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DATE: Oct 01, 2020 8:39pm
USER: oadigaliyev
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GENERAL NOTES

1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT THE PLANS IN THEIR POSSESSION ARE THE MOST CURRENT VERSION ISSUED, ARE FULLY COORDINATED WITH ALL SUBCONTRACTORS, AND PRESENT ON SITE AT ALL TIMES. CURRENT PLANS PREPARED BY OLSSON MAY BE OBTAINED AT THE DIRECTION OF OLSSON'S CLIENT. DIRECT REQUESTS TO OLSSON MAY REQUIRE ADDITIONAL AUTHORIZATIONS, AGREEMENTS, AND/OR FEES. PLEASE CONTACT THE ENGINEER FOR INFORMATION.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DEVIATIONS FROM THESE PLANS UNLESS WRITTEN APPROVAL FROM ENGINEER, OWNER, AND DEVELOPER.
3. ALL WORK AND MATERIALS SHALL BE SUBJECT TO INSPECTION AND APPROVAL BY THE OWNER OR THE OWNER'S REPRESENTATIVE.
4. ALL ESTIMATES OF QUANTITIES ARE FOR INFORMATIONAL PURPOSES ONLY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING QUANTITIES AND ITEMS OF WORK.
5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL LABOR, MATERIALS, AND EQUIPMENT REQUIRED TO COMPLETE THE WORK SHOWN IN THE PLANS.
6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL REQUIRED PERMITS, PAYING ALL FEES, AND FOR OTHERWISE COMPLYING WITH ALL APPLICABLE REGULATIONS GOVERNING THE WORK.
7. THE CONTRACTOR SHALL NOT ENGAGE IN ACTIVITIES THAT MAY ENCROACH ON WATERS OF THE U.S., INCLUDING WETLANDS, UNTIL ANY NECESSARY PERMITS MAY BE OBTAINED. THE CONTRACTOR SHALL REVIEW AND COMPLY WITH ALL CONDITIONS DESCRIBED IN THE PERMIT.
8. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, THE SAFETY OF ALL PERSONS INCLUDING VISITORS AND THE GENERAL PUBLIC, AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY THROUGHOUT THE PROJECT AND NOT BE LIMITED BY WORKING HOURS. ANY CONSTRUCTION OBSERVATION BY THE ENGINEER OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES.
9. PRIOR TO COMMENCEMENT OF WORK THE CONTRACTOR SHALL NOTIFY AND COORDINATE WITH ALL UTILITY COMPANIES AND OBTAIN ANY RELEVANT INFORMATION. NOTIFY ENGINEER OF ANY DISCREPANCIES.
10. THE CONTRACTOR IS RESPONSIBLE FOR THE PROTECTION OF ALL BOUNDARY CORNERS AND SECTION CORNERS. ANY BOUNDARY CORNER AND/OR SECTION CORNER DISTURBED OR DAMAGED BY CONSTRUCTION ACTIVITIES SHALL BE RESET BY A LAND SURVEYOR LICENSED IN THE STATE OF MISSOURI, AT THE CONTRACTOR'S EXPENSE.
11. THE CONTRACTOR IS RESPONSIBLE FOR THE PROTECTION OF ADJACENT PROPERTIES AND SHALL TAKE ALL PRECAUTIONS NECESSARY TO PREVENT DAMAGE DURING CONSTRUCTION. THE CONTRACTOR IS ALSO RESPONSIBLE FOR REPAIRING ANY DAMAGE RESULTING FROM CONSTRUCTION ACTIVITIES.
12. PRIOR TO MOVING OFF THE JOB THE CONTRACTOR SHALL NOTIFY THE OWNER AND ENGINEER TO PERFORM A FINAL WALK-THROUGH OF THE CONSTRUCTION SITE.

REFERENCES

1. UNLESS EXPLICITLY DESCRIBED OTHERWISE WITHIN THESE PLANS THE FOLLOWING SHALL APPLY:

A. ALL CONSTRUCTION, INCLUDING THOSE LISTED BELOW, SHALL CONFORM TO THE LATEST CODES AND ORDINANCES OF LEE'S SUMMIT, MISSOURI.

B. ALL CONSTRUCTION IN MODOT RIGHT-OF-WAY SHALL CONFORM TO THE LATEST SPECIFICATIONS ADOPTED BY U.S. DEPARTMENT OF TRANSPORTATION AND MODOT.

C. ALL TRAFFIC CONTROL SIGNAGE SHALL CONFORM WITH THE CURRENT EDITION OF THE MANUAL FOR UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).

D. ALL UTILITY EXTENSIONS AND CONSTRUCTION SHALL CONFORM TO THE STANDARDS AND SPECIFICATIONS OF THE UTILITY COMPANIES.

E. ALL EXTERIOR PAVEMENT (PCC, ASPHALT, ETC.) SHALL BE IN CONFORMANCE WITH THE SPECIFICATIONS OF LEE'S SUMMIT, MISSOURI
4. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING THE DELIVERY MANAGER AND COORDINATING ANY MAILBOXES THAT MAY BE DISTURBED. FAILURE TO DO SO MAY SUBJECT THE CONTRACTOR TO PROSECUTION BY THE FEDERAL GOVERNMENT.

EXISTING CONDITIONS

1. THE CONTRACTOR SHALL VISIT THE SITE AND BECOME FAMILIAR WITH THE EXISTING CONDITIONS OF THE PROJECT AREA.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PERFORMING THEIR OWN INVESTIGATIONS AND MAKING THEIR OWN ASSUMPTIONS REGARDING SITE SURFACE AND SUBSURFACE CONDITIONS. THIS INCLUDES THE LOCATION AND CONSISTENCY OF ANY EXISTING ROCK LAYERS UNDERLYING THE PROJECT SITE. CONTACT THE ENGINEER REGARDING ANY DISCREPANCIES THAT MAY AFFECT THE ABILITY TO CONSTRUCT FROM THESE PLANS AS DESIGNED.
3. EXISTING CONDITIONS WERE DETERMINED THROUGH A VARIETY OF METHODS THAT MAY INCLUDE SURVEY, AERIAL IMAGERY, AVAILABLE RECORDS, GIS DATA, ETC. SUBSURFACE CONDITIONS ARE APPROXIMATE AND MAY NOT INCLUDE ALL UTILITIES AND OTHER SITE IMPROVEMENTS PRESENT ON SITE. THE CONTRACTOR SHALL MAKE EXPLORATION EXCAVATIONS AND LOCATE EXISTING UNDERGROUND UTILITIES SUFFICIENTLY AHEAD OF CONSTRUCTION TO PERMIT REVISIONS TO PLANS WHEN CONFLICTS AND DISCREPANCIES ARE FOUND.

CONSTRUCTION

1. THE CONTRACTOR SHALL INSTALL TRAFFIC CONTROL WHILE WORKING IN THE PUBLIC RIGHT-OF-WAY AS SHOWN IN THESE PLANS. IF PLANS ARE NOT PROVIDED, CONTRACTOR SHALL COORDINATE AND PROVIDE CONTROLS TO THE SATISFACTION OF THE RIGHT-OF-WAY OWNER.
2. THE CONTRACTOR SHALL PROTECT ALL TREES OVER 3" CALIPER FROM DAMAGE. NO TREE SHALL BE REMOVED WITHOUT PERMISSION OF THE OWNER, UNLESS SHOWN OTHERWISE ON THESE PLANS.
3. THE CONTRACTOR SHALL DISPOSE ALL WASTE MATERIAL RESULTING

FROM THE PROJECT OFF-SITE AND IN STRICT CONFORMANCE WITH ALL LOCAL CODES AND ORDINANCES.

4. ALL MANHOLES, CATCH BASINS, UTILITY VALVES AND METER PITS ARE TO BE ADJUSTED OR REBUILT TO GRADE AS REQUIRED. NOT ALL ADJUSTMENTS ARE INDICATED IN THE PLANS.
5. THE CONTRACTOR SHALL STREET SWEEP OR OTHERWISE CLEAN ALL ACCESS ROUTES TO THE SITE AT CONCLUSION OF THE PROJECT.

SHOP DRAWINGS

1. THE CONTRACTOR SHALL SUBMIT SHOP DRAWING A MINIMUM OF 7 DAYS PRIOR TO THE REQUESTED DATE OF APPROVAL. ENGINEER SHALL REVIEW SHOP DRAWINGS OR SAMPLES CONFORMANCE WITH THE DESIGN FOR THIS PROJECT AS DESCRIBED IN THE PLANS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS. THE ENGINEER'S REVIEW SHALL NOT EXTEND TO MEANS OR METHODS OF CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY VARIATION FROM THE REQUIREMENTS OF THE CONTRACT DOCUMENTS UNLESS CONTRACTOR HAS NOTIFIED ENGINEER OF EACH SUCH VARIATION AT THE TIME OF SUBMISSION, AND OBTAINED ENGINEER'S WRITTEN APPROVAL OF EACH SUCH VARIATION. PRIOR TO SUBMITTING EACH SHOP DRAWING OR SAMPLE, CONTRACTOR SHALL HAVE REVIEWED AND VERIFIED:

A. ALL FIELD MEASUREMENTS, QUANTITIES, DIMENSIONS, SPECIFIED PERFORMANCE CRITERIA, INSTALLATION REQUIREMENTS, MATERIALS, CATALOG NUMBERS AND SIMILAR INFORMATION WITH RESPECT THERETO;

B. ALL MATERIALS WITH RESPECT TO INTENDED USE, FABRICATION, SHIPPING, HANDLING, STORAGE, ASSEMBLY AND INSTALLATION PERTAINING TO THE PERFORMANCE OF THE WORK;

C. ALL INFORMATION RELATIVE TO MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OF CONSTRUCTION AND SAFETY PRECAUTIONS AND PROGRAMS INCIDENT THERETO;

D. CONTRACTOR SHALL ALSO HAVE REVIEWED AND COORDINATED EACH SHOP DRAWING OR SAMPLE WITH OTHER SHOP DRAWINGS AND SAMPLES, AND WITH THE REQUIREMENTS OF THE WORK AND THE CONTRACT DOCUMENTS.

E. ALL SUBMITTED SHOP DRAWINGS SHALL BEAR A STAMP OR SPECIFIC WRITTEN INDICATION AND SIGNATURE THAT CONTRACTOR HAS FULLY COMPLETED THE ABOVE TASKS.
2. SHOP DRAWINGS AS DESCRIBED ABOVE ARE REQUIRED FOR, BUT NOT LIMITED TO, THE FOLLOWING:

A. ALL SANITARY SEWER STRUCTURES TO BE INSTALLED WITH THIS PROJECT.

B. ANY ITEMS IN THESE PLANS THAT ALLOW FOR AN "APPROVED EQUAL" ALTERNATIVE.

STORM SEWER GENERAL NOTES:

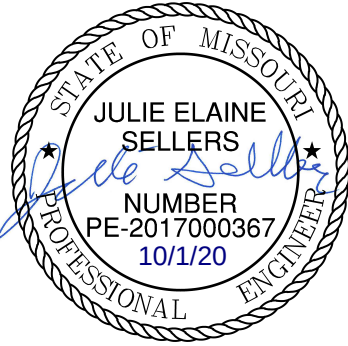
1. STORM STRUCTURES SHALL BE PER CURRENT CITY DETAILS. IF CITY DOES NOT HAVE PUBLISHED DETAILS STRUCTURES SHALL BE PER CURRENT APWA SPECIFICATIONS.
2. PRIOR TO COMMENCEMENT OF WORK THE CONTRACTOR SHALL NOTIFY AND COORDINATE CONSTRUCTION WITH CITY OF LEE'S SUMMIT, MISSOURI.
3. ALL PIPE LENGTHS AND ELEVATIONS ARE CALCULATED LINEARLY FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE.
4. ALL STRUCTURE DIMENSIONS ARE TO INSIDE FACE OF STRUCTURE.
5. COORDINATES ARE PROVIDED AT THE CENTER OF STRUCTURE. ADDITIONAL COORDINATES PROVIDED ARE PER LOCAL CODES AND ORDINANCES OR AS AN AID WHEN ORIENTING THE BOX DURING INSTALLATION.
6. THE CONTRACTOR SHALL EXPOSE EXISTING UTILITIES AT LOCATIONS OF POSSIBLE CONFLICT AND POINTS OF CONNECTION PRIOR TO ANY CONSTRUCTION OF STORM SEWER.
7. STORM SEWER TRENCHES SHALL BE CONSTRUCTED SUCH THAT UNDISTURBED EXISTING SOIL OR FILL COMPACTED TO 95% PROCTOR DENSITY IS AT A DEPTH THAT IS 18" ABOVE TOP OF PROPOSED PIPE.
8. STRUCTURE INVERT CHANNELS SHALL BE SMOOTH, CIRCULAR, AND CONFORMING TO ½ THE ADJACENT PIPE SECTION (INVERT TO CENTER). CHANGES IN DIRECTION OF FLOW SHALL BE MADE WITH A SMOOTH CURVE AND MAINTAIN SHAPE THROUGHOUT. CHANGES IN GRADE OF ADJACENT PIPES SHALL BE TRANSITIONED SMOOTHLY AND EVENLY THROUGH THE STRUCTURE.
9. PIPE PENETRATIONS SHALL BE GROUTED TO ENSURE WATERTIGHT SEALS.
10. MAINTAIN MINIMUM DEPTH OF COVER PER APWA 5606.06

ESTIMATE OF QUANTITIES

ITEM NO.	DESCRIPTION	UNIT	QUANTITY	AS-BUILT
STREET				
1	6" ASPHALT PAVEMENT	S.Y.	4764	
2	6" MODOT TYPE 5 ROCK SUBGRADE	S.Y.	5877	
3	SUBGRADE STABILIZATION (6" FLYASH TREATMENT)	S.Y.	5877	
4	CONCRETE CURB & GUTTER (CG-2)	L.F.	3334	
5	5' CONCRETE SIDEWALK	L.F.	58	
6	ADA RAMP	EA.	9	
7	STOP SIGNS	EA.	4	
8	STREET NAME SIGNS	EA.	4	
STORM				
9	STD. CURB INLET (5'x3' INSIDE)	EA.	14	
10	STD. FIELD INLET (4'x4' INSIDE)	EA.	6	
11	STD. MANHOLE (4' DIA. INSIDE)	EA.	1	
12	15" HDPE	L.F.	1996	
13	18" HDPE	L.F.	328	
14	24" HDPE	L.F.	203	
15	CONNECTION TO EXISTING	EA.	5	

CONTROL POINT TABLE

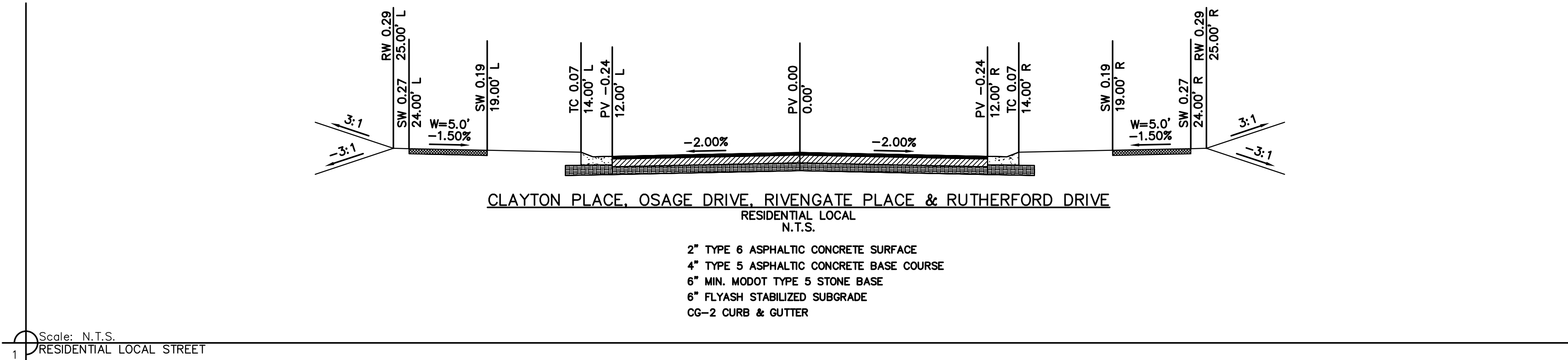
POINT NUMBER	NORTHING	EASTING	POINT ELEVATION	DESCRIPTION
11	978424.224	2811650.442	1030.85	3" IB/CAP SET IN THE CENTER MEDIAN OF ROUTE 150. IT IS DUE NORTH OF THE CENTER LINE OF THE DRIVEWAY TO PROPERTY ADDRESS 2025 ROUTE 150. IT IS 12.15 FT WEST OF A LIGHT POLE, 8.5 FT SOUTH OF THE BACK OF CURB, AND 8.1 FT NORTH OF THE BACK OF CURB.
12	978340.573	2812332.280	1036.11	3" IB/CAP SET ON THE EAST SIDE OF SW PRYOR RD AT THE SOUTHEAST CORNER OF THE INTERSECTION OF ROUTE 150 AND SW PRYOR RD. IT IS 2.15 FT EAST OF THE BACK OF CURB, 19 FT WEST OF THE TRAFFIC SIGNAL POLE, AND 9.6 FT WEST OF A VAULT.
13	977064.448	2812265.992	1012.10	3" IB/CAP SET ON THE NORTHEAST CORNER OF THE INTERSECTION OF SW PRYOR RD AND SW NAPA VALLEY DR. IT IS 6 FT EAST OF THE BACK OF CURB, 10.2 FT NORTH OF THE CORNER OF THE SIDEWALK, AND 7.25 FT WEST OF ROCK LANDSCAPE BORDER.
14	977092.478	2810924.856	1040.93	3" IB/CAP SET IN THE NORTHWEST CORNER OF THE SOUTH ADJOINING PROPERTY. IT IS 7.5 FT EAST OF THE FENCE LINE, AND 45 FT SOUTH OF OF THE TREE LINE.
18	978446.811	2810956.695	1050.47	3" IB/CAP SET IN THE CENTER MEDIAN OF ROUTE 150. THE POINT IS DIRECTLY SOUTH OF A WHITE MAILBOX ON AN ORANGE POST FOR HOUSE ADDRESS 2144 ROUTE 150. IT IS 6.8 FT SOUTH OF THE BACK OF CURB, 5.8 FT NORTH OF THE BACK OF CURB, AND 31 FT EAST OF A LIGHT POLE.
BENCHMARK				
BMK1	977045.350	2812211.924	1014.83	CHISELED PLUS ON THE EAST FLANGED BOLT OF THE FIRE HYDRANT ON THE WEST SIDE OF SW PRYOR ROAD ON ADJOINING PROPERTY SOUTH OF THE SOUTHWEST CORNER OF SUBJECT PROPERTY.
BMK2	978357.416	2811627.737	1031.31	RAILROAD SPIKE IN THE NORTH FACE OF POWER POLE LOCATED ON THE SOUTH SIDE MISSOURI STATE HIGHWAY 150 AT THE WEST SIDE OF THE DRIVEWAY TO 2025 MISSOURI STATE HIGHWAY 150, LEE'S SUMMIT, MO.



REV. NO.	DATE	REVISIONS DESCRIPTION

GENERAL NOTES STREET & STORM SEWER PLANS	OSAGE SECOND PLAT	2020
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drawn by: D.F./M.O./A.A.
checked by: C.G.W.
designed by: A.A.
QA/QC by: J.E.S.
project no.: C19-2339
drawing no.:
date: 2020.08.21



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checked by: C.G.W.
designed by: A.A.
QA/QC by: J.E.S.
project no.: C19-2339
drawing no.:
date: 2020.08.21

SHEET
C104

TYPICAL SECTIONS STREET & STORM SEWER PLANS	REV. NO.	DATE	REVISIONS DESCRIPTION
OSAGE SECOND PLAT			
	REVISIONS		
			2020

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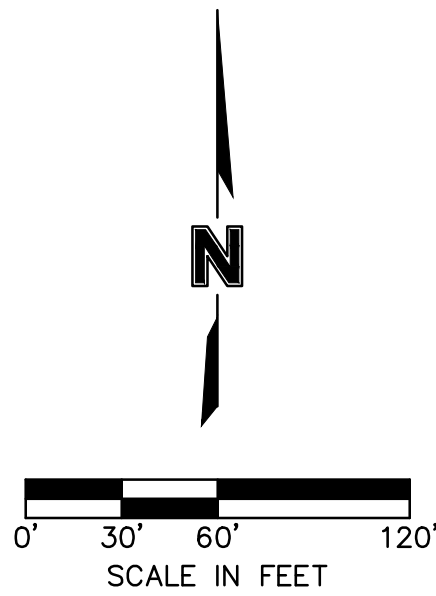
Olsson - Civil Engineering
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STATE OF MISSOURI
JULIE ELAINE SELLERS
Professional Engineer
NUMBER PE-2017000367
10/1/20

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GENERAL NOTES:

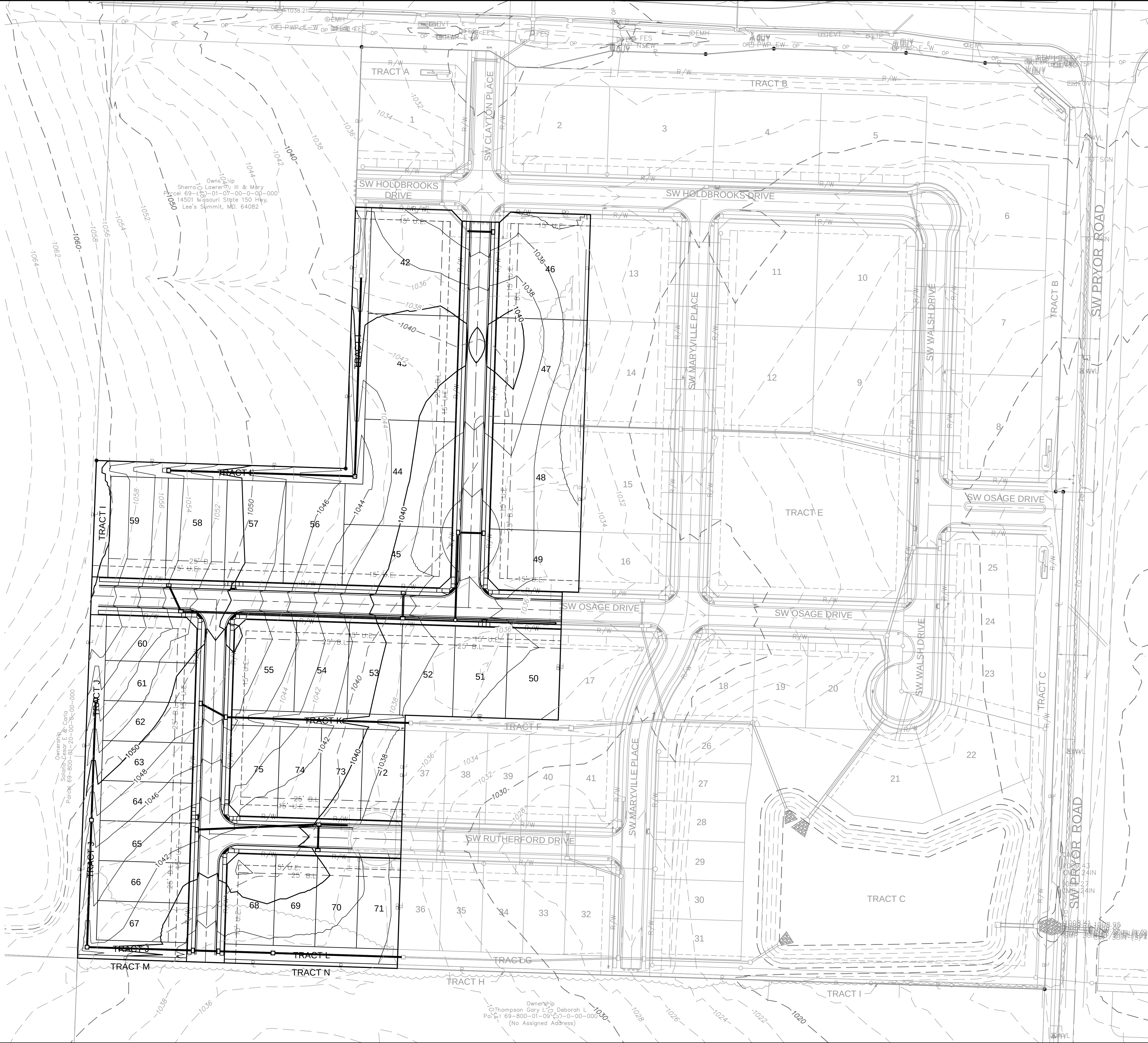
1. CONTRACTOR SHALL ADHERE TO THE "DESIGN AND CONSTRUCTION MANUAL" SECTION 2100 AS ADOPTED BY THE CITY OF LEE'S SUMMIT (LATEST EDITION), FOR EXCAVATION AND EMBANKMENT WORK WITHIN THE PROPOSED RIGHT-OF-WAY.
2. AREAS OF CONSTRUCTION SHALL BE STRIPPED OF ALL VEGETATION, ORGANIC MATTER AND TOPSOIL TO A DEPTH AS RECOMMENDED BY GEOTECHNICAL ENGINEER AND OR TESTING AGENCY. SOILS REMOVED DURING SITE STRIPPING SHOULD BE EVALUATED TO DETERMINE IF PORTIONS OF THE TOPSOIL STRATUM MAY BE UTILIZED AS STRUCTURAL FILL WITHIN PAVEMENT AREAS. ANY MATERIAL NOT DEEMED AS SUITABLE FILL MATERIAL BY THE GEOTECHNICAL ENGINEER AND OR TESTING AGENCY SHALL BE REMOVED FROM THE JOB SITE BY THE CONTRACTOR AT HIS EXPENSE.
3. ALL EMBANKMENT OUTSIDE OF RIGHT-OF-WAY SHOULD BE PLACED IN CONTROLLED LIFTS HAVING A MAXIMUM LOOSE LIFT THICKNESS OF 8". EMBANKMENT SHOULD BE COMPACTED TO A MINIMUM OF 95% OF THE MATERIALS MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-698 (STANDARD PROCTOR COMPACTION). MOISTURE CONTENT OF THE FILL AT THE TIME OF COMPACTION SHALL BE WITHIN A RANGE OF -0 TO +4 PERCENT OF OPTIMUM MOISTURE CONTENT.



LEGEND	
-100-	EXISTING INDEX CONTOURS
-100	EXISTING INTERMEDIATE CONTOURS
-100	PROPOSED INDEX CONTOURS
-100	PROPOSED INTERMEDIATE CONTOURS

EARTHWORK QUANTITIES		
LOCATION	CUT (C.Y.)	FILL (C.Y.)
STREET	2,438	827
SITE	7,812	5,520
TOTAL	10,250	6,347

- EARTHWORK QUANTITIES NOTES:
1. EARTHWORK QUANTITIES BASED ON FINISHED GRADE SURFACE AND DO NOT INCLUDE ADJUSTMENTS FOR TOPSOIL AND SHRINKAGE.
 2. EARTHWORK QUANTITIES DO NOT TAKE INTO CONSIDERATION EXCAVATION, REMOVAL AND DISPOSAL OF MATERIAL DEEMED UNSUITABLE BY A GEOTECHNICAL ENGINEER. THE EARTHWORK CONTRACTOR IS RESPONSIBLE FOR EXCAVATION, REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL AND FOR REPLACING IT WITH SUITABLE MATERIAL.



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STATE OF MISSOURI
JULIE ELAINE SELLERS
Professional Engineer
NUMBER PE-2017000367
10/1/20

REV. NO.	DATE	REVISIONS DESCRIPTION

GRADING PLAN (FOR REFERENCE ONLY)
STREET & STORM SEWER PLANS

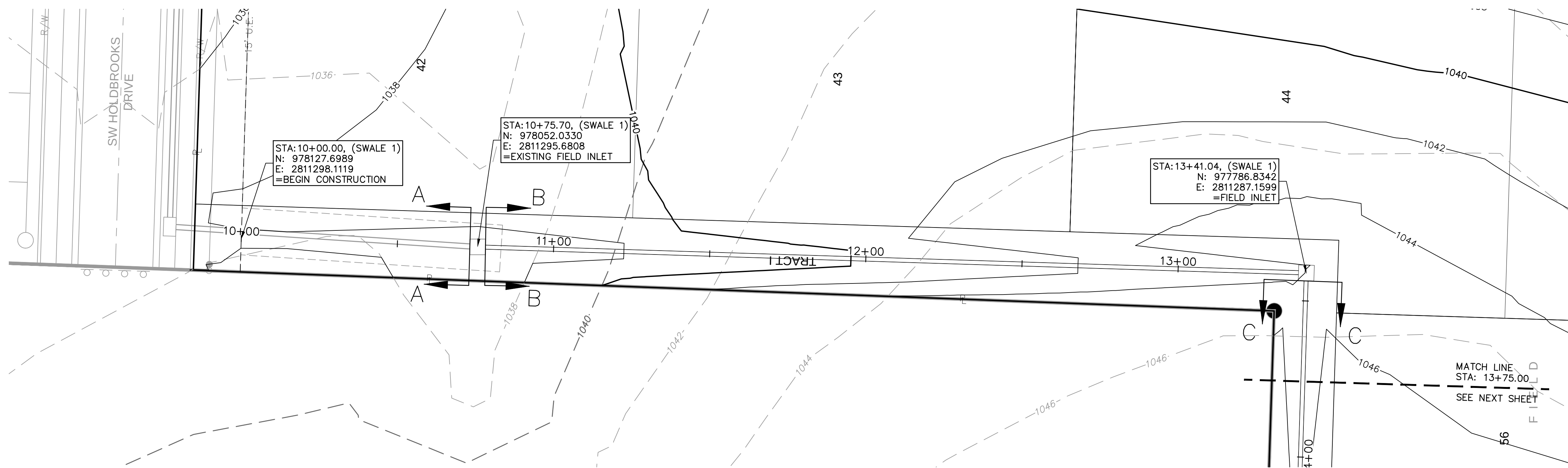
OSAGE
SECOND PLAT

2020

drawn by: D.F./M.O./A.A.
checked by: C.G.W.
designed by: A.A.
QA/QC by: J.E.S.
project no.: C19-2339
drawing no.:
date: 2020.08.21

SHEET
C105

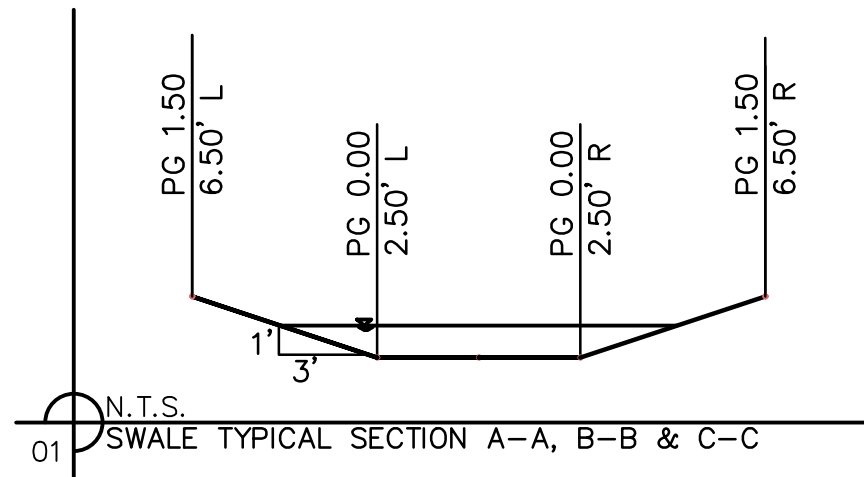
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SWALE GRADING NOTES:

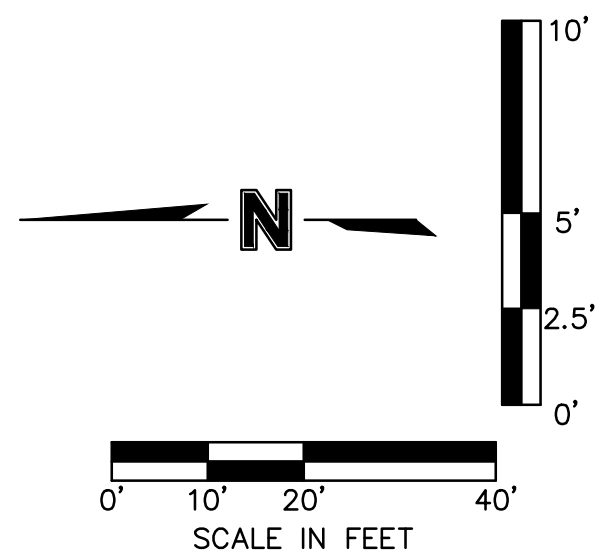
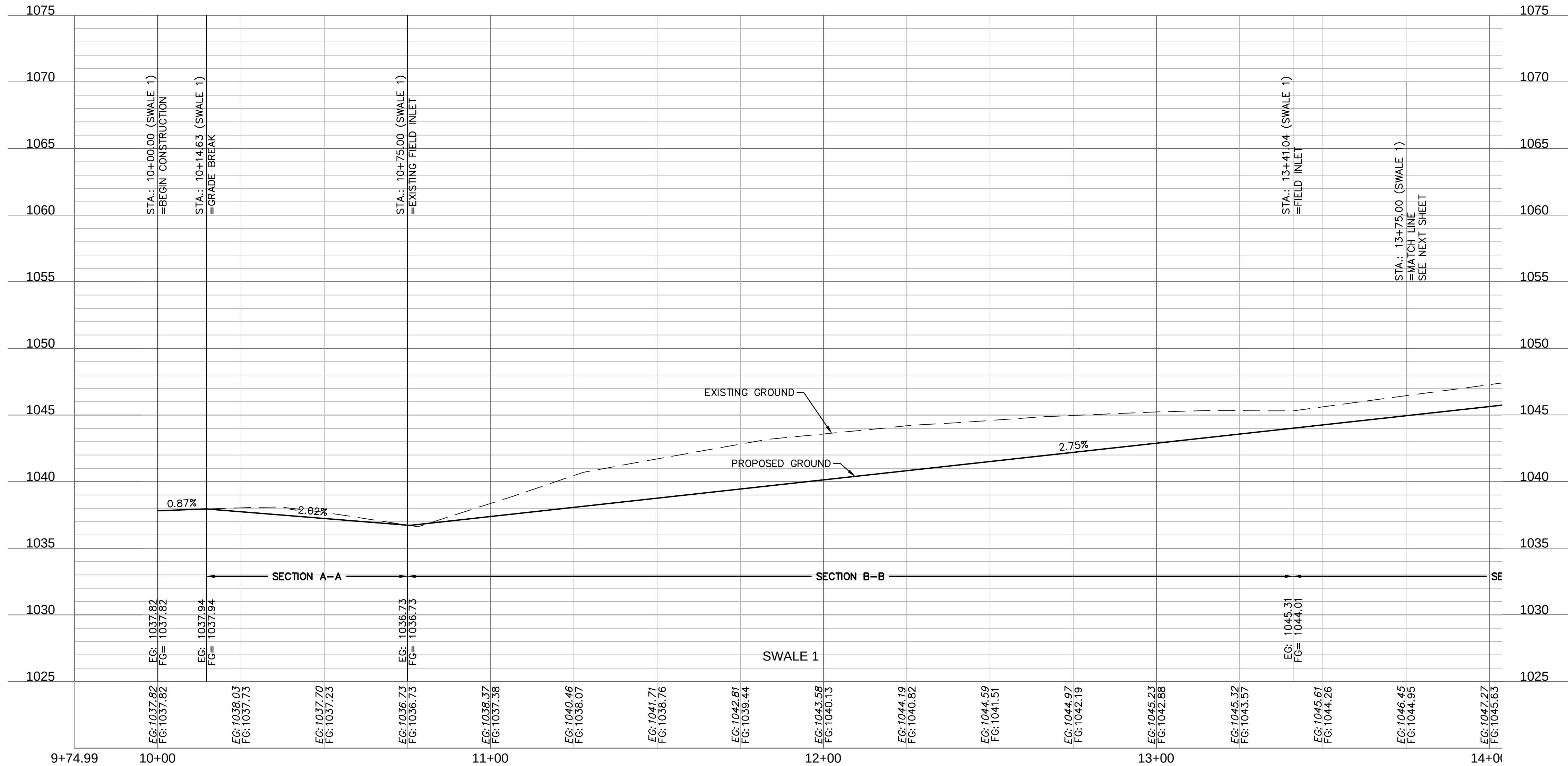
1. CONTRACTOR SHALL CONSTRUCT SWALES WITH MINIMUM SLOPE, WIDTH AND DEPTH AS SHOWN IN THE SWALE DESIGN TABLES.

2. AS-BUILT SURVEY IS REQUIRED/APPROVED BY CITY FOR ALL SWALES AND PRIOR TO APPROVAL FOR ANY BUILDING FOUNDATION PERMIT. CONTRACTOR SHALL BE REQUIRED TO REGRADE SWALES AT CONTRACTOR'S EXPENSE IF ABOVE REQUIREMENTS ARE NOT MET.



SWALE DESIGN TABLE (100-YEAR RETURN FREQUENCY)														
SECTION	Mannings Coefficient	SECTION DATA					RESULTS							
		Channel Slope (%)	Swale Depth (ft)	Left Side Slope (H:V)	Right Side Slope (H:V)	Bottom Width (ft)	Discharge (cfs)	Water Depth (ft)	Flow Area (ft ²)	Velocity (ft/sec)	Wetted Perimeter (ft)	Top Width (ft)	Specific Energy (ft)	Shear Stress (lbs/ft ²)
A-A	0.03	2.02%	1.5	3:1	3:1	5	7.76	0.39	2.41	3.22	7.47	7.34	0.39	0.41
B-B	0.03	2.75%	1.5	3:1	3:1	5	22.11	0.64	4.43	4.99	9.05	8.84	0.73	0.84
C-C	0.03	2.75%	2.5	3:1	3:1	6	6.05	0.31	1.84	3.29	6.96	6.86	0.34	0.45

Section	Drainage Area	C	Tc	i	K	Peak Flow
	(ac.)		(min)	(in/hr)		(cfs)
A-A	1.18	0.51	5	10.32	1.25	7.76
B-B	3.36	0.51	5	10.32	1.25	22.11
C-C	0.92	0.51	9.16	10.32	1.25	6.05



SWALE 1 PLAN & PROFILE

OSAGE
SECOND PLAT

REVISIONS

2020

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date: 2020.08.21

SHEET
C106

STATE OF MISSOURI
JULIE ELAINE
SELLERS
NUMBER
PE-2017000367
10/1/20
PROFESSIONAL ENGINEER

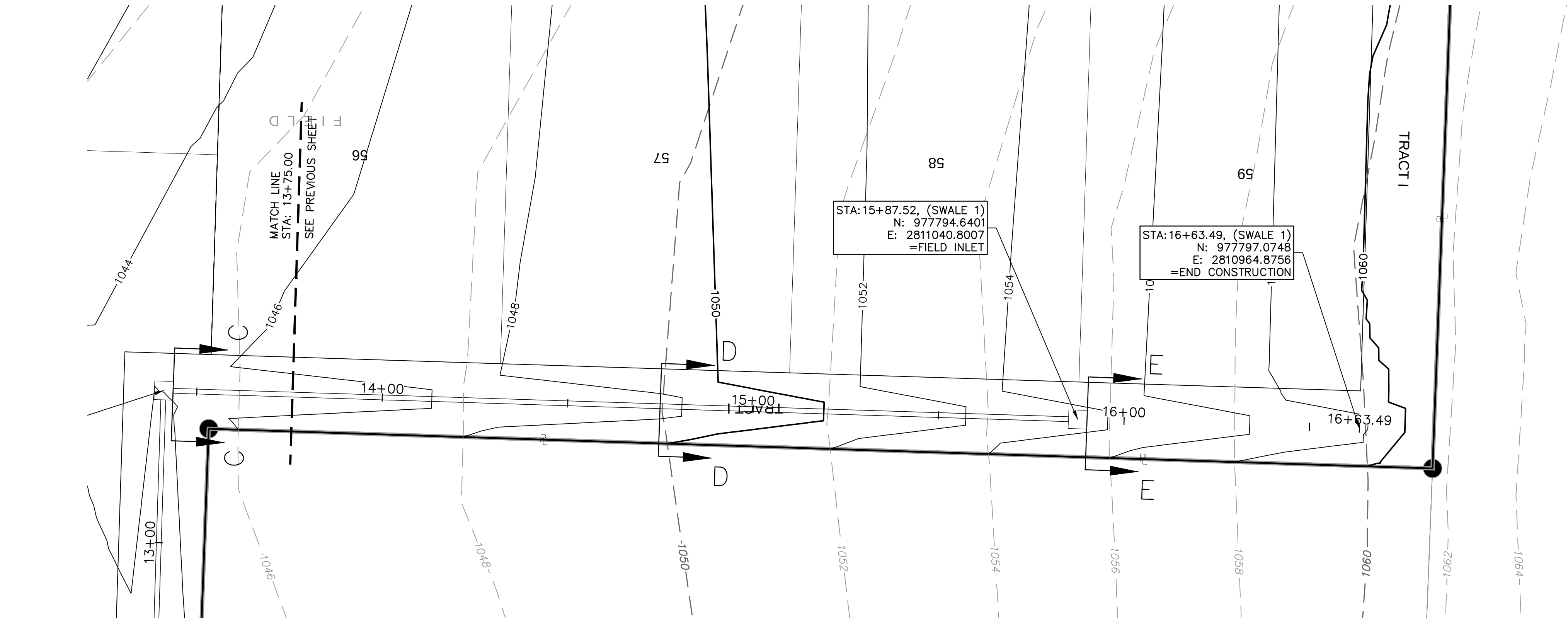
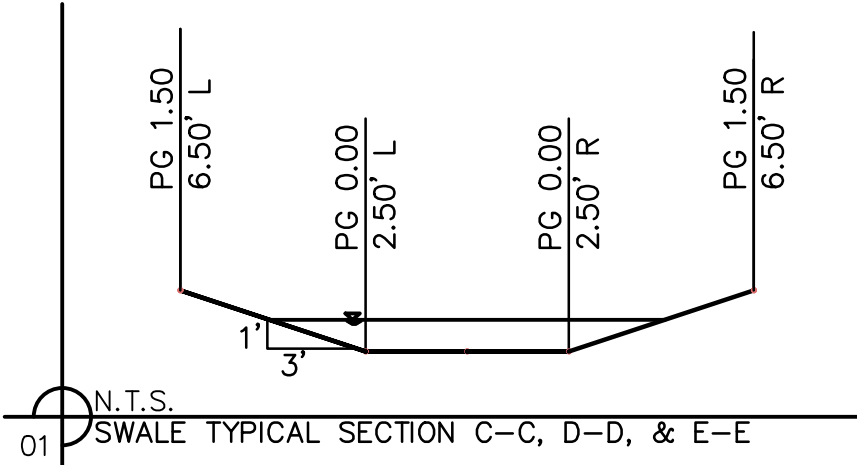
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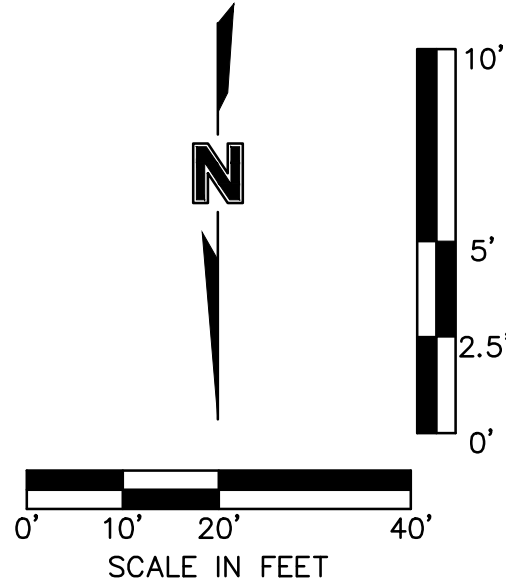
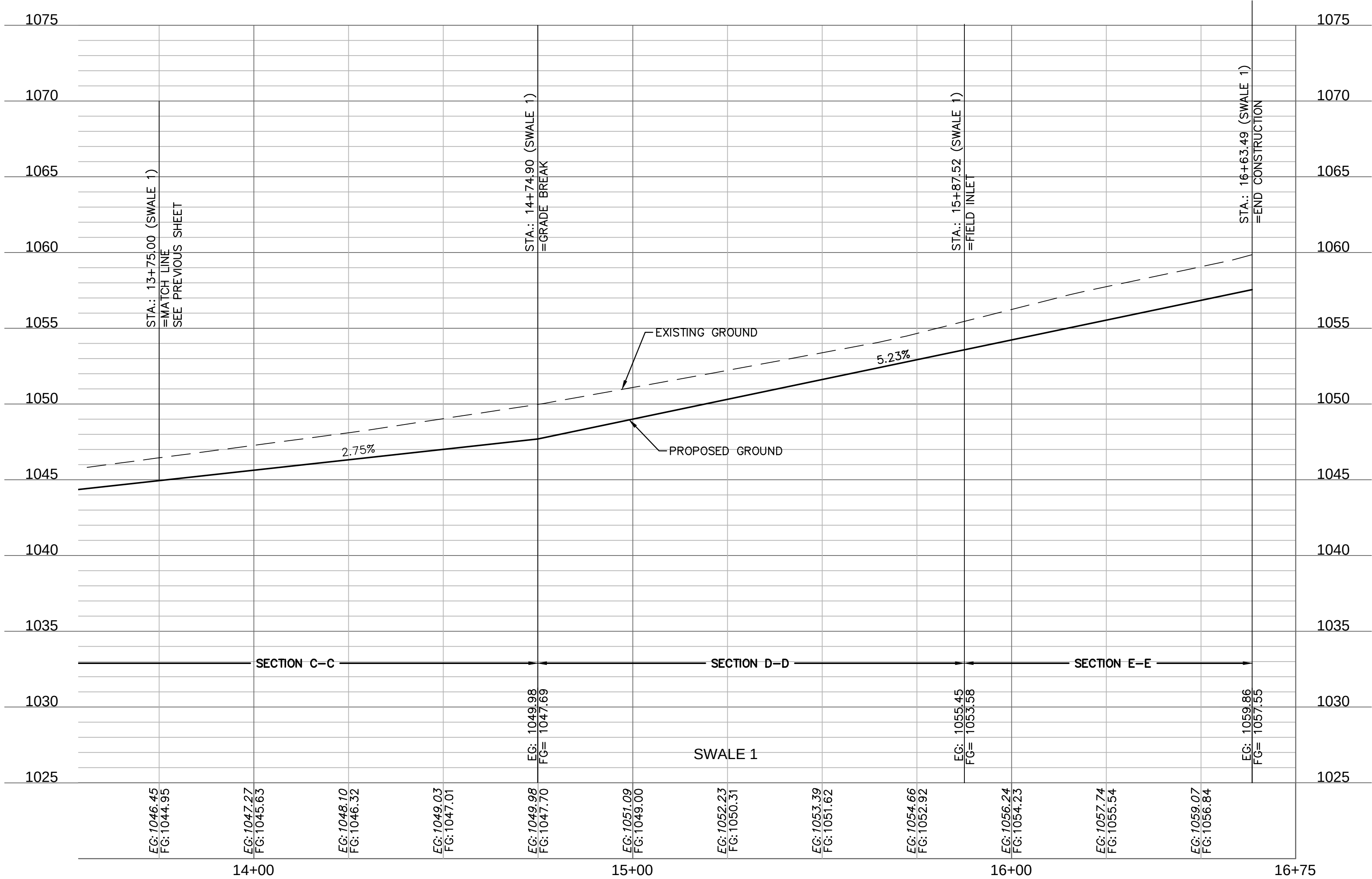
SWALE GRADING NOTES:

1. CONTRACTOR SHALL CONSTRUCT SWALES WITH MINIMUM SLOPE, WIDTH AND DEPTH AS SHOWN IN THE SWALE DESIGN TABLES.
2. AS-BUILT SURVEY IS REQUIRED/APPROVED BY CITY FOR ALL SWALES AND PRIOR TO APPROVAL FOR ANY BUILDING FOUNDATION PERMIT. CONTRACTOR SHALL BE REQUIRED TO REGRADE SWALES AT CONTRACTOR'S EXPENSE IF ABOVE REQUIREMENTS ARE NOT MET.



SWALE DESIGN TABLE (100-YEAR RETURN FREQUENCY)													
SECTION	Mannings Coefficient	SECTION DATA				RESULTS							
		Channel Slope (%)	Swale Depth (ft)	Left Side Slope (H:V)	Right Side Slope (H:V)	Bottom Width (ft)	Discharge (cfs)	Water Depth (ft)	Flow Area (ft²)	Velocity (ft/sec)	Wetted Perimeter (ft)	Top Width (ft)	Specific Energy (ft)
C-C	0.03	2.75%	1.5	3:1	3:1	5	6.05	0.31	1.84	3.29	6.96	0.34	0.45
D-D	0.03	5.23%	1.5	3:1	3:1	5	1.64	0.12	0.64	2.55	5.76	5.72	0.15
E-E	0.03	5.23%	2.5	3:1	3:1	6	1.71	0.13	0.70	2.44	5.82	5.78	0.15

Swale Drainage Area Table (100 year Return Frequency)					
Section	Drainage Area (ac.)	C	Tc (min)	i (in/hr)	K
C-C	0.92	0.51	9.16	10.32	1.25
D-D	0.25	0.51	9.16	10.32	1.25
E-E	0.26	0.51	5.00	10.32	1.25



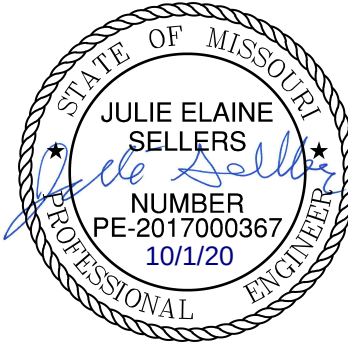
SWALE 1 PLAN & PROFILE (CONT'D)
STREET & STORM SEWER PLANS

OSAGE
SECOND PLAT

2020

drawn by: D.F./M.O./A.A.
checked by: C.G.W.
designed by: A.A.
QA/QC by: J.E.S.
project no.: C19-2339
drawing no.:
date: 2020.08.21

SHEET
C107



REVISIONS DESCRIPTION

REV. NO.

DATE

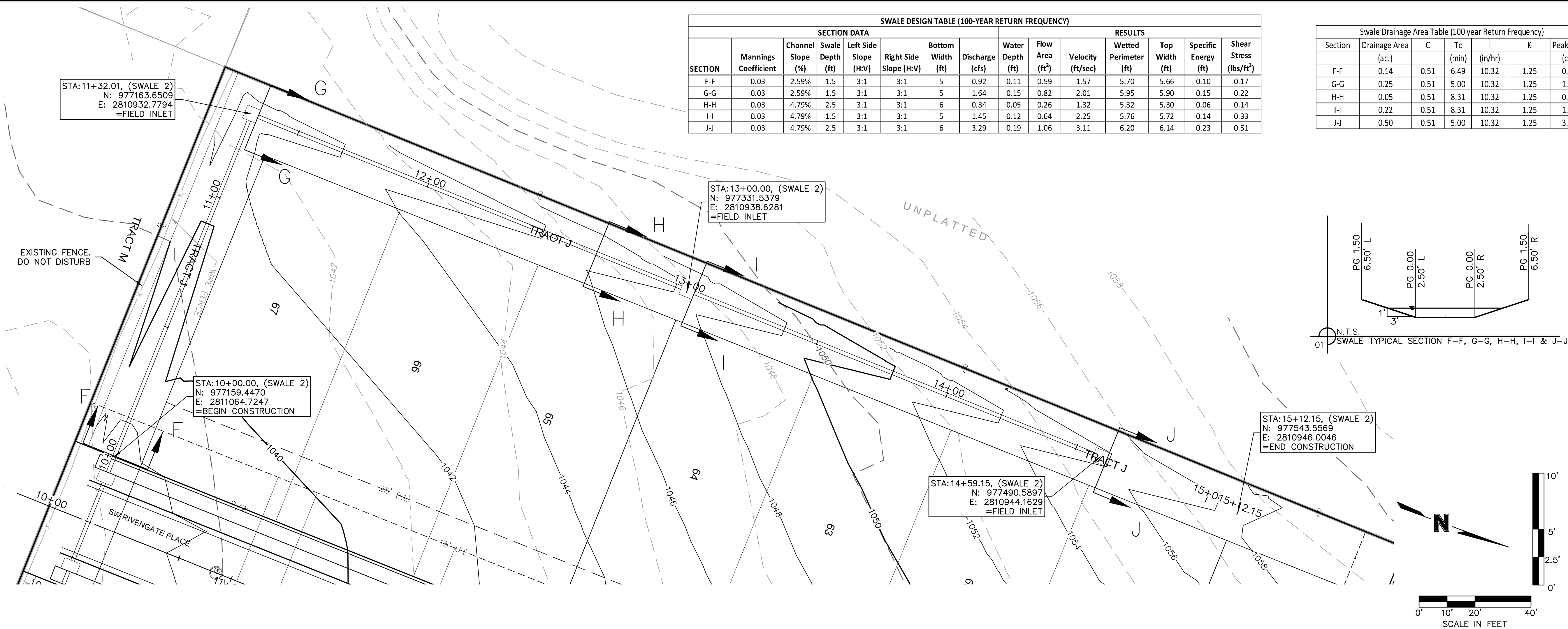
REVISIONS

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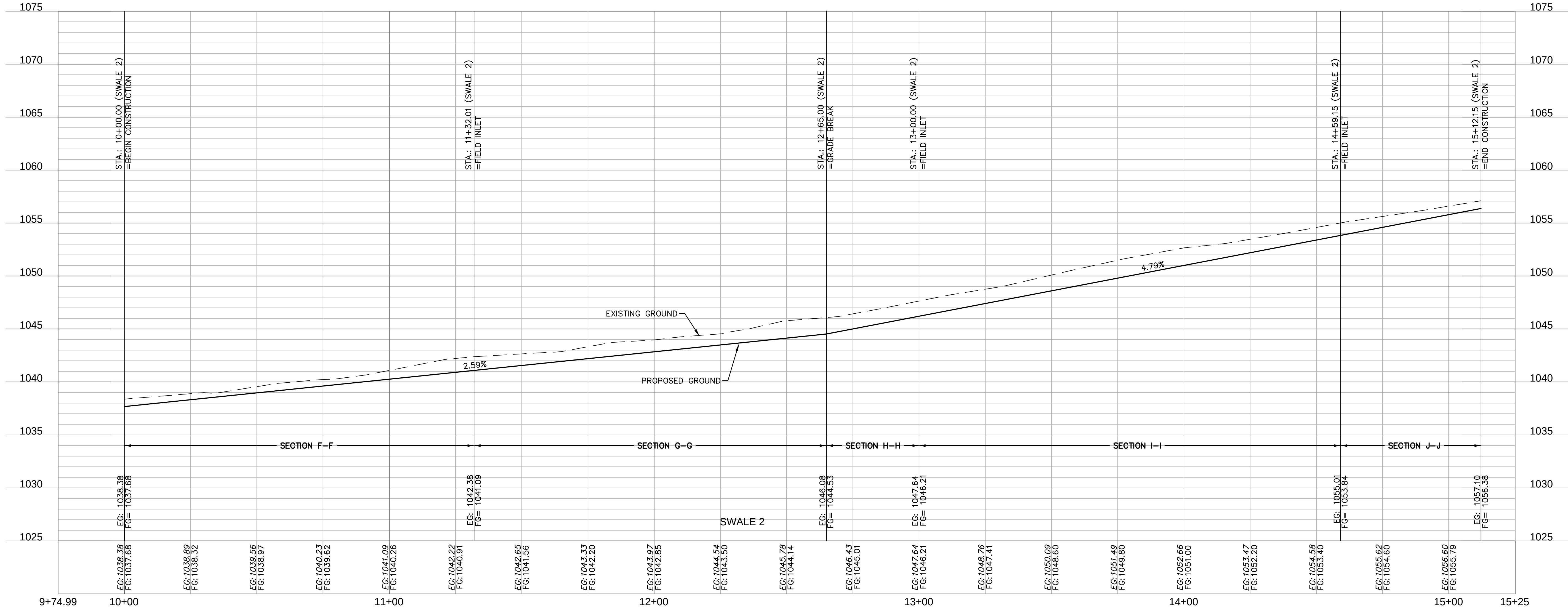
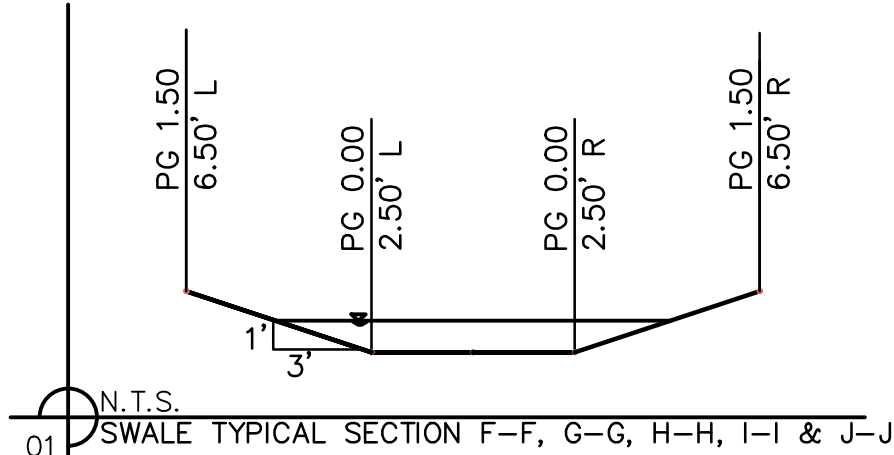
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USER: cbbdial@ev
C:\PBLK_C192339 C:\PSSWR_C192339 C:\PWATR_C192339 C:\PSURF_C192339

- SWALE GRADING NOTES:**
1. CONTRACTOR SHALL CONSTRUCT SWALES WITH MINIMUM SLOPE, WIDTH AND DEPTH AS SHOWN IN THE SWALE DESIGN TABLES.
 2. AS-BUILT SURVEY IS REQUIRED/APPROVED BY CITY FOR ALL SWALES AND PRIOR TO APPROVAL FOR ANY BUILDING FOUNDATION PERMIT. CONTRACTOR SHALL BE REQUIRED TO REGRADE SWALES AT CONTRACTOR'S EXPENSE IF ABOVE REQUIREMENTS ARE NOT MET.



SWALE DESIGN TABLE (100-YEAR RETURN FREQUENCY)														
SECTION DATA							RESULTS							
SECTION	Mannings Coefficient	Channel Slope (%)	Swale Depth (ft)	Left Side Slope (H:V)	Right Side Slope (H:V)	Bottom Width (ft)	Discharge (cfs)	Water Depth (ft)	Flow Area (ft²)	Velocity (ft/sec)	Wetted Perimeter (ft)	Top Width (ft)	Specific Energy (ft)	Shear Stress (lbs/ft²)
F-F	0.03	2.59%	1.5	3:1	3:1	5	0.92	0.11	0.59	1.57	5.70	5.66	0.10	0.17
G-G	0.03	2.59%	1.5	3:1	3:1	5	1.64	0.15	0.82	2.01	5.95	5.90	0.15	0.22
H-H	0.03	4.79%	2.5	3:1	3:1	6	0.34	0.05	0.26	1.32	5.32	5.30	0.06	0.14
I-I	0.03	4.79%	1.5	3:1	3:1	5	1.45	0.12	0.64	2.25	5.76	5.72	0.14	0.33
J-J	0.03	4.79%	2.5	3:1	3:1	6	3.29	0.19	1.06	3.11	6.20	6.14	0.23	0.51

Swale Drainage Area Table (100 year Return Frequency)						
Section	Drainage Area (ac.)	C	Tc (min)	i (in/hr)	K	Peak Flow (cfs)
F-F	0.14	0.51	6.49	10.32	1.25	0.92
G-G	0.25	0.51	5.00	10.32	1.25	1.64
H-H	0.05	0.51	8.31	10.32	1.25	0.34
I-I	0.22	0.51	8.31	10.32	1.25	1.45
J-J	0.50	0.51	5.00	10.32	1.25	3.29



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SWALE 2 PLAN & PROFILE
STREET & STORM SEWER PLANS

OSAGE
SECOND PLAT

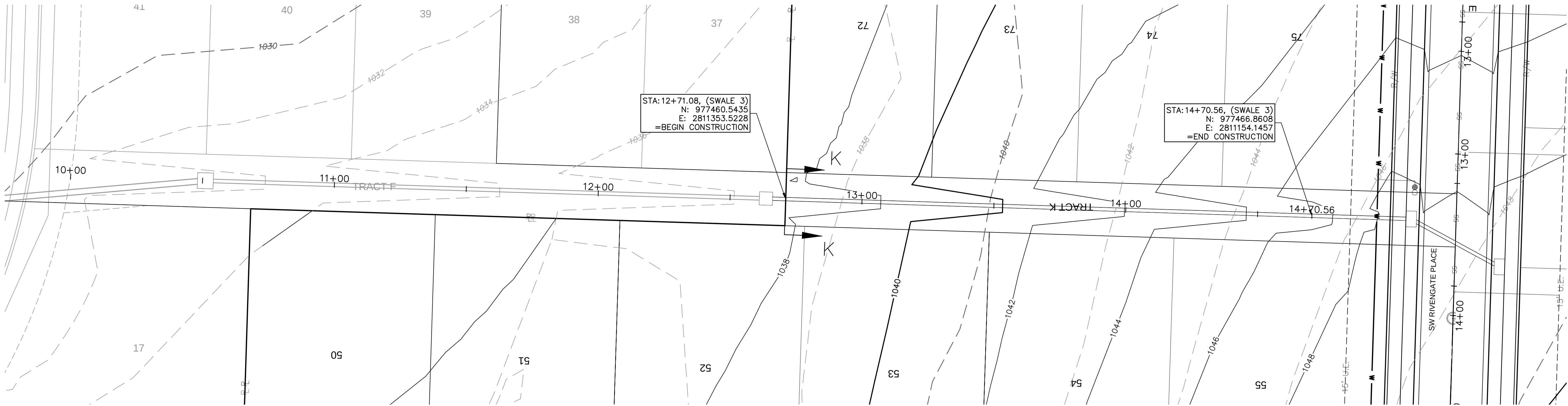
2020

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checked by: C.G.W.
designed by: A.A.
QA/QC by: J.E.S.
project no.: C19-2339
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date: 2020.08.21

SHEET
C108

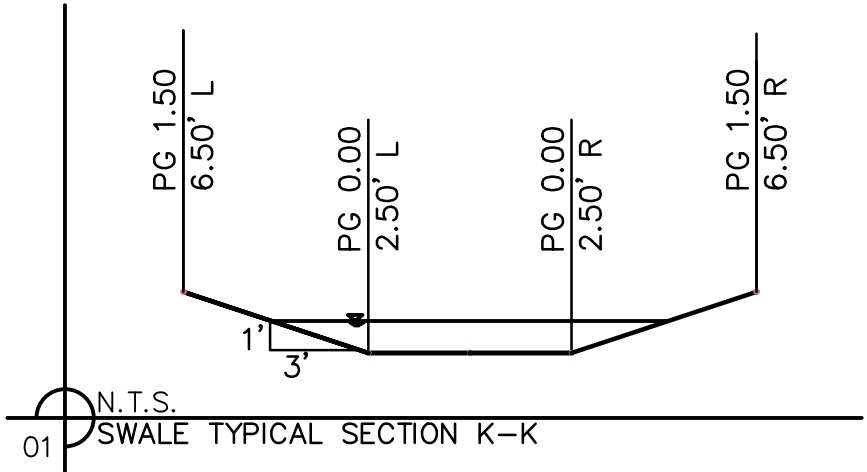
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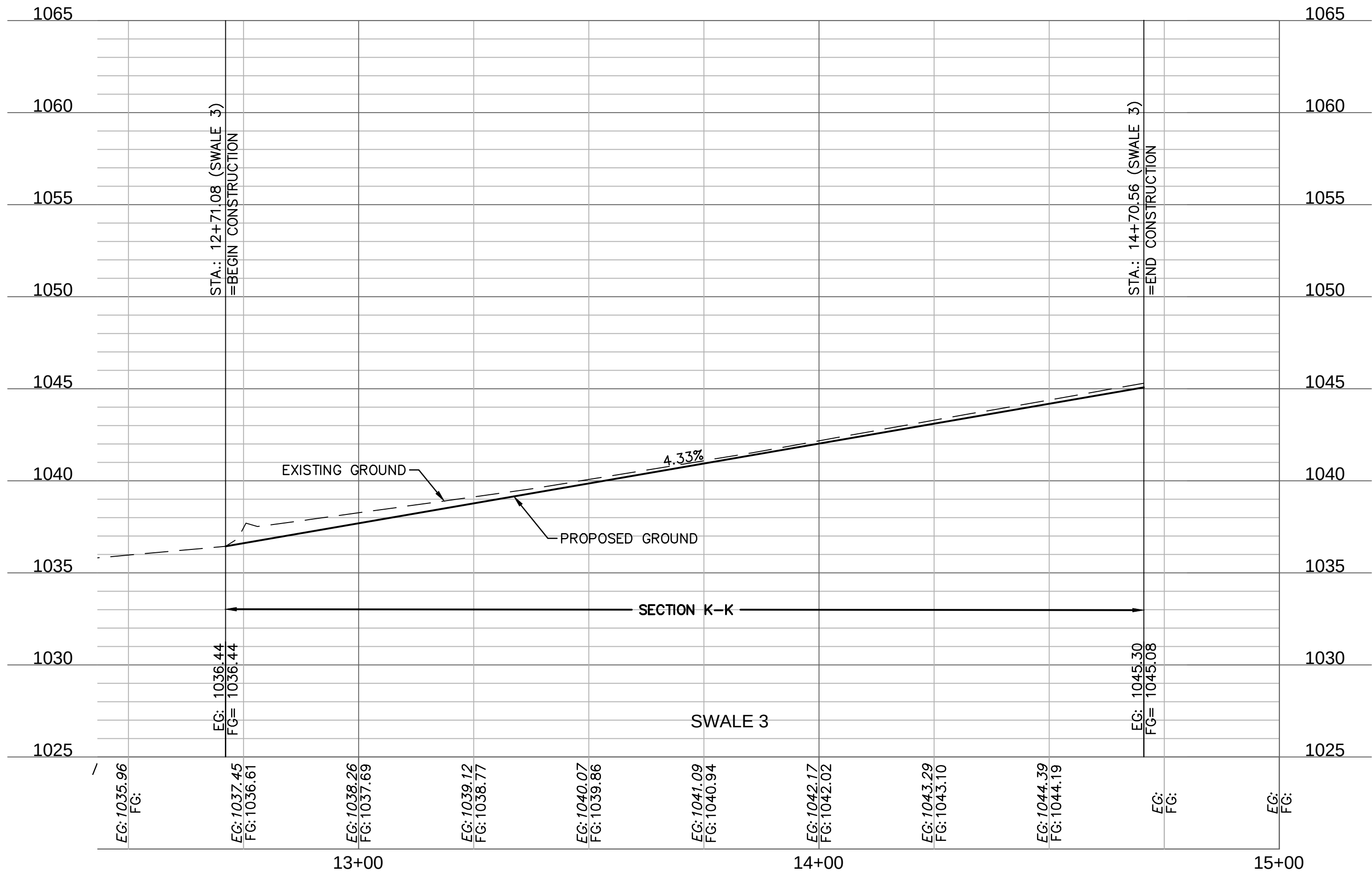
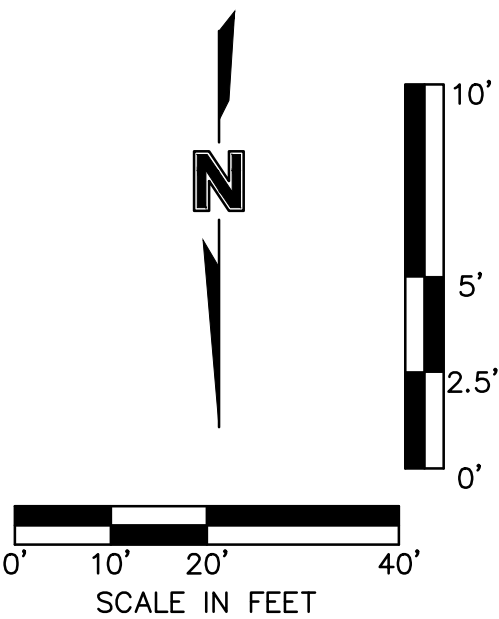
SWALE GRADING NOTES:

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SWALE DESIGN TABLE (100-YEAR RETURN FREQUENCY)													
SECTION DATA							RESULTS						
SECTION	Mannings Coefficient	Channel Slope (%)	Swale Depth (ft)	Left Side Slope (H:V)	Right Side Slope (H:V)	Bottom Width (ft)	Discharge (cfs)	Water Depth (ft)	Flow Area (ft²)	Velocity (ft/sec)	Wetted Perimeter (ft)	Top Width (ft)	Specific Energy (ft)
K-K	0.03	4.33%	1.5	3:1	3:1	5	1.68	0.13	0.70	2.40	5.82	5.78	0.15
													Shear Stress (lbs/ft²)
													0.33

Swale Drainage Area Table (100 year Return Frequency)						
Section	Drainage Area (ac.)	C	Tc (min)	i (in/hr)	K	Peak Flow (cfs)
K-K	0.26	0.51	5	10.32	1.25	1.68



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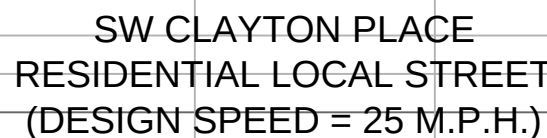
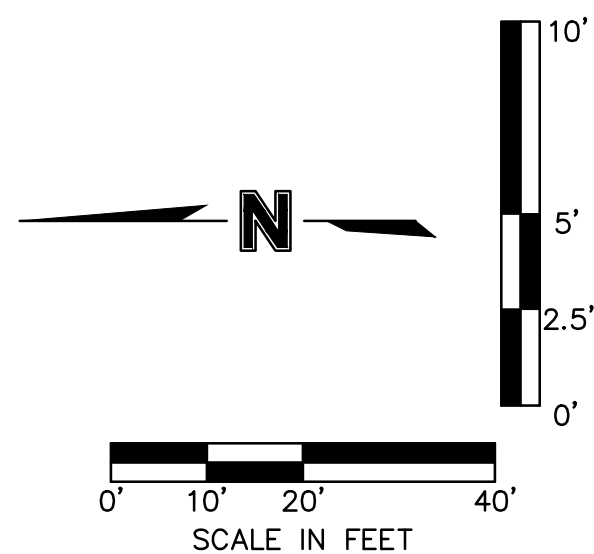
SWALE 3 PLAN & PROFILE
STREET & STORM SEWER PLANS
OSAGE
SECOND PLAT

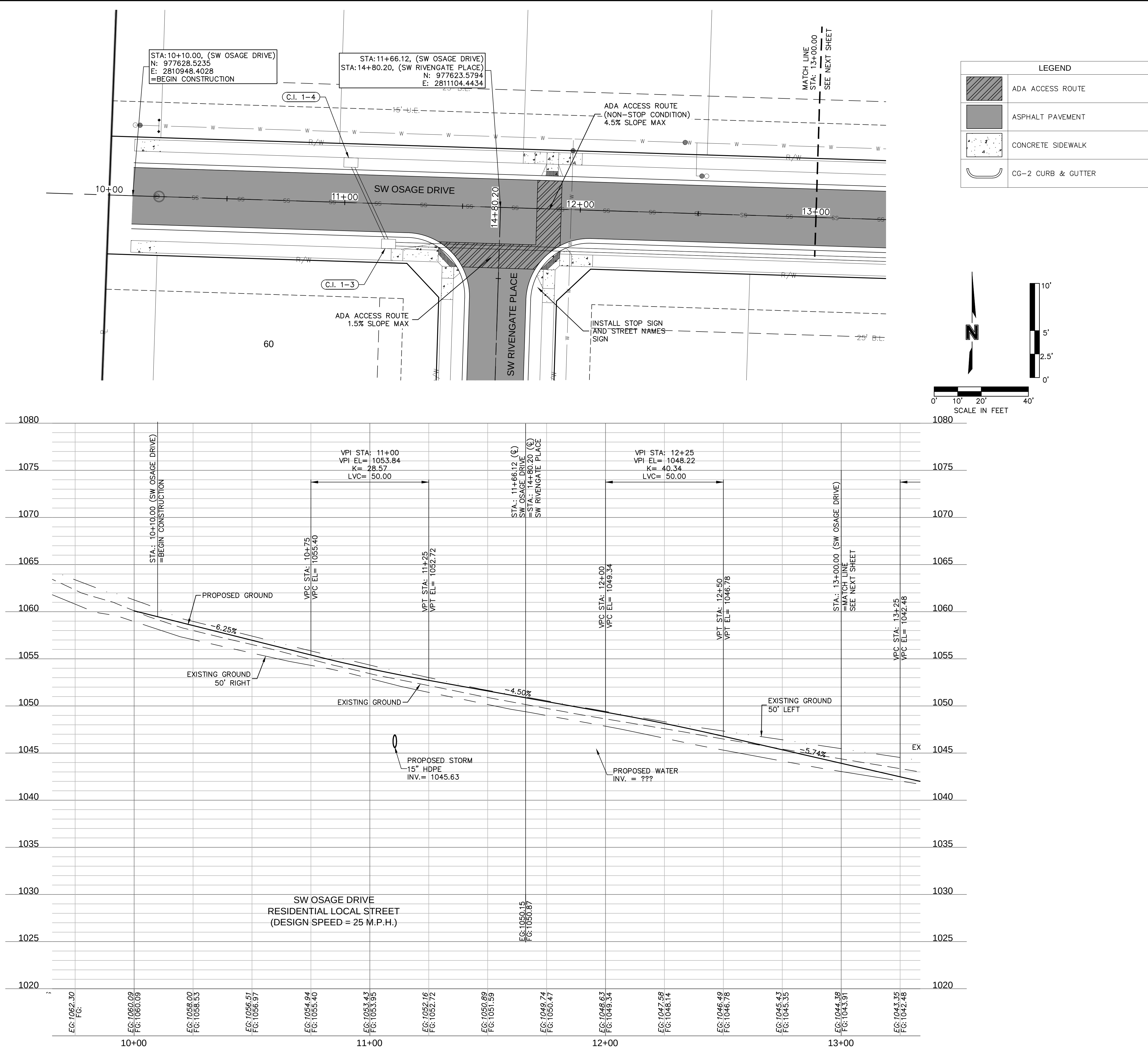
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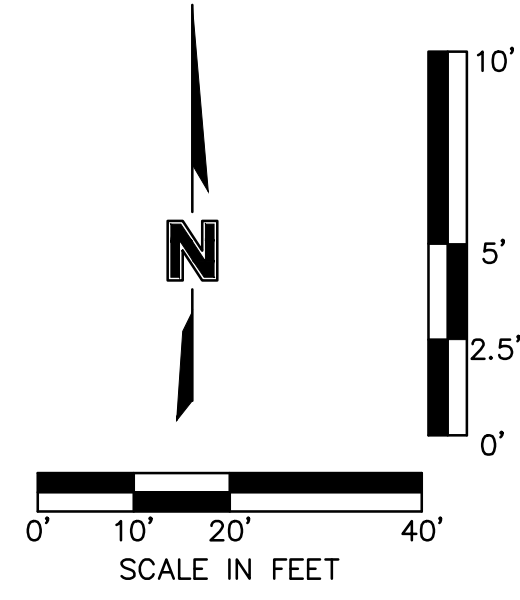
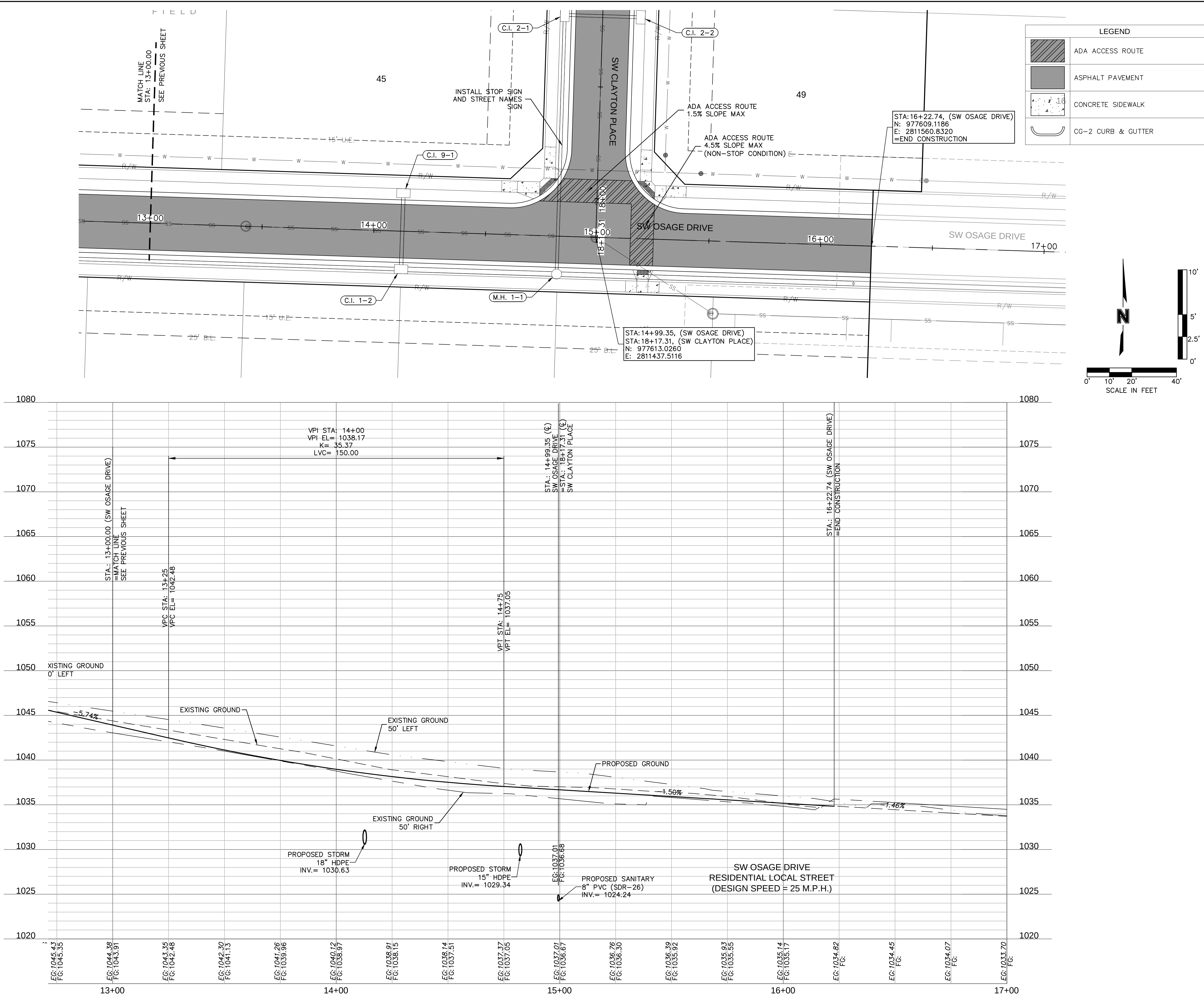
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checked by: C.G.W.
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SHEET
C109

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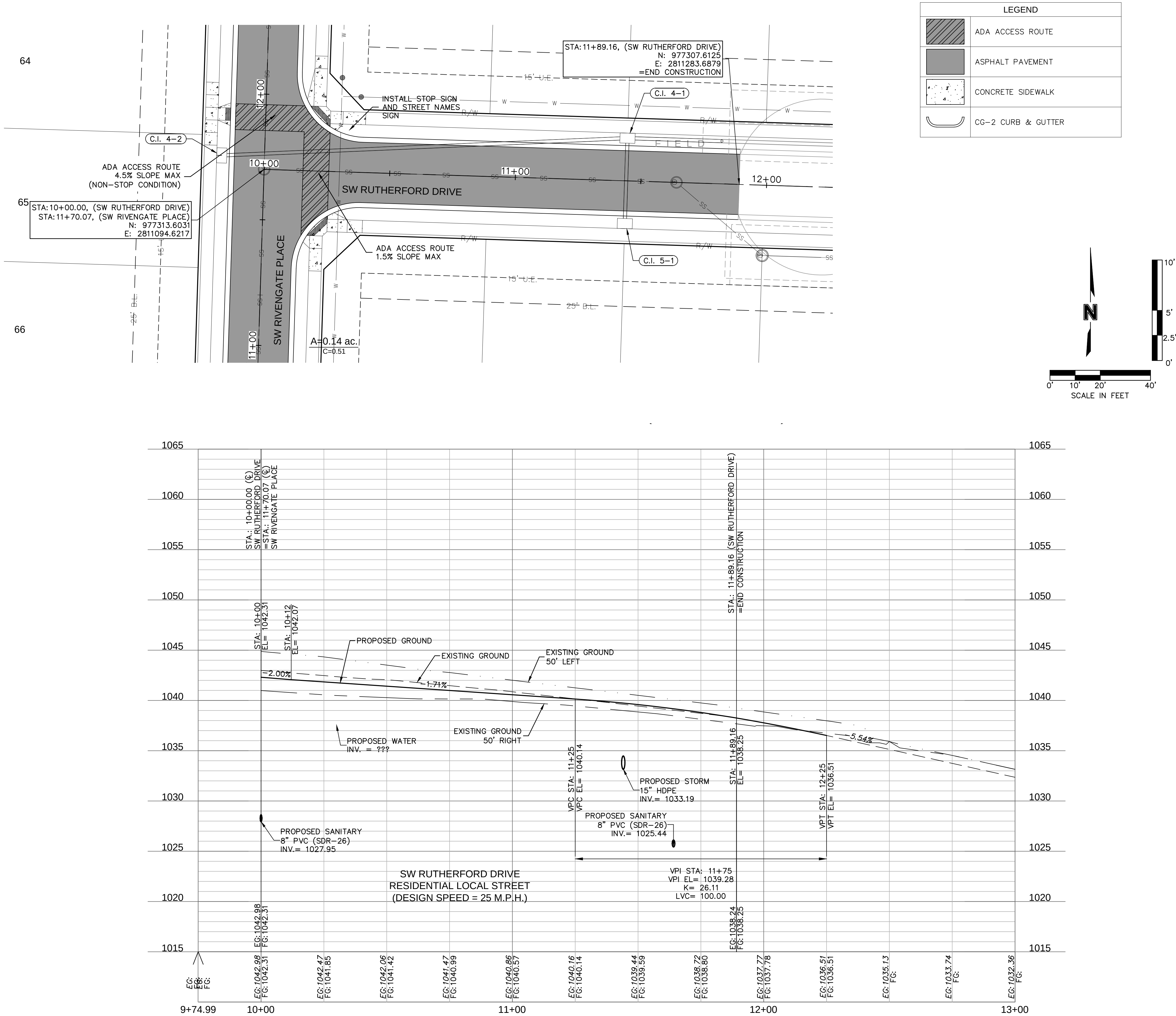
ROADWAY PLAN & PROFILE (SW OSAGE DRIVE CONT'D)
STREET & STORM SEWER PLANS

OSAGE
SECOND PLAT

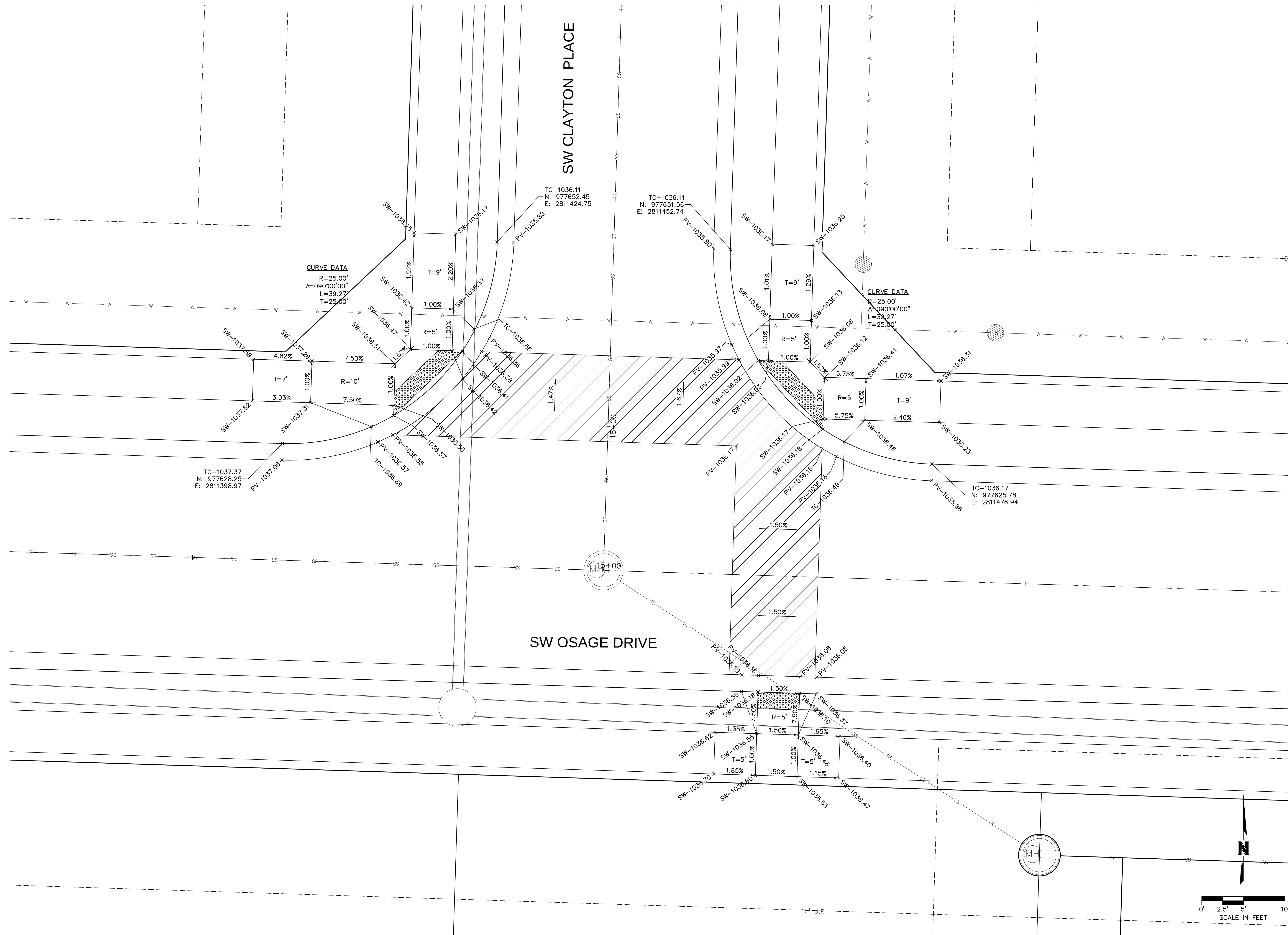
2020

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C112



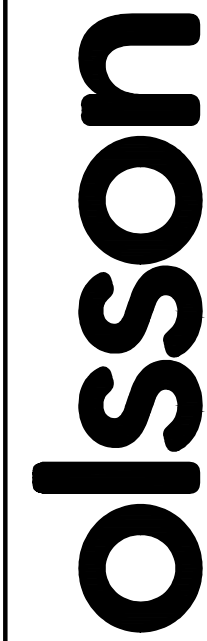
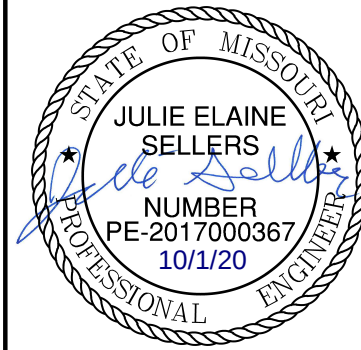
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SW OSAGE DRIVE AND SW CLAYTON PLACE INTERSECTION
STREET & STORM SEWER PLANSOSAGE
SECOND PLAT

2020

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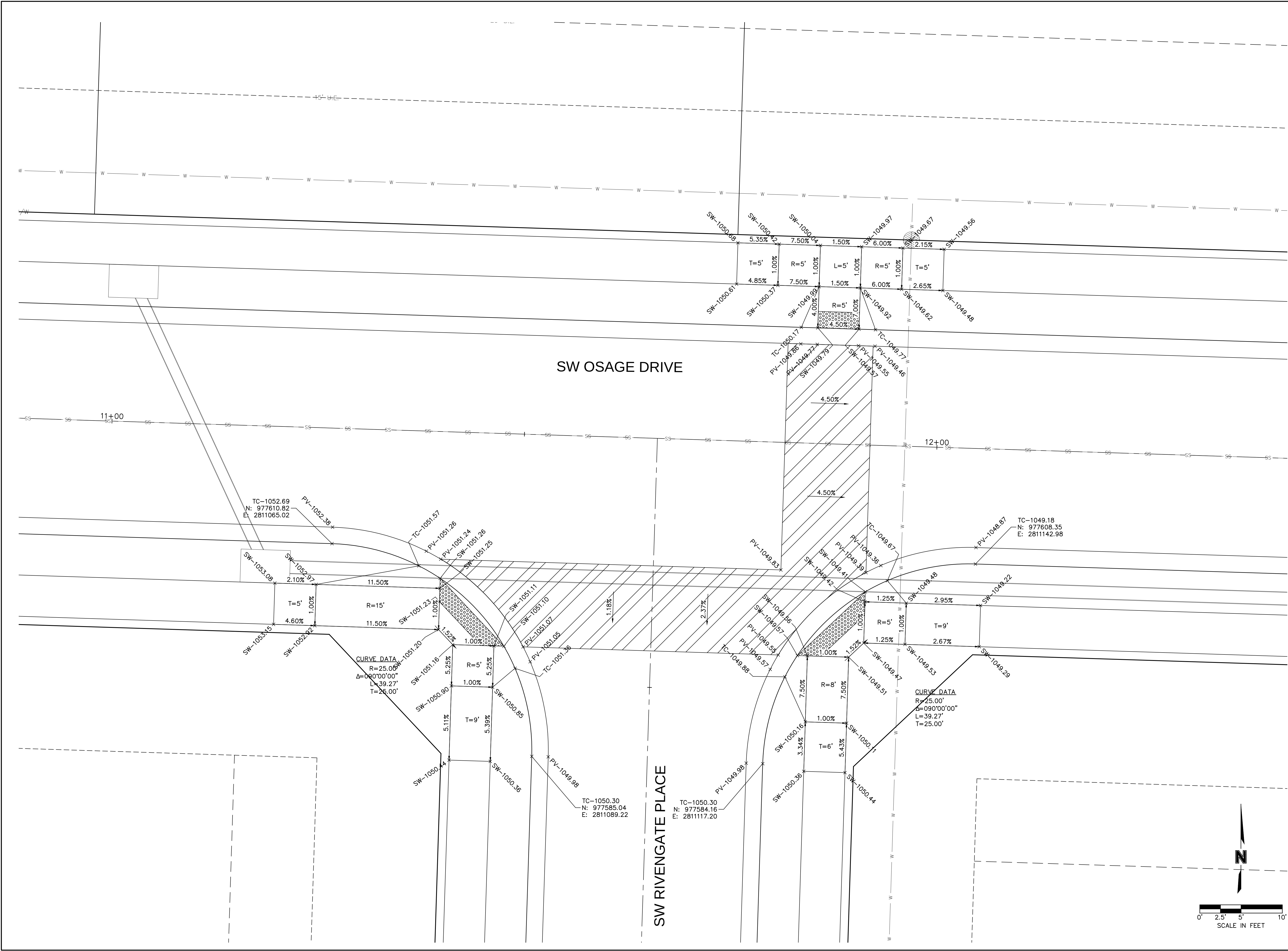


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SHEET
C115

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SW OSAGE DRIVE AND SW RIVENGATE PLACE INTERSECTION
STREET & STORM SEWER PLANS

OSAGE
SECOND PLAT

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designed by: A.A.
QA/QC by: J.E.S.
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drawing no.:
date: 2020.08.21

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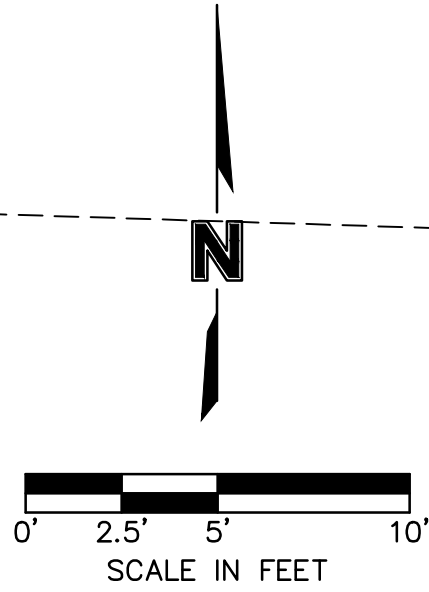
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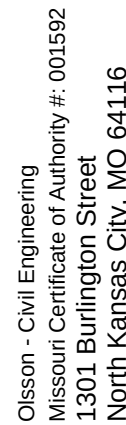
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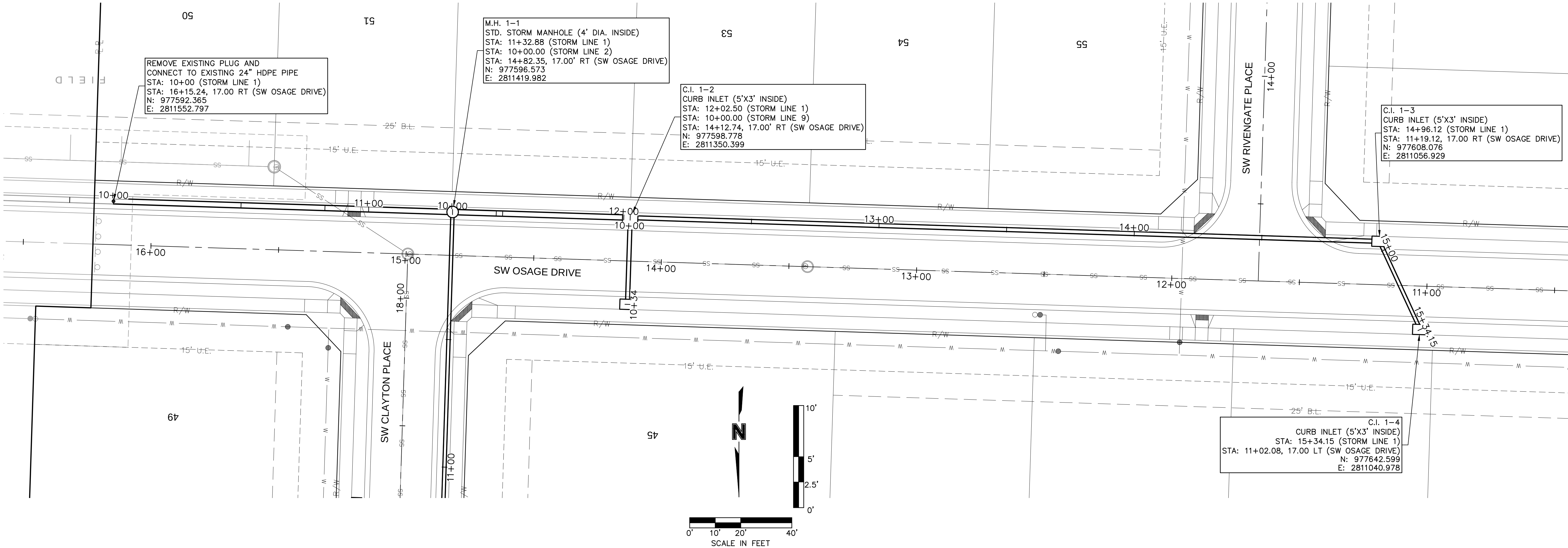
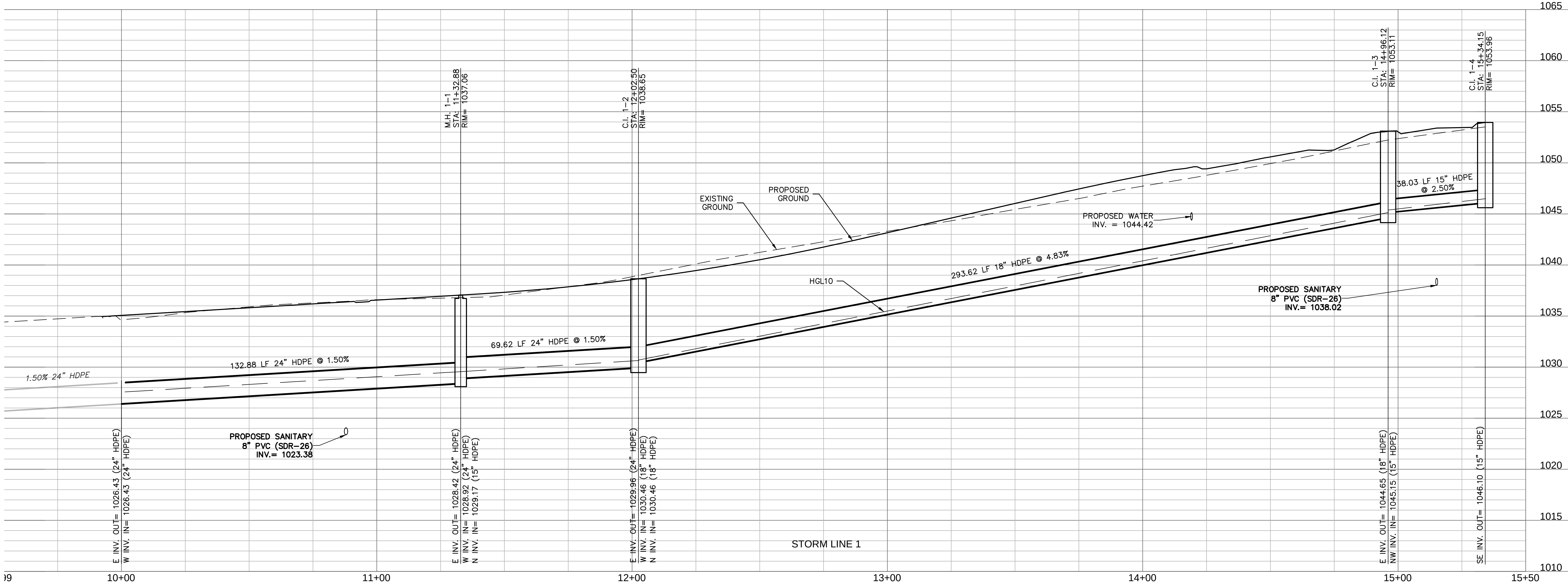
2020

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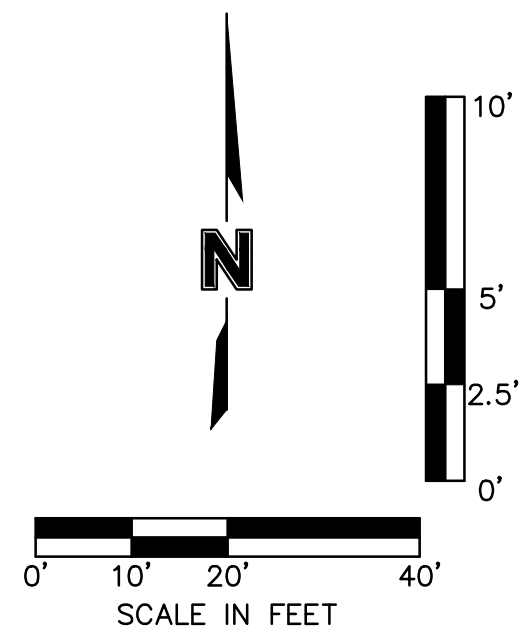
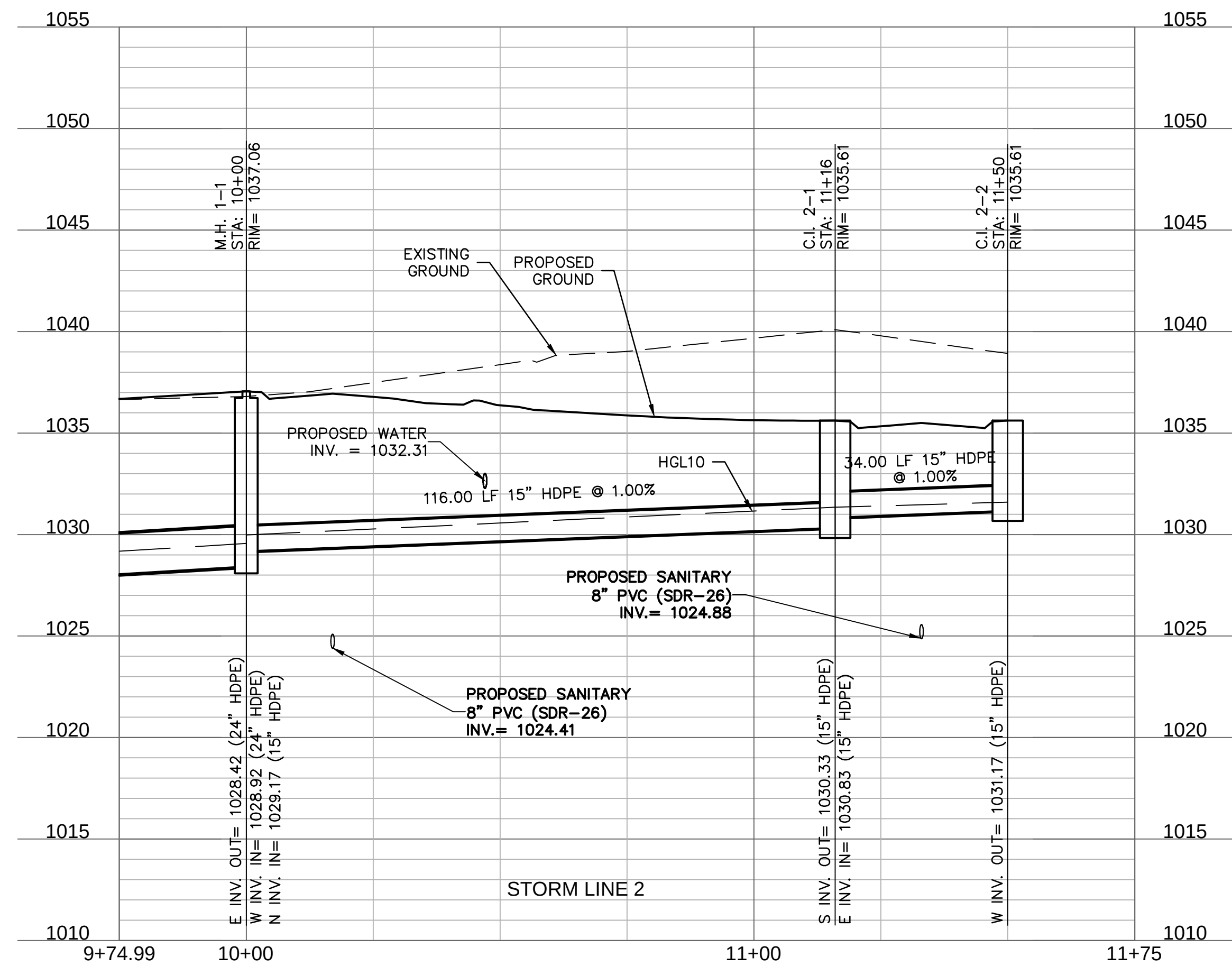
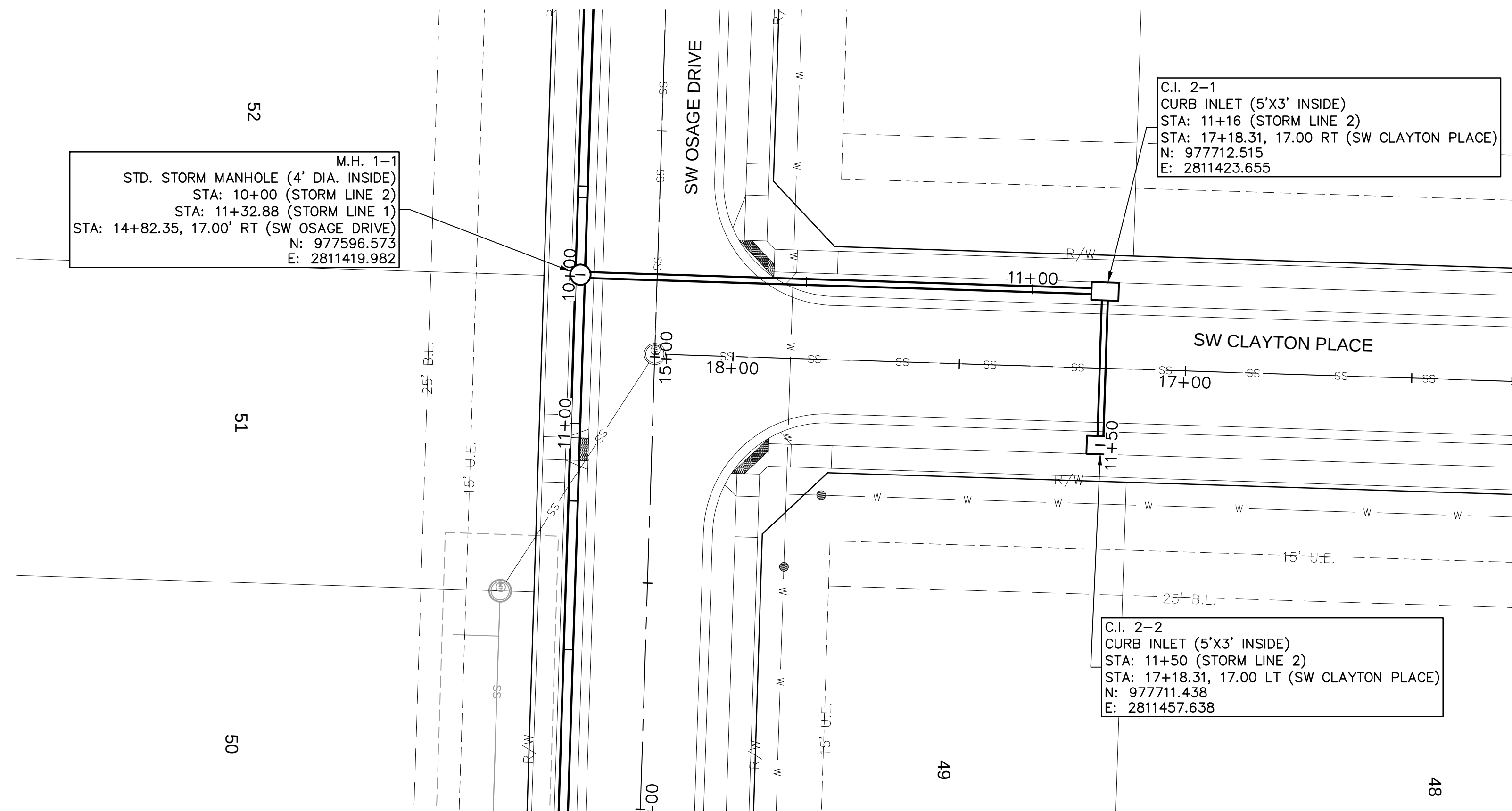
2020

STORM SEWER PLAN & PROFILE (LINE 1)
STREET & STORM SEWER PLANS
OSAGE
SECOND PLAT

SHEET
C118

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checked by: C.G.W.
designed by: A.A.
QA/QC by: J.E.S.
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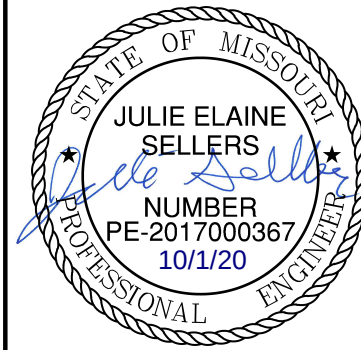


STORM SEWER PLAN & PROFILE (LINE 2)
STREET & STORM SEWER PLANS

OSAGE
SECOND PLAT

2020

REVISIONS

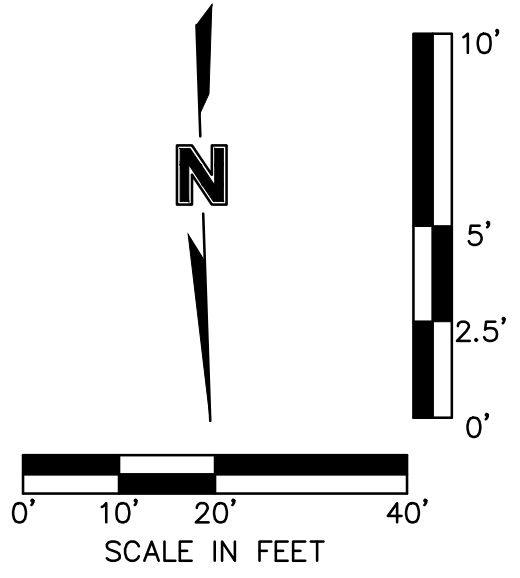
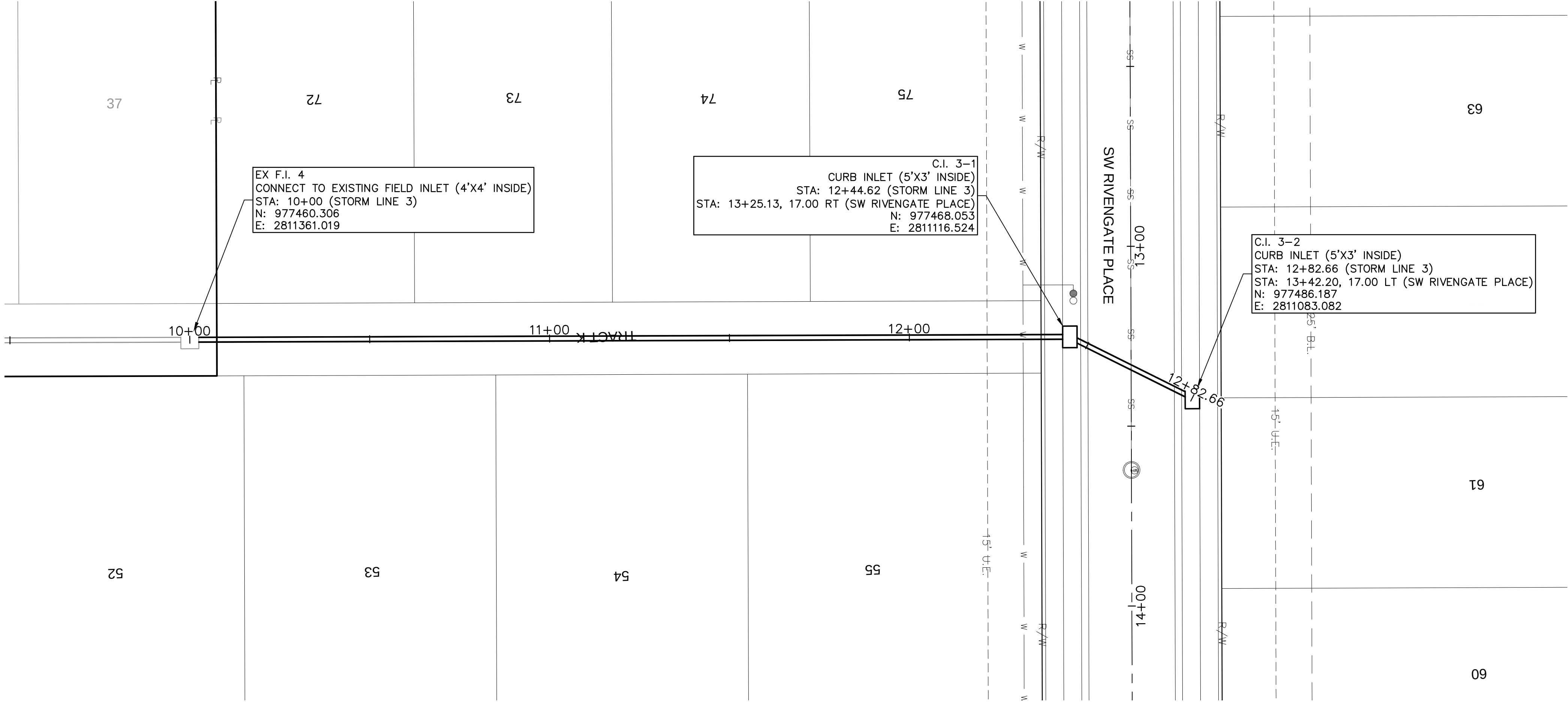
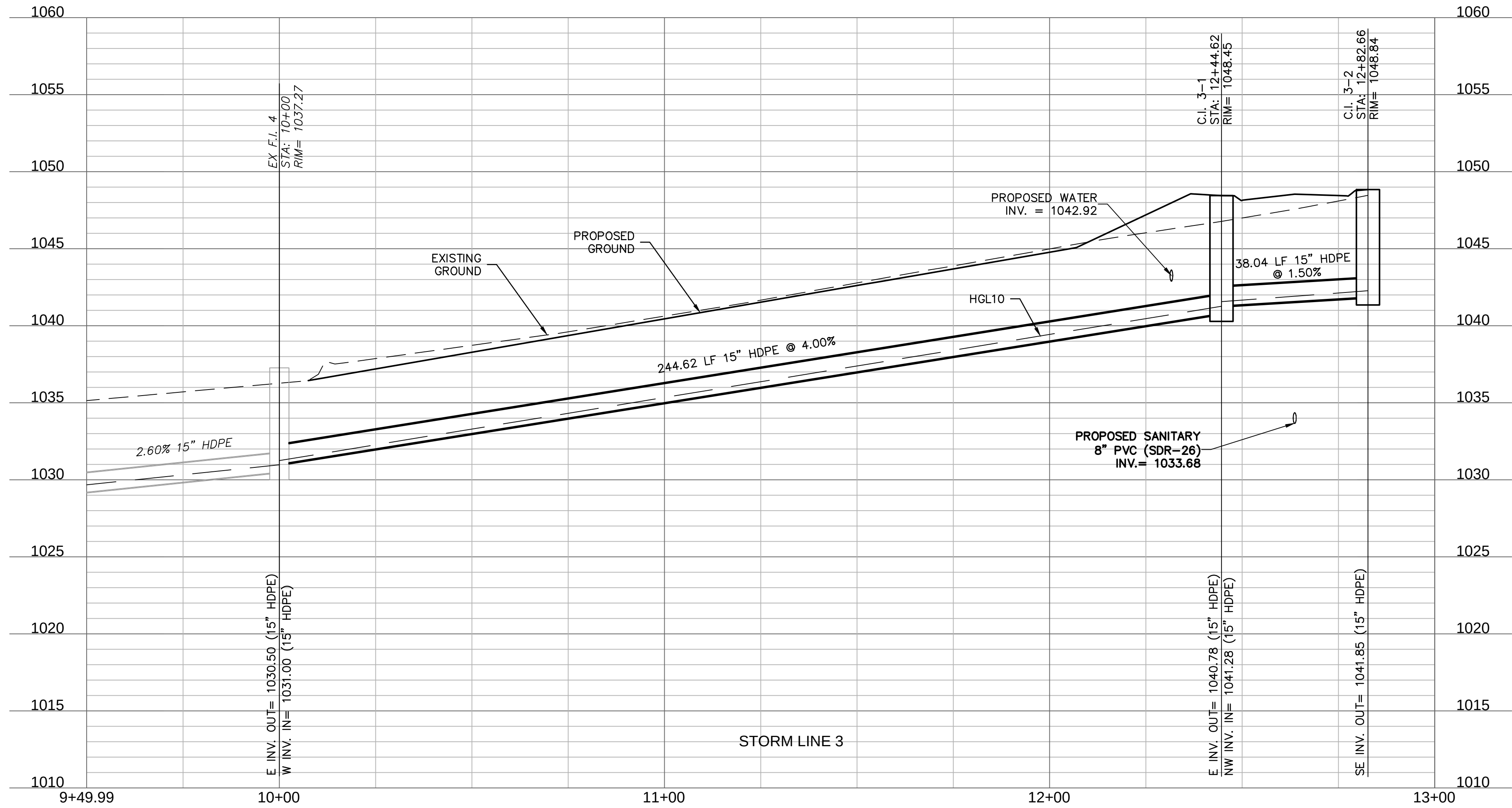
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2020

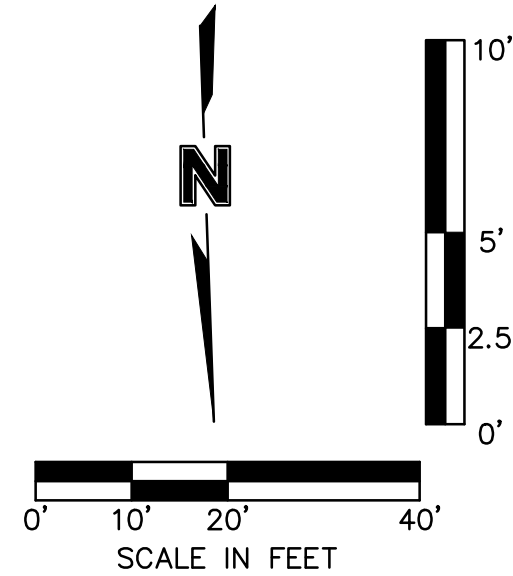
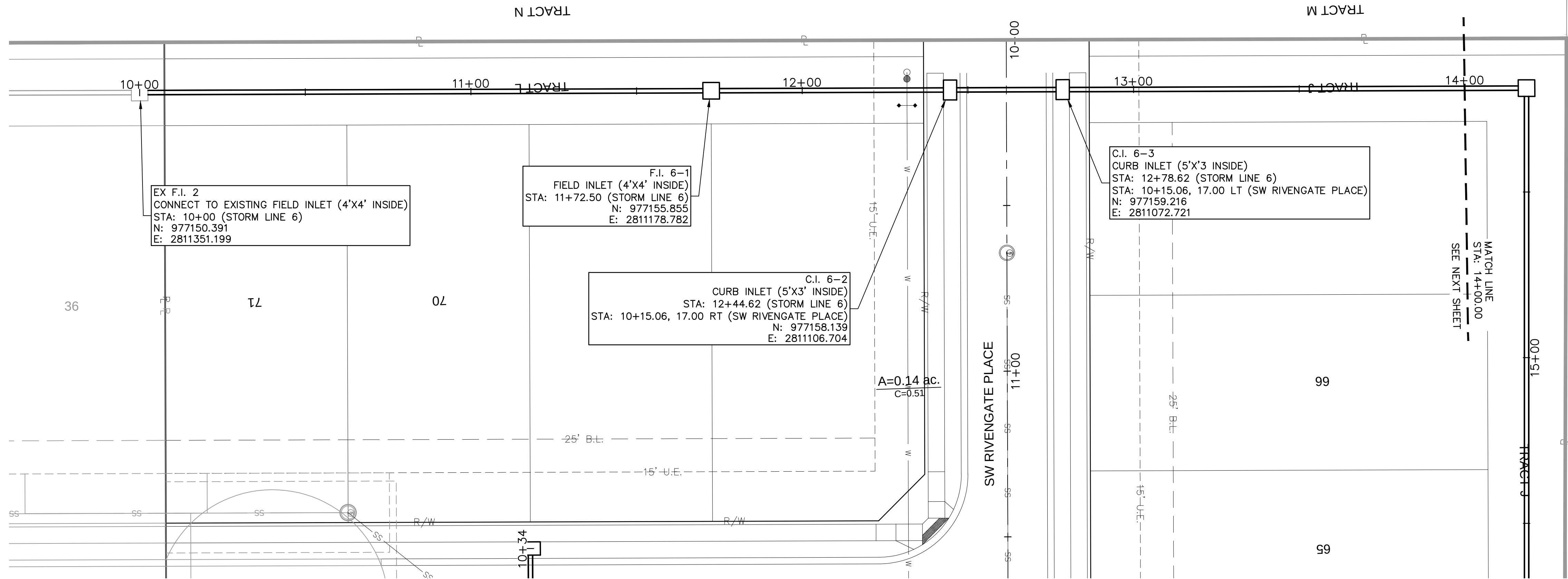
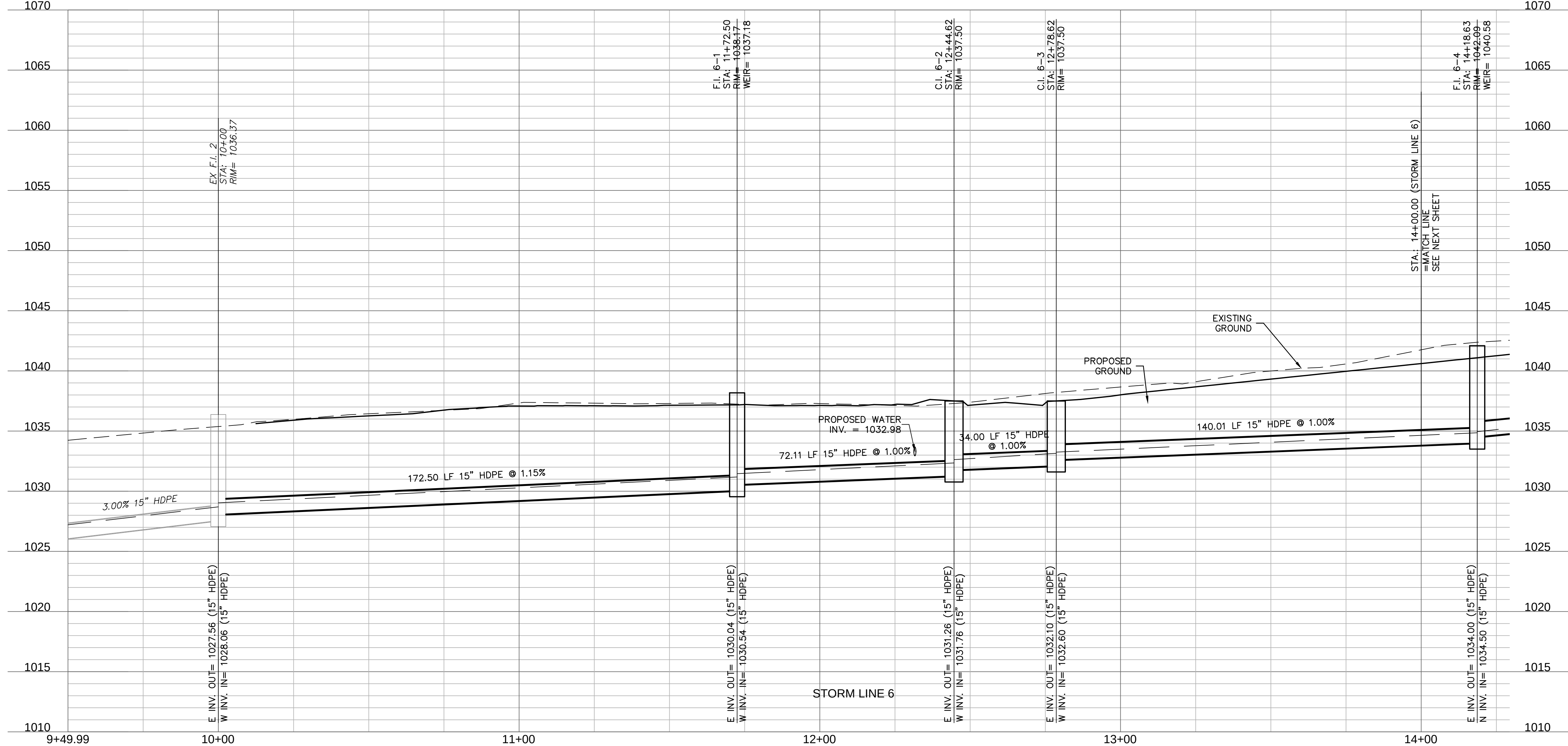
STORM SEWER PLAN & PROFILE (LINE 3)
STREET & STORM SEWER PLANS

OSAGE
SECOND PLAT

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designed by: A.A.
QA/QC by: J.E.S.
project no.: C19-2339
drawing no.:
date: 2020.08.21

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C120

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checked by: C.G.W.
designed by: A.A.
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drawing no.:
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STORM SEWER PLAN & PROFILE (LINE 6)
STREET & STORM SEWER PLANS
OSAGE
SECOND PLAT

2020

SHEET
C122

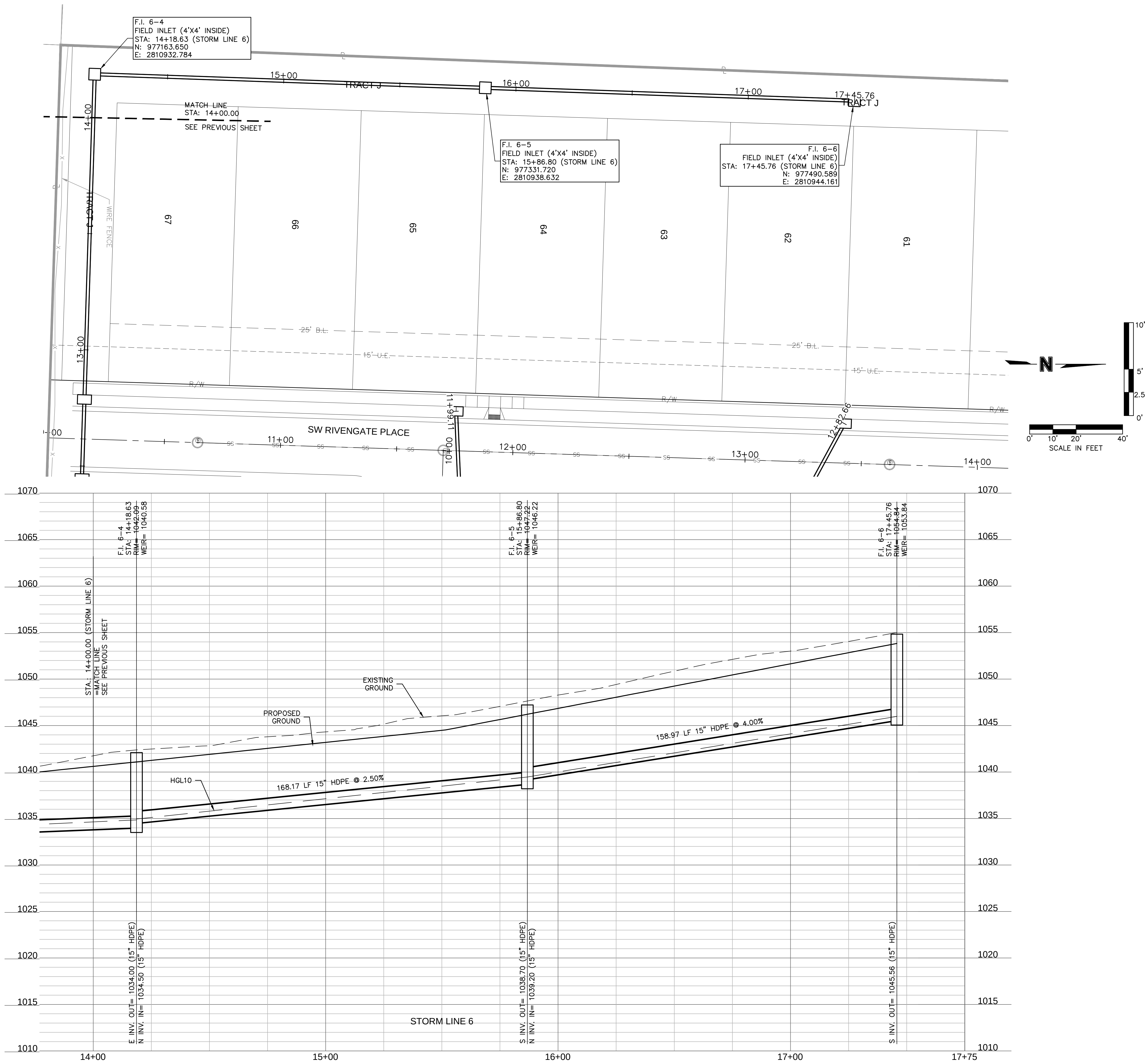
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STORM SEWER PLAN & PROFILE (LINE 6 CONT'D)		REV. NO.		DATE		REVISIONS DESCRIPTION	
OSAGE SECOND PLAT		1					
		2					
		3					
		4					
2020		REVISIONS					

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checked by: C.G.W.
designed by: A.A.
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drawing no.:
date: 2020.08.21

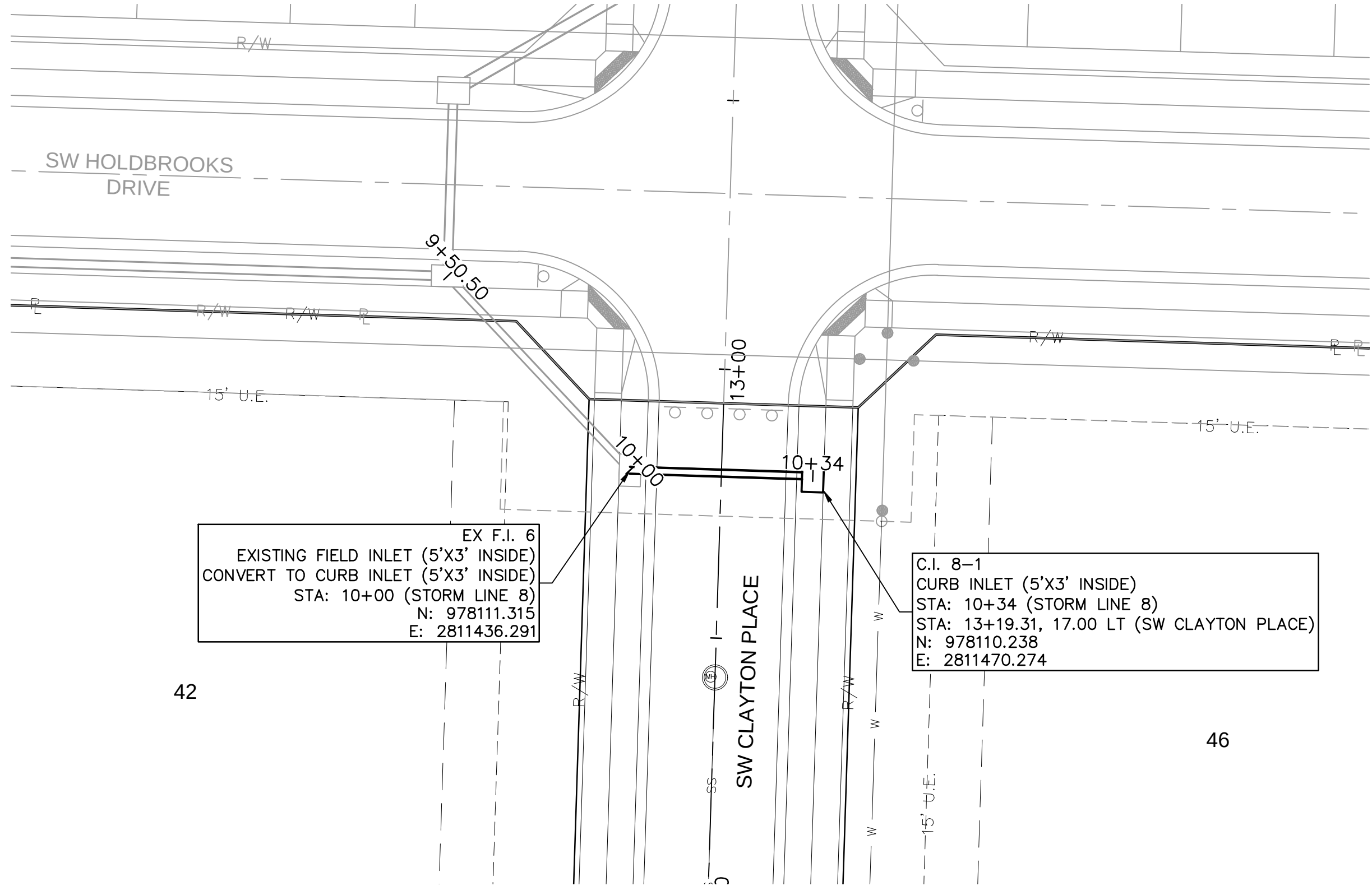
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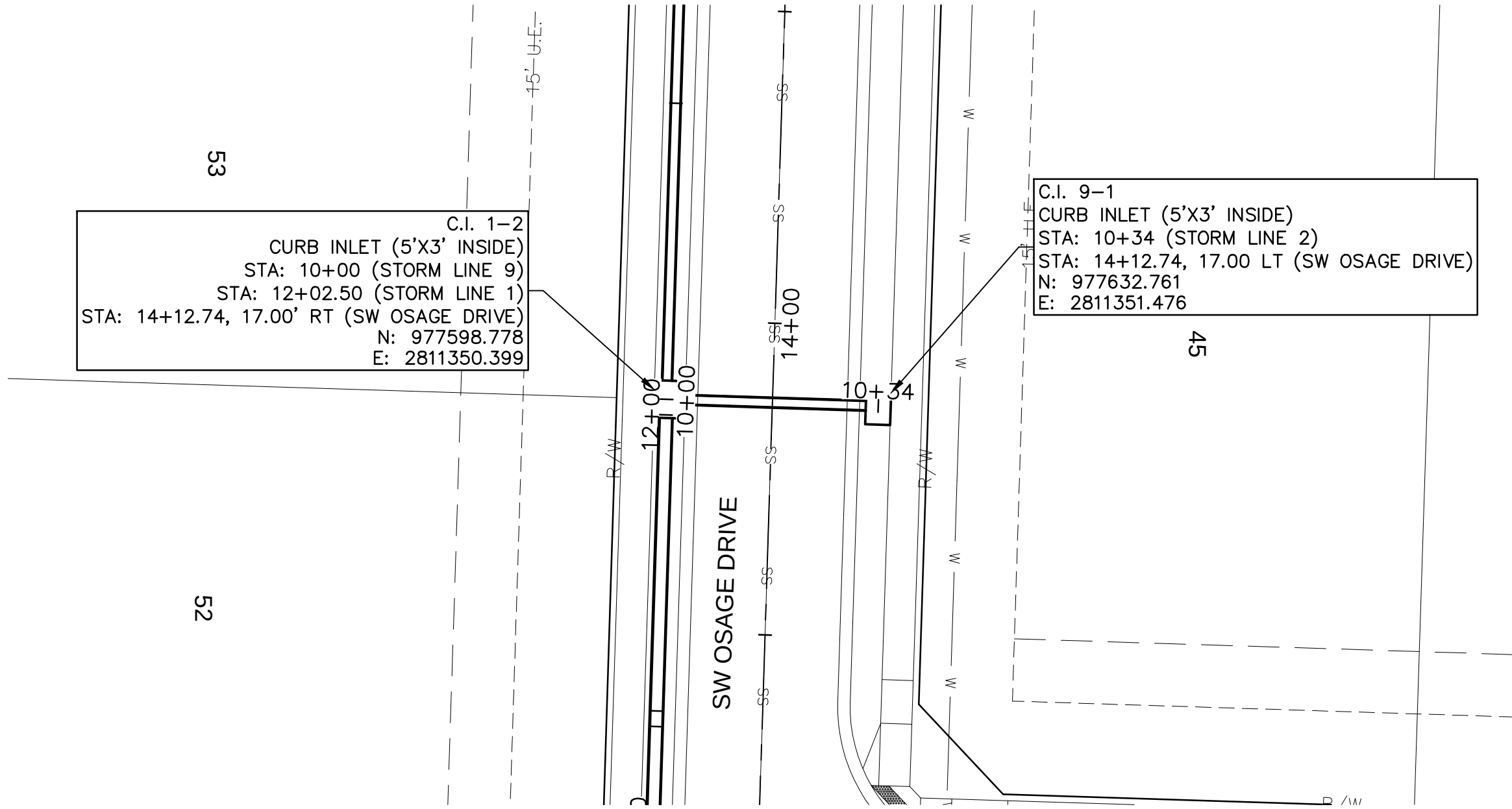
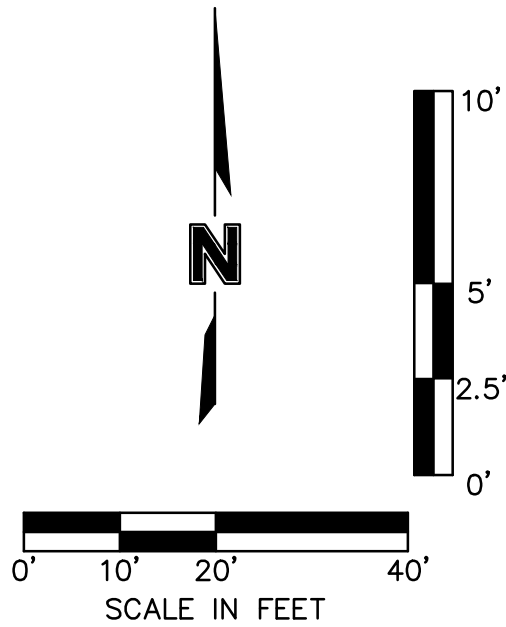
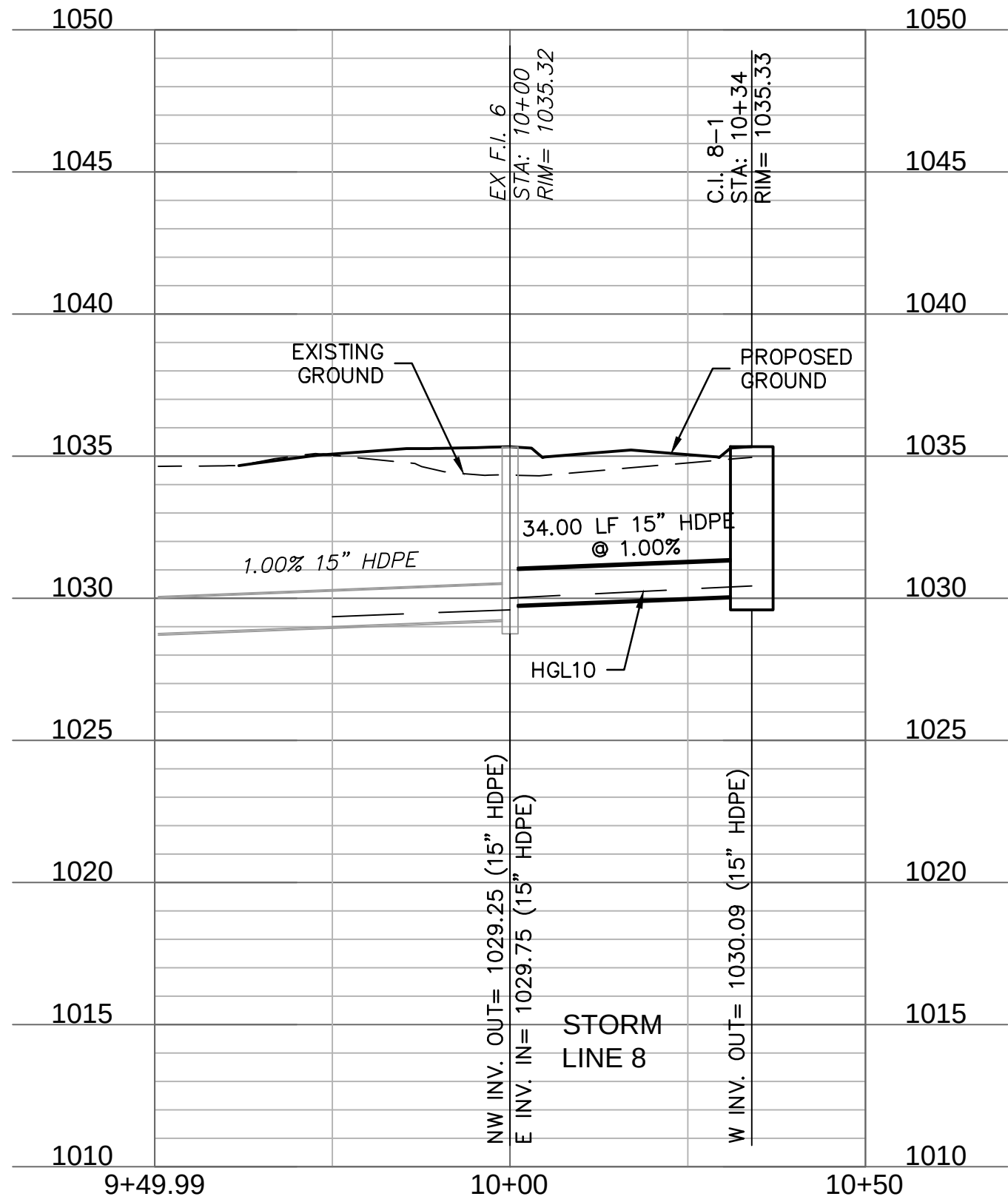
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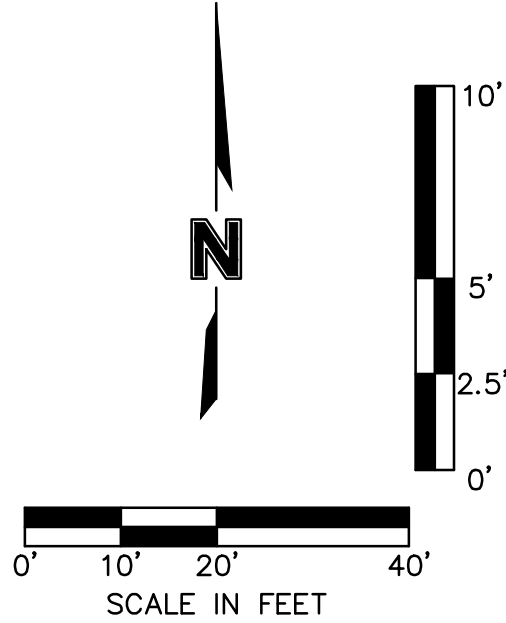
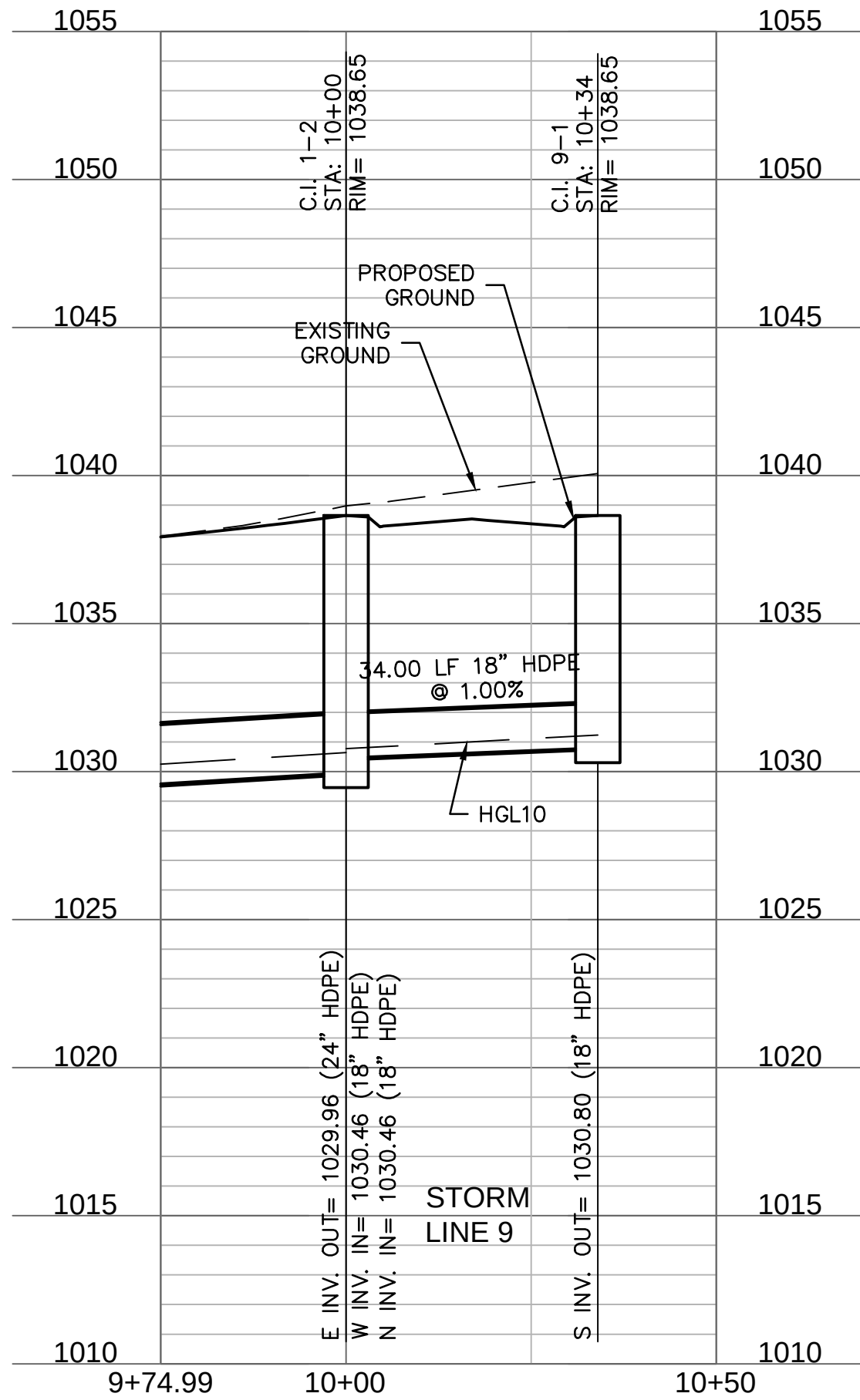
EX F.I. 6
EXISTING FIELD INLET (5'X3' INSIDE)
CONVERT TO CURB INLET (5'X3' INSIDE)
STA: 10+00 (STORM LINE 8)
N: 978111.315
E: 2811436.291

C.I. 8-1
CURB INLET (5'X3' INSIDE)
STA: 10+34 (STORM LINE 8)
STA: 13+19.31, 17.00 LT (SW CLAYTON PLACE)
N: 978110.238
E: 2811470.274



C.I. 1-2
CURB INLET (5'X3' INSIDE)
STA: 10+00 (STORM LINE 9)
STA: 12+02.50 (STORM LINE 1)
STA: 14+12.74, 17.00' RT (SW OSAGE DRIVE)
N: 977598.778
E: 2811350.399

C.I. 9-1
CURB INLET (5'X3' INSIDE)
STA: 10+34 (STORM LINE 2)
STA: 14+12.74, 17.00 LT (SW OSAGE DRIVE)
N: 977632.761
E: 2811351.476



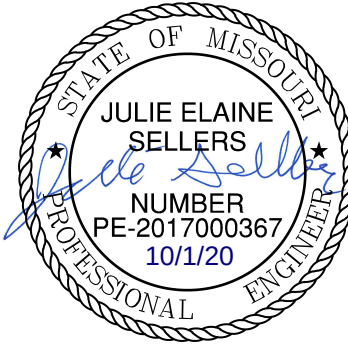
STORM SEWER PLAN & PROFILE (LINES 8 & 9)
STREET & STORM SEWER PLANS

OSAGE
SECOND PLAT

2020

drawn by: D.F./M.O./A.A.
checked by: C.G.W.
designed by: A.A.
QA/QC by: J.E.S.
project no.: C19-2339
drawing no.:
date: 2020.08.21

SHEET
C125



olsson

Olsson - Civil Engineering
Missouri Certificate of Authority #: 001592
1301 Burlington Street
North Kansas City, MO 64116
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REVISIONS

LEGEND	
	FINISHED INDEX CONTOURS
	FINISHED INTERMEDIATE CONTOURS

BASEMENT TYPES

- ☐ S STANDARD
☐ W WALKOUT
☐ D DAYLIGHT

NOTES:

MBOE – MINIMUM BUILDING OPENING ELEVATION

NOTES:

1. INDIVIDUAL LOT OWNERS SHALL NOT CHANGE OR OBSTRUCT THE DRAINAGE FLOW LINES OR PATHS ON THE LOTS, AS SHOWN ON THE MASTER DRAINAGE PLAN, UNLESS SPECIFIC APPLICATION IS MADE AND APPROVED BY THE CITY ENGINEER.

2. PLAT IS LOCATION IN ZONE X, "AREAS OUTSIDE THE 1-PERCENT ANNUAL CHANCE FLOODPLAINS, AREAS OF 1-PERCENT ANNUAL CHANCE SHEET FLOOD FLOODING WHERE THE AVERAGE DEPTHS ARE LESS THAN 1 FOOT, AREAS OF 1-PERCENT ANNUAL CHANCE STREAM FLOODING WHERE THE CONTRIBUTING DRAINAGE AREA IS LESS THAN 1 SQUARE MILE, OR AREAS PROTECTED FROM THE 1-PERCENT ANNUAL CHANCE FLOOD BY LEVEES. NO BASE FLOOD ELEVATIONS OR DEPTHS ARE SHOWN WITHIN THIS ZONE"

3. PLAT IS LOCATED OUTSIDE OF ANY REQUIRED BUFFER ZONES FOR NATURAL STREAMS.

4. MBOE ELEVATIONS HAVE BEEN PROVIDED AT EACH LOT CORNER. INTERPOLATION WILL BE ALLOWED BETWEEN THE RIGHT AND LEFT SIDE MBOE'S SHOWN ON THE MASTER DRAINAGE PLAN, DEPENDING ON THE LOCATION OF THE LOWEST OPENING ON THE PROPOSED STRUCTURE.

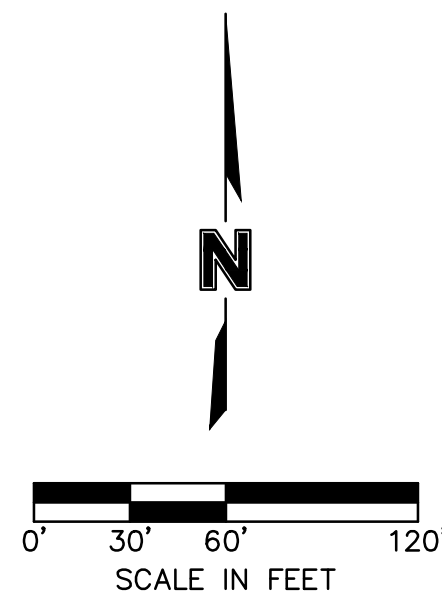
5. REFER TO SHEET C106-C109 FOR SWALE GRADING DETAILS.

6. DRAINAGE PATHS TO BE CONSTRUCTED BETWEEN EACH OF THE LOTS LABELED AS STANDARD LOTS TO DRAIN WEST.

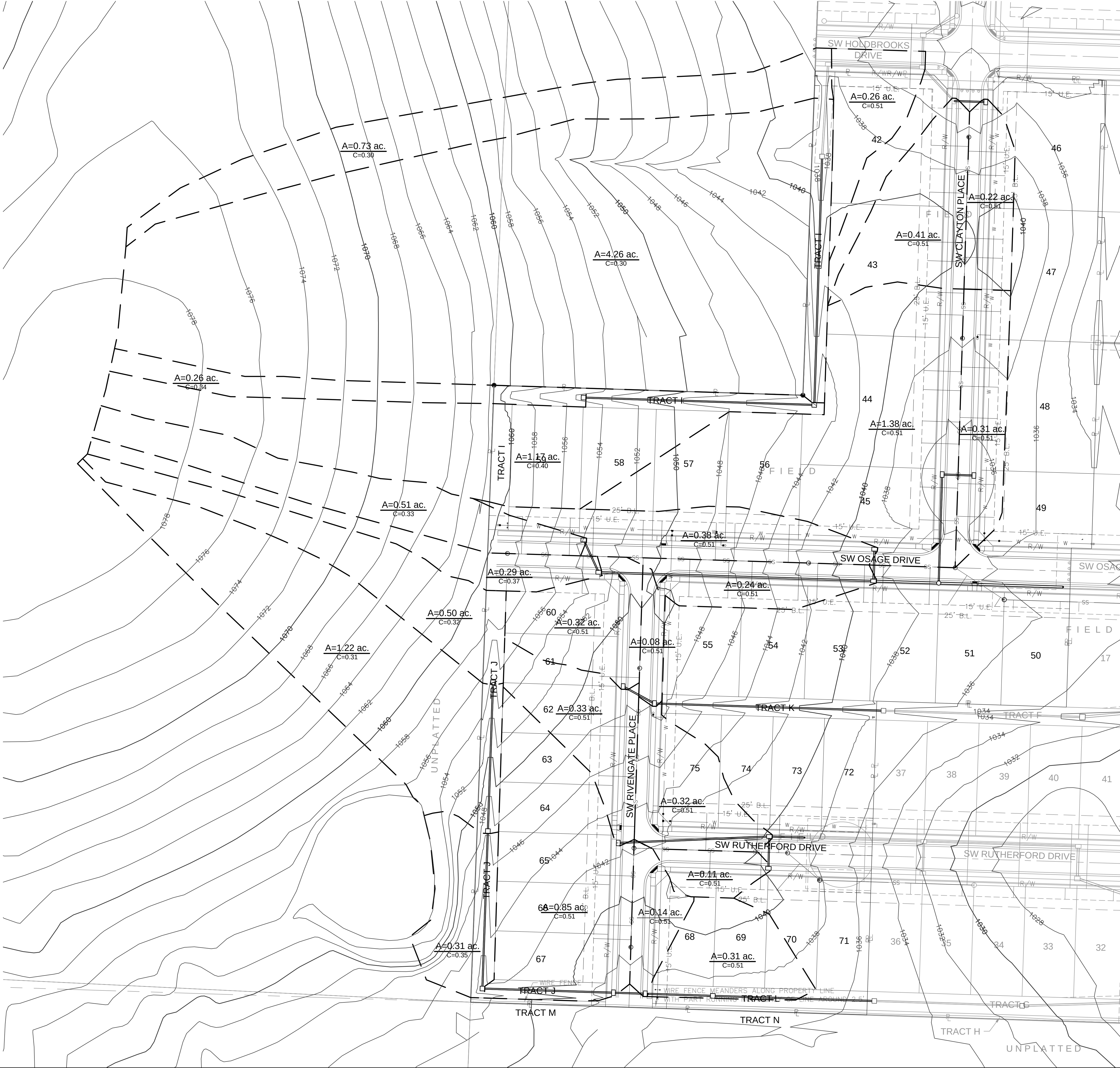
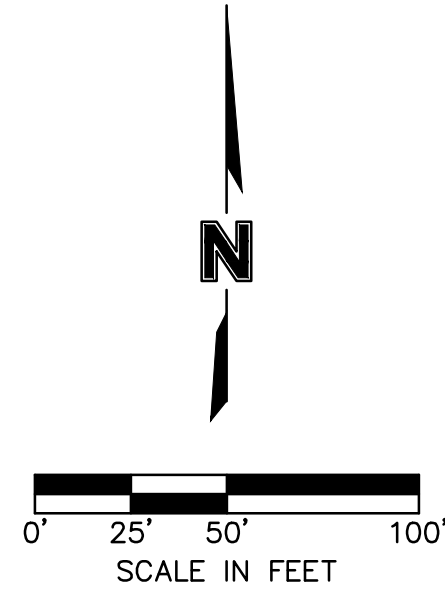
7. NO BUILDING PERMITS WILL BE ISSUED UNTIL AN AS-GRADED MASTER DRAINAGE PLAN HAS BEEN SUBMITTED TO THE CITY AND APPROVED BY THE CITY.

Osage 2nd Plat					
Minimum Building Opening Elevation					
Lot	Rear Left MBOE	Rear Right MBOE	Front Left MBOE	Front Right MBOE	As-Built Grading Plan Required
42	1042.46	1037.70	1040.48	1035.72	X
43	1041.00	1042.46	1039.02	1040.48	X
44	1038.28	1041.00	1036.30	1039.02	X
45	1039.11	1038.58	1036.83	1036.30	X
46	1037.70	1042.46	1035.72	1040.48	
47	1042.46	1041.00	1040.48	1039.02	
48	1041.00	1038.28	1039.02	1036.30	
49	1038.58	1039.11	1036.30	1036.83	
50	1034.57	1035.86	-	-	X
51	1035.86	1037.26	-	-	X
52	1040.00	1041.60	1037.72	1039.32	X
53	1041.60	1044.58	1039.32	1042.30	X
54	1044.58	1048.55	1042.30	1046.27	X
55	1048.55	1052.18	1046.27	1049.90	X
56	1049.71	1045.40	1047.43	1042.92	X
57	1053.54	1049.71	1051.26	1047.43	X
58	1057.19	1053.54	1054.91	1051.26	X
59	1061.86	1057.19	1059.58	1054.91	X
60	1052.61	1053.30	1050.33	1051.02	
61	1051.79	1052.61	1049.51	1050.33	X
62	1050.13	1051.79	1047.85	1049.51	X
63	1048.00	1050.13	1045.72	1047.85	X
64	1045.85	1048.00	1043.57	1045.72	X
65	1043.79	1045.85	1041.51	1043.57	X
66	1041.91	1043.79	1039.63	1041.51	X
67	1040.65	1041.91	1038.37	1039.63	X
68	1038.92	1038.63	-	-	
69	1038.62	1038.92	-	-	
70	1037.58	1038.62	-	-	
71	1037.97	1040.89	1035.69	1038.61	
72	1040.88	1037.97	1038.60	1035.69	X
73	1042.80	1040.88	1040.52	1038.60	X
74	1043.82	1042.80	1041.54	1040.52	X
75	1044.68	1043.82	1042.40	1041.54	X

X indicates condition applies to the lot.
MBOE's have been provided per lot lines, facing the lot from the street.
Interpolation will be allowed between the right and left side MBOE's provided, depending on the location of the lowest opening on the proposed structure.

[illegible]

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USER: cshdijay



DRAINAGE PLAN
STREET & STORM SEWER PLANS

OSAGE
SECOND PLAT

drawn by: D.F./M.O./A.A.
checked by: C.G.W.
designed by: A.A.
QA/QC by: J.E.S.
project no.: C19-2339
drawing no.:
date: 2020.08.21

SHEET
C127

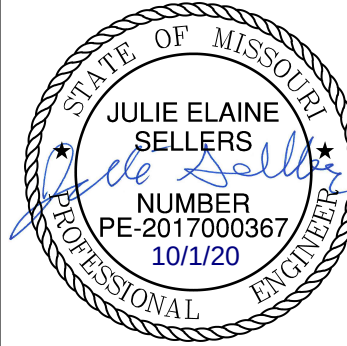
REVISIONS DESCRIPTION

REV. NO.

DATE

REVISIONS

2020



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Missouri Certificate of Authority #: 001592
1301 Burlington Street
North Kansas City, MO 64116
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www.olson.com

Inlet Design Table						
10 Year Return Frequency						
Inlet ID	Captured Flow	Bypass Flow	Inlet Efficiency (Note 2)	Gutter Depth	Gutter Spread	Ponding Depth
	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
C.I. 1-2	0.80	0.10	88.43%	0.10	5.13	...
C.I. 1-3	0.69	0.10	86.80%	0.09	4.57	...
C.I. 1-4	0.98	0.27	78.25%	0.11	5.30	...
C.I. 2-1(L)	...	...	...	0.11	5.50	...
C.I. 2-1(R)	...	...	...	0.14	7.10	...
C.I. 2-1	5.18	0.00	100.00%	...	...	...
C.I. 2-2(L)	...	...	...	0.05	2.34	...
C.I. 2-2(R)	...	...	...	0.09	4.71	...
C.I. 2-2	1.16	0.00	100.00%	...	...	...
C.I. 3-1	0.29	0.01	97.21%	0.07	3.52	...
C.I. 3-2	1.15	0.15	88.48%	0.13	6.52	...
C.I. 4-1	1.05	0.15	87.26%	0.12	6.05	...
C.I. 4-2	1.11	0.28	80.00%	0.12	5.82	...
C.I. 5-1	0.40	0.02	96.15%	0.08	4.04	...
F.I. 6-1	1.16	0.00	100.00%	...	...	0.08
C.I. 6-2	0.54	0.03	95.46%	0.10	4.76	...
C.I. 6-3	2.41	0.90	72.75%	0.19	9.25	...
F.I. 6-4	0.81	0.00	100.00%	...	...	0.06
F.I. 6-5	2.47	0.00	100.00%	...	...	0.13
F.I. 6-6	1.18	0.00	100.00%	...	...	0.08
F.I. 7-1	2.92	0.00	100.00%	...	...	0.15
F.I. 7-2	0.65	0.00	100.00%	...	...	0.05
C.I. 8-1	0.78	0.05	94.15%	0.11	5.75	...
C.I. 9-1	1.34	0.36	78.82%	0.13	6.51	...
Notes:						
1. Inlet capacity at sag location has been reduced by a clogging factor of 0.80, reducing Both theoretical capacity and reduced capacity are shown.						
2. Inlet efficiency shown in the tables is Captured Flow/Total Flow, denoting the actual						

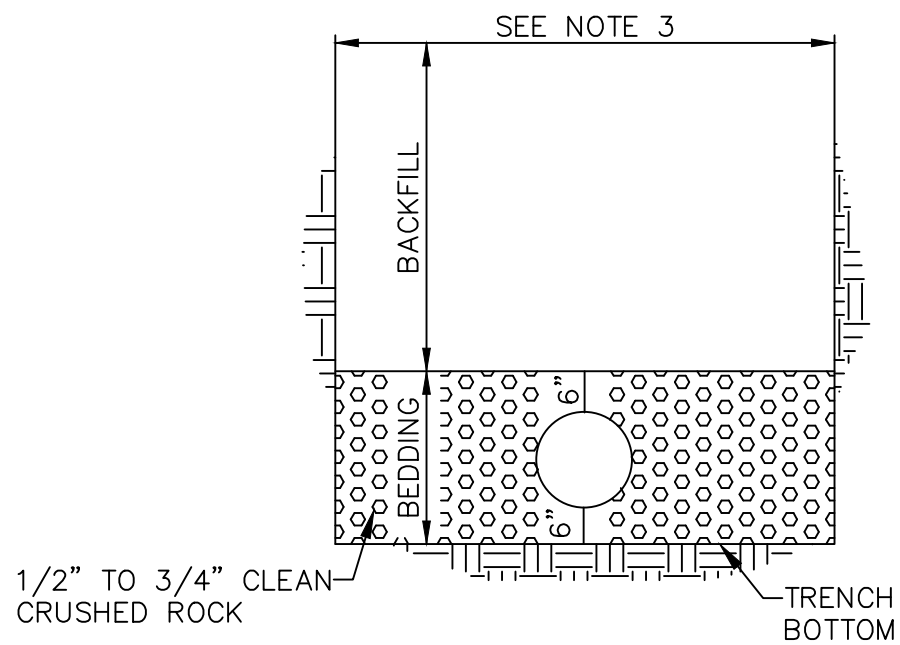
Inlet Design Table						
100 Year Return Frequency						
Inlet ID	Captured Flow	Bypass Flow	Inlet Efficiency (Note 2)	Gutter Depth	Gutter Spread	Ponding Depth
	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
C.I. 1-2	1.27	0.31	80.14%	0.13	6.34	...
C.I. 1-3	1.08	0.31	77.66%	0.11	5.64	...
C.I. 1-4	1.43	0.76	65.42%	0.13	6.55	...
C.I. 2-1(L)	...	...	...	0.16	7.76	...
C.I. 2-1(R)	...	...	...	0.18	8.77	...
C.I. 2-1	9.08	0.00	100.00%	...	...	...
C.I. 2-2(L)	...	...	...	0.06	2.89	...
C.I. 2-2(R)	...	...	...	0.12	5.81	...
C.I. 2-2	2.04	0.00	100.00%	...	...	...
C.I. 3-1	0.50	0.03	94.35%	0.09	4.34	...
C.I. 3-2	1.92	0.50	79.24%	0.16	8.22	...
C.I. 4-1	1.67	0.46	78.23%	0.15	7.49	...
C.I. 4-2	1.75	0.92	65.42%	0.15	7.44	...
C.I. 5-1	0.67	0.05	92.55%	0.10	4.99	...
F.I. 6-1	2.04	0.00	100.00%	...	...	0.11
C.I. 6-2	0.90	0.08	91.39%	0.12	5.88	...
C.I. 6-3	3.51	2.75	56.02%	0.24	11.75	...
F.I. 6-4	1.42	0.00	100.00%	...	...	0.09
F.I. 6-5	4.36	0.00	100.00%	...	...	0.19
F.I. 6-6	2.07	0.00	100.00%	...	...	0.12
F.I. 7-1	5.16	0.00	100.00%	...	...	0.21
F.I. 7-2	1.14	0.00	100.00%	...	...	0.08
C.I. 8-1	1.29	0.16	89.23%	0.14	7.10	...
C.I. 9-1	2.08	1.18	63.82%	0.17	8.31	...
Notes:						
1. Inlet capacity at sag location has been reduced by a clogging factor of 0.80, reducing Both theoretical capacity and reduced capacity are shown.						
2. Inlet efficiency shown in the tables is Captured Flow/Total Flow, denoting the actual						

Drainage Area Design Table						
10 Year Return Frequency						
Inlet ID	Drainage Area	C	Tc	i	K	Peak Flow
	(ac)		(min)	(in/hr)		(cfs)
C.I. 1-2	0.24	0.51	5.00	7.35	1.00	0.90
C.I. 1-3	0.29	0.51	5.00	7.35	1.00	0.79
C.I. 1-4	0.51	0.37	5.00	7.35	1.00	1.25
C.I. 2-1(L)	0.10	0.33	5.00	7.35	1.00	0.38
C.I. 2-1(R)	0.45	0.51	5.00	7.35	1.00	1.69
C.I. 2-1(B)	0.83	0.51	5.00	7.35	1.00	3.11
C.I. 2-1	1.38	0.51	5.00	7.35	1.00	5.18
C.I. 2-2(L)	0.02	0.51	5.00	7.35	1.00	0.08
C.I. 2-2(R)	0.15	0.51	5.00	7.35	1.00	0.56
C.I. 2-2(B)	0.14	0.51	5.00	7.35	1.00	0.53
C.I. 2-2	0.31	0.51	5.00	7.35	1.00	1.16
C.I. 3-1	0.08	0.51	5.00	7.35	1.00	0.30
C.I. 3-2	0.32	0.51	5.00	7.35	1.00	1.20
C.I. 4-1	0.32	0.51	5.00	7.35	1.00	1.20
C.I. 4-2	0.33	0.51	5.00	7.35	1.00	1.24
C.I. 5-1	0.11	0.51	5.00	7.35	1.00	0.41
F.I. 6-1	0.31	0.51	5.00	7.35	1.00	1.16
C.I. 6-2	0.15	0.51	5.00	7.35	1.00	0.56
C.I. 6-3	0.86	0.51	6.49	6.92	1.00	3.03
F.I. 6-4	0.31	0.51	5.00	7.35	1.00	0.81
F.I. 6-5	1.22	0.35	8.31	6.46	1.00	2.47
F.I. 6-6	0.50	0.31	5.00	7.35	1.00	1.18
F.I. 7-1	1.17	0.32	9.16	6.26	1.00	2.92
F.I. 7-2	0.26	0.40	5.00	7.35	1.00	0.65
C.I. 8-1	0.22	0.34	5.00	7.35	1.00	0.83
C.I. 9-1	0.38	0.51	5.00	7.35	1.00	1.43

Drainage Area Design Table						
100 Year Return Frequency						
Inlet ID	Drainage Area	C	Tc	i	K	Peak Flow
	(ac)		(min)	(in/hr)		(cfs)
C.I. 1-2	0.24	0.51	5.00	10.32	1.25	1.58
C.I. 1-3	0.29	0.51	5.00	10.32	1.25	1.39
C.I. 1-4	0.51	0.37	5.00	10.32	1.25	2.19
C.I. 2-1(L)	0.10	0.33	5.00	10.32	1.25	0.66
C.I. 2-1(R)	0.45	0.51	5.00	10.32	1.25	2.96
C.I. 2-1(B)	0.83	0.51	5.00	10.32	1.25	5.46
C.I. 2-1	1.38	0.51	5.00	10.32	1.25	9.08
C.I. 2-2(L)	0.02	0.51	5.00	10.32	1.25	0.13
C.I. 2-2(R)	0.15	0.51	5.00	10.32	1.25	0.99
C.I. 2-2(B)	0.14	0.51	5.00	10.32	1.25	0.92
C.I. 2-2	0.31	0.51	5.00	10.32	1.25	2.04
C.I. 3-1	0.08	0.51	5.00	10.32	1.25	0.53
C.I. 3-2	0.32	0.51	5.00	10.32	1.25	2.11
C.I. 4-1	0.32	0.51	5.00	10.32	1.25	2.11
C.I. 4-2	0.33	0.51	5.00	10.32	1.25	2.17
C.I. 5-1	0.11	0.51	5.00	10.32	1.25	0.72
F.I. 6-1	0.31	0.51	5.00	10.32	1.25	2.04
C.I. 6-2	0.15	0.51	5.00	10.32	1.25	0.99
C.I. 6-3	0.86	0.51	6.49	9.74	1.25	5.34
F.I. 6-4	0.31	0.51	5.00	10.32	1.25	1.42
F.I. 6-5	1.22	0.35	8.31	9.11	1.25	4.36
F.I. 6-6	0.50	0.31	5.00	10.32	1.25	2.07
F.I. 7-1	1.17	0.32	9.16	8.84	1.25	5.16
F.I. 7-2	0.26	0.40	5.00	10.32	1.25	1.14
C.I. 8-1	0.22	0.34	5.00	10.32	1.25	1.45
C.I. 9-1	0.38	0.51	5.00	10.32	1.25	2.50

Storm Sewer Design Calculation Table													
10 Year Return Frequency													
Upstream Structure	Downstream Structure	Length (ft)	Upstream Invert (ft)	Downstream Invert (ft)	Slope (%)	Diameter (in)	Manning's n	Total Flow (cfs)	Velocity (ft/s)	Capacity (cfs)	Flow Depth (ft)	Upstream Struct. HGL (ft)	Upstream Top Elev. (ft)
CONNECT TO EX. 1	EX C.I. 2	113.85	1026.43	1024.72	1.50	24	0.010	10.15	9.85	36.04	1.14	1027.57	1028.70
M.H. 1-1	CONNECT TO EX. 1	132.88	1028.42	1026.43	1.50	24	0.010	10.15	5.49	36.01	1.14	1029.56	1037.06
C.I. 1-2	M.H. 1-1	69.62	1029.96	1028.92	1.49	24	0.010	3.81	4.38	35.94	0.68	1030.64	1038.65
C.I. 1-3	C.I. 1-2	293.62	1044.65	1030.46	4.83	18	0.010	1.67	9.12	30.01	0.49	1045.14	1053.11
C.I. 1-4	C.I. 1-3	38.03	1046.10	1045.15	2.50	15	0.010	0.98	6.31	13.27	0.39	1046.49	1053.96
C.I. 2-1	M.H. 1-1	116.00	1030.33	1029.17	1.00	15	0.010	6.34	7.51	8.39	1.01	1031.35	1035.61
C.I. 2-2	C.I. 2-1	34.00	1031.17	1030.83	1.00	15	0.010	1.16	2.43	8.40	0.42	1031.59	1035.61
C.I. 9-1	C.I. 1-2	34.00	1030.80	1030.46	1.00	18	0.010	1.34	4.91	13.65	0.43	1031.23	1038.65
EX F.I. 4	EX F.I. 3	212.50	1030.50	1024.98	2.60	15	0.010	1.45	7.18	13.53	0.48	1030.98	1037.27
C.I. 3-1	EX F.I. 4	244.62	1040.79	1031.00	4.00	15	0.010	1.45	8.37	16.79	0.48	1041.27	1048.45
C.I. 3-2	C.I. 3-1	38.04	1041.85	1041.28	1.50	15	0.010	1.15	5.53	10.28	0.42	1042.27	1048.84
EX F.I. 2	EX F.I. 1	158.03	1027.56	1022.82	3.00	15	0.010	8.57	12.33	14.54	1.14	1028.70	1036.37
F.I. 6-1	EX F.I. 2	172.50	1030.04	1028.06	1.15	15	0.010	8.57	8.34	8.99	1.14	1031.18	1038.17
C.I. 6-2	F.I. 6-1	72.11	1031.26	1030.54	1.00	15	0.010	7.41	7.71	8.39	1.08	1032.34	1037.50
C.I. 6-3	C.I. 6-2	34.00	1032.10	1031.76	1.00	15	0.010	6.87	7.63	8.39	1.05	1033.15	1037.50
F.I. 6-4	C.I. 6-3	140.01	1034.00	1032.60	1.00	15	0.010	4.46	6.94	8.39	0.86	1034.86	1042.09
F.I. 6-5	F.I. 6-4	168.17	1038.70	1034.50	2.50	15	0.010	3.65	9.22	13.27	0.77	1039.47	1047.22
F.I. 6-6	F.I. 6-5	158.97	1045.56	1039.20	4.00	15	0.010	1.18	6.02	16.79	0.43	1045.99	1054.84
EX F.I. 5	EX C.I. 5	98.76	1031.25	1030.26	1.00	18	0.010	3.57	6.50	13.67	0.72	1031.97	1037.71
F.I. 7-1	EX F.I. 5	265.34	1037.06	1031.75	2.00	15	0.010	3.57	8.46	11.88	0.76	1037.82	1045.01
F.I. 7-2	F.I. 7-1	246.48	1045.57	1037.56	3.25	15	0.010	0.65	3.48	15.13	0.31	1045.89	1054.58
CONNECT TO EX. 2	EX C.I. 1	127.46	1030.64	1024.27	5.00	15	0.010	2.69	10.84	18.77	0.66	1031.30	1032.33
C.I. 4-1	CONNECT TO EX. 2	37.54	1032.52	1030.64	5.01	15	0.010	2.69	4.11	18.79	0.66	1033.18	1039.86
C.I. 4-2	C.I. 4-1	161.56	1034.64	1033.02	1.00	15	0.010	1.24	4.90	8.41	0.44	1035.08	10

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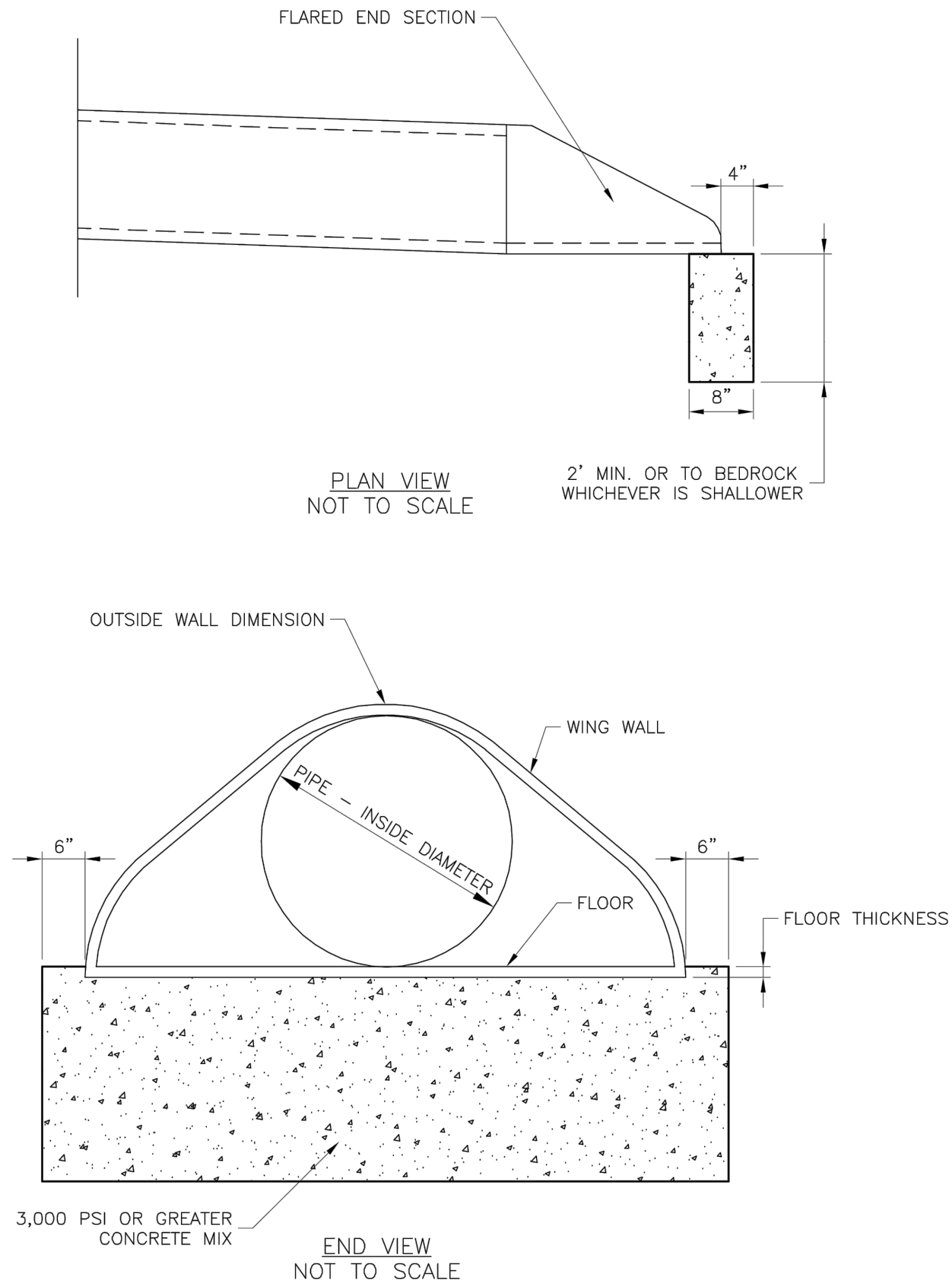


TYPICAL SECTION FOR PLASTIC PIPE
(IN ROCK OR SOIL)

UNDERGROUND PIPE INSTALLATION FOR STORM SEWER LINES

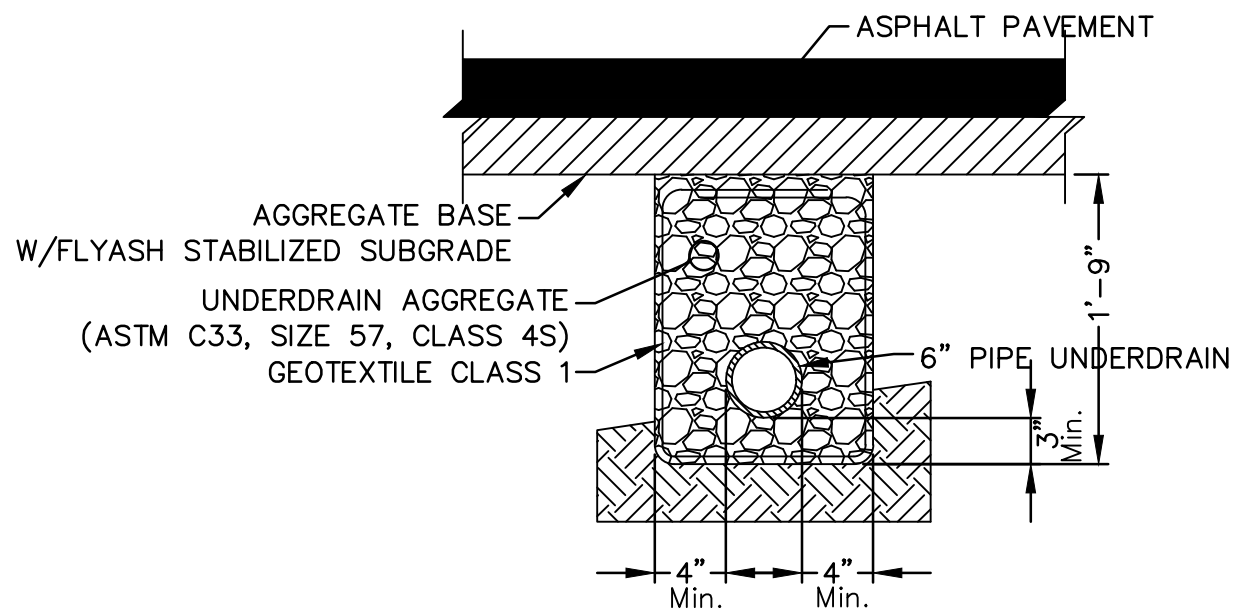
N.T.S.

- BACKFILL SHALL BE JOB EXCAVATED MATERIAL FREE FROM DEBRIS AND STONES COMPACTED TO 90% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT AS DETERMINED BY ASTM D698. BACKFILL UNDER PAVEMENT (EXISTING OR PROPOSED), SHALL BE FLOWABLE FILL.
- TRENCH BANKS MAY BE CUT BACK ON SLOPES IN ACCORDANCE WITH CURRENT OSHA REGULATIONS, BUT ONLY IN AREAS WHERE THE INCREASED TRENCH WIDTH WILL NOT INTERFERE WITH SURFACE FEATURES. SLOPES MUST NOT EXTEND BELOW TOP OF BEDDING.
- MINIMUM AND MAXIMUM WIDTHS SHALL BE IN ACCORDANCE WITH PIPE MANUFACTURER'S RECOMMENDATION AS APPROVED ON ENGINEERING PLANS.



LEE'S SUMMIT
MISSOURI
PUBLIC WORKS ENGINEERING DIVISION | 220 SE GREEN STREET | LEE'S SUMMIT, MO 64063
FLARED END SECTION SUPPORT DETAIL

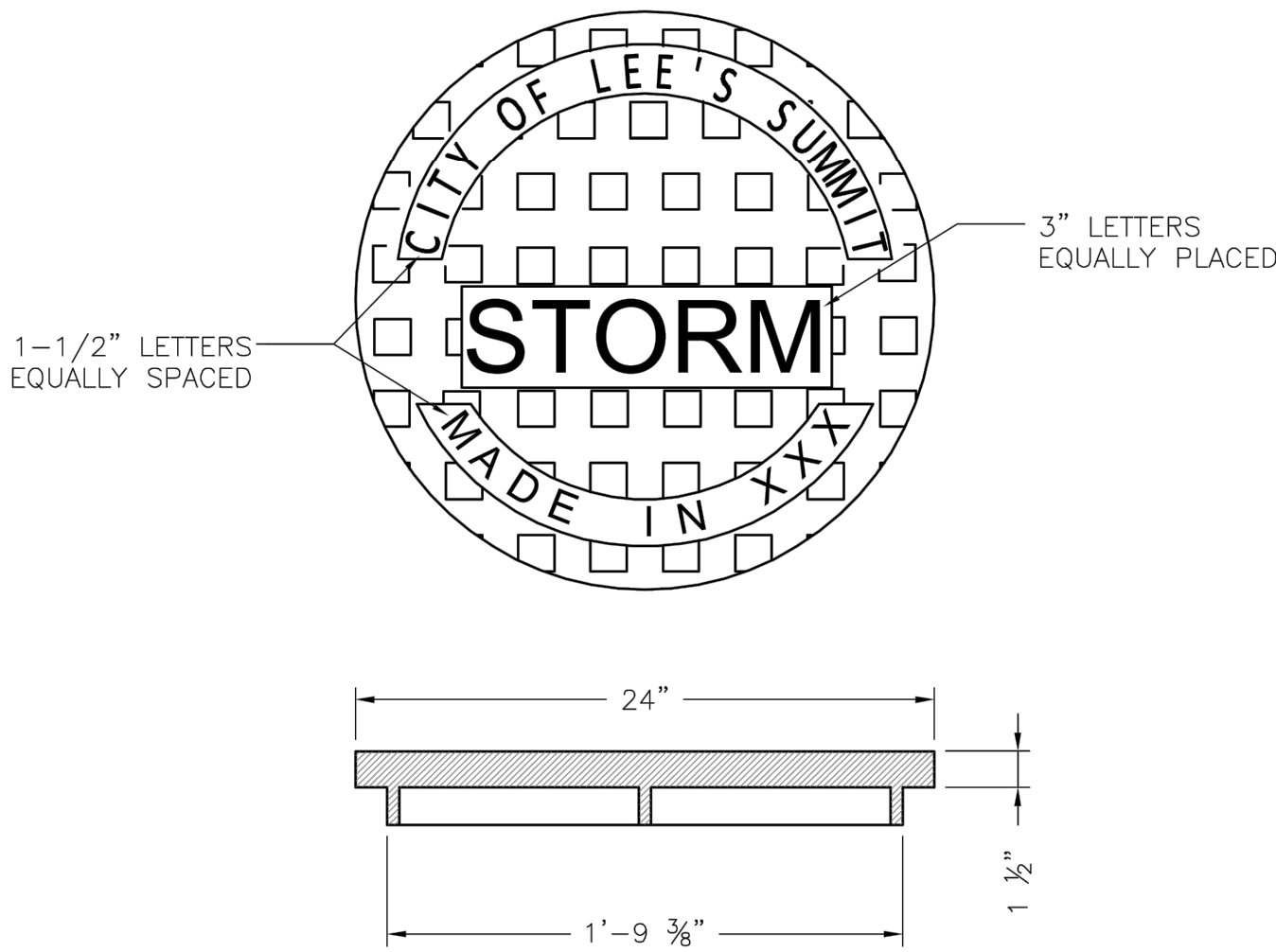
Date: 04/17
Drawn By: MIF
Checked By: DL
STM-5



PIPE UNDERDRAIN LATERAL
N.T.S.

NOTES:

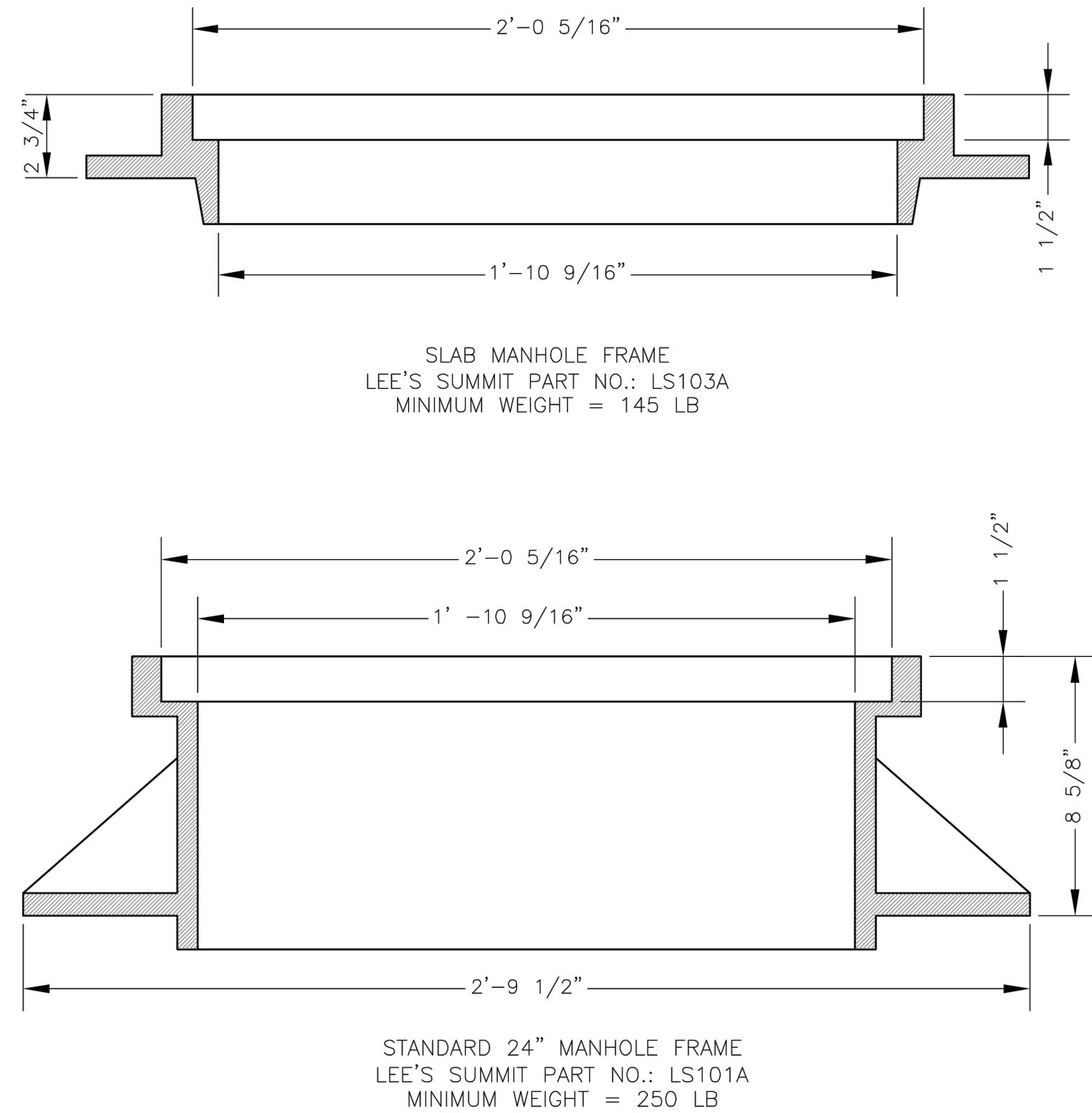
- Where Pipe Underdrains are used, all Underdrain Outlet Pipes shall be solid wall with watertight joints. All Outlets Pipes shall be tied into the nearest storm sewer inlets at roadway sag locations as indicated in the street profile.
- All Underdrain Pipes shall be installed at a minimum slope of 1%.
- Underdrain Pipe shall be installed with the perforations placed down.
- Blanket Underdrain Aggregate, Pipe Underdrain Aggregate, Pipe Underdrain, Edge Underdrain and Outlet Pipe shall conform to City of Lee's Summit Specifications.
- Overlap geotextile at top of trench a minimum of 12\"/>



STANDARD 24" MANHOLE COVER
MINIMUM WEIGHT = 160 LB
NOTE: PICK HOLES NOT SHOWN
*COVER AND FRAME MODEL INFORMATION REFER TO THE STORMWATER APPROVED PRODUCT LIST.

LEE'S SUMMIT
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STORM MANHOLE COVER DETAIL

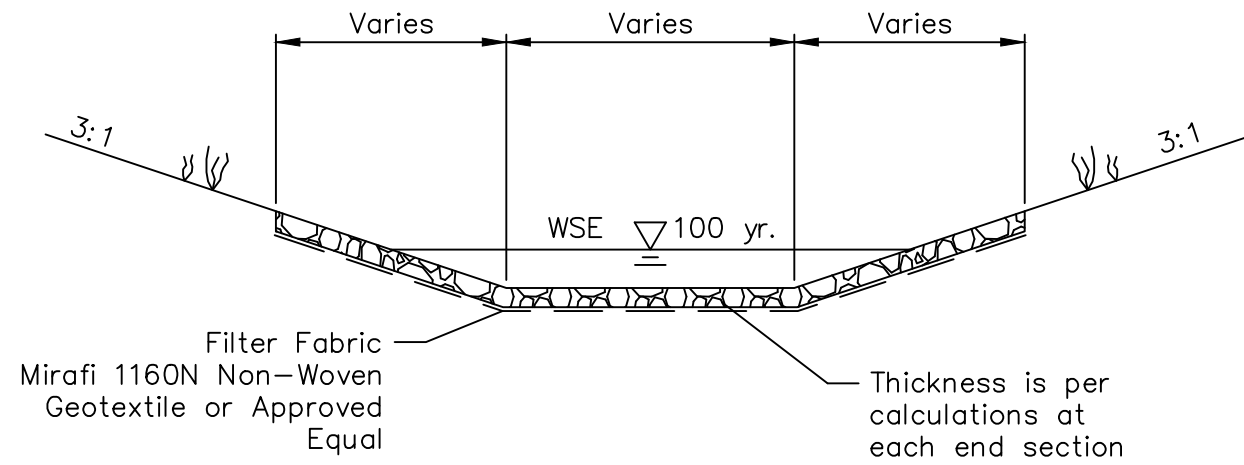
Date: 04/17
Drawn By: MIF
Checked By: DL
STM-6



*COVER AND FRAME MODEL INFORMATION REFER TO THE STORMWATER APPROVED PRODUCTS LIST.

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STORM MANHOLE FRAME DETAIL

Date: 04/17
Drawn By: MIF
Checked By: DL
STM-7

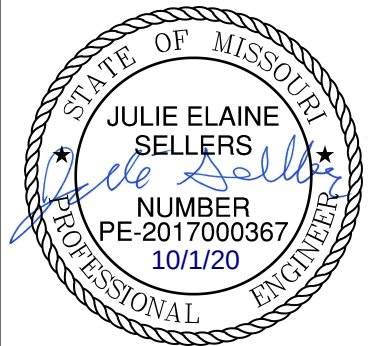


NOTE: DIMENSIONS ARE PER CALCULATIONS AT EACH END SECTION

RIPRAP DETAIL
N.T.S.

olsson

Olsson - Civil Engineering
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North Kansas City, MO 64116
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REVISIONS DESCRIPTION

DATE

REV. NO.

REVISIONS

2020

DETAILS
STREET & STORM SEWER PLANS

OSAGE
SECOND PLAT

drawn by: D.F./M.O./A.A.
checked by: C.G.W.
designed by: A.A.
QA/QC by: J.E.S.
project no.: C19-2339
drawing no.:
date: 2020.08.21

SHEET
C129

