FINAL STORMWATER STUDY

LS MIDDLE SCHOOL #4 Lee's Summit, Jackson County, Missouri

Prepared for:

Lee's Summit School District Lee's Summit, Missouri

Prepared By

Olsson 7301 W. 133rd Street, Suite 200 Overland Park, Kansas 66213



August 2020 Olsson Project No. 020-0103



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August 2020

APPENDICES

Appendix A Drainage Maps Appendix B Accompanying Documents Appendix C BMP Calculations and Information

SUMMARY

This storm drainage study is being submitted on behalf of the Lee's Summit School District for the proposed development of the property located south of Bailey Road between Dalton Drive and Ranson Road, in Lee's Summit, Jackson County, Missouri. This property is an existing terraced row crop field with no existing buildings. This preliminary report is being submitted to the City of Lee's Summit with the Preliminary Development Plans for approval of this institutional development.

1. INTRODUCTION

This final stormwater management study is being submitted on behalf of Lee's Summit School District for a development on a 51.85 acre parcel of land generally located south of Bailey Road between Dalton Drive and Ranson Road, in Lee's Summit, Jackson County, Missouri.

1.1. Project Location and Description

The proposed site is located in the northeast quarter of Section 16, Township 47 North, Range 31 West. The existing site is currently undeveloped consisting of terraced row crop. The site drains to the southeast into an unnamed tributary that runs along the entirety of the east side of the property. The tributary flows south to Big Creek. The site is not located within the flood plain.

1.2. Study Purpose

The purpose of this report is to verify this development's conformance with the City of Lee's Summit Design Criteria and Plan Requirements for Public Improvement Plans 2019 edition. Storm water drainage facilities are designed according to section F of aforementioned criteria. This study will outline methods to mitigate impacts on storm water runoff resulting from the development for the 1, 10 and 100-year rainfall events and for treatment of stormwater runoff with the use of permanent stormwater treatment facilities.

2. METHODOLOGY

2.1. General Criteria and References

Analytical and design criteria conform to those of Division V - Section 5600 – "Storm Drainage Systems and Facilities" of the Kansas City Metropolitan Chapter of the American Public Works Association's "Standard Specifications and Design Criteria". Based on these criteria's, Post-development discharge rates for the 2, 10, and 100-year storm events will be limited to provisions in section 5608.4-C1 Performance Criteria – "Comprehensive Control". Post-development discharge rates are limited to 0.5 cfs per acre for 2-Year, 2.0 cfs per acre for 10-year, and 3.0 cfs per acre for 100-year storm events.

Post development flows from the site are shown below and were calculated using HEC-HMS for the 2, 10 and 100-year storm events. Existing and proposed hydrographs were calculated using the 24-hour SCS Type II rainfall distribution. Existing times of concentration were determined using Inlet Time and Travel Time equations found in Section 5602.7 of APWA Section 5600.

2.2. Soils Description

Soil classifications by the United States Department of Agriculture (USDA) on the Natural Resources Conservation Service (NRCS) Soils website for Johnson County, Kansas show the existing site consisting of the following soil types:

10117 - Sampsel Silty Clay Loams, 5 to 9 percent slopes - HSG Type C/D

10082 - Arisburg-Urban Land Complex, 1 to 5 percent slopes - HSG Type C

*HSG – Hydrologic Soils Group (The NRCS information is included in the appendix).

3. HYDROLOGIC/HYDRAULIC ANALYSES

3.1. Existing Conditions

The property is bounded by residential lots to the west and north. Bailey Road is also located on the north side of the property. To the south and east the adjacent properties are currently undeveloped. A tributary to Big Creek, that flows to the south, lies along the entire east property line of the site. The storm drainage from the residential lots to the north comes under Bailey Road and empties into the north end of the tributary. The property currently contains no impervious area and runoff flows southeasterly to the tributary. An Exhibit of the Existing Conditions is included in Appendix A.

With the comprehensive control method is being used for drainage design, an existing curve number analysis is not required for the site.

3.2. Stream Protection and Buffer Zones

A portion of the property will be defined as a Stream Protection Buffer Zone. The proposed buffer zone will meet the requirements established Section 5603.5 of the APWA Section 5600 Design Criteria.

The width of the setback is determined by the drainage area to the stream. Using USGS contours the drainage area for the stream was determined at different points. A minor tributary from the east is the location for Point 1. The drainage area to Point 1 in 138 acres. Therefore a 60' offset from the ordinary high water mark (OHM), ie surveyed top of bank, is used to determine the buffer extent to that point. Point 2 is located where the stream exits at the southern edge of the property. The total drainage to this point is 210 acres. The setback from Point 1 to Point 2 is 100'. An exhibit of the stream setback drainage areas is included in Appendix A.

3.3. Proposed Conditions Analysis

Post development, the entirety of the of the property will continue to flow to the tributary to the east. The proposed site will include the middle school, a softball/baseball complex (with 4 fields), a track, practice fields, outdoor classrooms, parking, three extended dry detention basins, private storm, and associated utilities. Roof drains, private storm sewer pipe and inlets will allow adequate drainage of the proposed school, athletic facilities and parking areas. The private storm will drain into the detention basins and then be routed to the tributary. A private road will be constructed for access to the bus turnaround and parking areas. The road will connect to the existing Bailey Road on the north to proposed Cape Road on the south.

The proposed Cape Road will be located on the southern portion of the property. The right of way will be dedicated to the city with a portion the road being constructed to connect to existing Cape Road to the west. When the road is completely constructed a public storm system will drain to road to the tributary. Temporary provisions will be implements to drain the constructed road to the stream.

The backyards of the residential subdivisions to the west also drain onto the existing sight. This 2.5 acres will drain to the proposed north-south private road and will enter into the proposed storm system for the road.

The site will be divided into four main drainage areas. An exhibit of the Stormwater Management Plan drainage areas is included in Appendix A. Each drainage area will have collection system and an extended dry detention basin with control structure. A general description of what is draining to each area is as follows:

Drainage Area 1 (3.5 acres) - northwest ball field ball field and a portion of the common area in the ball fields

Drainage Area 2 (8.2 acres) - northeast ball field, a portion of the common areas in the

ball fields and the northeast quarter of the roof for the middle school/

Drainage Area 3 (22.6 acres) – 2 southern ball fields, the remainder of middle school

building roof, north-south drive, main parking area, bus turnaround area, the practice fields and the track.

The ball fields and the track will have synthetic surfacing with underdrain systems. The current plan is for the two north fields to have synthetic turf on the infields and outfields. The two south fields will have synthetic turf on the infields only. If the budget allows, there is a possibility that the two south fields will have also have synthetic turf in the outfields. For the purposes of this report, the ballfields will be treated as if they have a complete synthetic surface.

The synthetic surface will act as impervious surface. However, the underdrain system, with it's aggregate subsurface rock layer and underdrain piping, will increase the time of concentration (Tc) for the runoff that is guided through the underdrain system. The increase in Tc will be 32 minutes based on the final underdrain design. The HEC-HMS model based its Tc's on a time to inlet of 5 minutes and then an estimation of pipe travel time. This was used as the Tc for the impervious and pervious areas in the drainage area. The synthetic fields had an additional time of 32 minutes added to their Tc's.

Based on the completed models the peak flows for each area are included in the table below:

Site Description	Total Area (ac)	CN	Storm Event	Runoff Q (cfs)
			2-YR	5.7
Area 1	3.5	79.4	10-YR	8.6
			100-YR	12.5
			2-YR	10.9
Area 2	8.2	89.6	10-YR	25.5
			100-YR	37.5
			2-YR	38.4
Area 3	22.6	87.5	10-YR	90.7
			100-YR	133.8

Table 1. Post-Development Peak Flows

3.4. Stormwater Detention

As stated previously, a new detention basin will be constructed for each area to mitigate the increase in flow due to the increase in impervious area. A control structure will be located at the outlet of the basin. An orifice/weir plate in the control structure will limit outflow in the 2, 10, and 100-year storms.

The control structure for each basin is designed to limit the outlet flow to the allowable release rate for each storm based on the Comprehesive Control Strategy. Hydrographs for the combined flows of the detained and undetained areas are shown in Appendix D.

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To meet water treatment requirements, the basin will act as extended dry detention. The water quality volume (WQv) will be controlled by a series of 1" orifices at the bottom of the orifice plate. The conduit will release the water quality volume over a 40-hour period to allow pollutants to settle out of this precipitation event.

Description	Detention Basin
Bottom of Basin	1007.16
Total Storage Volume	0.85 ac-ft
Top of Dam Elevation	1012.20
WQv Orifice	1007.16, 6 – 1"
(IE Elevation, Pipe Size)	(ft, # hole - diam)
Water Quality Volume	1008.41, 0.18
WSE, Storage	(ft, ac-ft)
2-year & 10-Year Orifice	1008.43, 1-6"
(IE Elevation, Pipe Size)	(ft, orifice size)
10–Year Storm	1010.3, 0.5, 4.2
WSE, Storage, Peak Outflow	(ft, ac-ft, cfs)
100–Year Storm Weir	999.58, 6.0
(Elevation, Length)	(ft, lf)
100–Year Storm	1010.8, 0.6, 9.9
WSE, Storage, Peak Outflow	(ft, ac-ft, cfs)

Table 2. EDD-1 WSE's and Peak Flows

Description	Detention Basin
Bottom of Basin	997.23
Total Storage Volume	1.34 ac-ft
Top of Dam Elevation	1012.20
WQv Orifice	1001.65, 8 – 1"
(IE Elevation, Pipe Size)	(ft, # hole - diam)
Water Quality Volume	0., 0.18
WSE, Storage	(ft, ac-ft)
2-year & 10-Year Orifice	1001.73, 1-9"
(IE Elevation, Pipe Size)	(ft, orifice size)
10–Year Storm	1003.1, 0.8, 14.0
WSE, Storage, Peak Outflow	(ft, ac-ft, cfs)
100–Year Storm Weir	1001.8, 8.0
(Elevation, Length)	(ft, lf)
100–Year Storm	1003.8, 1.2, 24.0
WSE, Storage, Peak Outflow	(ft, ac-ft, cfs)

Table 3. EDD-2 WSE's and Peak Flows

Description	Detention Basin
Bottom of Basin	992.36
Total Storage Volume	0.85 ac-ft
Top of Dam Elevation	1012.20
WQv Orifice	992.36, 8 – 2"
(IE Elevation, Pipe Size)	(ft, # hole - diam)
Water Quality Volume	994.36, 0.18
WSE, Storage	(ft, ac-ft)
2-year & 10-Year Orifice	996.15, 1-15"
(IE Elevation, Pipe Size)	(ft, orifice size)
10–Year Storm	998.00
WSE, Storage, Peak Outflow	, 2.4, 38.9
	(ft, ac-ft, cfs)
100–Year Storm Weir	996.52, 14
(Elevation, Length)	(ft, lf)
100–Year Storm	999.00, 3.3, 66.8
WSE, Storage, Peak Outflow	(ft, ac-ft, cfs)

Table 4. EDD-3 WSE's and Peak Flows

Area	Drainage Area (acres)	Storm Event	Allowable Release Rate (cfs)	Design Release Rate (cfs)	Storage Required (ac-ft)	WSE (ft elev)
		2-YR	1.8	1.6	0.3	1009.7
1	3.5	10-YR	7.0	4.2	0.5	1010.3
		100-YR	10.4	9.9	0.6	1010.8
		2-YR	4.1	4.0	0.4	1001.5
2	8.2	10-YR	16.4	14.0	0.8	1003.1
		100-YR	24.5	24.0	1.2	1003.8
		2-YR	11.3	11.2	0.9	996.1
3	22.6	10-YR	45.3	38.9	2.4	998.0
		100-YR	67.9	66.8	3.3	999.0

Table 5. Detention Basin Information

4. STORM WATER TREATMENT REQUIREMENTS

As stated previously, the four detention basins will be designed to act as extended dry bottom detention facilities to treat stormwater per MARC water quality standards. The orifice plate for the basin are sized to release the water quality volume (1.37") over a 40-hour period to allow pollutants to settle from runoff before entering the public stormwater system. The size and quantities of the orifice holes are included in Tables 3-5.

5. CLEANWATER ACT SECTION 404 PERMITTING REQUIREMENTS

Construction will not be occurring within jurisdictional Waters of the United States. Therefore, a Section 404 permit is not required.

6. FEMA/DWR PERMIT REQUIREMENTS

No FEMA permitting or submittals will be required on this site because there are no FEMA delineated floodplains on the site. A copy of the FIRM map for this area has been included in Appendix B.

7. CONCLUSIONS AND RECOMMENDATIONS

As outlined in the preceding report, increased runoff rates in the post-development conditions are mitigated by the detention basin. Drainage patterns on the site remain will relatively unchanged. Lastly, four extended dry detention basins will be designed to limit site runoff to comprehensive control rates and to improve the storm water quality. Based on these facts and other information provided herein, we request approval of this stormwater study.plot

APPENDIX A Report Exhibits

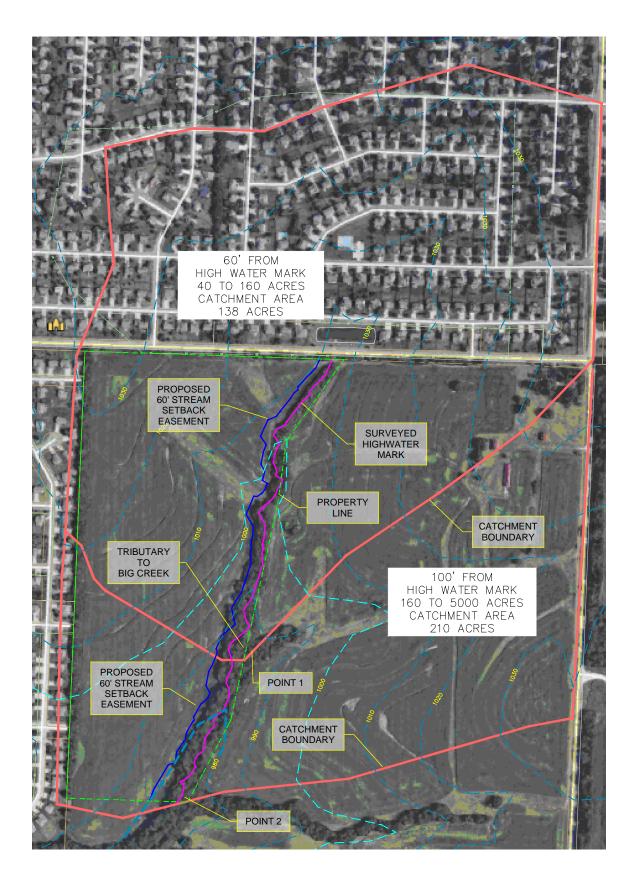
Existing Conditions

Stream Setback

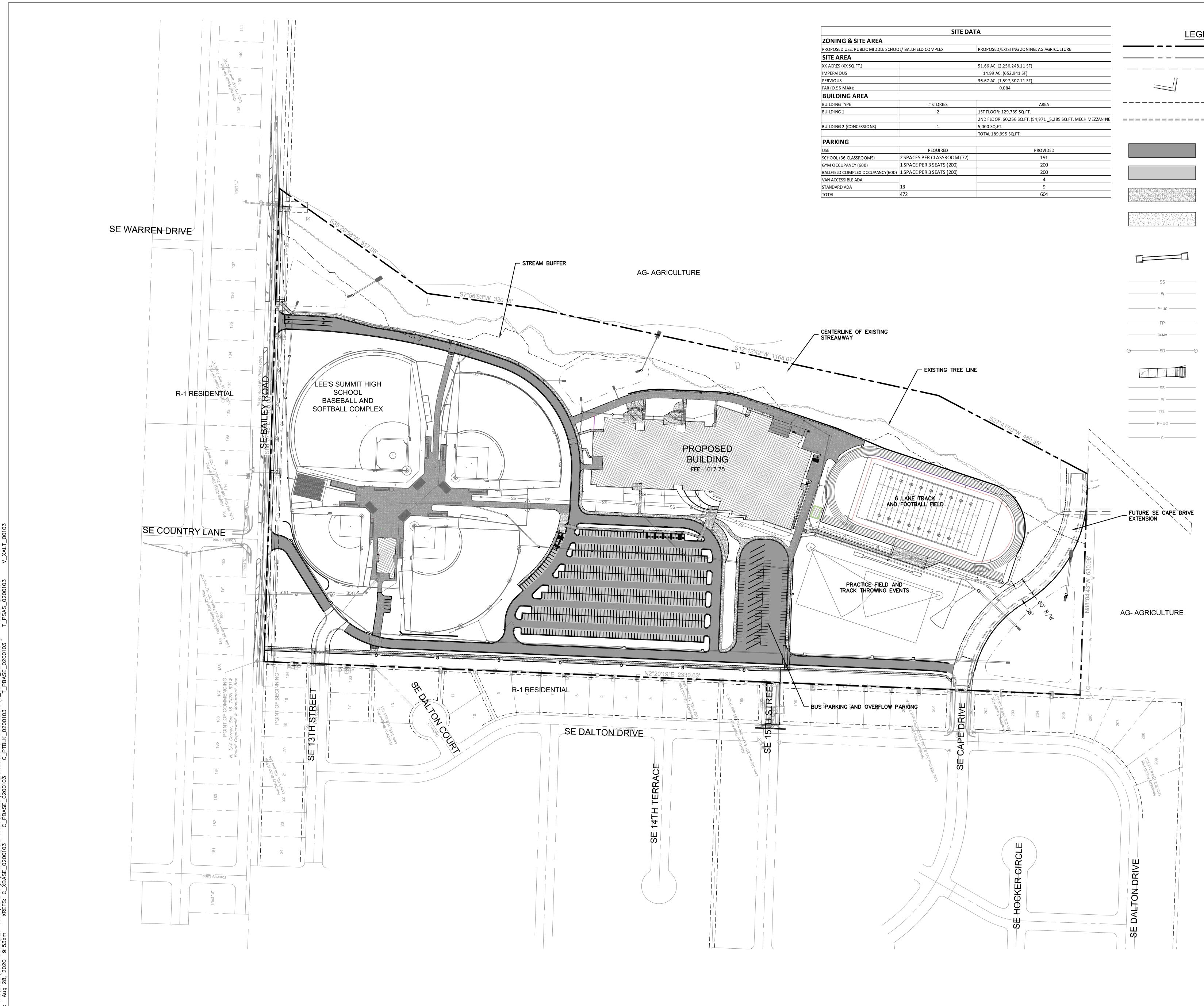
Site Plan

Stormwater Management Plan

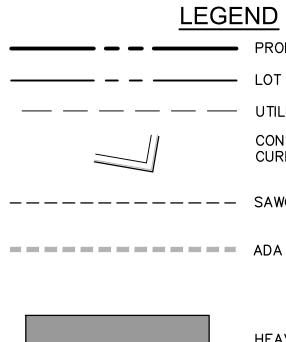
LS MIDDLE SCHOOL #4 STREAM SETBACK EXHIBIT







	SITE DA	ТА							
G & SITE AREA									
D USE: PUBLIC MIDDLE SCHOO	L/ BALLFIELD COMPLEX	PROPOSED/EXISTING ZONING: AG AGRICULTURE							
REA									
(XX SQ.FT.)		51.66 AC. (2,250,248.11 SF)							
US		14.99 AC. (652,941 SF)							
		36.67 AC. (1,597,307.11 SF)							
MAX): 0.084									
NG AREA									
ТҮРЕ	# STORIES	AREA							
1	2	1ST FLOOR: 129,739 SQ.FT.							
		2ND FLOOR: 60,256 SQ.FT. (54,971 _5,285 SQ.FT. MECH MEZZANIN							
2 (CONCESSIONS)	1	5,000 SQ.FT.							
		TOTAL 189,995 SQ.FT.							
NG									
	REQUIRED	PROVIDED							
36 CLASSROOMS)	2 SPACES PER CLASSROOM (72)	191							
JPANCY (600)	1 SPACE PER 3 SEATS (200)	200							
COMPLEX OCCUPANCY(600)	1 SPACE PER 3 SEATS (200)	200							
SSIBLE ADA		4							
) ADA	13	9							
	472	604							



 PROPERTY LINE
 LOT LINE
 UTILITY EASEMENT
CONSTRUCT CONCRETE CURB & GUTTER
 SAWCUT PAVEMENT FULL DEPTH
 ADA PATH
HEAVY DUTY ASPHALT PAVEMEN
LIGHT DUTY ASPHALT PAVEMENT
HEAVY DUTY CONCRETE PAVEME
SIDEWALK AND LANDSCAPE CONC
STORM SEWER
SANITARY SERVICE LINE

WATER SERVICE LINE

FIRE PROTECTION LINE

LANDSCAPE/ROOF DRAIN

EXISTING WATER MAIN

EXISTING ELECTRIC LINE

EXISTING GAS MAIN

Group

OLR

@2 All

AVEMENT

VEMENT

PAVEMENT

PE CONCRETE PAVEMENT

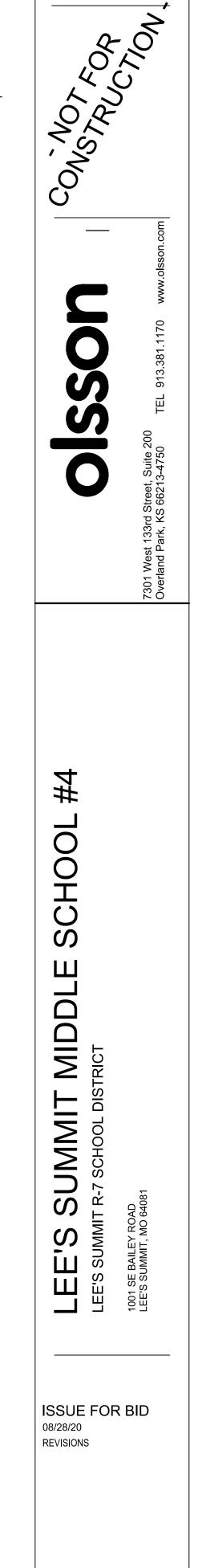
UNDERGROUND POWER SERVICE

COMMUNICATIONS SERVICE

ADA CONCRETE SIDEWALK AND RAMP

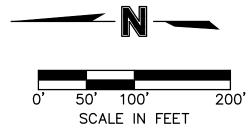
EXISTING SANITARY SEWER MAIN

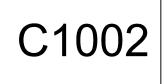
EXISTING COMMUNICATIONS LINE



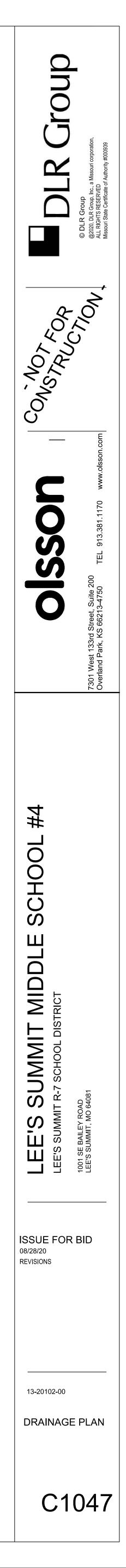
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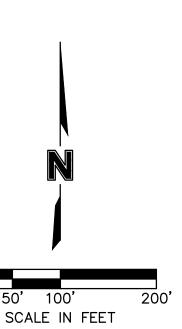
GENERAL LAYOUT PLAN



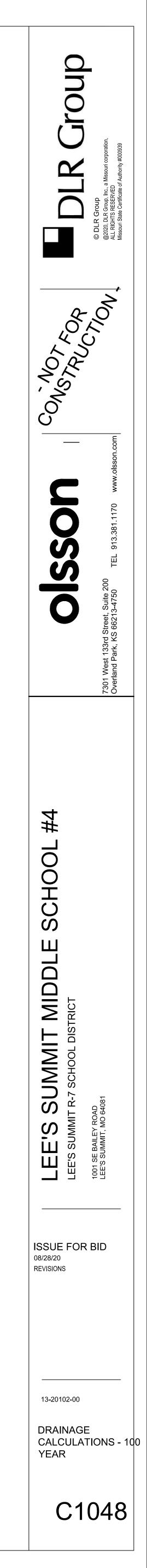








		STORM SEWER PIPE AND STRUCTURE TABLE																						
Image: interplane int		JOB	#: 020-0)103	RIVATE - 100	YFAR STO																		
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Image: start start Image: start Image: start<		F	ROM	то	AREA	AREA	_		Tc (MIN)		INTENSITY (IN/HR)		DESCRIPTION	LENGTH	SLOPE			AREA	V FULL (F/S)	DESIGN V (F/S) Hw/D	MH TOP ELEVATION			Comments
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2 0 - 0 - 0 - 0 - 0			A2				0.69 0.55	0.86 0.69	7.1 5.0		9.52 10.32	0.00	Area Inlet								1015.66			
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			C6		0.10		0.80	1.00	5.0		10.32	0.00	Area Inlet								1015.56			
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N N				C3		1.81	0.52	0.65	6.0	0.30	9.93	11.68	24 in. HDPE	128.71	1.00	24	22.68	3.14	7.22	7.26 0.95		1002.11	1000.82	
				C2			0.46	0.58 0.50	6.3 5.0		9.82	13.60 0.00	24 in. HDPE Area Inlet								1005.42			
			CC2	C1	0.50	2.97				0.17				58.52	0.50	24	16.04	3.14	5.11	5.81 1.18		999.79	999.50	
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Part Part Part Part							0.57 0.78	0.71 0.98	5.0 5.0		10.32 10.32	15.37 0.00	18 in. HDPE Area Inlet								1007.02			ADD LINE L
			L1	K1	1.33	2.27				0.08				36.25	1.00	24	22.68	3.14	7.22	7.93 1.27		999.86	999.50	
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n n				E7		2.60	0.60	0.74	6.4	0.49	9.77	18.89	24 in. HDPE	248.71	1.10	24	23.79	3.14	7.57	8.39 1.39		1012.36	1009.62	
PS C				E6			0.54 0.40	0.68 0.50	6.9 5.0		9.59	22.40 0.00	30 in. RCP Area Inlet								1016.91	1009.32		
if ·			E5		0.47		0.43	0.54	5.0		10.32	0.00	Area Inlet								1015.36			
1 1				E3			0.43	0.54 0.62	5.0 8.8		10.32 8.95	0.00 27.38	Area Inlet 36 in. RCP								1010.01			
Image: style				E2		5.45	0.49	0.61	9.3	0.28	8.81	29.44	36 in. RCP	117.67	0.50	36	47.29	7.07	6.69	7.04 0.90		999.13	998.54	
PT T 102 64 65 67 70 80 74 80 75 76 75			F 0	E1	4.00	5.76				0.56	8.72			247.97	0.50	36	47.29	7.07	6.69	7.37 1.03		999.14	997.00	
Image: Proper term Image: Properterm Image: Proper term Image:				F7		1.02	0.65	0.81	5.0	0.27	10.32	8.55	24 in. HDPE	82.70	0.50	24	16.04	3.14	5.11	5.18 0.82		1003.49	1003.08	
P5 - - - - No -			F6		0.15		0.85	1.00	5.0		10.32	0.00	Area Inlet								1006.34			
Phy Phy <td></td> <td></td> <td>F5</td> <td></td> <td>0.00</td> <td></td> <td>0.00</td> <td>0.00</td> <td>5.0</td> <td></td> <td>10.32</td> <td>0.00</td> <td>Area Inlet</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>1008.20</td> <td></td> <td></td> <td>ADD LINE G</td>			F5		0.00		0.00	0.00	5.0		10.32	0.00	Area Inlet								1008.20			ADD LINE G
N N				F3		13.67	0.51	0.64	6.1	0.40	9.89	86.16	48 in. RCP	219.96	0.50	48	101.84	12.57	8.10	9.07 1.14		998.70	997.60	
Image Image <th< td=""><td>ĒD</td><td></td><td></td><td></td><td></td><td></td><td>0.45</td><td>0.56 0.41</td><td>6.5 5.0</td><td></td><td>9.74 10.32</td><td>84.44 0.00</td><td>48 in. RCP Area Inlet</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>1004.88</td><td></td><td></td><td></td></th<>	ĒD						0.45	0.56 0.41	6.5 5.0		9.74 10.32	84.44 0.00	48 in. RCP Area Inlet								1004.88			
Image G8 U 0.26 0.30 0.30 0.30 0.30 0.30 0.32 0.30 0.3			G9	F1	0.34	17.17				0.28				154.13	0.50	48	101.84	12.57	8.10	9.07 1.14		995.64	997.12	
G7 w 0.17 v 0.84 0.69 5.0 10.12 0.00 Ace Midt v<							0.30	0.38 0.75	5.0 5.0		10.32 10.32	1.32 0.00	15 in. HDPE Area Inlet								1016.43			
G6 177 v 0.51 0.64 5.0 10132 0.00 Area het v v v v v 10138 v 1008 vo v 1008 vo			G7		0.17		0.54	0.68	5.0		10.32	0.00	Area Inlet								1014.97			
Image: Mode of the state of the st							0.51 0.43	0.64 0.54	5.0 7.0		10.32 9.56	0.00 13.00	Area Inlet 24 in. HDPE								1013.98			
Image: regione of the state 5.48 0.42 0.53 7.5 0.33 9.27 2.68 30 R.C 1.01 5.82 6.71 1.15 Image: regione of the state ADD LINE GG G3 3.78 0.28 0.01 0.02 0.03 9.27 0.03 0.02 0.01 102 0.00 Area line				G4		4.37	0.43	0.54	7.2	0.33	9.48	22.26	30 in. RCP	127.47	0.50	30	29.08	4.91	5.92	6.52 1.00		1005.02	1003.88	
G2 1.61 v 0.43 0.54 5.0 10.22 0.00 Area hele v					3.78		0.42	0.53 0.53	7.5 5.0		9.37 10.32	26.95 0.00	30 in. RCP Area Inlet								1007.65			
G1 1.61 0.43 0.43 0.54 5.0 10.32 0.00 Area Inlet 0			G2		1.61		0.43	0.54	5.0		10.32	0.00	Area Inlet								1007.45			ADD LINE H, N
G30.30.580.735.00.3810.3217.5124 in HDPE157.630.702418.983.146.046.841.291003.341002.241002.241001001001001000.241000.241000.241000.241000.241000.241000.241000.241101101101101101101101101100<			G1		1.61		0.43	0.54	5.0		10.32	0.00	Area Inlet								1007.45			
Image: Normal Series (Normal Series		(GG1	G3	2.34	2.34				0.38				157.63	0.70	24	18.98	3.14	6.04	6.84 1.29		1003.34	1002.24	
Image: Normal state			H1		0.84		0.38	0.48	5.0		10.32	0.00	Area Inlet								1010.25			
			N1		0.65		0.58	0.73	5.0			0.00	Area Inlet			18					1008.75			
				G2		0.65				0.45		4.86	15 in. HDPE	157.63	1.00	15	6.48	1.23	5.28	5.79 1.17		1003.75	1002.44	

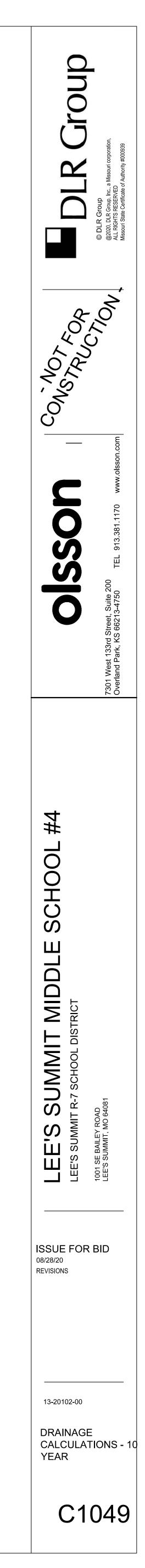


0' 50' 100' 200' SCALE IN FEET

STORM SEWER PIPE AND STRUCTURE TABLE																						
	-0103 DNDITIONS: PF I CTURES	RIVATE - 10 Y		I EVENT			ULATIONS								PIPE D	ERION						
FROM	то	DIRECT AREA (ACRES)	TOTAL AREA (ACRES)	с	KC (K=1.25)		FLOW TIME (MIN)	INTENSITY (IN/HR)	DESIGN Q (CFS)	DESCRIPTION	PIPE LENGTH (L.F.)	PIPE SLOPE (%)	PIPE DIA (IN)	Q FULL (CFS)	PIPE D PIPE AREA (SQ.FT.)	V FULL (F/S)	DESIGN V (F/S)	had D	h top Vation	UPSTREAM FLOWLINE	DOWNSTREAM FLOWLINE	Comments
A6	A5	0.35	0.35	0.90	0.90	5.0 5.0	0.87	10.32 10.32	0.00	Area Inlet 15 in. HDPE	210.61	0.50	15	4.58	1.23	3.73	4.05		015.81	1012.83	1011.78	
A5	A4	0.15	0.50	0.45 0.86	0.45 0.86	5.0 5.9	0.88	10.32 9.97	0.00 4.29	Area Inlet 18 in. HDPE	228.76	0.50	18	7.45	1.77	4.21	4.36	0.83	015.02	1011.48	1010.34	
A4 A3	A3	0.39	0.89	0.30 0.74 0.30	0.30 0.74 0.30	5.0 6.7 5.0	0.41	10.32 9.64 10.32	0.00 6.35 0.00	Area Inlet 24 in. HDPE Area Inlet	119.51	0.50	24	16.04	3.14	5.11	4.80	0.75	015.00	1010.04	1009.44	
A3	A2	0.05	1.16	0.69	0.69	7.2 5.0	0.40	9.50	7.60	24 in. HDPE Area Inlet	120.29	0.50	24	16.04	3.14	5.11	5.03	0.79	015.66	1009.14	1008.54	
C7	A1	0.15	1.21	0.69	0.69	7.6 5.0	0.11	9.36	7.81 0.00	24 in. HDPE	33.37	0.50	24	16.04	3.14	5.11	5.07		016.91	1008.24	1008.07	
C6	C6	0.10	0.15	0.85	0.85	5.0 5.0 5.0	0.48	10.32 10.32 10.32	1.32 0.00	15 in. HDPE Area Inlet	118.91	1.00	15	6.48	1.23	5.28	4.14	0.71	015.56	1011.68	1010.49	
C5	C5 C4	0.24	0.25	0.83 0.45 0.55	0.83 0.45 0.55	5.5 5.0 5.9	0.45	10.13 10.32 9.95	2.10 0.00 7.06	15 in. HDPE Area Inlet 15 in. HDPE	189.11 60.46	3.00 3.00	15 	11.22	1.23 1.23	9.14 9.14	9.65	1(010.82	1010.19	1004.52	ADD LINE CC
C4	C3	0.52	1.81	0.52 0.52	0.52	5.0 6.0	0.10	9.93 10.32 9.91	0.00	Area Inlet 24 in. HDPE	128.71	1.00	24	22.68	3.14	7.22	6.85	0.85	006.89	1004.22	1002.41	
C3 C2	C2	0.60	2.41	0.40 0.46 0.40	0.40 0.46 0.40	5.0 6.3 5.0	0.26	10.32 9.79	0.00 10.85 0.00	Area Inlet 24 in. HDPE Area Inlet	86.50	0.50	24	16.04	3.14	5.11	5.48	0.91	004.39	1000.09	1000.29	
	C1	0.50	2.97	0.40	0.40	6.6	0.17	<u> </u>	12.67	24 in. HDPE	58.52	0.50	24	16.04	3.14	5.11	5.65			999.79	999.50	
CC2	CC1	0.50	0.50	0.45 0.45 0.55	0.45 0.45 0.55	5.0 5.0	0.61	10.32 10.32	0.00 2.32 0.00	Curb Inlet 15 in. HDPE Curb Inlet	176.95	1.00	15	6.48	1.23	5.28	4.84	0.78	012.74	1007.62	1005.85	
CC1	C4	0.50	0.80	0.33	0.49	5.0 5.6	0.46	10.32 10.08	3.95	15 in. HDPE	179.24	1.50	15	7.93	1.23	6.46	6.45			1005.55	1002.86	
K3	K2	0.76	2.09	0.90	0.90	5.0 5.0	0.14	7.35 7.35	0.00 8.76	Area Inlet 18 in. HDPE	86.84	3.00	18	18.24	1.77	10.32	10.20	1.32	007.92	1002.97	1000.36	ADD LINE L
K2	K1	0.18	2.27	0.78 0.59	0.78	5.0 5.1	0.09	7.35	0.00 9.79	Area Inlet 24 in. HDPE	36.25	1.00	24	22.68	3.14	7.22	6.94	0.86	007.02	999.86	999.50	
L1	K2	1.33	1.33	0.38 0.38	0.38	5.0 5.0	0.17	7.35 7.35	0.00 3.72	Area Inlet 18 in. HDPE	90.75	3.92	18	20.85	1.77	11.80	8.90		007.92	1003.92	1000.36	
M4	 	0.28	0.28	0.52	0.52	5.0 5.0	0.40	10.32 10.32	0.00	Area Inlet 15 in. HDPE	103.39	1.00	15	6.48	1.23	5.28	4.29		006.67	1002.57	1001.54	
M3	M2	0.23	0.51	0.55 0.53	0.55 0.53	5.0 5.4	0.44	10.32 10.16	0.00 2.75	Area Inlet 15 in. HDPE	134.42	1.00	15	6.48	1.23	5.28	5.05	0.83	006.54	1001.14	999.80	
M2	M1	0.20	0.71	0.55 0.54	0.55	5.0 5.8	0.10	10.32 9.98	0.00 3.83	Area Inlet 15 in. HDPE	32.61	1.00	15	6.48	1.23	5.28	5.49		006.57	999.66	999.34	
E11	E10	0.13	0.13	0.45 0.45	0.45 0.45	5.0 5.0	1.02	7.35 7.35	0.00 0.43	Area Inlet 15 in. HDPE	212.64	1.50	15	7.93	1.23	6.46	3.47	0.67	027.14	1019.84	1016.65	
E10 E9	E9	0.54	0.67	0.41 0.42 0.48	0.41 0.42 0.48	5.0 6.0 5.0	0.43	7.35 7.05 7.35	0.00 1.97 0.00	Area Inlet 15 in. HDPE Area Inlet	138.26	1.50	15	7.93	1.23	6.46	5.37	0.75	025.31	1016.35	1014.30	
E8	E8	1.23	1.37	0.70 0.52	0.70 0.52	6.5 5.0	0.20	6.93 7.35	6.65 0.00	24 in. HDPE Area Inlet	87.11	1.50	24	27.78	3.14	8.84	7.25	0.76	019.97	1014.00	1012.69	
E7	E7 E6	0.85	2.60 3.45	0.60 0.41 0.54	0.60 0.41 0.54	6.7 5.0 7.2	0.56	6.88 7.35 6.73	10.64 0.00 12.58	24 in. HDPE Area Inlet 30 in. RCP	248.71 390.98	1.10 0.70	24 	23.79 34.41	3.14 4.91	7.57	7.35 (6.45 (1(019.91	1012.36	1009.62	
E6	E5	0.82	4.27	0.40 0.51	0.40 0.51	5.0 8.2	0.56	7.35 6.48	0.00 14.13	Area Inlet 30 in. RCP	224.24	0.70	30	34.41	4.91	7.01	6.65	0.80	016.91	1006.28	1004.71	
E5 	E4	0.47	4.74	0.43 0.50 0.43	0.43 0.50 0.43	5.0 8.8 5.0	0.63	7.35 6.34 7.35	0.00 15.10 0.00	Area Inlet 30 in. RCP Area Inlet	291.35	1.00	30	41.13	4.91	8.38	7.72	0.82	015.36	1004.44	1001.53	
E3	E3	0.55	4.90	0.50 0.42	0.50 0.42	9.4 5.0	0.54	6.20 7.35	15.18 0.00	36 in. RCP Area Inlet	192.68	0.50	36	47.29	7.07	6.69	5.95	0.73	006.10	1000.39	999.43	
E2	E2 E1	0.31	5.45 5.76	0.49 0.43 0.58	0.49 0.43 0.58	10.0 5.0 10.3	0.32	6.09 7.35 6.02	16.28 0.00 20.18	36 in. RCP Area Inlet 36 in. RCP	117.67 247.97	0.50	36 	47.29	7.07	6.69 6.69	6.06 6.41	1(004.92	999.13 999.14	998.54 997.00	
F8		1.02		0.65	0.65	5.0		7.35	0.00	Area Inlet								1(006.41			
F7	F7 F6	0.00	1.02	0.65 0.00 0.65	0.65 0.00 0.65	5.0 5.0 5.3	0.31	7.35 7.35 7.26	4.88 0.00 4.81	24 in. HDPE Area Inlet 24 in. HDPE	82.70 144.58	0.50	24 	16.04 16.04	3.14 3.14	5.11 5.11	4.47 (1(005.99	1003.49	1003.08	
F6	F5	0.15	2.50	0.85	0.85	5.0 5.8	0.30	7.20 7.35 7.10	0.00	Area Inlet 30 in. RCP	101.37	0.50	30	29.08	4.91	5.92	5.57	1(006.34	1001.76	1002.00	
F5 F4	F4	0.00	13.37	0.00 0.45 0.52	0.00 0.45 0.52	5.0 6.2 5.0	0.11	7.35 7.01 7.35	0.00 42.20 0.00	Area Inlet 48 in. RCP Area Inlet	50.65	0.50	48	101.84	12.57	8.10	7.71	0.78	008.20	1000.95	999.00	ADD LINE G
F3	F3	1.75	13.67	0.51 0.32	0.51	6.3 5.0	0.46	6.98 7.35	48.68 0.00	48 in. RCP Area Inlet	219.96	0.50	48	101.84	12.57	8.10	8.00	0.82	003.43	998.70	997.60	
F2	F2 F1	1.75	15.42 17.17	0.45 0.33 0.42	0.45 0.33 0.42	6.7 5.0 7.3	0.57	6.86 7.35 6.71	47.58 0.00 48.37	48 in. RCP Area Inlet 48 in. RCP	272.52 154.13	0.50	48	101.84 101.84	12.57 12.57	8.10 8.10	7.95		004.88	997.30 995.64	995.94 997.12	
G9		0.34		0.30	0.30	5.0		7.35	0.00	Area Inlet								1(017.00			
G8	G8 G7	0.25	0.34	0.30 0.60 0.41	0.30 0.60 0.41	5.0 5.0 6.1	1.07 0.63	7.35 7.35 7.04	0.75 0.00 1.70	15 in. HDPE Area Inlet 15 in. HDPE	176.44 131.73	0.50	15	4.58 4.58	1.23 1.23	3.73 3.73	2.76 3.46	1(016.43	1012.28	1011.40	
G7	G7 G6	0.17	0.59	0.54 0.40	0.54 0.40	5.0 6.7	0.63	7.04 7.35 6.86	0.00 2.09	Area Inlet 18 in. HDPE	127.30	0.50	15	4.58 7.45	1.23	4.21	3.61	0.71	014.97	1011.10	1010.44	
G6 G5	G5	1.77	2.53	0.51 0.43 0.62	0.51 0.43	5.0 7.3	0.28	7.35 6.71 7.35	0.00 7.30 0.00	Area Inlet 24 in. HDPE Area Inlet	152.56	2.61	24	36.65	3.14	11.66	9.08	0.78	013.98 010.41	1009.00	1005.02	
G5 G4	G4	1.84	4.37	0.62 0.43 0.62	0.62 0.43 0.62	5.0 7.6 5.0	0.37	7.35 6.64 7.35	0.00 12.47 0.00	30 in. RCP Area Inlet	127.47	0.50	30	29.08	4.91	5.92	5.69	0.77	008.10	1005.02	1003.88	
G3	G3	3.78	5.48 9.26	0.42	0.42	7.9 5.0 8.3	0.37	6.54 7.35 6.45	15.06 0.00 36.46	30 in. RCP Area Inlet 42 in RCP	132.76	0.50	30	29.08	4.91	5.92	5.97	1(007.65	1003.88	1002.92	
G2	G2 G!	1.61	9.26 10.87	0.61 0.43 0.62	0.61 0.43 0.62	8.3 5.0 9.0	0.68	6.45 7.35 6.30	36.46 0.00 42.43	42 in. RCP Area Inlet 48 in. RCP	305.13 98.07	0.50	42 48	71.33 101.84		7.41 8.10	7.44	1(007.45	1001.92 1000.19	1000.39 	ADD LINE H, N
G1	F4	1.61	12.48	0.43 0.62	0.43 0.62	5.0 9.2	0.08	7.35 6.25	0.00 48.35	Area Inlet 48 in. RCP	39.84	0.50	48	101.84		8.10	7.98	1(007.45	999.20	999.00	
GG1	G3	2.34	2.34	0.58 0.58	0.58 0.58	5.0 5.0	0.43	7.35 7.35	0.00 9.98	Area Inlet 24 in. HDPE	157.63	0.70	24	18.98	3.14	6.04	6.11		008.65	1003.34	1002.24	
H1		0.84		0.38	0.38	5.0		7.35	0.00	Area Inlet								1(010.25			
N1	G2	0.65	0.84	0.38	0.38	5.0	0.16	7.35	2.35 0.00	18 in. HDPE	81.63	5.28	18	24.20	1.77	13.70	8.67		008.75	1005.56	1001.25	
	G2	0.00	0.65		0.58	5.0	0.52	7.35	2.77	15 in. HDPE	157.63	1.00	15	6.48	1.23	5.28	5.07			1003.75	1002.44	

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ZBA XBA XBA



0' 50' 100' 200' SCALE IN FEET

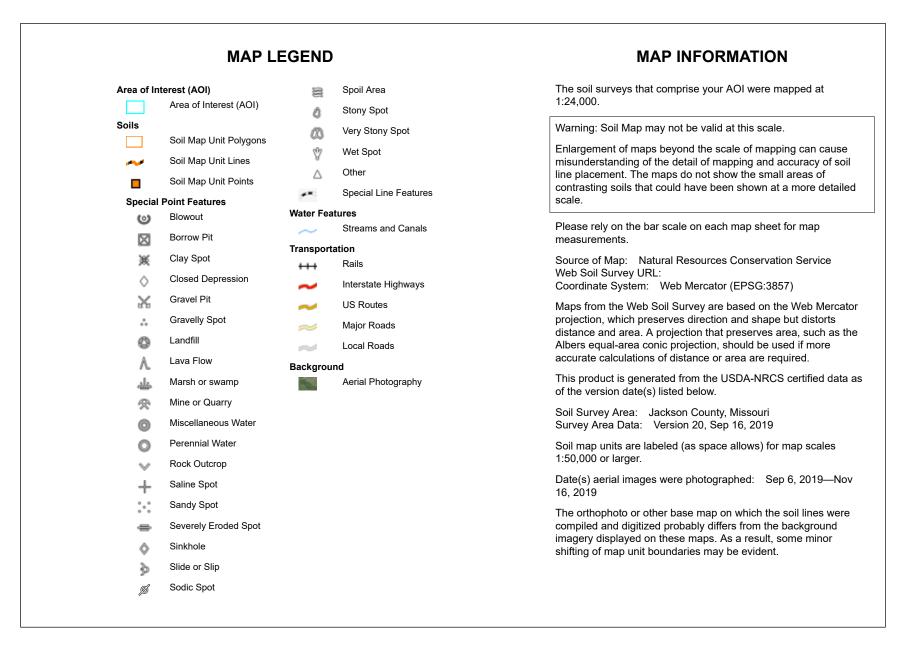
APPENDIX B Accompanying Documents

Soils Map

FEMA Firmette



Conservation Service



USDA

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI		
10082	Arisburg-Urban land complex, 1 to 5 percent slopes	28.5	61.1%		
10117	Sampsel silty clay loam, 5 to 9 percent slopes	18.1	38.9%		
Totals for Area of Interest		46.6	100.0%		



Jackson County, Missouri

10117—Sampsel silty clay loam, 5 to 9 percent slopes

Map Unit Setting

National map unit symbol: 2qkzz Elevation: 600 to 900 feet Mean annual precipitation: 33 to 41 inches Mean annual air temperature: 50 to 55 degrees F Frost-free period: 177 to 220 days Farmland classification: Prime farmland if drained

Map Unit Composition

Sampsel and similar soils: 85 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Sampsel

Setting

Landform: Hillslopes Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Concave Across-slope shape: Convex, concave Parent material: Residuum weathered from shale

Typical profile

Ap - 0 to 13 inches: silty clay loam *Bt - 13 to 80 inches:* silty clay

Properties and qualities

Slope: 5 to 9 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Somewhat poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 0 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: Moderate (about 8.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3e Hydrologic Soil Group: C/D Ecological site: Interbedded Sedimentary Upland Savanna (R109XY010MO)

USDA

Other vegetative classification: Grass/Prairie (Herbaceous Vegetation) *Hydric soil rating:* No

Data Source Information

Soil Survey Area: Jackson County, Missouri Survey Area Data: Version 20, Sep 16, 2019



Jackson County, Missouri

10082—Arisburg-Urban land complex, 1 to 5 percent slopes

Map Unit Setting

National map unit symbol: 2w7ld Elevation: 750 to 1,130 feet Mean annual precipitation: 39 to 45 inches Mean annual air temperature: 50 to 55 degrees F Frost-free period: 177 to 220 days Farmland classification: All areas are prime farmland

Map Unit Composition

Arisburg and similar soils: 61 percent
Urban land: 30 percent
Minor components: 9 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Arisburg

Setting

Landform: Interfluves Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Convex Across-slope shape: Convex Parent material: Loess

Typical profile

Ap - 0 to 6 inches: silt loam A - 6 to 13 inches: silt loam Bt - 13 to 19 inches: silty clay loam Btg - 19 to 56 inches: silty clay loam BCg - 56 to 79 inches: silty clay loam

Properties and qualities

Slope: 1 to 5 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Somewhat poorly drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: About 18 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: High (about 11.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e

USDA

Hydrologic Soil Group: C Ecological site: Loess Upland Prairie (R107BY007MO) Hydric soil rating: No

Description of Urban Land

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydric soil rating: No

Minor Components

Sampsel

Percent of map unit: 3 percent Landform: Hills Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Concave Ecological site: Interbedded Sedimentary Upland Savanna (R109XY010MO) Hydric soil rating: Yes

Greenton

Percent of map unit: 3 percent Landform: Hillslopes Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Convex Ecological site: Loess Upland Prairie (R109XY002MO) Hydric soil rating: No

Sharpsburg

Percent of map unit: 3 percent Landform: Ridges Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Linear Across-slope shape: Linear Ecological site: Loess Upland Prairie (R109XY002MO) Hydric soil rating: No

Data Source Information

Soil Survey Area: Jackson County, Missouri Survey Area Data: Version 20, Sep 16, 2019

National Flood Hazard Layer FIRMette

250

n

500

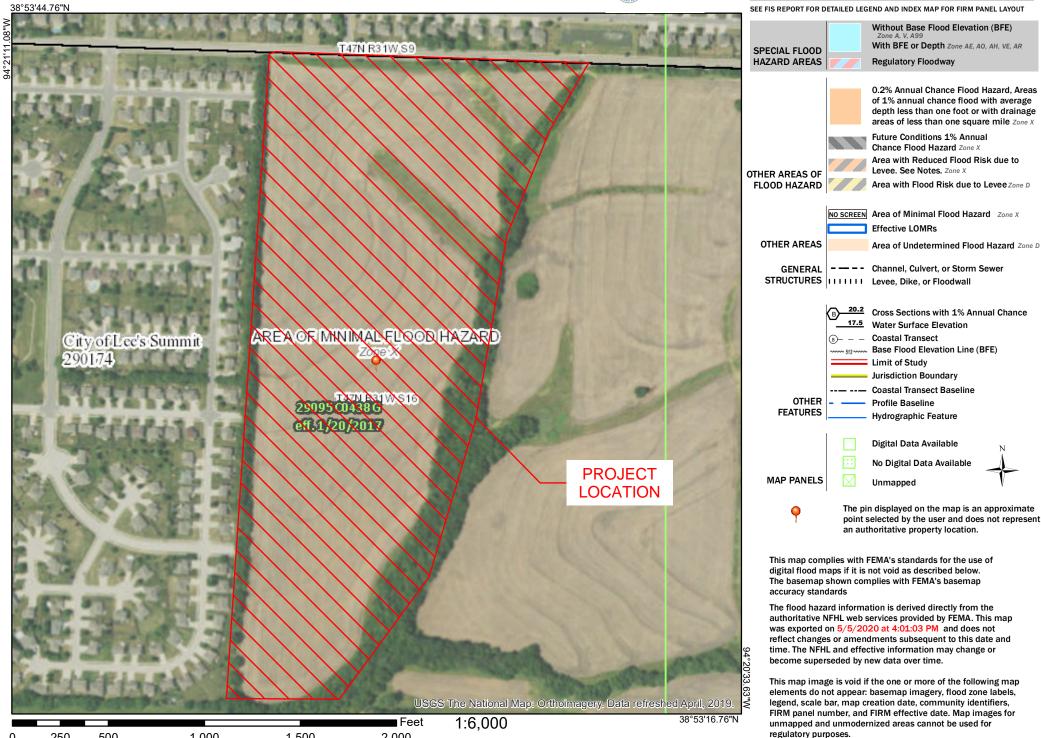
1,000

1,500

2,000

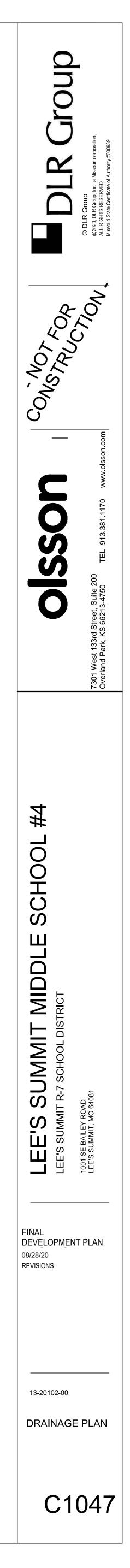


Legend



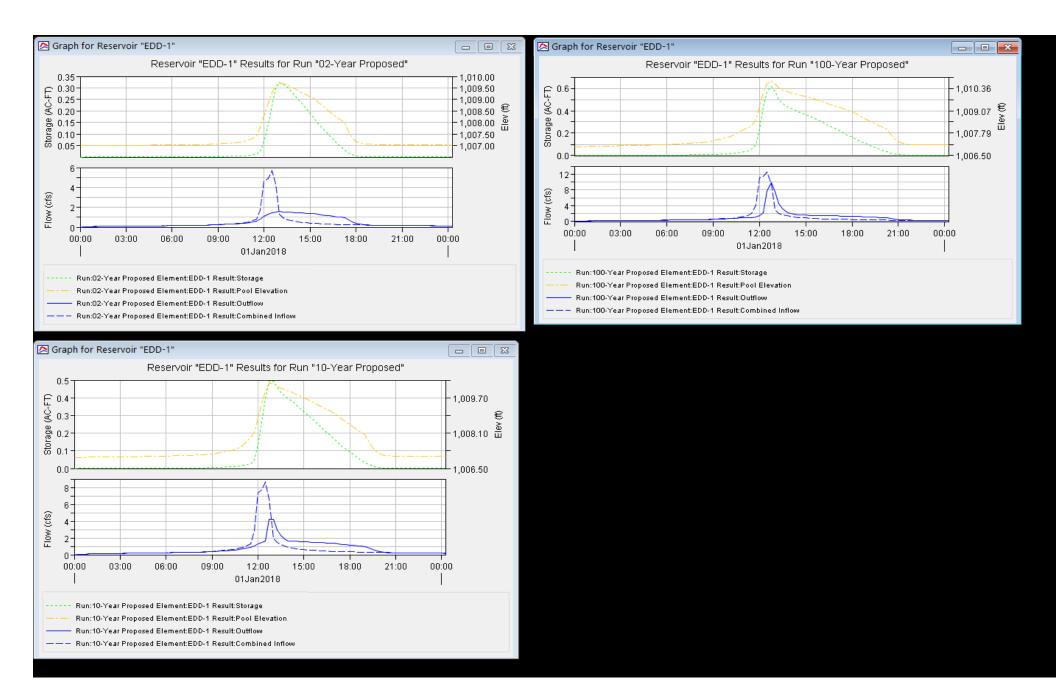
APPENDIX C Detention Calculations



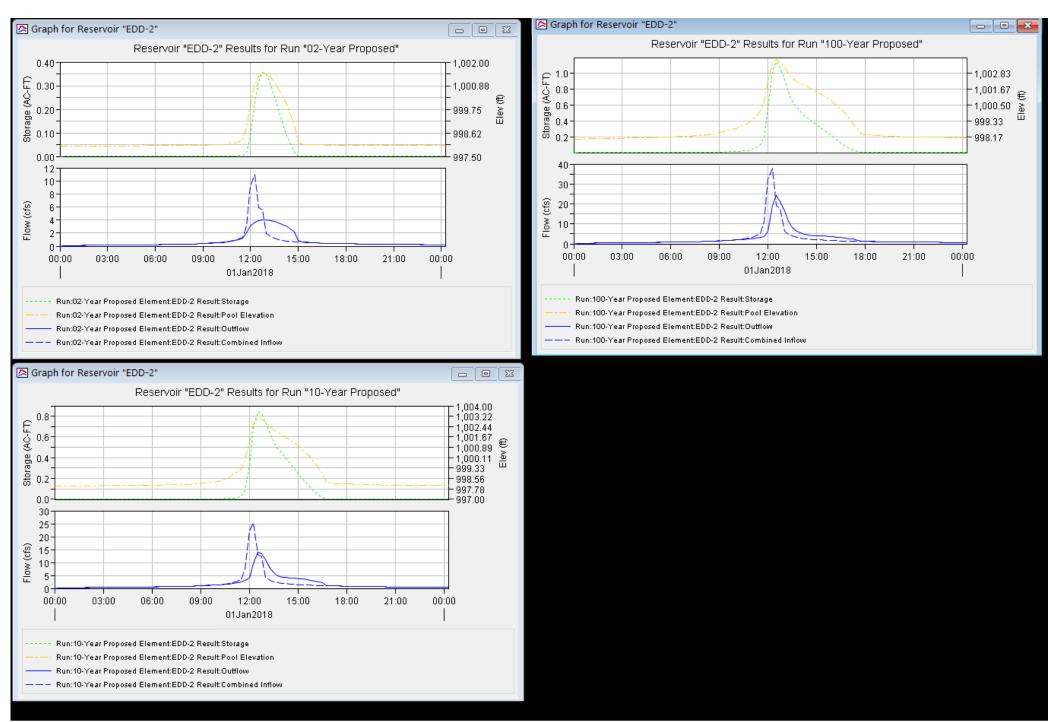


SCALE IN FEET

🔜 Summary Results for Reservoir "EDD-1"
Project: Area 1 Turf Simulation Run: 02-Year Proposed Reservoir: EDD-1
Start of Run: 01Jan2018, 00:00 Basin Model: Proposed End of Run: 02Jan2018, 00:15 Meteorologic Model: 2-Year Compute Time:01Sep2020, 11:03:49 Control Specifications:Control 1
Volume Units: IN OAC-FT
Computed Results
Peak Inflow:5.7 (CFS)Date/Time of Peak Inflow:01Jan2018, 12:30Peak Discharge:1.6 (CFS)Date/Time of Peak Discharge:01Jan2018, 13:00Inflow Volume:3.17 (IN)Peak Storage:0.3 (AC-FT)Discharge Volume:3.16 (IN)Peak Elevation:1009.7 (FT)
🔲 Summary Results for Reservoir "EDD-1"
Project: Area 1 Turf Simulation Run: 10-Year Proposed Reservoir: EDD-1
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:10-YearCompute Time:01Sep2020, 11:03:50Control Specifications:Control 1
Volume Units: IN OAC-FT
Computed Results
Peak Inflow:8.6 (CFS)Date/Time of Peak Inflow:01Jan2018, 12:30Peak Discharge:4.2 (CFS)Date/Time of Peak Discharge:01Jan2018, 13:00Inflow Volume:4.92 (IN)Peak Storage:0.5 (AC-FT)Discharge Volume:4.92 (IN)Peak Elevation:1010.3 (FT)
Summary Results for Reservoir "EDD-1"
Project: Area 1 Turf Simulation Run: 100-Year Proposed Reservoir: EDD-1
Start of Run: 01Jan2018, 00:00 Basin Model: Proposed End of Run: 02Jan2018, 00:15 Meteorologic Model: 100-Year Compute Time:01Sep2020, 11:03:50 Control Specifications:Control 1
Volume Units: IN OAC-FT
Computed Results
Peak Inflow:12.5 (CFS)Date/Time of Peak Inflow:01Jan2018, 12:30Peak Discharge:9.9 (CFS)Date/Time of Peak Discharge:01Jan2018, 12:45Inflow Volume:7.27 (IN)Peak Storage:0.6 (AC-FT)Discharge Volume:7.30 (IN)Peak Elevation:1010.8 (FT)



Summary Results for Reservoir "EDD-2"		23
Project: EDD-2 Simulation Run: 02-Year Proposed Reservoir: EDD-2		
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:2-YearCompute Time:01Sep2020, 14:29:47Control Specifications:Control 1		
Volume Units: IN OAC-FT Computed Results		
Peak Inflow: 10.9 (CFS) Date/Time of Peak Inflow: 01Jan2018, 1 Peak Discharge: 4.0 (CFS) Date/Time of Peak Discharge:01Jan2018, 1 Inflow Volume: 2.13 (IN) Peak Storage: 0.4 (AC-FT) Discharge Volume:2.12 (IN) Peak Elevation: 1001.5 (FT)		
Summary Results for Reservoir "EDD-2"		23
Project: EDD-2 Simulation Run: 10-Year Proposed Reservoir: EDD-2		
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:10-YearCompute Time:01Sep2020, 14:29:51Control Specifications:Control 1		
Volume Units: IN OAC-FT Computed Results		
Peak Inflow:25.5 (CFS)Date/Time of Peak Inflow:01Jan2018, 12Peak Discharge:14.0 (CFS)Date/Time of Peak Discharge:01Jan2018, 12Inflow Volume:4.96 (IN)Peak Storage:0.8 (AC-FT)Discharge Volume:4.98 (IN)Peak Elevation:1003.1 (FT)		
III Summary Results for Reservoir "EDD-2"		23
Project: EDD-2 Simulation Run: 100-Year Proposed Reservoir: EDD-2		
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:100-YearCompute Time:01Sep2020, 14:29:54Control Specifications:Control 1		
Volume Units: IN OAC-FT Computed Results		
Computed ResultsPeak Inflow:37.5 (CFS)Date/Time of Peak Inflow:01Jan2018, 12Peak Discharge:24.0 (CFS)Date/Time of Peak Discharge:01Jan2018, 12Inflow Volume:7.34 (IN)Peak Storage:1.2 (AC-FT)Discharge Volume:7.35 (IN)Peak Elevation:1003.8 (FT)		



Summary Results for Reservoir "EDD-3"
Project: EDD-3 Simulation Run: 2-Year Reservoir: EDD-3
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:2-YearCompute Time:01Sep2020, 15:15:43Control Specifications:Control 1
Volume Units: IN OAC-FT
Computed Results
Peak Inflow:38.4 (CFS)Date/Time of Peak Inflow:01Jan2018, 12:15Peak Discharge:11.2 (CFS)Date/Time of Peak Discharge:01Jan2018, 12:30Inflow Volume:2.05 (IN)Peak Storage:0.9 (AC-FT)Discharge Volume:2.03 (IN)Peak Elevation:996.1 (FT)
🔲 Summary Results for Reservoir "EDD-3"
Project: EDD-3 Simulation Run: 10-Year Proposed Reservoir: EDD-3
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:10-YearCompute Time:01Sep2020, 15:15:35Control Specifications:Control 1
Volume Units: IN OAC-FT Computed Results
Peak Inflow:90.7 (CFS)Date/Time of Peak Inflow:01Jan2018, 12:15Peak Discharge:38.9 (CFS)Date/Time of Peak Discharge:01Jan2018, 12:30Inflow Volume:4.87 (IN)Peak Storage:2.4 (AC-FT)Discharge Volume:4.89 (IN)Peak Elevation:998.0 (FT)
Summary Results for Reservoir "EDD-3"
Project: EDD-3 Simulation Run: 100-Year Proposed Reservoir: EDD-3
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:100-YearCompute Time:01Sep2020, 15:15:39Control Specifications:Control 1
Volume Units: IN OAC-FT Computed Results
Peak Inflow:133.8 (CFS)Date/Time of Peak Inflow:01Jan2018, 12:15Peak Discharge:66.8 (CFS)Date/Time of Peak Discharge:01Jan2018, 12:30Inflow Volume:7.24 (IN)Peak Storage:3.3 (AC-FT)Discharge Volume:7.26 (IN)Peak Elevation:999.0 (FT)

