

# FINAL STORMWATER STUDY

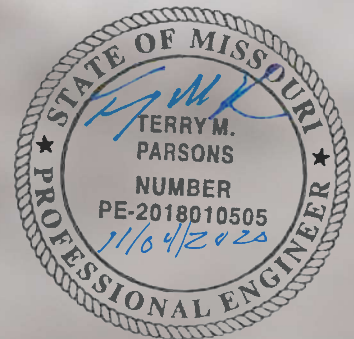
LS MIDDLE SCHOOL #4  
Lee's Summit, Jackson County, Missouri

**Prepared for:**

Lee's Summit School District  
Lee's Summit, Missouri

**Prepared By**

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Olsson Project No. 020-0103

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# **APPENDICES**

Appendix A Drainage Maps

Appendix B Accompanying Documents

Appendix C BMP Calculations and Information

## **SUMMARY**

This storm drainage study is being submitted on behalf of the Lee's Summit School District for the proposed development of the property located south of Bailey Road between Dalton Drive and Ranson Road, in Lee's Summit, Jackson County, Missouri. This property is an existing terraced row crop field with no existing buildings. This final report is being submitted to the City of Lee's Summit with the Final Development Plan for approval of this institutional development.

# 1. INTRODUCTION

This final stormwater management study is being submitted on behalf of Lee's Summit School District for a development on a 51.85 acre parcel of land generally located south of Bailey Road between Dalton Drive and Ranson Road, in Lee's Summit, Jackson County, Missouri. .

## 1.1. Project Location and Description

The proposed site is located in the northeast quarter of Section 16, Township 47 North, Range 31 West. The existing site is currently undeveloped consisting of terraced row crop. The site drains to the southeast into an unnamed tributary that runs along the entirety of the east side of the property. The tributary flows south to Big Creek. The site is not located within the flood plain.

## 1.2. Study Purpose

The purpose of this report is to verify this development's conformance with the City of Lee's Summit Design Criteria and Plan Requirements for Public Improvement Plans 2019 edition. Storm water drainage facilities are designed according to section F of the aforementioned criteria. This study will outline methods to mitigate impacts on storm water runoff resulting from the development for the 2, 10 and 100-year rainfall events and for treatment of stormwater runoff with the use of permanent stormwater treatment facilities.

# 2. METHODOLOGY

## 2.1. General Criteria and References

Analytical and design criteria conform to those of Division V - Section 5600 – "Storm Drainage Systems and Facilities" of the Kansas City Metropolitan Chapter of the American Public Works Association's "Standard Specifications and Design Criteria". Based on these criteria's, Post-development discharge rates for the 2, 10, and 100-year storm events will be limited to provisions in section 5608.4-C1 Performance Criteria – "Comprehensive Control". Post-development discharge rates are limited to 0.5 cfs per acre for 2-Year, 2.0 cfs per acre for 10-year, and 3.0 cfs per acre for 100-year storm events.

Post development flows from the site are shown below and were calculated using HEC-HMS for the 2, 10 and 100-year storm events. Existing and proposed hydrographs were calculated using the 24-hour SCS Type II rainfall distribution. Existing times of concentration were determined using Inlet Time and Travel Time equations found in Section 5602.7 of APWA Section 5600.

## 2.2. Soils Description

Soil classifications by the United States Department of Agriculture (USDA) on the Natural Resources Conservation Service (NRCS) Soils website for Johnson County, Kansas show the existing site consisting of the following soil types:

10117 – Sampsel Silty Clay Loams, 5 to 9 percent slopes – HSG Type C/D

10082 – Arisburg-Urban Land Complex, 1 to 5 percent slopes – HSG Type C

*\*HSG – Hydrologic Soils Group (The NRCS information is included in the appendix).*

## 3. HYDROLOGIC/HYDRAULIC ANALYSES

### 3.1. Existing Conditions

The property is bounded by residential lots to the west and north. Bailey Road is also located on the north side of the property. To the south and east the adjacent properties are currently undeveloped. A tributary to Big Creek, that flows to the south, lies along the entire east property line of the site. The storm drainage from the residential lots to the north comes under Bailey Road and empties into the north end of the tributary. The property currently contains no impervious area and runoff flows southeasterly to the tributary. An Exhibit of the Existing Conditions is included in Appendix A.

With the comprehensive control method is being used for drainage design, an existing curve number analysis is not required for the site.

There are existing developments to the west and north of the property. Runoff from the development to the north is collected by an enclosed drainage system that empties into the stream on the east side of the site. Runoff from a portion of the backyards of the residences to the west do drain on to the site (approximately 1.2 acres).

### 3.2. Stream Protection and Buffer Zones

A portion of the property will be defined as a Stream Protection Buffer Zone. The proposed buffer zone will meet the requirements established Section 5603.5 of the APWA Section 5600 Design Criteria.

The width of the setback is determined by the drainage area to the stream. Using USGS contours the drainage area for the stream was determined at different points. A minor tributary from the east is the location for Point 1. The drainage area to Point 1 is 138 acres. Therefore a 60' offset from the ordinary high-water mark (OHM), ie surveyed top of bank, is used to determine the buffer extent to that point. Point 2 is located where the stream exits at the

southern edge of the property. The total drainage to this point is 210 acres. The setback from Point 1 to Point 2 is 100'. An exhibit of the stream setback drainage areas is included in Appendix A.

The final development plan will require a waiver/modification to the Stream Setback Buffer in order to complete the construction of the improvements shown. The encroachment is needed due to the nature of the site being narrow in the east/west direction and meeting the program requirements for all of the components needed on site by the school district. The grade of the site falls from west to east towards the existing streamway. Again, with the narrow site it is difficult grade out the site for buildings, fields, and parking and not have the slope embankment encroach into the buffer.

According to the requirements of Section 5605.3, a total of 2.08 acres should be dedicated to the stream setback. To accommodate the improvements noted above a total of 0.74 acres of stream setback will be encroached upon. . To offset this, an additional 0.76 acres adjacent to the stream setback will be added back into the setback. This results in 2.10 acres of permanent stream setback dedication.

### **3.3. Proposed Conditions Analysis**

Post development, the entirety of the of the property will continue to flow to the tributary to the east. The proposed site will include the middle school, a softball/baseball complex (with 4 fields), a track, practice fields, outdoor classrooms, parking, three extended dry detention basins, private storm, and associated utilities. Roof drains, private storm sewer pipe and inlets will allow adequate drainage of the proposed school, athletic facilities and parking areas. The private storm will drain into the detention basins and then be routed to the tributary. A private road will be constructed for access to the bus turnaround and parking areas. The road will connect to the existing Bailey Road on the north to proposed Cape Road on the south.

The proposed Cape Road will be located on the southern portion of the property. The right of way will be dedicated to the city with a portion the road being constructed to connect to existing Cape Road to the west. When the road is completely constructed a public storm system will drain the road to the tributary. Temporary provisions will be implemented to drain the constructed road to the stream.

The backyards of the residential subdivisions to the west also drain onto the existing sight. This 2.5 acres will drain to the proposed north-south private road and will enter into the proposed storm system for the road.

The site will be divided into three main drainage areas. An exhibit of the Stormwater Management Plan drainage areas is included in Appendix A. Each drainage area will have

collection system and an extended dry detention basin with control structure. A general description of what is draining to each area is as follows:

Drainage Area 1 (3.5 acres) - northwest ball field ball field and a portion of the common area in the ball fields

Drainage Area 2 (8.2 acres) - northeast ball field, a portion of the common areas in the ball fields and the northeast quarter of the roof for the middle school

Drainage Area 3 (22.6 acres) – 2 southern ball fields, the remainder of middle school building roof, north-south drive, main parking area, bus turnaround area, the practice fields and the track.

The ball fields and the track will have synthetic surfacing with underdrain systems. The current plan is for the two north fields to have synthetic turf on the infields and outfields. The two south fields will have synthetic turf on the infields only. If the budget allows, there is a possibility that the two south fields will have also have synthetic turf in the outfields. For the purposes of this report, the ballfields will be treated as if they have a complete synthetic surface.

The synthetic surface will act as impervious surface. However, the underdrain system, with it's aggregate subsurface rock layer and underdrain piping, will increase the time of concentration (Tc) for the runoff that is guided through the underdrain system. The increase in Tc will be 32 minutes based on the final underdrain design. The HEC-HMS model based its Tc's on a time to inlet of 5 minutes and then an estimation of pipe travel time. This was used as the Tc for the impervious and pervious areas in the drainage area. The synthetic fields had an additional time of 32 minutes added to their Tc's.



Based on the completed models the peak flows for each area are included in the table below:

**Table 1. Post-Development Peak Flows**

Site Description	Total Area (ac)	CN	Storm Event	Runoff Q (cfs)
Area 1	3.5	79.4	2-YR	<b>5.7</b>
			10-YR	<b>8.6</b>
			100-YR	<b>12.5</b>
Area 2	8.2	89.6	2-YR	<b>10.9</b>
			10-YR	<b>25.5</b>
			100-YR	<b>37.5</b>
Area 3	22.6	87.5	2-YR	<b>38.4</b>
			10-YR	<b>90.7</b>
			100-YR	<b>133.8</b>

### 3.4. Stormwater Detention

As stated previously, a new detention basin will be constructed for each area to mitigate the increase in flow due to the increase in impervious area. A control structure will be located at the outlet of the basin. An orifice/weir plate in the control structure will limit outflow in the 2, 10, and 100-year storms.

The control structure for each basin is designed to limit the outlet flow to the allowable release rate for each storm based on the Comprehensive Control Strategy. Hydrographs for the combined flows of the detained and undetained areas are shown in Appendix D.

To meet water treatment requirements, the basin will act as extended dry detention. The water quality volume (WQv) will be controlled by a series of 1" orifices at the bottom of the orifice plate. The conduit will release the water quality volume over a 40-hour period to allow pollutants to settle out of this precipitation event.

Per Section 5600 of the Design and Construction Manual an emergency spillway has been designed for each basin. The emergency spillway for each basin is located the top of the dam of each structure. The emergency spillway has been sized to accommodate the 100-year event assuming that 100% of the primary outlet structure is clogged and there is zero available

storage in the basin. Each spillway is a minimum of 0.5 feet above the nominal 100-year water surface elevation. There is also least 1 foot of freeboard between the clogged condition with and the water surface elevation of the spillway handling the 100-year overflow and top of the dam. The details for the emergency spillway have been included in Appendix D of this report and in the construction documents.

**Table 2. EDD-1 WSE's and Peak Flows**

<b>Description</b>	<b>Detention Basin</b>
Bottom of Basin	1007.16
Total Storage Volume	0.85 ac-ft
Top of Dam Elevation	1012.60
WQv Orifice (IE Elevation, Pipe Size)	1007.16, 6 – 1” (ft, # hole - diam)
Water Quality Volume WSE, Storage	1008.41, 0.18 (ft, ac-ft)
2-year & 10-Year Orifice (IE Elevation, Pipe Size)	1008.43, 1-6” (ft, orifice size)
10–Year Storm WSE, Storage, Peak Outflow	1010.3, 0.5, 4.2 (ft, ac-ft, cfs)
100–Year Storm Weir (Elevation, Length)	1009.58, 6.0 (ft, lf)
100–Year Storm WSE, Storage, Peak Outflow	1010.8, 0.6, 9.9 (ft, ac-ft, cfs)

**Table 3. EDD-2 WSE's and Peak Flows**

<b>Description</b>	<b>Detention Basin</b>
Bottom of Basin	995.13
Total Storage Volume	1.34 ac-ft
Top of Dam Elevation	1002.99
WQv Orifice (IE Elevation, Pipe Size)	995.13, 10 – 1” (ft, # hole - diam)
Water Quality Volume WSE, Storage	997.40, 0.42 (ft, ac-ft)
2-year & 10-Year Orifice (IE Elevation, Pipe Size)	997.50, 1-9” (ft, orifice size)
10–Year Storm WSE, Storage, Peak Outflow	1000.3, 0.8, 14.0 (ft, ac-ft, cfs)
100–Year Storm Weir (Elevation, Length)	999.03, 8.0 (ft, lf)
100–Year Storm WSE, Storage, Peak Outflow	1001.10, 1.2, 24.1 (ft, ac-ft, cfs)

**Table 4. EDD-3 WSE's and Peak Flows**

<b>Description</b>	<b>Detention Basin</b>
Bottom of Basin	989.29
Total Storage Volume	5.3 ac-ft
Top of Dam Elevation	1001.07
WQv Orifice (IE Elevation, Pipe Size)	989.22, 15 – 2” (ft, # hole - diam)
Water Quality Volume WSE, Storage	994.40, 1.95 (ft, ac-ft)
2-year & 10-Year Orifice (IE Elevation, Pipe Size)	994.50, 1-15” (ft, orifice size)
10–Year Storm WSE, Storage, Peak Outflow	997.40, 3.7, 35.9 (ft, ac-ft, cfs)
100–Year Storm Weir (Elevation, Length)	996.10, 16 (ft, lf)
100–Year Storm WSE, Storage, Peak Outflow	998.40, 4.7, 63.0 (ft, ac-ft, cfs)

Table 5. Detention Basin Information

Area	Drainage Area (acres)	Storm Event	Allowable Release Rate (cfs)	Design Release Rate (cfs)	Storage Required (ac-ft)	WSE (ft elev)
1	3.5	2-YR	1.8	1.6	0.3	1009.7
		10-YR	7.0	4.2	0.5	1010.3
		100-YR	10.4	9.9	0.6	1010.8
2	8.2	2-YR	4.1	3.7	0.5	998.9
		10-YR	16.4	14.0	0.9	1000.3
		100-YR	24.5	24.1	1.3	1001.1
3	22.6	2-YR	11.3	11.1	2.9	995.7
		10-YR	45.3	35.9	3.7	997.4
		100-YR	67.9	63.0	4.7	998.4

## 4. STORM WATER TREATMENT REQUIREMENTS

As stated previously, the three detention basins will be designed to act as extended dry bottom detention facilities to treat stormwater per MARC water quality standards. The orifice plate for the basin are sized to release the water quality volume (1.37") over a 40-hour period to allow pollutants to settle from runoff before entering the public stormwater system. The size and quantities of the orifice holes are included in Tables 3-5.

## 5. CLEANWATER ACT SECTION 404 PERMITTING REQUIREMENTS

Construction will not be occurring within jurisdictional Waters of the United States. Therefore, a Section 404 permit is not required.

## 6. FEMA/DWR PERMIT REQUIREMENTS

No FEMA permitting or submittals will be required on this site because there are no FEMA delineated floodplains on the site. A copy of the FIRM map for this area has been included in Appendix B.

## **7. CONCLUSIONS AND RECOMMENDATIONS**

As outlined in the preceding report, the stormwater management plan for this development meets the requirements set forth in Section 5600 of the Lee's Summit Design and Construction Manual.

The development will increase impervious areas on the site and drainage patterns on the site remain will relatively unchanged. Increased runoff rates in the post-development condition will be controlled by three detention structures. The control structures have been designed to limit the peak flows below the allowable release rates defined in the Comprehensive Control Strategy of Section 5600. All three basins will be constructed as extended dry detention basins to satisfy the Section 5600 storm water quality requirements.

The eastern portion of the property will be dedicated in a stream buffer easement per Section 5605.3. To totally satisfy the stream setback requirements, a waiver will be required for areas where site improvements impede in the setback areas.

The existing property drains in a southeasterly direction limited on the east by the stream. There are no existing developments in that direction. Therefore no waivers are required for the existing development adjacent to the site under Section 5601.6B.

No 404 or FEMA will be required. Based on these facts and other information provided herein, we request approval of this stormwater study.

## **APPENDIX A**

### **Report Exhibits**

Existing Conditions

Stream Setback

Site Plan

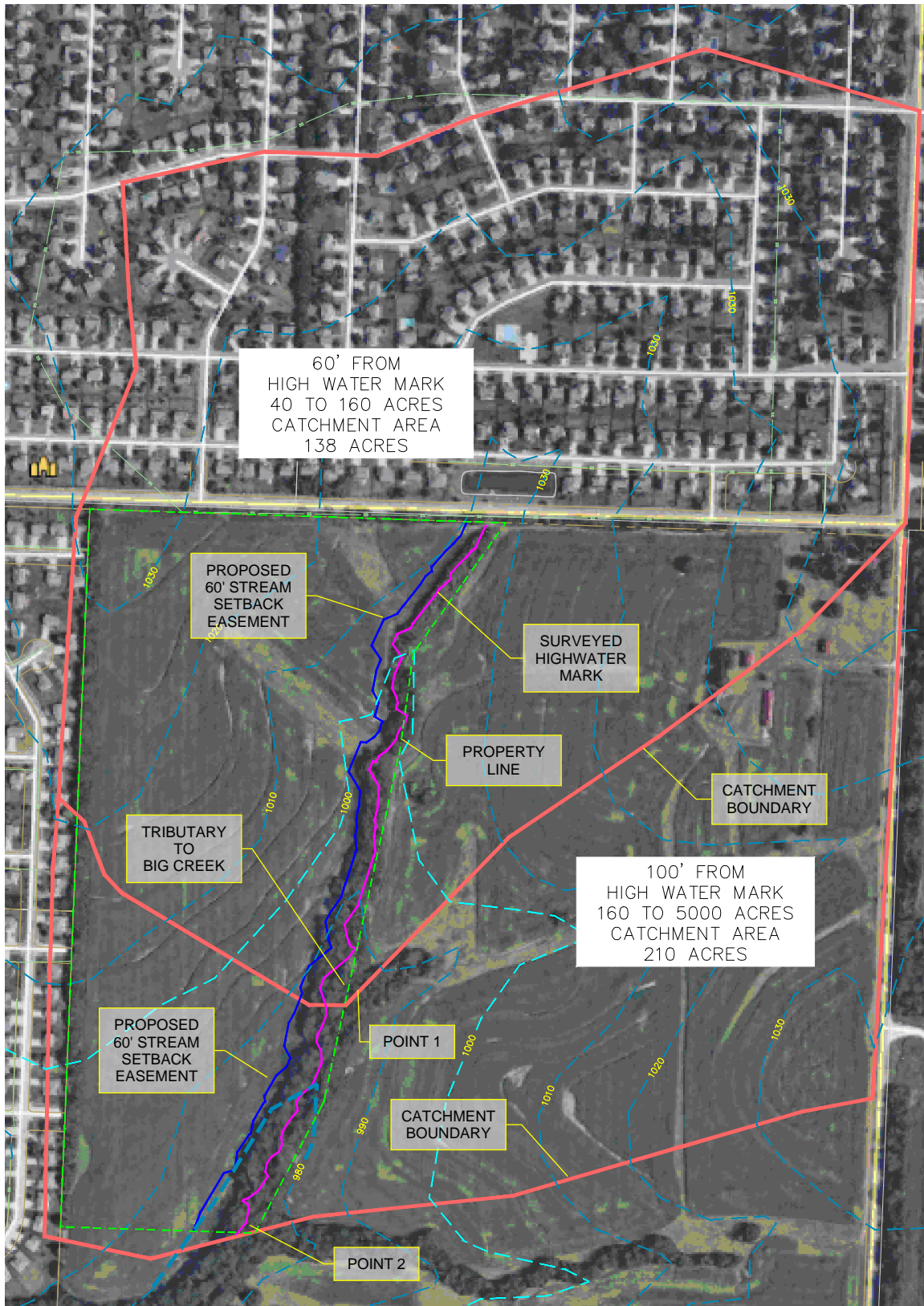
Stormwater Management Plan

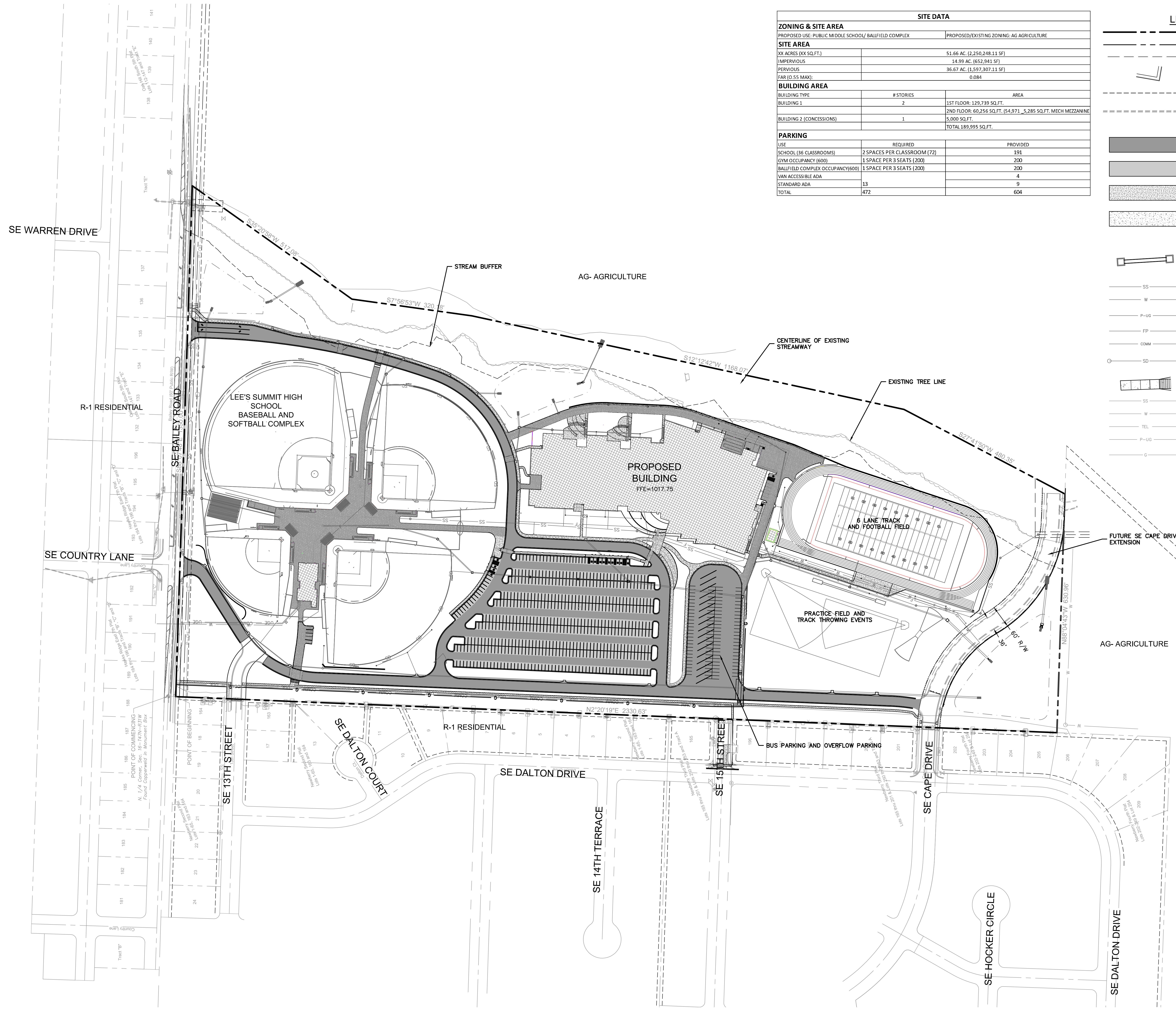
# LS MIDDLE SCHOOL #4 EXISTING CONDITIONS





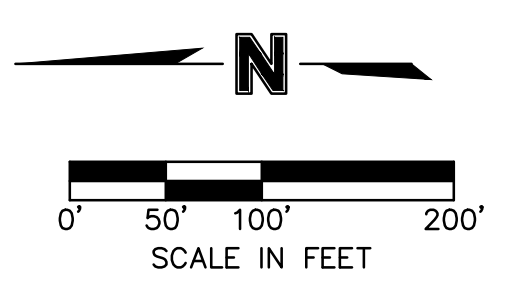
# LS MIDDLE SCHOOL #4 STREAM SETBACK EXHIBIT



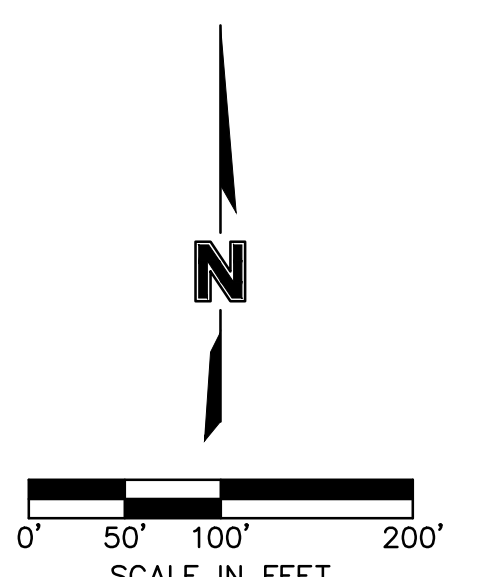
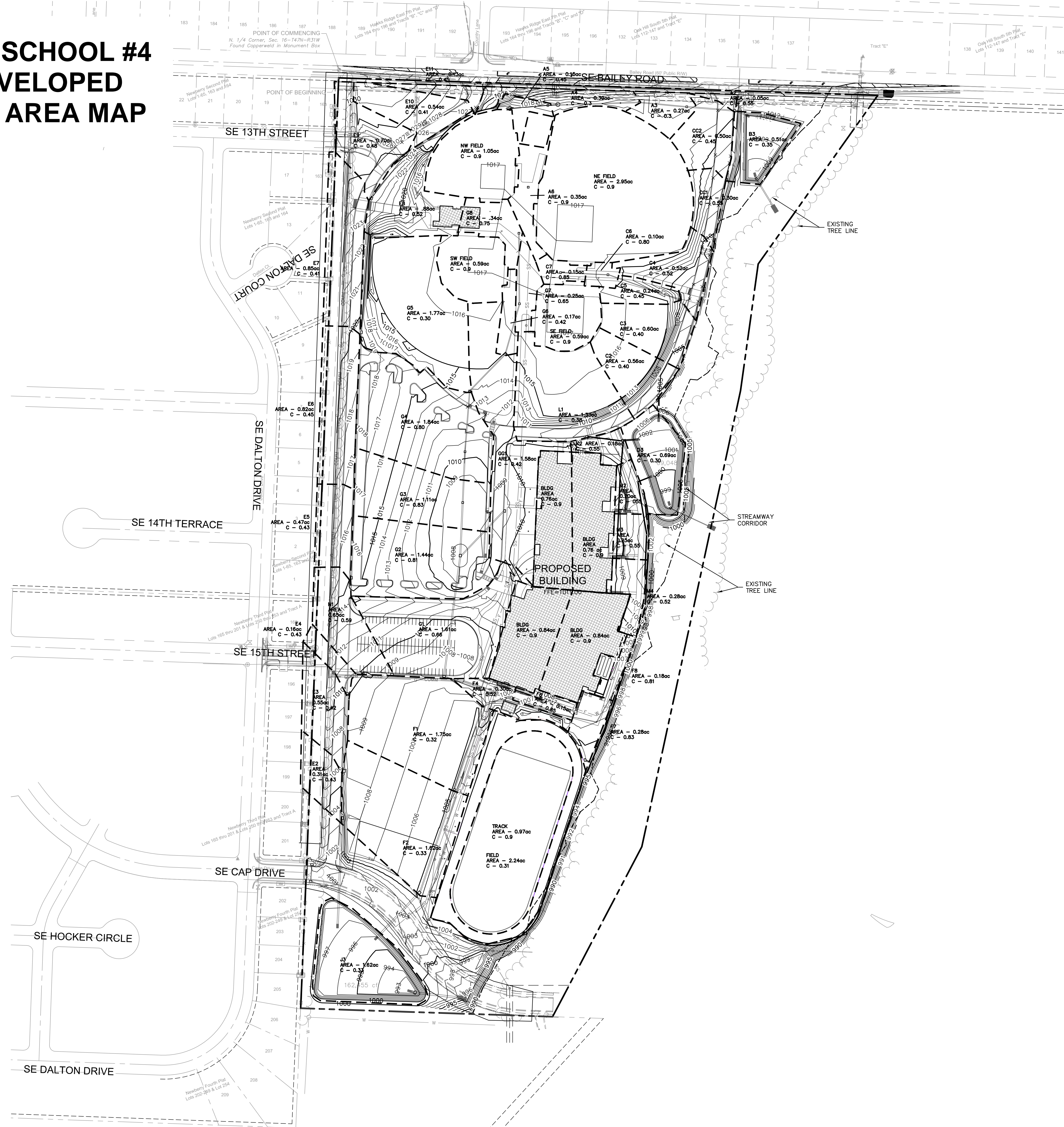


SITE DATA		
<b>ZONING &amp; SITE AREA</b>		PROPOSED/EXISTING ZONING: AG AGRICULTURE
<b>SITE AREA</b>		
XX ACRES (XX SQ.FT.)		51.66 AC. (2,250,248.11 SF)
IMPERVIOUS		14.99 AC. (652,941 SF)
PERVIOUS		36.67 AC. (1,597,307.11 SF)
FAR (0.55 MAX.)		0.084
<b>BUILDING AREA</b>		
BUILDING TYPE	# STORIES	AREA
BUILDING 1	2	1ST FLOOR: 129,739 SQ.FT.
		2ND FLOOR: 60,256 SQ.FT. (54,971_5,285 SQ.FT. MECH MEZZANINE)
BUILDING 2 (CONCESSIONS)	1	5,000 SQ.FT.
		TOTAL 189,995 SQ.FT.
<b>PARKING</b>		
USE	REQUIRED	PROVIDED
SCHOOL (36 CLASSROOMS)	2 SPACES PER CLASSROOM (72)	191
GYM OCCUPANCY (600)	1 SPACE PER 3 SEATS (200)	200
BALLFIELD COMPLEX OCCUPANCY(600)	1 SPACE PER 3 SEATS (200)	200
VAN ACCESSIBLE ADA		4
STANDARD ADA	13	9
TOTAL	472	604


LEGEND	
	PROPERTY LINE
	LOT LINE
	UTILITY EASEMENT
	CONSTRUCT CONCRETE CURB & GUTTER
	SAWCUT PAVEMENT FULL DEPTH
	ADA PATH
	HEAVY DUTY ASPHALT PAVEMENT
	LIGHT DUTY ASPHALT PAVEMENT
	HEAVY DUTY CONCRETE PAVEMENT
	SIDEWALK AND LANDSCAPE CONCRETE PAVEMENT
	STORM SEWER
	SANITARY SERVICE LINE
	WATER SERVICE LINE
	UNDERGROUND POWER SERVICE LINE
	FIRE PROTECTION LINE
	COMMUNICATIONS SERVICE LINE
	LANDSCAPE/ROOF DRAIN
	ADA CONCRETE SIDEWALK AND RAMP
	EXISTING SANITARY SEWER MAIN
	EXISTING WATER MAIN
	EXISTING COMMUNICATIONS LINE
	EXISTING ELECTRIC LINE
	EXISTING GAS MAIN

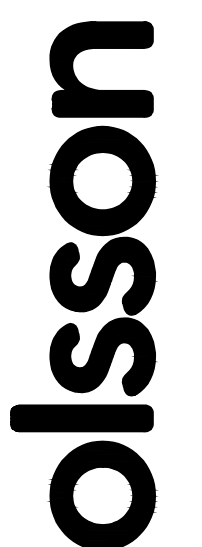


# LS MIDDLE SCHOOL #4 POST DEVELOPED DRAINAGE AREA MAP



DWG: F:\2020\0001-0500\020-0103\4D-Design\AutoCAD\Final Plans\Sheets\GNV\CONSTRUCTION DOCUMENTS\C\_DRN01\_0200103.dwg  
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 USER: tpersons


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**NOT FOR CONSTRUCTION**

**LEE'S SUMMIT MIDDLE SCHOOL #4**  
 LEE'S SUMMIT R-7 SCHOOL DISTRICT  
 1001 SE BAILEY ROAD  
 LEE'S SUMMIT, MO 64881

13-20102-00  
 POST DEVELOPED  
 DRAINAGE AREA  
 MAP

**C1047**





**APPENDIX B**  
**Accompanying Documents**

Soils Map

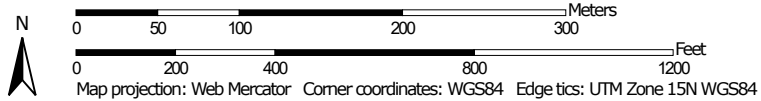
FEMA Firmette

Soil Map—Jackson County, Missouri  
(LS MIDDLE SCHOOL #4)



Soil Map may not be valid at this scale.

Map Scale: 1:4,630 if printed on A portrait (8.5" x 11") sheet.



## MAP LEGEND

### Area of Interest (AOI)

 Area of Interest (AOI)

### Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

### Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

### Water Features



Streams and Canals

### Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

### Background



Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Jackson County, Missouri

Survey Area Data: Version 20, Sep 16, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 6, 2019—Nov 16, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
10082	Arisburg-Urban land complex, 1 to 5 percent slopes	28.5	61.1%
10117	Sampsel silty clay loam, 5 to 9 percent slopes	18.1	38.9%
<b>Totals for Area of Interest</b>		<b>46.6</b>	<b>100.0%</b>

## Jackson County, Missouri

### 10117—Sampsel silty clay loam, 5 to 9 percent slopes

#### Map Unit Setting

*National map unit symbol:* 2qkzz

*Elevation:* 600 to 900 feet

*Mean annual precipitation:* 33 to 41 inches

*Mean annual air temperature:* 50 to 55 degrees F

*Frost-free period:* 177 to 220 days

*Farmland classification:* Prime farmland if drained

#### Map Unit Composition

*Sampsel and similar soils:* 85 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Sampsel

##### Setting

*Landform:* Hillslopes

*Landform position (two-dimensional):* Shoulder

*Landform position (three-dimensional):* Side slope

*Down-slope shape:* Concave

*Across-slope shape:* Convex, concave

*Parent material:* Residuum weathered from shale

##### Typical profile

*Ap - 0 to 13 inches:* silty clay loam

*Bt - 13 to 80 inches:* silty clay

##### Properties and qualities

*Slope:* 5 to 9 percent

*Depth to restrictive feature:* More than 80 inches

*Natural drainage class:* Somewhat poorly drained

*Runoff class:* Very high

*Capacity of the most limiting layer to transmit water (Ksat):*

Moderately low to moderately high (0.06 to 0.20 in/hr)

*Depth to water table:* About 0 to 18 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Salinity, maximum in profile:* Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

*Available water storage in profile:* Moderate (about 8.6 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 3e

*Hydrologic Soil Group:* C/D

*Ecological site:* Interbedded Sedimentary Upland Savanna (R109XY010MO)

*Other vegetative classification:* Grass/Prairie (Herbaceous  
Vegetation)

*Hydric soil rating:* No

## Data Source Information

Soil Survey Area: Jackson County, Missouri

Survey Area Data: Version 20, Sep 16, 2019

## Jackson County, Missouri

### 10082—Arisburg-Urban land complex, 1 to 5 percent slopes

#### Map Unit Setting

*National map unit symbol:* 2w7ld

*Elevation:* 750 to 1,130 feet

*Mean annual precipitation:* 39 to 45 inches

*Mean annual air temperature:* 50 to 55 degrees F

*Frost-free period:* 177 to 220 days

*Farmland classification:* All areas are prime farmland

#### Map Unit Composition

*Arisburg and similar soils:* 61 percent

*Urban land:* 30 percent

*Minor components:* 9 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Arisburg

##### Setting

*Landform:* Interfluves

*Landform position (two-dimensional):* Summit

*Landform position (three-dimensional):* Interfluve

*Down-slope shape:* Convex

*Across-slope shape:* Convex

*Parent material:* Loess

##### Typical profile

*Ap - 0 to 6 inches:* silt loam

*A - 6 to 13 inches:* silt loam

*Bt - 13 to 19 inches:* silty clay loam

*Btg - 19 to 56 inches:* silty clay loam

*BCg - 56 to 79 inches:* silty clay loam

##### Properties and qualities

*Slope:* 1 to 5 percent

*Depth to restrictive feature:* More than 80 inches

*Natural drainage class:* Somewhat poorly drained

*Capacity of the most limiting layer to transmit water (Ksat):*

Moderately high (0.20 to 0.60 in/hr)

*Depth to water table:* About 18 to 30 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Salinity, maximum in profile:* Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

*Available water storage in profile:* High (about 11.5 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 2e

*Hydrologic Soil Group:* C  
*Ecological site:* Loess Upland Prairie (R107BY007MO)  
*Hydric soil rating:* No

### **Description of Urban Land**

#### **Interpretive groups**

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 8  
*Hydric soil rating:* No

### **Minor Components**

#### **Sampsel**

*Percent of map unit:* 3 percent  
*Landform:* Hills  
*Landform position (two-dimensional):* Backslope  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Convex  
*Across-slope shape:* Concave  
*Ecological site:* Interbedded Sedimentary Upland Savanna  
(R109XY010MO)  
*Hydric soil rating:* Yes

#### **Greenton**

*Percent of map unit:* 3 percent  
*Landform:* Hillslopes  
*Landform position (two-dimensional):* Shoulder  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Convex  
*Across-slope shape:* Convex  
*Ecological site:* Loess Upland Prairie (R109XY002MO)  
*Hydric soil rating:* No

#### **Sharpsburg**

*Percent of map unit:* 3 percent  
*Landform:* Ridges  
*Landform position (two-dimensional):* Summit  
*Landform position (three-dimensional):* Interfluve  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Ecological site:* Loess Upland Prairie (R109XY002MO)  
*Hydric soil rating:* No

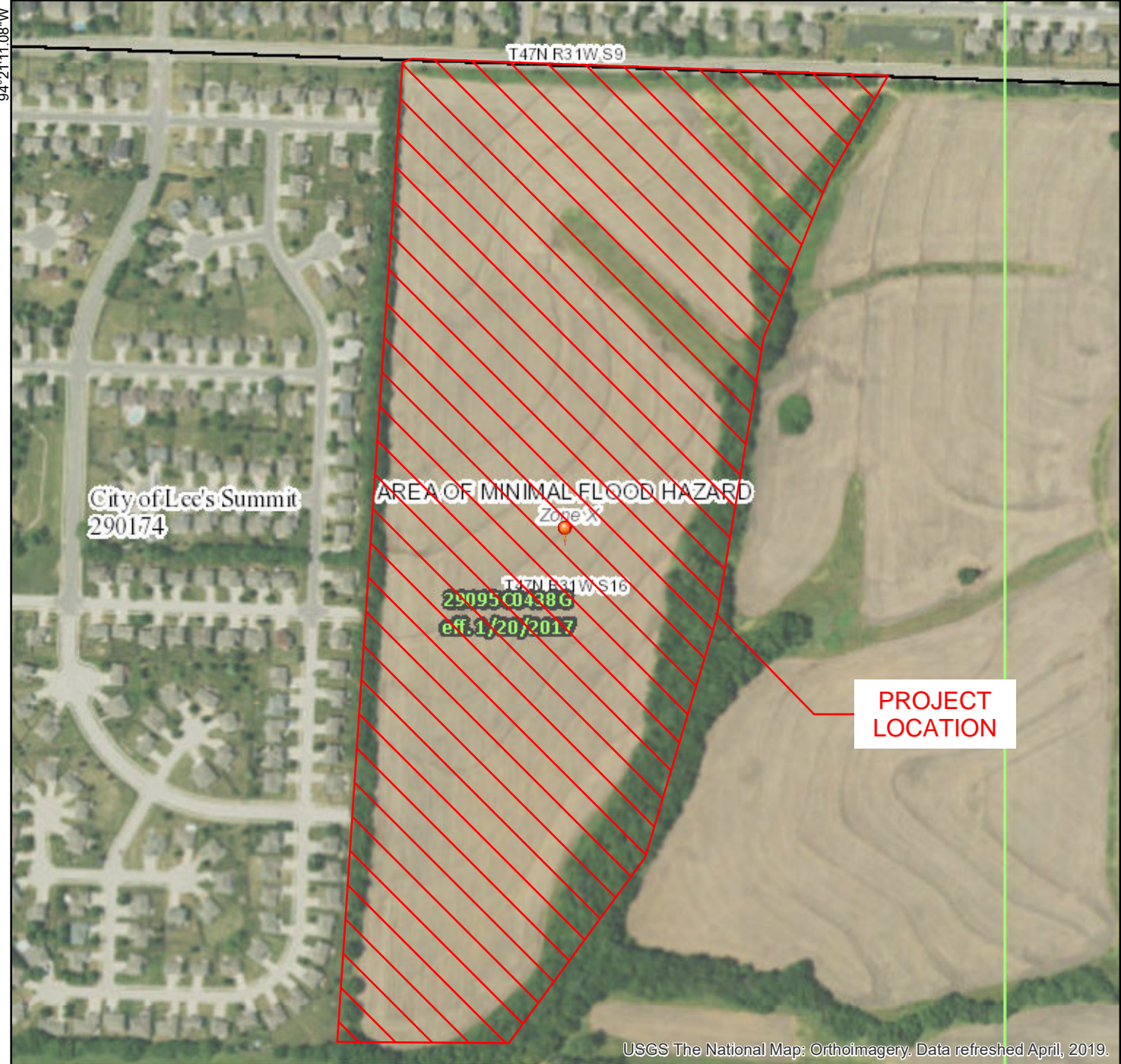
## **Data Source Information**

Soil Survey Area: Jackson County, Missouri  
Survey Area Data: Version 20, Sep 16, 2019

# National Flood Hazard Layer FIRMette



38°53'44.76"N



## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS	Without Base Flood Elevation (BFE) Zone A, V, A99	With BFE or Depth Zone AE, AO, AH, VE, AR
	Regulatory Floodway	

OTHER AREAS OF FLOOD HAZARD	0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
	Future Conditions 1% Annual Chance Flood Hazard Zone X
	Area with Reduced Flood Risk due to Levee. See Notes. Zone X
	Area with Flood Risk due to Levee Zone D

OTHER AREAS	Area of Minimal Flood Hazard Zone X
	NO SCREEN
	Effective LOMRs
	Area of Undetermined Flood Hazard Zone D

GENERAL STRUCTURES	Channel, Culvert, or Storm Sewer
	Levee, Dike, or Floodwall

OTHER FEATURES	Cross Sections with 1% Annual Chance Water Surface Elevation
	20.2
	17.5
	Coastal Transect
	Base Flood Elevation Line (BFE)
	Limit of Study
	Jurisdiction Boundary
	Coastal Transect Baseline
	Profile Baseline
	Hydrographic Feature

MAP PANELS	Digital Data Available
	No Digital Data Available
	Unmapped

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

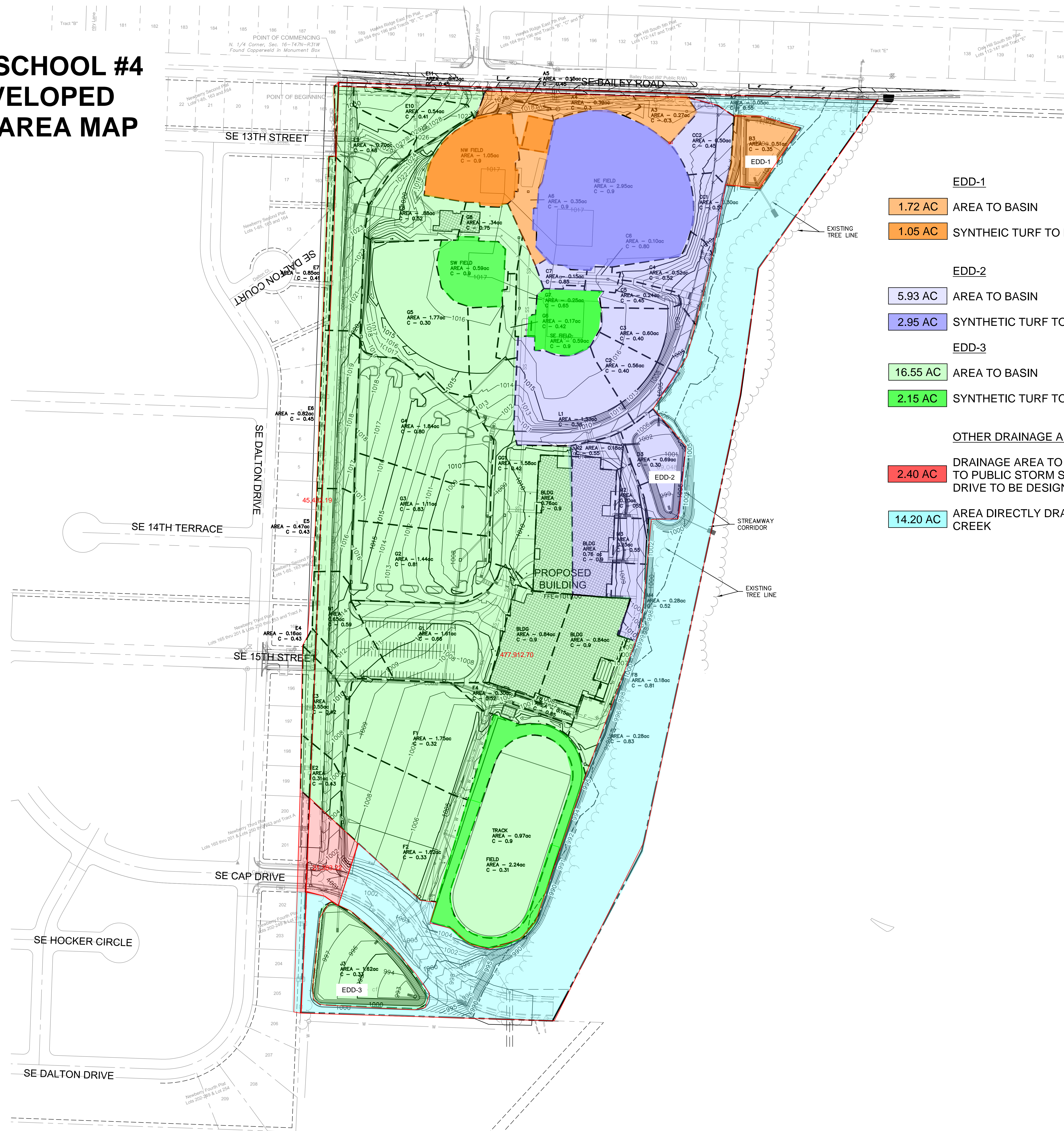
The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 5/5/2020 at 4:01:03 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

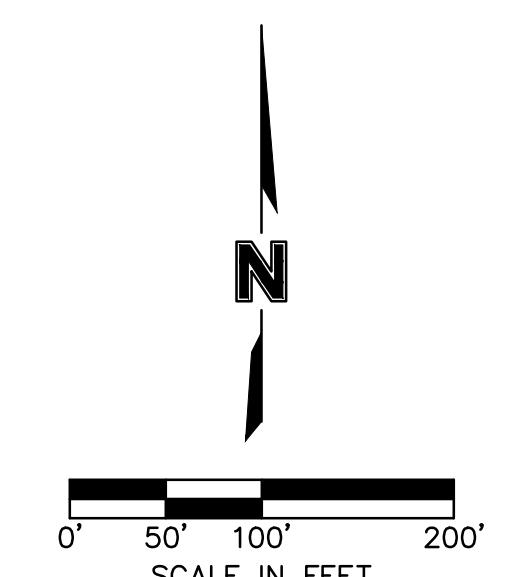
# **APPENDIX C**

## **Detention Calculations**

# LS MIDDLE SCHOOL #4 POST DEVELOPED DRAINAGE AREA MAP



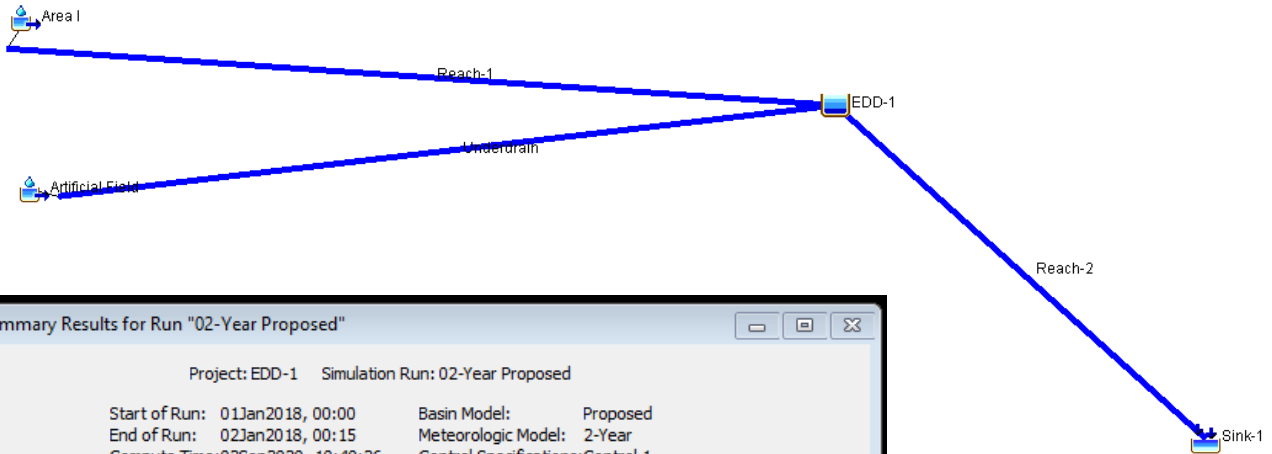
- EDD-1**
- 1.72 AC AREA TO BASIN
- 1.05 AC SYNTHETIC TURF TO BASIN
- EDD-2**
- 5.93 AC AREA TO BASIN
- 2.95 AC SYNTHETIC TURF TO BASIN
- EDD-3**
- 16.55 AC AREA TO BASIN
- 2.15 AC SYNTHETIC TURF TO BASIN
- OTHER DRAINAGE AREAS**
- 2.40 AC DRAINAGE AREA TO BE DIRECTED TO PUBLIC STORM SEWER ON CAPE DRIVE TO BE DESIGNED LATER
- 14.20 AC AREA DIRECTLY DRAINING TO CREEK



DWG: F:\2020\0001-0500\020-0103\40-Design\AutoCAD\Final Plans\Sheets\CONSTRUCTION DOCUMENTS\C\_DRN01\_0200103.dwg  
 DATE: Aug 28, 2020 8:43am XREFS: C:\XBASE\_0200103 C:\PBASE\_0200103 C:\PTBLK\_0200103 C:\PSURF\_0200103 T\_PBASE\_0200103 T\_PFSAS\_0200103 C\_PSTRM\_0200103 V\_XALT\_00103  
 USER: tpersons



# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 1 (EDD-1) HEC-HMS BASIN MODEL AND GLOBAL SUMMARIES



Global Summary Results for Run "02-Year Proposed"

Project: EDD-1 Simulation Run: 02-Year Proposed

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
 End of Run: 02Jan2018, 00:15 Meteorologic Model: 2-Year  
 Compute Time: 02Sep2020, 10:40:26 Control Specifications: Control 1

Show Elements: All Elements Volume Units:  IN  AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI <sup>2</sup> )	Peak Discharge (CFS)	Time of Peak	Volume (IN)
Artificial Field	0.0027	7.0	01Jan2018, 12:00	3.60
Underdrain	0.0027	4.9	01Jan2018, 12:30	3.58
Area I	0.0027	5.8	01Jan2018, 12:00	2.77
Reach-1	0.0027	4.0	01Jan2018, 12:00	2.77
EDD-1	0.0054	1.6	01Jan2018, 13:00	3.17
Reach-2	0.0054	1.6	01Jan2018, 13:00	3.17
Sink-1	0.0054	1.6	01Jan2018, 13:00	3.17

Global Summary Results for Run "10-Year Proposed"

Project: EDD-1 Simulation Run: 10-Year Proposed

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
 End of Run: 02Jan2018, 00:15 Meteorologic Model: 10-Year  
 Compute Time: 02Sep2020, 10:40:33 Control Specifications: Control 1

Show Elements: All Elements Volume Units:  IN  AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI <sup>2</sup> )	Peak Discharge (CFS)	Time of Peak	Volume (IN)
Artificial Field	0.0027	10.4	01Jan2018, 12:00	5.40
Underdrain	0.0027	7.4	01Jan2018, 12:30	5.37
Area I	0.0027	9.3	01Jan2018, 12:00	4.47
Reach-1	0.0027	6.5	01Jan2018, 12:00	4.47
EDD-1	0.0054	4.2	01Jan2018, 13:00	4.92
Reach-2	0.0054	4.2	01Jan2018, 13:00	4.92
Sink-1	0.0054	4.2	01Jan2018, 13:00	4.92

Global Summary Results for Run "100-Year Proposed"

Project: EDD-1 Simulation Run: 100-Year Proposed

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
 End of Run: 02Jan2018, 00:15 Meteorologic Model: 100-Year  
 Compute Time: 02Sep2020, 10:40:38 Control Specifications: Control 1

Show Elements: All Elements Volume Units:  IN  AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI <sup>2</sup> )	Peak Discharge (CFS)	Time of Peak	Volume (IN)
Artificial Field	0.0027	15.1	01Jan2018, 12:00	7.80
Underdrain	0.0027	10.6	01Jan2018, 12:30	7.76
Area I	0.0027	14.1	01Jan2018, 12:00	6.79
Reach-1	0.0027	9.8	01Jan2018, 12:00	6.79
EDD-1	0.0054	9.9	01Jan2018, 12:45	7.30
Reach-2	0.0054	9.9	01Jan2018, 12:45	7.30
Sink-1	0.0054	9.9	01Jan2018, 12:45	7.30

# LS MIDDLE SCHOOL #4

## HEC HMS-INPUT

### EDD-1 INPUT

**Basin Name: Proposed**  
**Element Name: EDD-1**

Description:

Downstream: Reach-2

Method: Outflow Structures

Storage Method: Elevation-Area

\*Elev-Area Function: Detention 1

Initial Condition: Inflow = Outflow

Main Tailwater: Assume None

Auxiliary: --None--

Time Step Method: Automatic Adaption

Outlets:  1

Spillways:  1

Dam Tops:  0

Pumps:  0

Dam Break: No

Dam Seepage: No

Release: No

Evaporation: No

**Basin Name: Proposed**  
**Element Name: EDD-1**

Method: Orifice Outlet

Direction: Main

Number Barrels:  1

\*Center Elevation (FT) 1008.68

\*Area (FT2) 0.196

\*Coefficient: 0.6

**Basin Name: Proposed**  
**Element Name: EDD-1**

Method: Broad-Crested Spillway

Direction: Main

\*Elevation (FT) 1009.58

\*Length (FT) 6

\*Coefficient (FT<sup>0.5</sup>/S) 1.2

Gates:  0

### EDD-2 INPUT

**Basin Name: Proposed**  
**Element Name: EDD-2**

Description:

Downstream: Reach-2

Method: Outflow Structures

Storage Method: Elevation-Area

\*Elev-Area Function: Detention 2

Initial Condition: Inflow = Outflow

Main Tailwater: Assume None

Auxiliary: --None--

Time Step Method: Automatic Adaption

Outlets:  1

Spillways:  1

Dam Tops:  0

Pumps:  0

Dam Break: No

Dam Seepage: No

Release: No

Evaporation: No

**Basin Name: Proposed**  
**Element Name: EDD-2**

Method: Orifice Outlet

Direction: Main

Number Barrels:  1

\*Center Elevation (FT) 997.87

\*Area (FT2) 0.442

\*Coefficient: 0.6

**Basin Name: Proposed**  
**Element Name: EDD-2**

Method: Broad-Crested Spillway

Direction: Main

\*Elevation (FT) 999.03

\*Length (FT) 8

\*Coefficient (FT<sup>0.5</sup>/S) 1.2

Gates:  0

### EDD-3 INPUT

**Basin Name: Proposed**  
**Element Name: EDD-3**

Description:

Downstream: Reach-2

Method: Outflow Structures

Storage Method: Elevation-Area

\*Elev-Area Function: Detention 3

Initial Condition: Inflow = Outflow

Main Tailwater: Assume None

Auxiliary: --None--

Time Step Method: Automatic Adaption

Outlets:  1

Spillways:  1

Dam Tops:  0

Pumps:  0

Dam Break: No

Dam Seepage: No

Release: No

Evaporation: No

**Basin Name: Proposed**  
**Element Name: EDD-3**

Method: Orifice Outlet

Direction: Main

Number Barrels:  1

\*Center Elevation (FT) 995.13

\*Area (FT2) 1.22

\*Coefficient: 0.6

**Basin Name: Proposed**  
**Element Name: EDD-3**

Method: Broad-Crested Spillway

Direction: Main

\*Elevation (FT) 996.1

\*Length (FT) 16

\*Coefficient (FT<sup>0.5</sup>/S) 1.2

Gates:  0

# LS MIDDLE SCHOOL #4 DETENTION BASIN ELEVATION AREA TABLE

## EDD-1 INPUT

Paired Data Table Graph	
Elevation (FT)	Area (AC)
1007.0	0.00
1008.0	0.04
1009.0	0.14
1010.0	0.26
1011.0	0.38
1012.0	0.41
1013.0	0.44
1014.0	0.48
1015.0	0.55

## EDD-2 INPUT

Paired Data Table Graph	
Elevation (FT)	Area (AC)
995.0	0.00
996.0	0.03
997.0	0.08
998.0	0.19
999.0	0.29
1000.0	0.36
1001.0	0.41
1002.0	0.45
1003.0	0.49
1004.0	0.54
1005.0	0.62

## EDD-3 INPUT

Components Compute Results	
Paired Data Table Graph	
Elevation (FT)	Area (AC)
889.0	0.00
990.0	0.04
991.0	0.28
992.0	0.65
993.0	0.76
994.0	0.82
995.0	0.88
996.0	0.93
997.0	0.99
998.0	1.06
999.0	1.12
1000.0	1.23
1001.0	1.27
1002.0	1.33

# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 1 (EDD-1) SUBBASIN SUMMARIES

Summary Results for Subbasin "Area I"

Project: EDD-1 Simulation Run: 02-Year Proposed  
Subbasin: Area I

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
End of Run: 02Jan2018, 00:15 Meteorologic Model: 2-Year  
Compute Time: 02Sep2020, 10:40:26 Control Specifications: Control 1

Volume Units:  IN  AC-FT

Computed Results

Peak Discharge: 5.8 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 12:00
Precipitation Volume: 3.60 (IN)	Direct Runoff Volume: 2.77 (IN)
Loss Volume: 0.83 (IN)	Baseflow Volume: 0.00 (IN)
Excess Volume: 2.77 (IN)	Discharge Volume: 2.77 (IN)

Summary Results for Subbasin "Artificial Field"

Project: EDD-1 Simulation Run: 02-Year Proposed  
Subbasin: Artificial Field

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
End of Run: 02Jan2018, 00:15 Meteorologic Model: 2-Year  
Compute Time: 02Sep2020, 10:40:26 Control Specifications: Control 1

Volume Units:  IN  AC-FT

Computed Results

Peak Discharge: 7.0 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 12:00
Precipitation Volume: 3.60 (IN)	Direct Runoff Volume: 3.60 (IN)
Loss Volume: 0.00 (IN)	Baseflow Volume: 0.00 (IN)
Excess Volume: 3.60 (IN)	Discharge Volume: 3.60 (IN)

Summary Results for Subbasin "Area I"

Project: EDD-1 Simulation Run: 10-Year Proposed  
Subbasin: Area I

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
End of Run: 02Jan2018, 00:15 Meteorologic Model: 10-Year  
Compute Time: 02Sep2020, 10:40:33 Control Specifications: Control 1

Volume Units:  IN  AC-FT

Computed Results

Peak Discharge: 9.3 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 12:00
Precipitation Volume: 5.40 (IN)	Direct Runoff Volume: 4.47 (IN)
Loss Volume: 0.93 (IN)	Baseflow Volume: 0.00 (IN)
Excess Volume: 4.47 (IN)	Discharge Volume: 4.47 (IN)

Summary Results for Subbasin "Artificial Field"

Project: EDD-1 Simulation Run: 10-Year Proposed  
Subbasin: Artificial Field

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
End of Run: 02Jan2018, 00:15 Meteorologic Model: 10-Year  
Compute Time: 02Sep2020, 10:40:33 Control Specifications: Control 1

Volume Units:  IN  AC-FT

Computed Results

Peak Discharge: 10.4 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 12:00
Precipitation Volume: 5.40 (IN)	Direct Runoff Volume: 5.40 (IN)
Loss Volume: 0.00 (IN)	Baseflow Volume: 0.00 (IN)
Excess Volume: 5.40 (IN)	Discharge Volume: 5.40 (IN)

Summary Results for Subbasin "Area I"

Project: EDD-1 Simulation Run: 100-Year Proposed  
Subbasin: Area I

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
End of Run: 02Jan2018, 00:15 Meteorologic Model: 100-Year  
Compute Time: 02Sep2020, 10:40:38 Control Specifications: Control 1

Volume Units:  IN  AC-FT

Computed Results

Peak Discharge: 14.1 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 12:00
Precipitation Volume: 7.80 (IN)	Direct Runoff Volume: 6.79 (IN)
Loss Volume: 1.01 (IN)	Baseflow Volume: 0.00 (IN)
Excess Volume: 6.79 (IN)	Discharge Volume: 6.79 (IN)

Summary Results for Subbasin "Artificial Field"

Project: EDD-1 Simulation Run: 100-Year Proposed  
Subbasin: Artificial Field

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
End of Run: 02Jan2018, 00:15 Meteorologic Model: 100-Year  
Compute Time: 02Sep2020, 10:40:38 Control Specifications: Control 1

Volume Units:  IN  AC-FT

Computed Results

Peak Discharge: 15.1 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 12:00
Precipitation Volume: 7.80 (IN)	Direct Runoff Volume: 7.80 (IN)
Loss Volume: 0.00 (IN)	Baseflow Volume: 0.00 (IN)
Excess Volume: 7.80 (IN)	Discharge Volume: 7.80 (IN)

# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 1 (EDD-1) DETENTION FLOWS

Summary Results for Reservoir "EDD-1"

Project: Area 1 Turf    Simulation Run: 02-Year Proposed  
Reservoir: EDD-1

Start of Run: 01Jan2018, 00:00    Basin Model: Proposed  
End of Run: 02Jan2018, 00:15    Meteorologic Model: 2-Year  
Compute Time: 01Sep2020, 11:03:49    Control Specifications: Control 1

Volume Units:  IN     AC-FT

Computed Results

Peak Inflow: 5.7 (CFS)	Date/Time of Peak Inflow: 01Jan2018, 12:30
Peak Discharge: 1.6 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 13:00
Inflow Volume: 3.17 (IN)	Peak Storage: 0.3 (AC-FT)
Discharge Volume: 3.16 (IN)	Peak Elevation: 1009.7 (FT)

Summary Results for Reservoir "EDD-1"

Project: Area 1 Turf    Simulation Run: 10-Year Proposed  
Reservoir: EDD-1

Start of Run: 01Jan2018, 00:00    Basin Model: Proposed  
End of Run: 02Jan2018, 00:15    Meteorologic Model: 10-Year  
Compute Time: 01Sep2020, 11:03:50    Control Specifications: Control 1

Volume Units:  IN     AC-FT

Computed Results

Peak Inflow: 8.6 (CFS)	Date/Time of Peak Inflow: 01Jan2018, 12:30
Peak Discharge: 4.2 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 13:00
Inflow Volume: 4.92 (IN)	Peak Storage: 0.5 (AC-FT)
Discharge Volume: 4.92 (IN)	Peak Elevation: 1010.3 (FT)

Summary Results for Reservoir "EDD-1"

Project: Area 1 Turf    Simulation Run: 100-Year Proposed  
Reservoir: EDD-1

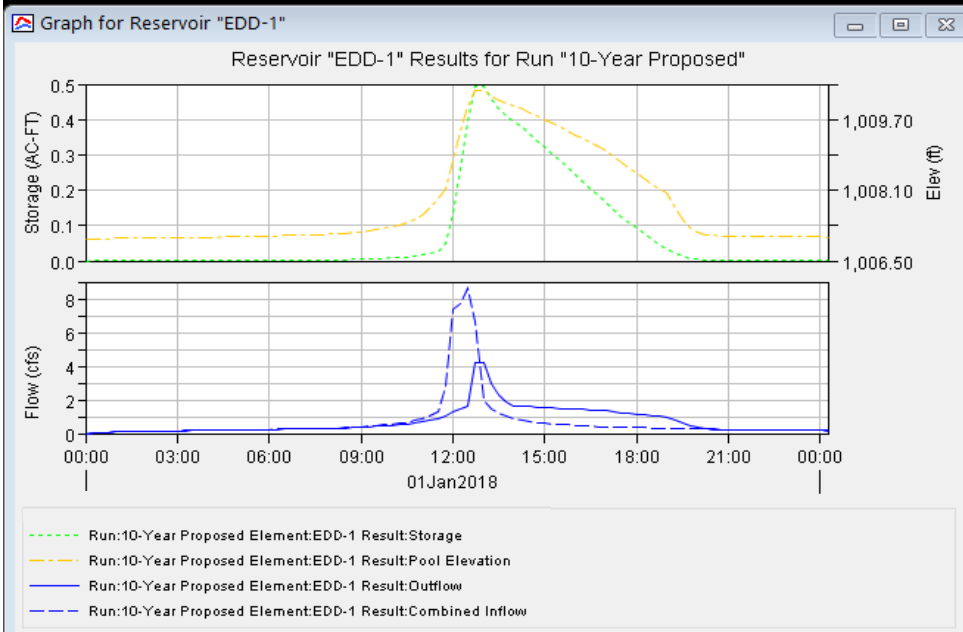
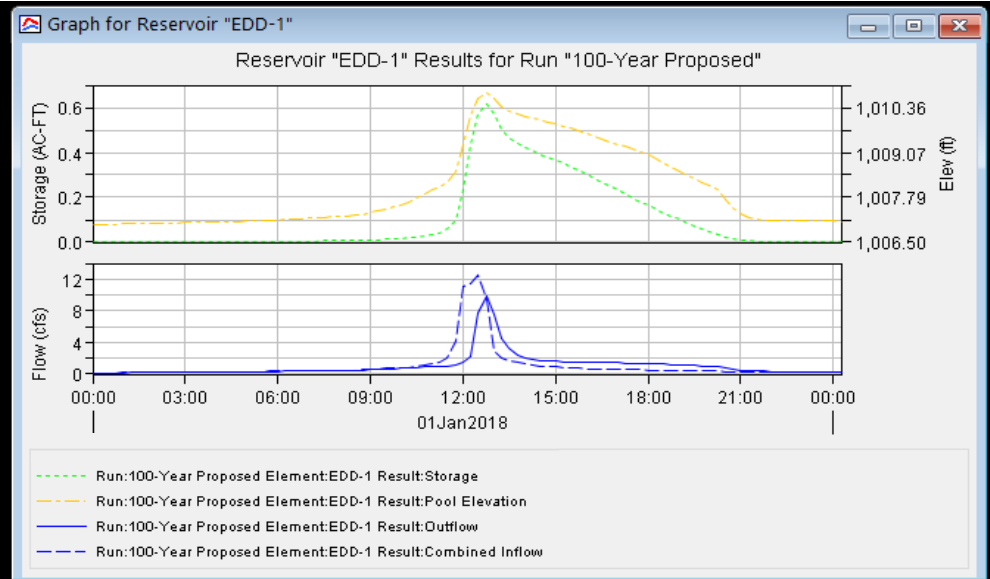
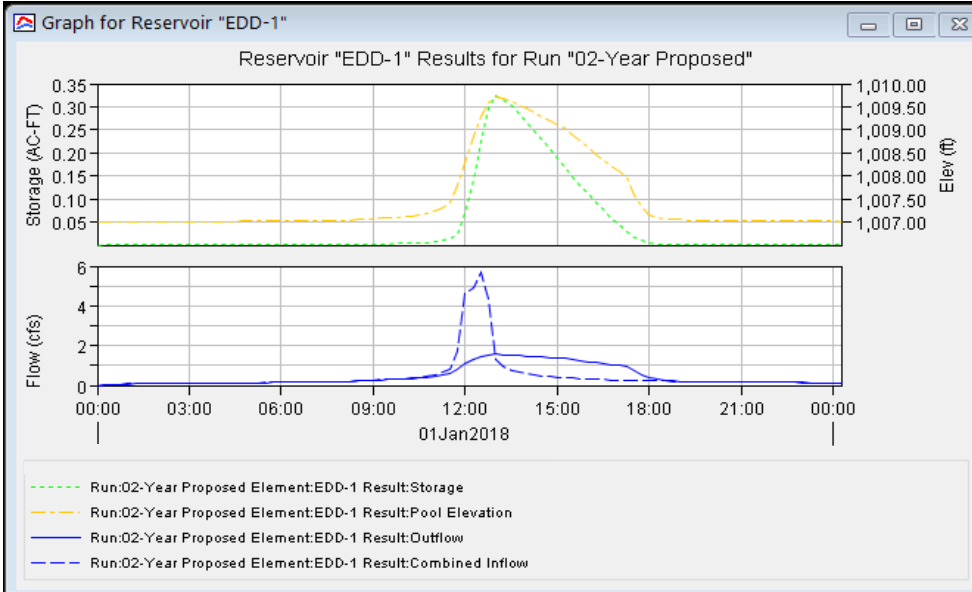
Start of Run: 01Jan2018, 00:00    Basin Model: Proposed  
End of Run: 02Jan2018, 00:15    Meteorologic Model: 100-Year  
Compute Time: 01Sep2020, 11:03:50    Control Specifications: Control 1

Volume Units:  IN     AC-FT

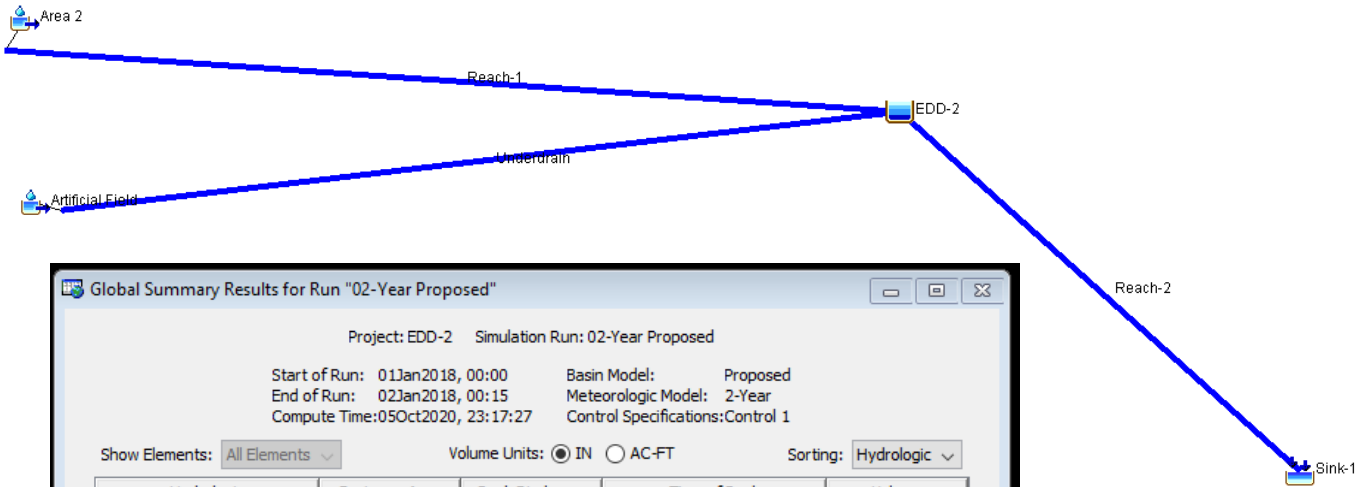
Computed Results

Peak Inflow: 12.5 (CFS)	Date/Time of Peak Inflow: 01Jan2018, 12:30
Peak Discharge: 9.9 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 12:45
Inflow Volume: 7.27 (IN)	Peak Storage: 0.6 (AC-FT)
Discharge Volume: 7.30 (IN)	Peak Elevation: 1010.8 (FT)

# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 1 (EDD-1) - HYDROGRAPHS



# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 2 (EDD-2) HEC-HMS BASIN MODEL AND GLOBAL SUMMARIES



Global Summary Results for Run "02-Year Proposed"

Project: EDD-2 Simulation Run: 02-Year Proposed

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
 End of Run: 02Jan2018, 00:15 Meteorologic Model: 2-Year  
 Compute Time: 05Oct2020, 23:17:27 Control Specifications: Control 1

Show Elements: All Elements Volume Units:  IN  AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
Area 2	0.0093	14.4	01Jan2018, 12:00	2.00
Reach-1	0.0093	9.6	01Jan2018, 12:15	2.00
Artificial Field	0.0035	6.3	01Jan2018, 12:00	2.50
Underdrain	0.0035	4.2	01Jan2018, 12:45	2.49
EDD-2	0.0128	3.7	01Jan2018, 12:45	2.13
Reach-2	0.0128	3.7	01Jan2018, 12:45	2.13
Sink-1	0.0128	3.7	01Jan2018, 12:45	2.13

Global Summary Results for Run "10-Year Proposed"

Project: EDD-2 Simulation Run: 10-Year Proposed

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
 End of Run: 02Jan2018, 00:15 Meteorologic Model: 10-Year  
 Compute Time: 05Oct2020, 23:17:32 Control Specifications: Control 1

Show Elements: All Elements Volume Units:  IN  AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
Area 2	0.0093	34.2	01Jan2018, 12:00	4.81
Reach-1	0.0093	22.7	01Jan2018, 12:15	4.81
Artificial Field	0.0035	13.5	01Jan2018, 12:00	5.40
Underdrain	0.0035	9.0	01Jan2018, 12:45	5.37
EDD-2	0.0128	14.0	01Jan2018, 12:30	4.99
Reach-2	0.0128	14.0	01Jan2018, 12:30	4.99
Sink-1	0.0128	14.0	01Jan2018, 12:30	4.99

Global Summary Results for Run "100-Year Proposed"

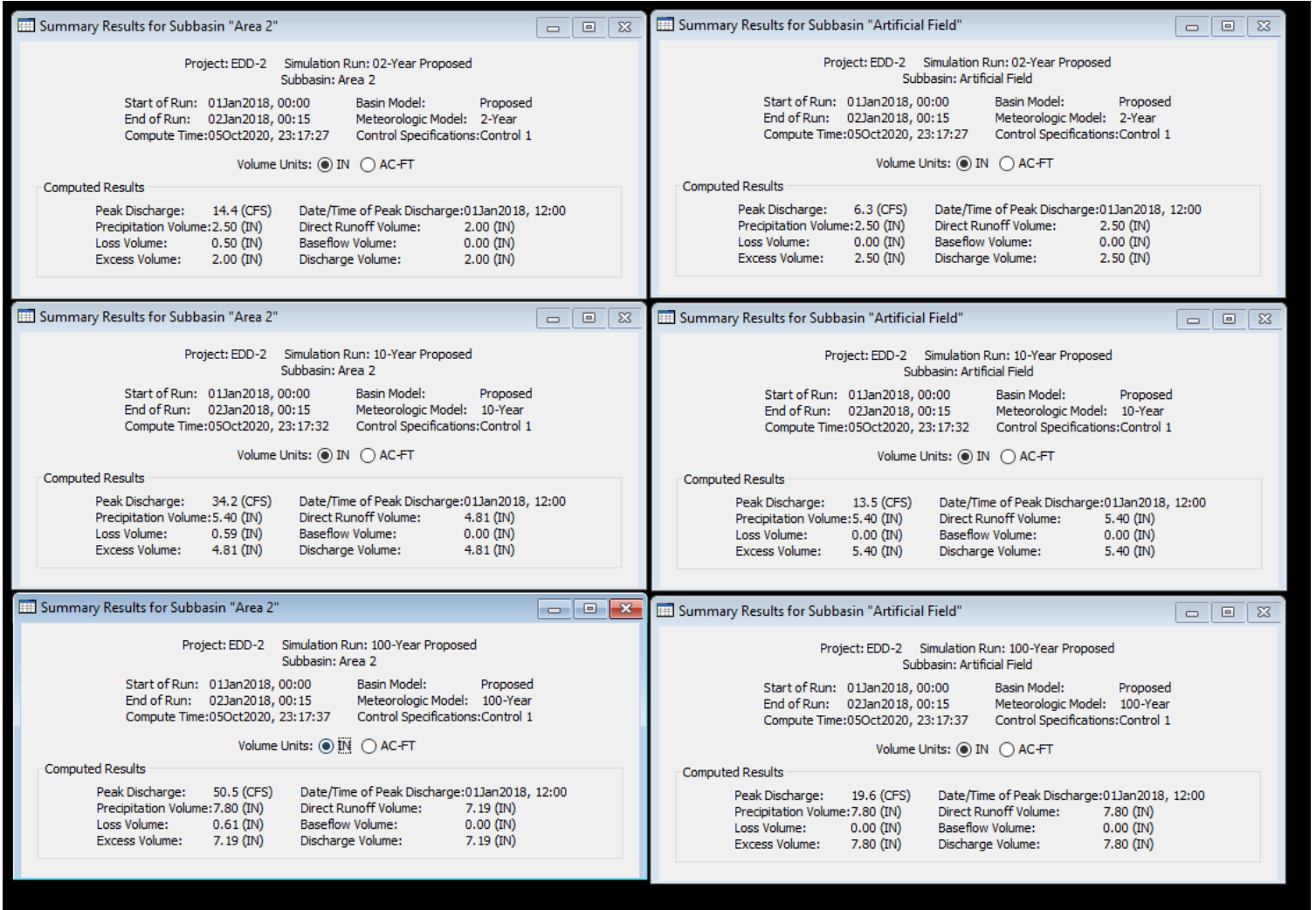
Project: EDD-2 Simulation Run: 100-Year Proposed

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
 End of Run: 02Jan2018, 00:15 Meteorologic Model: 100-Year  
 Compute Time: 05Oct2020, 23:17:37 Control Specifications: Control 1

Show Elements: All Elements Volume Units:  IN  AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
Area 2	0.0093	50.5	01Jan2018, 12:00	7.19
Reach-1	0.0093	33.5	01Jan2018, 12:15	7.18
Artificial Field	0.0035	19.6	01Jan2018, 12:00	7.80
Underdrain	0.0035	13.0	01Jan2018, 12:45	7.75
EDD-2	0.0128	24.1	01Jan2018, 12:30	7.36
Reach-2	0.0128	24.1	01Jan2018, 12:30	7.36
Sink-1	0.0128	24.1	01Jan2018, 12:30	7.36

# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 2 (EDD-2) SUBBASIN SUMMARIES





# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 2 (EDD-2) DETENTION FLOWS

Project: EDD-2    Simulation Run: 02-Year Proposed  
Reservoir: EDD-2

Start of Run: 01Jan2018, 00:00    Basin Model: Proposed  
End of Run: 02Jan2018, 00:15    Meteorologic Model: 2-Year  
Compute Time: 05Oct2020, 23:17:27    Control Specifications: Control 1

Volume Units:  IN     AC-FT

Computed Results

Peak Inflow: 10.9 (CFS)	Date/Time of Peak Inflow: 01Jan2018, 12:15
Peak Discharge: 3.7 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 12:45
Inflow Volume: 2.13 (IN)	Peak Storage: 0.5 (AC-FT)
Discharge Volume: 2.13 (IN)	Peak Elevation: 998.9 (FT)

Summary Results for Reservoir "EDD-2"

Project: EDD-2    Simulation Run: 10-Year Proposed  
Reservoir: EDD-2

Start of Run: 01Jan2018, 00:00    Basin Model: Proposed  
End of Run: 02Jan2018, 00:15    Meteorologic Model: 10-Year  
Compute Time: 05Oct2020, 23:17:32    Control Specifications: Control 1

Volume Units:  IN     AC-FT

Computed Results

Peak Inflow: 25.5 (CFS)	Date/Time of Peak Inflow: 01Jan2018, 12:15
Peak Discharge: 14.0 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 12:30
Inflow Volume: 4.96 (IN)	Peak Storage: 0.9 (AC-FT)
Discharge Volume: 4.99 (IN)	Peak Elevation: 1000.3 (FT)

Summary Results for Reservoir "EDD-2"

Project: EDD-2    Simulation Run: 100-Year Proposed  
Reservoir: EDD-2

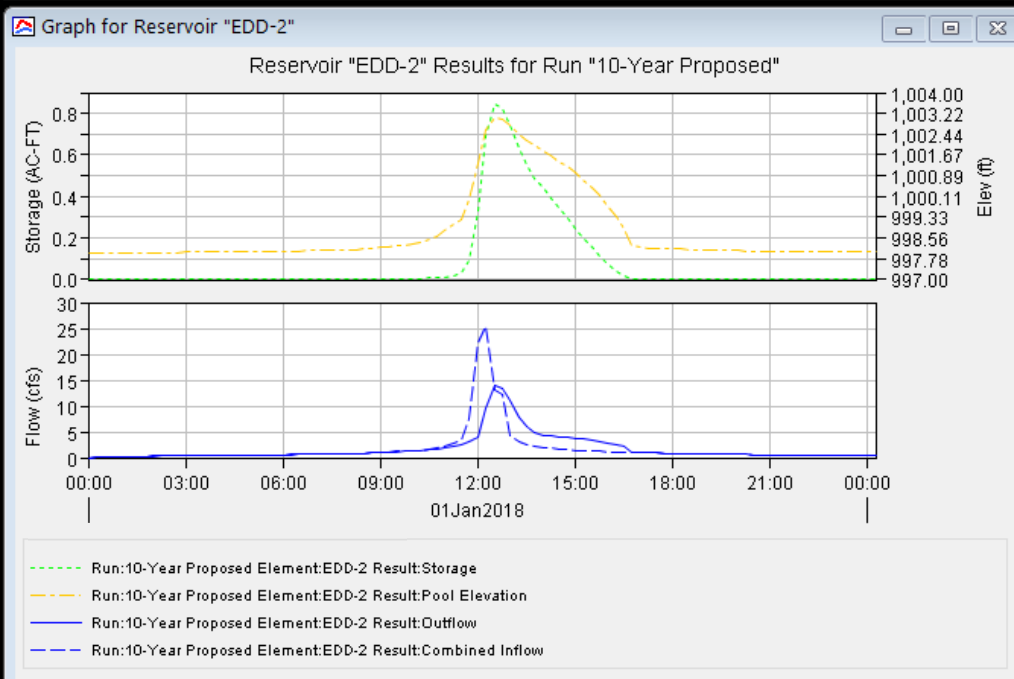
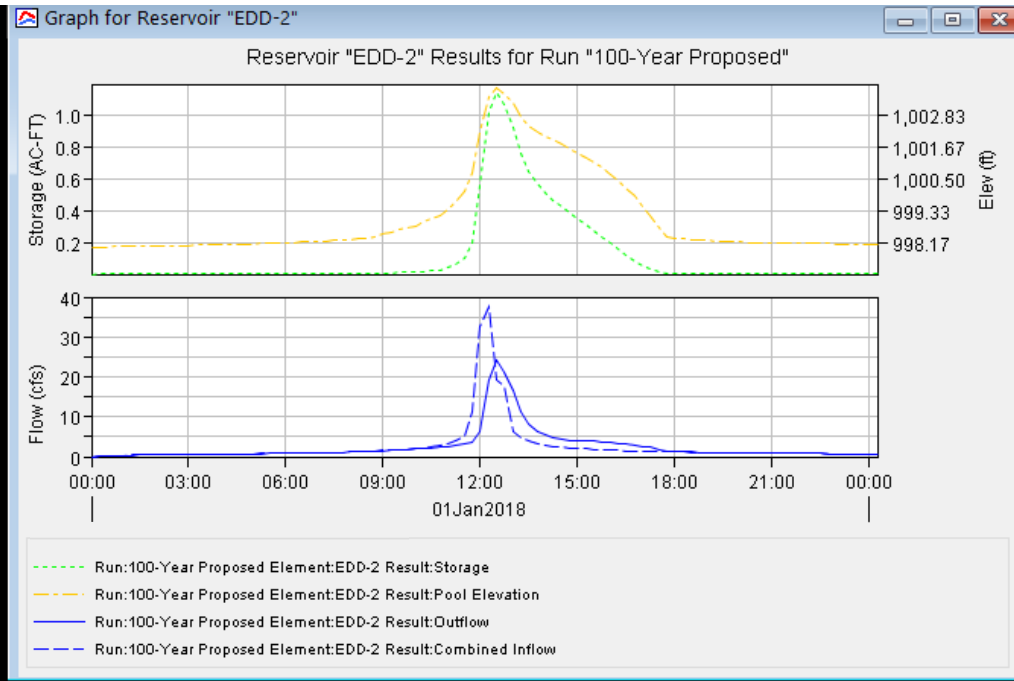
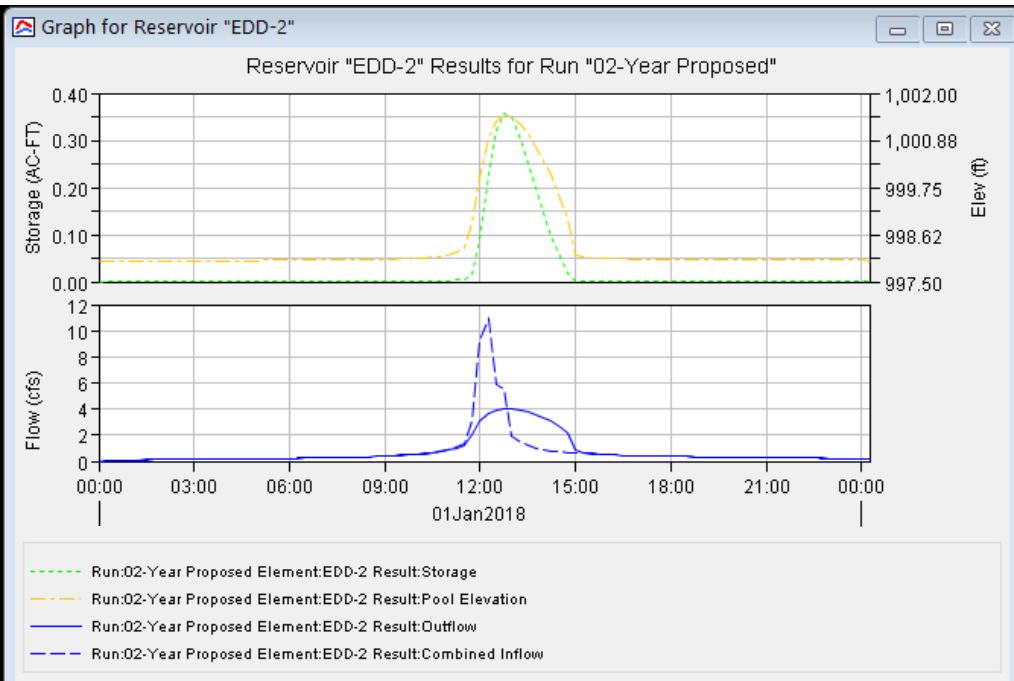
Start of Run: 01Jan2018, 00:00    Basin Model: Proposed  
End of Run: 02Jan2018, 00:15    Meteorologic Model: 100-Year  
Compute Time: 05Oct2020, 23:17:37    Control Specifications: Control 1

Volume Units:  IN     AC-FT

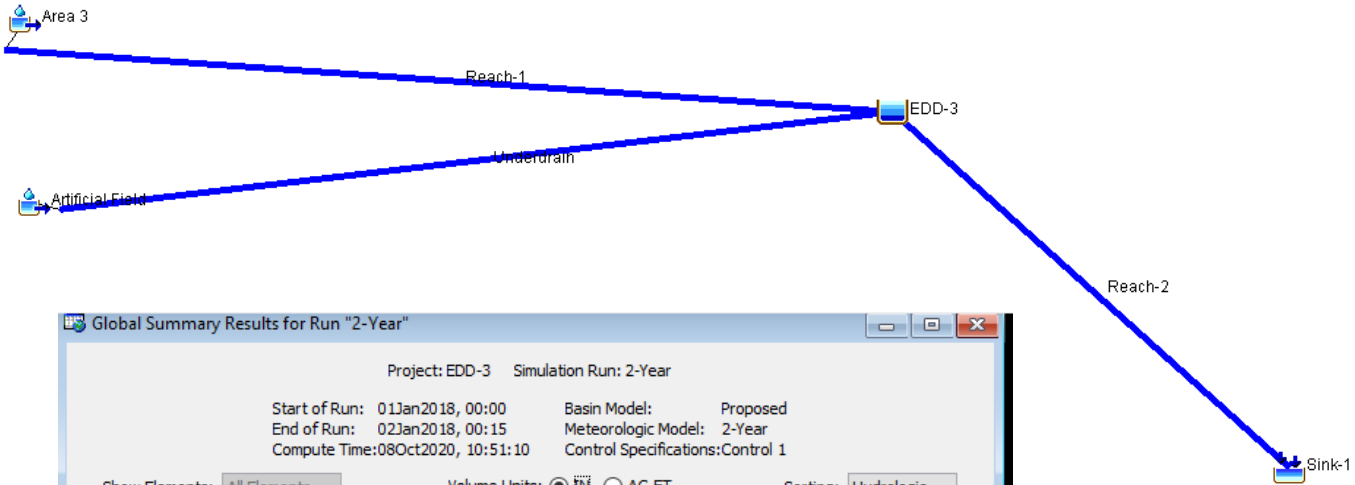
Computed Results

Peak Inflow: 37.5 (CFS)	Date/Time of Peak Inflow: 01Jan2018, 12:15
Peak Discharge: 24.1 (CFS)	Date/Time of Peak Discharge: 01Jan2018, 12:30
Inflow Volume: 7.34 (IN)	Peak Storage: 1.3 (AC-FT)
Discharge Volume: 7.36 (IN)	Peak Elevation: 1001.1 (FT)

# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 2 (EDD-2) - HYDROGRAPHS



# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 3 (EDD-3) HEC-HMS BASIN MODEL AND GLOBAL SUMMARIES



Global Summary Results for Run "2-Year"

Project: EDD-3 Simulation Run: 2-Year

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
 End of Run: 02Jan2018, 00:15 Meteorologic Model: 2-Year  
 Compute Time: 08Oct2020, 10:51:10 Control Specifications: Control 1

Show Elements: All Elements Volume Units:  IN  AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
Area 3	0.0290	45.1	01Jan2018, 12:00	2.00
Reach-1	0.0290	37.5	01Jan2018, 12:15	2.00
Artificial Field	0.0033	5.9	01Jan2018, 12:00	2.50
Underdrain	0.0033	4.9	01Jan2018, 12:45	2.48
EDD-3	0.0323	11.1	01Jan2018, 12:30	2.04
Reach-2	0.0323	11.1	01Jan2018, 12:30	2.04
Sink-1	0.0323	11.1	01Jan2018, 12:30	2.04

Global Summary Results for Run "10-Year Proposed"

Project: EDD-3 Simulation Run: 10-Year Proposed

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
 End of Run: 02Jan2018, 00:15 Meteorologic Model: 10-Year  
 Compute Time: 08Oct2020, 10:51:00 Control Specifications: Control 1

Show Elements: All Elements Volume Units:  IN  AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
Area 3	0.0290	106.7	01Jan2018, 12:00	4.81
Reach-1	0.0290	88.8	01Jan2018, 12:15	4.81
Artificial Field	0.0033	12.8	01Jan2018, 12:00	5.40
Underdrain	0.0033	10.6	01Jan2018, 12:45	5.37
EDD-3	0.0323	35.9	01Jan2018, 12:30	4.89
Reach-2	0.0323	35.9	01Jan2018, 12:30	4.89
Sink-1	0.0323	35.9	01Jan2018, 12:30	4.89

Global Summary Results for Run "100-Year Proposed"

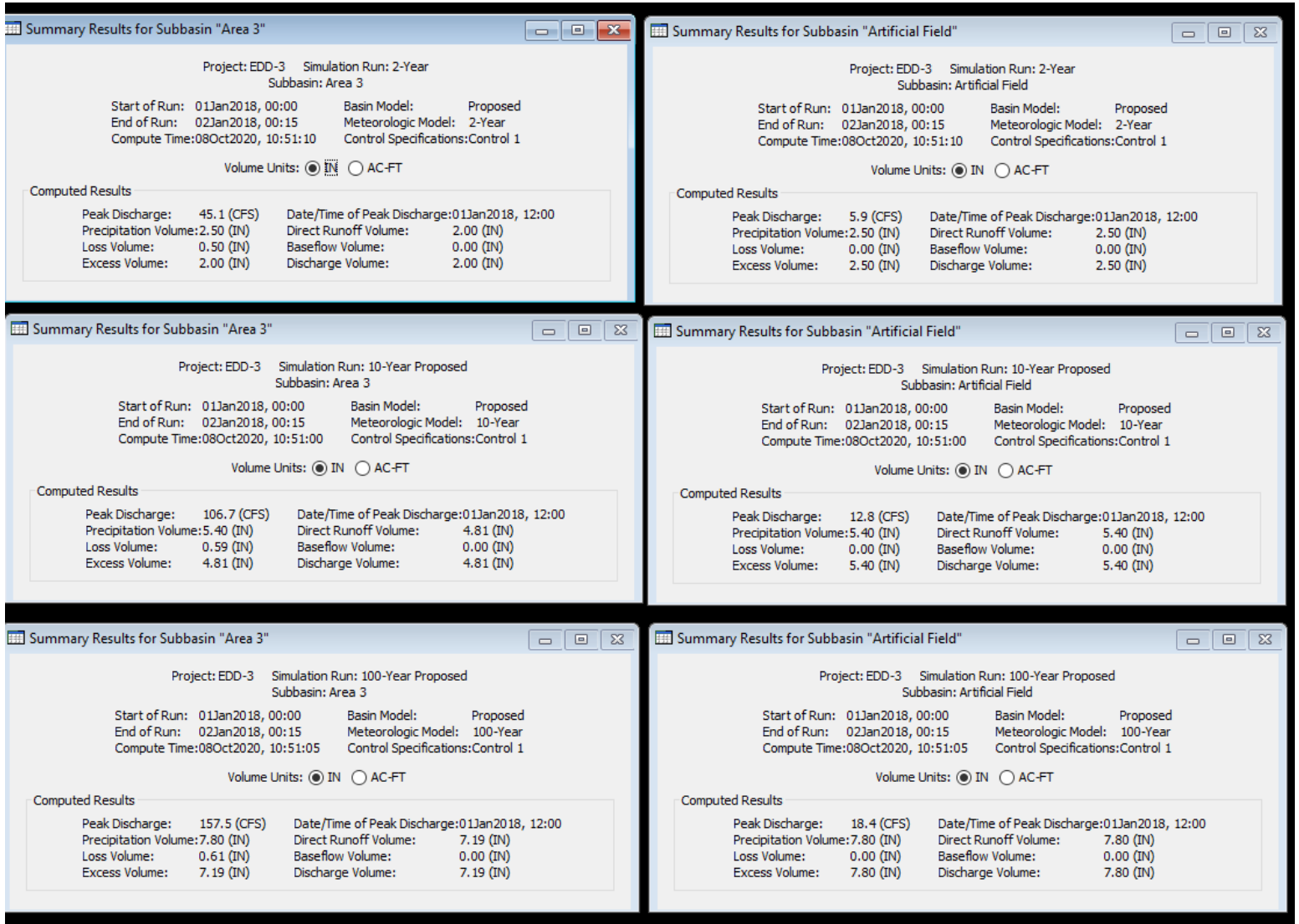
Project: EDD-3 Simulation Run: 100-Year Proposed

Start of Run: 01Jan2018, 00:00 Basin Model: Proposed  
 End of Run: 02Jan2018, 00:15 Meteorologic Model: 100-Year  
 Compute Time: 08Oct2020, 10:51:05 Control Specifications: Control 1

Show Elements: All Elements Volume Units:  IN  AC-FT Sorting: Hydrologic

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
Area 3	0.0290	157.5	01Jan2018, 12:00	7.19
Reach-1	0.0290	131.0	01Jan2018, 12:15	7.18
Artificial Field	0.0033	18.4	01Jan2018, 12:00	7.80
Underdrain	0.0033	15.3	01Jan2018, 12:45	7.75
EDD-3	0.0323	63.0	01Jan2018, 12:30	7.25
Reach-2	0.0323	63.0	01Jan2018, 12:30	7.25
Sink-1	0.0323	63.0	01Jan2018, 12:30	7.25

# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 3 (EDD-3) SUBBASIN SUMMARIES



# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 3 (EDD-3) DETENTION FLOWS

Summary Results for Reservoir "EDD-3"

Project: EDD-3    Simulation Run: 2-Year  
Reservoir: EDD-3

Start of Run: 01Jan2018, 00:00    Basin Model: Proposed  
End of Run: 02Jan2018, 00:15    Meteorologic Model: 2-Year  
Compute Time: 05Oct2020, 22:57:30    Control Specifications: Control 1

Volume Units:  IN     AC-FT

Computed Results

Peak Inflow:	38.4 (CFS)	Date/Time of Peak Inflow:	01Jan2018, 12:15
Peak Discharge:	11.1 (CFS)	Date/Time of Peak Discharge:	01Jan2018, 12:30
Inflow Volume:	2.05 (IN)	Peak Storage:	2.1 (AC-FT)
Discharge Volume:	2.04 (IN)	Peak Elevation:	995.7 (FT)

Summary Results for Reservoir "EDD-3"

Project: EDD-3    Simulation Run: 10-Year Proposed  
Reservoir: EDD-3

Start of Run: 01Jan2018, 00:00    Basin Model: Proposed  
End of Run: 02Jan2018, 00:15    Meteorologic Model: 10-Year  
Compute Time: 05Oct2020, 22:57:20    Control Specifications: Control 1

Volume Units:  IN     AC-FT

Computed Results

Peak Inflow:	90.7 (CFS)	Date/Time of Peak Inflow:	01Jan2018, 12:15
Peak Discharge:	35.9 (CFS)	Date/Time of Peak Discharge:	01Jan2018, 12:30
Inflow Volume:	4.87 (IN)	Peak Storage:	3.7 (AC-FT)
Discharge Volume:	4.89 (IN)	Peak Elevation:	997.4 (FT)

Summary Results for Reservoir "EDD-3"

Project: EDD-3    Simulation Run: 100-Year Proposed  
Reservoir: EDD-3

Start of Run: 01Jan2018, 00:00    Basin Model: Proposed  
End of Run: 02Jan2018, 00:15    Meteorologic Model: 100-Year  
Compute Time: 05Oct2020, 22:57:25    Control Specifications: Control 1

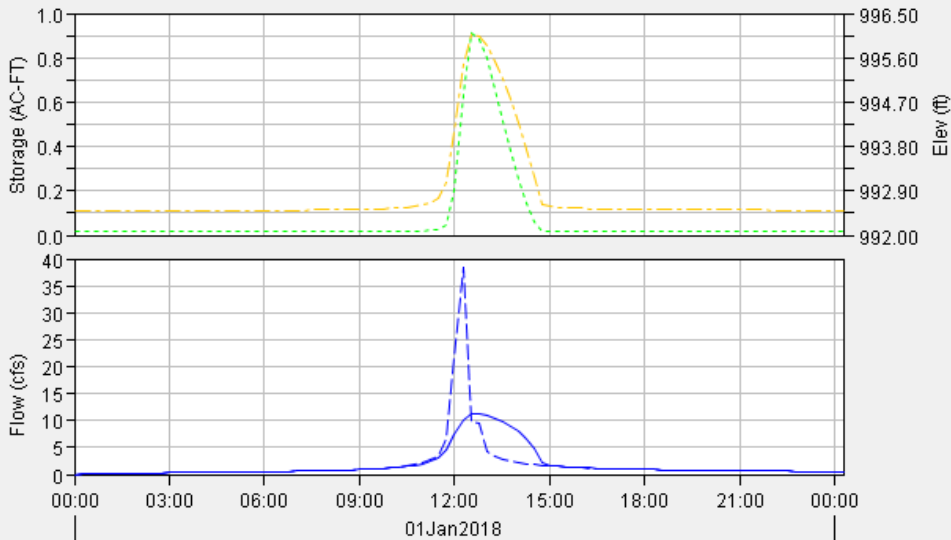
Volume Units:  IN     AC-FT

Computed Results

Peak Inflow:	133.8 (CFS)	Date/Time of Peak Inflow:	01Jan2018, 12:15
Peak Discharge:	63.0 (CFS)	Date/Time of Peak Discharge:	01Jan2018, 12:30
Inflow Volume:	7.24 (IN)	Peak Storage:	4.7 (AC-FT)
Discharge Volume:	7.25 (IN)	Peak Elevation:	998.4 (FT)

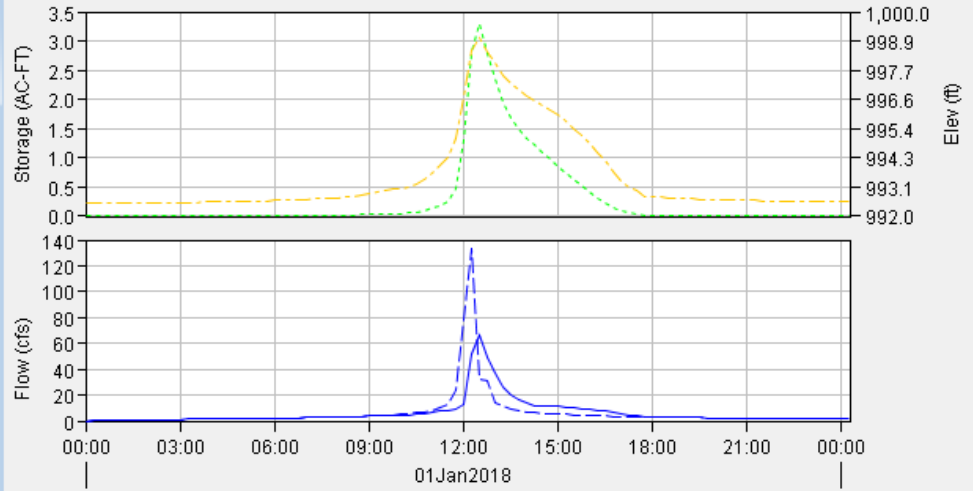
# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 3 (EDD-3) - HYDROGRAPHS

Reservoir "EDD-3" Results for Run "2-Year"



- Run:2-Year Element:EDD-3 Result:Storage
- Run:2-Year Element:EDD-3 Result:Pool Elevation
- Run:2-Year Element:EDD-3 Result:Outflow
- Run:2-Year Element:EDD-3 Result:Combined Inflow

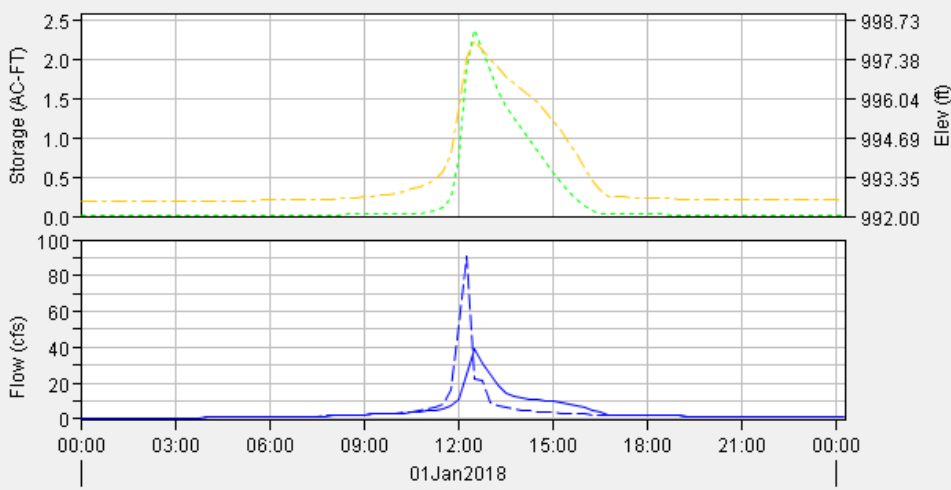
Reservoir "EDD-3" Results for Run "100-Year Proposed"



- Run:100-Year Proposed Element:EDD-3 Result:Storage
- Run:100-Year Proposed Element:EDD-3 Result:Pool Elevation
- Run:100-Year Proposed Element:EDD-3 Result:Outflow
- Run:100-Year Proposed Element:EDD-3 Result:Combined Inflow

Graph for Reservoir "EDD-3"

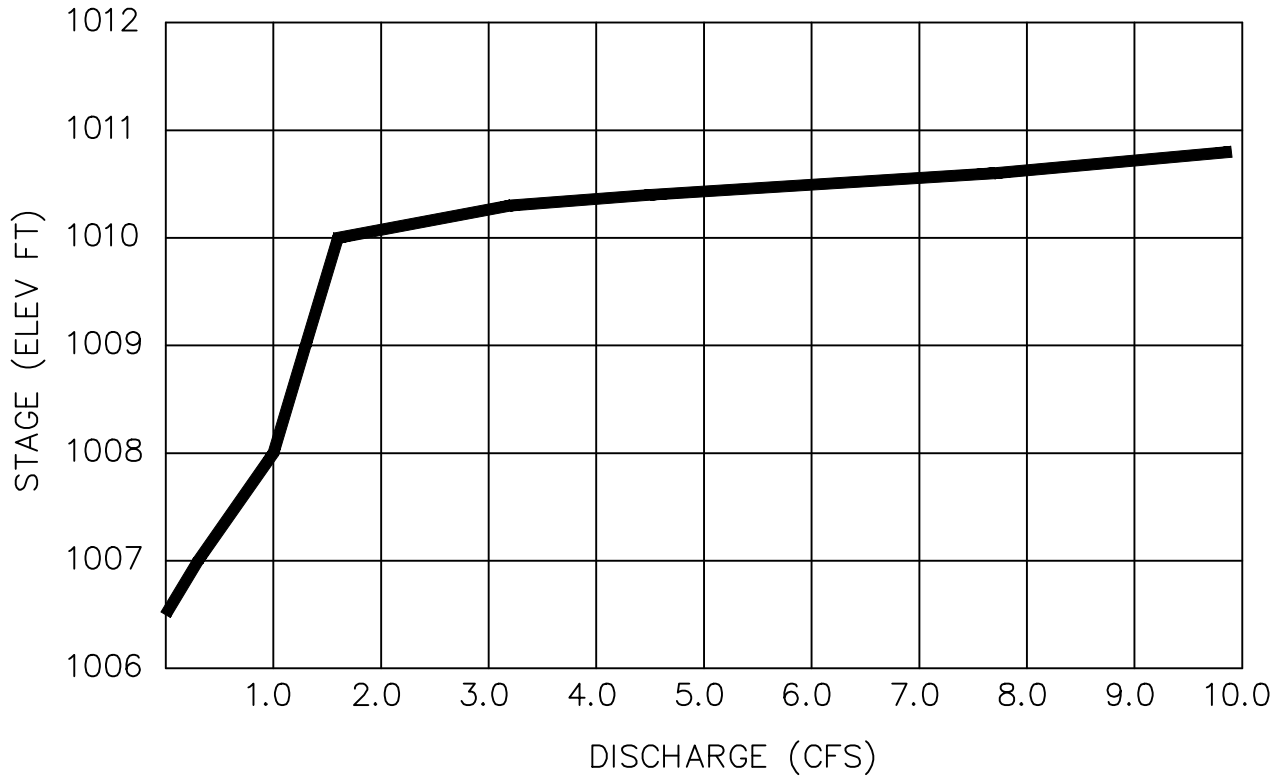
Reservoir "EDD-3" Results for Run "10-Year Proposed"



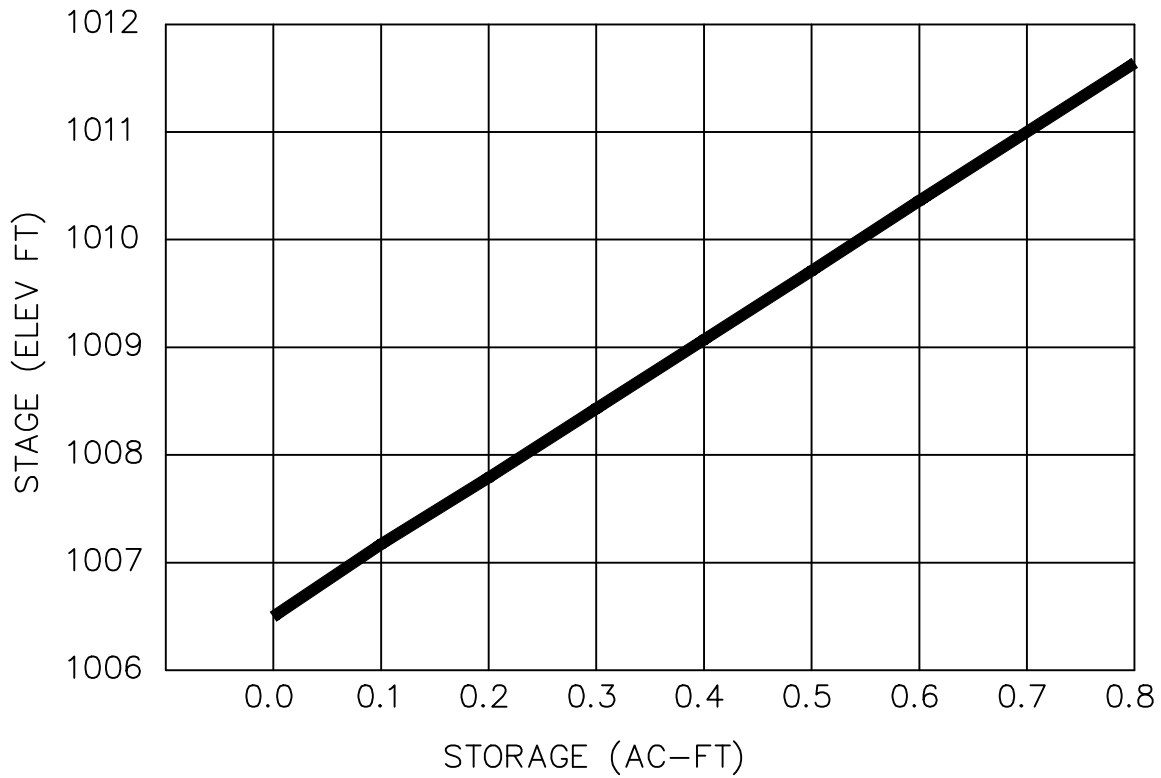
- Run:10-Year Proposed Element:EDD-3 Result:Storage
- Run:10-Year Proposed Element:EDD-3 Result:Pool Elevation
- Run:10-Year Proposed Element:EDD-3 Result:Outflow
- Run:10-Year Proposed Element:EDD-3 Result:Combined Inflow

# LSMS EDD-1

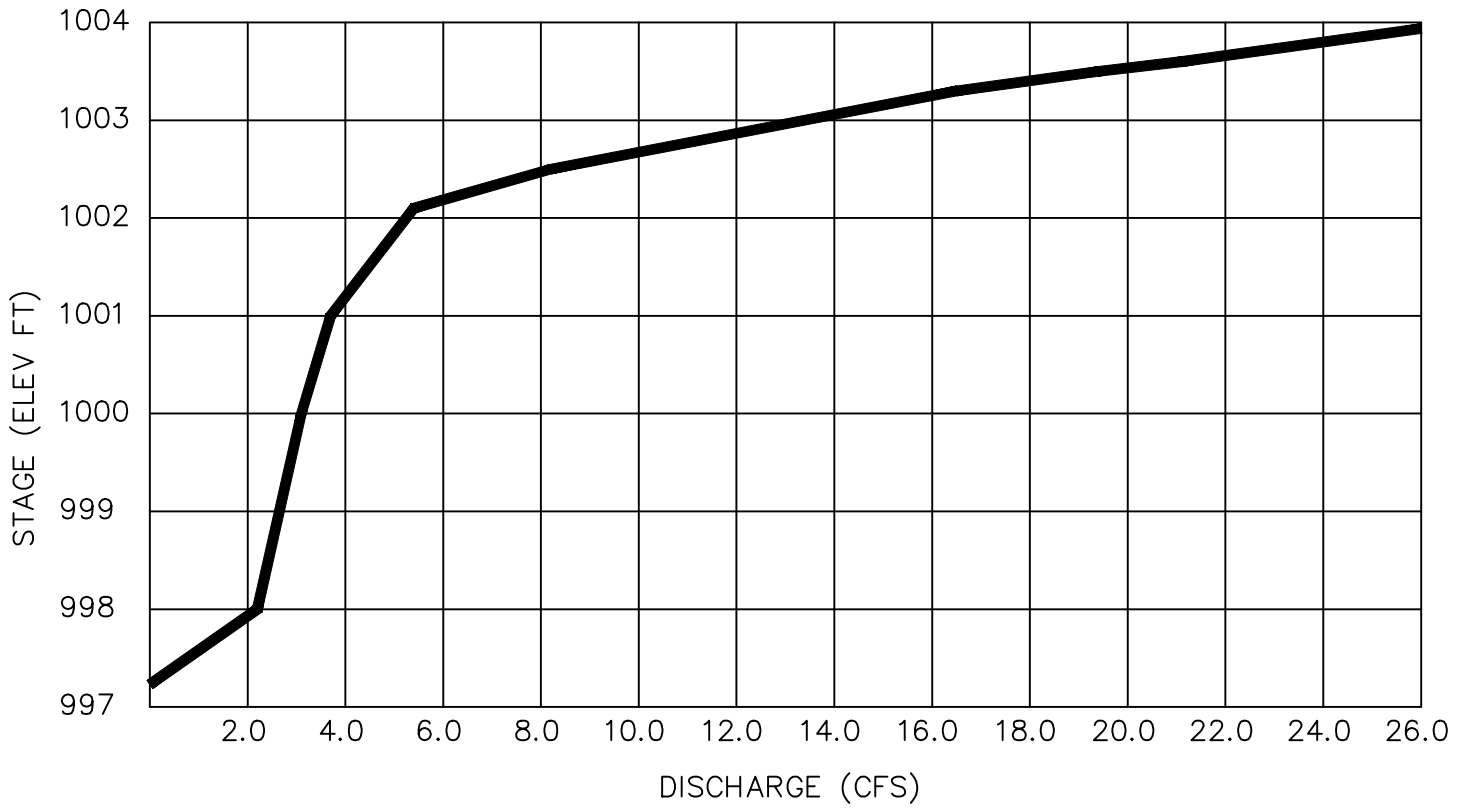
## STAGE-DISCHARGE CURVE



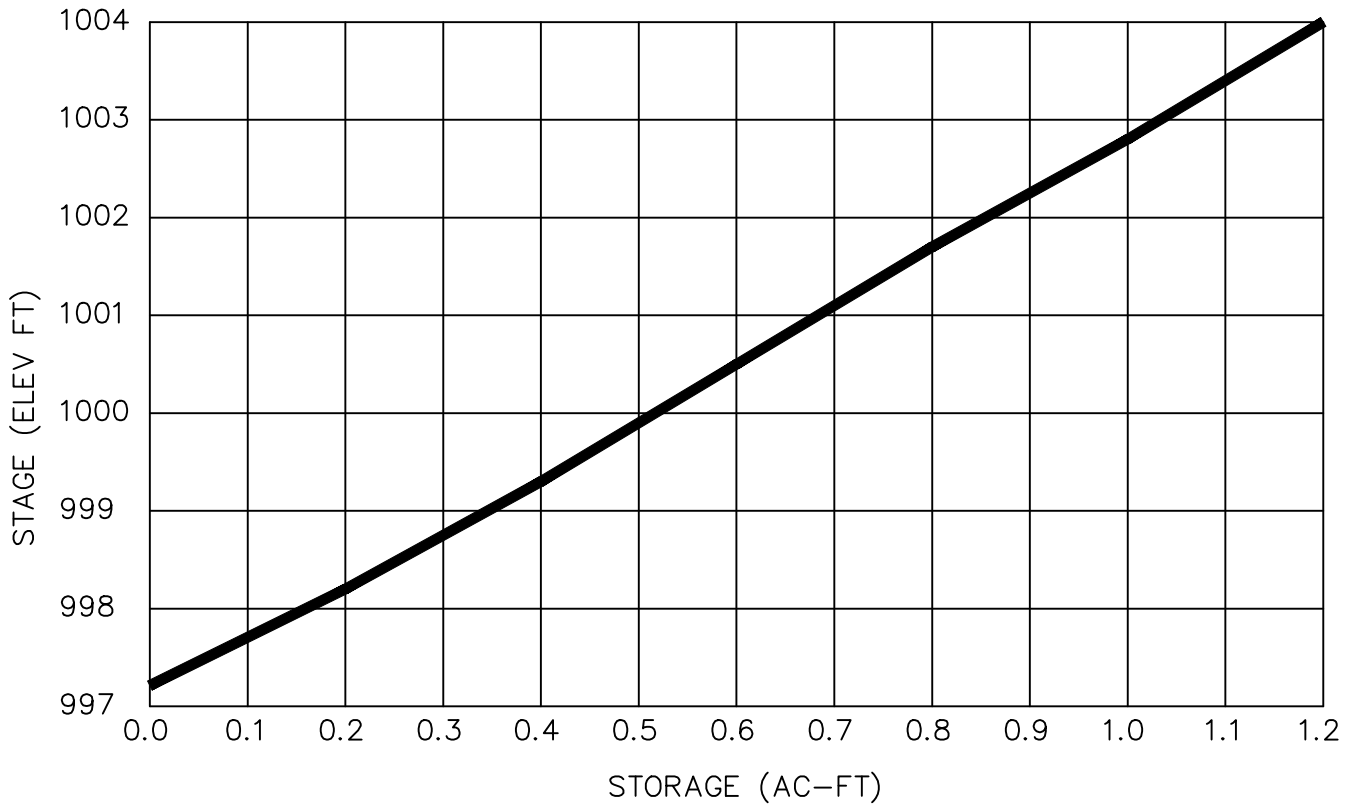
## STAGE-STORAGE CURVE



# LSMS EDD-2 STAGE-DISCHARGE CURVE



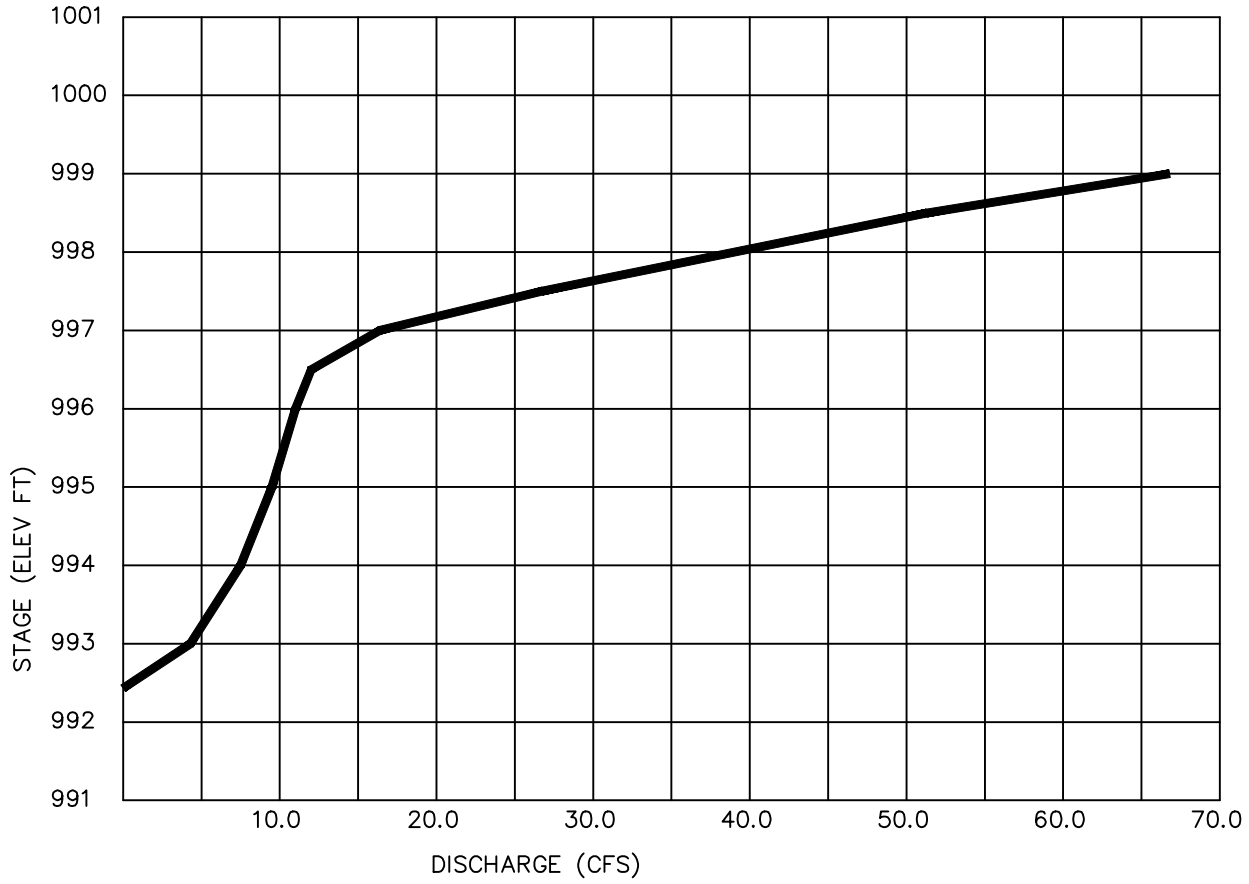
# STAGE-STORAGE CURVE



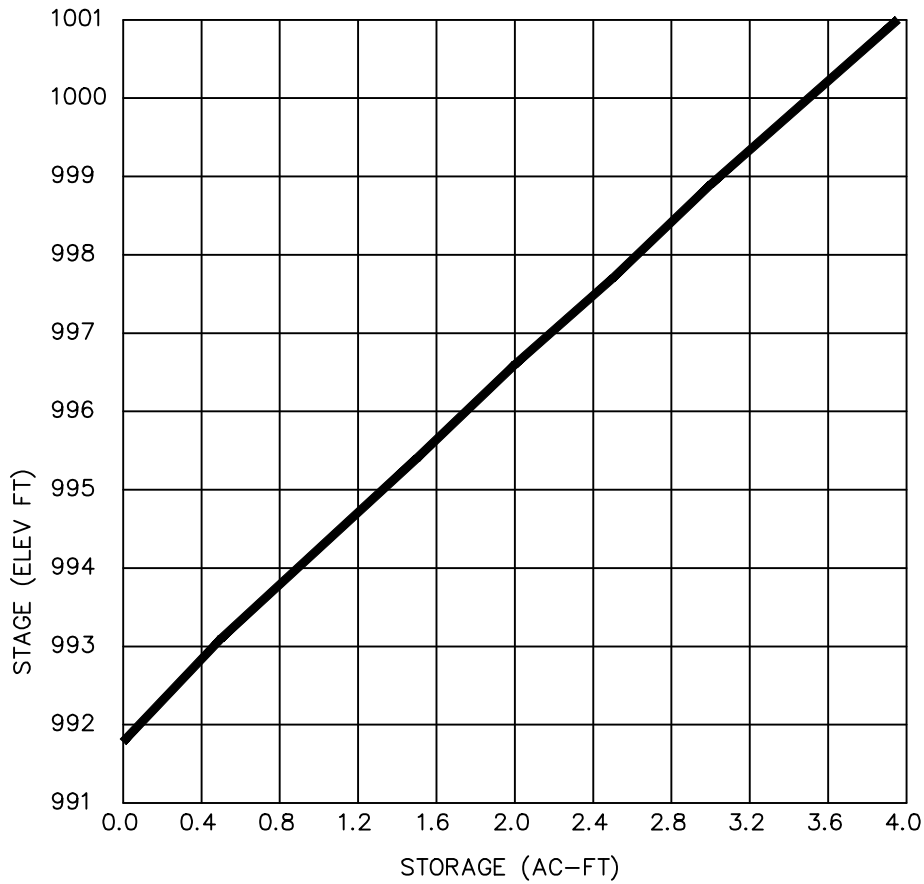


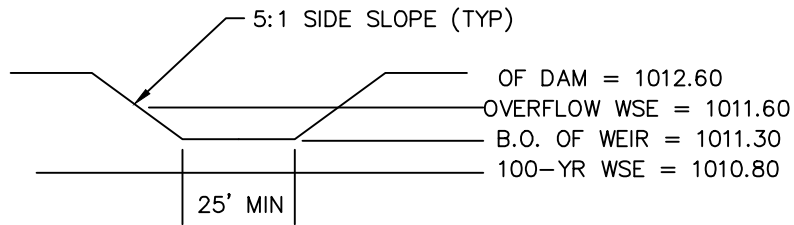
# LSMS EDD-3

## STAGE-DISCHARGE CURVE



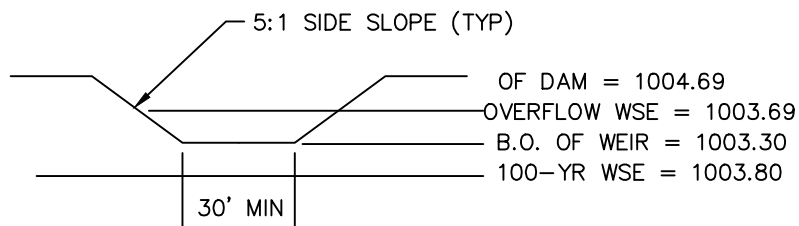
## STAGE-STORAGE CURVE





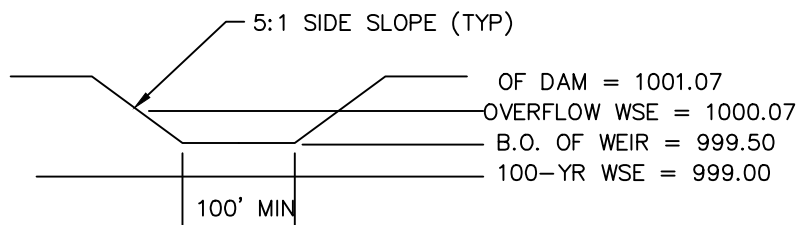
Q = 12.5 CFS  
 DEPTH = 0.30 FT  
 V = 2.2 FPS

EDD-1  
EMERGENCY SPILLWAY DETAIL



Q = 37.8 CFS  
 DEPTH = 0.30 FT  
 V = 2.2 FPS

EDD-2  
EMERGENCY SPILLWAY DETAIL



Q = 134 CFS  
 DEPTH = 0.57 FT  
 V = 2.3 FPS

EDD-3  
EMERGENCY SPILLWAY DETAIL