# PRELIMINARY STORMWATER STUDY

LS MIDDLE SCHOOL #4 Lee's Summit, Jackson County, Missouri

### **Prepared for:**

Lee's Summit School District Lee's Summit, Missouri

#### **Prepared By**

Olsson 7301 W. 133<sup>rd</sup> Street, Suite 200 Overland Park, Kansas 66213



May 2020 Olsson Project No. 020-0103



# **TABLE OF CONTENTS**

Su	mmar	у	1
1.	Intro	duction	2
	1.1.	Project Location and Description	2
	1.2.	Study Purpose	2
2.	Meth	odology	2
	2.1.	General Criteria and References	2
	2.2.	Soils Description	3
3.	Hydr	ologic/Hydraulic Analyses	3
	3.1.	Existing Conditions	3
	3.2.	Stream Protection and Buffer Zones	3
	3.3.	Proposed Conditions Analysis	4
	3.4.	Stormwater Detention	6
4.	Storr	n water treatment requirements	6
5.	clear	nwater act section 404 permitting requirements	7
6.	FEM	A/DWR Permit Requirements	7
7.	Cond	clusions and Recommendations	7

# **LIST OF TABLES**

Table 1. Post-Development Peak Flows	. 5
Table 2. Detention Basin Information	. 6

# **APPENDICES**

Appendix A **Exhibits** Existing Conditions Stream Setback Site Plan Utility Plan Stormwater Management Plan Appendix B **Accompanying Documents** Soils Map FEMA Firmette Appendix C **Detention Calculations** 

# **SUMMARY**

This storm drainage study is being submitted on behalf of the Lee's Summit School District for the proposed development of the property located south of Bailey Road between Dalton Drive and Ranson Road, in Lee's Summit, Jackson County, Missouri. This property is an existing terraced row crop field with no existing buildings. This preliminary report is being submitted to the City of Lee's Summit with the Preliminary Development Plans for approval of this institutional development.

# **1. INTRODUCTION**

This final stormwater management study is being submitted on behalf of Lee's Summit School District for a development on a 51.85 acre parcel of land generally located south of Bailey Road between Dalton Drive and Ranson Road, in Lee's Summit, Jackson County, Missouri.

## **1.1. Project Location and Description**

The proposed site is located in the northeast quarter of Section 16, Township 47 North, Range 31 West. The existing site is currently undeveloped consisting of terraced row crop. The site drains to the southeast into an unnamed tributary that runs along the entirety of the east side of the property. The tributary flows south to Big Creek. The site is not located within the flood plain.

## 1.2. Study Purpose

The purpose of this report is to verify this development's conformance with the City of Lee's Summit Design Criteria and Plan Requirements for Public Improvement Plans 2019 edition. Storm water drainage facilities are designed according to section F of aforementioned criteria. This study will outline methods to mitigate impacts on storm water runoff resulting from the development for the 1, 10 and 100-year rainfall events and for treatment of stormwater runoff with the use of permanent stormwater treatment facilities.

# 2. METHODOLOGY

## 2.1. General Criteria and References

Analytical and design criteria conform to those of Division V - Section 5600 – "Storm Drainage Systems and Facilities" of the Kansas City Metropolitan Chapter of the American Public Works Association's "Standard Specifications and Design Criteria". Based on these criteria's, Post-development discharge rates for the 2, 10, and 100-year storm events will be limited to provisions in section 5608.4-C1 Performance Criteria – "Comprehensive Control". Post-development discharge rates are limited to 0.5 cfs per acre for 2-Year, 2.0 cfs per acre for 10-year, and 3.0 cfs per acre for 100-year storm events.

Post development flows from the site are shown below and were calculated using HEC-HMS for the 2, 10 and 100-year storm events. Existing and proposed hydrographs were calculated using the 24-hour SCS Type II rainfall distribution. Existing times of concentration were determined using Inlet Time and Travel Time equations found in Section 5602.7 of APWA Section 5600.

## 2.2. Soils Description

Soil classifications by the United States Department of Agriculture (USDA) on the Natural Resources Conservation Service (NRCS) Soils website for Johnson County, Kansas show the existing site consisting of the following soil types:

10117 - Sampsel Silty Clay Loams, 5 to 9 percent slopes - HSG Type C/D

10082 - Arisburg-Urban Land Complex, 1 to 5 percent slopes - HSG Type C

\*HSG – Hydrologic Soils Group (The NRCS information is included in the Appendix B).

# **3. HYDROLOGIC/HYDRAULIC ANALYSES**

## 3.1. Existing Conditions

The property is bounded by residential lots to the west and north. Bailey Road is also located on the north side of the property. To the south and east the adjacent properties are currently undeveloped. A tributary to Big Creek, that flows to the south, lies along the entire east property line of the site. The storm drainage from the residential lots to the north comes under Bailey Road and empties into the north end of the tributary. The property currently contains no impervious area and runoff flows southeasterly to the tributary. An Exhibit of the Existing Conditions is included in Appendix A.

With the comprehensive control method is being used for drainage design, an existing curve number analysis is not required for the site.

## **3.2. Stream Protection and Buffer Zones**

A portion of the property will be defined as a Stream Protection Buffer Zone. The proposed buffer zone will meet the requirements established in Section 5603.5 of the APWA Section 5600 Design Criteria.

The width of the setback is determined by the drainage area to the stream. Using USGS contours the drainage area for the stream was determined at different points. A minor tributary from the east is the location for Point 1. The drainage area to Point 1 is 138 acres. Therefore, a 60' offset from the ordinary high water mark (OHM), i.e. surveyed top of bank, is used to determine the buffer extent to that point. Point 2 is located where the stream exits at the southern edge of the property. The total drainage to this point is 210 acres. The setback from Point 1 to Point 2 is 100'. An exhibit of the stream setback drainage areas is included in Appendix A.

## **3.3. Proposed Conditions Analysis**

Post development, the entirety of the of the property will continue to flow to the tributary to the east. The proposed site will include the middle school, a softball/baseball complex (with 4 fields), a track, practice fields, outdoor classrooms, parking, three extended dry detention basins, private storm, and associated utilities. Roof drains, private storm sewer pipe and inlets will allow adequate drainage of the proposed school, athletic facilities and parking areas. The private storm will drain into the detention basins and then be routed to the tributary. A private road will be constructed for access to the bus turnaround and parking areas. The road will connect to the existing Bailey Road on the north to proposed Cape Road on the south.

The proposed Cape Road will be located on the southern portion of the property. The right of way will be dedicated to the city with a portion the road being constructed to connect to existing Cape Road to the west. When the road is completely constructed a public storm system will drain to road to the tributary. Temporary provisions will be implemented to drain the constructed road to the stream.

The backyards of the residential subdivisions to the west also drain onto the existing sight. These 2.5 acres will drain to the proposed north-south private road and will enter into the proposed storm system for the road.

The site will be divided into four main drainage areas. An exhibit of the Stormwater Management Plan drainage areas is included in Appendix A. Each drainage area will have collection system and an extended dry detention basin with control structure. A general description of what is draining to each area is as follows:

Drainage Area 1 (6.3 acres)	Two north ball fields
Drainage Area 2 (8.6 acres)	Southeast ball field and north end of middle school
Drainage Area 3 (17.6 acres)	Southwest ball field, south end of middle school, north- south drive, main parking area, bus turnaround area, and practice fields
Drainage Area 4 (4.2 acres)	Track and track infield

Approximately 1.8 acres east of the proposed building and eastern drive will flow into the tributary. Improvement in this area are minimal so no detention will be required.

The ballfields and the track will have synthetic surfacing with underdrain systems. The current plan is for the two north fields to have synthetic turf on the infields and outfields. The two south fields will have synthetic turf on the infields only. If the budget allows, there is a possibility that the

two south fields will have also have synthetic turf in the outfields. For the purposes of this report, the ballfields will be treated as if they have a complete synthetic surface.

The synthetic surface will act as impervious surface. However, the underdrain system, with its aggregate subsurface rock layer and underdrain piping, will increase the time of concentration (Tc) for the runoff that is guided through the underdrain system. The increase in Tc will be from 30 to 45 minutes based on the final underdrain design. An increase of thirty minutes will be used for the purposes of the preliminary report. The HEC-HMS model based its Tc's on a time to inlet of 5 minutes and then an estimation of pipe travel time. This was used as the Tc for the impervious and pervious areas in the drainage area. The synthetic fields had an additional time of 30 minutes added to their Tc's.

Based on the completed models the peak flows for each area are included in the table below:

Site Description	Total Area (ac)	CN	Storm Event	Runoff Q (cfs)
	2-YR	10.2		
Area 1	Area 1 6.3 79.4	10-YR	15.4	
		100-YR	22.4	
	Area 2 8.6 90.6	2-YR	12.4	
Area 2		90.6	10-YR	29.1
		100-YR	42.9	
	Area 3 17.6 90.5	Area 3 17.6 90.5 10-YR	2-YR	33.1
Area 3			10-YR	78.1
			100-YR	115.2
		2-YR	8.3	
Area 4	4.2	80.7	10-YR	13.3
			100-YR	20.4

#### Table 1. Post-Development Peak Flows

# 3.4. Stormwater Detention

As stated previously, a new detention basin will be constructed for each area to mitigate the increase in flow due to the increase in impervious area. A control structure will be located at the outlet of the basin. An orifice/weir plate in the control structure will limit outflow in the 2, 10, and 100-year storms.

The control structure for each basin is designed to limit the outlet flow to the allowable release rate for each storm based on the Comprehensive Control Strategy. Hydrographs for the combined flows of the detained and undetained areas are shown in Appendix C.

To meet water treatment requirements, the basin will act as extended dry detention. The water quality volume (WQv) will be controlled by a series of 1" orifices at the bottom of the orifice plate. The conduit will release the water quality volume over a 40-hour period to allow pollutants to settle out of this precipitation event.

Area	Drainage Area (acres)	Storm Event	Allowable Release Rate (cfs)	Design Release Rate (cfs)	Storage Required (ac-ft)	WSE (ft elev)
		2-YR	3.2	3.2	0.5	1010.4
1	6.3	10-YR	12.6	10.6	0.7	1011.1
		100-YR	18.9	18.3	0.9	1011.5
		2-YR	4.3	4.1	0.4	1001.7
2	8.6	10-YR	17.2	14.7	1.0	1003.4
		100-YR	25.8	24.9	1.4	1004.3
		2-YR	8.8	8.5	0.8	996.0
3	17.6	10-YR	35.2	35.2	2.0	997.6
		100-YR	52.8	58.1	2.9	998.6
		2-YR	2.1	2.1	0.1	997.2
4	4.2	10-YR	8.4	7.6	0.3	998.4
		100-YR	12.6	12.3	0.4	999.0

#### Table 2. Detention Basin Information

# **4. STORM WATER TREATMENT REQUIREMENTS**

As stated previously, the four detention basins will be designed to act as extended dry bottom detention facilities to treat stormwater per MARC water quality standards. The orifice plate for

the basin will be sized to release the water quality volume (1.37") over a 40-hour period to allow pollutants to settle from runoff before entering the public stormwater system.

# 5. CLEANWATER ACT SECTION 404 PERMITTING REQUIREMENTS

Construction will not be occurring within jurisdictional Waters of the United States. Therefore, a Section 404 permit is not required.

# **6. FEMA/DWR PERMIT REQUIREMENTS**

No FEMA permitting or submittals will be required on this site because there are no FEMA delineated floodplains on the site. A copy of the FIRM map for this area has been included in Appendix B.

# 7. CONCLUSIONS AND RECOMMENDATIONS

As outlined in the preceding report, increased runoff rates in the post-development conditions are mitigated by the detention basin. Drainage patterns on the site remain will relatively unchanged. Lastly, four extended dry detention basins will be designed to limit site runoff to comprehensive control rates and to improve the storm water quality. Based on these facts and other information provided herein, we request approval of this stormwater study.

# APPENDIX A Exhibits

**Existing Conditions** 

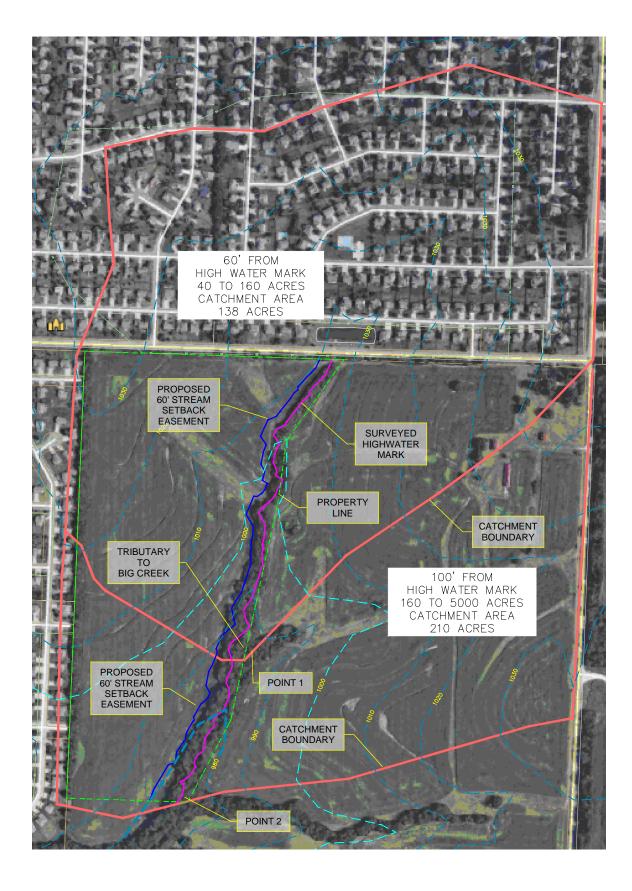
Stream Setback

Site Plan

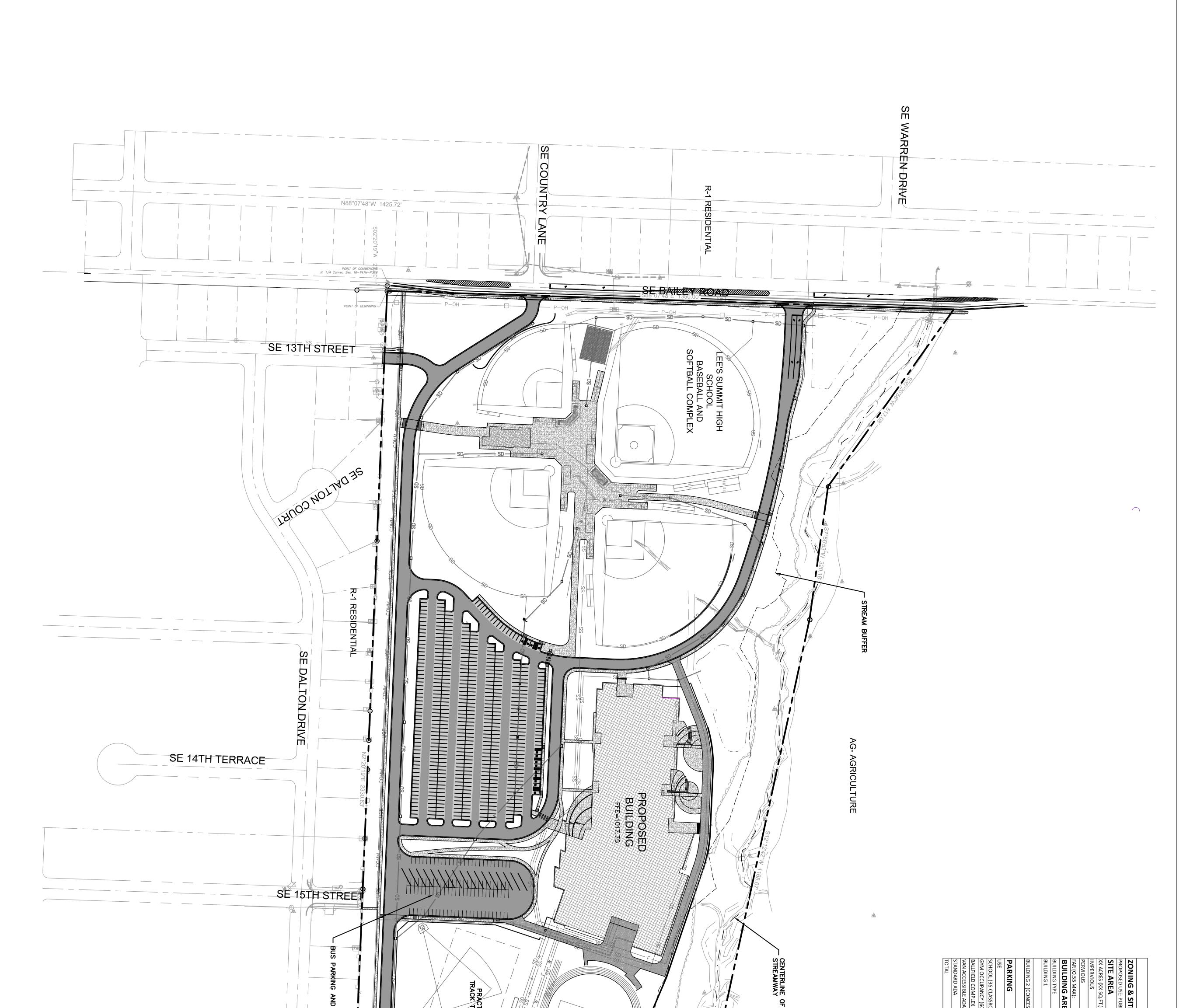
Utility Plan

Stormwater Management Plan

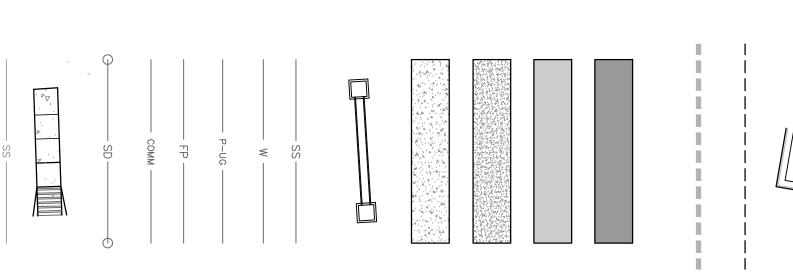
# LS MIDDLE SCHOOL #4 STREAM SETBACK EXHIBIT







	SITE DATA	TA
G & SITE AREA		
D USE: PUBLIC MIDDLE SCHOOL/ BALLFIELD COMPLEX	0L/ BALLFIELD COMPLEX	PROPOSED/EXISTING ZONING: AG AGRICULTURE
REA		
(XX SQ.FT.)		51.66 AC. (2,250,248.11 SF)
SUI		14.99 AC. (652,941 SF)
		36.67 AC. (1,597,307.11 SF)
MAX):		0.084
NG AREA		
ΤΥΡΕ	# STORIES	AREA
1	2	1ST FLOOR: 129,739 SQ.FT.
		2ND FLOOR: 60,256 SQ.FT. (54,971 _5,285 SQ.FT. MECH MEZZANINE
2 (CONCESSIONS)	1	5,000 SQ.FT.
		TOTAL 189,995 SQ.FT.
NG		
	REQUIRED	PROVIDED
36 CLASSROOMS)	2 SPACES PER CLASSROOM (72)	191
UPANCY (600)	1 SPACE PER 3 SEATS (200)	200
COMPLEX OCCUPANCY(600)	1 SPACE PER 3 SEATS (200)	200
SSIBLE ADA		4
) ADA	13	9
	472	604



LEGEND PR LOT LINE UTILITY EASEME CONSTRUCT CON CURB & GUTTEF ROPERTY LINE

XISTING

0, 50, 100, SCALE IN FEET		Concrete Pavement Storm Sewer Sanitary Service Line Water Service Line Underground Power Service Line Fire Protection Line Communications Service Landscape/Roof Drain Ada Concrete Sidewalk And Ramp Concrete Sidewalk And Existing Sanitary Sewer Main Existing Water Main Existing Communications Line Existing Electric Line Existing Gas Main	PROPERTY LINE LOT LINE UTILITY EASEMENT CONSTRUCT CONCRETE CURB & GUTTER SAWCUT PAVEMENT FULL DEPTH ADA PATH - SIDEWALKS NOT DELINEATED AS ADA PATHS WILL NOT BE ADA COMPLIANT. HEAVY DUTY ASPHALT PAVEMENT STANDARD DUTY ASPHALT PAVEMENT HEAVY DUTY CONCRETE PAVEMENT
13-20102-00 GENERAL LAYOUT PLAN 1000	TRUE DATE OF ORLAND	7301 West 133rd Street, Suite 200         Overland Park, KS 66213-4750         TEL 913.381.1170         www.olsson.com	DLR Group

PRACTICE FIELD AND ACK THROWING EVENTS

N88°04'43''W 630.96'

AG- AGRICULTURE

SE CAPE DRIVE

SE HOCKER CIRCLE

SE DALTON DRIVE

Yo

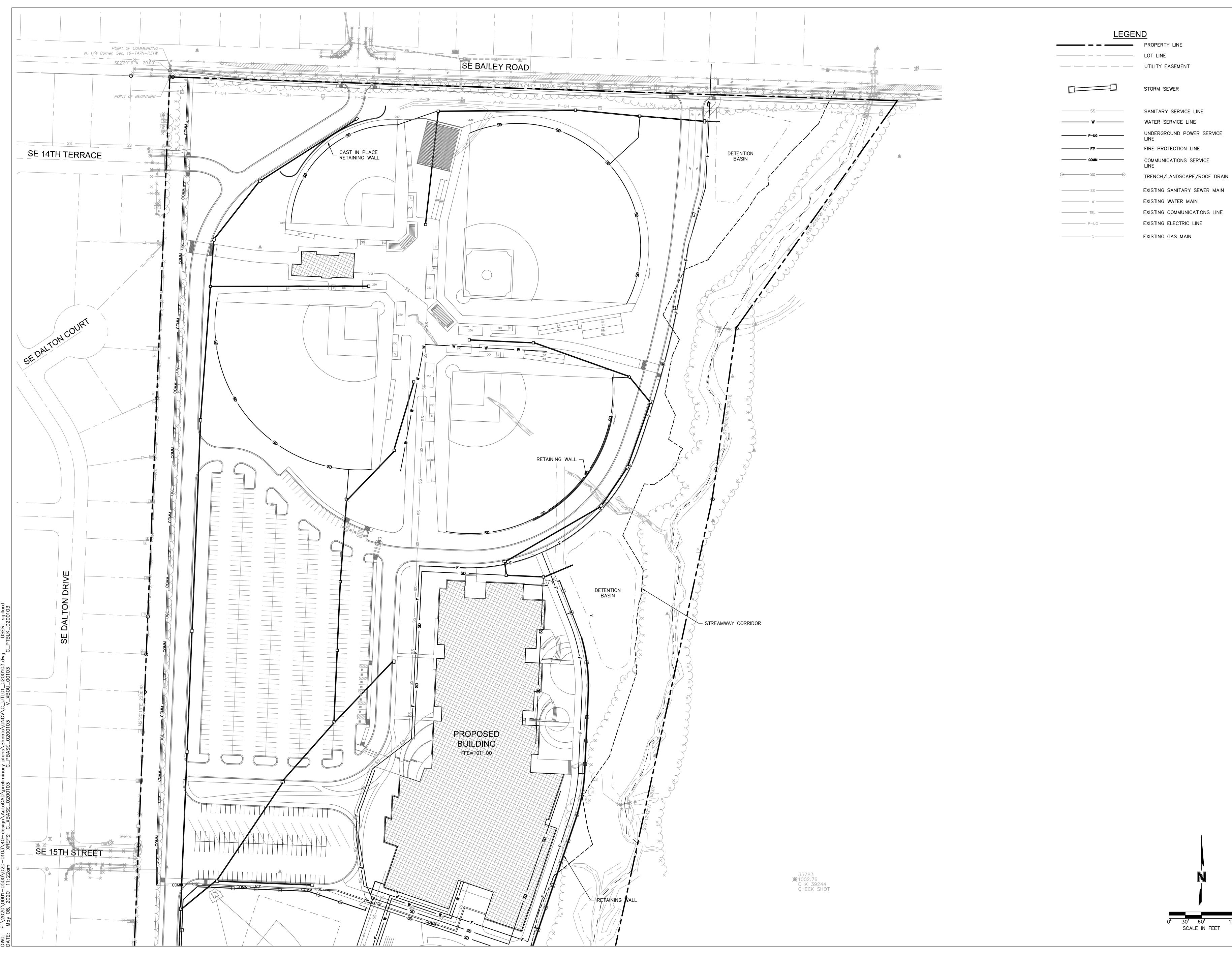
FOOTE

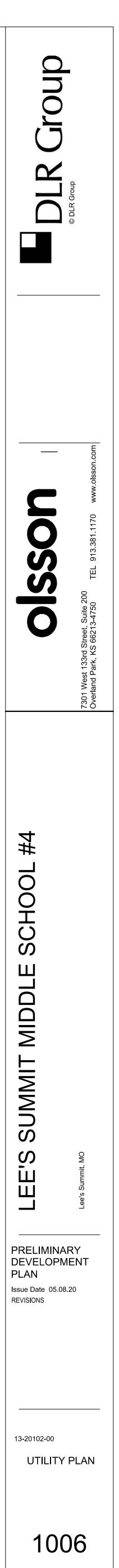
EXTENSION

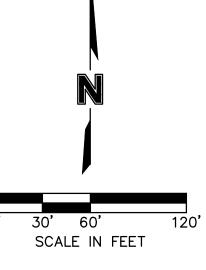
CAPE

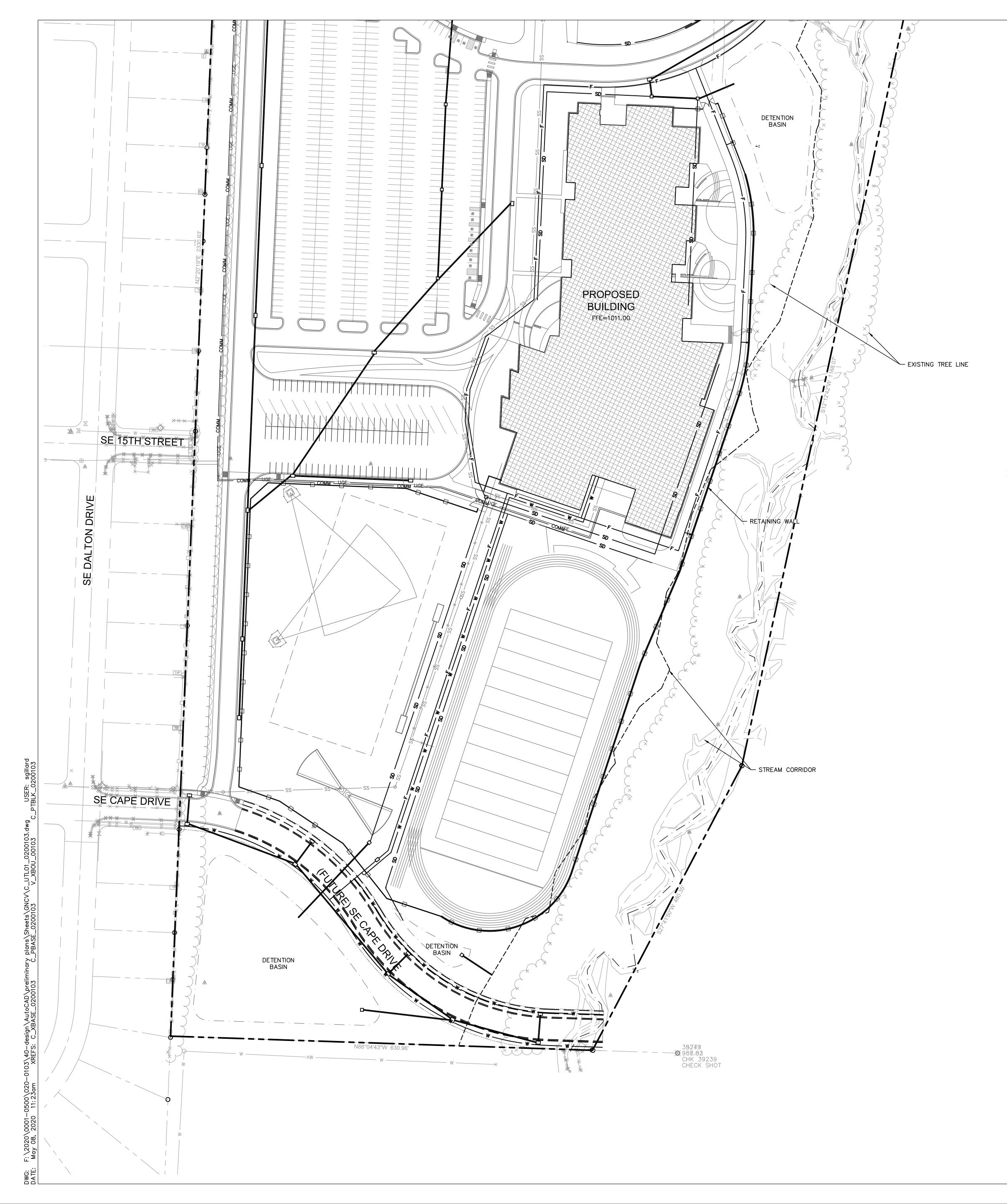
DRIVE

— — О







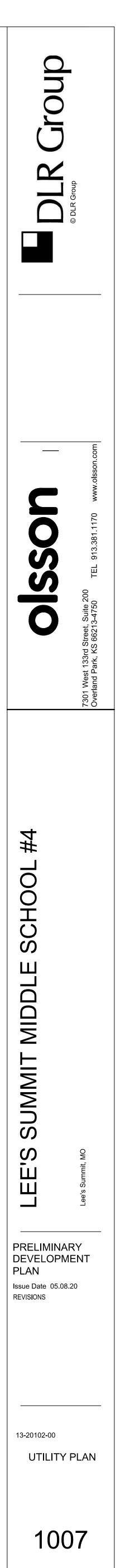


# <u>LEGEND</u>

PROPERTY LINE
LOT LINE
UTILITY EASEMENT
STORM SEWER
SANITARY SERVICE
WATER SERVICE LINI
UNDERGROUND POW LINE
FIRE PROTECTION LI
COMMUNICATIONS SE
TRENCH/LANDSCAPE
EXISTING SANITARY
EXISTING WATER MA
EXISTING COMMUNIC
EXISTING ELECTRIC L
EXISTING GAS MAIN



MENT R RVICE LINE CE LINE D POWER SERVICE TION LINE ONS SERVICE DSCAPE/ROOF DRAIN ITARY SEWER MAIN ER MAIN MUNICATIONS LINE CTRIC LINE



N 30' 60' 120 SCALE IN FEET

# LS MIDDLE SCHOOL #4

# STORMWATER MANAGEMENT PLAN



0

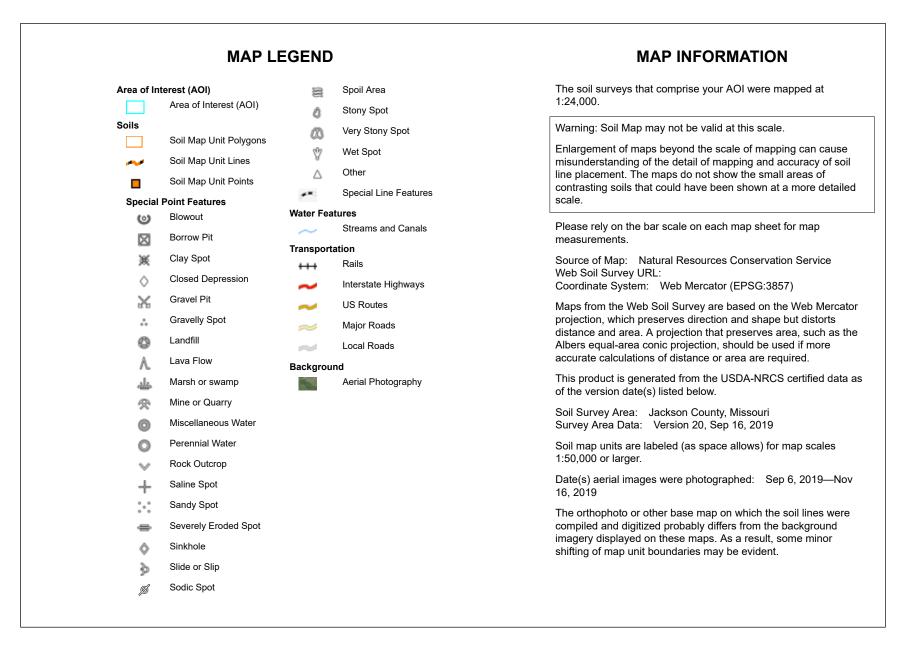
# **APPENDIX B** Accompanying Documents

Soils Map

**FEMA Firmette** 



**Conservation Service** 



USDA

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
10082	Arisburg-Urban land complex, 1 to 5 percent slopes	28.5	61.1%
10117	Sampsel silty clay loam, 5 to 9 percent slopes	18.1	38.9%
Totals for Area of Interest		46.6	100.0%



### Jackson County, Missouri

#### 10117—Sampsel silty clay loam, 5 to 9 percent slopes

#### Map Unit Setting

National map unit symbol: 2qkzz Elevation: 600 to 900 feet Mean annual precipitation: 33 to 41 inches Mean annual air temperature: 50 to 55 degrees F Frost-free period: 177 to 220 days Farmland classification: Prime farmland if drained

#### **Map Unit Composition**

Sampsel and similar soils: 85 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Sampsel**

#### Setting

Landform: Hillslopes Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Concave Across-slope shape: Convex, concave Parent material: Residuum weathered from shale

#### Typical profile

*Ap - 0 to 13 inches:* silty clay loam *Bt - 13 to 80 inches:* silty clay

#### **Properties and qualities**

Slope: 5 to 9 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Somewhat poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 0 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: Moderate (about 8.6 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3e Hydrologic Soil Group: C/D Ecological site: Interbedded Sedimentary Upland Savanna (R109XY010MO)

USDA

Other vegetative classification: Grass/Prairie (Herbaceous Vegetation) *Hydric soil rating:* No

## **Data Source Information**

Soil Survey Area: Jackson County, Missouri Survey Area Data: Version 20, Sep 16, 2019



## Jackson County, Missouri

#### 10082—Arisburg-Urban land complex, 1 to 5 percent slopes

#### Map Unit Setting

National map unit symbol: 2w7ld Elevation: 750 to 1,130 feet Mean annual precipitation: 39 to 45 inches Mean annual air temperature: 50 to 55 degrees F Frost-free period: 177 to 220 days Farmland classification: All areas are prime farmland

#### **Map Unit Composition**

Arisburg and similar soils: 61 percent
Urban land: 30 percent
Minor components: 9 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Arisburg**

#### Setting

Landform: Interfluves Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Convex Across-slope shape: Convex Parent material: Loess

#### **Typical profile**

Ap - 0 to 6 inches: silt loam A - 6 to 13 inches: silt loam Bt - 13 to 19 inches: silty clay loam Btg - 19 to 56 inches: silty clay loam BCg - 56 to 79 inches: silty clay loam

#### **Properties and qualities**

Slope: 1 to 5 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Somewhat poorly drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: About 18 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: High (about 11.5 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e

USDA

Hydrologic Soil Group: C Ecological site: Loess Upland Prairie (R107BY007MO) Hydric soil rating: No

#### **Description of Urban Land**

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydric soil rating: No

#### **Minor Components**

#### Sampsel

Percent of map unit: 3 percent Landform: Hills Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Concave Ecological site: Interbedded Sedimentary Upland Savanna (R109XY010MO) Hydric soil rating: Yes

#### Greenton

Percent of map unit: 3 percent Landform: Hillslopes Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Convex Ecological site: Loess Upland Prairie (R109XY002MO) Hydric soil rating: No

#### Sharpsburg

Percent of map unit: 3 percent Landform: Ridges Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Linear Across-slope shape: Linear Ecological site: Loess Upland Prairie (R109XY002MO) Hydric soil rating: No

## **Data Source Information**

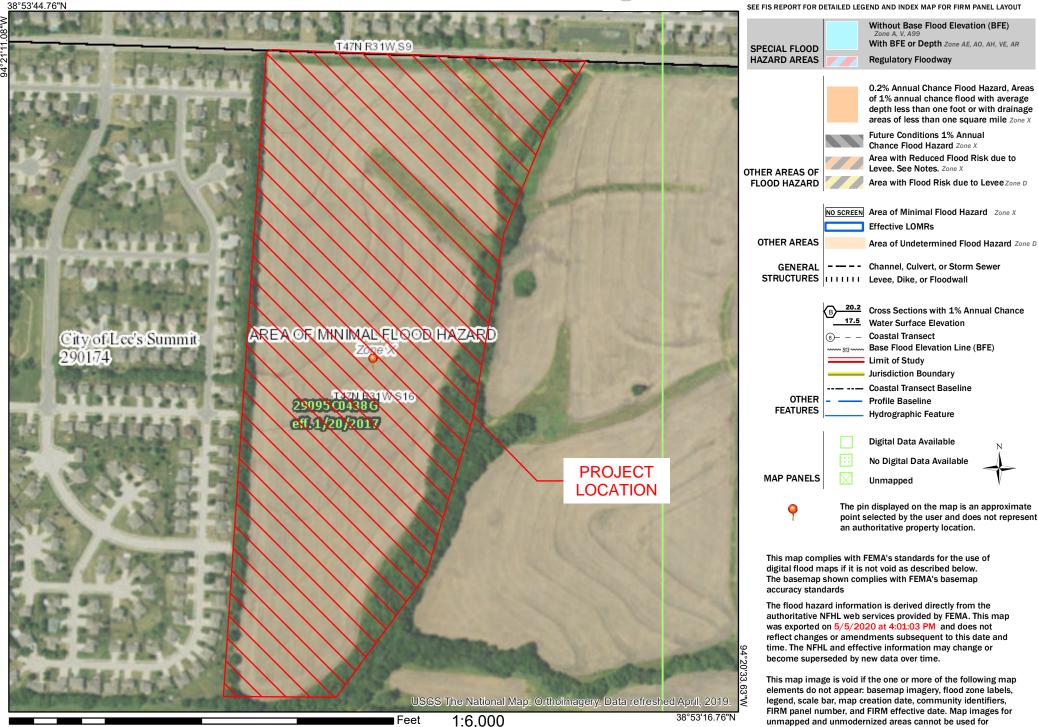
Soil Survey Area: Jackson County, Missouri Survey Area Data: Version 20, Sep 16, 2019

# National Flood Hazard Layer FIRMette



#### Legend

regulatory purposes.



250

n

500

1,000

1,500

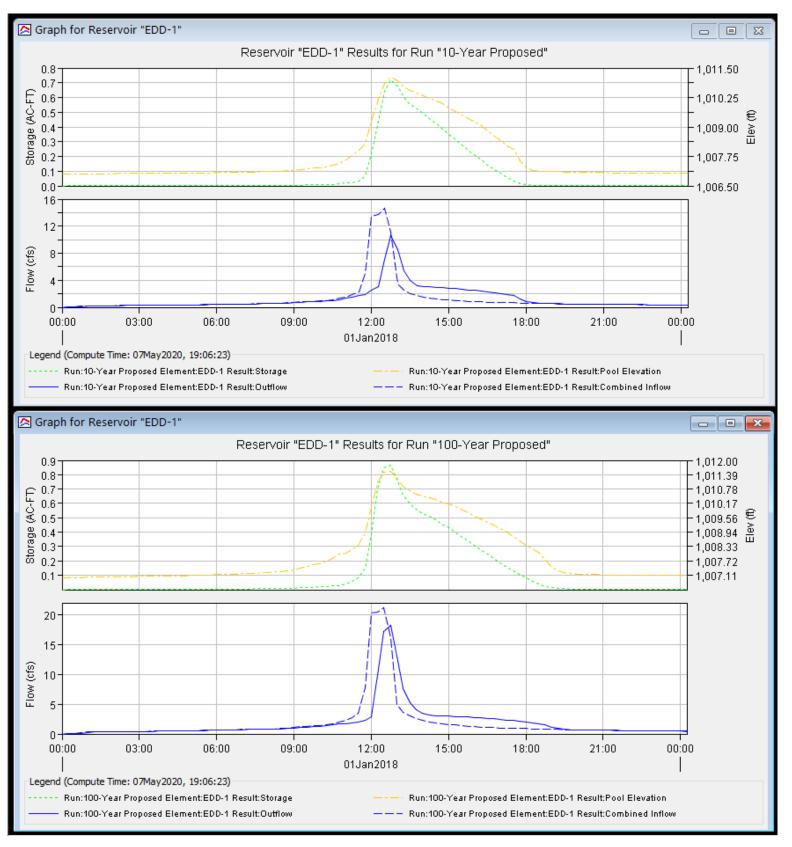
2,000

# **APPENDIX C** Detention Calculations

# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 1 HEC-HMS OUTPUT

🔜 Summary Results for Reservoir "EDD-1"	
Project: Area 1 Turf Simulation Run: 02-Year Proposed Reservoir: EDD-1	
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:2-YearCompute Time:07May2020, 19:06:22Control Specifications:Control 1	
Volume Units:      IN O AC-FT	
Computed Results         Peak Inflow:       9.6 (CFS)       Date/Time of Peak Inflow:       01Jan2018, 12:30         Peak Discharge:       3.2 (CFS)       Date/Time of Peak Discharge:01Jan2018, 13:00         Inflow Volume:       3.15 (IN)       Peak Storage:       0.5 (AC-FT)         Discharge Volume:       3.14 (IN)       Peak Elevation:       1010.4 (FT)	
Reach-1	
Summary Results for Reservoir "EDD-1"	×
Project: Area 1 Turf Simulation Run: 10-Year Proposed	
Reservoir: EDD-1 Start of Run: 01Jan2018, 00:00 Basin Model: Proposed End of Run: 02Jan2018, 00:15 Meteorologic Model: 10-Year Compute Time:07May2020, 19:06:23 Control Specifications:Control 1	
Volume Units:      IN O AC-FT Computed Results	
Peak Inflow: 14.6 (CFS) Date/Time of Peak Inflow: 01Jan2018, 12:30 Peak Discharge: 10.6 (CFS) Date/Time of Peak Discharge:01Jan2018, 12:45 Inflow Volume: 4.89 (IN) Peak Storage: 0.7 (AC-FT) Discharge Volume: 4.92 (IN) Peak Elevation: 1011.1 (FT)	
Summary Results for Reservoir "EDD-1"	
Project: Area 1 Turf Simulation Run: 100-Year Proposed Reservoir: EDD-1	
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:100-YearCompute Time:07May2020, 19:06:23Control Specifications:Control 1	
Volume Units:      IN O AC-FT	
Computed ResultsPeak Inflow:21.2 (CFS)Date/Time of Peak Inflow:01Jan2018, 12:30Peak Discharge:18.3 (CFS)Date/Time of Peak Discharge:01Jan2018, 12:45Inflow Volume:7.25 (IN)Peak Storage:0.9 (AC-FT)Discharge Volume:7.31 (IN)Peak Elevation:1011.5 (FT)	

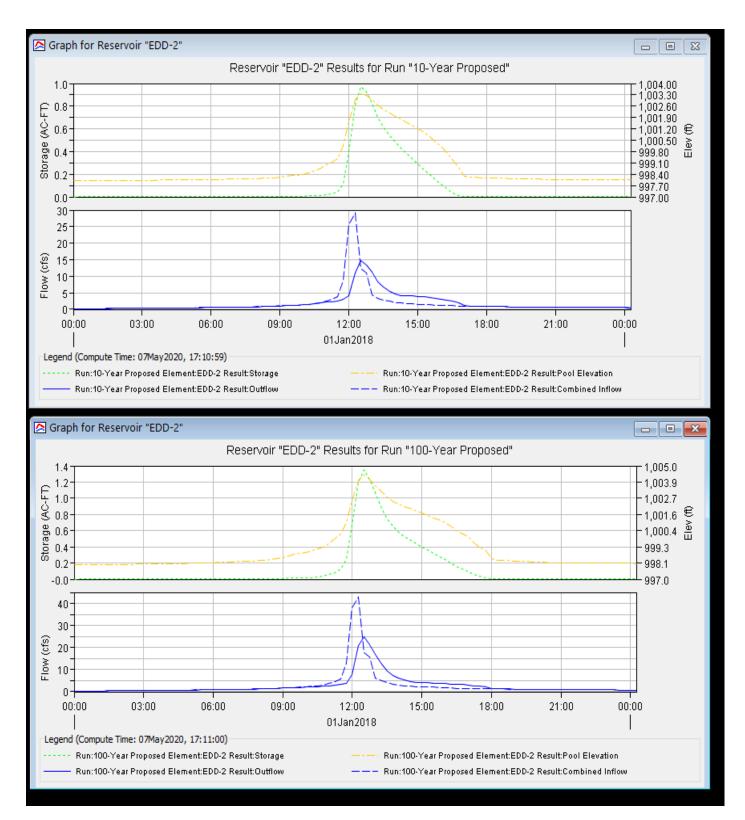
# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 1 HEC-HMS STAGE-STORAGE & INFLOW-OUTFLOW HYDROGRAPHS



# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 2 HEC-HMS OUTPUT

Summary Results for Reservoir "EDD-2"		8
Project: AREA 2 W FIELD Simulation Run: 02-Year Proposed Reservoir: EDD-2		
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:2-YearCompute Time:07May2020, 17:10:59Control Specifications:Control 1		
Volume Units: <ul> <li>IN O AC-FT</li> </ul>		
Computed Results         Peak Inflow:       12.4 (CFS)       Date/Time of Peak Inflow:       01Jan2018, 1         Peak Discharge:       4.1 (CFS)       Date/Time of Peak Discharge:01Jan2018, 1         Inflow Volume:       2.10 (IN)       Peak Storage:       0.4 (AC-FT)         Discharge Volume:       2.08 (IN)       Peak Elevation:       1001.7 (FT)		
Summary Results for Reservoir "EDD-2"		23
Project: AREA 2 W FIELD Simulation Run: 10-Year Proposed Reservoir: EDD-2		
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:10-YearCompute Time:07May2020, 17:10:59Control Specifications:Control 1		
Volume Units:  IN OAC-FT		
Computed Results         Peak Inflow:       29.1 (CFS)         Date/Time of Peak Inflow:       01Jan2018, 1         Peak Discharge:       14.7 (CFS)         Date/Time of Peak Discharge:01Jan2018, 1         Inflow Volume:       4.92 (IN)         Peak Elevation:       1003.4 (FT)		
Summary Results for Reservoir "EDD-2"		83
Project: AREA 2 W FIELD Simulation Run: 100-Year Proposed Reservoir: EDD-2		
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:100-YearCompute Time:07May2020, 17:11:00Control Specifications:Control 1		
Volume Units:      IN OAC-FT		
Computed Results	2.15	
Peak Inflow:       42.9 (CFS)       Date/Time of Peak Inflow:       01Jan2018, 1         Peak Discharge:       24.9 (CFS)       Date/Time of Peak Discharge:01Jan2018, 1         Inflow Volume:       7.30 (IN)       Peak Storage:       1.4 (AC-FT)         Discharge Volume:       7.32 (IN)       Peak Elevation:       1004.3 (FT)		

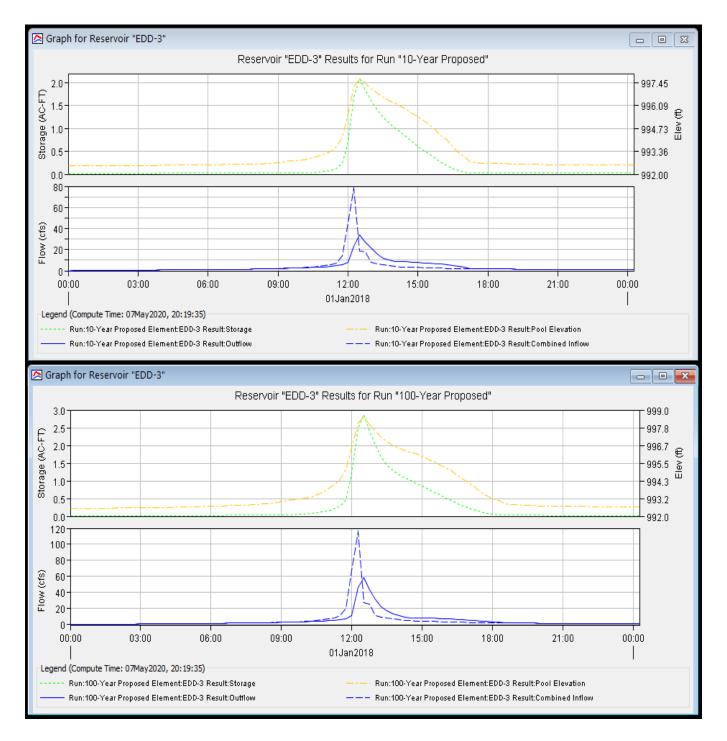
# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 2 HEC-HMS STAGE-STORAGE & INFLOW-OUTFLOW HYDROGRAPHS



# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 3 HEC-HMS OUTPUT

Summary Results for Reservoir "EDD-3"
Project: AREA 3 WITH FIELD Simulation Run: 2-Year Reservoir: EDD-3
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:2-YearCompute Time:07May2020, 20:19:36Control Specifications:Control 1
Volume Units:   IN OAC-FT
Computed Results
Peak Inflow:33.1 (CFS)Date/Time of Peak Inflow:01Jan2018, 12:15Peak Discharge:8.5 (CFS)Date/Time of Peak Discharge:01Jan2018, 12:30Inflow Volume:2.05 (IN)Peak Storage:0.8 (AC-FT)Discharge Volume:2.03 (IN)Peak Elevation:996.0 (FT)
🖽 Summary Results for Reservoir "EDD-3"
Project: AREA 3 WITH FIELD Simulation Run: 10-Year Proposed Reservoir: EDD-3
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:10-YearCompute Time:07May2020, 20:19:35Control Specifications:Control 1
Volume Units:      IN O AC-FT
Computed Results
Peak Inflow:78.1 (CFS)Date/Time of Peak Inflow:01Jan2018, 12:15Peak Discharge:34.1 (CFS)Date/Time of Peak Discharge:01Jan2018, 12:30Inflow Volume:4.86 (IN)Peak Storage:2.1 (AC-FT)Discharge Volume:4.89 (IN)Peak Elevation:997.7 (FT)
🖬 Summary Results for Reservoir "EDD-3"
Project: AREA 3 WITH FIELD Simulation Run: 100-Year Proposed Reservoir: EDD-3
Start of Run:01Jan2018, 00:00Basin Model:ProposedEnd of Run:02Jan2018, 00:15Meteorologic Model:100-YearCompute Time:07May2020, 20:19:35Control Specifications:Control 1
Volume Units:   IN OAC-FT
Computed Results
Peak Inflow:115.2 (CFS)Date/Time of Peak Inflow:01Jan2018, 12:15Peak Discharge:58.1 (CFS)Date/Time of Peak Discharge:01Jan2018, 12:30Inflow Volume:7.23 (IN)Peak Storage:2.9 (AC-FT)Discharge Volume:7.26 (IN)Peak Elevation:998.6 (FT)

# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 3 HEC-HMS STAGE-STORAGE & INFLOW-OUTFLOW HYDROGRAPHS



# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 4 HEC-HMS OUTPUT

Summary Results for Reservoir "EDD-4"	
Project: Area 4 Turf Simulation Run: 02-Year Proposed Reservoir: EDD-4	
Start of Run:01Jan2018, 00:00Basin Model:ProEnd of Run:02Jan2018, 00:15Meteorologic Model:2-YCompute Time:07May2020, 20:11:59Control Specifications:Control Spec	
Volume Units:   IN OAC-FT	
Computed Results	
Peak Inflow:3.8 (CFS)Date/Time of Peak Inflow:0.1Jan20Peak Discharge:2.1 (CFS)Date/Time of Peak Discharge:0.1Jan20Inflow Volume:1.40 (IN)Peak Storage:0.1 (AC-Discharge Volume:1.39 (IN)Peak Elevation:997.2 (F	-FT)
Summary Results for Reservoir "EDD-4"	
Project: Area 4 Turf Simulation Run: 10-Year Proposed Reservoir: EDD-4	
Volume Units:   IN OAC-FT	
Computed Results	
Peak Inflow:13.3 (CFS)Date/Time of Peak Inflow:0.1Jan20Peak Discharge:7.6 (CFS)Date/Time of Peak Discharge:01Jan20Inflow Volume:4.49 (IN)Peak Storage:0.3 (AC-Discharge Volume:4.50 (IN)Peak Elevation:998.4 (F	-FT)
Summary Results for Reservoir "EDD-4"	
Project: Area 4 Turf Simulation Run: 100-Year Proposed Reservoir: EDD-4	
	posed )-Year htrol 1
Volume Units:      IN OAC-FT	
Computed Results	
Peak Inflow:20.4 (CFS)Date/Time of Peak Inflow:01Jan20Peak Discharge:12.3 (CFS)Date/Time of Peak Discharge:01Jan20Inflow Volume:6.79 (IN)Peak Storage:0.4 (ACDischarge Volume:6.78 (IN)Peak Elevation:999.0 (F	-FT)

# LS MIDDLE SCHOOL #4 EXTENDED DRY DETENTION BASIN 4 HEC-HMS STAGE-STORAGE & INFLOW-OUTFLOW HYDROGRAPHS

