Summit Point Apartments, Phase-II 504 NE Chipman Road Lee's Summit, Missouri 64063 CFS Project No. 21-5065/19-5293

SW ¹/₄, Section 32 Township 48 North, Range 31 West Jackson County, Missouri Tributary P3 to Prairie Lee Lake Watershed

Base Flood Elevation Determination in Unnumbered A Zone at Summit Point Apartments, Phase-II for Tributary P3 To Prairie Lee Lake

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May 13, 2021



Base Flood Determination in Unnumbered A Zone for Tributary P3 To Prairie Lee Lake Summit Point Apartments, Phase-II 504 NE Chipman Road City of Lee's Summit, Jackson County, Missouri 64063 CFS Project #21-5065 / #19-5293 May 13, 2021

<u>Purpose:</u> The purpose of the study is to determine the 100-year base flood elevations in the unnumbered Zone A on the Summit Point Apartments Phase-II property. Tributary P3 to Prairie Lee Lake flows along the northern side of the proposed Summit Point site. There is a small local flow drainage channel on the proposed Summit Point site that branches off of the main Tributary P3 channel where the 100-year base flood elevations are key to setting the finish floor elevations of the proposed apartment buildings. The City of Lee's Summit requires that all new building developments have to have finish floors a minimum of 2 ft higher than the highest 100-year base flood elevation extending onto the site.



Vicinity Map of the Summit Point Apartments at 504 NE Chipman Road in Lee's Summit

<u>General:</u> The owners of the proposed Summit Point Apartments at 504 NE Chipman Road have proposed to develop the second phase of their property. The Phase-II addition would be constructed

directly to the north of the existing Phase-I apartments located at 504 NE Chipman Road in Lee's Summit, Missouri. Phase I included five multi-unit apartment buildings plus a swimming pool on a 6.49 acre site constructed in 1980. The proposed Phase-II addition would cover 7.21 acres and include six new multi-apartment buildings along with parking lots and service drives.

The site slopes downwards to the north where the existing Tributary P3 to Prairie Lee Lake flows eastwards along the site's northern boundary. The existing Tributary P3 creek has flowline elevations ranging between approximately 994' to 1000' along the northern side of the Summit Point Apartments, Phase II. NE Swann Circle is located directly to the east of Summit Point and NE Independence Avenue is located approximately 800 ft downstream. The creek crossing at NE Swann Circle has triple 48" HDPE culverts have upstream flowline elevations of approximately 986.91' and the top of the roadway has an overflow elevation of approximately 994'. The creek bed upstream of NE Swann Circle serves as a stormwater detention basin for the Maple Tree Manor subdivision.

A small local flow drainage channel branches off of the main Tributary P3 channel and extends onto the northern portion of the Summit Point site. The small drainage channel converges with the main channel of Tributary P3 near the northeast corner of the Summit Point site, and extends westwards approximately 420 ft. The small drainage channel has an invert elevation of approximately 987.00' at the downstream convergence point with Tributary P3, and has an upstream invert elevation of approximately 997.30'. Drainage contributing to the small drainage channel flows off of the Summit Point site to the south.



FEMA FIRM Flood Map 29095C0436G, Showing the Existing Tributary P3 to Prairie Lee Lake Flowing along the Northern Border of the Summit Point Apartments

NE Independence Avenue is located approximately 1000 ft downstream from NE Swann Circle downstream and has a single 48" RCP culvert with an upstream flowline elevation of approximately

979.25' and an overflow elevation across the top of NE Independence Avenue of approximately 985.41'. The creek bed upstream of NE Independence Avenue serves as a stormwater detention basin for the Maple Tree Manor subdivision.

The FEMA flood map shows the defined 1% (100-year) flood elevations to the east along the creek stop short of Independence Avenue. A portion of the northern side of the site is within the FEMA 1% (100-year) floodplain, with the remaining ground above the flood limits.

<u>HEC-RAS Model Setup</u>: CFS Engineers created a HEC-RAS model of the study segment of Tributary P3 to Prairie Lee Lake extending from the western side of the Summit Point property downstream to Independence Avenue. The channel geometry and cross-sections were derived using the available GIS topography maps from the City of Lee's Summit and Jackson County, and also the topographic survey of the Summit Point site performed by CFS Engineers in 2019.

Cross-sections were cut across the digital contours to set the station-elevations. The cross-sections in the river channel were stationed based on an assumed 10000 ft at the point where the centerline of NE Swann Circle crossed the Tributary P3 to Prairie Lee Lake creek. The left and right stream bank locations were set based on the apparent break in slope locations along the sides of the main creek channel. The main channel lengths were based on the digital alignment of the creek's flowline and the left and right overbank lengths were estimated based on the general curvature of the creek alignment.

Stream flows were derived using the USGS' StreamStats web-based hydrological software to determine flow rates at several locations along Tributary P3. The StreamStats flows calculated at the point where Tributary P3 crossed Highway 291 were compared to the values listed in the FEMA Flood Insurance Study and were summarized in the table below:

Return Interval (Percentage)	Return Interval (Year Probability)	FEMA FIS Study Peak Annual Chance Discharge (cfs)	StreamStats Peak Flows (Peak Urban Statewide SIR 2010 5073) Discharge (cfs)
10%	10-Year	450 cfs	366 cfs
2%	50-Year	650 cfs	619 cfs
1%	100-Year	750 cfs	733 cfs
0.2%	500-Year	900 cfs	1004 cfs

FEMA and StreamStats Channel Flow Rates on Tributary P3 to Prairie Lee Lake at Highway 291

Mannings Roughness Factors were determined from direct field observation of the existing channel conditions from a May 2021 site visit. Several photographs were taken at various locations along the creek in mid-spring when the foliage was nearly full grown. A photo collage including eight pictures taken at different locations along the creek have been included in the appendix of this study. After examining the photos, a Manning's Roughness Factor of n=0.045 was applied to the main channel based on the Table 3-1 recommendations for a Condition A.1.d 'Clean, winding, some pools and shoals, some weeds and stones.' CFS also used n=0.100 for the overbank flood plains based on the Table 3-1 recommendations for a Condition to dense brush in summer.'

The HEC-RAS model was setup as a 1D, steady state flow. A mixed flow regime was used to allow for changes between subcritical and supercritical flow at different locations along the channel. Divided channel sections at RS-10856.09, RS-10658.06, RS-10495.32 and RS-10280.58 were blocked-out using the stream obstruction routines in the cross-section modeler. The roadway surfaces at Swann Circle and Independence Avenue were treated as weirs with with weir-flow coefficients of 2.9 for modeling stream flows that overtopped the streets in heavy storms. Roadway surface widths were approximated from the GIS topography or from available roadway plans. The Reach Boundary conditions used the Normal Flow conditions with the approximate channel bed slopes at the upstream and downstream-most cross-sections.



Schematic Off-Site Drainage Area Map for Tributary P3 to Prairie Lee Lake

<u>HEC-RAS Analysis:</u> CFS Engineers created a HEC-RAS model to evaluate the water surface elevations of stormwater in the Tributary P3 to Prairie Lee Lake creek channel along the north side of the proposed Summit Point Apartments, Phase II site. Stream flows were calculated at four locations along the northern property line and where the creek crosses NE Swann Circle and NE Independence Avenue.

Drainage areas were estimated using the City GIS mapping. Channel cross-sections for the HEC-RAS models were cut across the surveyed ground surface, based on a recent topographic field survey of the site performed by CFS Engineers. The FEMA FIRM Flood Map of the region (FEMA FIRM Flood Map 29095C0436G, Panel 436 of 625, January 20, 2017), shows the Tributary P3 to Prairie Lee Lake directly to the north of the site as Zone A, defined as a Special Flood Hazard Area subject to inundation by the 1% annual chance flood where no base flood elevations have been determined. The time of

concentration for each drainage basin was determined using TR-55 methodology for overland flow, shallow concentrated flow and channelized flow segments. Estimates of flow path length and slope were estimated using Jackson County GIS topography and the USGS Quadrangle maps of the vicinity. The USGS StreamStats web-based hydrologic analysis program was used to check the contributing drainage areas to the points of interest along the creek. The StreamStat flows appeared to be overly conservative when checked against the flows calculated using the conventional TR-55 methodologies. Calculations have been included in the appendix.

The existing triple 48" HDPE culverts at NE Swann Circle and the existing 48" RCP culvert at NE Independence Avenue were also included in the HEC-RAS model to evaluate the potential back-up of flood water in the creek from the culvert crossings. Channel cross-sections were cut along the Summit Point Apartments, Phase-II site, and additional cross-sections were cut downstream to model the NE Swann Circle and NE Independence Avenue culverts. CFS surveyors measured the invert elevations of all three 48" HDPE culverts along with the top of road elevation at NE Swann Circle. Available storm sewer as-built plans were used to model the existing 48" RCP culvert at NE Independence Avenue.

<u>Current Effective Model:</u> The FEMA FIRM flood map showed the creek within the limits of Zone AE east of NE Independence Avenue, and within Zone A upstream of NE Independence Avenue to the Summit Point site. The flood map showed that the creek along the northern side of Summit Point, was set inside Zone A where the defined base flood elevations were not determined. This indicates that the detailed HEC-RAS model of the Tributary P3 to Prairie Lee Lake stopped short of NE Independence Avenue and did not extend upstream to cover the Summit Point site.

Duplicate Effective Model: CFS Engineers created a HEC-RAS model using the recent topographic survey and the Jackson County GIS data. The cross-sections cut along the northern side of the Summit Point site and other cross-sections cut further downstream to model the NE Swann Circle roadway crossing and the NE Independence Avenue roadway crossing. The methods for setting the left and right bank stations and overbank lengths were described above. RS 8693.92 was the furthest downstream located approximately 530 ft downstream from the centerline of NE Independence Avenue. RS 11275.44 was the furthest upstream located on the western side of the Summit Point site. The stream flows for the 2 year storm ranged from 144 cfs at the lower end of the creek by NE Independence Avenue, to 77 cfs at the upstream end. Likewise, the 10-year stream flows ranged from 273 cfs to 146 cfs, and the 100-year stream flows ranged from 485 cfs to 264 cfs. The channel slope averaged approximately 1.1%. 100-year flow depths along the Summit Point site ranged from 2.58 ft to 4.42 ft, with corresponding flow velocities ranging from 3.52 fps to 6.78 fps.

<u>Alternate Scenario Model for Clogged Independence Avenue Culvert:</u> Using the Duplicate Effective Model, CFS modified the model to change the 48" diameter RCP culvert at Independence Avenue to a 6" diameter pipe for the purpose of simulating a clogged culvert condition. With the dramatically reduced culvert size, the heavier stream flows would overtop the roadway and increase the backwater depth at the Swann Circle culvert crossing. The reason for performing the scenario was two-fold. The first reason was that the available record construction plans for Independence Avenue showed a 48" RCP culvert under the roadway connecting into a 54" RCP before discharging into the open creek, and the City's current digital stormwater utility data showing that two 24" diameter pipes were installed downstream from the existing 48" RCP and 54" RCP. While the configuration of installing two 24" pipes downstream from a 54" makes little hydraulic sense, it could have been done purposely to constrict the channel flow and store runoff in the channel upstream of Independence Avenue. Since the Independence Avenue channel crossing was over 1000' downstream from the Summit Point site, CFS Engineers did not survey the area. The second reason for performing the clogged Independence Avenue culvert scenario was to model the worst-case conditions where the stream flow would be forced to overtop the roadway and possibly cause an increased tailwater backup at the Swann Circle culvert and higher WSEL's along the Summit Point site. After running the clogged Independence Avenue culvert scenario, it was found that the increased downstream tailwater depth had negligible impact on the WSEL's, and that the base flood elevations were unaffected along the Summit Point site.

<u>On-Site Small Local Flow Drainage Channel South of Tributary P3:</u> A HEC-RAS model was created to model the small local flow drainage channel which branches south off the main Tributary P3, and to determine the maximum 100-year WSEL on the proposed site. The small channel model re-used the same cross-sections as the Tributary P3 channel except the stream obstructions were switched so that the main channel was blocked from conveying the lower amount of runoff contributed directly from the site. The stormwater drainage and detention study for the proposed Summit Point Phase-II Apartments included pre-development and post-development runoff calculations from the 7.21 acre Phase-II site and portions of the 6.49 acre Phase-I site that drained northwards over the Phase-II site grounds. The total runoff from the Phase I and II sites was routed into the existing small channel to determine the 100-year WSEL's, which are summarized in the following table:

Cross Section	100-Year Flow	100-Year WSEL On-Site South Channel	100-Year WSEL Main Trib P3 Channel
RS 10097.67	97 cfs	994.92'	994.92'
RS 10280.58	97 cfs	994.87'	994.65'
RS 10495.32	65 cfs	996.88'	997.34'
RS 10658.06	32 cfs	998.81'	999.79'

<u>Summary:</u> The HEC-RAS calculations for the Tributary P3 main channel comparing the open and clogged culvert conditions at Independence Avenue showed no change in 100-WSEL's in the channel section along the proposed Summit Point Phase-II site. The HEC-RAS calculations for the small local flow drainage channel branching off of Tributary P3 showed the highest 100-year WSEL on the site at 998.81', and the lowest proposed building had a finish floor elevations of 1005.00'. Since the Summit Point Phase-II site is providing post-development detention, and since the 7.21 acre site comprises approximately six percent of the overall contributing watershed area, Tributary P3 would experience minimal changes in the overall pre and post-development flow rates.



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	JMMIT POINT Designed by: RP Date: Rev. NE Chipman Road RP 03-25-2015 - NE Chipman Road Dwn by: Ckd by: Reviewed by: Summit, Missouri RP LWS Nary Development Plan File name#re_Development Drainage area Mapdgn File name#re_Development Drainage area Mapdgn
	PRE-DEVELOPMENT CONDITION DRAINAGE AREA MAP Prelimino
● 50′ 100′ 50′ FEET	Sheet reference number: DAM-1





STORMWATER DETENTION BASIN CONTRIBUTING DRAINAGE AREA ON-SITE A = 5.53 ACRES CN = 90.5Tc = 5 min. ON-SITE A = 4.21 ACRES CN = 88.5Tc = 9 min.

PEAK 100 YR WATER SURFACE ELEVATION = 1000.5 30' OVERFLOW SPILLWAY CREST = 1001.00 EMERGENCY OVERFLOW = 1002.01 TOP OF DAM = 1003.25

				CISE.COM 1421 E. 104th Street, Ste. 100 KCMO 64131 0: 816-333-4477 f: 816-333-6688		
HIGH RECOSTON				312	L MANAGER & MINING	
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HEC-RAS Study for Summit Point-II Tributary P3 to Prairie Lee Lake Photographs of Main Creek Channel May, 2021



RS-10097-Main Channel by Swann Circle Detention Basin Looking DS, 05-11-21



RS-10280-Main Channel Looking DS, 05-11-21



RS-10495-1 Main Channel Looking DS, 05-11-21



RS-10495-2 South Bank near Center of Proposed Building C1-2, 05-11-21

HEC-RAS Study for Summit Point-II Tributary P3 to Prairie Lee Lake Photographs of Main Creek Channel May, 2021



RS-10658-1 Main Channel Looking DS by West End of Prop Building C1-2, 05-11-21



RS-10658-2 Side Tributary Looking US, 05-11-21



RS-10856 Main Channel Looking DS, 05-11-21



RS-11275 Main Channel Looking DS, 05-11-21

NOTES TO USERS

s map is for use in administering the National Flood Insurance Program. It does necessarily identify all areas subject to flooding, particularly from local drainage urces of small size. The community **map repository** should be consulted for suble updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) Report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent mounded whole/how elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS Report should be utilized in conjunction with the EIDM for murpase of construction particip flooding meanagement. FIRM for purposes of construction and/or floodplain management

Boundaries of the floodways were computed at cross sections and interpolate between cross sections. The floodways were based on hydraulic considerations wit regard to requirements of the National Flood Insurance Program. Floodway with and other pertinent floodway data are provided in the Flood Insurance Study Repor

Certain areas not in Special Flood Hazard Areas may be protected by **flood contro** structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insuranc Study Report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Missouri State Plane WestZone (FIPS zone 2403). The horizontal datum was NAD 83. GRS 1980 spheroid. Differences in datum, spheroid, projection or UTX zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do no affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <u>http://www.ngs.ngaa.gov</u> or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3 #9202 33MC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282 (301) 713-3242

To obtain current elevation, description, and/or location information for bench mark hown on this map, please contact the Information Services Branch of the Nation eodetic Survey at (301) 713- 3242, or visit its website at <u>http://www.ngs.noaa.gov</u>.

Base map information shown on this FIRM was derived from the U.S.D.A Farm Service National Agriculture ImageryProgram (NAIP) dated 2014. Produced at scale of 1:24,000.

The profile baselines depicted on this map represent the hydraulic modeling baseline that match the flood profiles in the FIS report. As a result of improved topographic data the profile baseline, in some cases, may deviate significantly from the channe centerline or appear outside the SFHA.

Based on updated topographic information, this map reflects more detailed and up-to-date stream channel configurations and floodplain delineations than those shown on the previous FRM for this jurisdicion. As a result, the Flood Profiles and Floodway Data tables for multiple streams in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on the map. Also, the road to floodplain relationships for unrevised streams may differ from what is shown on previous maps.

orporate limits shown on this map are based on the best data available at the time f publication. Because changes due to annexations or de-annexations may have ccurred after this map was published, map users should contact appropriate smmunity officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community lownow

For information on available products associated with this FIRM visit the Map Service Center (MSC) website at <u>http://msc.fema.gov</u>, Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the MSC website.



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38° 54' 22.5"

94" 22' 30"

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TABLE 3 – SUMMARY OF DISCHARGES (CONT'D)	DRAINAGE AREA I-Percent DRAINAGE AREA 10-Percent 4-Percent- 2-Percent- 1-Percent (sq. miles) Annual Chance Annual-Chance Annual-Chance Annual-Chance Annual-Chance	0.6 700 N/A 1,000 1,100 1,400	2.9 2,400 N/A 3,400 3,900 1,800	2.7 2,200 N/A 3,200 3,700 4,400	1.9 1,900 N/A 2,800 3,200 3,800		1.7 1,800 N/A 2,600 3,000 3.600		0.7 1,000 N/A 1,400 1,600 1,900			0.0 1,100 1,100 1,300 1,300	0.4 450 N/A 650 750 900		
TABLE 3 – SUMMARY OI	AINAGE AREA <u>10-Percent</u> (sq. miles) Annual Chance	0.6 700	2.9 2,400	2.7 2,200	1.9 1.900		1.7 1,800		0.7 1,000			0.0	0.4 450		0.8 1100
	FLOODING SOURCE DR AND LOCATION TRIBUTARY P1 TO PRAIRIE LEE TABUTARY P1 TO PRAIRIE LEE	ADDE (CONT. D) Approximately 750 feet downstream of State HWY 291 TRIBUTARY P2 TO PRAIRIE LEE LAKE	At confluence with Prairie Lee Lake	Approximately 0.5 miles upstream of confluence with Prairie Lee Lake	At confluence of Tributary P3 to Prairie Lee Lake	Approximately 1,900 feet	downstream of confluence of	Tributary P4 to Prairie Lee Lake	At confluence of Tributary P4 to Prairie Lee Lake	 TRIBUTARY P3 TO PRAIRIE LEE LAKE 	At confluence with Tributary P2 to	Prairie Lee Lake	At State HWY 291	TRIBUTARY P4 TO PRAIRIE LEE LAKE	At confluence with Triburary P2 to

StreamStats Report-291 Highway over Tributary P-3 to Prairie Lee Lake

 Region ID:
 MO

 Workspace ID:
 MO20210511145832820000

 Clicked Point (Latitude, Longitude):
 38.92910, -94.35968

 Time:
 2021-05-11 09:58:50 -0500

StreamStat-RS-04349-Hwy-291



Basin Characteristics

Parameter			
Code	Parameter Description	Value	Unit
DRNAREA	Area that drains to a point on a stream	0.37	square miles
BSHAPE	Basin Shape Factor for Area	4.83	dimensionless
IMPNLCD01	Percentage of impervious area determined from NLCD 2001 impervious dataset	36.8	percent
CSL10_85	Change in elevation divided by length between points 10 and 85 percent of distance along main channel to basin divide - main channel method not known	62.9	feet per mi
LFPLENGTH	Length of longest flow path	1.34	miles
STORAGE	Percentage of area of storage (lakes ponds reservoirs wetlands)	0.1872	percent
STREAM_VARG	Streamflow variability index as defined in WRIR 02-4068, computed from regional grid	0.56	dimension l ess

Peak-Flow Statistics Param	eters [Peak Rural Statewide Region 1 SIR 2014 5	165]				
Parameter Code	Parameter Name	Value	Units		Min Limit	Max Limit
DRNAREA	Drainage Area	0.37	square	miles	0.11	8212.38
BSHAPE	Basin Shape Factor	4.83	dimens	sionless	2.25	26.59
Peak-Flow Statistics Param	eters [Peak Urban Statewide SIR 2010 5073]					
Parameter Code	Parameter Name		Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area		0.37	square miles	0.28	189
IMPNLCD01	Percent Impervious NLCD2001		36.8	percent	2.3	46
Peak-Flow Statistics Flow R	eport [Peak Rural Statewide Region 1 SIR 2014 5	i165]				
PII: Prediction Interval-	Lower, Plu: Prediction Interval-Upper, SE	p: Standard Er	ror of Pred	iction, SE: Standar	d Error (other see	report)
Statistic			Value	Un	iit	SEp
50-percent AEP flood			136	ft^	3/s	38.4
20-percent AEP flood			265	ft^	3/s	30.8

021		StreamStats	StreamStat	t-RS-04	349-Hwy	/-291
Statistic		Value	Unit		SEp	
10-percent AEP flood		366	ft^3/s		29.1	
4-percent AEP flood		507	ft^3/s		28.8	
2-percent AEP flood		619	ft^3/s		28.7	
1-percent AEP flood		733	ft^3/s		29.8	
0.5-percent AEP flood		848	ft^3/s		31	
0.2-percent AEP flood		NaN	ft^3/s		33.2	
PII: Prediction Interval-Lower, PIu: Predicti Statistic	on Interval-Upper, SEp: Standard Value	Error of Predictic Unit	on, SE: Standard Error PII	⁻ (other see Plu	report) SEp	
50-percent AEP flood	339	ft^3/s	212	F 4 2		
				545	26.7	
20-percent AEP flood	496	ft^3/s	332	741	26.7 23.3	
20-percent AEP flood 10-percent AEP flood	496 626	ft^3/s ft^3/s	332 419	741 935	26.7 23.3 22.1	
20-percent AEP flood 10-percent AEP flood 4-percent AEP flood	496 626 729	ft^3/s ft^3/s ft^3/s	332 419 494	741 935 1080	26.7 23.3 22.1 22.1	
20-percent AEP flood 10-percent AEP flood 4-percent AEP flood 2-percent AEP flood	496 626 729 892	ft^3/s ft^3/s ft^3/s ft^3/s	332 419 494 584	741 935 1080 1360	26.7 23.3 22.1 22.1 23.3	

0.2-percent AEP flood

Peak-Flow Statistics Citations

Southard, R.E.,2010, Estimation of the Magnituude and Frequency of Floods in Urban Basins in Missouri: U.S. Geological Survey Scientific Investigations Report 2010-5073, 27 p. (http://pubs.usgs.gov/sir/2010/5073/)

ft^3/s

698

2350

35.2

1280

Southard, R.E., and Veilleux, A.G.,2014, Methods for estimating annual exceedance-probability discharges and largest recorded floods for unregulated streams in rural Missouri: U.S. Geological Survey Scientific Investigations Report 2014–5165, 39 p. (http://pubs.usgs.gov/sir/2014/5165/)

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Application Version: 4.5.3 StreamStats Services Version: 1.2.22 NSS Services Version: 2.1.1

StreamStats Report - Summit Point Phase-II, RS 8962



Basin Characterist	ics		
Parameter Code	Parameter Description	Value	Unit
DRNAREA	Area that drains to a point on a stream	0.26	square miles
BSHAPE	Basin Shape Factor for Area	3.68	dimensionless
IMPNLCD01	Percentage of impervious area determined from NLCD 2001 impervious dataset	32	percent
CSL10_85	Change in elevation divided by length between points 10 and 85 percent of distance along main channel to basin divide - main channel method not known	69.2	feet per mi
LFPLENGTH	Length of longest flow path	0.99	miles

StreamStats

Parameter	StreamStat-RS-08962-Independence Ave						
Code	Parameter Description	Value	Unit				
STORAGE	Percentage of area of storage (lakes ponds reservoirs wetlands)	0.1314	percent				
STREAM_VARG	Streamflow variability index as defined in WRIR 02- 4068, computed from regional grid	0.56	dimensionless				

Peak-Flow Statistics Parameters [Peak Rural Statewide Region 1 SIR 2014 5165]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	0.26	square miles	0.11	8212.38
BSHAPE	Basin Shape Factor	3.68	dimensionless	2.25	26.59

Peak-Flow Statistics Parameters [Peak Urban Statewide SIR 2010 5073]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	0.26	square miles	0.28	189
IMPNLCD01	Percent Impervious NLCD2001	32	percent	2.3	46

Peak-Flow Statistics Flow Report [Peak Rural Statewide Region 1 SIR 2014 5165]

PII: Prediction Interval-Lower, Plu: Prediction Interval-Upper, SEp: Standard Error of Prediction, SE: Standard Error (other -- see report)

Statistic	Value	Unit	SEp
50-percent AEP flood	118	ft^3/s	38.4
20-percent AEP flood	231	ft^3/s	30.8
10-percent AEP flood	320	ft^3/s	29.1
4-percent AEP flood	444	ft^3/s	28.8
2-percent AEP flood	543	ft^3/s	28.7
1-percent AEP flood	643	ft^3/s	29.8
0.5-percent AEP flood	744	ft^3/s	31
0.2-percent AEP flood	881	ft^3/s	33.2

Peak-Flow Statistics Disclaimers [Peak Urban Statewide SIR 2010 5073]

One or more of the parameters is outside the suggested range. Estimates were extrapolated with unknown errors

Peak-Flow Statistics Flow Report [Peak Urban Statewide SIR 2010 5073]

Statistic	Value	Unit
50-percent AEP flood	235	ft^3/s
20-percent AEP flood	357	ft^3/s
10-percent AEP flood	454	ft^3/s
4-percent AEP flood	540	ft^3/s
2-percent AEP flood	660	ft^3/s
1-percent AEP flood	732	ft^3/s
0.2-percent AEP flood	960	ft^3/s

Peak-Flow Statistics Citations

Southard, R.E.,2010, Estimation of the Magnituude and Frequency of Floods in Urban Basins in Missouri: U.S. Geological Survey Scientific Investigations Report 2010-5073, 27 p. (http://pubs.usgs.gov/sir/2010/5073/)

Southard, R.E., and Veilleux, A.G.,2014, Methods for estimating annual exceedanceprobability discharges and largest recorded floods for unregulated streams in rural Missouri: U.S. Geological Survey Scientific Investigations Report 2014-5165, 39 p. (http://pubs.usgs.gov/sir/2014/5165/)

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Application Version: 4.5.1 StreamStats Services Version: 1.2.22 NSS Services Version: 2.1.1

StreamStats Report

StreamStat-RS-10280-Swann Circle

 Region ID:
 MO

 Workspace ID:
 MO20200521225634594000

 Clicked Point (Latitude, Longitude):
 38.92757, -94.36734

 Time:
 2020-05-21 17:56:54 -0500



Basin Characteristics					
Parameter Code	Parameter Description	Value	Unit		
DRNAREA	Area that drains to a point on a stream	0.21	square miles		
BSHAPE	Basin Shape Factor for Area	2.95	dimensionless		
IMPNLCD01	Percentage of impervious area determined from NLCD 2001 impervious dataset	25.9	percent		
LFPLENGTH	Length of longest flow path	0.78	miles		
STREAM_VARG	Streamflow variability index as defined in WRIR 02- 4068, computed from regional grid	0.56	dimensionless		

Parameter	StreamStat-RS-10280-Swann Circle			
Code	Parameter Description	Value	Unit	
CSL10_85	Change in elevation divided by length betweer points 10 and 85 percent of distance along ma channel to basin divide - main channel methoo known	n 75.8 ain d not	feet per mi	
STORAGE	Percentage of area of storage (lakes ponds reservoirs wetlands)	0.1673	percent	

Peak-Flow Statistics Parameters [Peak Rural Statewide Region 1 SIR 2014 5165]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	0.21	square miles	0.11	8212.38
BSHAPE	Basin Shape Factor	2.95	dimensionless	2.25	26.59

Peak-Flow Statistics Parameters [Peak Urban Statewide SIR 2010 5073]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	0.21	square miles	0.28	189
IMPNLCD01	Percent Impervious NLCD2001	25.9	percent	2.3	46

Peak-Flow Statistics Flow Report [Peak Rural Statewide Region 1 SIR 2014 5165]

PII: Prediction Interval-Lower, PIu: Prediction Interval-Upper, SEp: Standard Error of Prediction, SE: Standard Error (other -- see report)

Statistic	Value	Unit	SEp
2 Year Peak Flood	110	ft^3/s	38.4
5 Year Peak Flood	216	ft^3/s	30.8
10 Year Peak Flood	299	ft^3/s	29.1
25 Year Peak Flood	415	ft^3/s	28.8
50 Year Peak Flood	508	ft^3/s	28.7
100 Year Peak Flood	602	ft^3/s	29.8
200 Year Peak Flood	696	ft^3/s	31
500 Year Peak Flood	824	ft^3/s	33.2

Peak-Flow Statistics Disclaimers[Peak Urban Statewide SIR 2010 5073]

One or more of the parameters is outside the suggested range. Estimates were extrapolated with unknown errors

Peak-Flow Statistics Flow Report [Peak Urban Statewide SIR 2010 5073]

Statistic	Value	Unit
2 Year Peak Flood	170	ft^3/s
5 Year Peak Flood	269	ft^3/s
10 Year Peak Flood	348	ft^3/s
25 Year Peak Flood	425	ft^3/s
50 Year Peak Flood	519	ft^3/s
100 Year Peak Flood	582	ft^3/s
500 Year Peak Flood	773	ft^3/s

Peak-Flow Statistics Citations

Southard, R.E., and Veilleux, A.G.,2014, Methods for estimating annual exceedanceprobability discharges and largest recorded floods for unregulated streams in rural Missouri: U.S. Geological Survey Scientific Investigations Report 2014–5165, 39 p. (http://pubs.usgs.gov/sir/2014/5165/)

Southard, R.E.,2010, Estimation of the Magnituude and Frequency of Floods in Urban Basins in Missouri: U.S. Geological Survey Scientific Investigations Report 2010-5073, 27 p. (http://pubs.usgs.gov/sir/2010/5073/)

Low-Flow Statistics Parameters [LowFlow Region 1 SIR 2013 5090]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	0.21	square miles	0.34	4320
LFPLENGTH	LFP length	0.78	miles	1.28	268
STREAM_VARG	Streamflow Variability Index from Grid	0.56	dimensionless	0.376	1.03

Low-Flow Statistics Disclaimers[LowFlow Region 1 SIR 2013 5090]

One or more of the parameters is outside the suggested range. Estimates were extrapolated with unknown errors

Low-Flow Statistics Flow Report[LowFlow Region 1 SIR 2013 5090]

Statistic	StreamStat-RS-1 Value	0280-Swann Circle Unit
1 Day 10 Year Low Flow	0.0000296	ft^3/s
2 Day 10 Year Low Flow	0.0000309	ft^3/s
3 Day 10 Year Low Flow	0.0000369	ft^3/s
7 Day 10 Year Low Flow	0.0000494	ft^3/s
10 Day 10 Year Low Flow	0.0000536	ft^3/s
30 Day 10 Year Low Flow	0.000411	ft^3/s
60 Day 10 Year Low Flow	0.00065	ft^3/s

Low-Flow Statistics Citations

Southard, R.E.,2013, Computed statistics at streamgages, and methods for estimating lowflow frequency statistics and development of regional regression equations for estimating low-flow frequency statistics at ungaged locations in Missouri: U.S. Geological Survey Scientific Investigations Report 2013-5090, 28 p. (http://pubs.usgs.gov/sir/2013/5090/)

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Application Version: 4.3.11

StreamStats Report

StreamStat-RS-10658

 Region ID:
 MO

 Workspace ID:
 MO20200522140912600000

 Clicked Point (Latitude, Longitude):
 38.92745, -94.36902

 Time:
 2020-05-22 09:09:30 -0500



Basin Characteristics					
Parameter Code	Parameter Description	Value	Unit		
DRNAREA	Area that drains to a point on a stream	0.18	square miles		
BSHAPE	Basin Shape Factor for Area	2.52	dimensionless		
IMPNLCD01	Percentage of impervious area determined from NLCD 2001 impervious dataset	23.6	percent		
LFPLENGTH	Length of longest flow path	0.67	miles		
STREAM_VARG	Streamflow variability index as defined in WRIR 02- 4068, computed from regional grid	0.56	dimensionless		

Parameter Code	Parameter Description	Stream Value	Stat-RS-10658 Unit
CSL10_85	Change in elevation divided by length between points 10 and 85 percent of distance along main channel to basin divide - main channel method not known	88.2	feet per mi
STORAGE	Percentage of area of storage (lakes ponds reservoirs wetlands)	0.086	percent

Peak-Flow Statistics Parameters [Peak Rural Statewide Region 1 SIR 2014 5165]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	0.18	square miles	0.11	8212.38
BSHAPE	Basin Shape Factor	2.52	dimensionless	2.25	26.59

Peak-Flow Statistics Parameters [Peak Urban Statewide SIR 2010 5073]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	0.18	square miles	0.28	189
IMPNLCD01	Percent Impervious NLCD2001	23.6	percent	2.3	46

Peak-Flow Statistics Flow Report [Peak Rural Statewide Region 1 SIR 2014 5165]

PII: Prediction Interval-Lower, PIu: Prediction Interval-Upper, SEp: Standard Error of Prediction, SE: Standard Error (other -- see report)

Statistic	Value	Unit	SEp
2 Year Peak Flood	105	ft^3/s	38.4
5 Year Peak Flood	205	ft^3/s	30.8
10 Year Peak Flood	285	ft^3/s	29.1
25 Year Peak Flood	396	ft^3/s	28.8
50 Year Peak Flood	484	ft^3/s	28.7
100 Year Peak Flood	574	ft^3/s	29.8
200 Year Peak Flood	663	ft^3/s	31
500 Year Peak Flood	786	ft^3/s	33.2

Peak-Flow Statistics Disclaimers[Peak Urban Statewide SIR 2010 5073]

One or more of the parameters is outside the suggested range. Estimates were extrapolated with unknown errors

Peak-Flow Statistics Flow Report [Peak Urban Statewide SIR 2010 5073]

Statistic	Value	Unit
2 Year Peak Flood	144	ft^3/s
5 Year Peak Flood	232	ft^3/s
10 Year Peak Flood	301	ft^3/s
25 Year Peak Flood	371	ft^3/s
50 Year Peak Flood	453	ft^3/s
100 Year Peak Flood	511	ft^3/s
500 Year Peak Flood	681	ft^3/s

Peak-Flow Statistics Citations

Southard, R.E., and Veilleux, A.G.,2014, Methods for estimating annual exceedanceprobability discharges and largest recorded floods for unregulated streams in rural Missouri: U.S. Geological Survey Scientific Investigations Report 2014–5165, 39 p. (http://pubs.usgs.gov/sir/2014/5165/)

Southard, R.E.,2010, Estimation of the Magnituude and Frequency of Floods in Urban Basins in Missouri: U.S. Geological Survey Scientific Investigations Report 2010-5073, 27 p. (http://pubs.usgs.gov/sir/2010/5073/)

Low-Flow Statistics Parameters[LowFlow Region 1 SIR 2013 5090]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	0.18	square miles	0.34	4320
LFPLENGTH	LFP length	0.67	miles	1.28	268
STREAM_VARG	Streamflow Variability Index from Grid	0.56	dimensionless	0.376	1.03

Low-Flow Statistics Disclaimers[LowFlow Region 1 SIR 2013 5090]

One or more of the parameters is outside the suggested range. Estimates were extrapolated with unknown errors

Low-Flow Statistics Flow Report[LowFlow Region 1 SIR 2013 5090]

Statistic	Value	StreamStat-RS-10658 Unit
1 Day 10 Year Low Flow	0.0000265	ft^3/s
2 Day 10 Year Low Flow	0.0000278	ft^3/s
3 Day 10 Year Low Flow	0.0000331	ft^3/s
7 Day 10 Year Low Flow	0.0000434	ft^3/s
10 Day 10 Year Low Flow	0.0000482	ft^3/s
30 Day 10 Year Low Flow	0.000372	ft^3/s
60 Day 10 Year Low Flow	0.00061	ft^3/s

Low-Flow Statistics Citations

Southard, R.E.,2013, Computed statistics at streamgages, and methods for estimating lowflow frequency statistics and development of regional regression equations for estimating low-flow frequency statistics at ungaged locations in Missouri: U.S. Geological Survey Scientific Investigations Report 2013-5090, 28 p. (http://pubs.usgs.gov/sir/2013/5090/)

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Application Version: 4.3.11

StreamStats Report

StreamStat-RS-11275

 Region ID:
 MO

 Workspace ID:
 MO20200522140203726000

 Clicked Point (Latitude, Longitude):
 38.92611, -94.37097

 Time:
 2020-05-22 09:02:21 -0500



Basin Characteristi	CS		
Parameter Code	Parameter Description	Value	Unit
DRNAREA	Area that drains to a point on a stream	0.13	square miles
BSHAPE	Basin Shape Factor for Area	1.89	dimensionless
IMPNLCD01	Percentage of impervious area determined from NLCD 2001 impervious dataset	23	percent
LFPLENGTH	Length of longest flow path	0.5	miles
STREAM_VARG	Streamflow variability index as defined in WRIR 02- 4068, computed from regional grid	0.56	dimensionless

Parameter		Stream	Stat-RS-11275
Code	Parameter Description	Value	Unit
CSL10_85	Change in elevation divided by length between points 10 and 85 percent of distance along main channel to basin divide - main channel method not known	104	feet per mi
STORAGE	Percentage of area of storage (lakes ponds reservoirs wetlands)	0	percent

Peak-Flow Statistics Parameters [Peak Rural Statewide Region 1 SIR 2014 5165]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	0.13	square miles	0.11	8212.38
BSHAPE	Basin Shape Factor	1.89	dimensionless	2.25	26.59

Peak-Flow Statistics Parameters [Peak Urban Statewide SIR 2010 5073]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	0.13	square miles	0.28	189
IMPNLCD01	Percent Impervious NLCD2001	23	percent	2.3	46

Peak-Flow Statistics Disclaimers[Peak Rural Statewide Region 1 SIR 2014 5165]

One or more of the parameters is outside the suggested range. Estimates were extrapolated with unknown errors

Peak-Flow Statistics Flow Report [Peak Rural Statewide Region 1 SIR 2014 5165]

Statistic	Value	Unit
2 Year Peak Flood	93	ft^3/s
5 Year Peak Flood	183	ft^3/s
10 Year Peak Flood	254	ft^3/s
25 Year Peak Flood	353	ft^3/s
50 Year Peak Flood	432	ft^3/s
100 Year Peak Flood	513	ft^3/s
200 Year Peak Flood	593	ft^3/s

Statistic	Strea Value	amStat-RS-11275 Unit
500 Year Peak Flood	702	ft^3/s

Peak-Flow Statistics Disclaimers[Peak Urban Statewide SIR 2010 5073]

One or more of the parameters is outside the suggested range. Estimates were extrapolated with unknown errors

Peak-Flow Statistics Flow Report [Peak Urban Statewide SIR 2010 5073]

Statistic	Value	Unit
2 Year Peak Flood	116	ft^3/s
5 Year Peak Flood	188	ft^3/s
10 Year Peak Flood	245	ft^3/s
25 Year Peak Flood	302	ft^3/s
50 Year Peak Flood	368	ft^3/s
100 Year Peak Flood	415	ft^3/s
500 Year Peak Flood	552	ft^3/s

Peak-Flow Statistics Citations

Southard, R.E., and Veilleux, A.G.,2014, Methods for estimating annual exceedanceprobability discharges and largest recorded floods for unregulated streams in rural Missouri: U.S. Geological Survey Scientific Investigations Report 2014–5165, 39 p. (http://pubs.usgs.gov/sir/2014/5165/)

Southard, R.E.,2010, Estimation of the Magnituude and Frequency of Floods in Urban Basins in Missouri: U.S. Geological Survey Scientific Investigations Report 2010-5073, 27 p. (http://pubs.usgs.gov/sir/2010/5073/)

Low-Flow Statistics Parameters[LowFlow Region 1 SIR 2013 5090]										
Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit					
DRNAREA	Drainage Area	0.13	square miles	0.34	4320					
LFPLENGTH	LFP length	0.5	miles	1.28	268					
STREAM_VARG	Streamflow Variability Index from Grid	0.56	dimensionless	0.376	1.03					

One or more of the parameters is outside the suggested range. Estimates were extrapolated with unknown errors

Low-Flow Statistics Flow Report[LowFlow Region 1 SIR 2013 5090]

Statistic	Value	Unit
1 Day 10 Year Low Flow	0.0000199	ft^3/s
2 Day 10 Year Low Flow	0.000021	ft^3/s
3 Day 10 Year Low Flow	0.0000249	ft^3/s
7 Day 10 Year Low Flow	0.0000315	ft^3/s
10 Day 10 Year Low Flow	0.0000364	ft^3/s
30 Day 10 Year Low Flow	0.000289	ft^3/s
60 Day 10 Year Low Flow	0.000503	ft^3/s

Low-Flow Statistics Citations

Southard, R.E.,2013, Computed statistics at streamgages, and methods for estimating lowflow frequency statistics and development of regional regression equations for estimating low-flow frequency statistics at ungaged locations in Missouri: U.S. Geological Survey Scientific Investigations Report 2013-5090, 28 p. (http://pubs.usgs.gov/sir/2013/5090/)

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Application Version: 4.3.11

Tributary P-3 to Prairie Lee Lake Open Channel Manning's Roughness Coefficients Sensitivity Comparison Summit Point Apartments, Phase-II 05/12/21

n = 0.050 (MC) N = 0.160 (OB) Maximum W.S. Elev 976.53 987.46 979.27 981.28 989.33 994.94 999.88 1001.89 1003.84 1005.35 987.34 994.69 997.61 991 (ft) n = 0.045 (MC) N = 0.100 (OB) W.S. Elev 1005.15 987.33 987.38 990.82 994.92 994.65 1001.68 976.15 979.04 989.34 1003.71 Normal 981.09 997.34 999.79 (ft) n = 0.035 (MC) N = 0.070 (OB)W.S. Elev Minimum 987.34 999.35 1003.44 975.56 989.33 1004.66 979.06 980.34 987.34 994.89 994.59 1001.27 997.21 90.06 (ft) Culvert – Independence Avenue Min Ch El 972.75 1000.7 974.61 978.2 979.3 989.9 993.4 995.9 997.5 9.996 970 984 986 987 Culvert – Swahn Circle (ft) Q Total (cfs) 643 643 643 602 602 602 602 602 602 574 574 513 513 513 Profile 100yr CFS Project No. 19-5293 / 21-5065 10658.06 10032.93 10280.58 10495.32 10856.09 11086.04 **River Sta** 10097.67 11275.44 9223.75 9303.52 9890.22 8833.36 9566.40 8693.92 8962.51 9759.91 SummitPoint Reach

Channel Roughness Coefficients Selection (Taken from Table 3-1, Mannings 'n' Values, from Chapter 3-Basic Data Requirements HEC-RAS River Analysis Systems, Hydraulic Reference Manual, Version 4.1, January 2010)

Main Channel - Condition-d, Clean, winding, some pools and shoals, some weeds and stones Minimum n=0.035, Normal n=0.045, Maximum n=0.050

Flood Plains – Condition-c, Brush, No. 5, Medium to dense brush, in summer Minimum n=0.070, Normal n=0.100, Maximum n=0.160

Table 3-1 Manning's 'n' Values

		Type of Channel and Description	Minimum	Normal	Maximum
A. Nat	ural Strea	ums			
. Mai	n Chann	els			
a.	Clean, str	aight, full, no rifts or deep pools	0.025	0.020	0.022
b.	Same as a	above, but more stones and weeds	0.023	0.030	0.033
с.	Clean, wi	nding, some pools and shoals	0.030	0.035	0.040
d.	Same as a	above, but some weeds and stones	0.035	0.040	0.045
e.	Same as a	bove, lower stages, more ineffective slopes and	0.033	0.045	0.050
se	ctions		0.040	0.048	0.033
f.	Same as "	d" but more stones	0.045	0.050	0.060
g.	Sluggish	reaches, weedy. deep pools	0.043	0.030	0.000
h.	Very wee	dy reaches, deep pools, or floodways with heavy stands	0.050	0.070	0.080
of	timber an	d brush	0.070	0.100	0.150
Floo	d Plaine				
. 1100	Pacture	no brush			
a.	1	Short grass	0.025	0.030	0.035
	1.	High grass	0.030	0.035	0.050
h	2. Cultiva	ted areas			
υ.	1	No grop	0.020	0.030	0.040
	1. 2	Matura row groups	0.025	0.035	0.045
	2.	Mature field groups	0.030	0.040	0.050
	J. Druch	Mature neid crops			
С.		Southand house heavy woods	0.035	0.050	0.070
	1.	Light haugh and trace in winter	0.035	0.050	0.060
	2.	Light brush and trees, in winter	0.040	0.060	0.080
	э. 1	Madium to dongo hrush in winter	0.045	0.070	0.110
	4.	Medium to dense brush, in winter	0.070	0.100	0.160
L.	Э. Ттоос	weatum to dense brush, in summer			
a.	1 rees	Cleared land with tree sturning the amount-	0.030	0.040	0.050
	1.	Cleared land with tree stumps, no sprouts	0.050	0.060	0.080
	2.	Same as above, but neavy sprouts	0.080	0.100	0.120
	3.	Heavy stand of timber, few down trees, little			
	4	undergrowth, flow below branches	0.100	0.120	0.160
	4.	Same as above, but with flow into branches			
	5.	Dense willows, summer, straight	0 1 1 0	0 1 5 0	0.200

with trees and brush on banks submerged

a. Bottom: gravels, cobbles, and few bouldersb. Bottom: cobbles with large boulders	0.030 0.040	0.040 0.050	0.050 0.070	
--	----------------	----------------	----------------	--

HEC-RAS Plan:	TribP3Channel	River: Summ	itPointOpenC	Reach: Summi	tPoint							
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
SummitPoint	8693.92	10yr	366.00	970.00	975.32	975.32	976.34	0.017216	8.49	61.24	44.97	0.82
SummitPoint	8693.92	50yr	619.00	970.00	976.10	976.33	977.45	0.019293	10.33	103.98	64.20	0.89
SummitPoint	8693.92	100yr	733.00	970.00	976.31	976.66	977.85	0.021323	11.22	117.87	68.47	0.95
SummitPoint	8693.92	500yr	1004.00	970.00	976.81	977.30	978.62	0.023397	12.64	154.71	78.68	1.01
SummitPoint	8833.36	10yr	366.00	972.75	978.10	977.87	978.93	0.019870	7.31	50.33	27.96	0.87
SummitPoint	8833.36	50yr	619.00	972.75	978.96	978.96	980.04	0.017349	8.50	92.73	70.92	0.86
SummitPoint	8833.36	100yr	733.00	972.75	979.34	979.34	980.39	0.015130	8.58	123.09	89.84	0.82
SummitPoint	8833.36	500yr	1004.00	972.75	980.02	980.02	981.04	0.012695	8.87	196.26	123.55	0.77
SummitPoint	8962.51	10yr	366.00	974.61	980.02		980.47	0.007556	5.37	68.19	24.31	0.56
SummitPoint	8962.51	50vr	619.00	974.61	981.03		981.68	0.009276	6.44	96.04	30.56	0.64
SummitPoint	8962 51	100vr	733.00	974 61	981.27		982.05	0.010770	7.08	103 51	32.03	0.69
SummitPoint	8962.51	500vr	1004.00	974.61	981.70		982.83	0.014585	8.53	117 70	34.64	0.82
	0002.01		100 1.00	07.1.01			002.00	0.011000	0.00		0	0.02
SummitPoint	9223 75		Culvert									
	5220.10		Guivent									
SummitDoint	0202 52	10.0	220.00	079.20	096.46	092.60	096.47	0.000150	1 2 2	679.04	202.54	0.00
SummitDaint	9303.52	FOrm	520.00	978.20	960.40	962.00	900.47	0.000139	1.33	070.94	202.04	0.09
SummiPoint	9303.52	50yi	543.00	976.20	960.99	963.57	967.01	0.000273	1.04	633.35	296.30	0.12
SummitPoint	9303.52	TODYF	643.00	978.20	987.15	983.83	987.18	0.000332	2.06	881.14	300.43	0.14
SummitPoint	9303.52	SUUYr	881.00	978.20	987.51	984.35	987.55	0.000459	2.51	991.68	309.78	0.16
0	0500.45	10										
SummitPoint	9566.40	10yr	320.00	979.30	986.49	983.01	986.55	0.000581	2.23	273.90	148.70	0.17
SummitPoint	9566.40	50yr	543.00	979.30	987.05	983.97	987.16	0.000997	3.13	363.86	174.45	0.23
SummitPoint	9566.40	100yr	643.00	979.30	987.22	984.45	987.36	0.001198	3.50	394.63	182.43	0.25
SummitPoint	9566.40	500yr	881.00	979.30	987.61	985.38	987.80	0.001609	4.24	469.03	200.41	0.30
SummitPoint	9759.91	10yr	299.00	984.00	987.92	987.92	988.92	0.027328	8.01	37.34	19.49	1.02
SummitPoint	9759.91	50yr	508.00	984.00	989.03	989.03	989.93	0.014736	7.91	91.92	82.66	0.81
SummitPoint	9759.91	100yr	602.00	984.00	989.35	989.35	990.24	0.013368	8.05	120.89	101.73	0.78
SummitPoint	9759.91	500yr	825.00	984.00	989.91	989.91	990.79	0.012024	8.47	187.12	135.61	0.76
SummitPoint	9890.22	10vr	299.00	986.00	989 79		989 86	0.002654	2.22	134.61	89.82	0.32
SummitPoint	9890.22	50vr	508.00	986.00	990.56		990.65	0.002169	2.40	211 56	107.95	0.02
SummitPoint	0800.22	100yr	602.00	986.00	990.82		000.00	0.002100	2.40	240.55	113.07	0.00
SummitPoint	9090.22	100yi	925.00	980.00	990.02		990.92	0.002112	2.50	240.55	102.07	0.30
Summeron	9090.22	SUUyr	625.00	900.00	991.33		991.45	0.002096	2.14	301.29	123.10	0.31
CummitDaint	10022.02		Culuert									
Summeron	10032.93		Cuiven									
Our sector	40007.07	40 -	000.00	007.00	000.00	000.04	000.07	0.000004	0.00	007.40	400.75	0.40
SummitPoint	10097.67	10yr	299.00	987.00	992.36	989.34	992.37	0.000224	0.89	337.48	139.75	0.10
SummitPoint	10097.67	50yr	508.00	987.00	994.63	989.93	994.64	0.000073	0.76	681.57	182.56	0.06
SummitPoint	10097.67	100yr	602.00	987.00	994.92	990.12	994.93	0.000083	0.84	736.90	196.68	0.07
SummitPoint	10097.67	500yr	825.00	987.00	995.30	990.49	995.32	0.000119	1.06	815.92	215.25	0.08
SummitPoint	10280.58	10yr	299.00	989.90	992.82	992.82	993.60	0.027697	7.10	42.14	27.79	1.02
SummitPoint	10280.58	50yr	508.00	989.90	994.38		994.81	0.009551	5.24	96.94	45.51	0.63
SummitPoint	10280.58	100yr	602.00	989.90	994.65		995.12	0.010110	5.47	110.01	50.60	0.65
SummitPoint	10280.58	500yr	825.00	989.90	994.90	994.43	995.60	0.014544	6.69	124.33	95.93	0.79
SummitPoint	10495.32	10yr	285.00	993.40	996.84	996.34	997.21	0.010847	4.86	58.70	33.98	0.65
SummitPoint	10495.32	50yr	484.00	993.40	997.06	996.97	997.89	0.022180	7.32	66.10	35.31	0.94
SummitPoint	10495.32	100yr	574.00	993.40	997.34	997.21	998.22	0.020571	7.53	76.22	36.82	0.92
SummitPoint	10495.32	500yr	786.00	993.40	998.04	997.72	998.94	0.016065	7.61	103.35	40.89	0.84
SummitPoint	10658,06	10vr	285.00	995.90	998.58		998.93	0,010327	4.78	59.60	34.37	0.64
SummitPoint	10658.06	50vr	484 00	905.00 905.00	900.50		999 93	0.007776	5 10	04 QR	40 05	0.54
SummitPoint	10658.06	100vr	574 00	995 00	900.00		1000.25	830800 0	5.10	105.67	41.56	0.00
SummitPoint	10658.06	500vr	706.00	005.00	1000.00	000 #4	1000.20	0.000000	5.43 6.00	142.04	41.30	0.00
Summe one	10030.00	500yi	700.00	333.30	1000.20	333.41	1000.03	0.000743	0.20	142.34	134.10	0.04
SummitPoint	10856.00	10vr	254.00	007 50	1000 73		1001.07	0.011402	4 70	E2 00	24.40	0.66
SummitDaint	10050.09	50yr	204.00	997.00	1000.73		1001.07	0.011402	4.72	23.60	34.12	0.00
SummitPoint	10856.09	SUYF	432.00	997.50	1001.39		1001.85	0.012439	5.44	79.43	43.47	0.71
SummitPoint	10856.09	TODYF	513.00	997.50	1001.68		1002.15	0.011937	5.54	92.60	47.82	0.70
SummitPoint	10856.09	SUUyr	702.00	997.50	1002.26		1002.77	0.010792	5.71	122.94	56.32	0.68
0	44000.04	40	0517-	000.07	4000.5		4000.0	0.000.05	0.6-			
SummitPoint	11086.04	10yr	254.00	999.60	1002.81		1003.01	0.006424	3.57	71.12	44.84	0.50
SummitPoint	11086.04	50yr	432.00	999.60	1003.48		1003.75	0.005823	4.16	111.96	85.82	0.50
SummitPoint	11086.04	100yr	513.00	999.60	1003.71		1004.01	0.005694	4.41	133.87	104.03	0.50
SummitPoint	11086.04	500yr	702.00	999.60	1004.16		1004.51	0.005538	4.88	188.54	137.73	0.51
SummitPoint	11275.44	10yr	254.00	1000.70	1004.15	1003.43	1004.48	0.008943	4.57	55.52	30.21	0.59
SummitPoint	11275.44	50yr	432.00	1000.70	1004.88	1004.17	1005.31	0.011619	5.22	82.82	45.49	0.68
SummitPoint	11275.44	100yr	513.00	1000.70	1005.15	1004.49	1005.59	0.012569	5.36	95.73	53.74	0.71
SummitPoint	11275.44	500yr	702.00	1000.70	1005.63	1005.07	1006.11	0.013505	5.59	125.47	70.02	0.74









Errors Warni	ngs and Notes for Plan : TribP3Channel
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 11275.44 Profile: 1yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 11086.04 Profile: 1yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10856.09 Profile: 1yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10658.06 Profile: 1yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10495.32 Profile: 1yr
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10280.58 Profile: 1yr
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
	used critical depth for the water surface and continued on with the calculations.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Warning:	During the standard step iterations, when the assumed water surface was set equal to critical depth,
	the calculated water surface came back below critical depth. This indicates that there is not a valid
	subcritical answer. The program defaulted to critical depth.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 1yr Culv: Culvert #1
Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 1yr Culv: Culvert #2
Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 1yr Culv: Culvert #3
Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 9890.22 Profile: 1yr
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
vvarning:	I ne energy loss was greater than 1.0 tt (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 9759.91 Profile: 1yr
warning:	The energy equation could not be balanced within the specified number of iterations. The program
10/	used critical depth for the water surface and continued on with the calculations.
vvarning:	The velocity head has changed by more than 0.5 π (0.15 m). This may indicate the need for
14/	additional cross sections.
vvarning:	I ne conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
Maria	or greater than 1.4. This may indicate the need for additional cross sections.
vvarning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
10/	I his may indicate the need for additional cross sections.
vvarning:	During the standard step iterations, when the assumed water surface was set equal to critical depth,
	the calculated water surface came back below critical depth. This indicates that there is not a valid
1	subcritical answer. The program defaulted to critical depth.
L OCATION.	rkiver Summireointubenu Reach Summireoint RS 9566 40 Profile 1Vr

Errors Warnings and Notes for Plan : TribP3Channel (Continued)

Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7									
	or greater than 1.4. This may indicate the need for additional cross sections.									
Note:	Hydraulic jump has occurred between this cross section and the previous upstream section.									
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 9223.75 Profile: 1yr									
Note:	During the supercritical calculations a hydraulic jump occurred at the outlet of (leaving) the culvert.									
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 9223.75 Profile: 1yr Culv: Culvert #1									
Warning:	During the supercritical analysis, the program could not converge on a supercritical answer in the									
	downstream cross section. The program used the solution with the least error.									
Note:	The flow in the culvert is entirely supercritical.									
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 8962.51 Profile: 1yr									
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7									
	or greater than 1.4. This may indicate the need for additional cross sections.									
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.									
	This may indicate the need for additional cross sections.									
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 8833.36 Profile: 1yr									
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.									
	This may indicate the need for additional cross sections.									

HEC-RAS Plan: (ClogIndepende	nce River: Su	mmitPointOper	C Reach: Sui	mmitPoint							
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sa ft)	(ft)	
CummitDaint	8603.03	10	266.00	070.00	075.22	075.22	076.24	0.017016	(103)	(39 10)	44.07	0.02
Summeron	0093.92	ТОУГ	300.00	970.00	975.32	975.32	970.34	0.017210	0.49	01.24	44.97	0.02
SummitPoint	8693.92	50yr	619.00	970.00	976.10	976.33	977.45	0.019293	10.33	103.98	64.20	0.89
SummitPoint	8693.92	100yr	733.00	970.00	976.31	976.66	977.85	0.021323	11.22	117.87	68.47	0.95
SummitPoint	8693.92	500yr	1004.00	970.00	976.81	977.30	978.62	0.023397	12.64	154.71	78.68	1.01
		,										
CummitDaint	0000.00	10.0	266.00	070.75	070 10	077.07	079.02	0.010970	7.01	50.22	27.06	0.07
Summeron	0033.30	ТОУГ	306.00	972.75	976.10	911.01	970.93	0.019670	7.31	50.55	27.90	0.07
SummitPoint	8833.36	50yr	619.00	972.75	978.96	978.96	980.04	0.017349	8.50	92.73	70.92	0.86
SummitPoint	8833.36	100yr	733.00	972.75	979.34	979.34	980.39	0.015130	8.58	123.09	89.84	0.82
SummitPoint	8833 36	500vr	1004.00	972 75	980.02	980.02	981 04	0.012695	8.87	196.26	123 55	0.77
	0000.00	00031	1004.00	012.10	000.02	000.02	001.04	0.012000	0.07	100.20	120.00	0.11
SummitPoint	8962.51	10yr	366.00	974.61	980.02		980.47	0.007556	5.37	68.19	24.31	0.56
SummitPoint	8962.51	50yr	619.00	974.61	981.03		981.68	0.009276	6.44	96.04	30.56	0.64
SummitPoint	8062 51	100vr	733.00	974.61	081 27		082.05	0.010770	7.08	103 51	32.03	0.60
Ournmar office	0002.01	500,0	100.00	074.01	001.27		002.00	0.014505	0.50	100.01	04.00	0.00
SummitPoint	8962.51	SUUyr	1004.00	974.61	981.70		982.83	0.014585	8.53	117.70	34.64	0.82
SummitPoint	9223.75		Culvert									
Ourse in the limit	0000 50	40	200.00	070.00	000.00	000.00	000.04	0.000440	4.45	700.40	000.40	0.00
SummitPoint	9303.52	TUyr	320.00	978.20	986.83	982.60	986.84	0.000110	1.15	786.42	292.19	0.08
SummitPoint	9303.52	50yr	543.00	978.20	987.25	983.57	987.26	0.000217	1.68	910.75	302.97	0.11
SummitPoint	9303.52	100yr	643.00	978.20	987.39	983.83	987.42	0.000269	1.90	955.87	306.79	0.12
SummitPoint	9303.52	500vr	881.00	978 20	987 72	984 35	987 75	0.000388	2 35	1056 58	315 14	0.15
- anna onn	2.500.52	2009.	001.00	57 0.20	301.72	004.00	307.73	0.000000	2.00	.000.00	010.14	0.15
-												
SummitPoint	9566.40	10yr	320.00	979.30	986.85	983.01	986.90	0.000414	1.97	330.55	165.39	0.15
SummitPoint	9566.40	50yr	543.00	979.30	987.29	983.97	987.39	0.000802	2.89	407.84	185.75	0.21
SummitPoint	9566.40	100vr	643.00	070 20	087 /6	084 45	097 67	0.00021	3.75	137 02	103.00	0.00
	0500.40	100yi	043.00	919.30	907.43	904.40	901.37	0.000901	3.25	431.82	193.09	0.23
SummitPoint	9566.40	SUUyr	881.00	979.30	987.80	985.38	987.96	0.001373	3.99	508.10	209.23	0.28
SummitPoint	9759.91	10yr	299.00	984.00	987.92	987.92	988.92	0.027402	8.01	37.31	19.48	1.02
SummitPoint	9759 91	50vr	508.00	984.00	989.02	989 02	080 03	0.014933	7 94	91 15	82.09	0.81
Ournmar office	0750.04	400	000.00	004.00	000.02	000.02	000.00	0.014000	1.04	400.70	404.07	0.01
SummitPoint	9759.91	100yr	602.00	984.00	989.35	989.35	990.24	0.013388	8.06	120.78	101.67	0.78
SummitPoint	9759.91	500yr	825.00	984.00	989.93	989.93	990.79	0.011612	8.37	190.87	137.28	0.75
SummitPoint	0800.22	10vr	200.00	00 380	080 70		98 980	0.002652	2.22	134.65	80.84	0.32
Summeron	9090.22	1091	299.00	900.00	909.79		909.00	0.002032	2.22	134.03	09.04	0.32
SummitPoint	9890.22	50yr	508.00	986.00	990.56		990.65	0.002160	2.40	211.87	108.01	0.30
SummitPoint	9890.22	100yr	602.00	986.00	990.82		990.92	0.002111	2.50	240.59	113.07	0.30
SummitPoint	9890.22	500yr	825.00	986.00	991.33		991.44	0.002112	2.74	300.58	122.99	0.31
Our service in the service of the se	40000.00		Outwart									
SummitPoint	10032.93		Cuiven									
SummitPoint	10097.67	10yr	299.00	987.00	992.36	989.34	992.37	0.000224	0.89	337.48	139.75	0.10
SummitPoint	10097 67	50vr	508.00	987.00	994 64	989.93	994 65	0.000072	0.75	683.80	183 15	0.06
	10007.07	400	000.00	007.00	001.01	000.00	001.00	0.000072	0.10	700.00	100.10	0.00
Summeron	10097.07	TUUyr	602.00	967.00	994.92	990.12	994.93	0.000063	0.04	/ 30.09	190.00	0.07
SummitPoint	10097.67	500yr	825.00	987.00	995.30	990.49	995.32	0.000119	1.06	815.78	215.22	0.08
SummitPoint	10280 58	10vr	200.00	080 00	992.82	992.82	993.60	0.027684	7 09	42 15	27 79	1.02
CummitDaint	10200.00	50.0	508.00	000.00	004.40	002.02	004.83	0.000303	5.00	07.69	45.00	0.62
Summeroint	10200.58	JUyi	508.00	999.90	994.40		994.82	0.009393	5.20	80.16	45.82	0.63
SummitPoint	10280.58	100yr	602.00	989.90	994.65		995.12	0.010111	5.47	110.01	50.60	0.65
SummitPoint	10280.58	500yr	825.00	989.90	994.90	994.43	995.60	0.014570	6.69	124.22	95.91	0.79
SummitDaint	10405 22	10µr	205.00	002.40	000 04	006.04	007.04	0.040050	4.00	E0.00	22.00	0.05
ounning-onit	10490.32	TOyl	205.00	993.40	990.64	990.34	997.21	0.010600	4.60	20.09	33.98	0.05
SummitPoint	10495.32	SUyr	484.00	993.40	997.05	996.97	997.89	0.022487	7.36	65.80	35.26	0.95
SummitPoint	10495.32	100yr	574.00	993.40	997.34	997.21	998.22	0.020569	7.53	76.22	36.82	0.92
SummitPoint	10495.32	500yr	786.00	993.40	998.04	997.72	998.94	0.016045	7.60	103.40	40.91	0.84
												2.01
0	40050.00	40	005.6-	005.5-	000 5-		000.07	0.01000-			0.1.0-	
SummitPoint	10658.06	TUyr	285.00	995.90	998.58		998.93	0.010325	4.78	59.61	34.37	0.64
SummitPoint	10658.06	50yr	484.00	995.90	999.53		999.93	0.007732	5.09	95.17	40.07	0.58
SummitPoint	10658.06	100yr	574.00	995.90	999.79		1000.25	0.008068	5.43	105.66	41.56	0.60
SummitPoint	10658.06	500vr	786.00	005.00	1000.26	000 / 1	1000.95	0.008750	6 20	1/2 20	15/ 00	0.64
Summeron	10056.00	500yi	780.00	995.90	1000.20	999.41	1000.65	0.000730	0.20	142.00	134.00	0.04
SummitPoint	10856.09	10yr	254.00	997.50	1000.73		1001.07	0.011404	4.72	53.85	34.12	0.66
SummitPoint	10856.09	50yr	432.00	997.50	1001.38		1001.85	0.012529	5.45	79.20	43.39	0.71
SummitPoint	10856.09	100vr	513.00	997 50	1001 68		1002 15	0 011037	5 54	02 60	47 82	0.70
SummitDaint	10956.00	500)r	702.00	007.50	1000.00		1002.10	0.010700	E 74	100.00	FC 00	0.70
SummitPoint	10856.09	SUUYr	702.00	997.50	1002.26		1002.77	0.010788	5.71	122.96	56.33	0.68
SummitPoint	11086.04	10yr	254.00	999.60	1002.81		1003.01	0.006424	3.57	71.12	44.84	0.50
SummitPoint	11086.04	50vr	432.00	09 60	1003 48		1003 75	0.005808	4 16	112 10	85.95	0.50
Cuma mait Daint	11000.04	100.00		000.00	1000.40		1004.01	0.000000		100.07	404.00	0.50
SummitPoint	11060.04	TUUY	513.00	999.60	1003.71		1004.01	0.005694	4.41	133.87	104.03	0.50
SummitPoint	11086.04	500yr	702.00	999.60	1004.16		1004.51	0.005539	4.88	188.53	137.72	0.51
SummitPoint	11275,44	10vr	254.00	1000.70	1004.15	1003.43	1004.48	0,008943	4.57	55.52	30.21	0.59
SummitPoint	11275 44	50vr	422.00	1000 70	1004.00	1004 47	1005.24	0.011600	E 22	00.02	AE 40	0.00
ounnine-onit	112/0.44	Jog	432.00	1000.70	1004.68	1004.17	1005.31	0.011629	5.22	02.78	45.46	0.08
SummitPoint	11275.44	TOUyr	513.00	1000.70	1005.15	1004.49	1005.59	0.012569	5.36	95.73	53.74	0.71
SummitPoint	11275.44	500yr	702.00	1000.70	1005.63	1005.07	1006.11	0.013505	5.59	125.47	70.02	0.74









Errors Warnir	ngs and Notes for Plan : Clogindependence
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 11275.44 Profile: 100yr
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 11086.04 Profile: 100yr
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10856.09 Profile: 100yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10658.06 Profile: 100yr
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10495.32 Profile: 100yr
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10280.58 Profile: 100yr
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #1
Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert.
Note: Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the
Note: Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet
Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used.
Note: Note: Location:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2
Note: Note: Location: Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations a hydraulic jump occurred inside of the culvert.
Note: Note: Location: Note: Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the
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Note: Note: Location: Note: Note: Location:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPoint Result flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #3
Note: Note: Location: Note: Note: Location: Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #3 During the supercritical calculations a hydraulic jump occurred inside of the culvert.
Note: Note: Location: Note: Note: Location: Note: Note:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The outlet inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #3 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the
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Note: Note: Location: Note: Location: Note: Note: Location: Warning:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #3 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 9890.22 Profile: 100yr The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for additional cross sections.
Note: Note: Location: Note: Note: Location: Note: Note: Location: Warning: Warning:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #3 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 9890.22 Profile: 100yr The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for additional cross sections. The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
Note: Note: Location: Note: Location: Note: Note: Note: Uccation: Warning:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #3 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #3 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 9890.22 Profile: 100yr The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for additional cross sections. The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.
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Note: Note: Location: Note: Note: Location: Note: Note: Uocation: Warning: Uocation: Warning: Warning: Warning:	During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #2 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 10032.93 Profile: 100yr Culv: Culvert #3 During the supercritical calculations a hydraulic jump occurred inside of the culvert. The culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet is submerged and the culvert flows full over part or all of its length. Therefore, the culvert inlet equations are not valid and the supercritical result has been discarded. The outlet answer will be used. River: SummitPointOpenC Reach: SummitPoint RS: 9890.22 Profile: 100yr The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for additional cross sections. The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections. River: SummitPointOpenC Reach: SummitPoint RS: 9759.91 Profile: 100yr The energy equation could not be balanced within the specified number of iterations. The program used critical depth for the water surface and continued on with the calculations. The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for additional cross sections.
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Errors Warnings and Notes for Plan : ClogIndependence (Continued)

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Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Warning:	During the standard step iterations, when the assumed water surface was set equal to critical depth,
	the calculated water surface came back below critical depth. This indicates that there is not a valid
	subcritical answer. The program defaulted to critical depth.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 9566.40 Profile: 100yr
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Note:	Hydraulic jump has occurred between this cross section and the previous upstream section.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 9223.75 Profile: 100yr
Warning:	During subcritical analysis, while trying to calculate culvert and weir flow, the program could not get a
	balance of energy within the specified tolerance and number of trials. The program used the solution
	with the minimum error.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 8962.51 Profile: 100yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 8833.36 Profile: 100yr
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
	used critical depth for the water surface and continued on with the calculations.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Warning:	During the standard step iterations, when the assumed water surface was set equal to critical depth,
	the calculated water surface came back below critical depth. This indicates that there is not a valid
	subcritical answer. The program defaulted to critical depth.
Location:	River: SummitPointOpenC Reach: SummitPoint RS: 8693.92 Profile: 100yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.

Small Local Flow Drainage Channel on Summit Point Phase-II Site

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
South Channel	10097.67	2yr	30.00	987.00	989.93	987.93	989.93	0.000099	0.38	79.22	63.54	0.06
South Channel	10097.67	10yr	55.00	987.00	992.36	988.18	992.36	0.00008	0.16	337.19	139.74	0.02
South Channel	10097.67	100yr	97.00	987.00	994.92	988.48	994.92	0.000002	0.13	737.08	196.73	0.01
South Channel	10280.58	2yr	30.00	990.50	992.21	992.21	992.73	0.041033	5.81	5.16	5.03	1.01
South Channel	10280.58	10yr	55.00	990.50	992.76	992.76	993.45	0.039018	6.64	8.29	6.20	1.01
South Channel	10280.58	100yr	97.00	990.50	994.87		994.95	0.003608	2.30	45.18	39.69	0.35
South Channel	10495.32	2yr	20.00	995.80	996.72		996.77	0.008413	1.76	11.39	25.71	0.47
South Channel	10495.32	10yr	37.00	995.80	996.99		997.05	0.007130	1.92	19.24	33.48	0.45
South Channel	10495.32	100yr	65.00	995.80	996.88	996.88	997.14	0.038070	4.15	15.66	30.19	1.02
South Channel	10658.06	2yr	10.00	997.30	998.21		998.24	0.010659	1.56	6.40	20.47	0.49
South Channel	10658.06	10yr	18.00	997.30	998.35		998.40	0.012010	1.80	10.00	28.35	0.53
South Channel	10658.06	100yr	32.00	997.30	998.81		998.83	0.002600	1.12	28.66	52.88	0.27

HEC-RAS Plan: OnSiteChan River: South Sub-Branch Reach: South Channel

Small Local Flow Drainage Channel on Summit Point Phase-II Site



Small Local Flow Drainage Channel on Summit Point Phase-II Site



Small Local Flow Drainage Channel on Summit Point Phase-II Site



Small Local Flow Drainage Channel on Summit Point Phase-II Site

Errors Warnings and Notes for Plan : OnSiteChan

Location:	River: South Sub-Branch Reach: South Channel RS: 10658.06 Profile: 100vr
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: South Sub-Branch Reach: South Channel RS: 10495.32 Profile: 100yr
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
	used critical depth for the water surface and continued on with the calculations.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Warning:	During the standard step iterations, when the assumed water surface was set equal to critical depth,
	the calculated water surface came back below critical depth. This indicates that there is not a valid
	subcritical answer. The program defaulted to critical depth.
Location:	River: South Sub-Branch Reach: South Channel RS: 10280.58 Profile: 100yr
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.