5	GRADE	2.70	0.00	2.70	1.00	2.17	2.17	2.17	0.53	80.31%	0.18	8.97	
C.I. 6-1	GRADE	2.18	0.00	2.18	1.00	1.82	1.82	1.82	0.35	83.89%	0.17	8.27	***
HR2 F.I. 3-2	SAG	4.31	0.00	4.31	0.80	18.67	14.93	4.31	0.00	100.00%	•••	•••	0.19
HR2 F.I. 3-3	SAG	1.95	0.00	1.95	0.80	18.67	14.93	1.95	0.00	100.00%	•••		0.11
Notes:													
Inlet canacity	at car location	on has boon re	ducad by a d	logging factor	of 0.80 rodu	cing theoretic	al canacity to	80% canacity	as required n	or ADMA Soc	tion 5600		

1. Inlet capacity at sag location has been reduced by a clogging factor of 0.80, reducing theoretical capacity to 80% capacity, as required per APWA Section 5600. Both theoretical capacity and reduced capacity are shown.

2. Inlet efficiency shown in the tables is Captured Flow/Total Flow, denoting the actual percentage of flow captured after the capacity has been reduced to 80% of theoretical capacity.

25	Year Return F	requency				
Inlet ID	Drainage Area	С	Тс	i	K	Peak Flow
	(ac)		(min)	(in/hr)		(cfs)
C.I. 1A-1	1.21	0.51	5.00	8.53	1.10	5.79
F.I. 2-1	1.13	0.51	5.00	8.53	1.10	5.41
C.I. 3-1	0.43	0.51	5.00	8.53	1.10	2.06
C.I. 3-2	0.56	0.51	5.00	8.53	1.10	2.68
C.I. 3-3	1.16	0.51	5.00	8.53	1.10	5.55
C.I. 4-1(L)	0.32	0.51	5.00	8.53	1.10	1.53
C.I. 4-1(R)	0.00	0.51	5.00	8.53	1.10	0.00
C.I. 4-1(B)	0.09	0.51	5.00	8.53	1.10	0.43
C.I. 4-1	0.41	0.51	5.00	8.53	1.10	1.96
C.I. 4-2(L)	0.00	0.51	5.00	8.53	1.10	0.00
C.I. 4-2(R)	0.38	0.51	5.00	8.53	1.10	1.82
C.I. 4-2(B)	0.57	0.51	5.00	8.53	1.10	2.73
C.I. 4-2	0.95	0.51	5.00	8.53	1.10	4.55
C.I. 4-3	0.37	0.51	5.00	8.53	1.10	1.77
C.I. 5-2	0.21	0.51	5.00	8.53	1.10	1.00
C.I. 5-3	0.42	0.51	5.00	8.53	1.10	2.01
C.I. 5-4	0.81	0.51	5.00	8.53	1.10	3.88
C.I. 5-5	0.72	0.51	5.00	8.53	1.10	3.45
C.I. 6-1	0.58	0.51	5.00	8.53	1.10	2.78
HR2 F.I. 3-2	1.15	0.51	5.00	8.53	1.10	5.50
HR2 F.I. 3-3	0.52	0.51	5.00	8.53	1.10	2.49

Inlet Design Tak	ole												
25	Year Return I	Frequency											
	Inlet		Upstream		Clogging	Inlet	Sag Inlet	Captured		Inlet	Gutter	Gutter	Ponding
Inlet ID	Location	Peak Flow	Bypass	Total Flow	Factor	Capacity	Capacity	Flow	Bypass Flow	Efficiency	Depth	Spread	Depth
							(Note 1)			(Note 2)			
		(cfs)	(cfs)	(cfs)		(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
C.I. 1A-1	GRADE	5.79	0.56	6.35	1.00	4.54	4.54	4.54	1.81	71.43%	0.23	10.78	
F.I. 2-1	SAG	5.41	0.00	5.41	0.80	18.67	14.93	5.41	0.00	100.00%	•••		0.22
C.I. 3-1	GRADE	2.06	2.39	4.44	1.00	2.26	2.26	2.26	2.18	50.84%	0.18	8.99	
C.I. 3-2	GRADE	2.68	2.02	4.70	1.00	2.31	2.31	2.31	2.39	49.18%	0.18	9.18	•••
C.I. 3-3	GRADE	5.55	0.00	5.55	1.00	3.53	3.53	3.53	2.02	63.67%	0.23	11.69	•••
C.I. 4-1(L)	SAG	1.53			•••			•••			0.15	7.68	•••
C.I. 4-1(R)	SAG	0.00									0.00	0.00	
C.I. 4-1(B)	SAG	0.43			•••						•••		•••
C.I. 4-1	SAG	1.96	0.00	1.96	0.80	19.40	15.52	1.96	0.00	100.00%	•••		•••
C.I. 4-2(L)	SAG	0.00						•••			0.10	5.13	•••
C.I. 4-2(R)	SAG	1.82			•••			•••			0.16	8.19	• • • •
C.I. 4-2(B)	SAG	2.73	•••			•••	•••		•••	•••	•••		
C.I. 4-2	SAG	4.55	0.52	5.07	0.80	19.40	15.52	5.07	0.00	100.00%	•••		
C.I. 4-3	GRADE	1.77	0.00	1.77	1.00	1.25	1.25	1.25	0.52	70.59%	0.12	6.03	
C.I. 5-2	GRADE	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	100.00%	0.14	6.39	•••
C.I. 5-3	GRADE	2.01	1.49	3.50	1.00	3.22	3.22	3.22	0.28	92.07%	0.22	10.09	•••
C.I. 5-4	GRADE	3.88	0.84	4.72	1.00	3.22	3.22	3.22	1.49	68.37%	0.22	11.05	
C.I. 5-5	GRADE	3.45	0.00	3.45	1.00	2.60	2.60	2.60	0.84	75.58%	0.20	9.83	
C.I. 6-1	GRADE	2.78	0.00	2.78	1.00	2.22	2.22	2.22	0.56	79.81%	0.18	9.06	•••
HR2 F.I. 3-2	SAG	5.50	0.00	5.50	0.80	18.67	14.93	5.50	0.00	100.00%	•••		0.22
HR2 F.I. 3-3	SAG	2.49	0.00	2.49	0.80	18.67	14.93	2.49	0.00	100.00%	•••		0.13
Notes:													

1. Inlet capacity at sag location has been reduced by a clogging factor of 0.80, reducing theoretical capacity to 80% capacity, as required per APWA Section 5600.

1. Inlet capacity at sag location has been reduced by a clogging factor of 0.80, reducing theoretical capacity to 80% capacity, as required per APWA Section 5600.

2. Inlet efficiency shown in the tables is Captured Flow/Total Flow, denoting the actual percentage of flow captured after the capacity has been reduced to 80% of theoretical capacity.

2. Inlet efficiency shown in the tables is Captured Flow/Total Flow, denoting the actual percentage of flow captured after the capacity has been reduced to 80% of theoretical capacity.

Both theoretical capacity and reduced capacity are shown.

Both theoretical capacity and reduced capacity are shown.

100	Year Return F	requency				
Inlet ID	Drainage Area	С	Тс	i	К	Peak Flow
	(ac)		(min)	(in/hr)		(cfs)
C.I. 1A-1	1.21	0.51	5.00	10.32	1.25	7.96
F.I. 2-1	1.13	0.51	5.00	10.32	1.25	7.44
C.I. 3-1	0.43	0.51	5.00	10.32	1.25	2.83
C.I. 3-2	0.56	0.51	5.00	10.32	1.25	3.69
C.I. 3-3	1.16	0.51	5.00	10.32	1.25	7.63
C.I. 4-1(L)	0.32	0.51	5.00	10.32	1.25	2.11
C.I. 4-1(R)	0.00	0.51	5.00	10.32	1.25	0.00
C.I. 4-1(B)	0.09	0.51	5.00	10.32	1.25	0.59
C.I. 4-1	0.41	0.51	5.00	10.32	1.25	2.70
C.I. 4-2(L)	0.00	0.51	5.00	10.32	1.25	0.00
C.I. 4-2(R)	0.38	0.51	5.00	10.32	1.25	2.50
C.I. 4-2(B)	0.57	0.51	5.00	10.32	1.25	3.75
C.I. 4-2	0.95	0.51	5.00	10.32	1.25	6.25
C.I. 4-3	0.37	0.51	5.00	10.32	1.25	2.43
C.I. 5-2	0.21	0.51	5.00	10.32	1.25	1.38
C.I. 5-3	0.42	0.51	5.00	10.32	1.25	2.76
C.I. 5-4	0.81	0.51	5.00	10.32	1.25	5.33
C.I. 5-5	0.72	0.51	5.00	10.32	1.25	4.74
C.I. 6-1	0.58	0.51	5.00	10.32	1.25	3.82
HR2 F.I. 3-2	1.15	0.51	5.00	10.32	1.25	7.57
HR2 F.I. 3-3	0.52	0.51	5.00	10.32	1.25	3.42

100	Year Return	Frequency											
	Inlet		Upstream		Clogging	Inlet	Sag Inlet	Captured		Inlet	Gutter	Gutter	Ponding
Inlet ID	Location	Peak Flow	Bypass	Total Flow	Factor	Capacity	Capacity	Flow	Bypass Flow	Efficiency	Depth	Spread	Depth
							(Note 1)			(Note 2)			
		(cfs)	(cfs)	(cfs)		(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
C.I. 1A-1	GRADE	7.96	1.02	8.98	1.00	5.50	5.50	5.50	3.48	61.22%	0.27	12.28	
F.I. 2-1	SAG	7.44	0.00	7.44	0.80	18.67	14.93	7.44	0.00	100.00%	***	•••	0.27
C.I. 3-1	GRADE	2.83	4.53	7.36	1.00	2.64	2.64	2.64	4.72	35.82%	0.22	10.87	
C.I. 3-2	GRADE	3.69	3.46	7.15	1.00	2.62	2.62	2.62	4.53	36.65%	0.22	10.75	
C.I. 3-3	GRADE	7.63	0.00	7.63	1.00	4.17	4.17	4.17	3.46	54.63%	0.26	13.18	
C.I. 4-1(L)	SAG	2.11		•••		•••			•••		0.17	8.66	
C.I. 4-1(R)	SAG	0.00									0.00	0.00	
C.I. 4-1(B)	SAG	0.59	•••		•••	•••		•••	•••	•••	• • • •	•••	
C.I. 4-1	SAG	2.70	0.00	2.70	0.80	19.40	15.52	2.70	0.00	100.00%	•••	•••	
C.I. 4-2(L)	SAG	0.00	•••		•••	•••		•••	•••	•••	0.13	6.34	
C.I. 4-2(R)	SAG	2.50	•••		•••	•••				•••	0.18	9.23	•••
C.I. 4-2(B)	SAG	3.75				•••				•••	•••		
C.I. 4-2	SAG	6.25	0.92	7.17	0.80	19.40	15.52	7.17	0.00	100.00%	• • • •	•••	
C.I. 4-3	GRADE	2.43	0.00	2.43	1.00	1.52	1.52	1.52	0.92	62.41%	0.14	6.80	•
C.I. 5-2	GRADE	1.38	0.00	1.38	1.00	1.38	1.38	1.38	0.01	99.56%	0.16	7.20	
C.I. 5-3	GRADE	2.76	2.84	5.61	1.00	4.73	4.73	4.73	0.88	84.31%	0.26	12.04	•••
C.I. 5-4	GRADE	5.33	1.50	6.83	1.00	3.99	3.99	3.99	2.84	58.39%	0.25	12.70	•••
C.I. 5-5	GRADE	4.74	0.00	4.74	1.00	3.23	3.23	3.23	1.50	68.26%	0.22	11.07	
C.I. 6-1	GRADE	3.82	0.00	3.82	1.00	2.80	2.80	2.80	1.02	73.37%	0.20	10.21	•
HR2 F.I. 3-2	SAG	7.57	0.00	7.57	0.80	18.67	14.93	7.57	0.00	100.00%	•••		0.27
HR2 F.I. 3-3	SAG	3.42	0.00	3.42	0.80	18.67	14.93	3.42	0.00	100.00%	•••	•••	0.16

	ign Calculation Ta												
	Year Return Freq	uency I	L lin ation nine	Daywaataaaaa			N de maine ele					l lin atura a un	Linatus sus
Upstream	Downstream		Upstream	Downstream	61	D	Manning's					Upstream	Upstream
Structure	Structure	Length	Invert	Invert	Slope		n	Total Flow	Velocity	Capacity	Flow Depth	Struct. HGL	Top Elev.
		(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(ft/s)	(cfs)	(ft)	(ft)	(ft)
J.B. 1-2	E.S. 1-1	197.33	1007.18	1006.00	0.60	54	0.010	95.43	10.62	197.70	2.20	1010.05	1028.00
HR2 E.S. 2-3	J.B. 1-2	122.83	1008.67	1007.78	0.72	48	0.010	91.61	10.93	158.96	2.27	1011.57	1013.00
C.I. 1A-1	J.B. 1-2	61.17	1022.15	1020.92	2.01	15	0.013	3.82	5.90	9.16	0.56	1022.94	1026.96
HR2 F.I. 3-2	HR2 M.H. 3-1	141.17	1020.03	1013.60	4.55	18	0.010	10.50	10.92	29.14	0.62	1021.27	1027.13
HR2 F.I. 3-3	HR2 F.I. 3-2	177.37	1024.49	1020.43	2.29	15	0.010	1.95	2.96	12.70	0.84	1025.05	1030.59
F.I. 2-1	HR2 F.I. 3-2	307.10	1027.91	1020.23	2.50	15	0.010	4.24	4.38	13.28	1.04	1028.74	1034.10
HR2 PLUG 6-3	HR2 C.I. 6-2	109.97	1026.71	1024.79	1.75	24	0.010	13.40	6.75	38.85	1.12	1028.03	1029.09
C.I. 3-1	HR2 PLUG 6-3	85.23	1028.20	1026.71	1.75	24	0.010	13.40	6.11	38.88	1.32	1029.52	1036.48
C.I. 3-2	C.I. 3-1	146.00	1034.68	1029.20	3.75	15	0.010	5.04	8.48	16.26	0.48	1035.59	1042.31
C.I. 3-3	C.I. 3-2	215.25	1046.68	1035.38	5.25	15	0.010	3.04	7.86	19.23	0.34	1047.38	1051.39
C.I. 4-1	C.I. 3-1	82.66	1030.85	1029.20	2.00	15	0.010	6.49	7.95	11.86	0.66	1031.88	1038.35
C.I. 4-2	C.I. 4-1	34.00	1033.26	1032.75	1.50	15	0.010	4.95	6.76	10.28	0.61	1034.16	1038.35
C.I. 4-3	C.I. 4-2	46.10	1034.65	1033.96	1.50	15	0.010	1.05	4.23	10.27	0.27	1035.05	1040.82
C.I. 5-2	E.S. 5-1	145.90	1023.31	1018.36	3.39	15	0.010	9.82	10.76	15.46	0.72	1024.49	1032.73
C.I. 5-3	C.I. 5-2	42.00	1024.85	1024.01	2.00	15	0.013	9.03	8.05	9.13	1.01	1026.01	1032.74
C.I. 5-4	C.I. 5-3	129.39	1028.14	1025.55	2.00	15	0.010	6.66	8.04	11.88	0.67	1029.18	1035.18
C.I. 5-5	C.I. 5-4	154.95	1035.03	1028.83	4.00	15	0.010	2.17	5.81	16.79	0.35	1035.62	1040.06
C.I. 6-1	C.I. 5-4	34.00	1030.36	1028.83	4.50	15	0.010	1.82	5.08	17.81	0.35	1030.90	1035.18

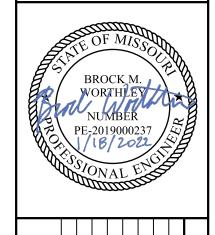
NOTE:
SW ARBOR SOUND DRIVE AND ARBORWAY TERRACE ARE RESIDENTIAL LOCAL
ROADS REQUIRING INLET AND GUTTER CAPACITY FOR THE 10—YEAR STORM EVENT, THEREFORE, INLET DESIGN, PIPE DESIGN, AND DRAINAGE AREA DESIGN TABLES HAVE BEEN INCLUDED FOR THE 10-YEAR STORM EVENT. ALL STORM SEWER PIPES HAVE BEEN DESIGNED TO CONVEY THE 25-YEAR STORM EVENT, PER RESIDENTIAL COLLECTOR STANDARDS FOR BUCKTHORN STREET.

Storm Sewer Des	ign Calculation Ta	ble											
25	Year Return Freq	uency											
Upstream	Downstream		Upstream	Downstream			Manning's					Upstream	Upstream
Structure	Structure	Length	Invert	Invert	Slope	Diameter	n	Total Flow	Velocity	Capacity	Flow Depth	Struct. HGL	Top Elev.
		(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(ft/s)	(cfs)	(ft)	(ft)	(ft)
J.B. 1-2	E.S. 1-1	197.33	1007.18	1006.00	0.60	54	0.010	121.64	11.49	197.70	2.55	1010.42	1028.00
HR2 E.S. 2-3	J.B. 1-2	122.83	1008.67	1007.78	0.72	48	0.010	117.10	11.98	158.96	2.64	1011.93	1013.00
C.I. 1A-1	J.B. 1-2	61.17	1022.15	1020.92	2.01	15	0.013	4.54	6.23	9.16	0.62	1023.01	1026.96
HR2 F.I. 3-2	HR2 M.H. 3-1	141.17	1020.03	1013.60	4.55	18	0.010	13.40	8.97	29.14	1.06	1021.39	1027.13
HR2 F.I. 3-3	HR2 F.I. 3-2	177.37	1024.49	1020.43	2.29	15	0.010	2.49	3.23	12.70	0.96	1025.12	1030.59
F.I. 2-1	HR2 F.I. 3-2	307.10	1027.91	1020.23	2.50	15	0.010	5.41	5.00	13.28	1.16	1028.85	1034.10
HR2 PLUG 6-3	HR2 C.I. 6-2	109.97	1026.71	1024.79	1.75	24	0.010	16.38	7.15	38.85	1.29	1028.17	1029.09
C.I. 3-1	HR2 PLUG 6-3	85.23	1028.20	1026.71	1.75	24	0.010	16.38	6.68	38.88	1.46	1029.66	1036.48
C.I. 3-2	C.I. 3-1	146.00	1034.68	1029.20	3.75	15	0.010	5.84	8.92	16.26	0.52	1035.66	1042.31
C.I. 3-3	C.I. 3-2	215.25	1046.68	1035.38	5.25	15	0.010	3.53	8.23	19.23	0.36	1047.44	1051.39
C.I. 4-1	C.I. 3-1	82.66	1030.85	1029.20	2.00	15	0.010	8.28	8.78	11.86	0.77	1031.98	1038.35
C.I. 4-2	C.I. 4-1	34.00	1033.26	1032.75	1.50	15	0.010	6.32	7.37	10.28	0.71	1034.27	1038.35
C.I. 4-3	C.I. 4-2	46.10	1034.65	1033.96	1.50	15	0.010	1.25	4.21	10.27	0.31	1035.09	1040.82
C.I. 5-2	E.S. 5-1	145.90	1023.31	1018.36	3.39	15	0.010	12.26	12.02	15.46	0.84	1024.53	1032.73
C.I. 5-3	C.I. 5-2	42.00	1024.85	1024.01	2.00	15	0.013	11.26	9.18	9.13	1.25	1026.54	1032.74
C.I. 5-4	C.I. 5-3	129.39	1028.14	1025.55	2.00	15	0.010	8.04	6.75	11.88	1.25	1029.26	1035.18
C.I. 5-5	C.I. 5-4	154.95	1035.03	1028.83	4.00	15	0.010	2.60	5.55	16.79	0.43	1035.68	1040.06
C.I. 6-1	C.I. 5-4	34.00	1030.36	1028.83	4.50	15	0.010	2.22	4.93	17.81	0.43	1030.95	1035.18

NOT ASBUILT These plans have been reviewed for accuracy and are accepted for basic conformance to the approved construction drawings.

Accepted Record Drawings

100	Year Return Freq	uency											
Upstream	Downstream	·	Upstream	Downstream			Manning's					Upstream	Upstream
Structure	Structure	Length	Invert	Invert	Slope	Diameter	n	Total Flow	Velocity	Capacity	Flow Depth	Struct. HGL	Top Elev.
		(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(ft/s)	(cfs)	(ft)	(ft)	(ft)
J.B. 1-2	E.S. 1-1	197.33	1007.18	1006.00	0.60	54	0.010	167.23	12.84	197.70	3.18	1010.95	1028.00
HR2 E.S. 2-3	J.B. 1-2	122.83	1008.67	1007.78	0.72	48	0.010	161.73	13.89	158.96	3.35	1012.35	1013.00
C.I. 1A-1	J.B. 1-2	61.17	1022.15	1020.92	2.01	15	0.013	5.50	6.65	9.16	0.70	1023.10	1026.96
HR2 F.I. 3-2	HR2 M.H. 3-1	141.17	1020.03	1013.60	4.55	18	0.010	18.43	10.48	29.14	1.50	1021.49	1027.13
HR2 F.I. 3-3	HR2 F.I. 3-2	177.37	1024.49	1020.43	2.29	15	0.010	3.42	3.79	12.70	1.06	1025.24	1030.59
F.I. 2-1	HR2 F.I. 3-2	307.10	1027.91	1020.23	2.50	15	0.010	7.44	6.32	13.28	1.25	1029.00	1034.10
IR2 PLUG 6-3	HR2 C.I. 6-2	109.97	1026.71	1024.79	1.75	24	0.010	20.82	6.63	38.85	2.00	1029.12	1029.09
C.I. 3-1	HR2 PLUG 6-3	85.23	1028.20	1026.71	1.75	24	0.010	20.82	7.10	38.88	2.00	1029.83	1036.48
C.I. 3-2	C.I. 3-1	146.00	1034.68	1029.20	3.75	15	0.010	6.79	8.53	16.26	0.63	1035.73	1042.31
C.I. 3-3	C.I. 3-2	215.25	1046.68	1035.38	5.25	15	0.010	4.17	8.68	19.23	0.40	1047.51	1051.39
C.I. 4-1	C.I. 3-1	82.66	1030.85	1029.20	2.00	15	0.010	11.39	10.19	11.86	0.98	1032.06	1038.35
C.I. 4-2	C.I. 4-1	34.00	1033.26	1032.75	1.50	15	0.010	8.69	8.39	10.28	0.88	1034.40	1038.35
C.I. 4-3	C.I. 4-2	46.10	1034.65	1033.96	1.50	15	0.010	1.52	3.66	10.27	0.44	1035.14	1040.82
C.I. 5-2	E.S. 5-1	145.90	1023.31	1018.36	3.39	15	0.010	16.13	13.73	15.46	1.08	1024.55	1032.73
C.I. 5-3	C.I. 5-2	42.00	1024.85	1024.01	2.00	15	0.013	14.75	12.02	9.13	1.25	1027.45	1032.74
C.I. 5-4	C.I. 5-3	129.39	1028.14	1025.55	2.00	15	0.010	10.02	8.17	11.88	1.25	1030.20	1035.18
C.I. 5-5	C.I. 5-4	154.95	1035.03	1028.83	4.00	15	0.010	3.23	3.51	16.79	1.25	1035.75	1040.06
C.I. 6-1	C.I. 5-4	34.00	1030.36	1028.83	4.50	15	0.010	2.80	3.23	17.81	1.25	1031.03	1035.18

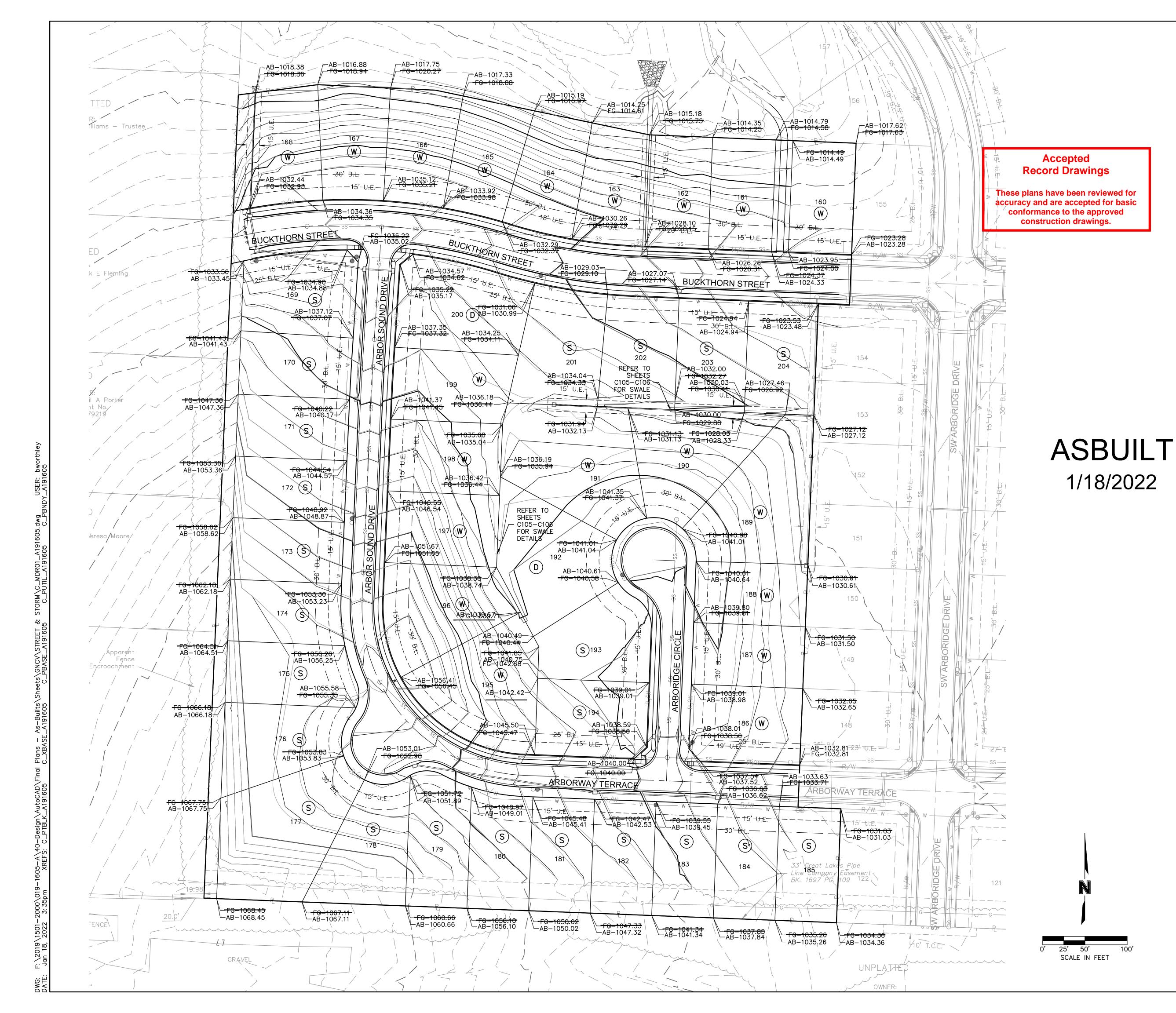


REVISIONS DESCRIPTION B					REVISIONS
DATE					
REV. DA					
					_ 070

DRAINAGE TABLES STREET & STORM SEWER PLANS

checked by: BMW
approved by: BMW
QA/QC by: JES
project no.: A19-1605
drawing no.: C DRN01 A191605
date: 10/02/2020

SHEET



NOTES:

MBOE — MINIMUM BUILDING OPENING ELEVATION FG — FINISHED GRADE

NOTES:
1. INDIVIDUAL LOT OWNERS SHALL NOT CHANGE OR OBSTRUCT THE DRAINAGE FLOW LINES OR PATHS ON THE LOTS, AS SHOWN ON THE MATER DRAINAGE PLAN, UNLESS SPECIFIC APPLICATION IS MADE AND APPROVED BY THE CITY ENGINEER.

2. PLAT IS LOCATION IN ZONE X, "AREAS OUTSIDE THE 1—PERCENT ANNUAL CHANCE FLOODPLAINS, AREAS OF 1—PERCENT ANNUAL CHANCE SHEET FLOW FLOODING WHERE THE AVERAGE DEPTHS ARE LESS THAN 1 FOOT, AREAS OF 1—PERCENT ANNUAL CHANCE STREAM FLOODING WHERE THE CONTRIBUTING DRAINAGE AREA IS LESS THAN 1 SQUARE MILE, OR AREAS PROTECTED FROM THE 1—PERCENT ANNUAL CHANCE FLOOD BY LEVEES. NO BASE FLOOD ELEVATIONS OR DEPTHS ARE SHOWN WITHIN THIS ZONE"

3. PLAT IS LOCATED OUTSIDE OF ANY REQUIRED BUFFER ZONES FOR NATURAL STREAMS.

4. MBOE ELEVATIONS HAVE BEEN PROVIDED AT EACH LOT CORNER. INTERPOLATION WILL BE ALLOWED BETWEEN THE RIGHT AND LEFT SIDE MBOE'S SHOWN ON THE MASTER DRAINAGE PLAN, DEPENDING ON THE LOCATION OF THE LOWEST OPENING ON THE PROPOSED STRUCTURE.

5. REFER TO SHEET C105—C106 FOR SWALE GRADING DETAILS.

6. DRAINAGE PATHS TO BE CONSTRUCTED BETWEEN EACH OF THE LOTS LABELED AS STANDARD LOTS TO DRAIN WEST.

7. NO BUILDING PERMITS WILL BE ISSUED UNTIL AN AS-GRADED MASTER DRAINAGE PLAN HAS BEEN SUBMITTED TO THE CITY AND APPROVED BY THE CITY.

BASEMENT TYPES

STANDARD

WALKOUT

D DAYLIGHT

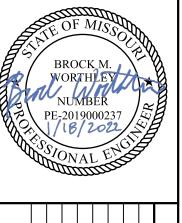
			Ridge Thir		n
	IVIININ		ing Openin		
Lot	Rear Left	Rear Right	Front Left	Front Right	As-Built Grading Pla
LOT	MBOE	MBOE	MBOE	MBOE	Required
160					1,00 411.04
161					
162					
163					
164					
165					
166					
167					
168					
169					
170					
171					
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173					
174					
175					
176					
177					
178					
179					
180					
181					
182					
183					
184					
185					
186					
187	1032.64	1033.55	-	-	X
188	1031.75	1032.64	-	-	X
189	1028.66	1031.75	-	-	X
190	1032.33	1028.66	-	-	Х
191	1037.46	1032.33	-	-	Х
192	1041.76	1037.46	-	-	Х
193					Х
194					
195	1040.94	1041.65	-	-	Х
196	1040.01	1040.94	-	-	Х
197	1037.69	1040.01	-	-	X
198	1036.31	1037.69	-	-	X
199	1035.32	1037.20	-	-	X
200	1032.73	1035.32	-	-	X
201					X
202					X
203					X
204					X

X indiciates condition applies to the lot.

MBOE's have been provided per lot lines, facing the lot from the street. Interpolation will be allowed between the right and left side MBOE's provided, depending on the location of the lowest opening on the proposed structure.

SSO

Olsson - Civil Engineering
Missouri Certificate of Author 1301 Burlington Street

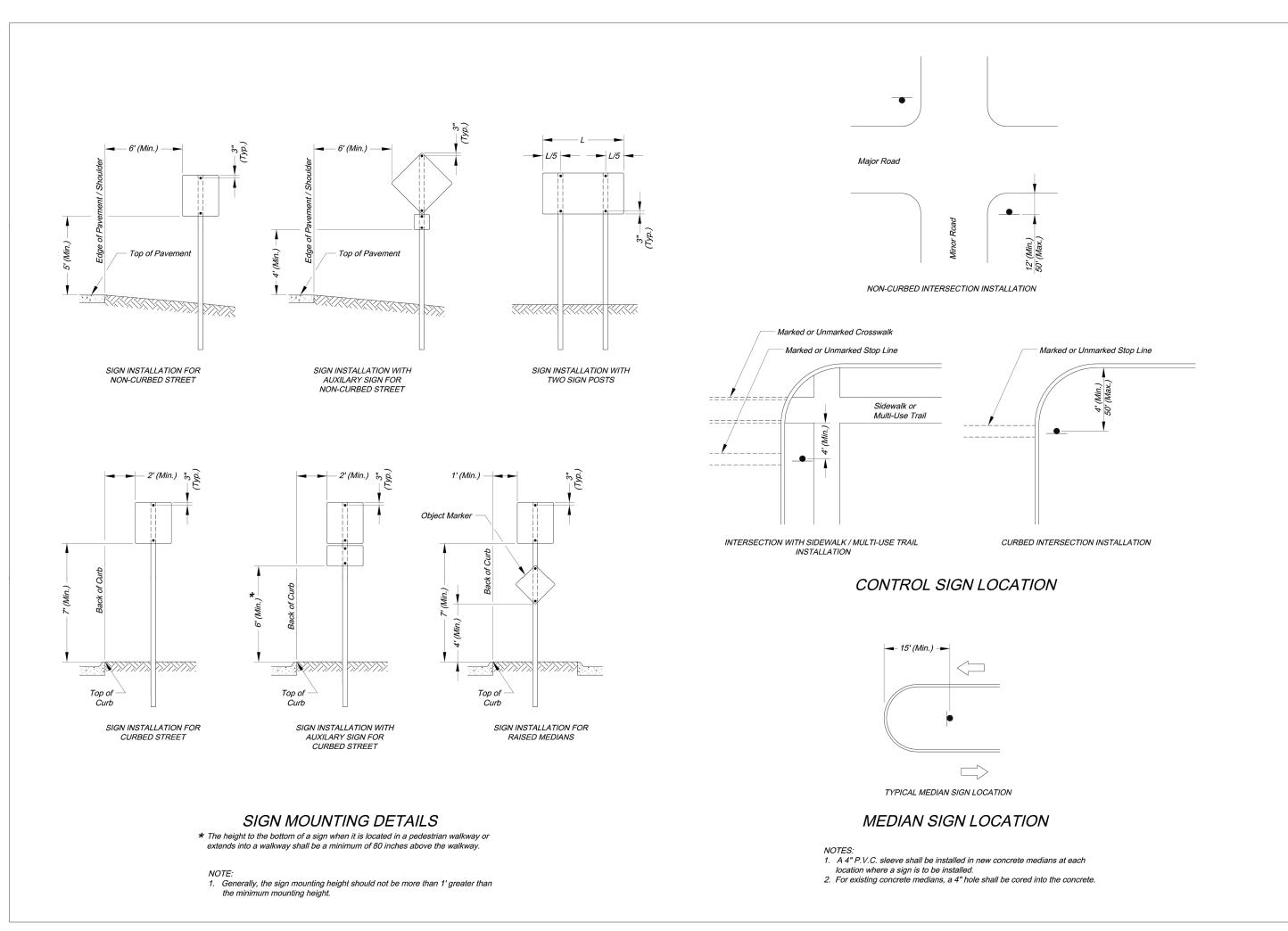


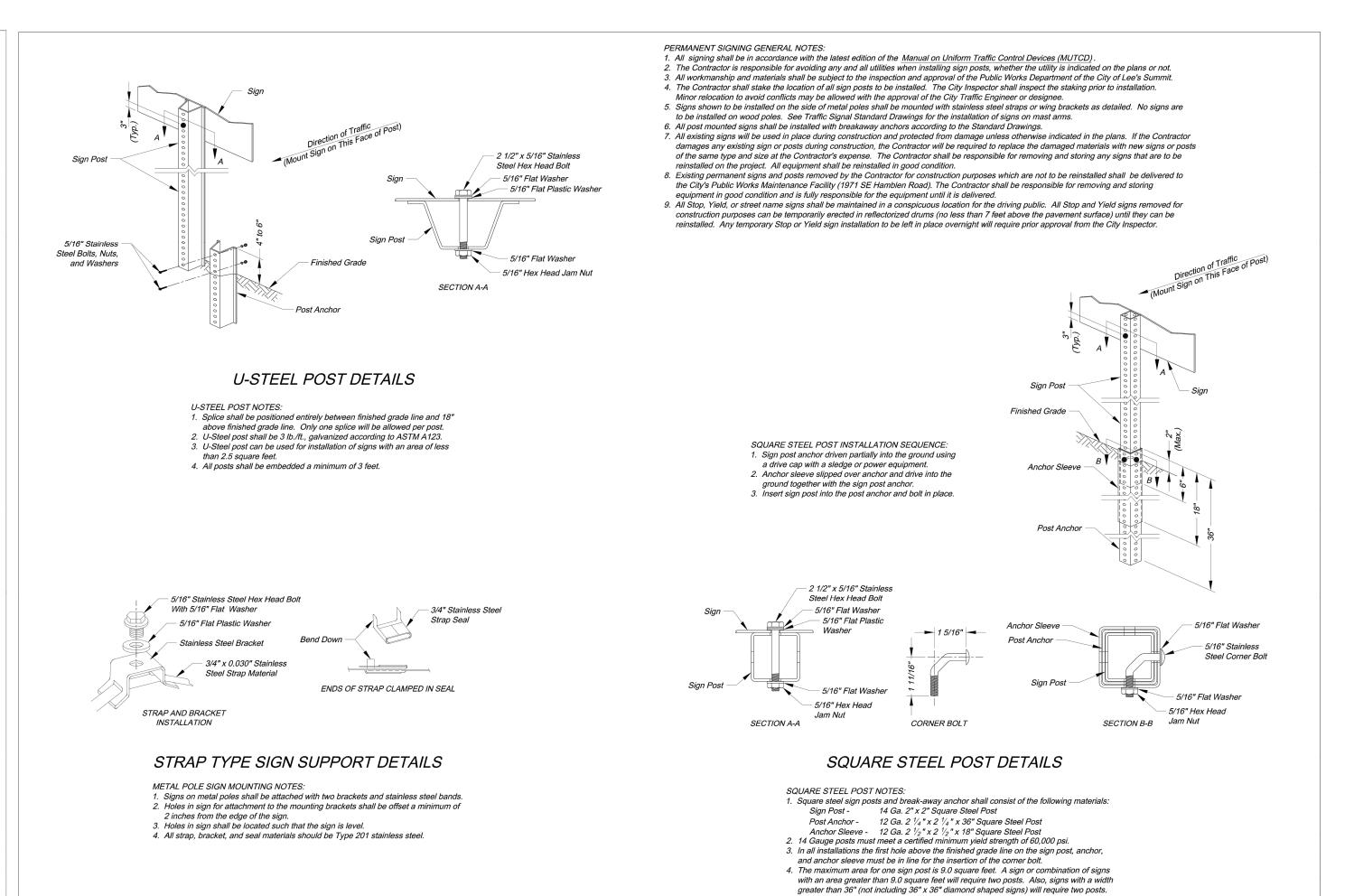
REVISIONS DESCRIPTION	REVISED PER CITY COMMENTS						REVISIONS	
DATE	11/23/2020							
REV.								
	STREET & STORM SEWER PLANS			HAWTHORN RIDGE	THIRD PLAT		LEE'S SUMMIT, MO	
drawn checke approv	ed b red	by:	_			OL BM BM	<u>w</u>	

drawing no.: C_MDR01_A191605

SHEET

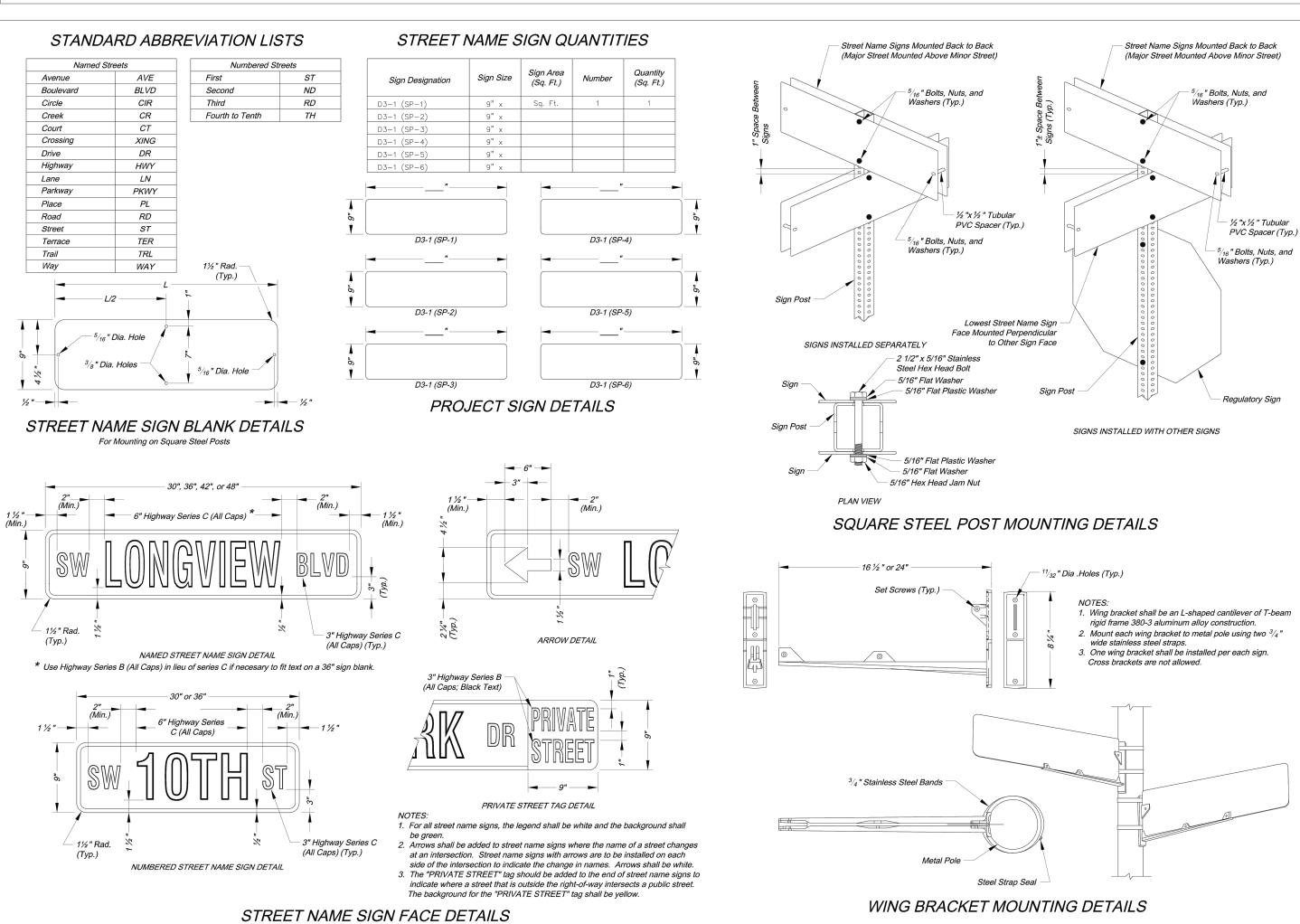
C125







These plans have been reviewed for accuracy and are accepted for basic conformance to the approved construction drawings.



NOT ASBUILT

SIGN DETAILS
- & STORM SEWER F IAWTHORN RIDGE THIRD PLAT

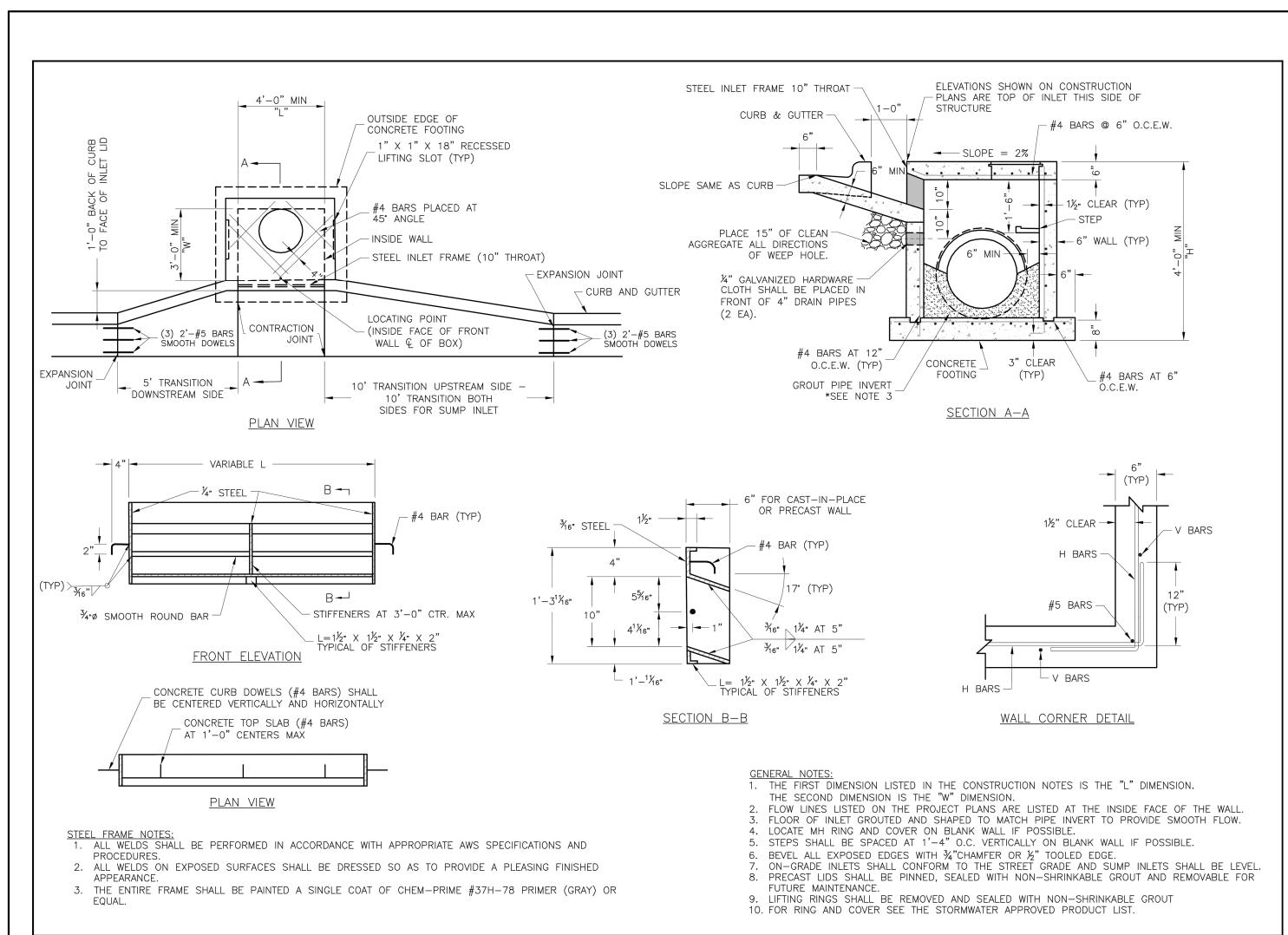
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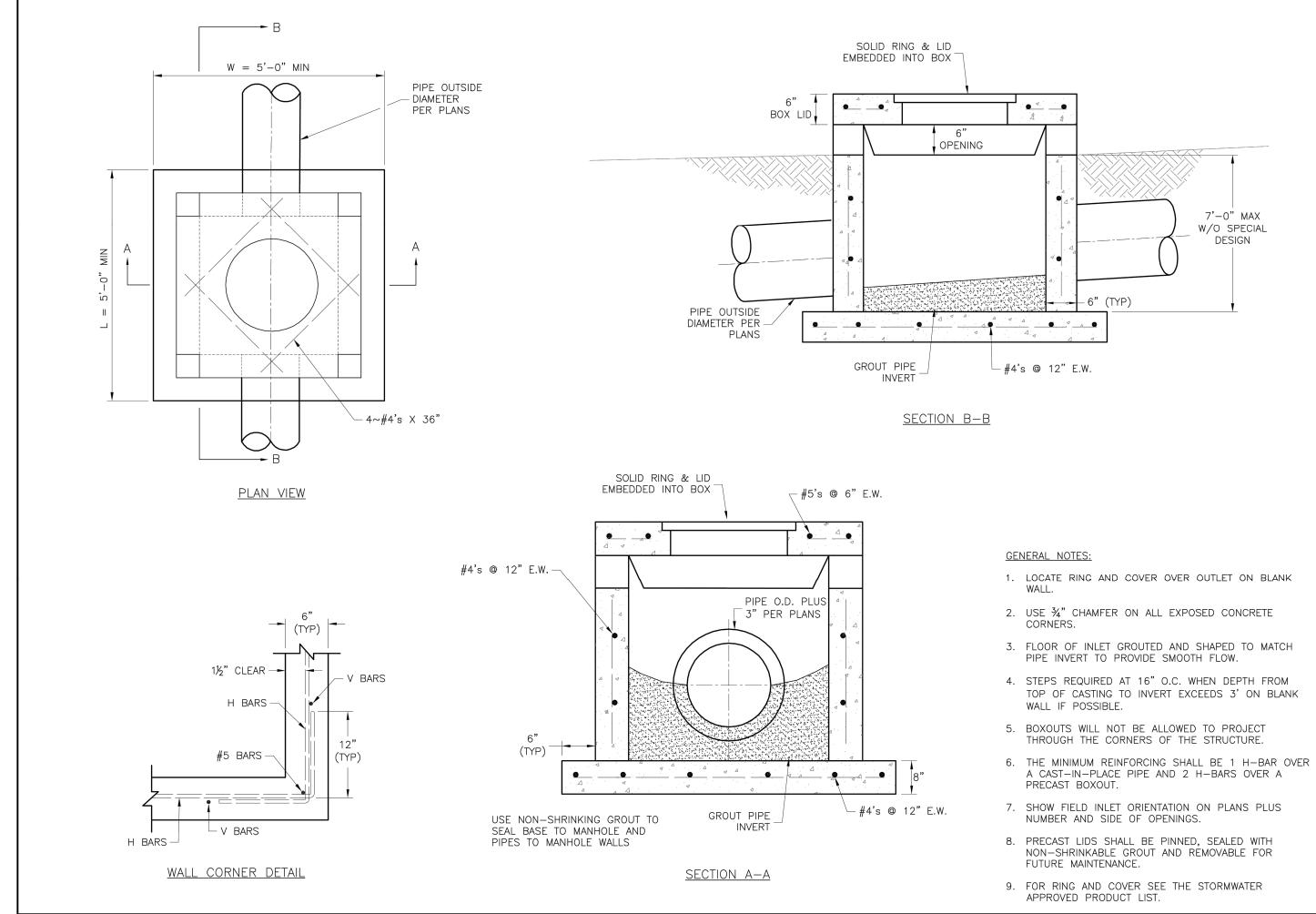
SHEET C126

project no.: A19-1605 drawing no.: C DTL01 A191605

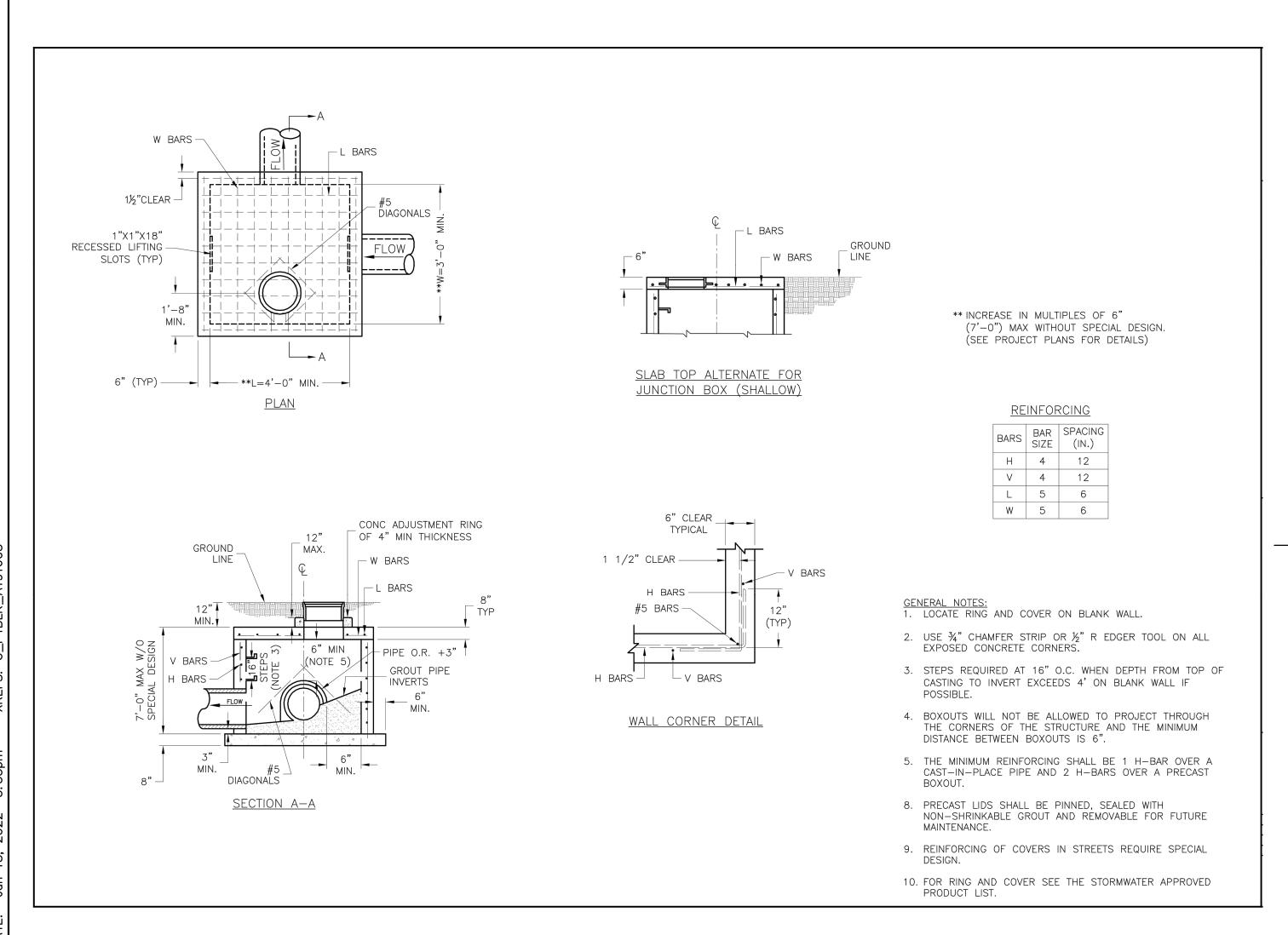
checked by: approved by:

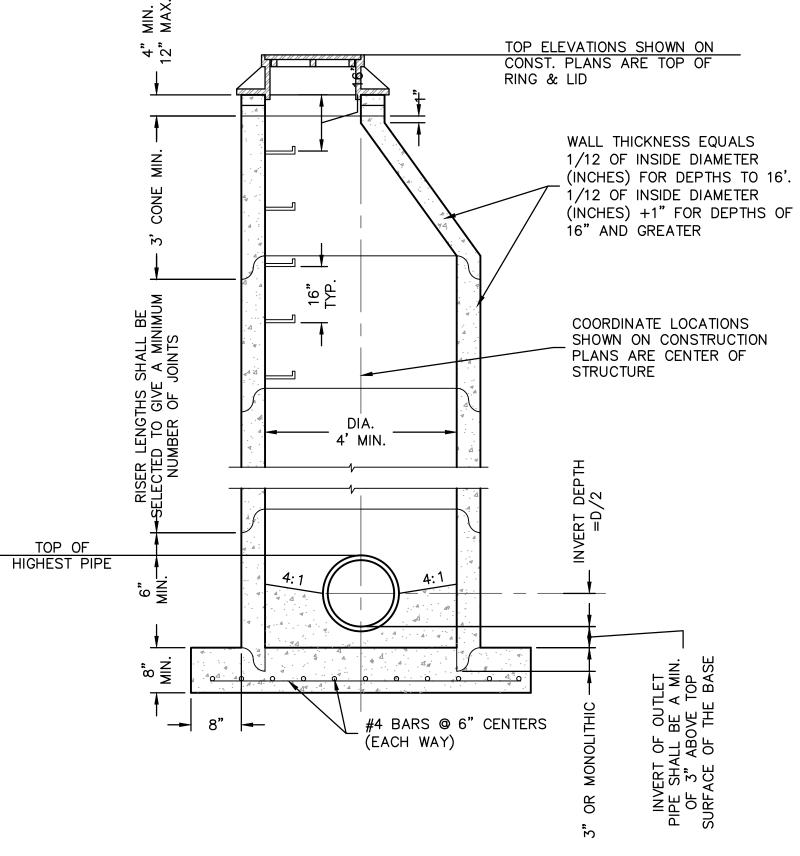
QA/QC by:





STANDARD PRECAST STORM SEWER MANHOLE





NOT TO SCALE

STORM MANHOLE NOTES

- 1. ALL MANHOLES ARE TO BE PRECAST CONCRETE AND OF ECCENTRIC CONE TYPE UNLESS OTHERWISE SPECIFIED.
- MANHOLE TOP ADJUSTMENTS SHALL BE ACCOMPLISHED BY THE USE OF CONCRETE ADJUSTMENT RINGS.
 TOP OF MANHOLE CASTING SHALL BE SET FLUSH AND ON SAME
- SLOPE AS FINISHED SURFACE OR AS DIRECTED BY THE ENGINEER.
- 4. REINFORCEMENT IN ALL SECTIONS SHALL EQUAL OR EXCEED A.S.T.M. C-478 SPECIFICATIONS.
- THE ENGINEER SHALL DESIGNATE MODIFICATIONS FOR MANHOLES WITH SPECIAL DESIGNS.
 THE INSIDE DIAMETER OF THE MANHOLE SHALL BE 4'-0" FOR
- PIPE DIAMETERS FROM 12" THRU 24", 5'-0" FOR PIPE DIAMETERS FROM 27" THRU 36", AND 6'-0" FOR PIPE DIAMETERS 42" THRU 48".
- 7. CLEARANCE TOLERANCE OF PIPE OPENINGS: THE MAXIMUM ALLOWABLE PIPE OPENING ON A HORIZONTAL AXIS SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 12". THE MAXIMUM ALLOWABLE PIPE OPENING ON VERTICAL AXIS SHALL BE THE OUTSIDE DIAMETER PLUS 8". THE MAXIMUM CLEARANCE BETWEEN THE OUTSIDE SURFACE OF AN INSTALLED PIPE AND
- THE CONCRETE OF THE MANHOLE SHALL BE 2".

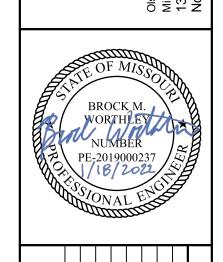
 8. INSTALLATION OF PIPE OPENINGS: ALL REQUIRED PIPE OPENINGS SHALL BE PLANT CAST IN MANHOLE UNITS. FIELD ALTERATIONS OF OPENINGS WILL BE PERMITTED PROVIDED WALLS ARE SCORED WITH A MASONRY SAW TO A DEPTH SUFFICIENT TO SEVER REINFOECING STEEL. A CHIPPING HAMMER MAY THEN BE USED TO REMOVE THE CONCRETE. MINIMUM DISTANCE BETWEEN ANY TWO ADJACENT PIPES SHALL BE 2".
- 9. NO DIRECT PAYMENT FOR SHAPING FLOOR OR CONNECTING PIPES AS SHOWN ON PLANS.
- 10. RING AND COVER TO BE NEENAH R-1736, CLAY & BAILEY #2008, DEETER # 1316, OR APPROVED EQUAL. (CASTING MAY VARY BY MUNICIPALITY, REFER TO PLANS & CONTRACT DOCUMENTS.)

Accepted Record Drawings

These plans have been reviewed for accuracy and are accepted for basic conformance to the approved construction drawings.

NOT ASBUILT

Civil Engineering



REVISIONS DESCRIPTION						REVISIONS	
DATE							
REV.							
STORM SEWER DETAILS	STREET & STORM SEWER PLANS		HAWTHORN RIDGE	THIRD PLAT		2020	
	STR	1				0	

 drawn by:
 OLS

 checked by:
 BMW

 approved by:
 BMW

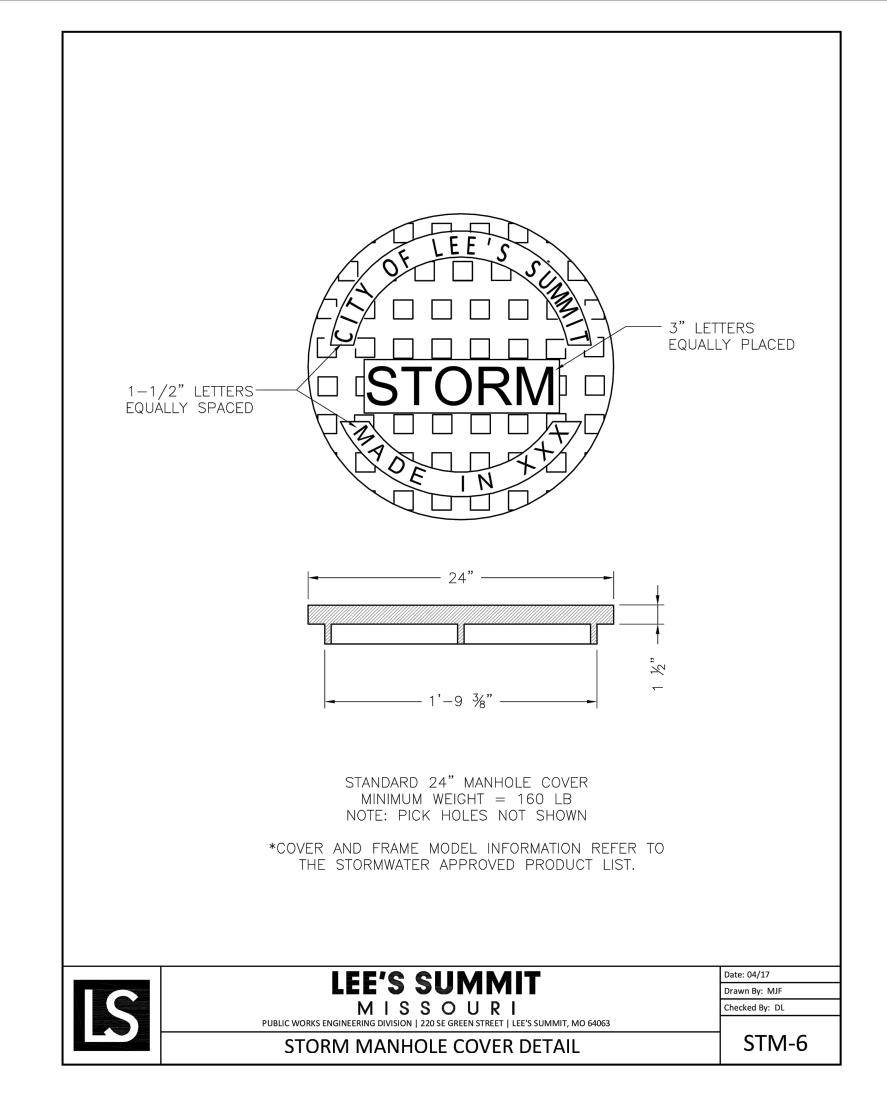
 QA/QC by:
 JES

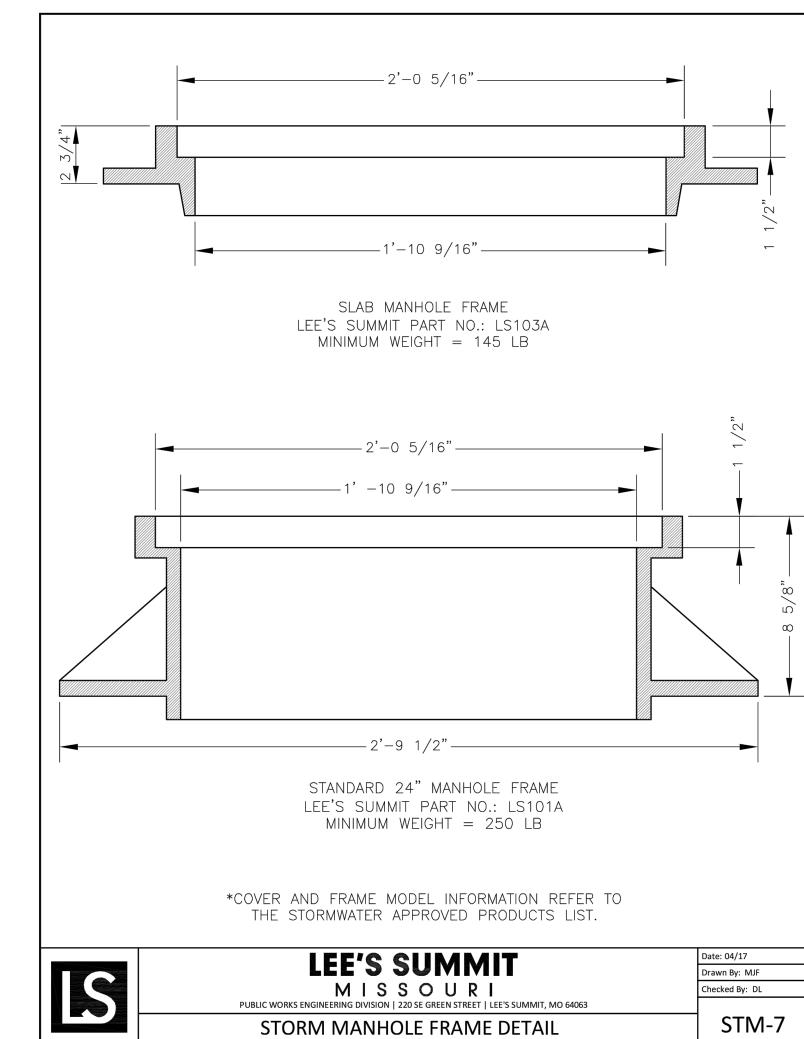
 project no.:
 A19-1605

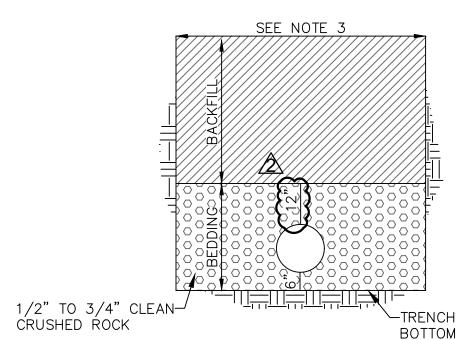
 drawing no.:
 C DTL01 A191605

 date:
 10/02/2020

SHEET C127







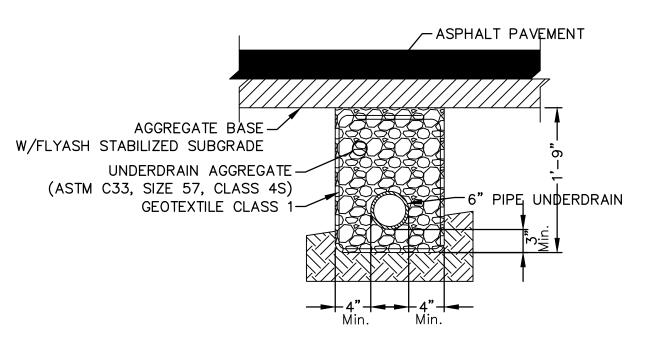
TYPICAL SECTION FOR PLASTIC PIPE (IN ROCK OR SOIL)

UNDERGROUND PIPE INSTALLATION FOR STORM SEWER LINES N.T.S.

1. BACKFILL SHALL BE JOB EXECAVATED MATERIAL FREE FROM DEBRIS AND STONES COMPACTED TO 90% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT AS DETERMINED BY ASTM D698. BACKFILL UNDER PAVEMENT (EXISTING OR PROPOSED), SHALL BE FLOWABLE FILL.

2. TRENCH BANKS MAY BE CUT BACK ON SLOPES IN ACCORDANCE WITH CURRENT OSHA REGULATIONS, BUT ONLY IN AREAS WHERE THE INCREASED TRENCH WIDTH WILL NOT INTERFERE WITH SURFACE FEATURES. SLOPES MUST NOT EXTEND BELOW TOP OF BEDDING.

3. MINIMUM AND MAXIMUM WIDTHS SHALL BE IN ACCORDANCE WITH PIPE MANUFACTURER'S RECOMMENDATION AS APPROVED ON ENGINEERING PLANS.



PIPE UNDERDRAIN LATERAL N.T.S.

NOTES:

1. Where Pipe Underdrains are used, all Underdrain Outlet Pipes shall be solid wall with watertight joints. All Outlets Pipes shall be tied into the nearest storm sewer inlets at roadway sag locations as indicated in the street profile.

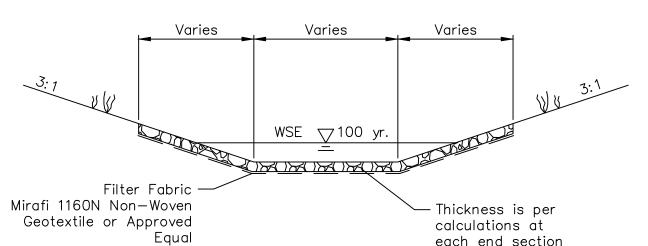
2. All the decadrain Pipes shall be installed at a minimum slope of 1%.

 All Underdrain Pipes shall be installed at a minimum slope of 1%.
 Underdrain Pipe shall be installed with the perforations placed down. 4. Blanket Underdrain Aggregate, Pipe Underdrain Aggregate, Pipe Underdrain, Edge Underdrain and Outlet Pipe shall conform to City of Lee's Summit Specifications.

5. Overlap geotextile at top of trench a minimum of 12".

Accepted **Record Drawings**

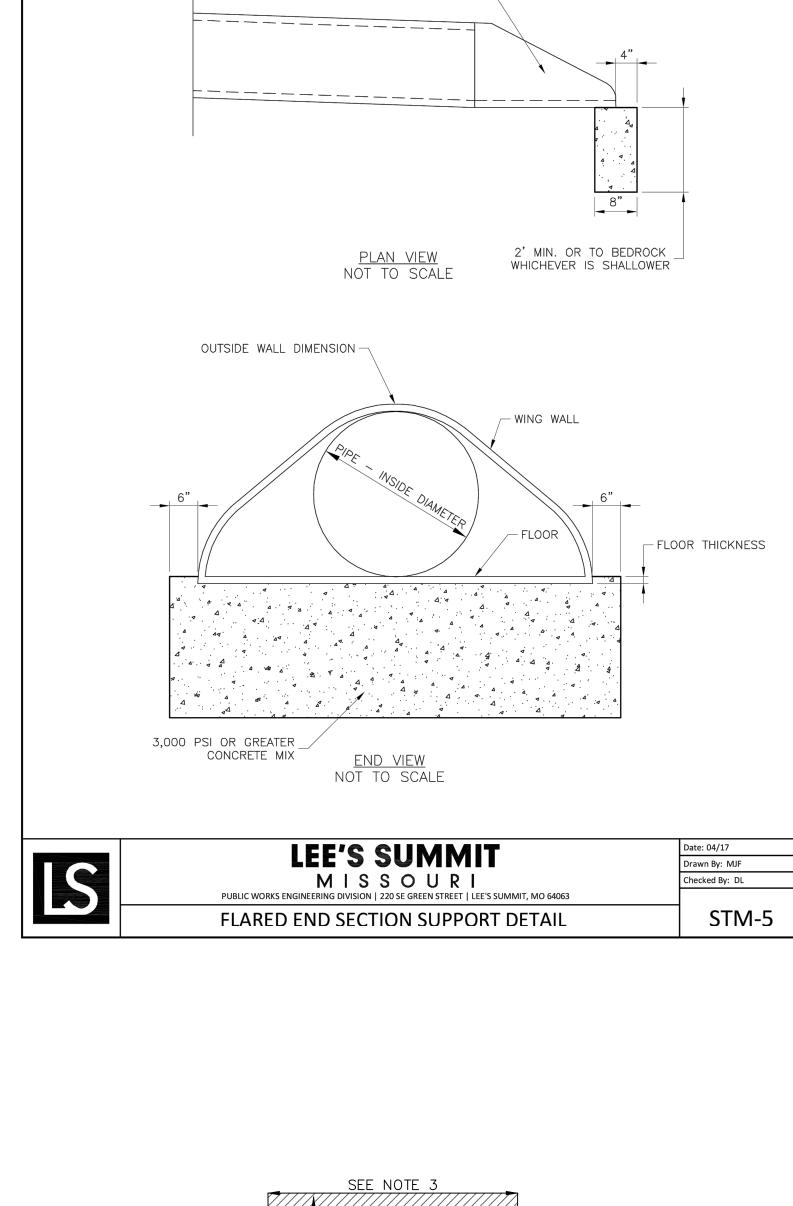
These plans have been reviewed for accuracy and are accepted for basic conformance to the approved construction drawings.

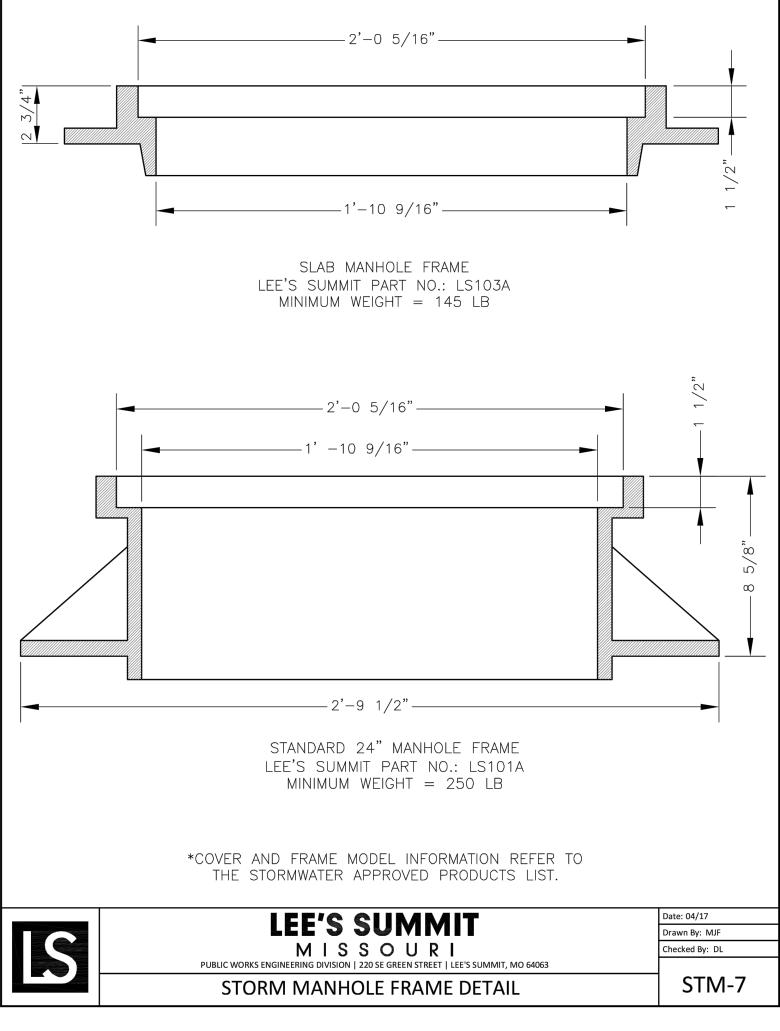


NOTE: DIMENSIONS ARE PER CALCULATIONS AT EACH END SECTION

RIPRAP DETAIL N.T.S.

NOT ASBUILT





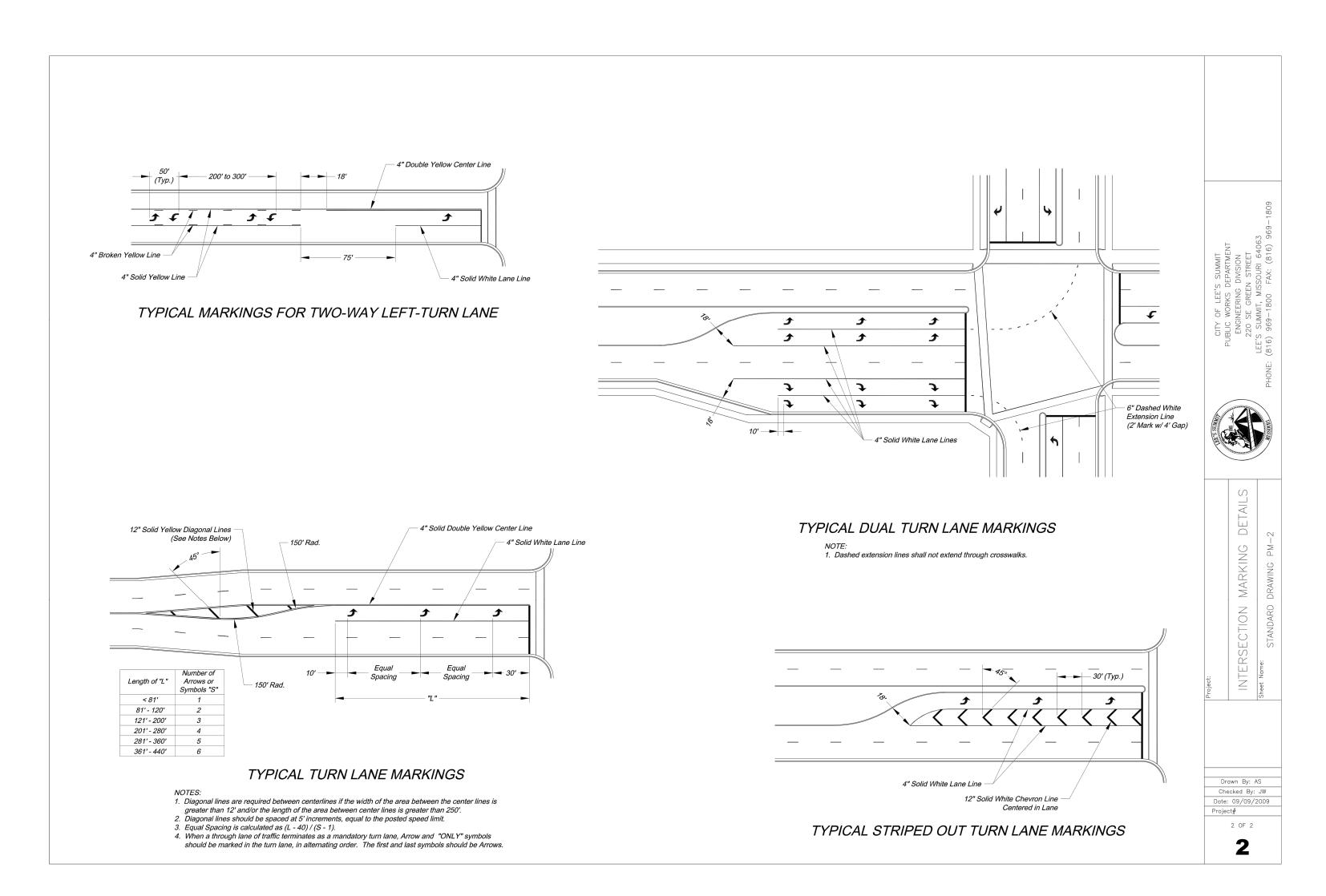
STORM SEWER DETAILS REET & STORM SEWER PLANS HAWTHORN RIDGE THIRD PLAT

each end section

checked by: BMW BMW approved by: QA/QC by: project no.: A19-1605 drawing no.: C DTL01 A191605

> SHEET C128

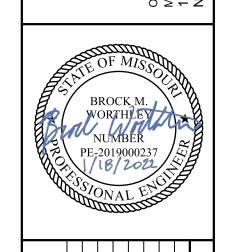
10/02/2020



Accepted Record Drawings

These plans have been reviewed for accuracy and are accepted for basic conformance to the approved construction drawings.

Civil Engineering



ВУ						
REVISIONS DESCRIPTION					REVISIONS	
DATE						
REV. NO.						
				0000	2020	

NOT ASBUILT

drawn by: OLS
checked by: BMW
approved by: BMW
QA/QC by: JES
project no.: A19-1605
drawing no.: C DTL01 A191605
date: 10/02/2020

ROADWAY MARKING DETAILS STREET & STORM SEWER PLANS

HAWTHORN RIDGE THIRD PLAT

SHEET C129