

Final Micro Storm Water Drainage Study

Sequoia, Lee's Summit

Southwest Corner of NW Olive St and NW Orchard Dr
City of Lee's Summit, Jackson County, Missouri

Created On:

May 8, 2020



Prepared by:



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GENERAL INFORMATION**A. Project Location**

The proposed Sequoia development is in the City of Lee's Summit, Jackson County, MO. The project is located on the southwest corner of NW Olive St and NW Orchard Dr and is 3.78 acres in size. The proposed location is currently 4 lots zoned for single family residential or vacant residential land. The entire site is located within the Cedar Creek Watershed. Table 1 lists the parcel information for each of the 4 proposed lots and all adjacent properties.

Table 1: Existing Lot Information

Parcel Description	Address	Parcel ID	Land Use Type
Proposed Parcel Information			
NE Corner of Proposed Lot	502 NW Olive St	61-310-05-12-00-0-00-000	1110 – Single Family Residence
NW Corner of Proposed Lot	500 NW Olive St	61-320-01-06-00-0-00-000	1101 – Vacant Residential Land
SE Corner of Proposed Lot	408 NW Olive St	61-310-06-01-00-0-00-000	1110 – Single Family Residence
SW Corner of Proposed Lot	No Address Assigned by City Lee's Summit, MO	61-320-07-01-00-0-00-000	1101-Vacant Residential Land
Adjacent Parcel Information			
N of Proposed Lot	221 NW Chipman Rd	61-320-01-02-00-0-00-000	3216 – Wholesale Trade
NE of Proposed Lot	504 NW Olive St	61-310-05-11-00-0-00-000	1110 – Single Family Residence
NE of Proposed Lot	502 NW Olive St	61-310-05-12-00-0-00-000	1110 – Single Family Residence
SE of Proposed Lot	406 NW Olive St	61-310-06-02-00-0-00-000	1110 – Single Family Residence
S of Proposed Lot	404 NW Olive St	61-310-06-03-00-0-00-000	1110 – Single Family Residence

Activities include the construction of a proposed duplex development and associated infrastructure. The proposed site will not impact downstream infrastructure because none exists. See Exhibit A for a site location map.

B. Federal Emergency Management Agency (FEMA) Classification

According to the Flood Insurance Rate Map (FIRM) panel number 29095C0417G, dated January 20, 2017, the property lies within Zone "X" (future base flood) as defined as areas having a one percent annual chance flood based on future conditions hydrology. See Exhibit B for a site location FEMA FIRM map.

C. Soil Classification

Soil classifications published by the United States Department of Agriculture/Natural Resources Conservation Service (USDA/NRCS) website for Jackson County, MO on September 13, 2018 indicate the existing site is made up of three soil types:

10082	Arisburg-Urban Land Complex, 1 to 5 percent slopes Hydraulic Soil Group (HSG) Type C
10128	Sharpsburg-Urban Land Complex, 2 to 5 percent slopes Hydraulic Soils Group (HSG) Type D
10181	Udarents -Urban Land - Sampsel, 5 to 9 percent slopes Hydraulic Soils Group (HSG) Type C

See Exhibit C for a detailed soil report.

D. Drainage Patterns

Two existing sub basins were identified at the project location. ExNW was identified as the northern drainage area with a discharge point at the northwest corner of the sub basin. The second existing sub basin was identified as ExSE with a discharge point at the southeast corner of the sub basin. One offsite drainage area was identified at the project location. ExOffsite was identified at the southwest corner of the proposed lot contributing to the ExNW sub basin. See Exhibit D for an existing drainage map.

METHODOLOGY

This study was prepared in accordance with the provisions of "Section 5600 – Storm Drainage Systems and Facilities" (February 15, 2006) of the Kansas City Metropolitan Chapter of the American Public Works Association as adopted and modified (City of Lee's Summit Section 5600, August 8, 2011) for use in storm facilities design by the City of Lee's Summit, MO. Pre and post development runoff were determined using the curve number method described in SCS (now NRCS) Technical Release No. 55 "Urban Hydrology for Small Watersheds" (2nd Edition, June 1986) as provided for in APWA Sub-section 5602.2. Pre and post undetained fringe lot drainage area runoff was determined using the Rational Method described in APWA 5602.2.A. Storm water management controls included in the post development TR55 analyses were designed to reduce peak discharges to or below pre-development values as stipulated in Sub-section 5601.5. The analyses were performed using the Type II 24-hour storm distribution for 2-year, 10-year and 100-year storm events. The rainfall depths used in the analyses corresponding to those events are shown in Table 2.

Table 2: Storm Analysis Table

Storm	Percent	Rainfall Depth (in)
2-Year	50%	3.50
10-Year	10%	5.30
100-Year	1%	7.70

EXISTING CONDITIONS ANALYSIS

Existing site drainage patterns are shown in Exhibit D – Existing Drainage Map. Exhibit D shows two on-site and one off-site drainage areas that were analyzed for existing conditions. The total drainage area of the existing site is 3.78 acres and includes 0.02 acres of offsite drainages.

The curve numbers used in the TR55 existing condition analysis are 74.0 (ExNW, >75% grass cover, good) and 83.0 (ExSE, ¼ acre lots, 38% impervious).

The existing drainage map (Exhibit D) identifies each sub basin discharge point and related area shown in Table 3 below. The existing conditions model results have been provided in Exhibit E. The time of concentration determined for each sub basin is shown in Table 4. The sub basin discharge for the three storm events investigated are shown in Table 5 and summarized in Table 6.

Comprehensive control was used in accordance with APWA 5608.4 to determine maximum release rates for each post development sub basin. This allows for a maximum discharge (cfs/acre) for 2-yr, 10-yr, and 100-yr storm events. The single off-site drainage contributor was documented with the existing conditions analysis. The sub basin allowable release rates for the three storm events investigated are shown in Table 7.

Table 3. Existing Discharge Points

Outfall	Direction
ExNW	Flow travels across the lot from east (NW Olive St) to west (Railroad ROW). Runoff that is discharged across the western property line is conveyed to the NW corner parallel to the railroad.
ExSW	Flow travels across the lot from north to south parallel to NW Olive St. Runoff is discharged in the SE corner of the sub basin.
ExOffsite	Flat portion of SW corner along the railroad ROW draining into ExNW. Discharge conveyed to ExNW Discharge Point A.

Table 4. Existing Time of Concentration Calculations

Sub Basin	Overland Flow	Shallow Concentrated Flow	Channel Flow	T _c (Min.)
ExNW	Length=100 ft Slope=2.8% N Value=0.30	Length= 380 ft Slope= 3.0% Short Grass Pasture	Length= n/a Slope= n/a Cross Section Area= n/a Wetted Perimeter= n/a	19.49
ExSE	Length=100 ft Slope=3.0% N Value=0.30	Length=150 ft Slope=3.70% Short Grass Pasture	Length= n/a Slope= n/a Cross Section Area= n/a Wetted Perimeter= n/a	15.72
ExOffsite	Length= 10 ft Slope= 0.1% N Value= 0.30	Length = n/a Slope = n/a	Length= n/a Slope= n/a Cross Section Area= n/a Wetted Perimeter= n/a	8.57

Table 5: Existing Site Hydrology and Flows

Sub Basin	Discharge Point	Outfall	Outfall Type	Area (Ac.)	T _c (min)	CN Value	Q ₂ (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)
ExNW	A	NW	Low Point	2.74	19.49	74.00	3.67	7.95	14.21
ExSE	B	SE	Low Point	1.04	15.72	83.00	2.37	4.34	7.04
ExOffsite	A	NW	Low Point	0.02	8.57	74.00	0.03	0.07	0.13

Table 6: Total Outflow Summary

Sub Basin	Q ₂ (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)
ExNW	3.67	7.95	14.21
ExSE	2.37	4.34	7.04
ExOffsite	0.03	0.07	0.13

Table 7: Allowable Release Rates per Existing Discharge Point

Sub Basin	Q ₂ (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)
ExNW	1.37	5.48	8.22
ExSE	0.52	2.08	3.12

PROPOSED CONDITIONS ANALYSIS

The overall drainage pattern for the proposed condition has been updated to four sub basins with three discharge points. See Exhibit F for a proposed drainage map. The development will not add any area to the existing 3.78 acres, but the area of each sub basin has changed.

The curve number used for the proposed site was 90.0 (1/8 acre lots, 65% impervious). HSG C was assumed for the curve number calculations.

The proposed drainage map (Exhibit F) identifies the sub basin discharge points and related area shown in Table 8 below. The proposed conditions model results have been provided in Exhibit G. The time of concentration assumptions for each sub basin are shown in Table 9. The sub basin discharge for the three storm events investigated are shown in Table 10 and summarized in Table 11. The sub basin allowable release rates for the three storm events investigated are shown in Table 12.

Table 8. Proposed Discharge Points

Outfall	Direction
Northwest (ProNW)	Runoff is conveyed W through a swale to the proposed NW detention basin. Basin outlets discharge point A at the NW property corner.
South (ProS)	Runoff is conveyed from proposed cul-de-sac curb, gutter, and enclosed storm sewer to proposed S detention basin. Basin outlets at discharge point C along the W property line.
Southeast (ProSE)	Runoff is conveyed SE across the ProSE sub basin to an existing roadway ditch and discharge point B in the SE corner of the proposed lot.
West (ProW)	Runoff is conveyed across the ProW sub basin to a discharge point along the W property line of the proposed lot.

Table 9. Proposed Time of Concentration Calculations

Sub Basin	Overland Flow	Shallow Concentrated Flow	Channel Flow	T _c (Min.)
ProNW	Length= 40 ft Slope= 2.0% N Value= 0.30	Length= 110 ft Slope= 2.1% Short Grass	Length= 310 ft Slope= 2.0% Cross Section Area= 15 ft ² Wetted Perimeter= 15 ft	10.09
ProS	Length= 50 ft Slope= 2.0% N Value= 0.30	Length= 175 ft Slope= 2.0% Paved	Length= 125 ft Slope= 1.5% Cross Section Area= 1.33 ft ² Wetted Perimeter= 3.0 ft	10.62
ProSE	Length= 70 ft Slope= 2.5% N Value= 0.30	Length= 110 ft Slope= 3.5% Short Grass	Length= n/a Slope= n/a Cross Section Area= n/a Wetted Perimeter= n/a	12.61
ProW	Length= 29 ft Slope= 4.0% N Value= 0.30	Length= 105 ft Slope= 7.0% Short Grass	Length= n/a Slope= n/a Cross Section Area= n/a Wetted Perimeter= n/a	5.54

Table 10: Proposed Site Hydrology and Flows

Sub Basin	Discharge Point	Outfall Type	Area (Ac.)	T _c (min)	CN	Q ₂ (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)
ProNW	A	Railroad ROW	0.66	10.09	90.00	2.19	3.63	5.53
ProS	C	Railroad ROW	1.98	10.62	90.00	6.48	10.75	16.38
ProSE	B	Un-Detained Discharge	0.58	12.61	90.00	1.83	3.03	4.62
ProW	C	Un-Detained Discharge	0.56	5.54	90.00	2.11	3.50	5.33

Table 11: Total Outflow Summary

Sub Basin	Q ₂ (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)
ProNW	2.19	3.66	5.53
ProS	6.48	10.75	16.38
ProSE	1.83	3.03	4.62
ProW	2.11	3.50	5.33

Table 12: Comprehensive Control Allowable Release Rates

Sub Basin	Q ₂ (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)
ProNW	0.33	1.32	1.98
ProS	0.99	3.96	5.94

DETENTION ANALYSIS

Detention analysis was completed according to APWA Section 5608: Stormwater Detention and Retention. The proposed detention analysis was completed per APWA 5608.4.C.1.a (pg 92) which allows a maximum peak discharge rate of 0.5 (2-yr), 2.0 (10-yr), and 3.0 (100-yr) cfs/acre for any development under runoff control strategies. Criteria from APWA 5608.4.C.1.b (pg 92) was also applied to ensure 40-hour extended detention of runoff for local 90% mean annual event. (1.37"/24-hour rainfall)

All outflow conditions assume free flow. All downstream pipes of the detention basin will be sized using manning's equation to carry the 100-year flow condition to site development. To mitigate this, we are proposing two detention basins on-site.

The proposed on-site detention consists of two above ground extended dry detention basins (EDDB) which accommodate wet detention for a 40-hour extended period. The extended detention pool depth will not exceed 18" for either detention basin. Native vegetation will be planted in the basin to increase infiltration rates and discharge the extended detention pool.

The proposed northwest basin (ProNW) will have an invert elevation of 1007.00', a top of dam of 1012.80', and a 100-year HGL of 1008.87'. The total volume of the storage basin at the 100-year HGL is 0.19 acre-feet. Runoff is to be conveyed through 1-8" HDPE Pipe (invert = 1008.00'). The 50-linear foot 8" HDPE pipe will be built at a 1.00% slope. Runoff from the outfall pipe will daylight on the existing property (Invert = 1007.50') and flow towards railroad right-of-way.

The emergency overflow structure consists of a 90' wide naturally graded trapezoidal weir at an elevation of 1011.10'. A minimum of 0.50' of freeboard is required between the emergency spillway crest and the maximum 100-year. For the 100-year maximum water surface elevation of 1008.87' the total provided freeboard is 2.23'.

The proposed south basin (ProS) will have an invert elevation of 1012.00', a top of dam of 1018.50', and a 100-year HGL of 1015.30'. The total volume of the storage basin at the 100-year HGL is 0.52 acre-feet. Runoff is to be conveyed through 1-12" HDPE Pipe (invert = 1013.55'). The 42.5-linear foot 12" pipe will be built at a 3.65% slope. Runoff from the outfall pipe will daylight on the existing property (Invert = 1012.00') and flow towards railroad right-of-way.

The emergency overflow structure consists of a 50' wide naturally graded trapezoidal weir at an elevation of 1017.00'. A minimum of 0.50' of freeboard is required between the emergency spillway crest and the maximum 100-year WSE. For the 100-year maximum water surface elevation of 1015.53' the total provided freeboard is 1.47'.

Please see Table 13 below for a summary of pipe velocities during 2, 10, and 100-year storms, Table 14 for a detention basin inflow/outflow summary, Table 15 for a detention basin summary, and Table 16 for an APWA 5608 peak discharge requirement summary.

Table 13: Summary of Pipe Velocities

Pipe	V ₂ (fps)	V ₁₀ (fps)	V ₁₀₀ (fps)
Proposed NW Detention Basin			
8" HDPE	0.29	1.67	3.60
Proposed S Detention Basin			
12" HDPE	1.22	4.00	5.99

Table 14: Detention Basin Inflow/Outflow Summary

Storm Event	Q _{in} (cfs)	Ponding Elevation (ft)	Max Depth Attained (ft)	Q _{out} (cfs)
Proposed NW Pond				
100- Year	5.43	1008.87	1.87	1.26
10-Year	3.57	1008.47	1.47	0.58
2-Year	2.16	1008.15	1.15	0.10
Proposed S Pond				
100- Year	16.21	1015.53	3.53	4.70
10-Year	10.67	1014.71	2.71	3.14
2-Year	6.45	1014.05	2.05	0.96

Table 15: Summary of Detention Basin Design

Proposed NW Detention Basin	
Drainage Area	0.66 AC
Curve Number	90.00
Basin Flow Line Outfall	1008.50'
Pond Base Elevation	1007.00'
Outlet Structure	1 – 8" HDPE Pipes @ 1008.65'
Max 100-year HGL	1008.87'
100-Year Emergency Weir Elevation	1011.30'
Top of Dam	1012.80'
Proposed S Detention Basin	
Drainage Area	1.98 AC
Curve Number	90.00
Basin Flow Line Outfall	1013.50'
Pond Base Elevation	1012.00'
Outlet Structure	1 – 12" HDPE Pipe @ 1013.50'
Max 100-year HGL	1015.53'
100-Year Emergency Weir Elevation	1017.00'
Top of Dam	1018.50'

Table 16. Summary of APWA 5608 Peak Discharge Requirements

Outfall Desc.	Q ₂ (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)
ProNW Allowable	0.33	1.32	1.98
ProNW Actual	0.10	0.58	1.26
Difference	-0.23	-0.74	-0.72
ProS Allowable	0.99	3.96	5.94
ProS Actual	0.96	3.14	4.70
Difference	-0.03	-0.82	-1.24

APWA Section 5608.4.F.2 requires that the detention basin emergency spillway performance provides a minimum of 1.0 ft of freeboard from the design stage to the top of dam, assuming zero available storage in the basin and zero flow through the primary outlet. (100% clogged condition) FHWA HEC-22, Table 8-1, pg. 8-27 was used to determine a broad-crested weir coefficient of 2.7. Total 100-yr runoff flowrates were used to calculate the maximum energy grade line (EGL) for each pond assuming zero storage in the pond. Table 17 shows a summary of emergency spillway performance for the 100-yr storm event assuming zero flow through the primary outlet. Reference Exhibit H for 100-yr spillway flowrate and EGL performance calculations.

Table 17. Summary of Emergency Spillway Performance (100-Yr Event)

Outfall Desc.	Max Inflow (cfs)	Weir Elev (ft)	Length (ft)	Top of Dam Elev (ft)	Max WSE (ft)	Max EGL (ft)	Freeboard (ft)
ProNW	7.89	1011.30	90.00	1012.80	1011.38	1011.39	1.40
ProS	14.68	1017.00	50.00	1018.5	1017.23	1017.25	1.25

UN-DETAINED DRAINAGE AREA ANALYSIS

The proposed southeast sub basin (ProSE) is an un-detained drainage area. The existing discharge point (Discharge points B and C on Exhibits D & F) will remain the same for the ProSE sub basin but the drainage area has decreased. The decreased area will be un-detained and discharge to the existing roadside ditch at discharge point B. Updated curve number and drainage area data for the SE basin shows an overall reduction in runoff conveyed to discharge point B. See Table 18 below for a summary of existing and proposed conditions at discharge point B.

Table 18. Summary of Discharge Point B Conditions

Outfall Desc.	Area (AC)	CN	Q ₂ (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)
ExSE	1.03	83.00	3.24	4.29	6.95
ProSE	0.58	90.00	1.83	3.03	4.62

The proposed west sub basin (ProW) is a 0.56 ac un-detained drainage area. This sub basin is a fringe lot that drains the back half of 4 proposed units towards the railroad right-of-way. A native vegetation swale and native vegetation are located along the western edge of this proposed sub basin. Runoff from the proposed development will be conveyed through this swale or sheet flow through native vegetation. See Exhibit F for the ProW sub basin location.

Existing and proposed conditions analyses were completed using the Rational Method per APWA 5602.2.A. A non-standard land use Rational C value was calculated using APWA 5602.2.C based on a total impervious area of 20% for the proposed conditions. The existing conditions Rational C = 0.30 and the proposed condition Rational C = 0.42. Time of concentration was assumed to be 5 minutes for existing and

proposed conditions to determine a conservative runoff estimate. See Table 19 for a summary of existing and proposed runoff for the ProW sub basin.

Table 19: ProW Existing & Proposed Conditions Summary

Storm Event	Intensity (in/hr)	Existing Flowrate (cfs)	Proposed Flowrate (cfs)	Increase (cfs)
2-Yr	5.41	0.91	1.27	+0.36
10-Yr	7.35	1.36	1.90	+0.54
100-Yr	10.32	2.17	3.03	+0.86

A runoff increase for all storm events has been shown for the ProW sub basin under proposed conditions. Stormwater BMPs in the form of a native vegetation swale and native grass plantings have been proposed to treat the increased runoff. All excess runoff will be discharged into the railroad right-of-way.

WATER QUALITY ANALYSIS

MARC BMP Manual Section 4.0 was used to determine BMP requirements for the proposed site. Worksheet 1A (Required level of Service – Developed Site) was used to determine the existing site value rating based on the current single-family residential land use. An existing value rating of 17.65 was calculated based on the existing and proposed impervious area for the site. See Exhibit I for Worksheet 1A calculations.

MARC BMP Manual Section 4.0, Worksheet 2 was used to analyze the proposed site BMP mitigation package. Native vegetation swales, an inlet filter, extended-dry detention basins, and native vegetation plantings were used to treat 2.69 ac of the proposed site. See Exhibit I for MARC BMP Manual - Worksheet calculations and BMP descriptions. See Exhibit J for a proposed BMP location map.

APWA 5608.4 and Chapter 6 of the MARC/APWA BMP Manual require 40-hour extended detention to treat the Water Quality Storm. MARC BMP Manual Chapter 6 section 6.2 Short-Cut Method (pg 6-1) was used to determine the water quality volume for a proposed drainage area of less than 10 acres. Table 20 lists rainfall event, percent impervious area, and volumetric runoff coefficient assumptions made for the ProNW and ProS detention basin design. Table 21 lists the water quality volume calculations for each sub basin. EDDB calculations have been provided in Exhibit I.

Table 20. APWA/MARC Water Quality Volume

Rainfall Event (P, in/24-hrs)	1.37
Percent Site Imperviousness (I, %)	35
Volumetric Runoff Coefficient (Rv)	0.635

Table 21. APWA/MARC Water Quality Volume

Detention Basin	Area (AC)	Water Quality Volume (ac-ft)	Provided Water Quality Volume (ac-ft)
ProNW	0.66	0.05	0.05
ProS	1.98	0.14	0.14

SUMMARY

The proposed site will require stormwater detention because the development will increase runoff from the existing conditions. Table 22 summarizes the existing and proposed peak flows from the entire site with no stormwater detention.

Table 22. Summary of Existing and Proposed Peak Flows

Outfall Desc.	Q₂ (cfs)	Q₁₀ (cfs)	Q₁₀₀ (cfs)
Total Existing Site	6.04	12.29	21.25
Total Proposed Site w/ out Detention	11.77	19.31	28.70

Two above ground extended dry detention basins, two vegetated swales, a curb inlet filter, and native vegetation have been added to the proposed site (ProNW, ProS, and ProW) to reduce the proposed site peak runoff, improve water quality, and control release rates for all required design storms. Table 23 summarizes the existing and proposed peak flowrate decrease with the included stormwater detention. The proposed detention meets all APWA 5608 peak discharge requirements. Table 24 summarizes allowable and actual proposed site peak discharge requirements.

Table 23. Summary of Total Existing and Proposed Peak Discharges

Outfall Desc.	Q₂ (cfs)	Q₁₀ (cfs)	Q₁₀₀ (cfs)
Total Existing Site	6.04	12.29	21.25
Total Proposed Site w/ Detention	3.30	8.65	13.61

Table 24. Summary of Proposed Peak Discharge Requirements

Outfall Desc.	Q₂ (cfs)	Q₁₀ (cfs)	Q₁₀₀ (cfs)
ProNW Allowable	0.33	1.32	1.98
ProNW Actual	0.10	0.58	1.26
Difference	-0.23	-0.74	-0.72
ProS Allowable	0.99	3.96	5.94
ProS Actual	0.96	3.14	4.70
Difference	-0.03	-0.82	-1.24

The proposed site will have two un-detained sub basins (ProSE & ProW). A request for waiver from the City of Lee's Summit Design and Construction Manual requirement has been proposed based on an overall decrease in peak flowrate discharging to outlet point B (ProSE) and a fringe lot (ProW). Table 25 summarizes the existing and proposed peak flowrates for ProSE. Table 26 summarizes the existing and proposed peak flowrates for ProW.

Table 25. Summary of ProSE Existing & Proposed Conditions

Outfall Desc.	Area (AC)	CN	Q₂ (cfs)	Q₁₀ (cfs)	Q₁₀₀ (cfs)
ExSE	1.03	83.00	2.34	4.26	6.95
ProSE	0.58	90.00	1.83	3.03	4.62

Table 26: Summary of ProW Existing & Proposed Conditions

Storm Event	Intensity (in/hr)	Existing Flowrate (cfs)	Proposed Flowrate (cfs)	Increase (cfs)
2-Yr	5.41	0.91	1.27	+0.36
10-Yr	7.35	1.36	1.90	+0.54
100-Yr	10.32	2.17	3.03	+0.86

CONCLUSION

The proposed Sequoia development is a 3.78 acre site in Lee's Summit, MO that will include the construction of 12 duplex units and associated infrastructure. Two above ground extended-dry detention basins have been proposed to control the increase runoff produced by the development.

The proposed development meets all stormwater criteria set forth by the City of Lee's Summit, Missouri and APWA 5600 design criteria. These requirements include an overall decrease in post development peak flowrates, 40-hour water quality extended detention, and a maximum allowable sub basin discharge rate.

A request for waiver from the City of Lee's Summit Design and Construction Manual requirement has been proposed for two un-detained sub basins (ProSE & ProW) based on a peak flowrate discharge decrease under proposed conditions and fringe lot conditions.

Based on this information, Renaissance Infrastructure Consulting recommends approval of this storm study. If you have any questions or need additional information, please contact me.

Sincerely,



Mick Slutter, PE



Jonathan Daldalian, EI

RENAISSANCE INFRASTRUCTURE CONSULTING

Exhibit A

Project Location Map

Sequoia , Lee's Summit, MO



Scale: 1" = 1000'

Location Map
18-0251

Prepared: 5/1/2020



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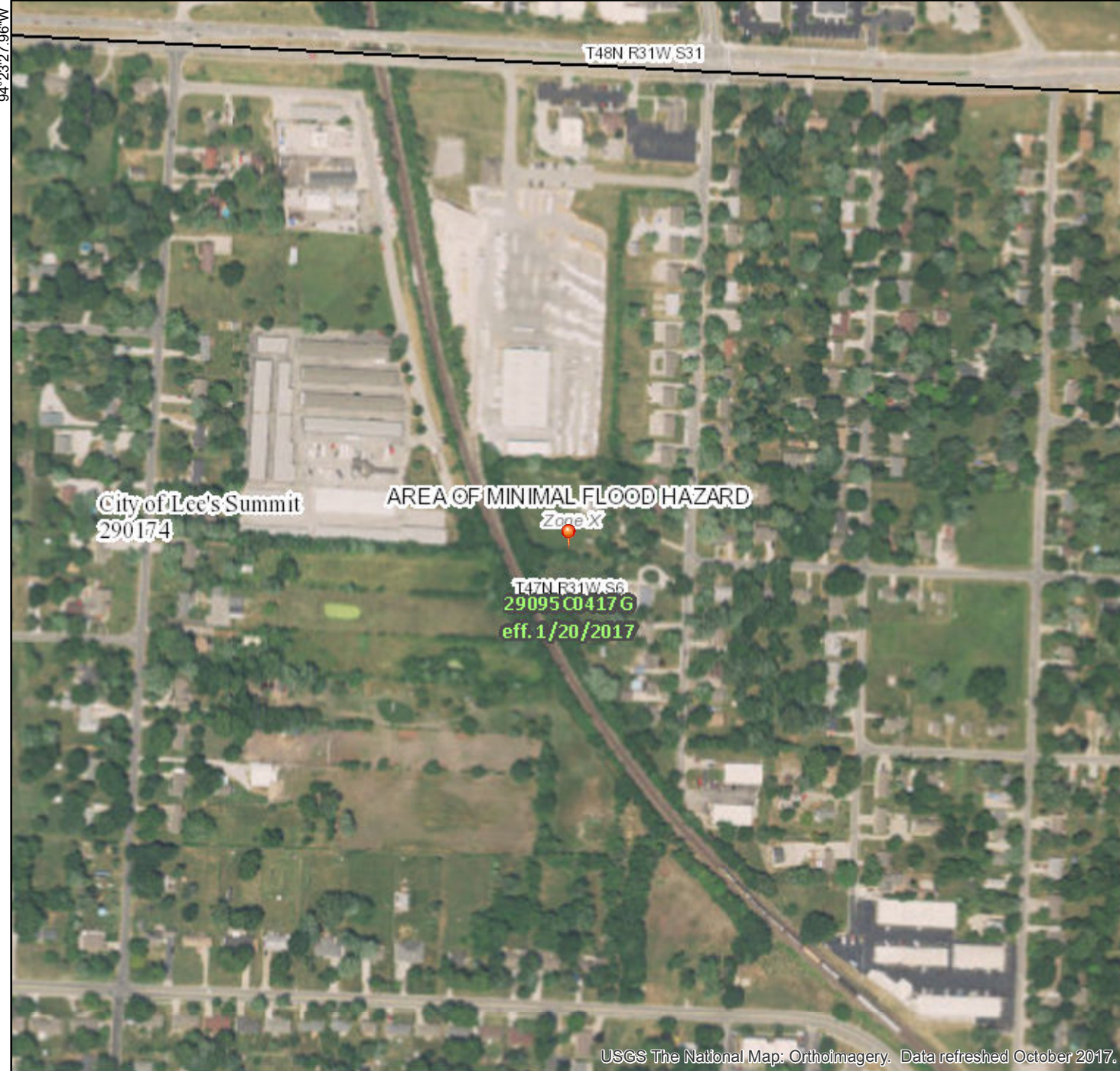
Exhibit B

FEMA FIRM Panel

National Flood Hazard Layer FIRMette



38°55'32.16"N



0 250 500 1,000 1,500 2,000 Feet

1:6,000

38°55'4.17"N

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
		Area of Undetermined Flood Hazard Zone D
GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance Water Surface Elevation
		17.5
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
MAP PANELS		Digital Data Available
		No Digital Data Available
		Unmapped



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **10/24/2018 at 10:40:52 AM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

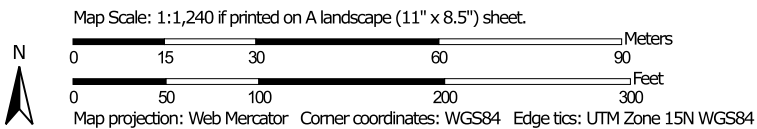
This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

94°22'50.51"W

Exhibit C

NRCS Web Soil Survey Map

Soil Map—Jackson County, Missouri
(18-0251 Burton Townhomes Soil Map)



Natural Resources
Conservation Service

Web Soil Survey
National Cooperative Soil Survey

3/4/2019
Page 1 of 3

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Jackson County, Missouri

Survey Area Data: Version 19, Sep 13, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 11, 2017—Sep 22, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
10082	Arisburg-Urban land complex, 1 to 5 percent slopes	0.4	9.6%
10128	Sharpsburg-Urban land complex, 2 to 5 percent slopes	3.4	88.4%
10181	Udarents-Urban land-Sampsel complex, 5 to 9 percent slopes	0.1	2.0%
Totals for Area of Interest		3.8	100.0%

Exhibit D

Existing Drainage Map

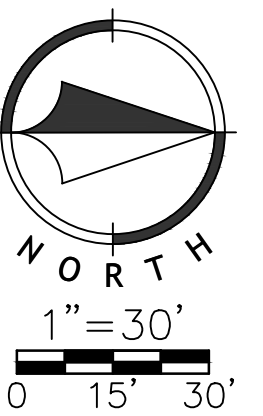
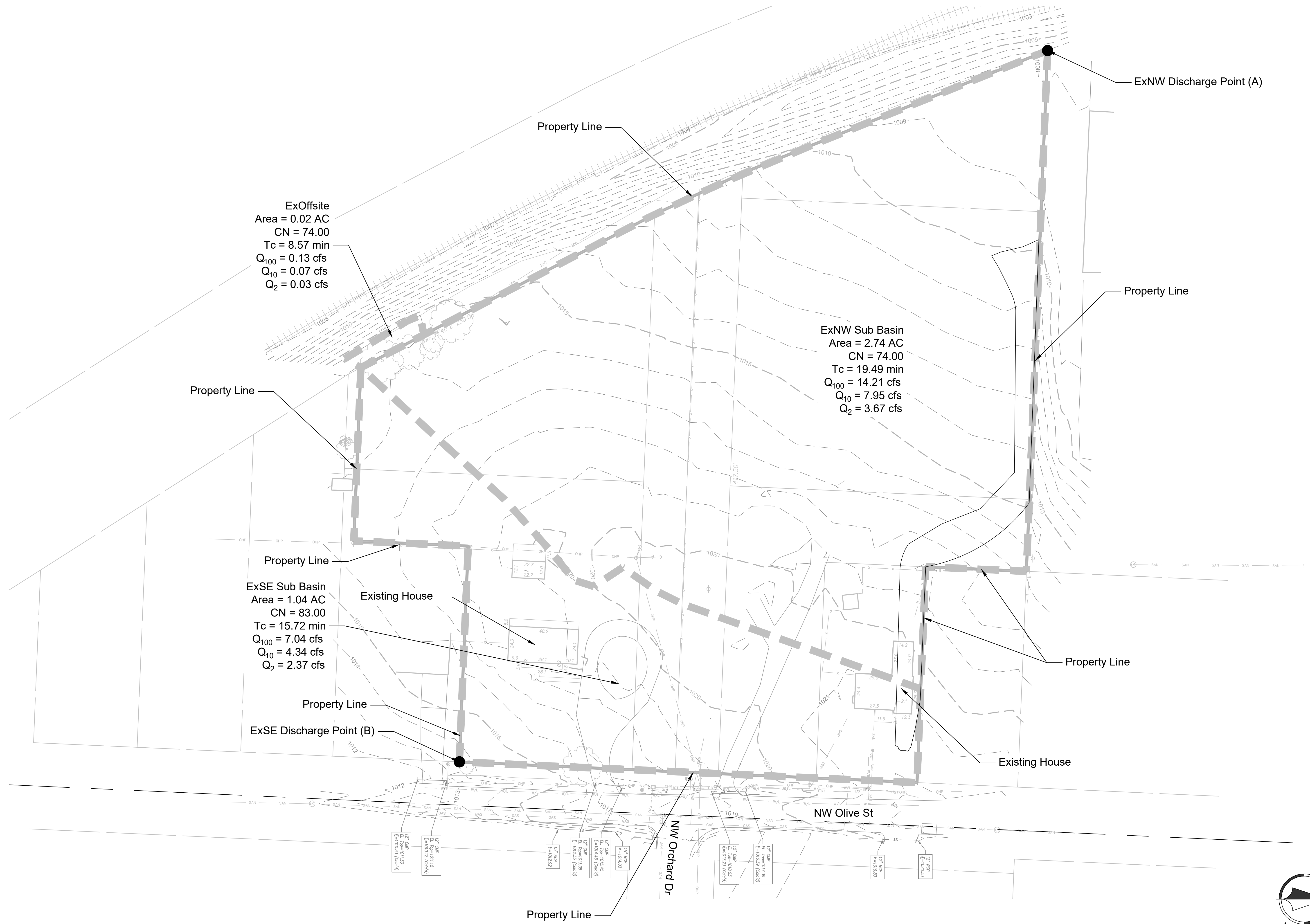
[illegible]

Exhibit E

Existing Conditions Analysis

Project Description

File Name Existing.SPF

Project Options

Flow Units CFS
Elevation Type Elevation
Hydrology Method SCS TR-55
Time of Concentration (TOC) Method SCS TR-55
Link Routing Method Kinematic Wave
Enable Overflow Ponding at Nodes YES
Skip Steady State Analysis Time Periods ... NO

Analysis Options

Start Analysis On Oct 16, 2018 00:00:00
End Analysis On Oct 17, 2018 00:00:00
Start Reporting On Oct 16, 2018 00:00:00
Antecedent Dry Days 0 days
Runoff (Dry Weather) Time Step 0 01:00:00 days hh:mm:ss
Runoff (Wet Weather) Time Step 0 00:05:00 days hh:mm:ss
Reporting Time Step 0 00:05:00 days hh:mm:ss
Routing Time Step 30 seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins.....	3
Nodes.....	3
<i>Junctions</i>	0
<i>Outfalls</i>	3
<i>Flow Diversions</i>	0
<i>Inlets</i>	0
<i>Storage Nodes</i>	0
Links.....	0
<i>Channels</i>	0
<i>Pipes</i>	0
<i>Pumps</i>	0
<i>Orifices</i>	0
<i>Weirs</i>	0
<i>Outlets</i>	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	2-Year	Cumulative	inches	Missouri	Jackson	2	3.50	SCS Type II 24-hr

Subbasin Summary

SN	Subbasin ID	Area	Weighted Curve Number	Total Rainfall	Total Runoff	Total Runoff Volume	Peak Runoff	Time of Concentration
		(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1	ExNW	2.74	74.00	3.50	1.24	3.40	3.67	0 00:19:29
2	ExOffsite	0.02	74.00	3.50	0.95	0.02	0.03	0 00:08:34
3	ExSE	1.04	83.00	3.50	1.86	1.93	2.37	0 00:15:43

Node Summary

SN	Element ID	Element Type	Invert Elevation	Ground/Rim (Max) Elevation	Initial Water Elevation	Surcharge Elevation	Ponded Area	Peak Inflow	Max HGL Elevation Attained	Max Surcharge Depth Attained	Min Freeboard Attained	Time of Flooding Occurrence	Total Flooded Volume	Total Time Flooded
			(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	(ft)	(days hh:mm)	(ac-in)	(min)
1	Out-01	Outfall	0.00					0.00	0.00					
2	Out-03	Outfall	0.00					0.00	0.00					
3	Out-05	Outfall	0.00					0.00	0.00					

Subbasin Hydrology

Subbasin : ExNW

Input Data

Area (ac) 2.74
Weighted Curve Number 74.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
> 75% grass cover, Good	2.74	C	74.00
Composite Area & Weighted CN	2.74		74.00

Time of Concentration

TOC Method : SCS TR-55

Sheet Flow Equation :

$$T_c = (0.007 * ((n * L_f)^{0.8})) / ((P^{0.5}) * (S_f^{0.4}))$$

Where :

T_c = Time of Concentration (hr)
n = Manning's roughness
L_f = Flow Length (ft)
P = 2 yr, 24 hr Rainfall (inches)
S_f = Slope (ft/ft)

Shallow Concentrated Flow Equation :

V = 16.1345 * (S_f^{0.5}) (unpaved surface)
V = 20.3282 * (S_f^{0.5}) (paved surface)
V = 15.0 * (S_f^{0.5}) (grassed waterway surface)
V = 10.0 * (S_f^{0.5}) (nearly bare & untilled surface)
V = 9.0 * (S_f^{0.5}) (cultivated straight rows surface)
V = 7.0 * (S_f^{0.5}) (short grass pasture surface)
V = 5.0 * (S_f^{0.5}) (woodland surface)
V = 2.5 * (S_f^{0.5}) (forest w/heavy litter surface)
T_c = (L_f / V) / (3600 sec/hr)

Where:

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)

Channel Flow Equation :

V = (1.49 * (R^{2/3}) * (S_f^{0.5})) / n
R = A_q / W_p
T_c = (L_f / V) / (3600 sec/hr)

Where :

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
R = Hydraulic Radius (ft)
A_q = Flow Area (ft²)
W_p = Wetted Perimeter (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)
n = Manning's roughness

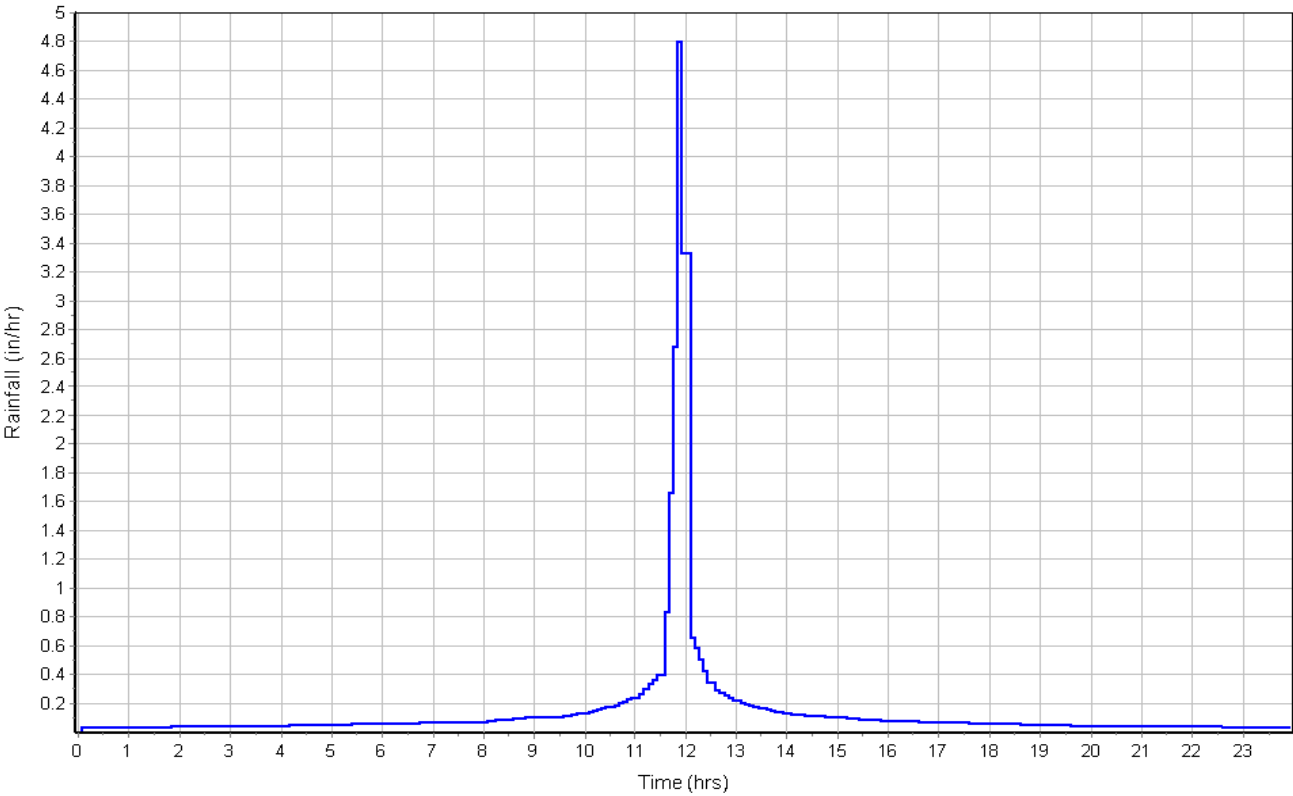
	Subarea	Subarea	Subarea
	A	B	C
Sheet Flow Computations			
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	100	0.00	0.00
Slope (%) :	2.8	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.12	0.00	0.00
Computed Flow Time (min) :	14.26	0.00	0.00
	Subarea	Subarea	Subarea
	A	B	C
Shallow Concentrated Flow Computations			
Flow Length (ft) :	380	0.00	0.00
Slope (%) :	3.0	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.21	0.00	0.00
Computed Flow Time (min) :	5.23	0.00	0.00
Total TOC (min)	19.49		

Subbasin Runoff Results

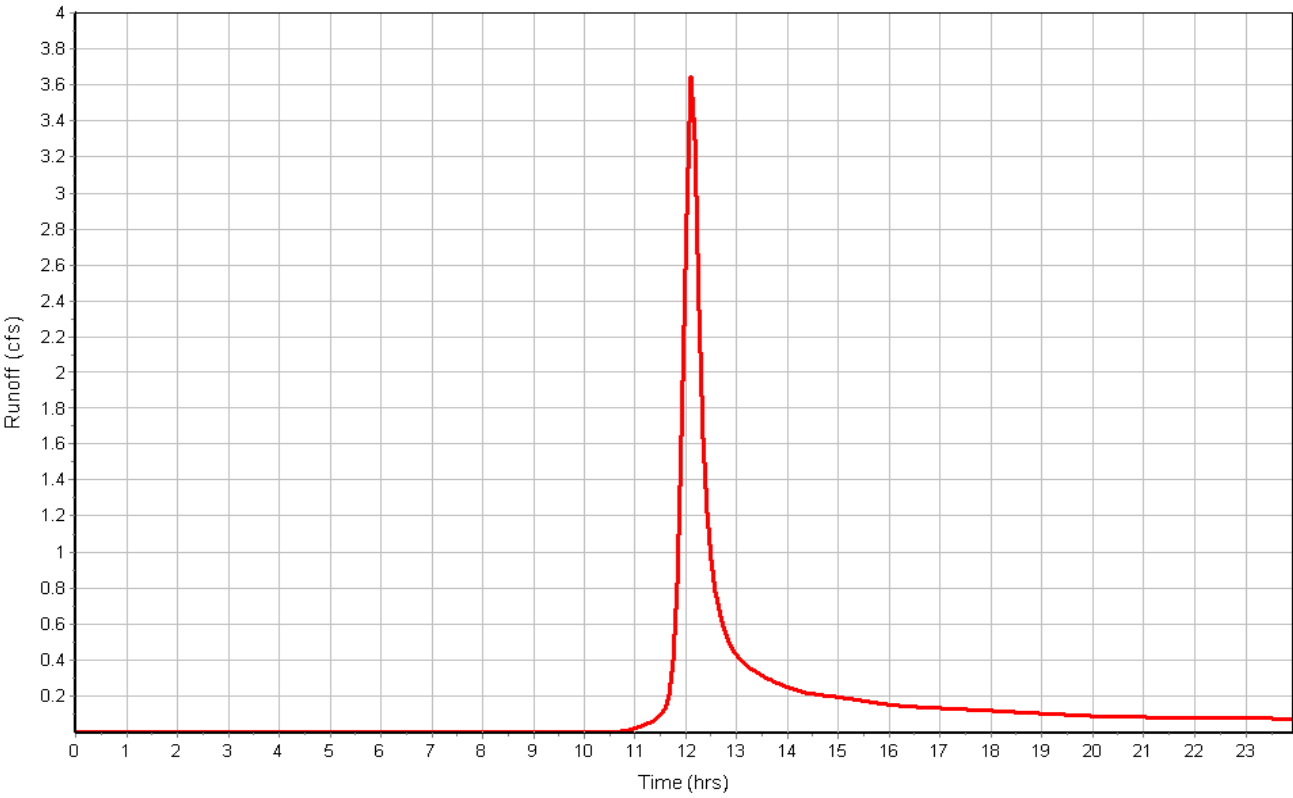
Total Rainfall (in)	3.50
Total Runoff (in)	1.24
Peak Runoff (cfs)	3.67
Weighted Curve Number	74.00
Time of Concentration (days hh:mm:ss)	0 00:19:29

Subbasin : ExNW

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : ExOffsite

Input Data

Area (ac) 0.02
Weighted Curve Number 74.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
> 75% grass cover, Good	0.02	C	74.00
Composite Area & Weighted CN	0.02		74.00

Time of Concentration

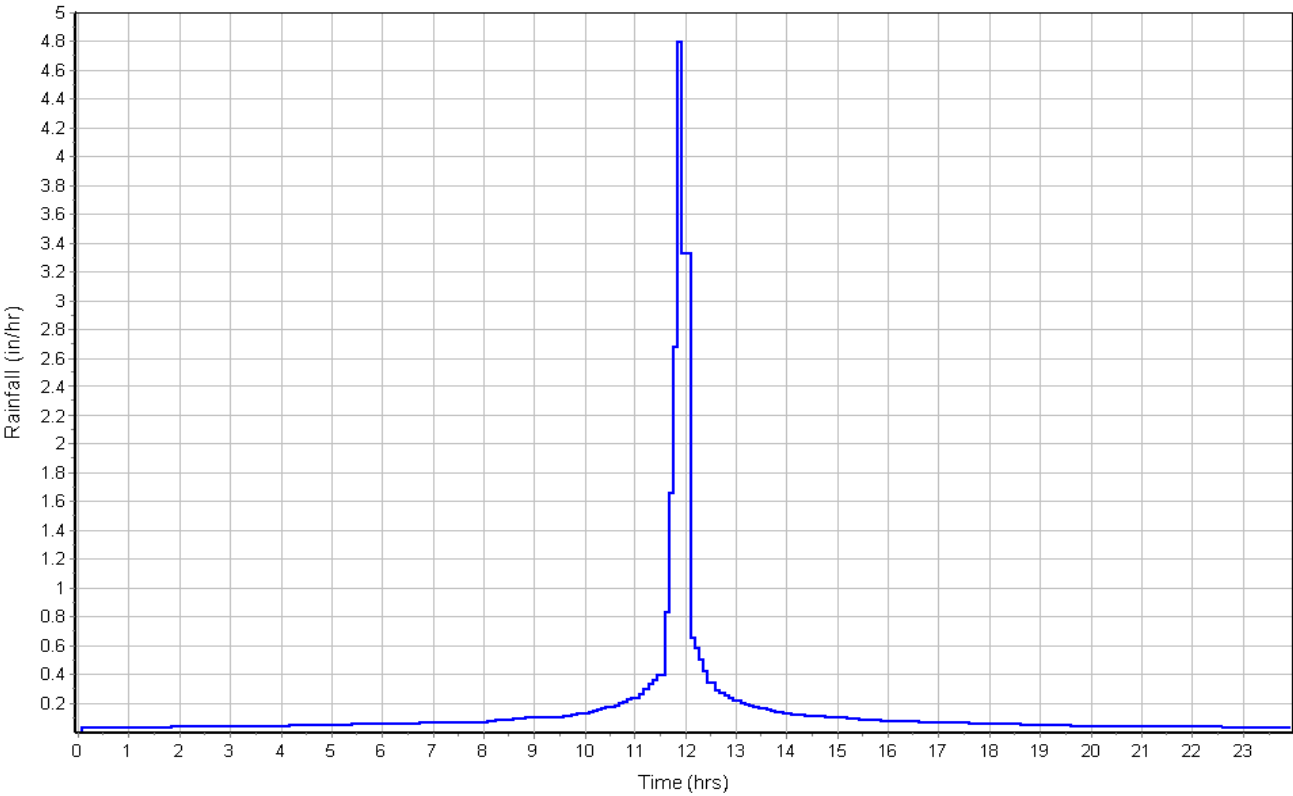
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	10	0.00	0.00
Slope (%) :	0.1	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.02	0.00	0.00
Computed Flow Time (min) :	8.57	0.00	0.00
Total TOC (min)	8.57		

Subbasin Runoff Results

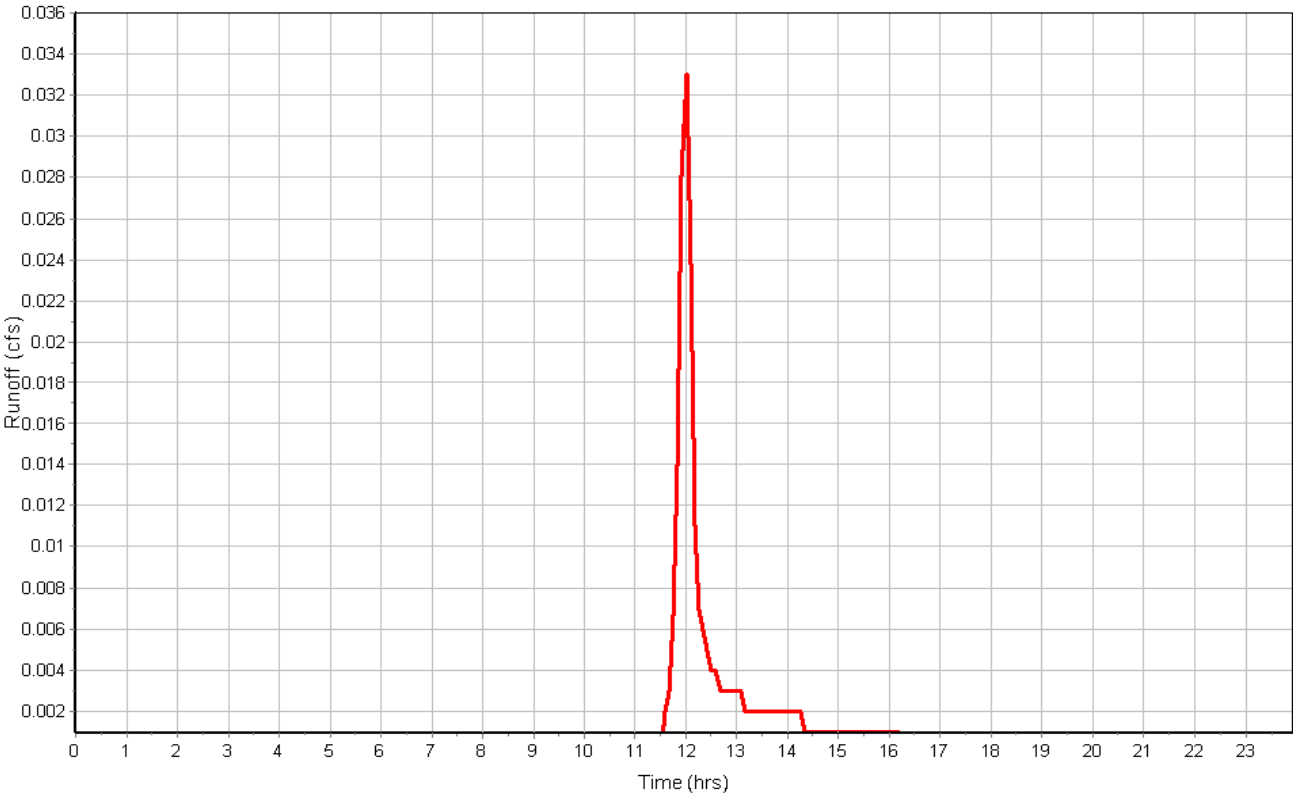
Total Rainfall (in) 3.50
Total Runoff (in) 0.95
Peak Runoff (cfs) 0.03
Weighted Curve Number 74.00
Time of Concentration (days hh:mm:ss) 0 00:08:34

Subbasin : ExOffsite

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : ExSE

Input Data

Area (ac) 1.04
Weighted Curve Number 83.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/4 acre lots, 38% impervious	1.04	C	83.00
Composite Area & Weighted CN	1.04		83.00

Time of Concentration

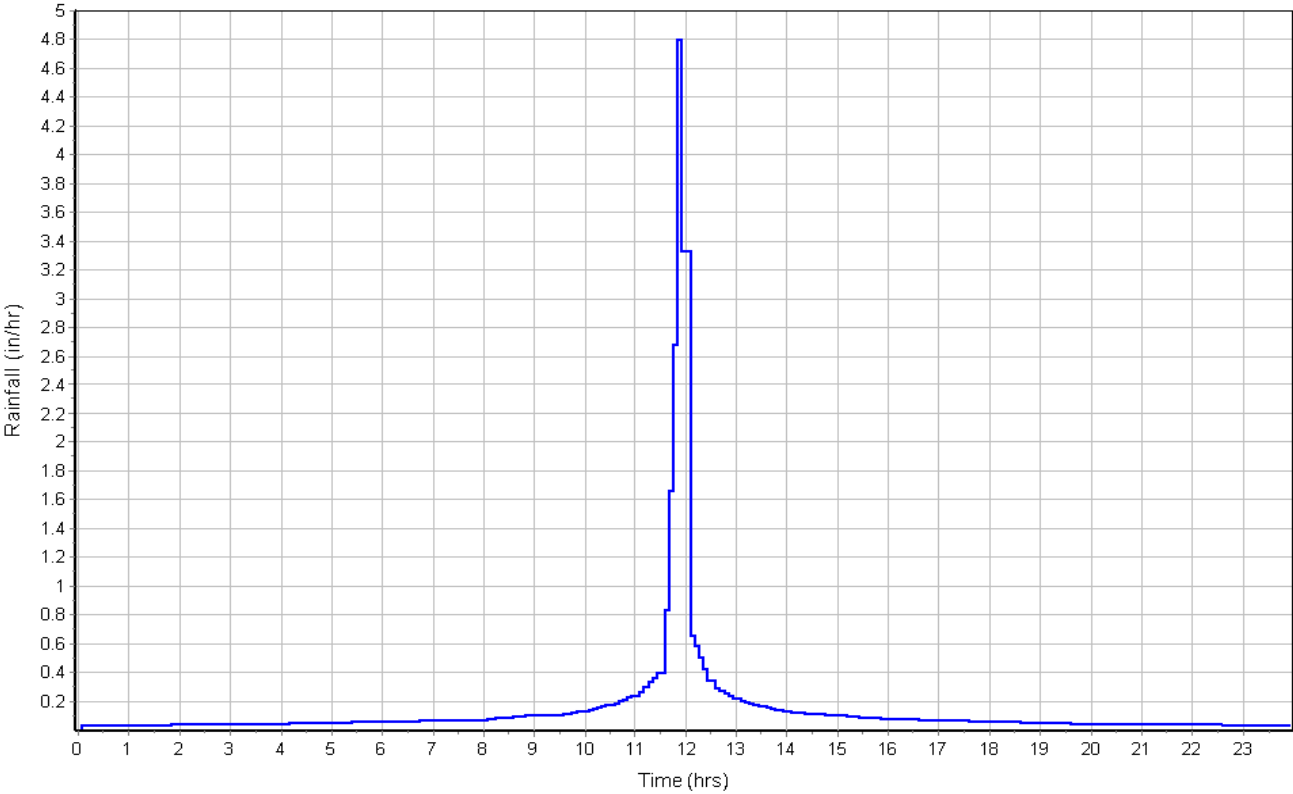
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	100	0.00	0.00
Slope (%) :	3.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.12	0.00	0.00
Computed Flow Time (min) :	13.87	0.00	0.00
Shallow Concentrated Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Flow Length (ft) :	150	0.00	0.00
Slope (%) :	3.7	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.35	0.00	0.00
Computed Flow Time (min) :	1.85	0.00	0.00
Total TOC (min)	15.72		

Subbasin Runoff Results

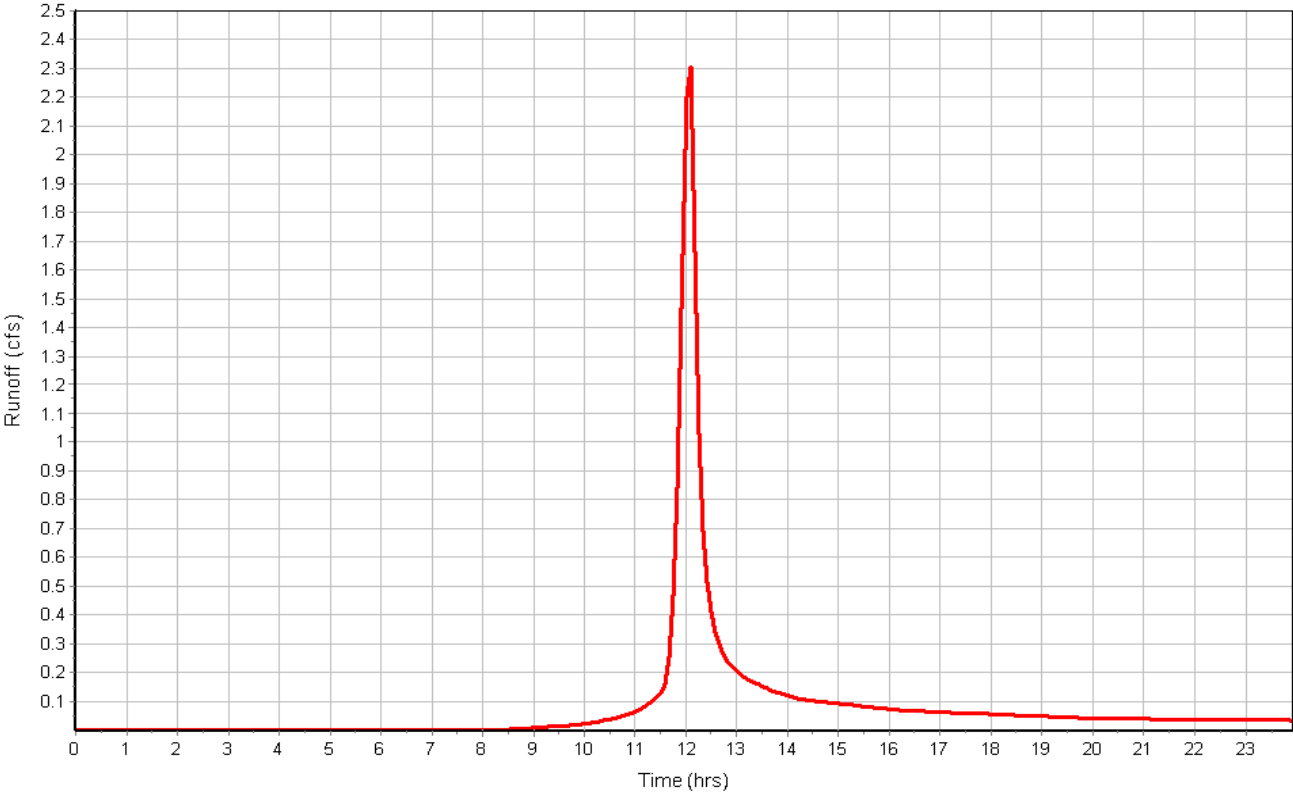
Total Rainfall (in) 3.50
Total Runoff (in) 1.86
Peak Runoff (cfs) 2.37
Weighted Curve Number 83.00
Time of Concentration (days hh:mm:ss) 0 00:15:43

Subbasin : ExSE

Rainfall Intensity Graph



Runoff Hydrograph



Project Description

File Name Existing.SPF

Project Options

Flow Units CFS
Elevation Type Elevation
Hydrology Method SCS TR-55
Time of Concentration (TOC) Method SCS TR-55
Link Routing Method Kinematic Wave
Enable Overflow Ponding at Nodes YES
Skip Steady State Analysis Time Periods ... NO

Analysis Options

Start Analysis On Oct 16, 2018 00:00:00
End Analysis On Oct 17, 2018 00:00:00
Start Reporting On Oct 16, 2018 00:00:00
Antecedent Dry Days 0 days
Runoff (Dry Weather) Time Step 0 01:00:00 days hh:mm:ss
Runoff (Wet Weather) Time Step 0 00:05:00 days hh:mm:ss
Reporting Time Step 0 00:05:00 days hh:mm:ss
Routing Time Step 30 seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins.....	3
Nodes.....	3
<i>Junctions</i>	0
<i>Outfalls</i>	3
<i>Flow Diversions</i>	0
<i>Inlets</i>	0
<i>Storage Nodes</i>	0
Links.....	0
<i>Channels</i>	0
<i>Pipes</i>	0
<i>Pumps</i>	0
<i>Orifices</i>	0
<i>Weirs</i>	0
<i>Outlets</i>	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	10-Year	Cumulative	inches	Missouri	Jackson	10	5.30	SCS Type II 24-hr

Subbasin Summary

SN	Subbasin ID	Area	Weighted Curve Number	Total Rainfall	Total Runoff	Total Runoff Volume	Peak Runoff	Time of Concentration
		(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1	ExNW	2.74	74.00	5.30	2.61	7.14	7.95	0 00:19:29
2	ExOffsite	0.02	74.00	5.30	2.46	0.05	0.07	0 00:08:34
3	ExSE	1.04	83.00	5.30	3.45	3.58	4.34	0 00:15:43

Node Summary

SN	Element ID	Element Type	Invert Elevation	Ground/Rim (Max) Elevation	Initial Water Elevation	Surcharge Elevation	Ponded Area	Peak Inflow	Max HGL Elevation Attained	Max Surcharge Depth Attained	Min Freeboard Attained	Time of Flooding Occurrence	Total Flooded Volume	Total Time Flooded
			(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	(ft)	(days hh:mm)	(ac-in)	(min)
1	Out-01	Outfall	0.00					0.00	0.00					
2	Out-03	Outfall	0.00					0.00	0.00					
3	Out-05	Outfall	0.00					0.00	0.00					

Subbasin Hydrology

Subbasin : ExNW

Input Data

Area (ac) 2.74
Weighted Curve Number 74.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
> 75% grass cover, Good	2.74	C	74.00
Composite Area & Weighted CN	2.74		74.00

Time of Concentration

TOC Method : SCS TR-55

Sheet Flow Equation :

$$T_c = (0.007 * ((n * L_f)^{0.8})) / ((P^{0.5}) * (S_f^{0.4}))$$

Where :

T_c = Time of Concentration (hr)
n = Manning's roughness
L_f = Flow Length (ft)
P = 2 yr, 24 hr Rainfall (inches)
S_f = Slope (ft/ft)

Shallow Concentrated Flow Equation :

V = 16.1345 * (S_f^{0.5}) (unpaved surface)
V = 20.3282 * (S_f^{0.5}) (paved surface)
V = 15.0 * (S_f^{0.5}) (grassed waterway surface)
V = 10.0 * (S_f^{0.5}) (nearly bare & untilled surface)
V = 9.0 * (S_f^{0.5}) (cultivated straight rows surface)
V = 7.0 * (S_f^{0.5}) (short grass pasture surface)
V = 5.0 * (S_f^{0.5}) (woodland surface)
V = 2.5 * (S_f^{0.5}) (forest w/heavy litter surface)
T_c = (L_f / V) / (3600 sec/hr)

Where:

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)

Channel Flow Equation :

V = (1.49 * (R^{2/3}) * (S_f^{0.5})) / n
R = A_q / W_p
T_c = (L_f / V) / (3600 sec/hr)

Where :

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
R = Hydraulic Radius (ft)
A_q = Flow Area (ft²)
W_p = Wetted Perimeter (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)
n = Manning's roughness

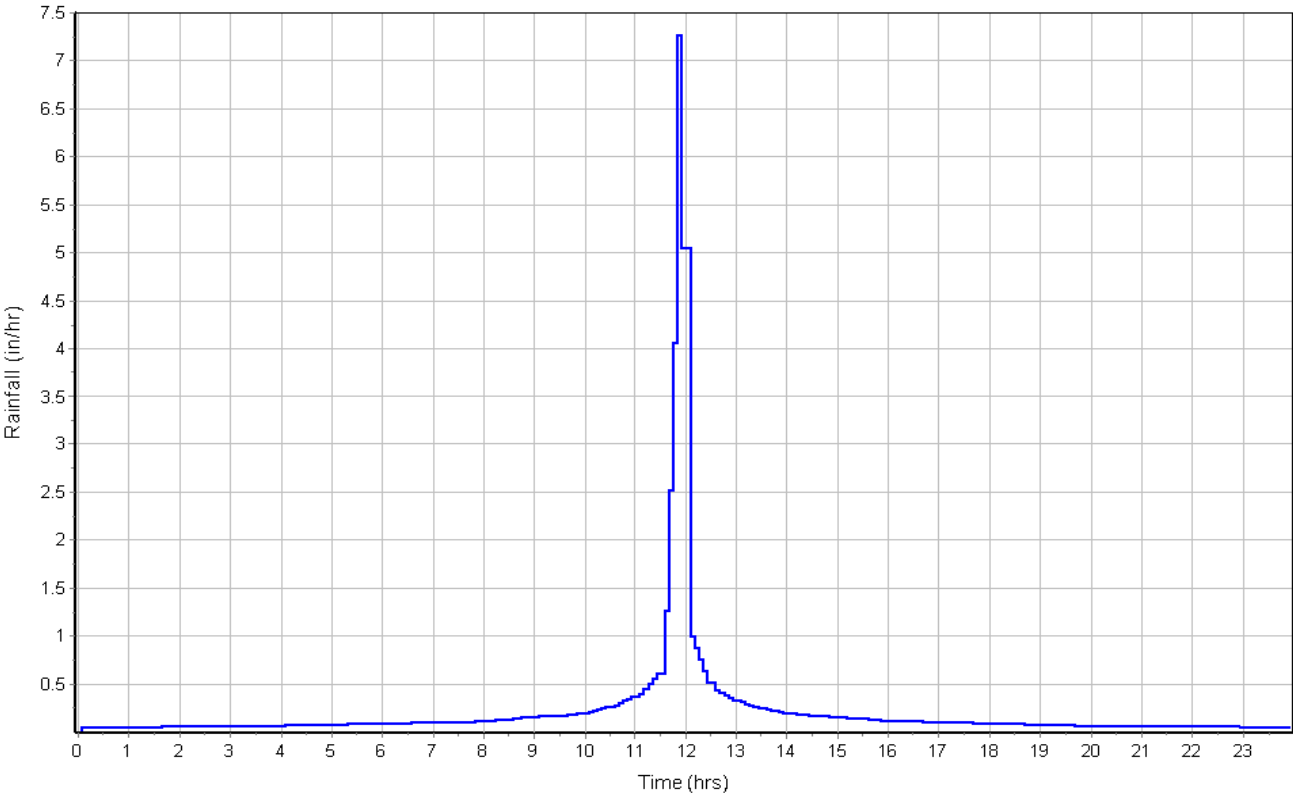
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	100	0.00	0.00
Slope (%) :	2.8	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.12	0.00	0.00
Computed Flow Time (min) :	14.26	0.00	0.00
Shallow Concentrated Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Flow Length (ft) :	380	0.00	0.00
Slope (%) :	3.0	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.21	0.00	0.00
Computed Flow Time (min) :	5.23	0.00	0.00
Total TOC (min)	19.49		

Subbasin Runoff Results

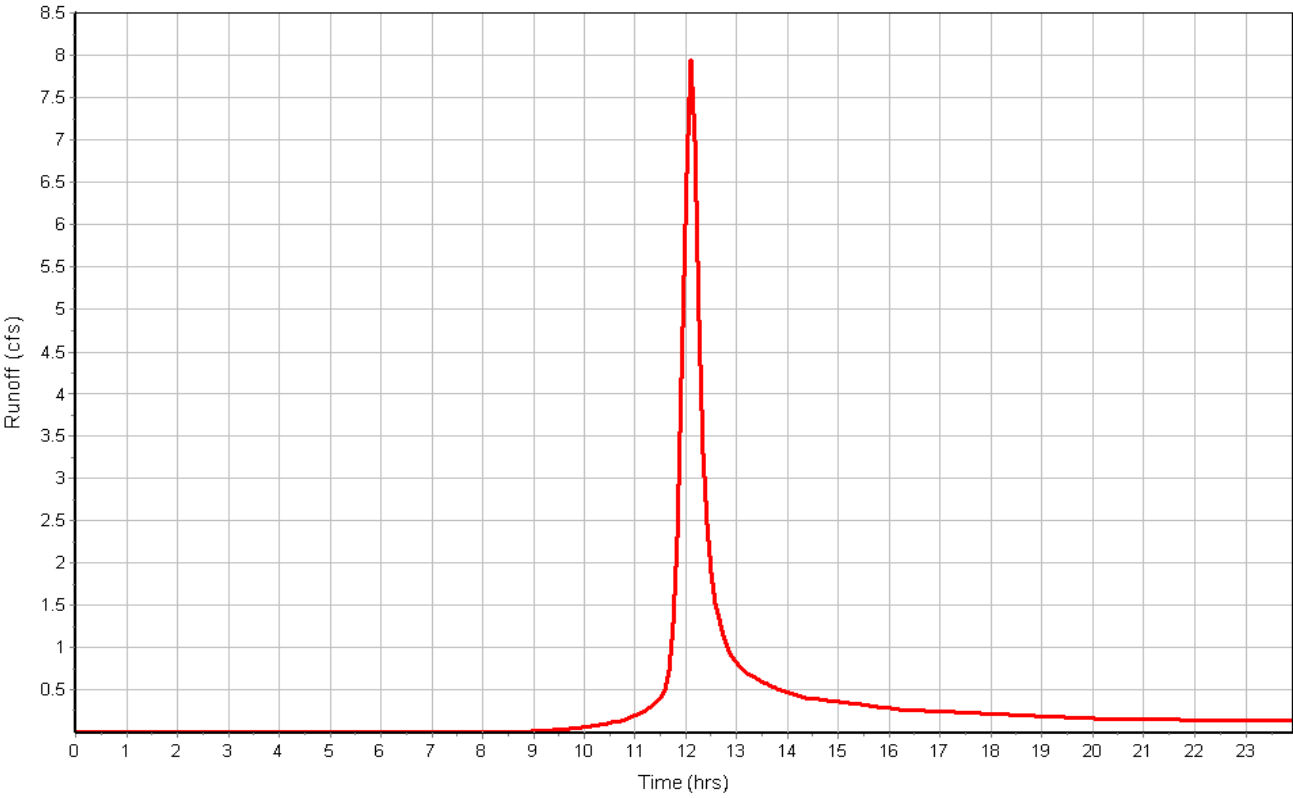
Total Rainfall (in)	5.30
Total Runoff (in)	2.61
Peak Runoff (cfs)	7.95
Weighted Curve Number	74.00
Time of Concentration (days hh:mm:ss)	0 00:19:29

Subbasin : ExNW

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : ExOffsite

Input Data

Area (ac) 0.02
Weighted Curve Number 74.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
> 75% grass cover, Good	0.02	C	74.00
Composite Area & Weighted CN	0.02		74.00

Time of Concentration

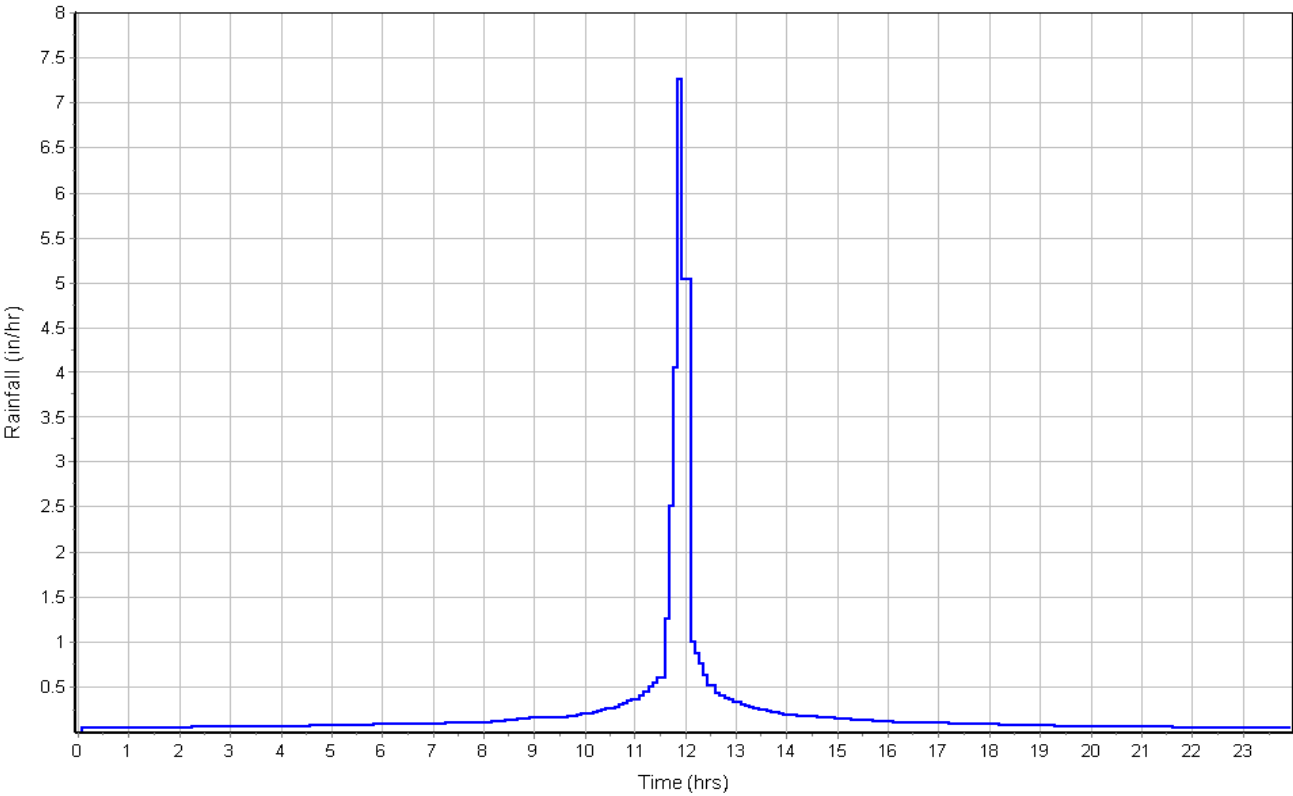
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	10	0.00	0.00
Slope (%) :	0.1	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.02	0.00	0.00
Computed Flow Time (min) :	8.57	0.00	0.00
Total TOC (min)	8.57		

Subbasin Runoff Results

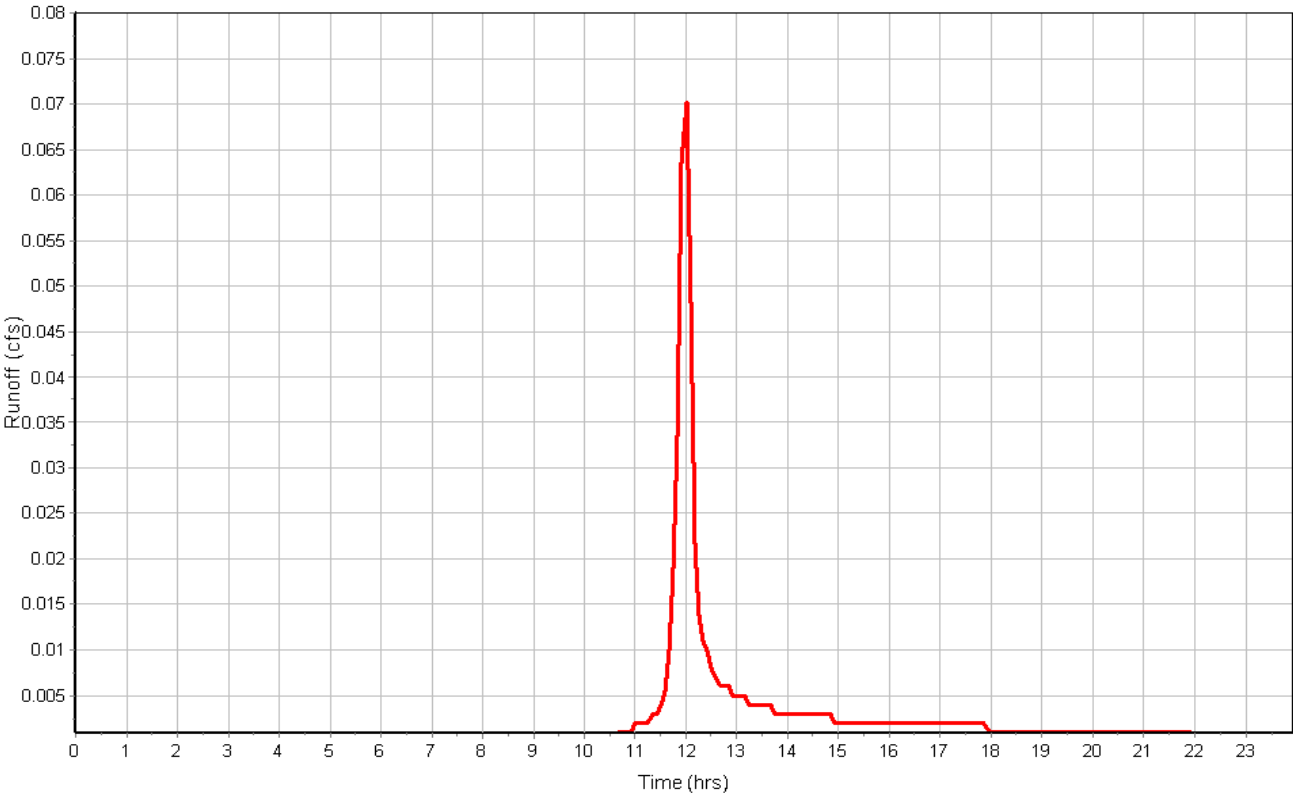
Total Rainfall (in) 5.30
Total Runoff (in) 2.46
Peak Runoff (cfs) 0.07
Weighted Curve Number 74.00
Time of Concentration (days hh:mm:ss) 0 00:08:34

Subbasin : ExOffsite

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : ExSE

Input Data

Area (ac) 1.04
Weighted Curve Number 83.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/4 acre lots, 38% impervious	1.04	C	83.00
Composite Area & Weighted CN	1.04		83.00

Time of Concentration

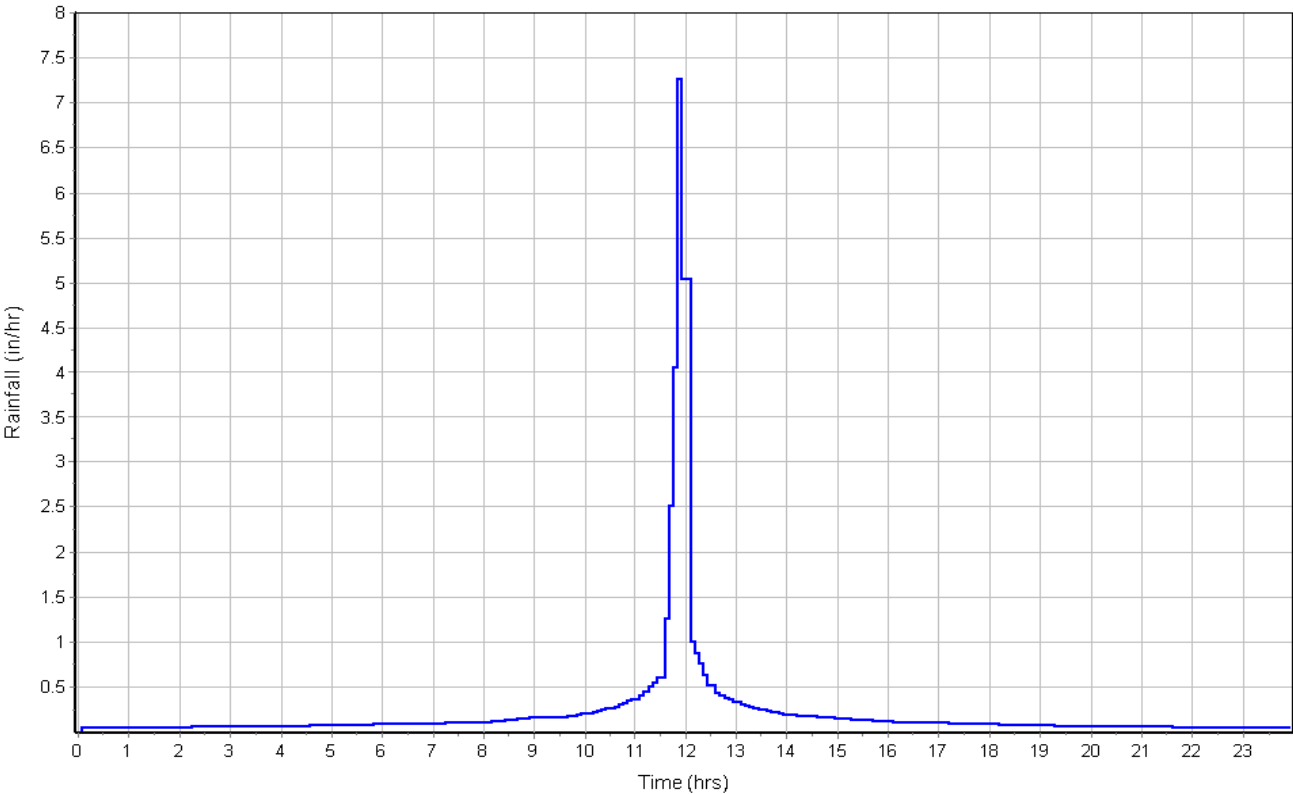
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	100	0.00	0.00
Slope (%) :	3.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.12	0.00	0.00
Computed Flow Time (min) :	13.87	0.00	0.00
Shallow Concentrated Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Flow Length (ft) :	150	0.00	0.00
Slope (%) :	3.7	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.35	0.00	0.00
Computed Flow Time (min) :	1.85	0.00	0.00
Total TOC (min)	15.72		

Subbasin Runoff Results

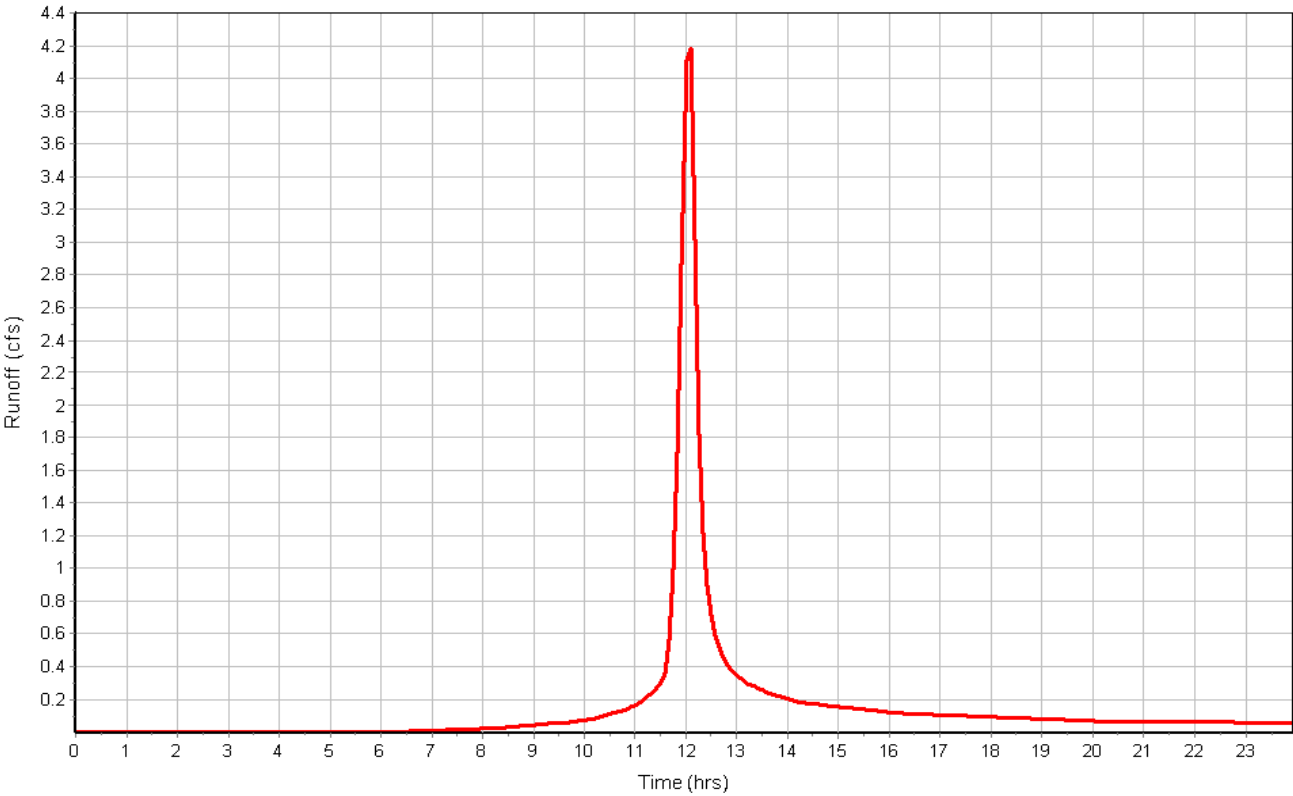
Total Rainfall (in) 5.30
Total Runoff (in) 3.45
Peak Runoff (cfs) 4.34
Weighted Curve Number 83.00
Time of Concentration (days hh:mm:ss) 0 00:15:43

Subbasin : ExSE

Rainfall Intensity Graph



Runoff Hydrograph



Project Description

File Name Existing.SPF

Project Options

Flow Units CFS
Elevation Type Elevation
Hydrology Method SCS TR-55
Time of Concentration (TOC) Method SCS TR-55
Link Routing Method Kinematic Wave
Enable Overflow Ponding at Nodes YES
Skip Steady State Analysis Time Periods ... NO

Analysis Options

Start Analysis On Oct 16, 2018 00:00:00
End Analysis On Oct 17, 2018 00:00:00
Start Reporting On Oct 16, 2018 00:00:00
Antecedent Dry Days 0 days
Runoff (Dry Weather) Time Step 0 01:00:00 days hh:mm:ss
Runoff (Wet Weather) Time Step 0 00:05:00 days hh:mm:ss
Reporting Time Step 0 00:05:00 days hh:mm:ss
Routing Time Step 30 seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins.....	3
Nodes.....	3
<i>Junctions</i>	0
<i>Outfalls</i>	3
<i>Flow Diversions</i>	0
<i>Inlets</i>	0
<i>Storage Nodes</i>	0
Links.....	0
<i>Channels</i>	0
<i>Pipes</i>	0
<i>Pumps</i>	0
<i>Orifices</i>	0
<i>Weirs</i>	0
<i>Outlets</i>	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	100-Year	Cumulative	inches	Missouri	Jackson	100	7.70	SCS Type II 24-hr

Subbasin Summary

SN	Subbasin ID	Area	Weighted Curve Number	Total Rainfall	Total Runoff	Total Runoff Volume	Peak Runoff	Time of Concentration
		(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1	ExNW	2.74	74.00	7.70	4.66	12.76	14.21	0 00:19:29
2	ExOffsite	0.02	74.00	7.70	4.61	0.09	0.13	0 00:08:34
3	ExSE	1.04	83.00	7.70	5.69	5.92	7.04	0 00:15:43

Node Summary

SN	Element ID	Element Type	Invert Elevation	Ground/Rim (Max) Elevation	Initial Water Elevation	Surcharge Elevation	Ponded Area	Peak Inflow	Max HGL Elevation Attained	Max Surcharge Depth Attained	Min Freeboard Attained	Time of Flooding Occurrence	Total Flooded Volume	Total Time Flooded
			(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	(ft)	(days hh:mm)	(ac-in)	(min)
1	Out-01	Outfall	0.00					0.00	0.00					
2	Out-03	Outfall	0.00					0.00	0.00					
3	Out-05	Outfall	0.00					0.00	0.00					

Subbasin Hydrology

Subbasin : ExNW

Input Data

Area (ac) 2.74
Weighted Curve Number 74.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
> 75% grass cover, Good	2.74	C	74.00
Composite Area & Weighted CN	2.74		74.00

Time of Concentration

TOC Method : SCS TR-55

Sheet Flow Equation :

$$T_c = (0.007 * ((n * L_f)^{0.8})) / ((P^{0.5}) * (S_f^{0.4}))$$

Where :

T_c = Time of Concentration (hr)
n = Manning's roughness
L_f = Flow Length (ft)
P = 2 yr, 24 hr Rainfall (inches)
S_f = Slope (ft/ft)

Shallow Concentrated Flow Equation :

V = 16.1345 * (S_f^{0.5}) (unpaved surface)
V = 20.3282 * (S_f^{0.5}) (paved surface)
V = 15.0 * (S_f^{0.5}) (grassed waterway surface)
V = 10.0 * (S_f^{0.5}) (nearly bare & untilled surface)
V = 9.0 * (S_f^{0.5}) (cultivated straight rows surface)
V = 7.0 * (S_f^{0.5}) (short grass pasture surface)
V = 5.0 * (S_f^{0.5}) (woodland surface)
V = 2.5 * (S_f^{0.5}) (forest w/heavy litter surface)
T_c = (L_f / V) / (3600 sec/hr)

Where:

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)

Channel Flow Equation :

V = (1.49 * (R^{2/3}) * (S_f^{0.5})) / n
R = A_q / W_p
T_c = (L_f / V) / (3600 sec/hr)

Where :

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
R = Hydraulic Radius (ft)
A_q = Flow Area (ft²)
W_p = Wetted Perimeter (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)
n = Manning's roughness

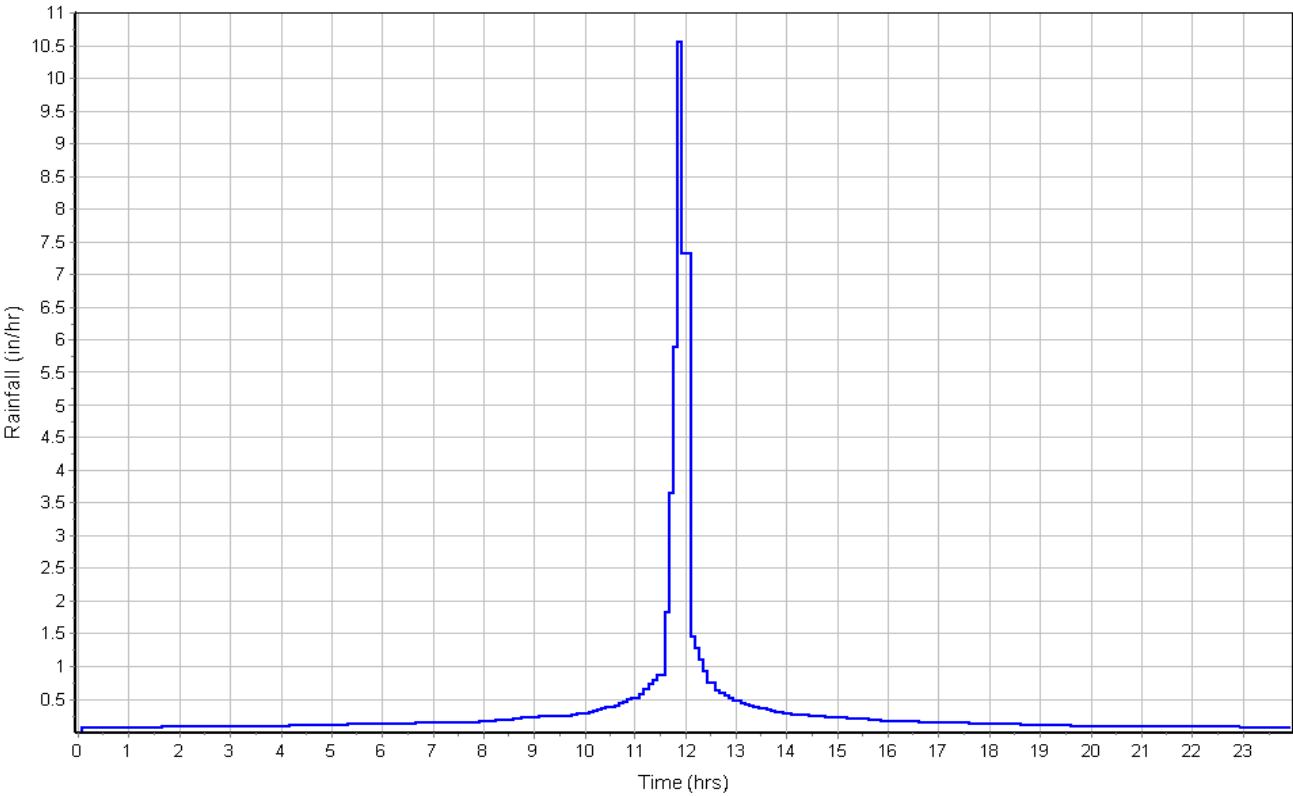
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	100	0.00	0.00
Slope (%) :	2.8	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.12	0.00	0.00
Computed Flow Time (min) :	14.26	0.00	0.00
Shallow Concentrated Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Flow Length (ft) :	380	0.00	0.00
Slope (%) :	3.0	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.21	0.00	0.00
Computed Flow Time (min) :	5.23	0.00	0.00
Total TOC (min)	19.49		

Subbasin Runoff Results

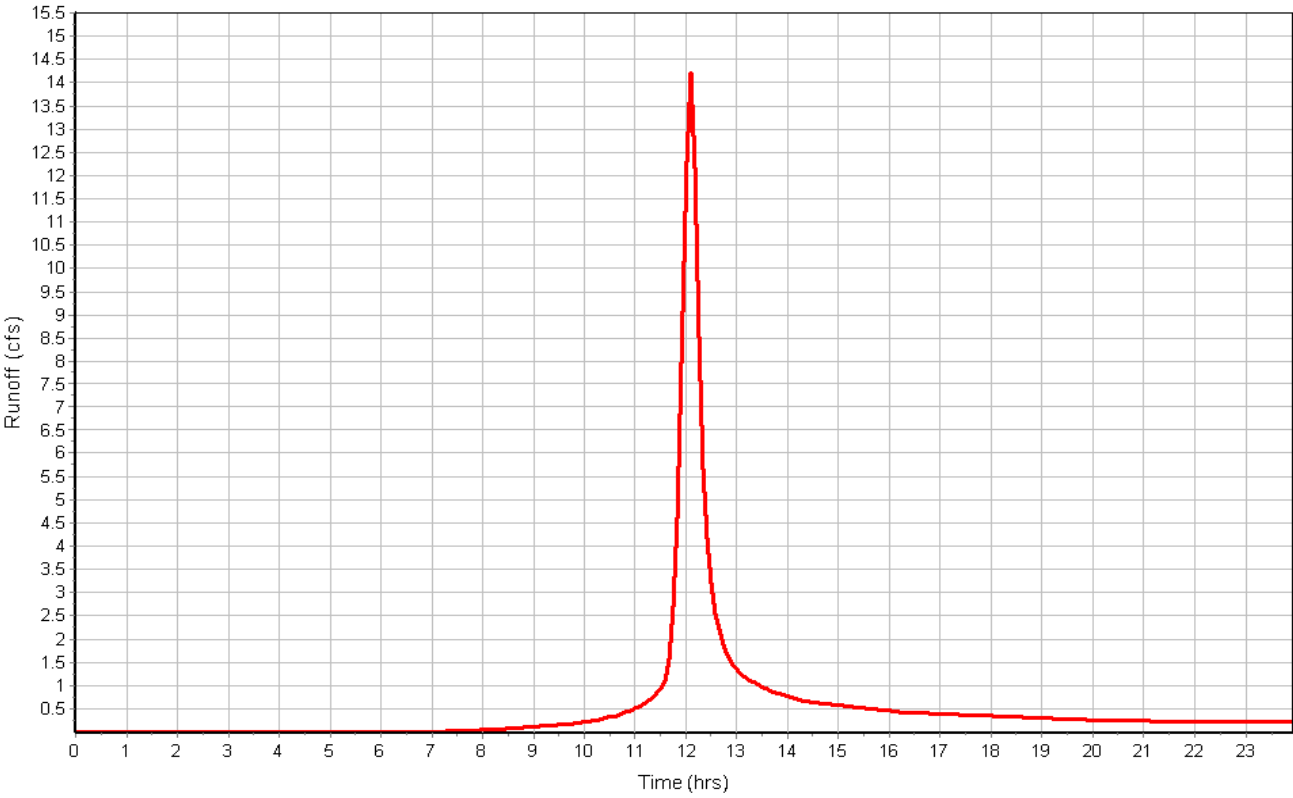
Total Rainfall (in)	7.70
Total Runoff (in)	4.66
Peak Runoff (cfs)	14.21
Weighted Curve Number	74.00
Time of Concentration (days hh:mm:ss)	0 00:19:29

Subbasin : ExNW

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : ExOffsite

Input Data

Area (ac) 0.02
Weighted Curve Number 74.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
> 75% grass cover, Good	0.02	C	74.00
Composite Area & Weighted CN	0.02		74.00

Time of Concentration

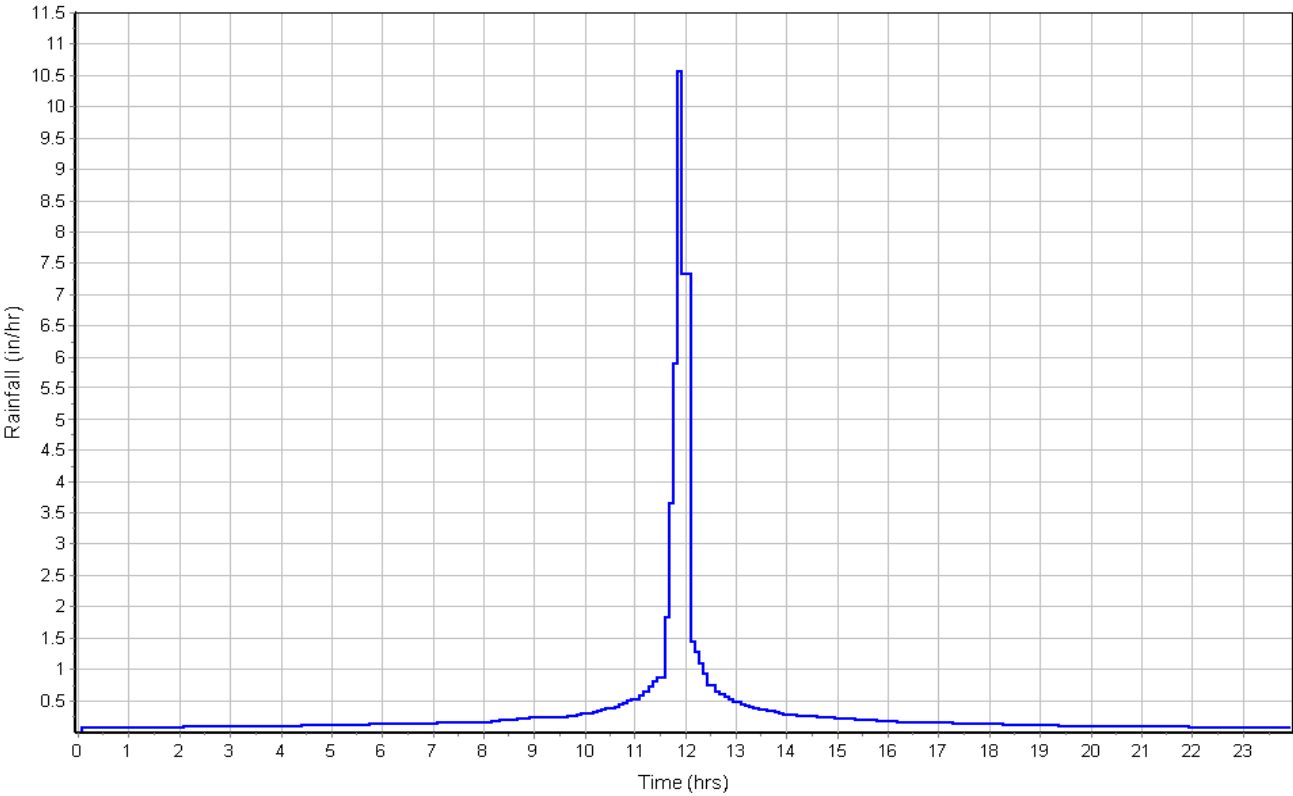
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	10	0.00	0.00
Slope (%) :	0.1	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.02	0.00	0.00
Computed Flow Time (min) :	8.57	0.00	0.00
Total TOC (min)	8.57		

Subbasin Runoff Results

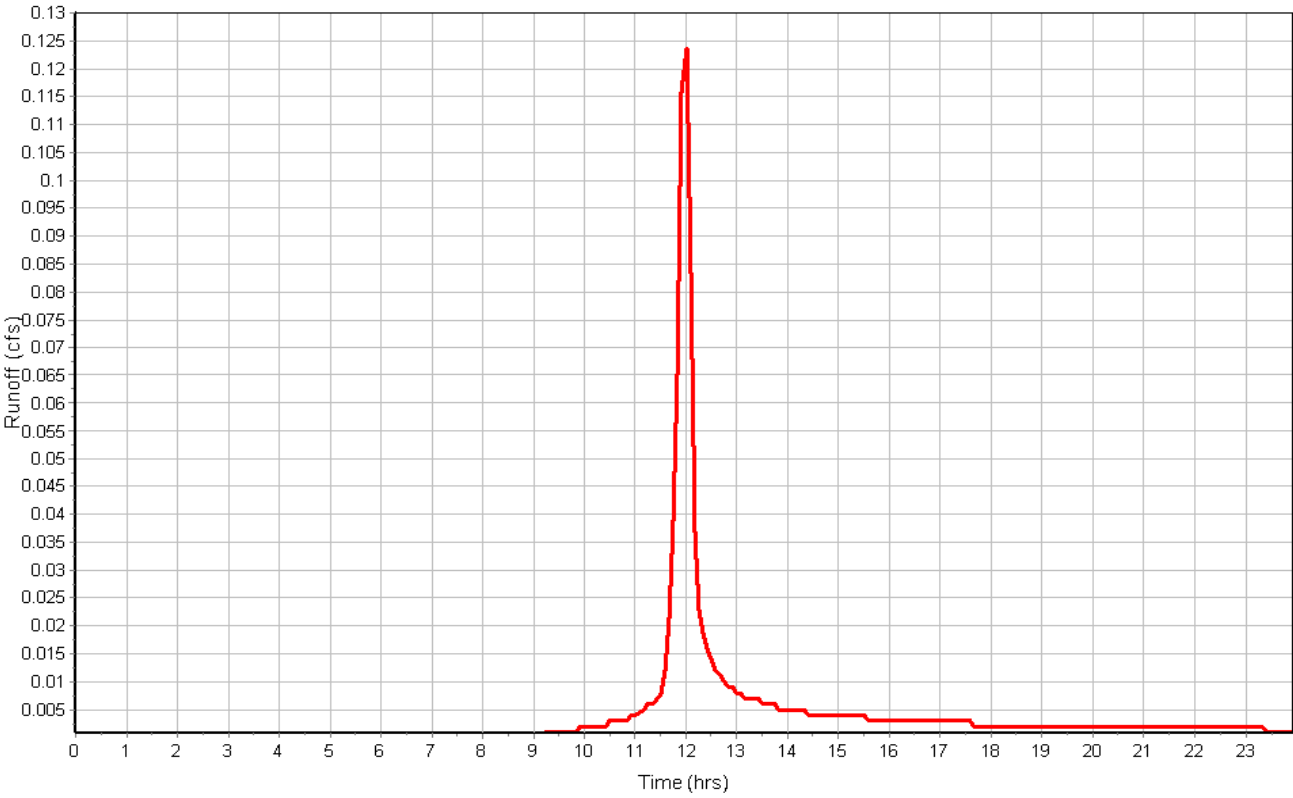
Total Rainfall (in) 7.70
Total Runoff (in) 4.61
Peak Runoff (cfs) 0.13
Weighted Curve Number 74.00
Time of Concentration (days hh:mm:ss) 0 00:08:34

Subbasin : ExOffsite

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : ExSE

Input Data

Area (ac) 1.04
Weighted Curve Number 83.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/4 acre lots, 38% impervious	1.04	C	83.00
Composite Area & Weighted CN	1.04		83.00

Time of Concentration

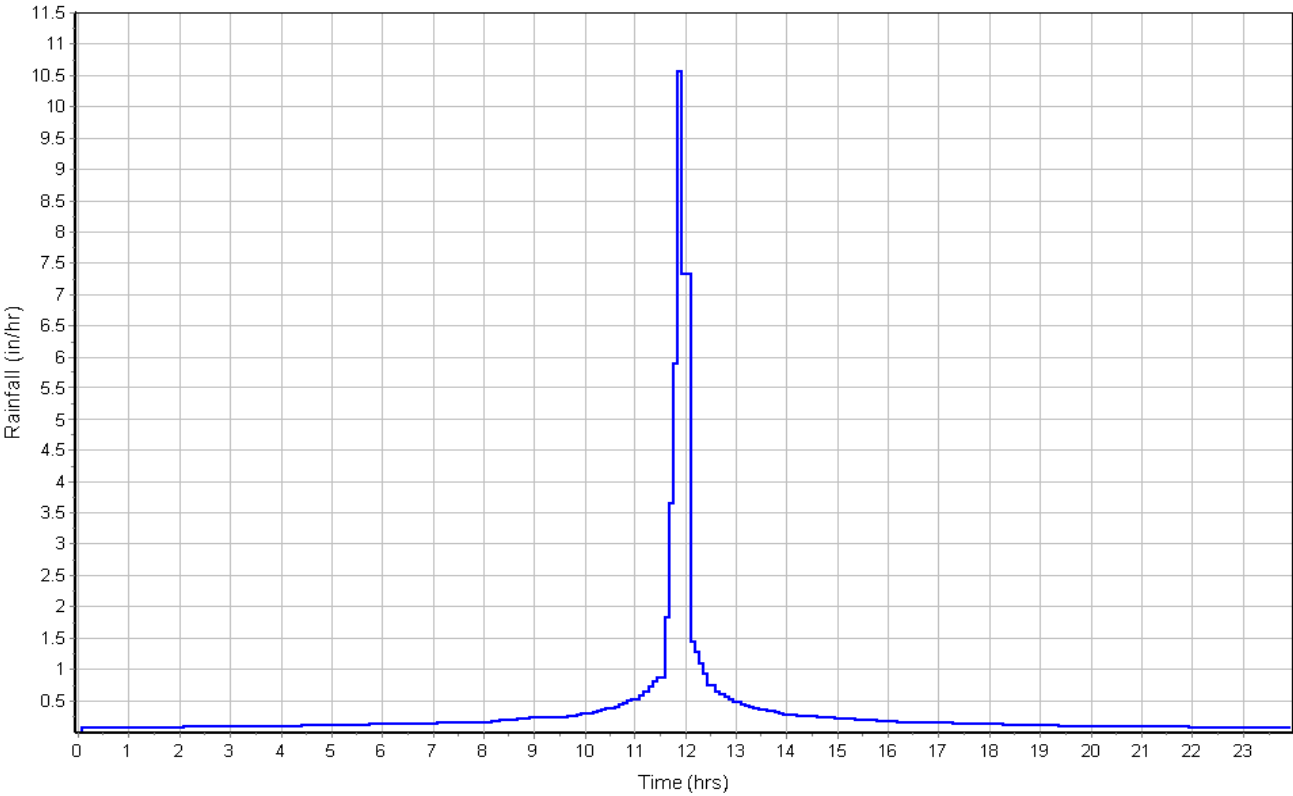
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	100	0.00	0.00
Slope (%) :	3.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.12	0.00	0.00
Computed Flow Time (min) :	13.87	0.00	0.00
Shallow Concentrated Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Flow Length (ft) :	150	0.00	0.00
Slope (%) :	3.7	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.35	0.00	0.00
Computed Flow Time (min) :	1.85	0.00	0.00
Total TOC (min)	15.72		

Subbasin Runoff Results

Total Rainfall (in) 7.70
Total Runoff (in) 5.69
Peak Runoff (cfs) 7.04
Weighted Curve Number 83.00
Time of Concentration (days hh:mm:ss) 0 00:15:43

Subbasin : ExSE

Rainfall Intensity Graph



Runoff Hydrograph

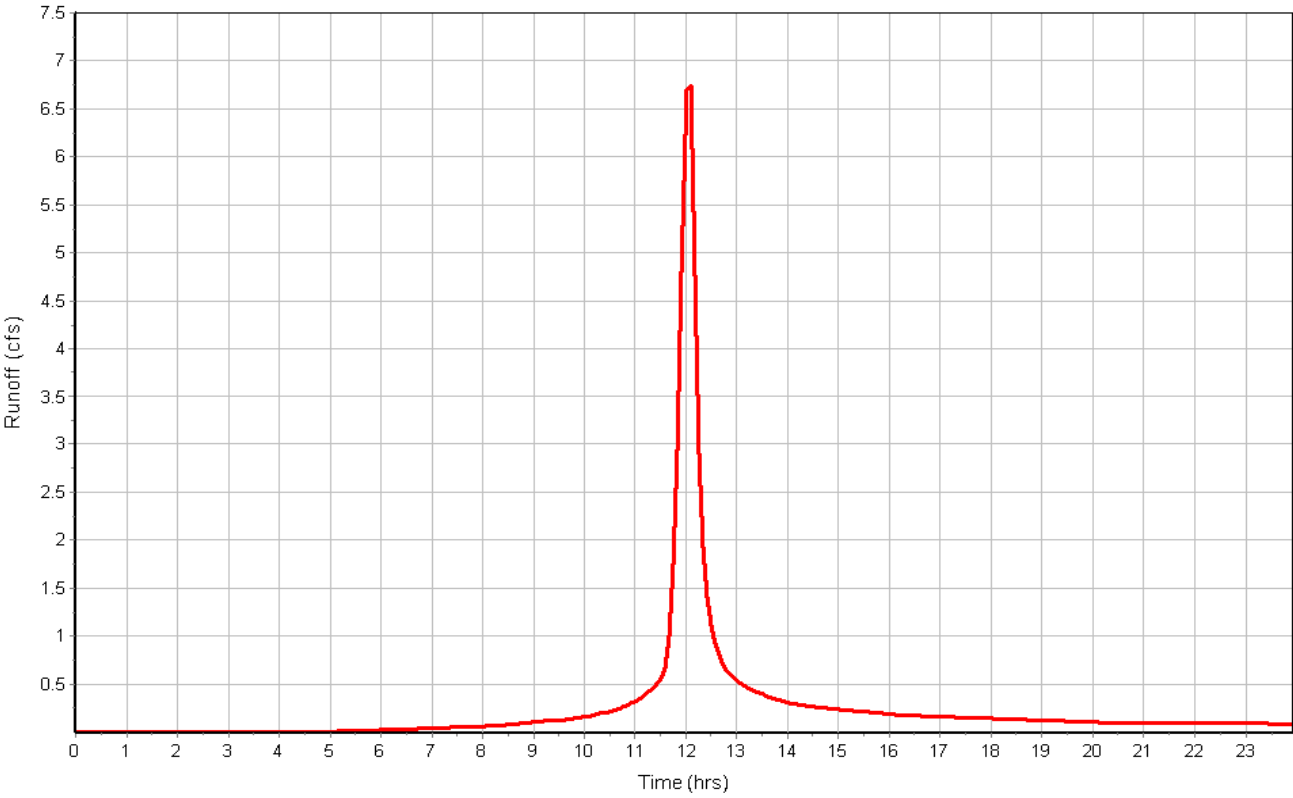
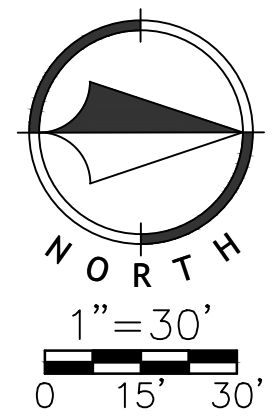
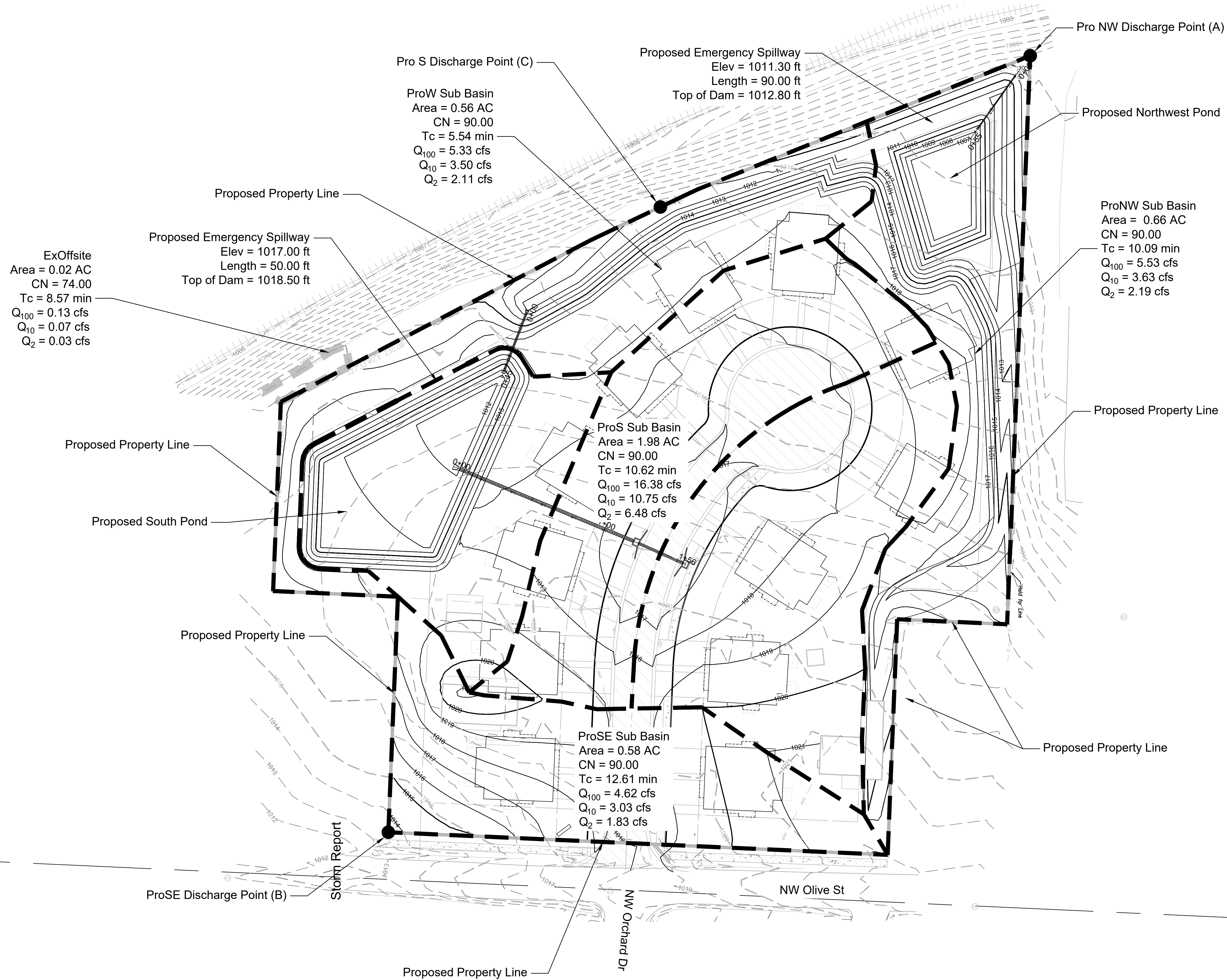


Exhibit F

Proposed Drainage Map

May 01, 2020 10:10am
2:16 PM Design 2018-05-02 18:02:51 Burton Townhomes Lees Summit/Office/Storm Report/DWG18-0251 - SWR.dwg



NO.	BY	CD	DATE	ORIGINAL SUBMISSION	REVISION
1	JGD	MES	5/1/20		

Exhibit G

Proposed Conditions Analysis

Project Description

File Name 18-0251 Proposed.SPF

Project Options

Flow Units CFS
Elevation Type Elevation
Hydrology Method SCS TR-55
Time of Concentration (TOC) Method SCS TR-55
Link Routing Method Kinematic Wave
Enable Overflow Ponding at Nodes YES
Skip Steady State Analysis Time Periods ... NO

Analysis Options

Start Analysis On Oct 28, 2019 00:00:00
End Analysis On Oct 29, 2019 00:00:00
Start Reporting On Oct 28, 2019 00:00:00
Antecedent Dry Days 0 days
Runoff (Dry Weather) Time Step 0 01:00:00 days hh:mm:ss
Runoff (Wet Weather) Time Step 0 00:05:00 days hh:mm:ss
Reporting Time Step 0 00:05:00 days hh:mm:ss
Routing Time Step 30 seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins.....	4
Nodes.....	4
<i>Junctions</i>	0
<i>Outfalls</i>	4
<i>Flow Diversions</i>	0
<i>Inlets</i>	0
<i>Storage Nodes</i>	0
Links.....	0
<i>Channels</i>	0
<i>Pipes</i>	0
<i>Pumps</i>	0
<i>Orifices</i>	0
<i>Weirs</i>	0
<i>Outlets</i>	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	2-yr	Cumulative	inches	Missouri	Jackson	2	3.50	SCS Type II 24-hr

Subbasin Summary

SN	Subbasin ID	Area	Weighted Curve Number	Total Rainfall	Total Runoff	Total Runoff Volume	Peak Runoff	Time of Concentration
		(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1	Pro-NW	0.66	90.00	3.50	2.45	1.62	2.19	0 00:10:05
2	Pro-S	1.98	90.00	3.50	2.45	4.85	6.48	0 00:10:37
3	Pro-SE	0.58	90.00	3.50	2.45	1.42	1.83	0 00:12:36
4	Pro-W	0.56	90.00	3.50	2.45	1.37	2.11	0 00:05:32

Node Summary

SN	Element ID	Element Type	Invert Elevation	Ground/Rim (Max) Elevation	Initial Water Elevation	Surcharge Elevation	Ponded Area	Peak Inflow	Max HGL Elevation Attained	Max Surcharge Depth Attained	Min Freeboard Attained	Time of Peak Flooding Occurrence	Total Flooded Volume	Total Time Flooded
			(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	(ft)	(days hh:mm)	(ac-in)	(min)
1	Out-NW	Outfall	1008.00					0.00	0.00					
2	Out-S	Outfall	1013.75					0.00	0.00					
3	Out-SE	Outfall	0.00					0.00	0.00					
4	Out-W	Outfall	0.00					0.00	0.00					

Subbasin Hydrology

Subbasin : Pro-NW

Input Data

Area (ac) 0.66
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	0.70	C	90.00
Composite Area & Weighted CN	0.70		90.00

Time of Concentration

TOC Method : SCS TR-55

Sheet Flow Equation :

$T_c = (0.007 * ((n * L_f)^{0.8})) / ((P^{0.5}) * (S_f^{0.4}))$

Where :

Tc = Time of Concentration (hr)
n = Manning's roughness
Lf = Flow Length (ft)
P = 2 yr, 24 hr Rainfall (inches)
Sf = Slope (ft/ft)

Shallow Concentrated Flow Equation :

V = 16.1345 * (Sf^0.5) (unpaved surface)
V = 20.3282 * (Sf^0.5) (paved surface)
V = 15.0 * (Sf^0.5) (grassed waterway surface)
V = 10.0 * (Sf^0.5) (nearly bare & untilled surface)
V = 9.0 * (Sf^0.5) (cultivated straight rows surface)
V = 7.0 * (Sf^0.5) (short grass pasture surface)
V = 5.0 * (Sf^0.5) (woodland surface)
V = 2.5 * (Sf^0.5) (forest w/heavy litter surface)
Tc = (Lf / V) / (3600 sec/hr)

Where:

Tc = Time of Concentration (hr)
Lf = Flow Length (ft)
V = Velocity (ft/sec)
Sf = Slope (ft/ft)

Channel Flow Equation :

$V = (1.49 * (R^{2/3}) * (S_f^{0.5})) / n$
R = Aq / Wp
 $T_c = (L_f / V) / (3600 \text{ sec/hr})$

Where :

Tc = Time of Concentration (hr)
Lf = Flow Length (ft)
R = Hydraulic Radius (ft)
Aq = Flow Area (ft²)
Wp = Wetted Perimeter (ft)
V = Velocity (ft/sec)
Sf = Slope (ft/ft)
n = Manning's roughness

	Subarea	Subarea	Subarea
	A	B	C
Sheet Flow Computations			
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	40	0.00	0.00
Slope (%) :	2.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.09	0.00	0.00
Computed Flow Time (min) :	7.84	0.00	0.00

	Subarea	Subarea	Subarea
	A	B	C
Shallow Concentrated Flow Computations			
Flow Length (ft) :	110	0.00	0.00
Slope (%) :	2.1	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.01	0.00	0.00
Computed Flow Time (min) :	1.82	0.00	0.00

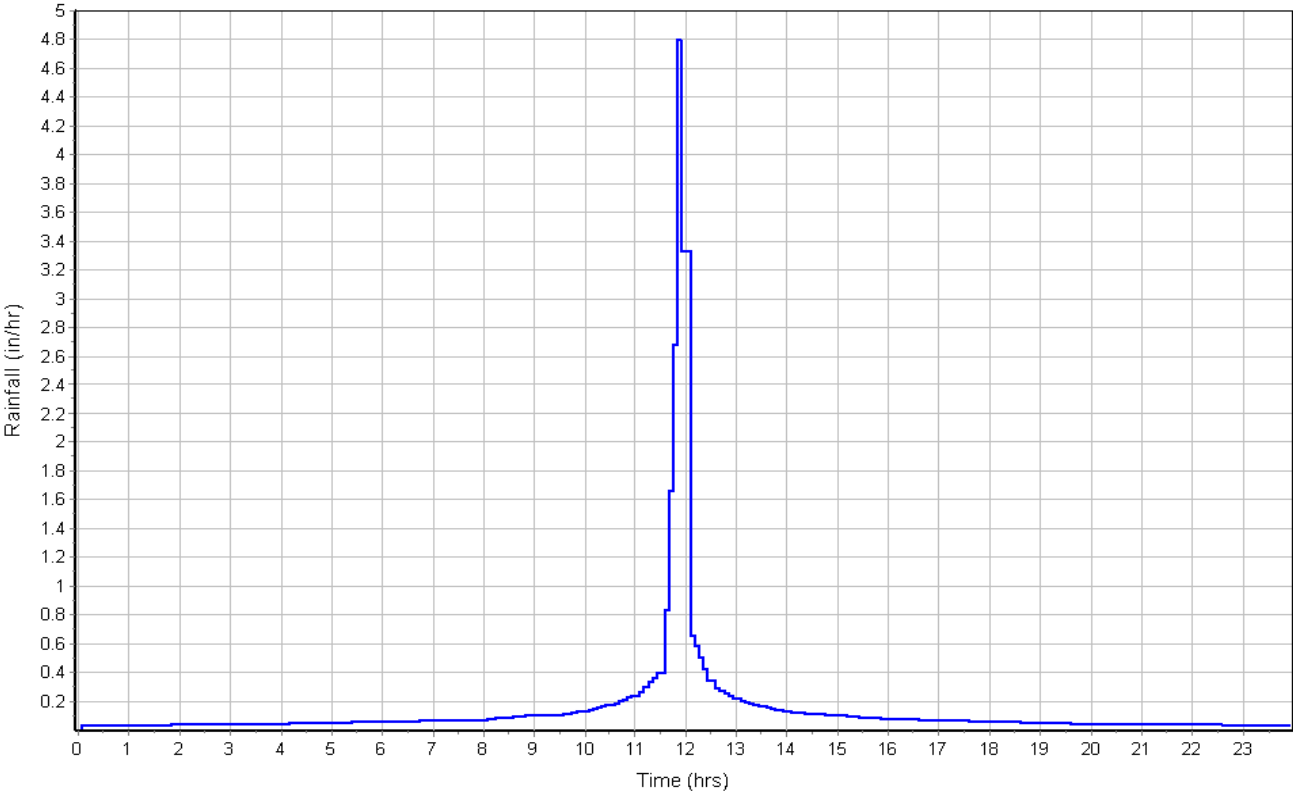
	Subarea	Subarea	Subarea
	A	B	C
Channel Flow Computations			
Manning's Roughness :	0.018	0.00	0.00
Flow Length (ft) :	310	0.00	0.00
Channel Slope (%) :	2.0	0.00	0.00
Cross Section Area (ft²) :	15	0.00	0.00
Wetted Perimeter (ft) :	15	0.00	0.00
Velocity (ft/sec) :	11.71	0.00	0.00
Computed Flow Time (min) :	0.44	0.00	0.00
Total TOC (min)	10.09		

Subbasin Runoff Results

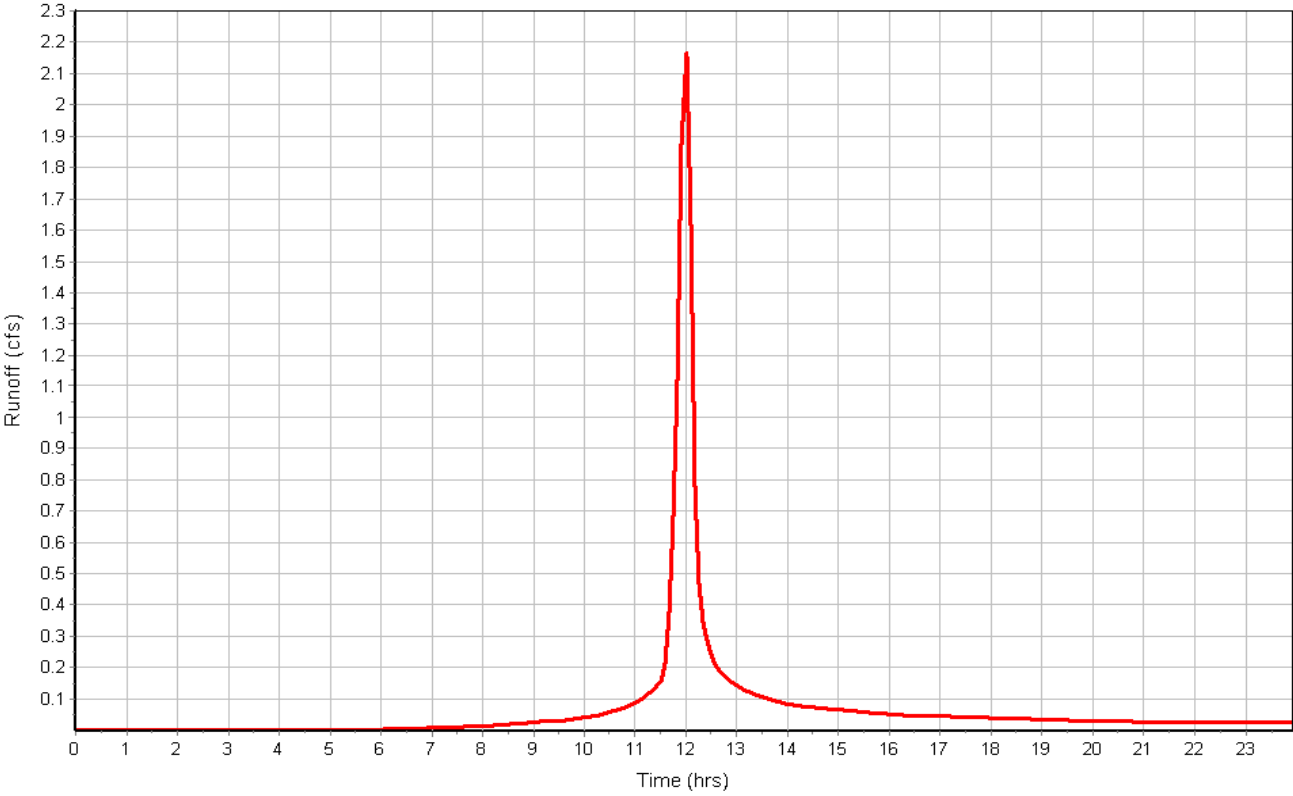
Total Rainfall (in)	3.50
Total Runoff (in)	2.45
Peak Runoff (cfs)	2.19
Weighted Curve Number	90.00
Time of Concentration (days hh:mm:ss)	0 00:10:05

Subbasin : Pro-NW

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : Pro-S

Input Data

Area (ac) 1.98
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	1.99	C	90.00
Composite Area & Weighted CN	1.99		90.00

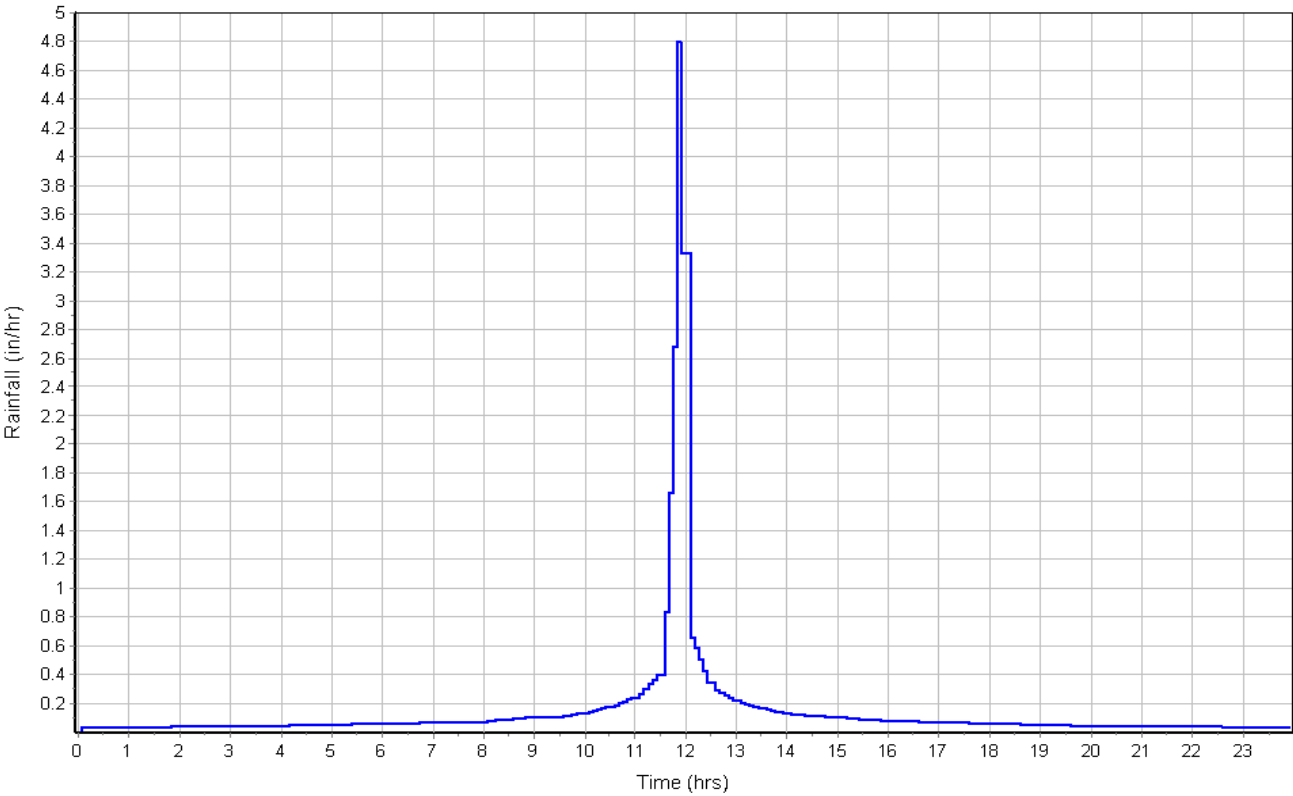
Time of Concentration

	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	50	0.00	0.00
Slope (%) :	2.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.09	0.00	0.00
Computed Flow Time (min) :	9.37	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	175	0.00	0.00
Slope (%) :	2.0	0.00	0.00
Surface Type :	Paved	Unpaved	Unpaved
Velocity (ft/sec) :	2.87	0.00	0.00
Computed Flow Time (min) :	1.02	0.00	0.00
Channel Flow Computations			
Manning's Roughness :	0.012	0.00	0.00
Flow Length (ft) :	125	0.00	0.00
Channel Slope (%) :	1.5	0.00	0.00
Cross Section Area (ft²) :	1.33	0.00	0.00
Wetted Perimeter (ft) :	3.0	0.00	0.00
Velocity (ft/sec) :	8.84	0.00	0.00
Computed Flow Time (min) :	0.24	0.00	0.00
Total TOC (min)	10.62		

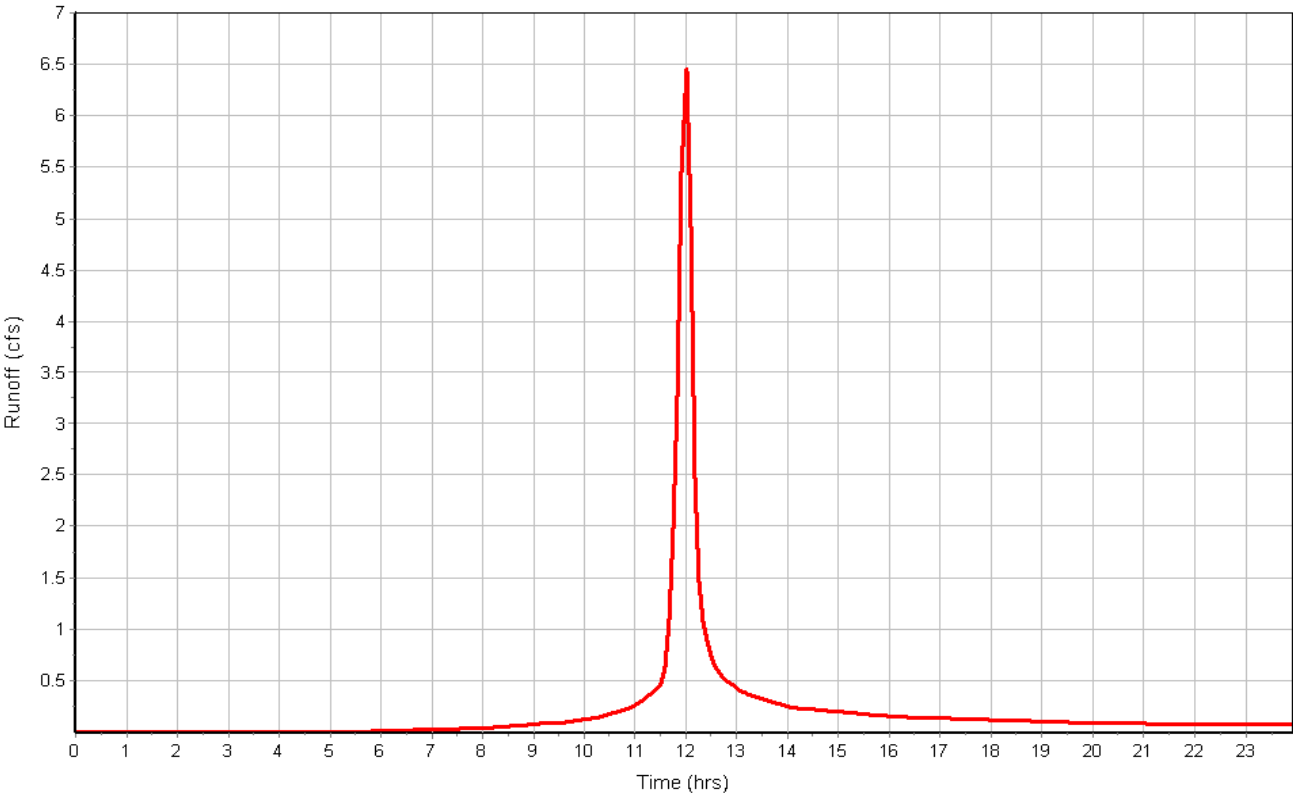
Subbasin Runoff Results

Total Rainfall (in) 3.50
Total Runoff (in) 2.45
Peak Runoff (cfs) 6.48
Weighted Curve Number 90.00
Time of Concentration (days hh:mm:ss) 0 00:10:37

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : Pro-SE

Input Data

Area (ac) 0.58
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	0.62	C	90.00
Composite Area & Weighted CN	0.62		90.00

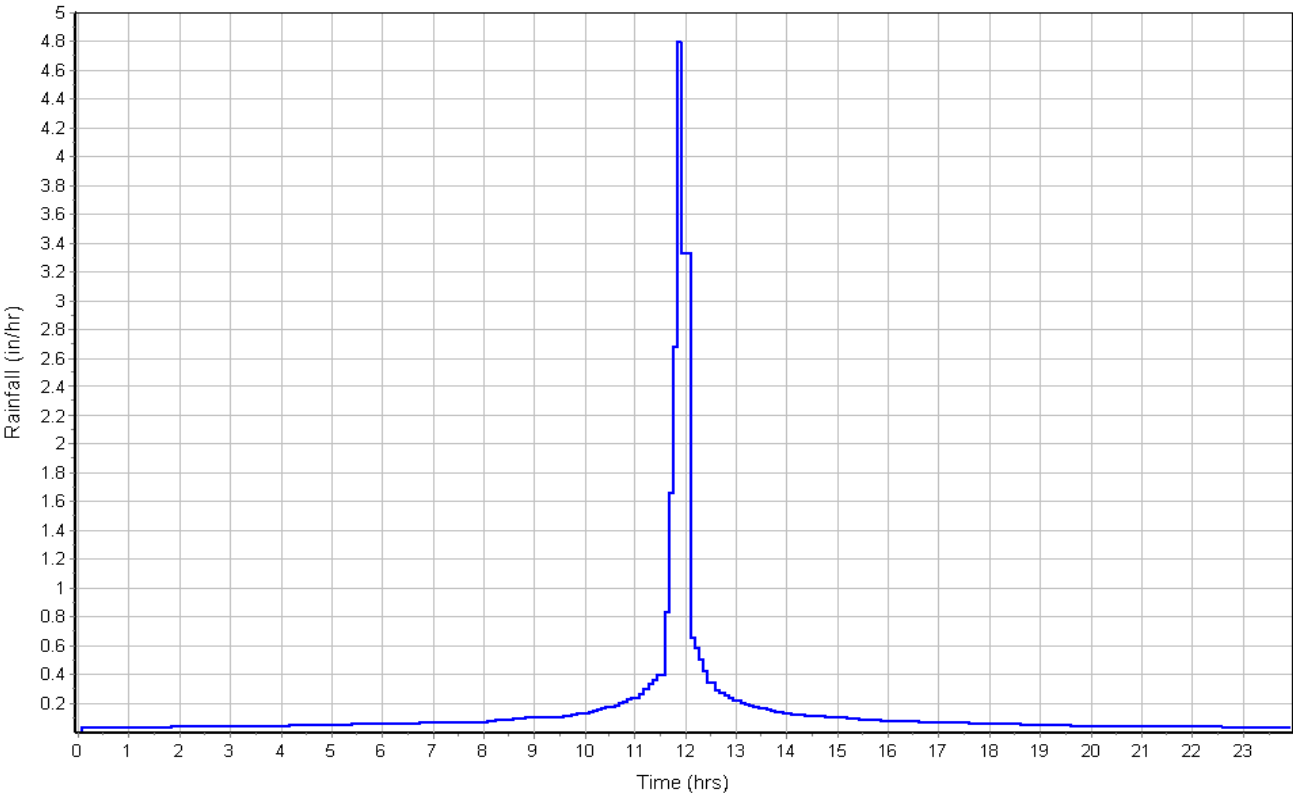
Time of Concentration

	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	70	0.00	0.00
Slope (%) :	2.5	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.10	0.00	0.00
Computed Flow Time (min) :	11.22	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	110	0.00	0.00
Slope (%) :	3.5	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.31	0.00	0.00
Computed Flow Time (min) :	1.40	0.00	0.00
Total TOC (min)	12.61		

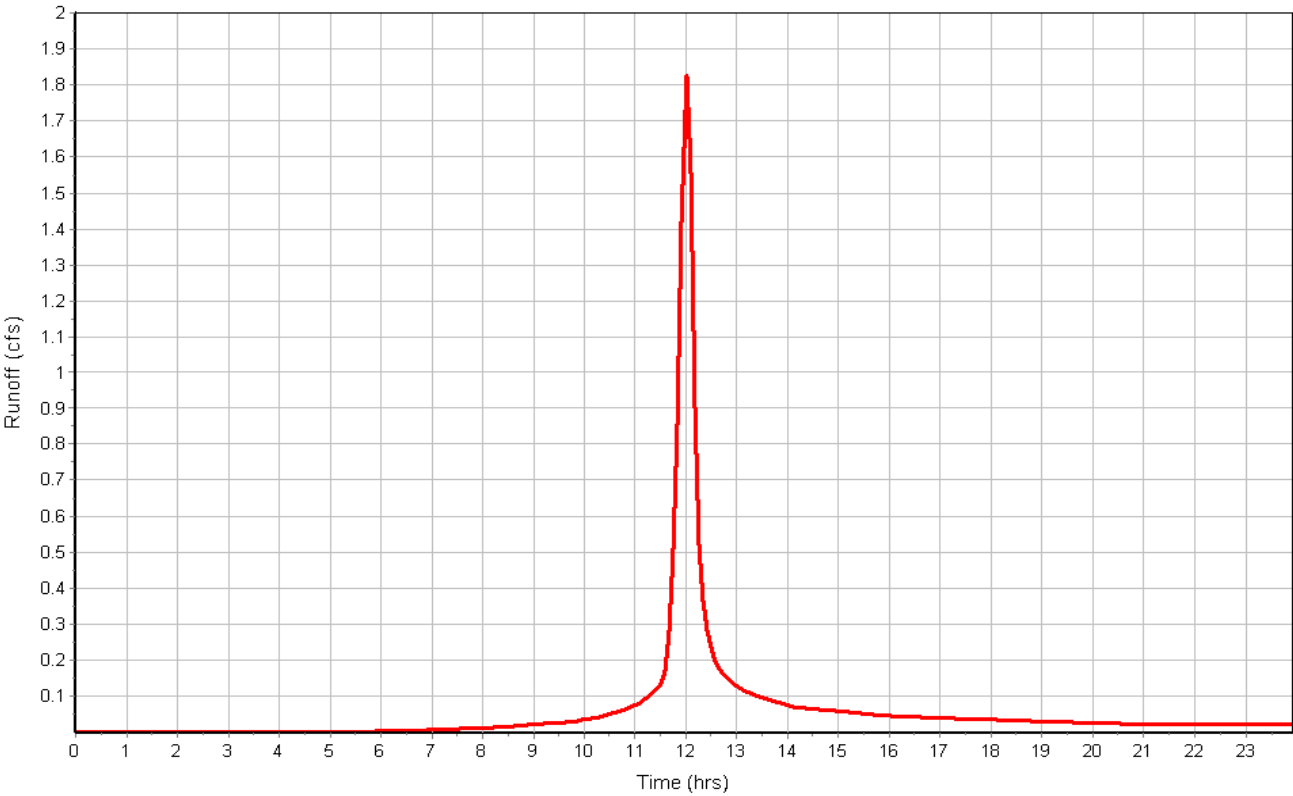
Subbasin Runoff Results

Total Rainfall (in) 3.50
Total Runoff (in) 2.45
Peak Runoff (cfs) 1.83
Weighted Curve Number 90.00
Time of Concentration (days hh:mm:ss) 0 00:12:37

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : Pro-W

Input Data

Area (ac) 0.56
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	0.46	C	90.00
Composite Area & Weighted CN	0.46		90.00

Time of Concentration

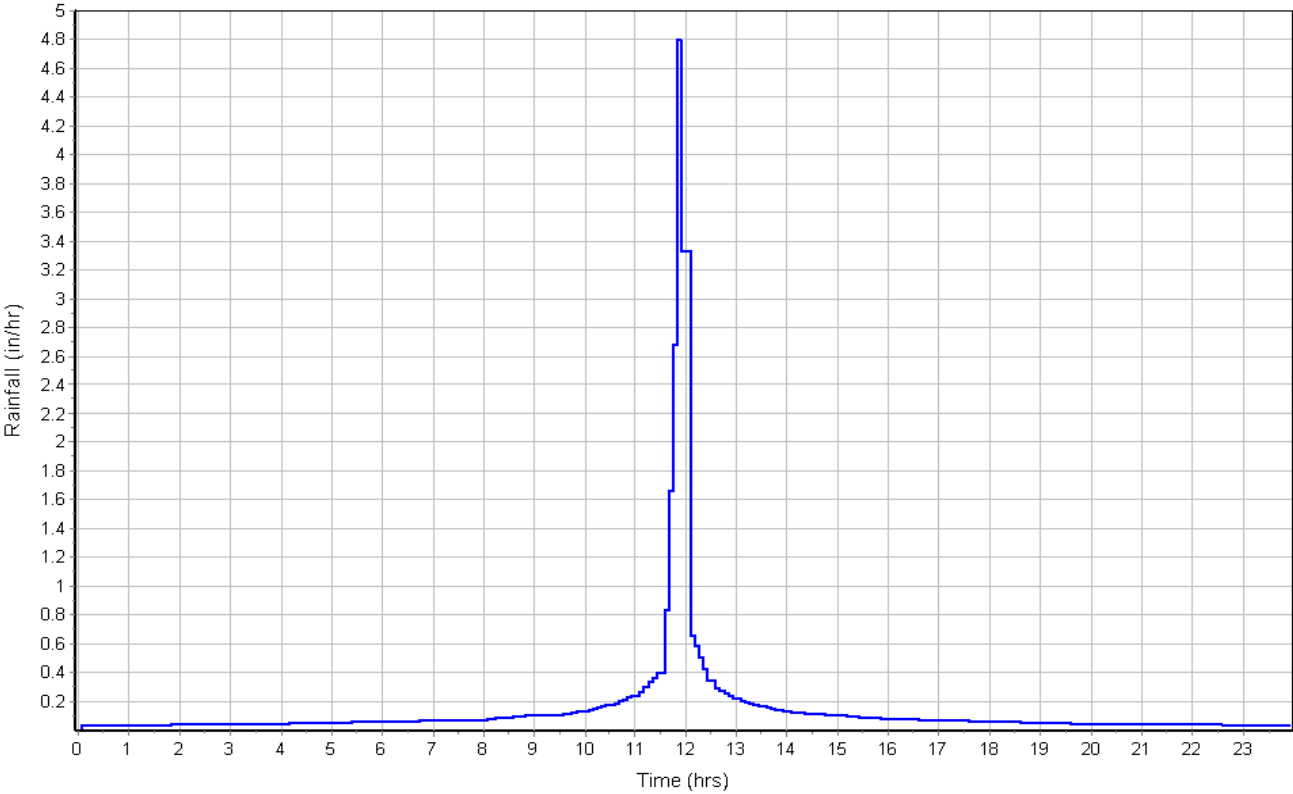
	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	29	0.00	0.00
Slope (%) :	4.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.11	0.00	0.00
Computed Flow Time (min) :	4.59	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	105	0.00	0.00
Slope (%) :	7.0	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.85	0.00	0.00
Computed Flow Time (min) :	0.95	0.00	0.00
Total TOC (min)	5.54		

Subbasin Runoff Results

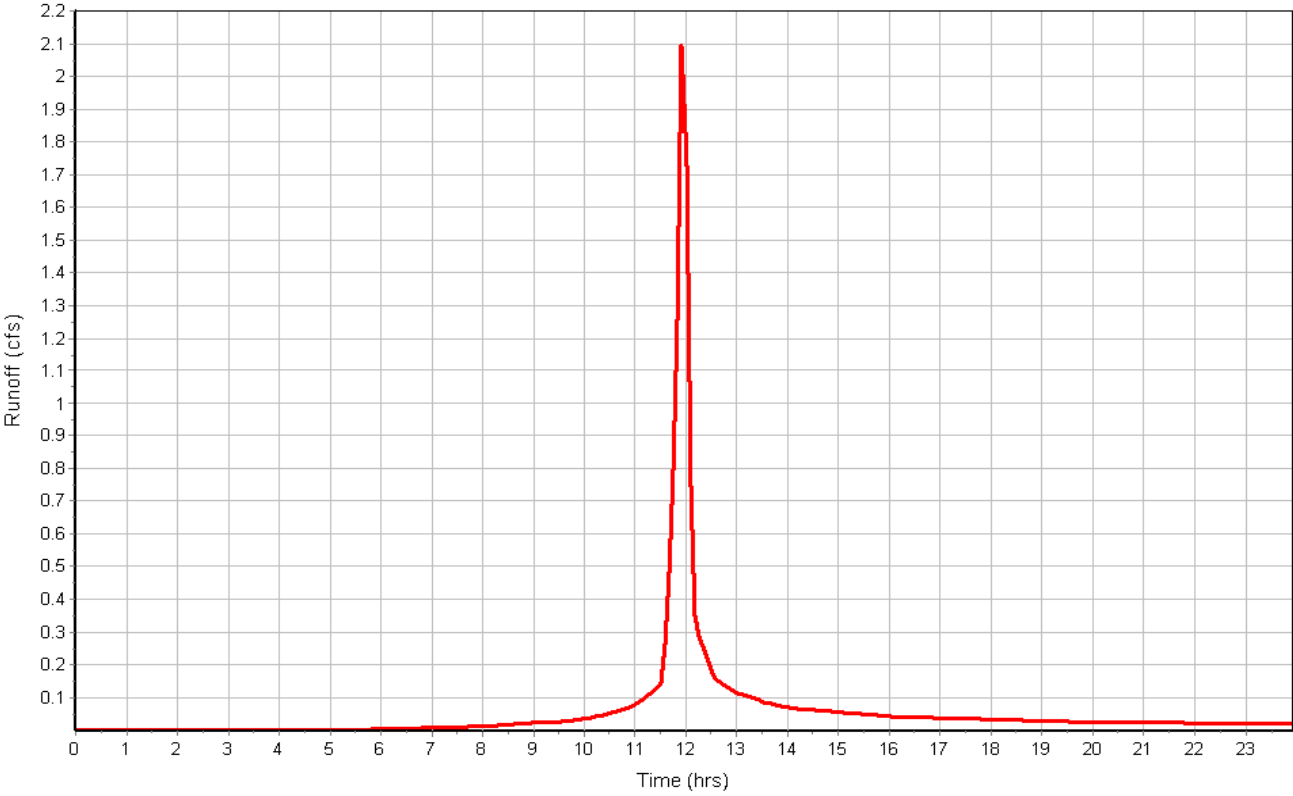
Total Rainfall (in) 3.50
Total Runoff (in) 2.45
Peak Runoff (cfs) 2.11
Weighted Curve Number 90.00
Time of Concentration (days hh:mm:ss) 0 00:05:32

Subbasin : Pro-W

Rainfall Intensity Graph



Runoff Hydrograph



Project Description

File Name 18-0251 Proposed.SPF

Project Options

Flow Units CFS
Elevation Type Elevation
Hydrology Method SCS TR-55
Time of Concentration (TOC) Method SCS TR-55
Link Routing Method Kinematic Wave
Enable Overflow Ponding at Nodes YES
Skip Steady State Analysis Time Periods ... NO

Analysis Options

Start Analysis On Oct 28, 2019 00:00:00
End Analysis On Oct 29, 2019 00:00:00
Start Reporting On Oct 28, 2019 00:00:00
Antecedent Dry Days 0 days
Runoff (Dry Weather) Time Step 0 01:00:00 days hh:mm:ss
Runoff (Wet Weather) Time Step 0 00:05:00 days hh:mm:ss
Reporting Time Step 0 00:05:00 days hh:mm:ss
Routing Time Step 30 seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins.....	4
Nodes.....	4
<i>Junctions</i>	0
<i>Outfalls</i>	4
<i>Flow Diversions</i>	0
<i>Inlets</i>	0
<i>Storage Nodes</i>	0
Links.....	0
<i>Channels</i>	0
<i>Pipes</i>	0
<i>Pumps</i>	0
<i>Orifices</i>	0
<i>Weirs</i>	0
<i>Outlets</i>	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	10-yr	Cumulative	inches	Missouri	Jackson	10	5.30	SCS Type II 24-hr

Subbasin Summary

SN	Subbasin ID	Area	Weighted Curve Number	Total Rainfall	Total Runoff	Total Runoff Volume	Peak Runoff	Time of Concentration
		(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1	Pro-NW	0.66	90.00	5.30	4.17	2.75	3.63	0 00:10:05
2	Pro-S	1.98	90.00	5.30	4.17	8.25	10.75	0 00:10:37
3	Pro-SE	0.58	90.00	5.30	4.17	2.42	3.03	0 00:12:36
4	Pro-W	0.56	90.00	5.30	4.17	2.33	3.50	0 00:05:32

Node Summary

SN	Element ID	Element Type	Invert Elevation	Ground/Rim (Max) Elevation	Initial Water Elevation	Surcharge Elevation	Ponded Area	Peak Inflow	Max HGL Elevation Attained	Max Surcharge Depth Attained	Min Freeboard Attained	Time of Peak Flooding Occurrence	Total Flooded Volume	Total Time Flooded
			(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	(ft)	(days hh:mm)	(ac-in)	(min)
1	Out-NW	Outfall	1008.00					0.00	0.00					
2	Out-S	Outfall	1013.75					0.00	0.00					
3	Out-SE	Outfall	0.00					0.00	0.00					
4	Out-W	Outfall	0.00					0.00	0.00					

Subbasin Hydrology

Subbasin : Pro-NW

Input Data

Area (ac) 0.66
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	0.70	C	90.00
Composite Area & Weighted CN	0.70		90.00

Time of Concentration

TOC Method : SCS TR-55

Sheet Flow Equation :

$$T_c = (0.007 * ((n * L_f)^{0.8})) / ((P^{0.5}) * (S_f^{0.4}))$$

Where :

T_c = Time of Concentration (hr)
n = Manning's roughness
L_f = Flow Length (ft)
P = 2 yr, 24 hr Rainfall (inches)
S_f = Slope (ft/ft)

Shallow Concentrated Flow Equation :

V = 16.1345 * (S_f^{0.5}) (unpaved surface)
V = 20.3282 * (S_f^{0.5}) (paved surface)
V = 15.0 * (S_f^{0.5}) (grassed waterway surface)
V = 10.0 * (S_f^{0.5}) (nearly bare & untilled surface)
V = 9.0 * (S_f^{0.5}) (cultivated straight rows surface)
V = 7.0 * (S_f^{0.5}) (short grass pasture surface)
V = 5.0 * (S_f^{0.5}) (woodland surface)
V = 2.5 * (S_f^{0.5}) (forest w/heavy litter surface)
T_c = (L_f / V) / (3600 sec/hr)

Where:

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)

Channel Flow Equation :

V = (1.49 * (R^{2/3}) * (S_f^{0.5})) / n
R = A_q / W_p
T_c = (L_f / V) / (3600 sec/hr)

Where :

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
R = Hydraulic Radius (ft)
A_q = Flow Area (ft²)
W_p = Wetted Perimeter (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)
n = Manning's roughness

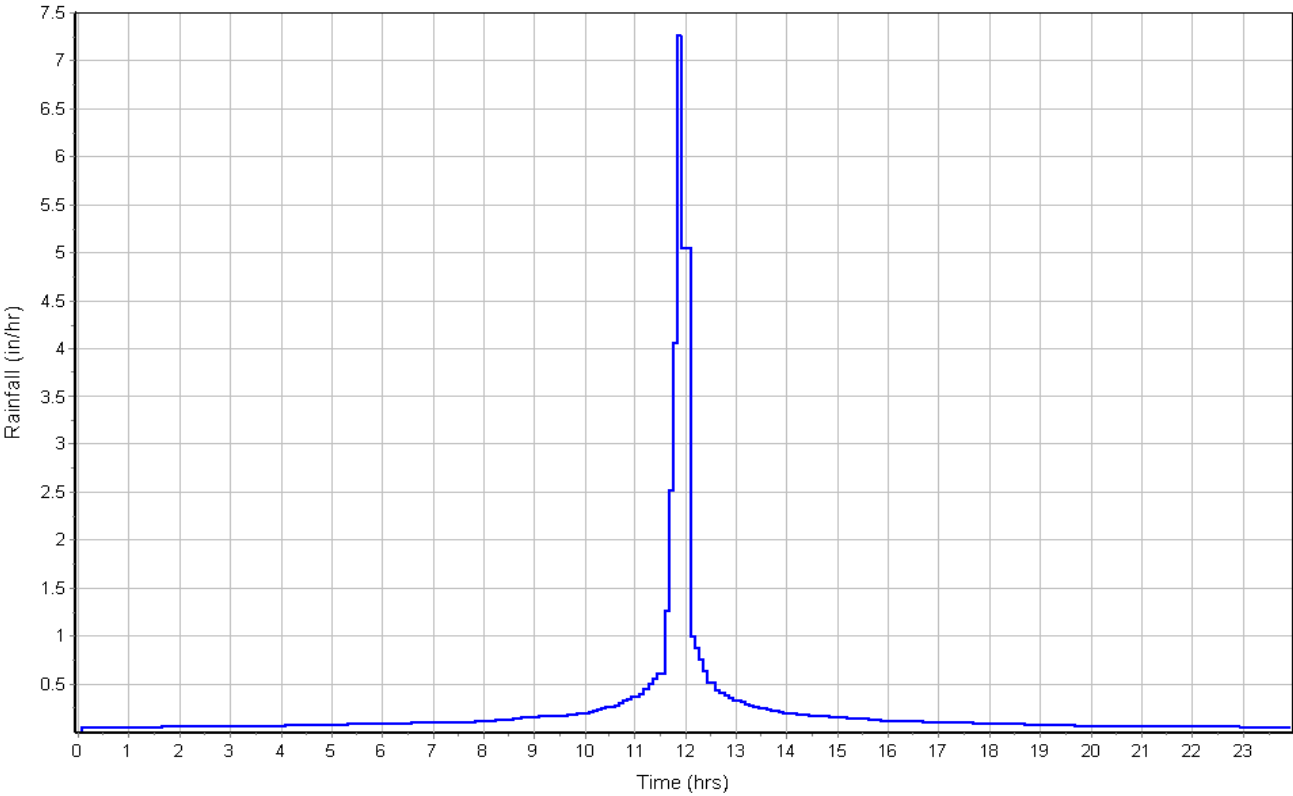
	Subarea	Subarea	Subarea
	A	B	C
Sheet Flow Computations			
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	40	0.00	0.00
Slope (%) :	2.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.09	0.00	0.00
Computed Flow Time (min) :	7.84	0.00	0.00
	Subarea	Subarea	Subarea
	A	B	C
Shallow Concentrated Flow Computations			
Flow Length (ft) :	110	0.00	0.00
Slope (%) :	2.1	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.01	0.00	0.00
Computed Flow Time (min) :	1.82	0.00	0.00
	Subarea	Subarea	Subarea
	A	B	C
Channel Flow Computations			
Manning's Roughness :	0.018	0.00	0.00
Flow Length (ft) :	310	0.00	0.00
Channel Slope (%) :	2.0	0.00	0.00
Cross Section Area (ft²) :	15	0.00	0.00
Wetted Perimeter (ft) :	15	0.00	0.00
Velocity (ft/sec) :	11.71	0.00	0.00
Computed Flow Time (min) :	0.44	0.00	0.00
Total TOC (min)	10.09		

Subbasin Runoff Results

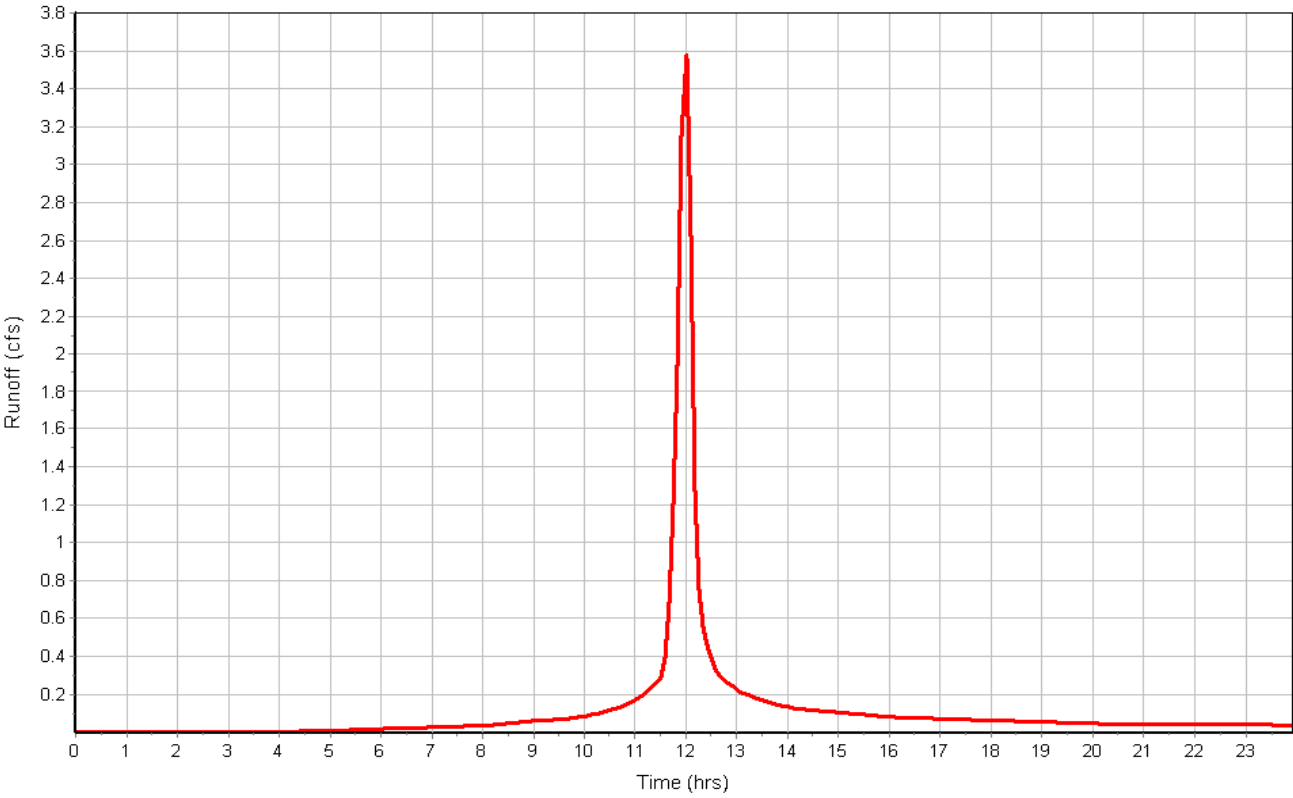
Total Rainfall (in)	5.30
Total Runoff (in)	4.17
Peak Runoff (cfs)	3.63
Weighted Curve Number	90.00
Time of Concentration (days hh:mm:ss)	0 00:10:05

Subbasin : Pro-NW

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : Pro-S

Input Data

Area (ac) 1.98
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	1.99	C	90.00
Composite Area & Weighted CN	1.99		90.00

Time of Concentration

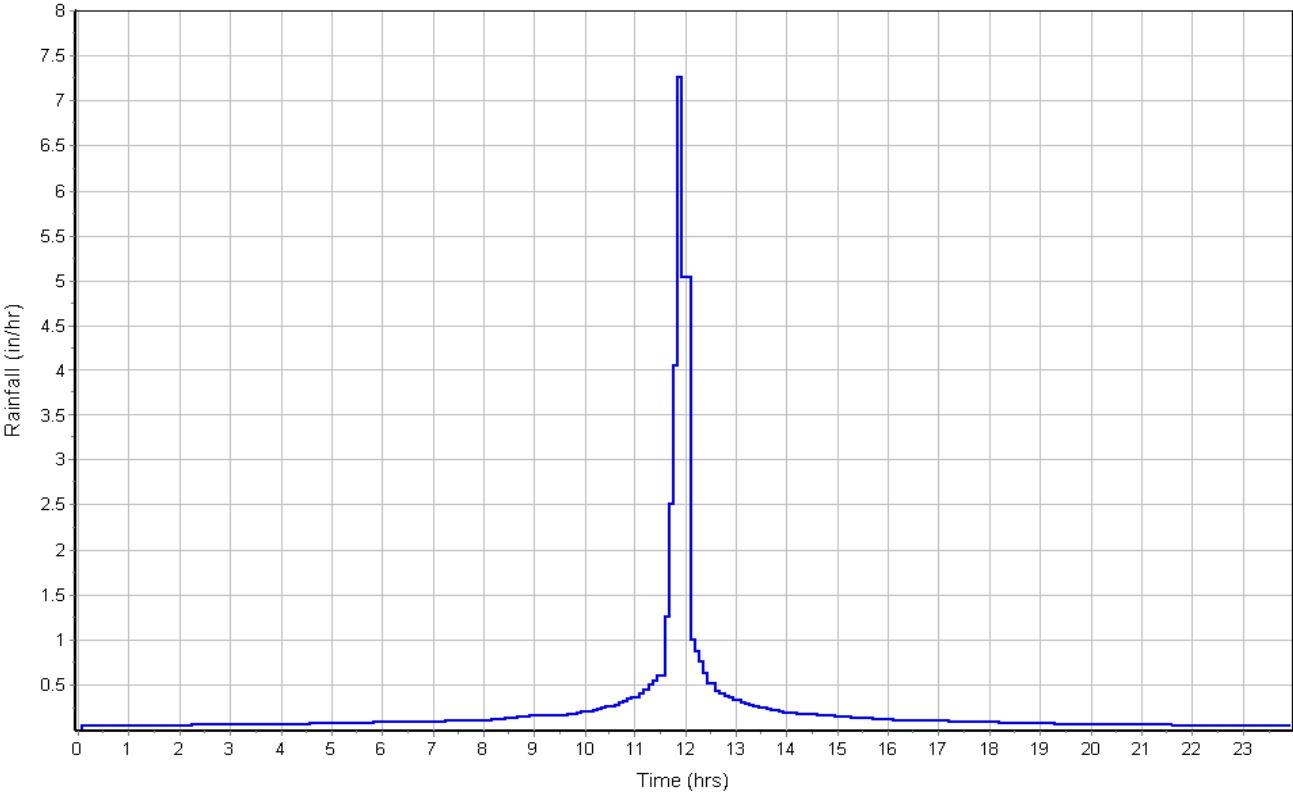
	Subarea	Subarea	Subarea
	A	B	C
Sheet Flow Computations			
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	50	0.00	0.00
Slope (%) :	2.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.09	0.00	0.00
Computed Flow Time (min) :	9.37	0.00	0.00
	Subarea	Subarea	Subarea
	A	B	C
Shallow Concentrated Flow Computations			
Flow Length (ft) :	175	0.00	0.00
Slope (%) :	2.0	0.00	0.00
Surface Type :	Paved	Unpaved	Unpaved
Velocity (ft/sec) :	2.87	0.00	0.00
Computed Flow Time (min) :	1.02	0.00	0.00
	Subarea	Subarea	Subarea
	A	B	C
Channel Flow Computations			
Manning's Roughness :	0.012	0.00	0.00
Flow Length (ft) :	125	0.00	0.00
Channel Slope (%) :	1.5	0.00	0.00
Cross Section Area (ft²) :	1.33	0.00	0.00
Wetted Perimeter (ft) :	3.0	0.00	0.00
Velocity (ft/sec) :	8.84	0.00	0.00
Computed Flow Time (min) :	0.24	0.00	0.00
Total TOC (min)	10.62		

Subbasin Runoff Results

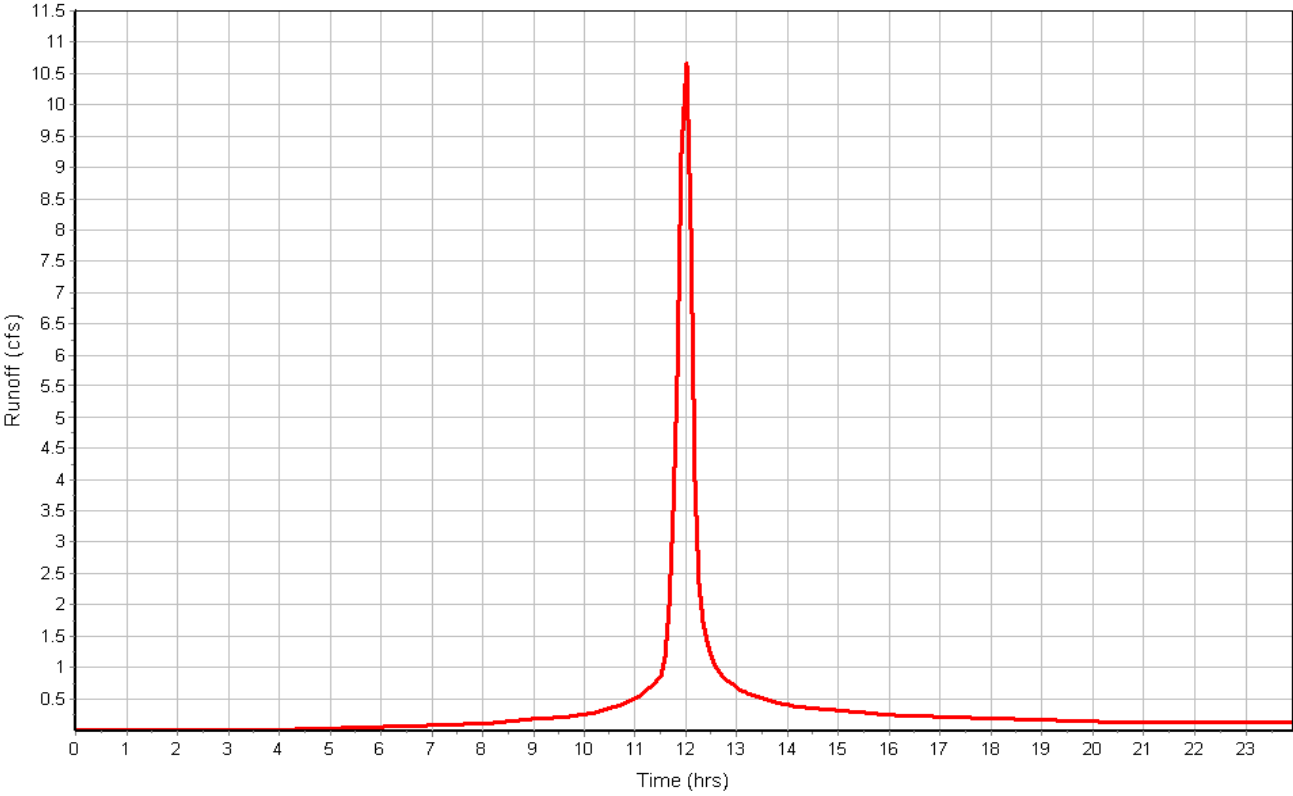
Total Rainfall (in) 5.30
Total Runoff (in) 4.17
Peak Runoff (cfs) 10.75
Weighted Curve Number 90.00
Time of Concentration (days hh:mm:ss) 0 00:10:37

Subbasin : Pro-S

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : Pro-SE

Input Data

Area (ac) 0.58
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	0.62	C	90.00
Composite Area & Weighted CN	0.62		90.00

Time of Concentration

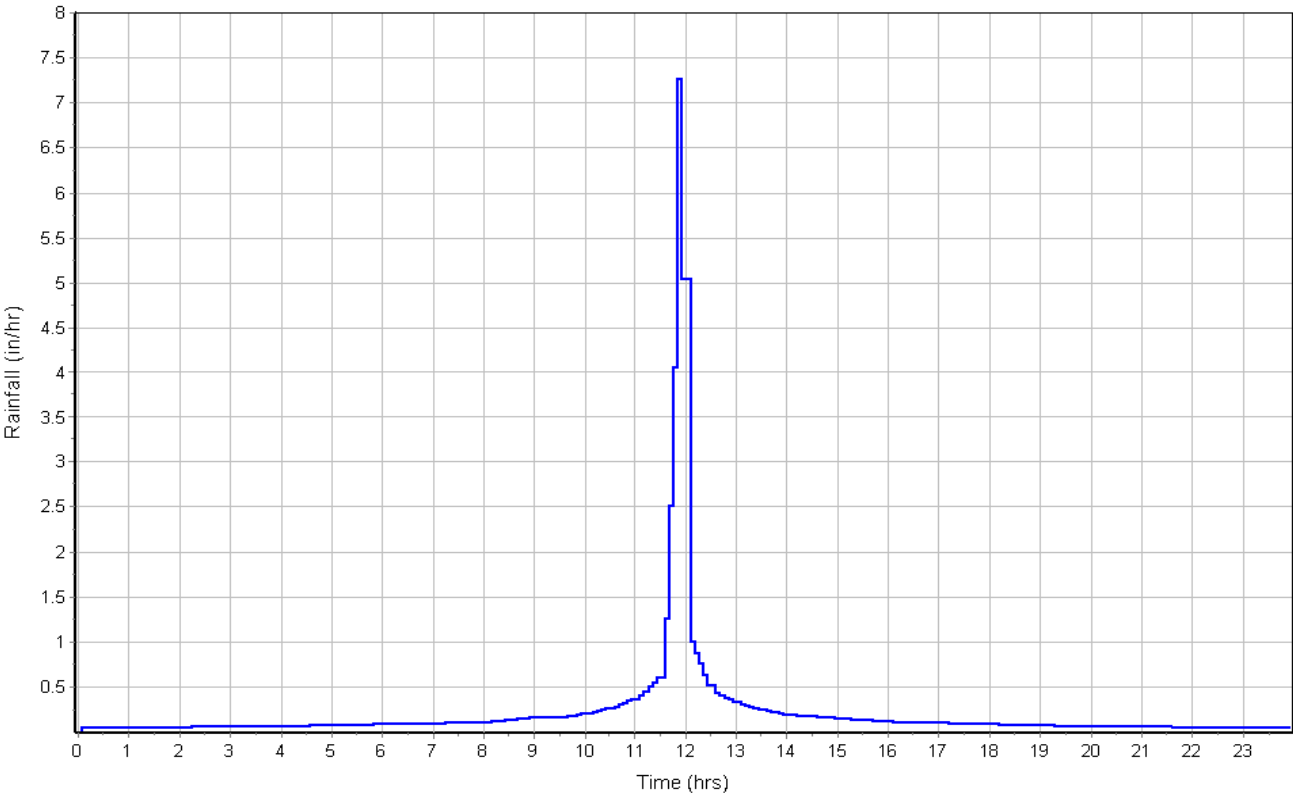
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	70	0.00	0.00
Slope (%) :	2.5	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.10	0.00	0.00
Computed Flow Time (min) :	11.22	0.00	0.00
Shallow Concentrated Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Flow Length (ft) :	110	0.00	0.00
Slope (%) :	3.5	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.31	0.00	0.00
Computed Flow Time (min) :	1.40	0.00	0.00
Total TOC (min)	12.61		

Subbasin Runoff Results

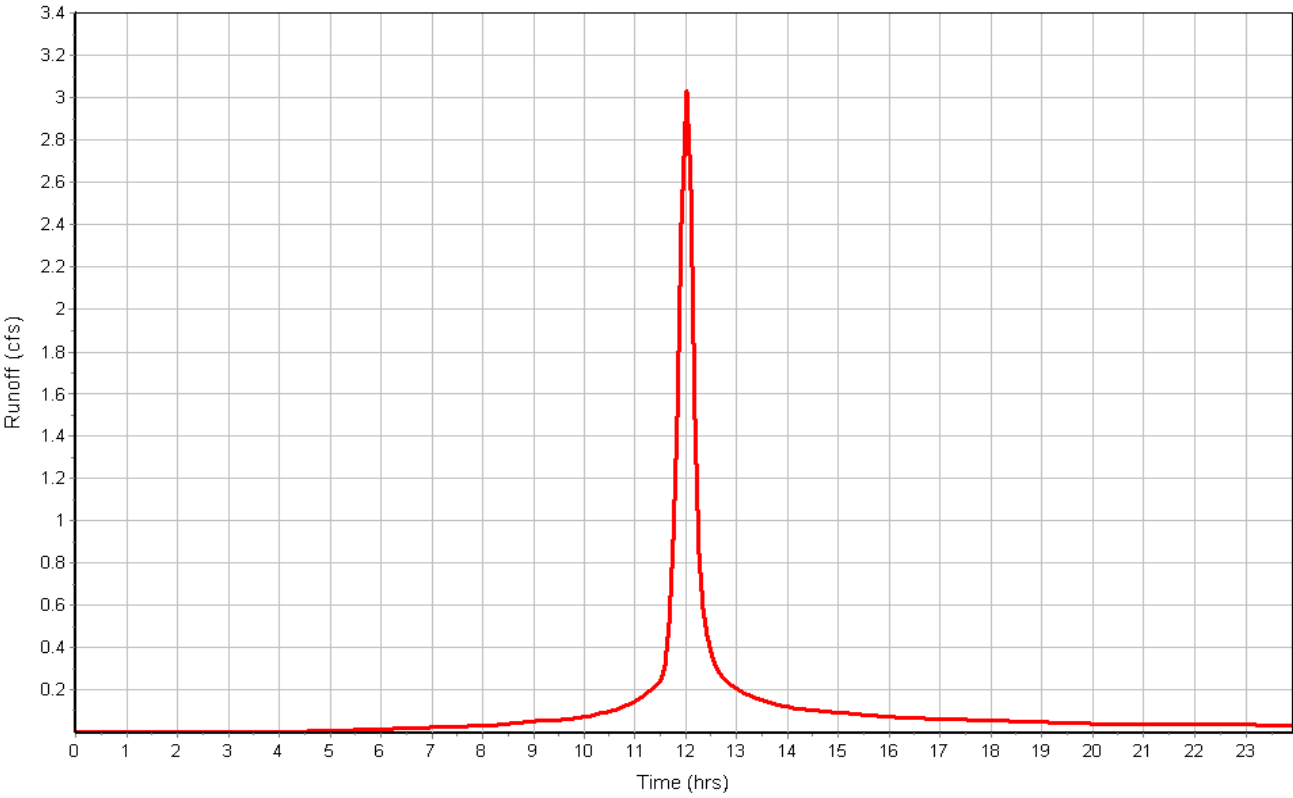
Total Rainfall (in) 5.30
Total Runoff (in) 4.17
Peak Runoff (cfs) 3.03
Weighted Curve Number 90.00
Time of Concentration (days hh:mm:ss) 0 00:12:37

Subbasin : Pro-SE

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : Pro-W

Input Data

Area (ac) 0.56
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	0.46	C	90.00
Composite Area & Weighted CN	0.46		90.00

Time of Concentration

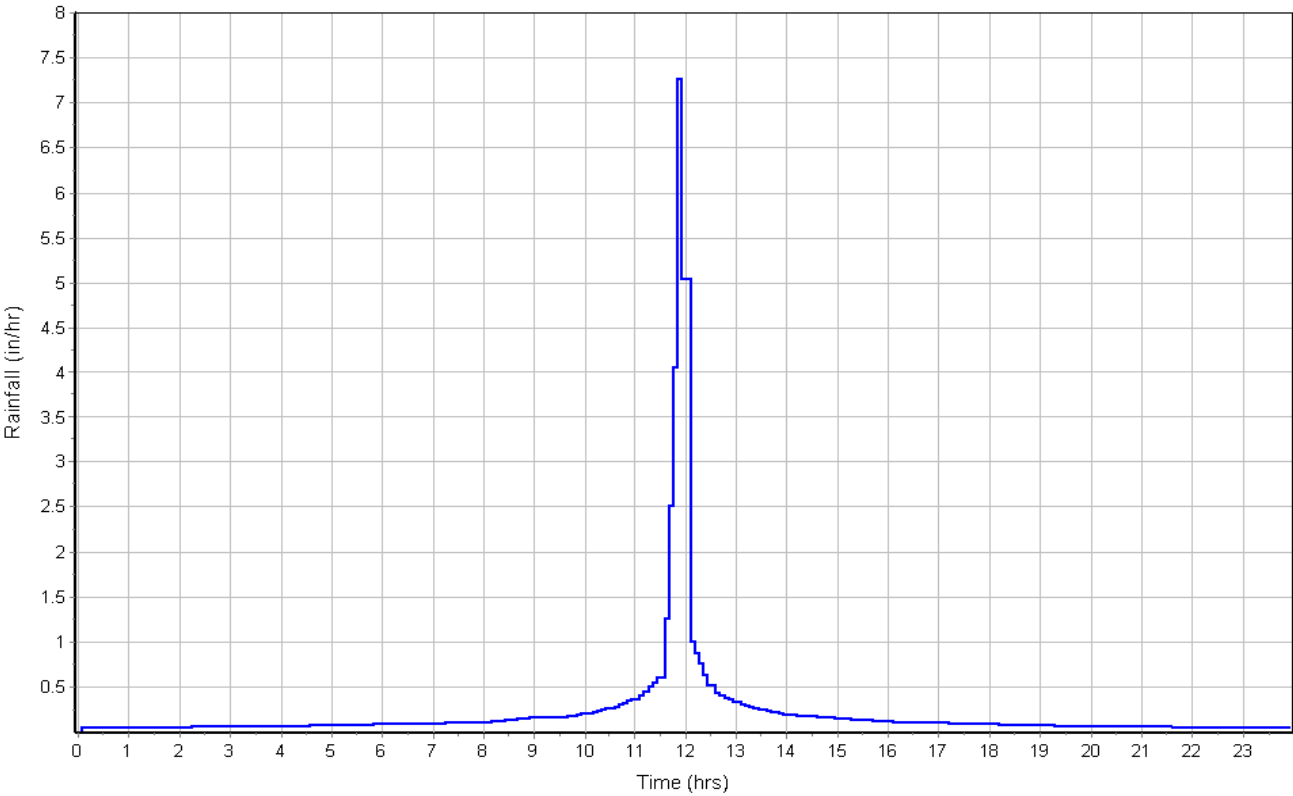
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	29	0.00	0.00
Slope (%) :	4.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.11	0.00	0.00
Computed Flow Time (min) :	4.59	0.00	0.00
Shallow Concentrated Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Flow Length (ft) :	105	0.00	0.00
Slope (%) :	7.0	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.85	0.00	0.00
Computed Flow Time (min) :	0.95	0.00	0.00
Total TOC (min)	5.54		

Subbasin Runoff Results

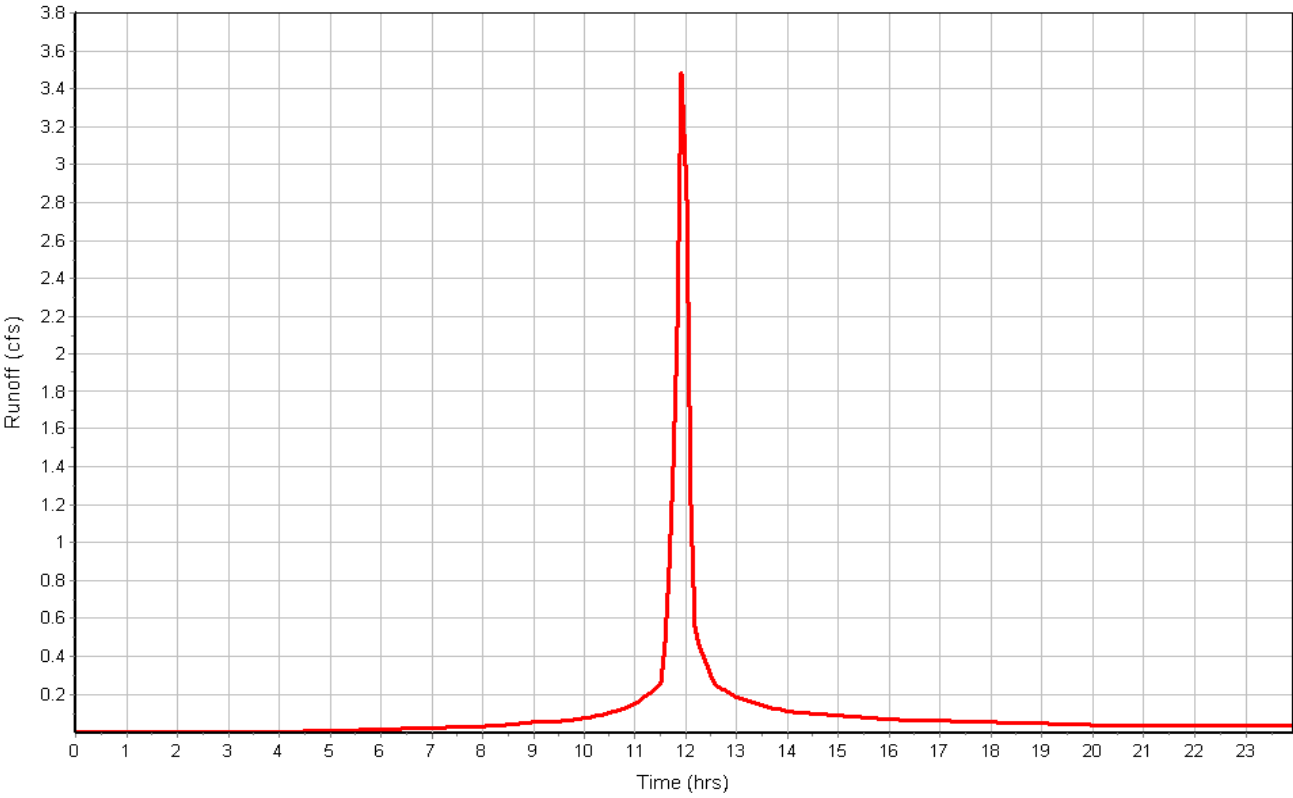
Total Rainfall (in) 5.30
Total Runoff (in) 4.17
Peak Runoff (cfs) 3.50
Weighted Curve Number 90.00
Time of Concentration (days hh:mm:ss) 0 00:05:32

Subbasin : Pro-W

Rainfall Intensity Graph



Runoff Hydrograph



Project Description

File Name 18-0251 Proposed.SPF

Project Options

Flow Units CFS
Elevation Type Elevation
Hydrology Method SCS TR-55
Time of Concentration (TOC) Method SCS TR-55
Link Routing Method Kinematic Wave
Enable Overflow Ponding at Nodes YES
Skip Steady State Analysis Time Periods ... NO

Analysis Options

Start Analysis On Oct 28, 2019 00:00:00
End Analysis On Oct 29, 2019 00:00:00
Start Reporting On Oct 28, 2019 00:00:00
Antecedent Dry Days 0 days
Runoff (Dry Weather) Time Step 0 01:00:00 days hh:mm:ss
Runoff (Wet Weather) Time Step 0 00:05:00 days hh:mm:ss
Reporting Time Step 0 00:05:00 days hh:mm:ss
Routing Time Step 30 seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins.....	4
Nodes.....	4
<i>Junctions</i>	0
<i>Outfalls</i>	4
<i>Flow Diversions</i>	0
<i>Inlets</i>	0
<i>Storage Nodes</i>	0
Links.....	0
<i>Channels</i>	0
<i>Pipes</i>	0
<i>Pumps</i>	0
<i>Orifices</i>	0
<i>Weirs</i>	0
<i>Outlets</i>	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	100-yr	Cumulative	inches	Missouri	Jackson	100	7.70	SCS Type II 24-hr

Subbasin Summary

SN	Subbasin ID	Area	Weighted Curve Number	Total Rainfall	Total Runoff	Total Runoff Volume	Peak Runoff	Time of Concentration
		(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1	Pro-NW	0.66	90.00	7.70	6.51	4.30	5.53	0 00:10:05
2	Pro-S	1.98	90.00	7.70	6.51	12.89	16.38	0 00:10:37
3	Pro-SE	0.58	90.00	7.70	6.51	3.78	4.62	0 00:12:36
4	Pro-W	0.56	90.00	7.70	6.51	3.65	5.33	0 00:05:32

Node Summary

SN	Element ID	Element Type	Invert Elevation	Ground/Rim (Max) Elevation	Initial Water Elevation	Surcharge Elevation	Ponded Area	Peak Inflow	Max HGL Elevation Attained	Max Surcharge Depth Attained	Min Freeboard Attained	Time of Peak Flooding Occurrence	Total Flooded Volume	Total Time Flooded
			(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	(ft)	(days hh:mm)	(ac-in)	(min)
1	Out-NW	Outfall	1008.00					0.00	0.00					
2	Out-S	Outfall	1013.75					0.00	0.00					
3	Out-SE	Outfall	0.00					0.00	0.00					
4	Out-W	Outfall	0.00					0.00	0.00					

Subbasin Hydrology

Subbasin : Pro-NW

Input Data

Area (ac) 0.66
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	0.70	C	90.00
Composite Area & Weighted CN	0.70		90.00

Time of Concentration

TOC Method : SCS TR-55

Sheet Flow Equation :

$$T_c = (0.007 * ((n * L_f)^{0.8})) / ((P^{0.5}) * (S_f^{0.4}))$$

Where :

T_c = Time of Concentration (hr)
n = Manning's roughness
L_f = Flow Length (ft)
P = 2 yr, 24 hr Rainfall (inches)
S_f = Slope (ft/ft)

Shallow Concentrated Flow Equation :

V = 16.1345 * (S_f^{0.5}) (unpaved surface)
V = 20.3282 * (S_f^{0.5}) (paved surface)
V = 15.0 * (S_f^{0.5}) (grassed waterway surface)
V = 10.0 * (S_f^{0.5}) (nearly bare & untilled surface)
V = 9.0 * (S_f^{0.5}) (cultivated straight rows surface)
V = 7.0 * (S_f^{0.5}) (short grass pasture surface)
V = 5.0 * (S_f^{0.5}) (woodland surface)
V = 2.5 * (S_f^{0.5}) (forest w/heavy litter surface)
T_c = (L_f / V) / (3600 sec/hr)

Where:

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)

Channel Flow Equation :

V = (1.49 * (R^{2/3}) * (S_f^{0.5})) / n
R = A_q / W_p
T_c = (L_f / V) / (3600 sec/hr)

Where :

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
R = Hydraulic Radius (ft)
A_q = Flow Area (ft²)
W_p = Wetted Perimeter (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)
n = Manning's roughness

	Subarea	Subarea	Subarea
	A	B	C
Sheet Flow Computations			
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	40	0.00	0.00
Slope (%) :	2.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.09	0.00	0.00
Computed Flow Time (min) :	7.84	0.00	0.00

	Subarea	Subarea	Subarea
	A	B	C
Shallow Concentrated Flow Computations			
Flow Length (ft) :	110	0.00	0.00
Slope (%) :	2.1	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.01	0.00	0.00
Computed Flow Time (min) :	1.82	0.00	0.00

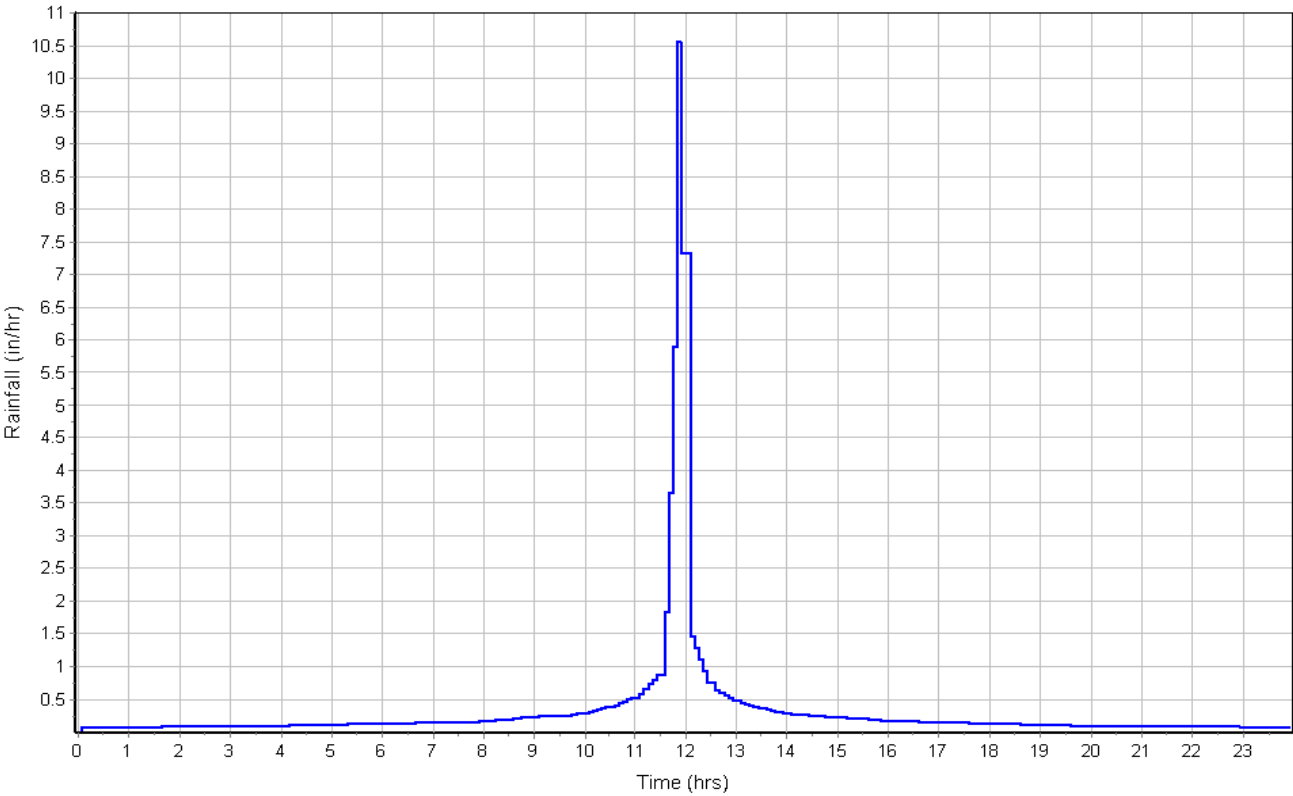
	Subarea	Subarea	Subarea
	A	B	C
Channel Flow Computations			
Manning's Roughness :	0.018	0.00	0.00
Flow Length (ft) :	310	0.00	0.00
Channel Slope (%) :	2.0	0.00	0.00
Cross Section Area (ft²) :	15	0.00	0.00
Wetted Perimeter (ft) :	15	0.00	0.00
Velocity (ft/sec) :	11.71	0.00	0.00
Computed Flow Time (min) :	0.44	0.00	0.00
Total TOC (min)	10.09		

Subbasin Runoff Results

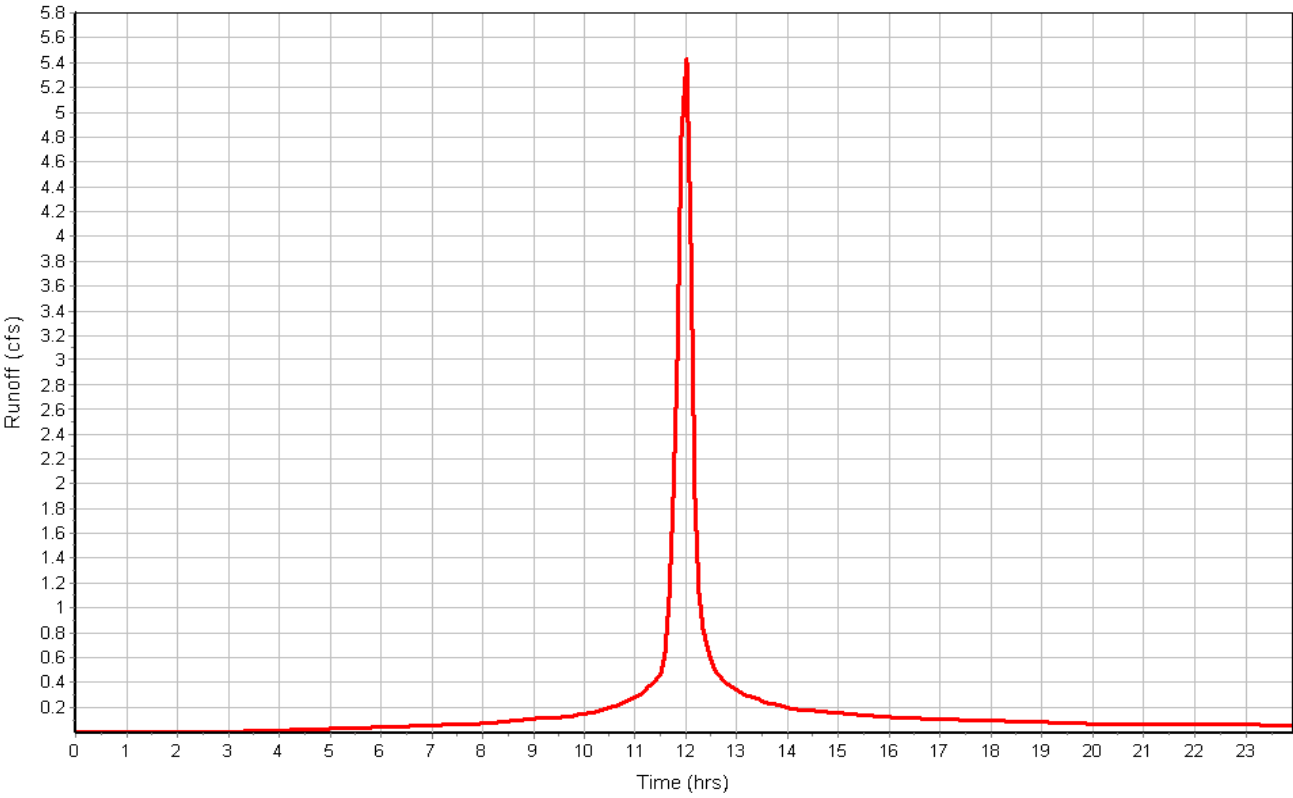
Total Rainfall (in)	7.70
Total Runoff (in)	6.51
Peak Runoff (cfs)	5.53
Weighted Curve Number	90.00
Time of Concentration (days hh:mm:ss)	0 00:10:05

Subbasin : Pro-NW

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : Pro-S

Input Data

Area (ac) 1.98
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	1.99	C	90.00
Composite Area & Weighted CN	1.99		90.00

Time of Concentration

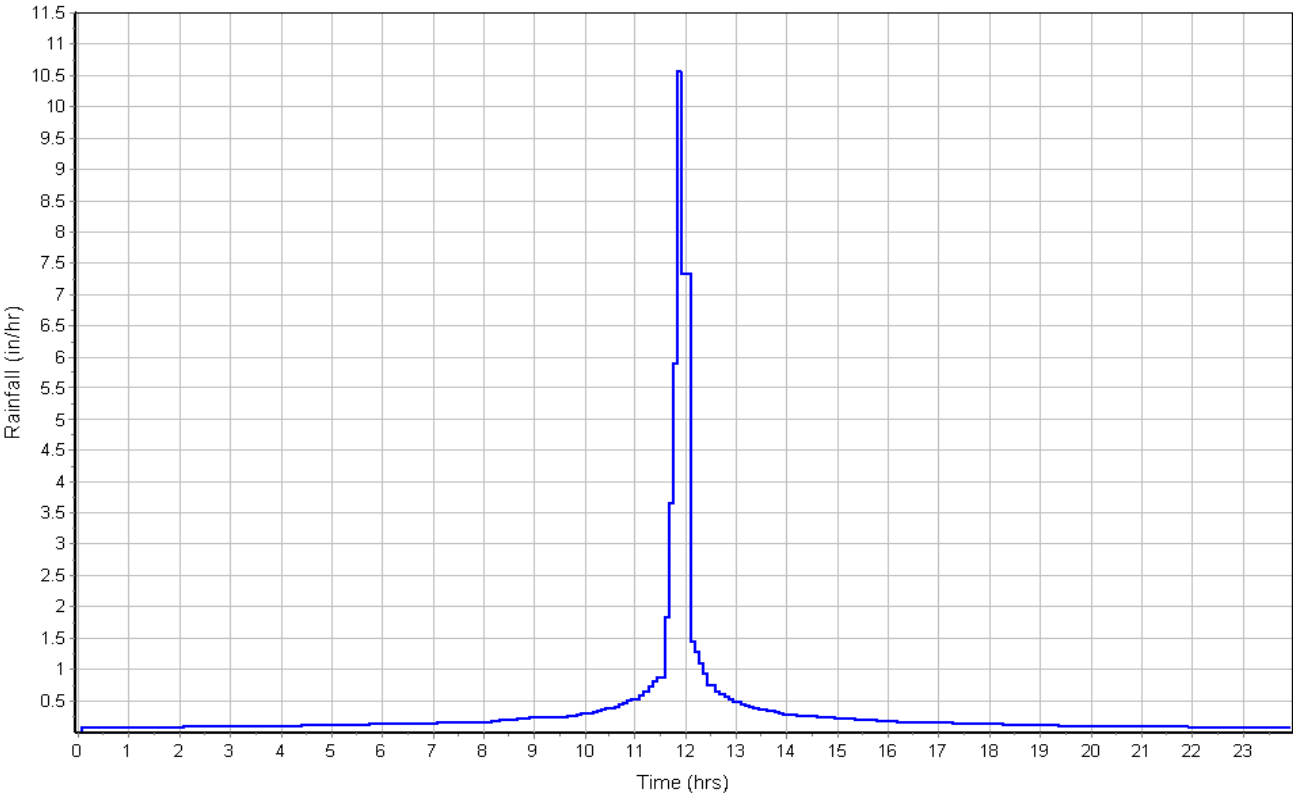
	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	50	0.00	0.00
Slope (%) :	2.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.09	0.00	0.00
Computed Flow Time (min) :	9.37	0.00	0.00
	Subarea A	Subarea B	Subarea C
Shallow Concentrated Flow Computations			
Flow Length (ft) :	175	0.00	0.00
Slope (%) :	2.0	0.00	0.00
Surface Type :	Paved	Unpaved	Unpaved
Velocity (ft/sec) :	2.87	0.00	0.00
Computed Flow Time (min) :	1.02	0.00	0.00
	Subarea A	Subarea B	Subarea C
Channel Flow Computations			
Manning's Roughness :	0.012	0.00	0.00
Flow Length (ft) :	125	0.00	0.00
Channel Slope (%) :	1.5	0.00	0.00
Cross Section Area (ft ²) :	1.33	0.00	0.00
Wetted Perimeter (ft) :	3.0	0.00	0.00
Velocity (ft/sec) :	8.84	0.00	0.00
Computed Flow Time (min) :	0.24	0.00	0.00
Total TOC (min)	10.62		

Subbasin Runoff Results

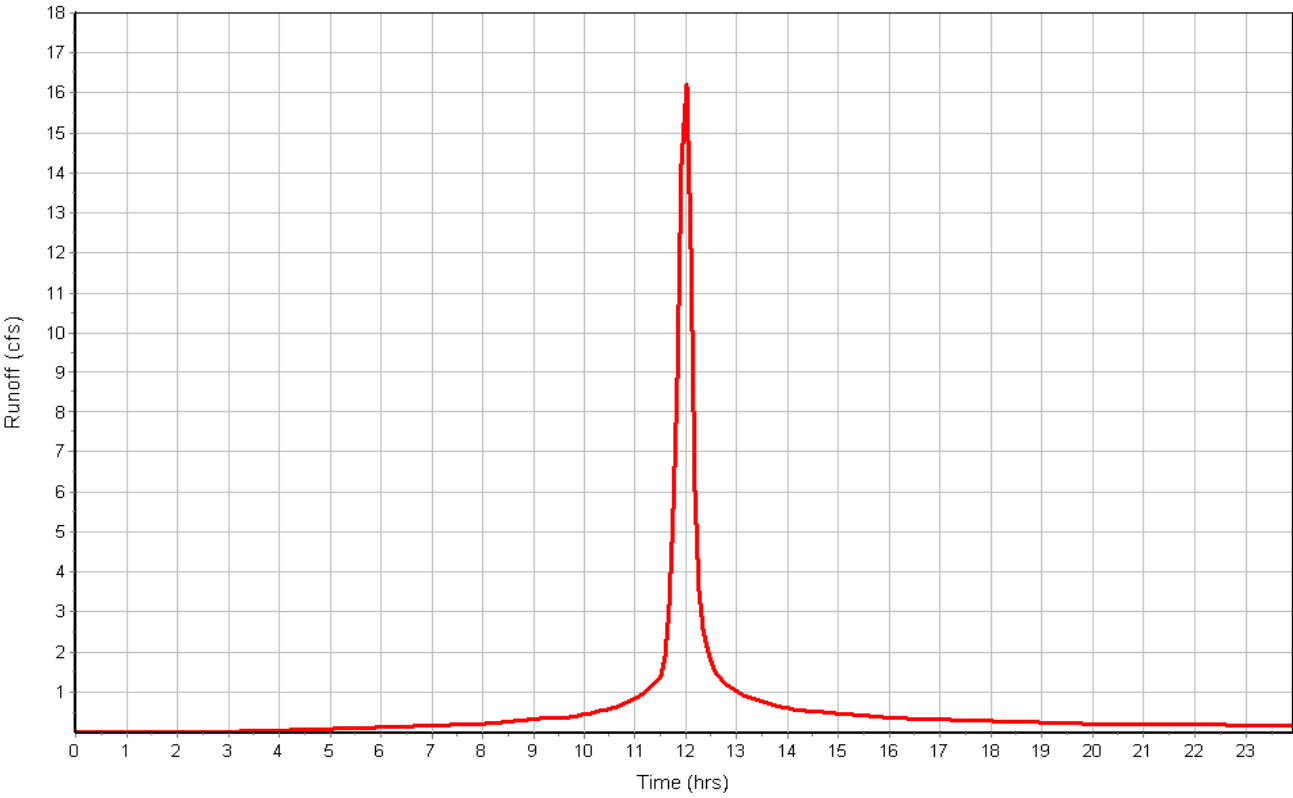
Total Rainfall (in) 7.70
Total Runoff (in) 6.51
Peak Runoff (cfs) 16.38
Weighted Curve Number 90.00
Time of Concentration (days hh:mm:ss) 0 00:10:37

Subbasin : Pro-S

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : Pro-SE

Input Data

Area (ac) 0.58
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	0.62	C	90.00
Composite Area & Weighted CN	0.62		90.00

Time of Concentration

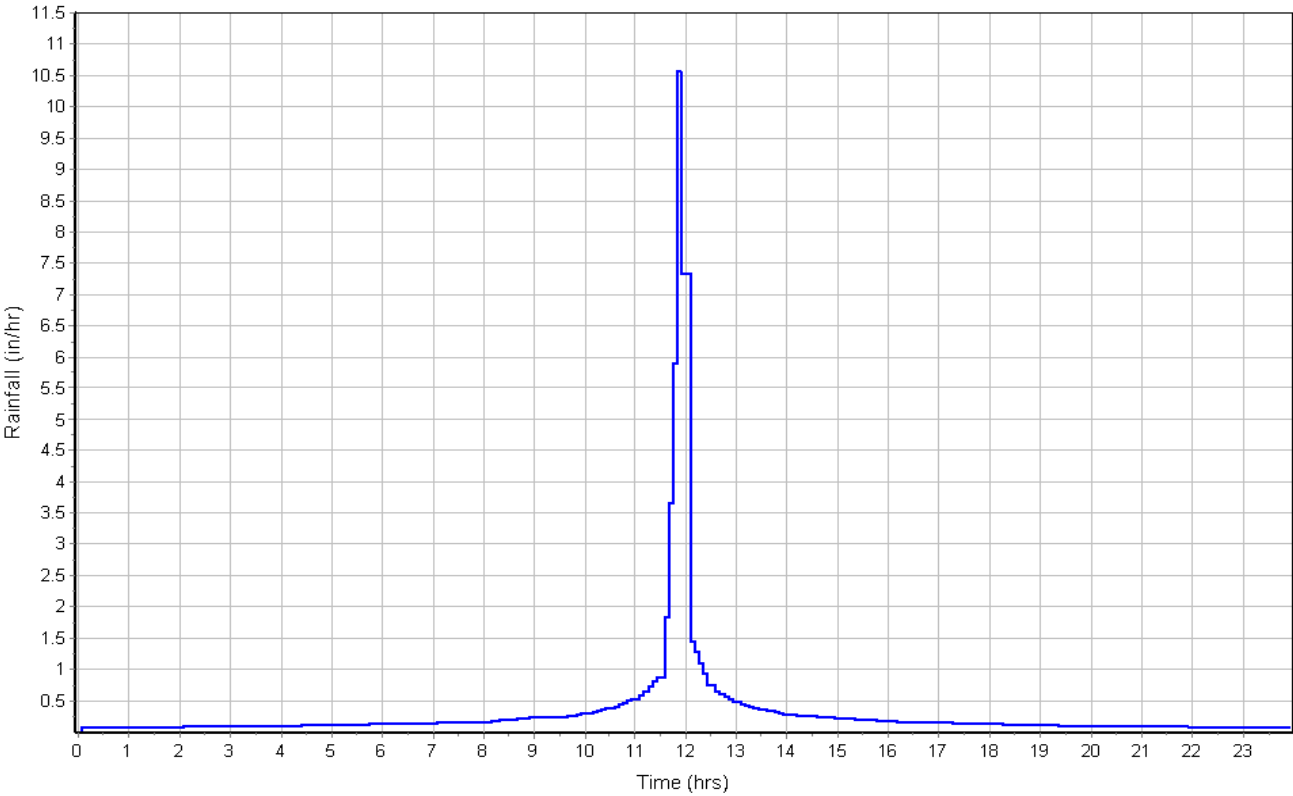
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	70	0.00	0.00
Slope (%) :	2.5	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.10	0.00	0.00
Computed Flow Time (min) :	11.22	0.00	0.00
Shallow Concentrated Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Flow Length (ft) :	110	0.00	0.00
Slope (%) :	3.5	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.31	0.00	0.00
Computed Flow Time (min) :	1.40	0.00	0.00
Total TOC (min)	12.61		

Subbasin Runoff Results

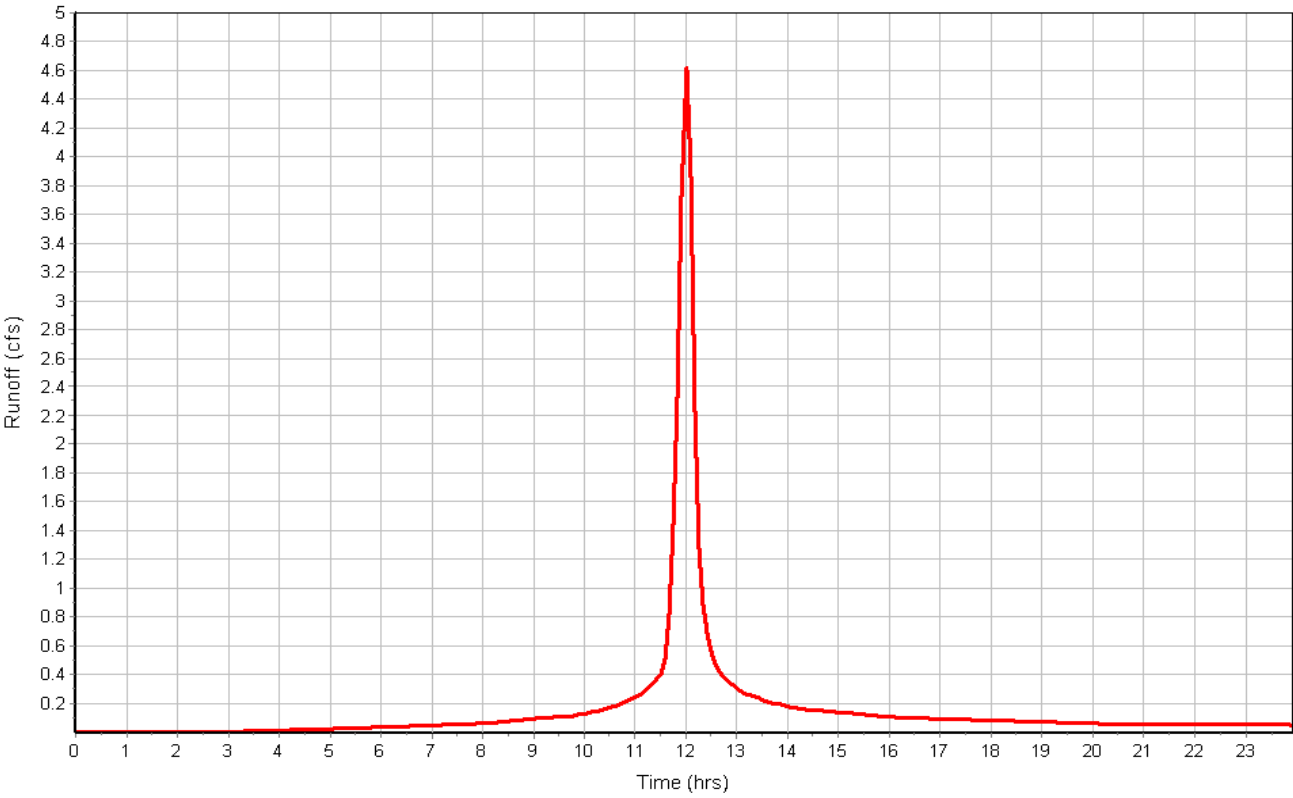
Total Rainfall (in) 7.70
Total Runoff (in) 6.51
Peak Runoff (cfs) 4.62
Weighted Curve Number 90.00
Time of Concentration (days hh:mm:ss) 0 00:12:37

Subbasin : Pro-SE

Rainfall Intensity Graph



Runoff Hydrograph



Subbasin : Pro-W

Input Data

Area (ac) 0.56
Weighted Curve Number 90.00
Rain Gage ID Rain Gage-01

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
1/8 acre lots, 65% impervious	0.46	C	90.00
Composite Area & Weighted CN	0.46		90.00

Time of Concentration

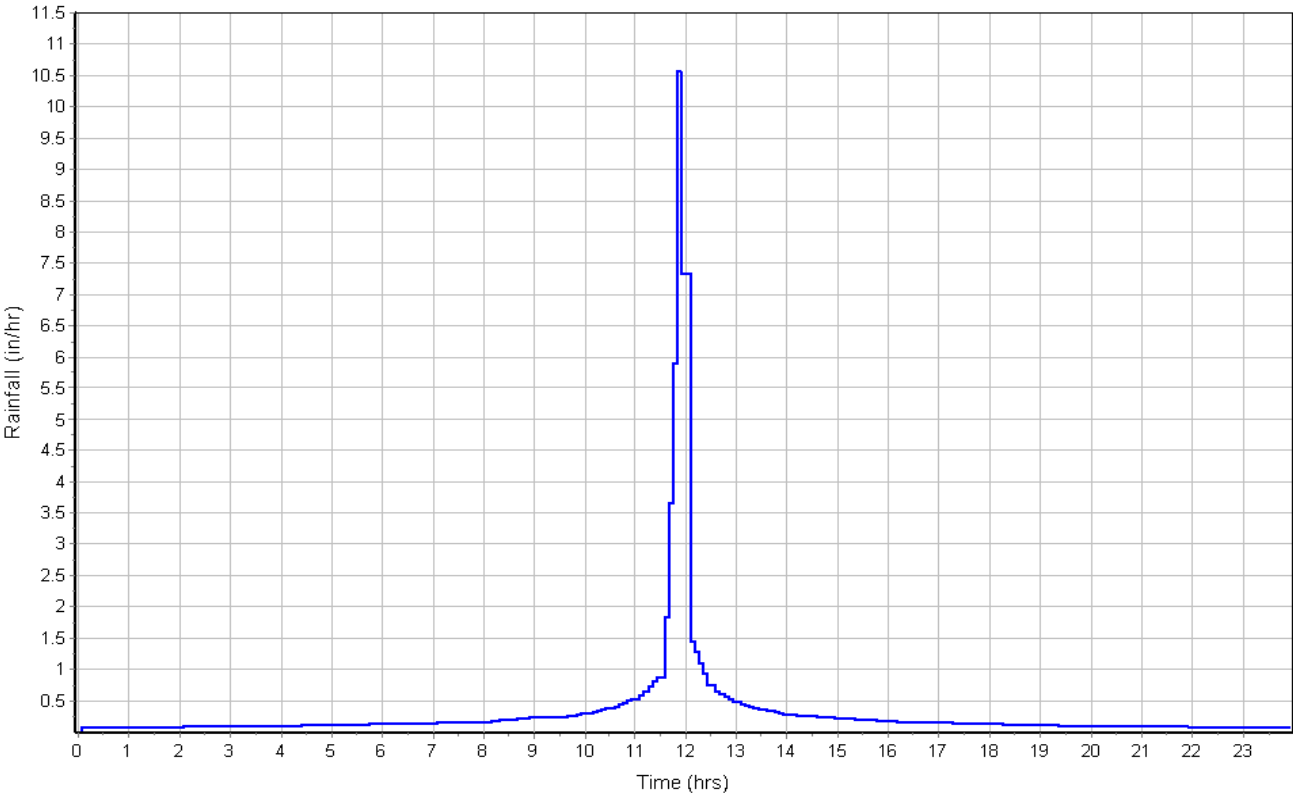
Sheet Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Manning's Roughness :	0.30	0.00	0.00
Flow Length (ft) :	29	0.00	0.00
Slope (%) :	4.0	0.00	0.00
2 yr, 24 hr Rainfall (in) :	3.50	0.00	0.00
Velocity (ft/sec) :	0.11	0.00	0.00
Computed Flow Time (min) :	4.59	0.00	0.00
Shallow Concentrated Flow Computations	Subarea	Subarea	Subarea
	A	B	C
Flow Length (ft) :	105	0.00	0.00
Slope (%) :	7.0	0.00	0.00
Surface Type :	Grass pasture	Unpaved	Unpaved
Velocity (ft/sec) :	1.85	0.00	0.00
Computed Flow Time (min) :	0.95	0.00	0.00
Total TOC (min)	5.54		

Subbasin Runoff Results

Total Rainfall (in) 7.70
Total Runoff (in) 6.51
Peak Runoff (cfs) 5.33
Weighted Curve Number 90.00
Time of Concentration (days hh:mm:ss) 0 00:05:32

Subbasin : Pro-W

Rainfall Intensity Graph



Runoff Hydrograph

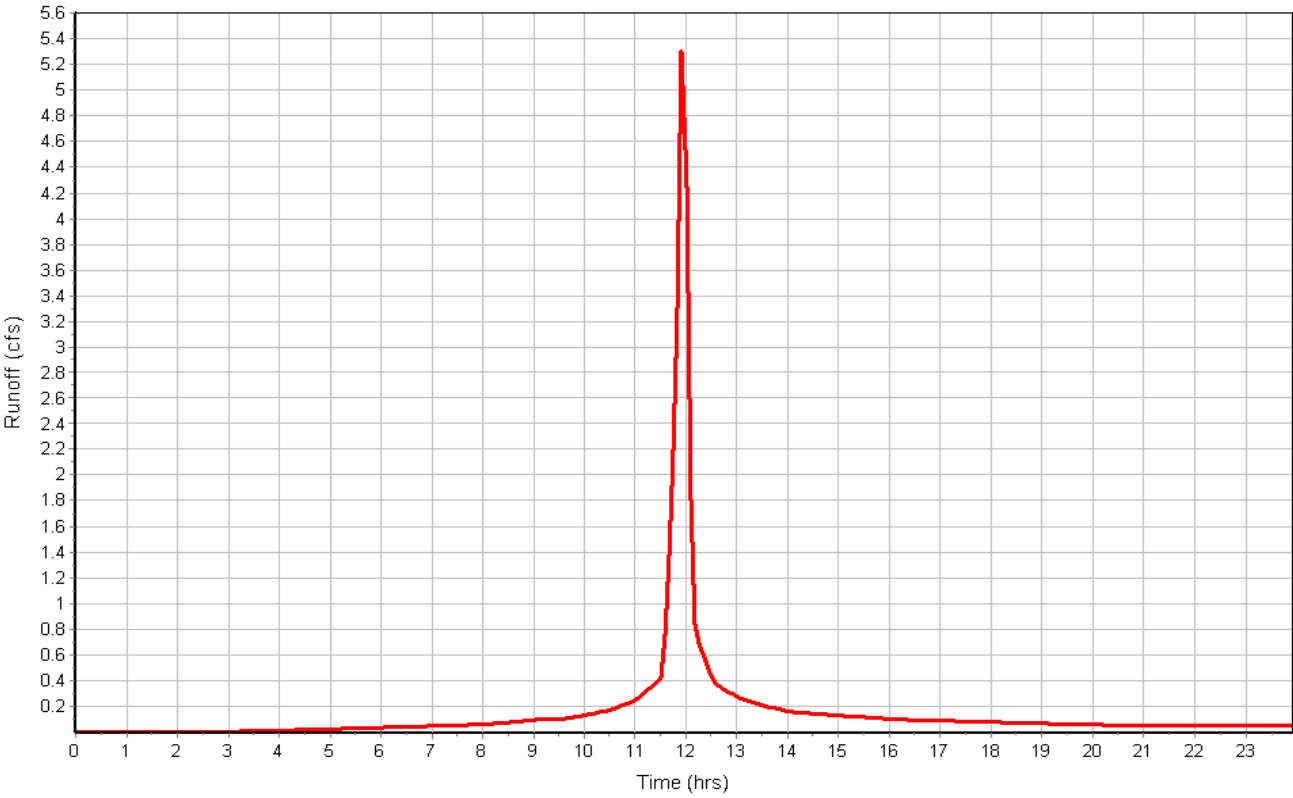


Exhibit H

Proposed Detention Stage-Storage Curves

Storage Nodes

Storage Node : NW-Pond

Input Data

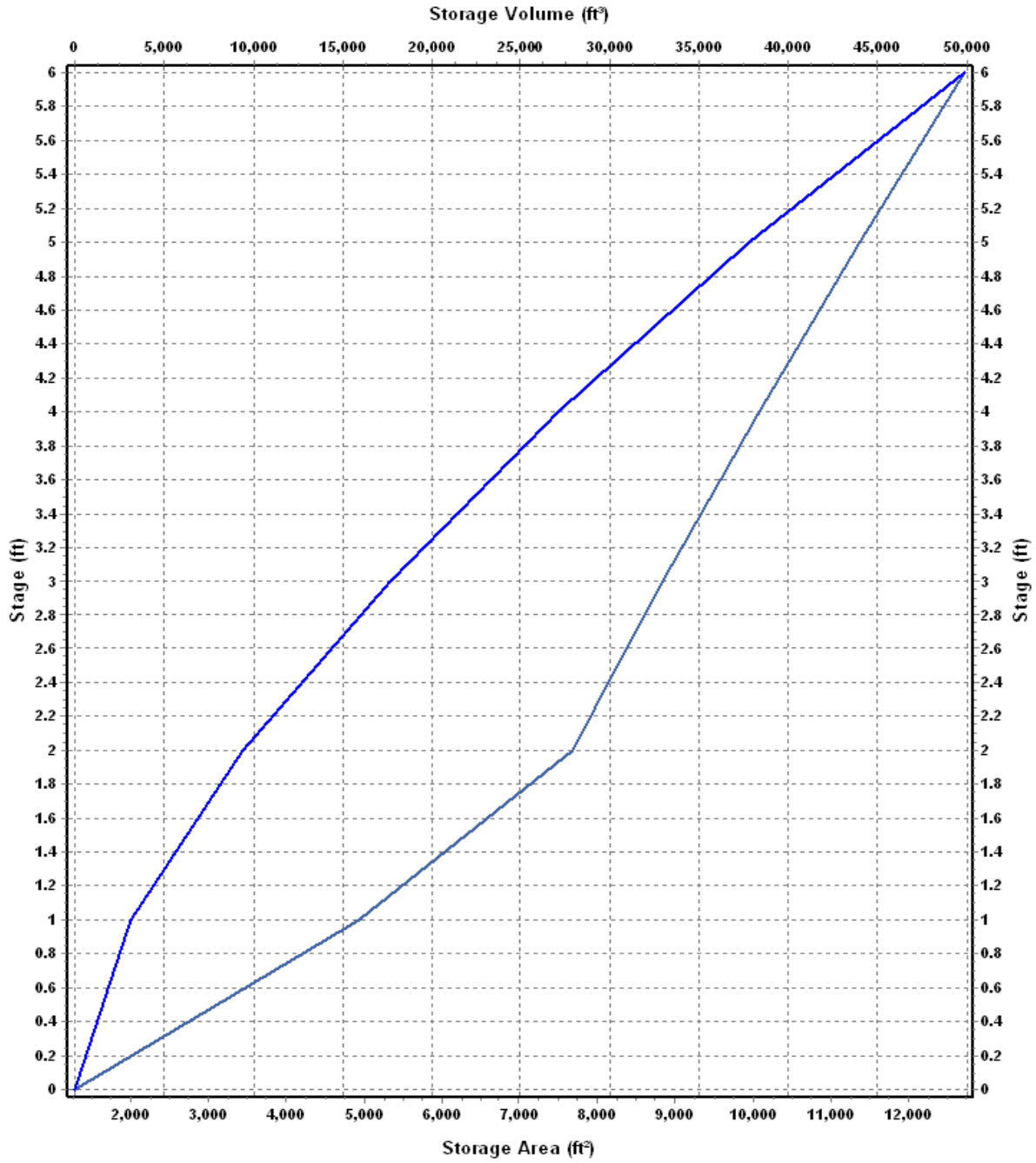
Invert Elevation (ft)	1007.00
Max (Rim) Elevation (ft)	1012.80
Max (Rim) Offset (ft)	5.80
Initial Water Elevation (ft)	1007.00
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Storage Area Volume Curves

Storage Curve : S-Pond

Stage	Storage	Storage
(ft)	Area	Volume
	(ft²)	(ft³)
0	1290	0.000
1	4925	3107.50
2	7663	9401.50
3	8839	17652.50
4	10073	27108.50
5	11364	37827.00
6	12711	49864.50

Storage Area Volume Curves



— Storage Area — Storage Volume

Storage Node : NW-Pond (continued)

Outflow Orifices

SN	Element ID	Orifice Type	Orifice Shape	Flap Gate	Circular Orifice Diameter (in)	Rectangular Orifice Height (in)	Rectangular Orifice Width (in)	Orifice Invert Elevation (ft)	Orifice Coefficient
1	Orifice-NW	Side	CIRCULAR	No	8.00			1008.00	0.61

Output Summary Results

Peak Inflow (cfs)	2.16
Peak Lateral Inflow (cfs)	2.16
Peak Outflow (cfs)	0.10
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	1008.15
Max HGL Depth Attained (ft)	1.15
Average HGL Elevation Attained (ft)	1007.58
Average HGL Depth Attained (ft)	0.58
Time of Max HGL Occurrence (days hh:mm)	0 13:37
Total Exfiltration Volume (1000-ft ³)	0.000
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Storage Node : S-Pond

Input Data

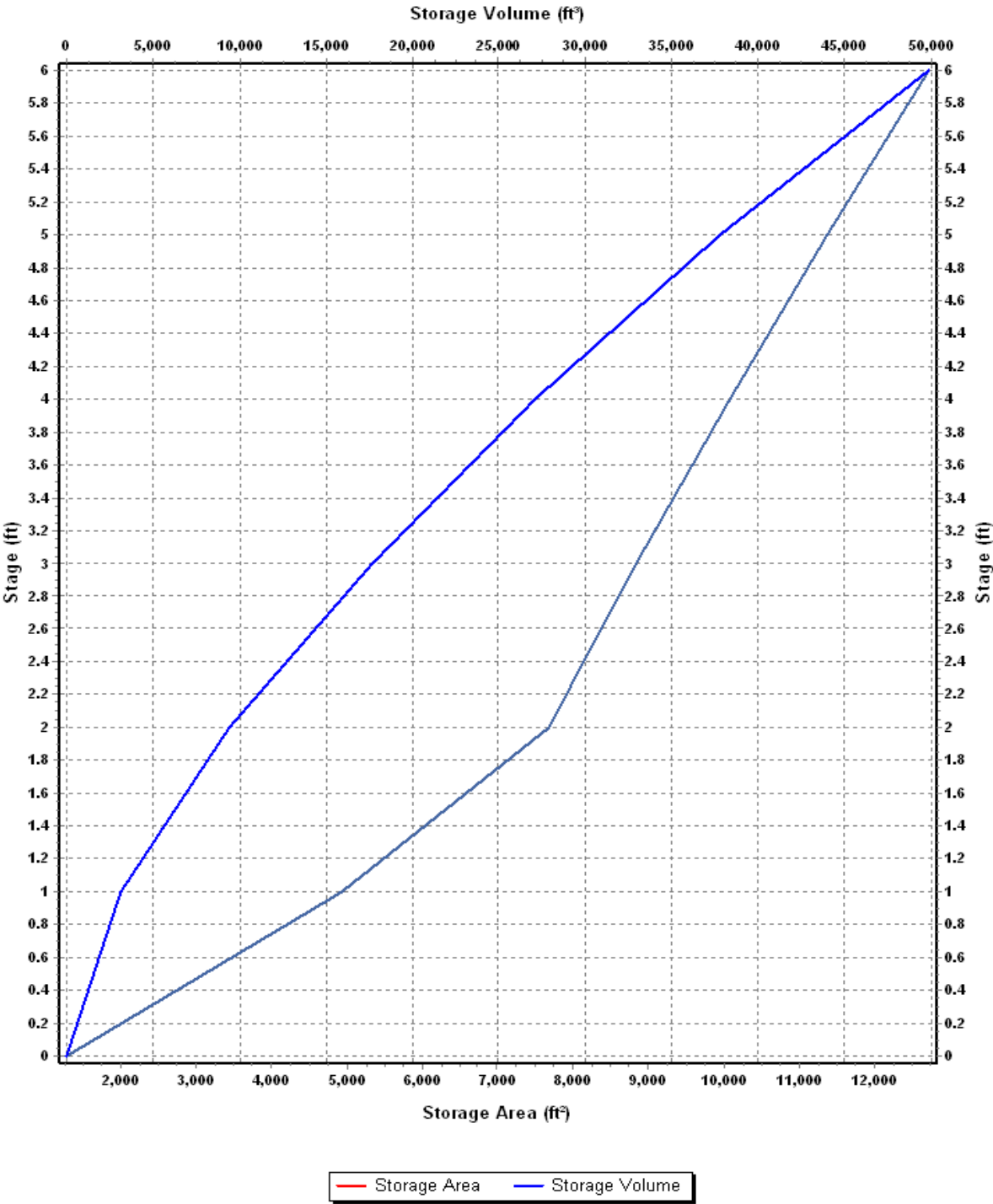
Invert Elevation (ft)	1012.00
Max (Rim) Elevation (ft)	1017.00
Max (Rim) Offset (ft)	5.00
Initial Water Elevation (ft)	1012.00
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Storage Area Volume Curves

Storage Curve : S-Pond

Stage	Storage	Storage
(ft)	Area	Volume
	(ft²)	(ft³)
0	1290	0.000
1	4925	3107.50
2	7663	9401.50
3	8839	17652.50
4	10073	27108.50
5	11364	37827.00
6	12711	49864.50

Storage Area Volume Curves



Storage Node : S-Pond (continued)**Outflow Orifices**

SN Element ID	Orifice Type	Orifice Shape	Flap Gate	Circular Orifice Diameter (in)	Rectangular Orifice Height (in)	Rectangular Orifice Width (in)	Orifice Invert Elevation (ft)	Orifice Coefficient
1 Orifice-S	Side	CIRCULAR	No	12.00			1013.55	0.61

Output Summary Results

Peak Inflow (cfs)	6.45
Peak Lateral Inflow (cfs)	6.45
Peak Outflow (cfs)	0.96
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	1014.05
Max HGL Depth Attained (ft)	2.05
Average HGL Elevation Attained (ft)	1012.94
Average HGL Depth Attained (ft)	0.94
Time of Max HGL Occurrence (days hh:mm)	0 12:28
Total Exfiltration Volume (1000-ft ³)	0.000
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Storage Nodes

Storage Node : NW-Pond

Input Data

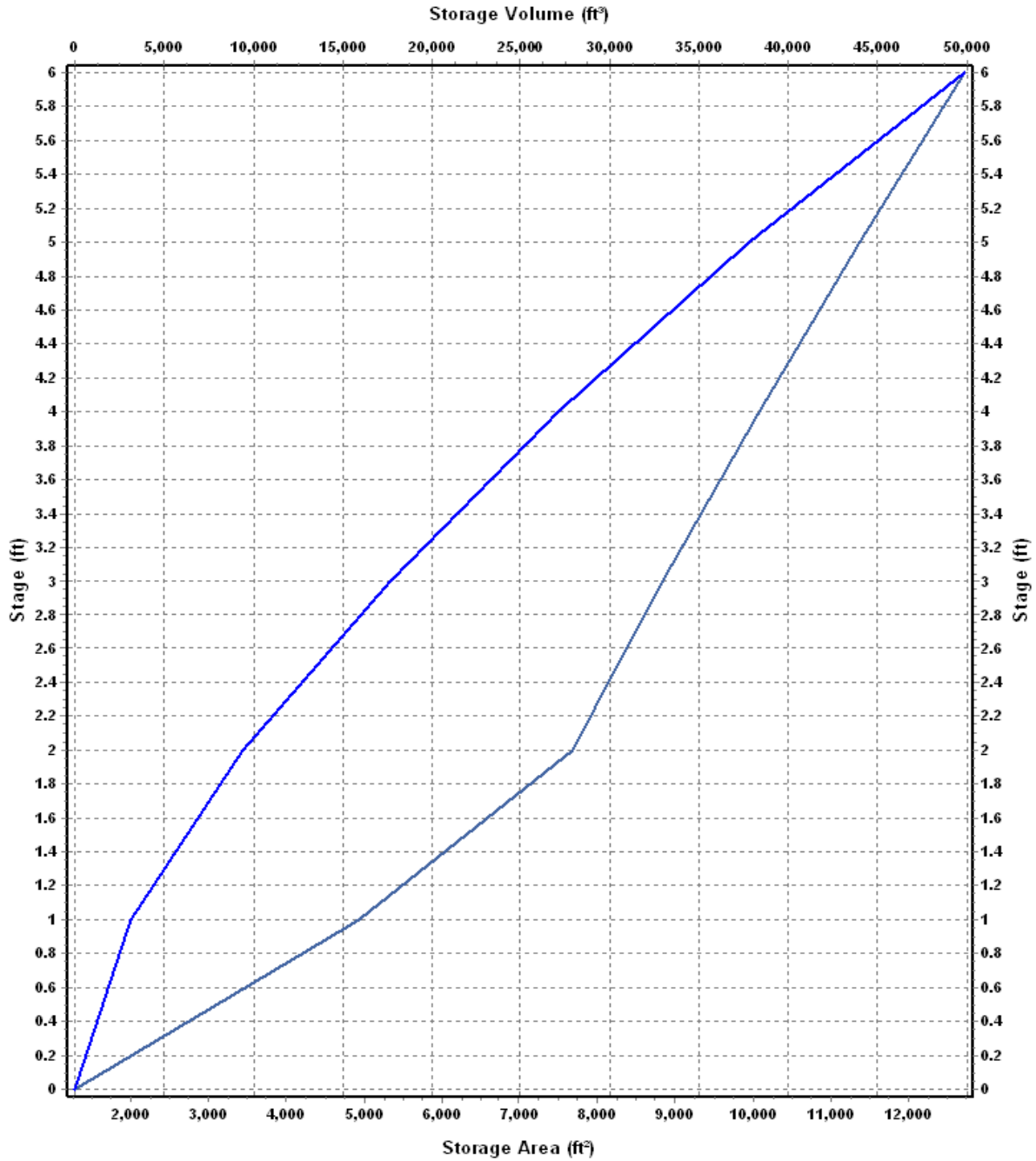
Invert Elevation (ft)	1007.00
Max (Rim) Elevation (ft)	1012.80
Max (Rim) Offset (ft)	5.80
Initial Water Elevation (ft)	1007.00
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Storage Area Volume Curves

Storage Curve : S-Pond

Stage	Storage Area	Storage Volume
(ft)	(ft²)	(ft³)
0	1290	0.000
1	4925	3107.50
2	7663	9401.50
3	8839	17652.50
4	10073	27108.50
5	11364	37827.00
6	12711	49864.50

Storage Area Volume Curves



— Storage Area — Storage Volume

Storage Node : NW-Pond (continued)

Outflow Orifices

SN	Element ID	Orifice Type	Orifice Shape	Flap Gate	Circular Orifice Diameter (in)	Rectangular Orifice Height (in)	Rectangular Orifice Width (in)	Orifice Invert Elevation (ft)	Orifice Coefficient
1	Orifice-NW	Side	CIRCULAR	No	8.00			1008.00	0.61

Output Summary Results

Peak Inflow (cfs)	3.57
Peak Lateral Inflow (cfs)	3.57
Peak Outflow (cfs)	0.58
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	1008.47
Max HGL Depth Attained (ft)	1.47
Average HGL Elevation Attained (ft)	1007.66
Average HGL Depth Attained (ft)	0.66
Time of Max HGL Occurrence (days hh:mm)	0 12:25
Total Exfiltration Volume (1000-ft ³)	0.000
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Storage Node : S-Pond

Input Data

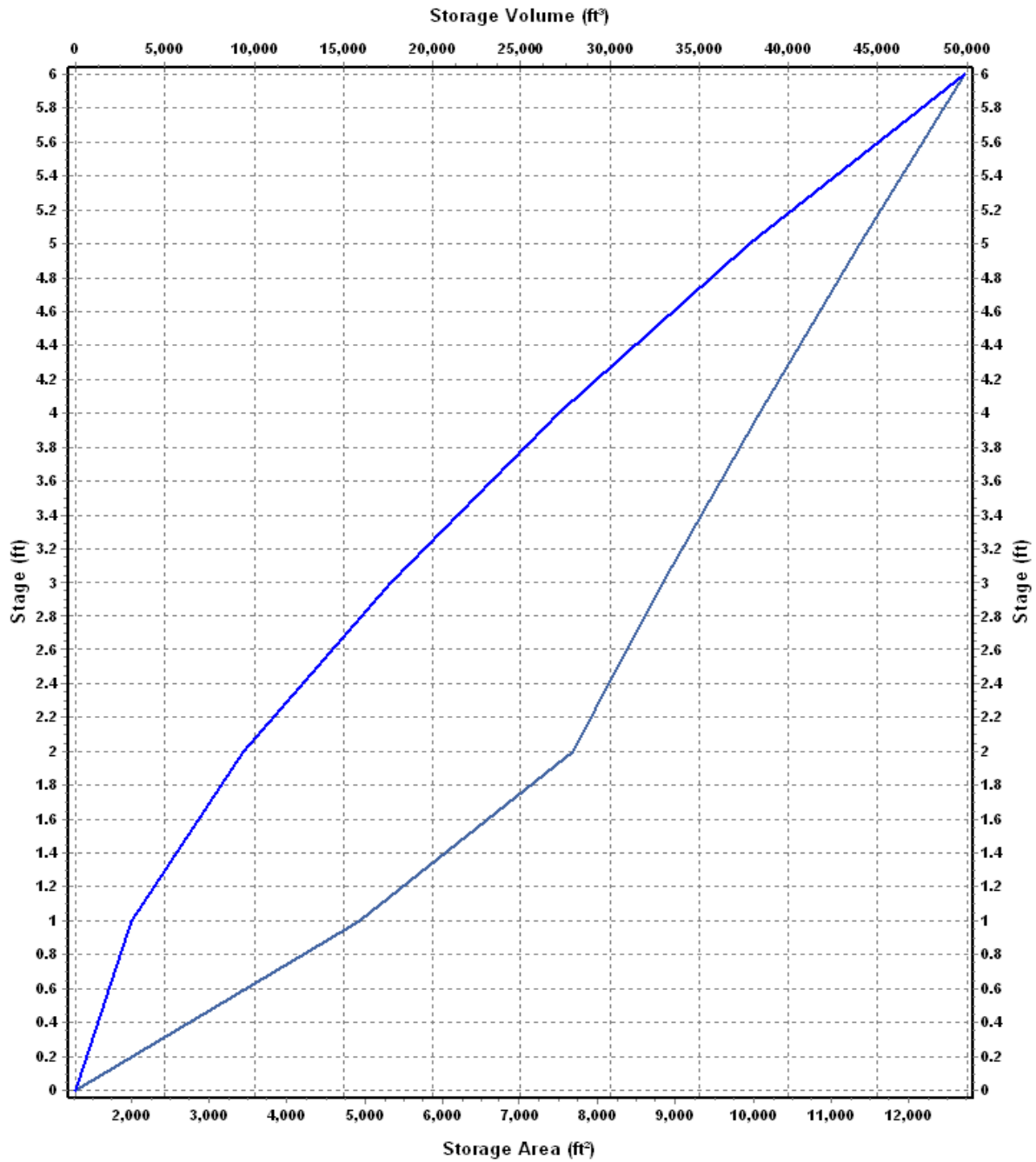
Invert Elevation (ft)	1012.00
Max (Rim) Elevation (ft)	1017.00
Max (Rim) Offset (ft)	5.00
Initial Water Elevation (ft)	1012.00
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Storage Area Volume Curves

Storage Curve : S-Pond

Stage	Storage	Storage
(ft)	Area	Volume
	(ft²)	(ft³)
0	1290	0.000
1	4925	3107.50
2	7663	9401.50
3	8839	17652.50
4	10073	27108.50
5	11364	37827.00
6	12711	49864.50

Storage Area Volume Curves



— Storage Area — Storage Volume

Storage Node : S-Pond (continued)

Outflow Orifices

SN	Element ID	Orifice Type	Orifice Shape	Flap Gate	Circular Orifice Diameter (in)	Rectangular Orifice Height (in)	Rectangular Orifice Width (in)	Orifice Invert Elevation (ft)	Orifice Coefficient
1	Orifice-S	Side	CIRCULAR	No	12.00			1013.55	0.61

Output Summary Results

Peak Inflow (cfs)	10.67
Peak Lateral Inflow (cfs)	10.67
Peak Outflow (cfs)	3.14
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	1014.71
Max HGL Depth Attained (ft)	2.71
Average HGL Elevation Attained (ft)	1013.08
Average HGL Depth Attained (ft)	1.08
Time of Max HGL Occurrence (days hh:mm)	0 12:18
Total Exfiltration Volume (1000-ft³)	0.000
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Storage Nodes

Storage Node : NW-Pond

Input Data

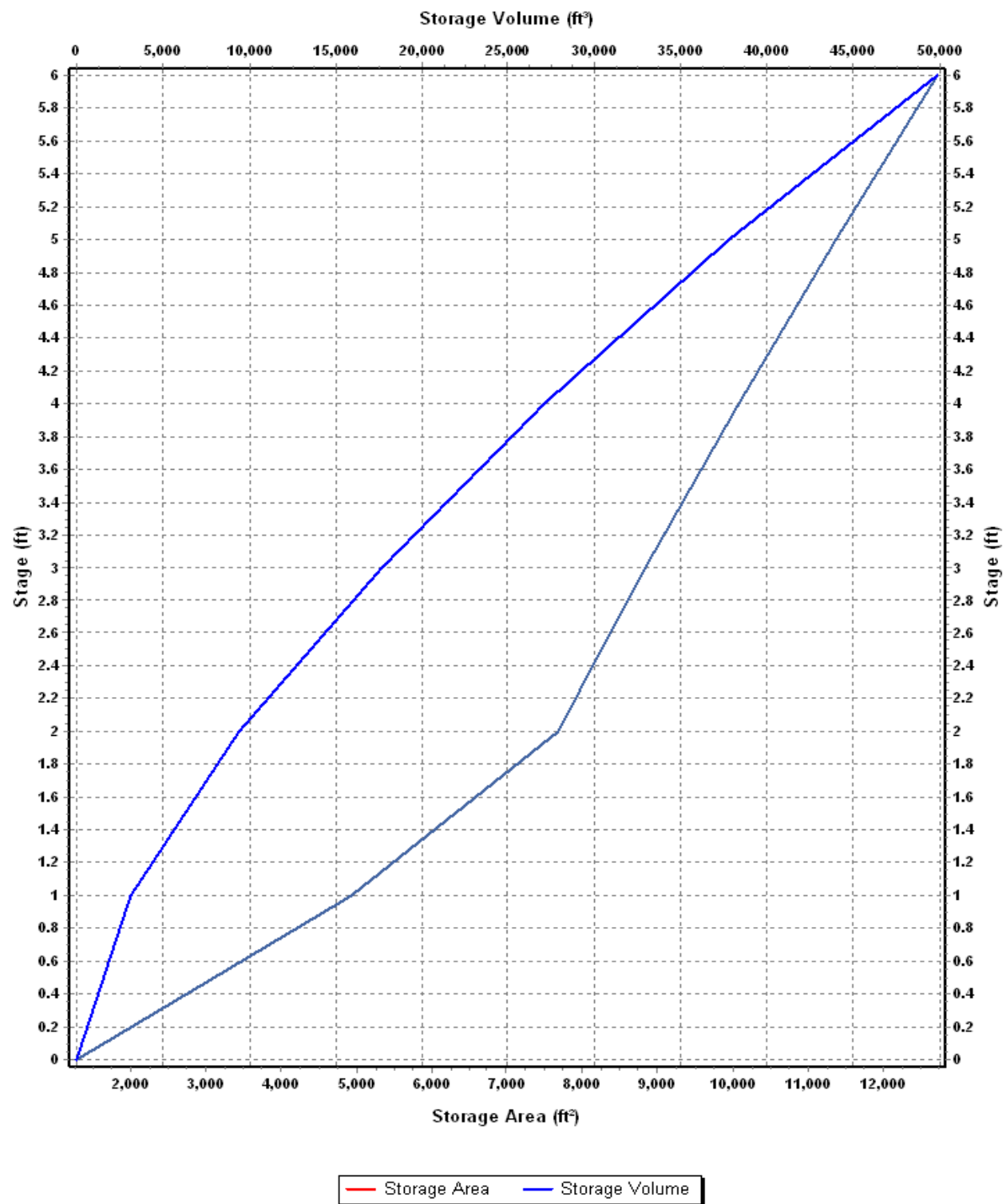
Invert Elevation (ft)	1007.00
Max (Rim) Elevation (ft)	1012.80
Max (Rim) Offset (ft)	5.80
Initial Water Elevation (ft)	1007.00
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Storage Area Volume Curves

Storage Curve : S-Pond

Stage	Storage Area	Storage Volume
(ft)	(ft²)	(ft³)
0	1290	0.000
1	4925	3107.50
2	7663	9401.50
3	8839	17652.50
4	10073	27108.50
5	11364	37827.00
6	12711	49864.50

Storage Area Volume Curves



Storage Node : NW-Pond (continued)

Outflow Orifices

SN	Element ID	Orifice Type	Orifice Shape	Flap Gate	Circular Orifice Diameter (in)	Rectangular Orifice Height (in)	Rectangular Orifice Width (in)	Orifice Invert Elevation (ft)	Orifice Coefficient
1	Orifice-NW	Side	CIRCULAR	No	8.00			1008.00	0.61

Output Summary Results

Peak Inflow (cfs)	5.43
Peak Lateral Inflow (cfs)	5.43
Peak Outflow (cfs)	1.26
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	1008.87
Max HGL Depth Attained (ft)	1.87
Average HGL Elevation Attained (ft)	1007.76
Average HGL Depth Attained (ft)	0.76
Time of Max HGL Occurrence (days hh:mm)	0 12:19
Total Exfiltration Volume (1000-ft³)	0.000
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Storage Node : S-Pond

Input Data

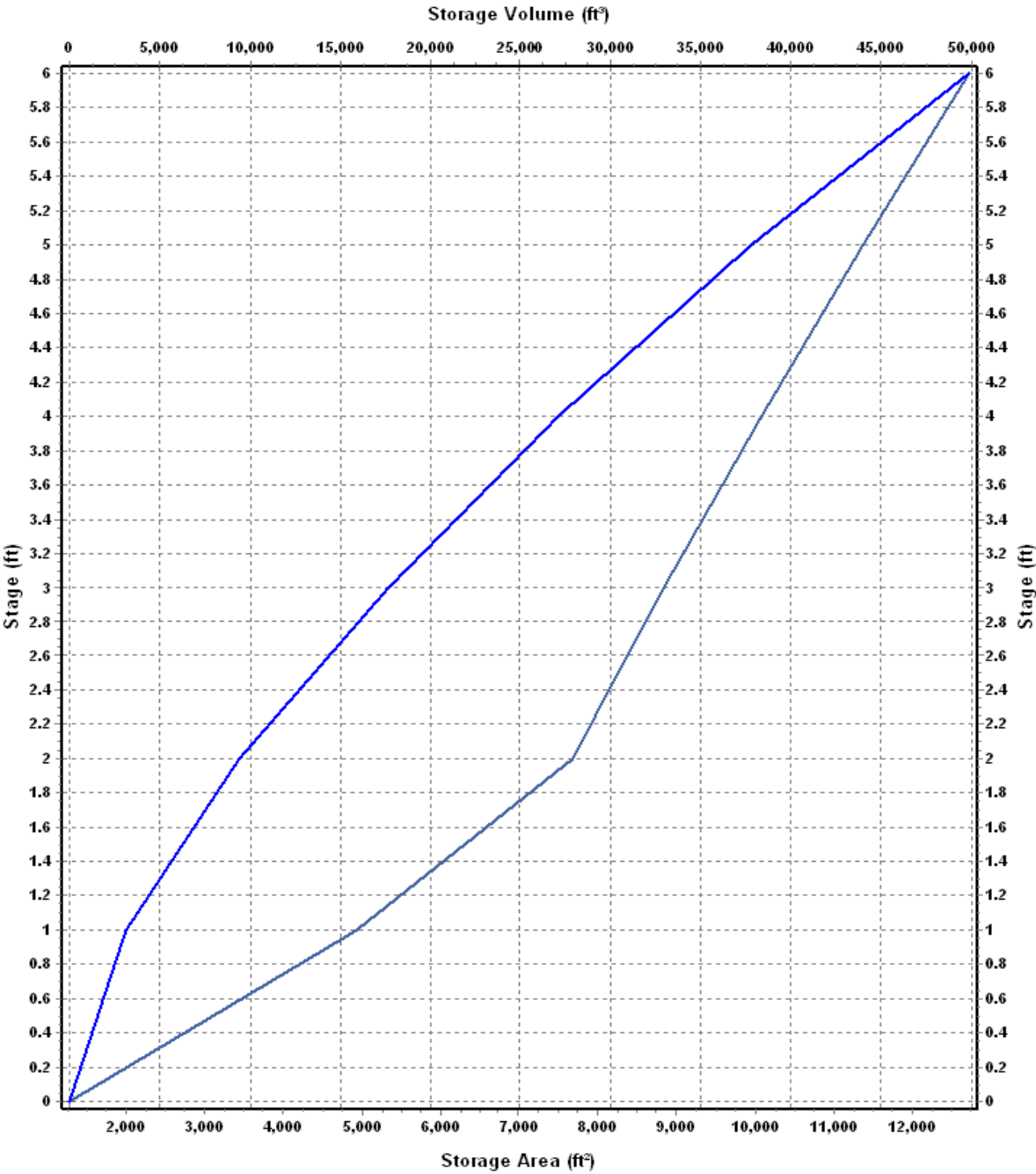
Invert Elevation (ft)	1012.00
Max (Rim) Elevation (ft)	1017.00
Max (Rim) Offset (ft)	5.00
Initial Water Elevation (ft)	1012.00
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Storage Area Volume Curves

Storage Curve : S-Pond

Stage	Storage Area	Storage Volume
(ft)	(ft²)	(ft³)
0	1290	0.000
1	4925	3107.50
2	7663	9401.50
3	8839	17652.50
4	10073	27108.50
5	11364	37827.00
6	12711	49864.50

Storage Area Volume Curves



Storage Area Storage Volume

Storage Node : S-Pond (continued)

Outflow Orifices

SN	Element ID	Orifice Type	Orifice Shape	Flap Gate	Circular Orifice Diameter (in)	Rectangular Orifice Height (in)	Rectangular Orifice Width (in)	Orifice Invert Elevation (ft)	Orifice Coefficient
1	Orifice-S	Side	CIRCULAR	No	12.00			1013.55	0.61

Output Summary Results

Peak Inflow (cfs)	16.21
Peak Lateral Inflow (cfs)	16.21
Peak Outflow (cfs)	4.70
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	1015.53
Max HGL Depth Attained (ft)	3.53
Average HGL Elevation Attained (ft)	1013.26
Average HGL Depth Attained (ft)	1.26
Time of Max HGL Occurrence (days hh:mm)	0 12:18
Total Exfiltration Volume (1000-ft³)	0.000
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit I

MARC BMP Design Worksheets and Calculations

WORKSHEET 1A: REQUIRED LEVEL OF SERVICE - DEVELOPED SITE

Project: 18-0251 Sequoia

By: JGD

Date: 1/15/2020

Location: Lee's Summit, MO

Checked: MES

Date: 1/15/2020

1. Required Treatment Area**A. Total Area Disturbed by Redevelopment Activity (ac.)**

Disturbed Area Description	Acres
Existing Single Family Residential Lots	3.78
"1A" Total:	3.78

B. Existing Impervious Area Inside Disturbed Area (ac.)

Existing Impervious Area Description	Acres
Driveways	0.16
House and Garage	0.09
"1B" Total:	0.25

C. Required Treatment Area (ac.)

"1A" Total Less "1B" Total **"1C"** 3.53

2. Percent Impervious in Postdevelopment Conditions and Level of Service (LS)**A. Total Postdevelopment Impervious Area Inside Disturbed Area (ac.)**

Postdevelopment Impervious Area Description	Acres
Parking/Roof/Impervious Area	1.15
"2A" Total:	1.15

B. Existing Impervious Area Inside Disturbed Area (ac.)

"1B" Total 0.25

C. Net Increase in Impervious Area (ac.)

"2A" Total Less "1B" Total **"2C"** 0.90

D. Percent Impervious

Net Increase in Impervious Area/Required Treatment Area

"2C"/"1C" x 100 **"2C"** 25 (Round to Integer)

E. Level of Service

Use Percent Impervious to Enter Table 4.

LS = 5

3. Minimum Required Total Value Rating of BMP Package

Total Value Rating = LS x Required Treatment Area

LS x "1C" **VR =** 17.65

WORKSHEET 2: DEVELOP MITIGATION PACKAGE(S) THAT MEET THE REQUIRED LS

Project: 18-0251 Sequoia
Location: Lee's Summit, MO

By: JGD
Checked: MES

Date: 1/15/2020
Date: 1/15/2020

Sheet ___ of ___

1. Required LS (New Development, Wksht 1) or Total VR (Redevelopment, Wksht 1A)

17.65

Note: Various BMPs may alter CN of proposed development, and LS; recalculate both if applicable.

2. Proposed BMP Option Package No. 1

Cover/BMP Description	Treatment Area	VR from Table 4.4 or 4.6 ¹	Product of VR x Area
Native Veg. Swale + Extended Dry Detention Basin (EDDB)	0.94	7.00	6.58
Extended Dry Detention	1.79	6.00	10.74
Native Vegetation	0.05	9.25	0.4625
			0
			0
Total²:	2.78	Total:	17.78

***Weighted VR:** **17.65** = total product/total area

¹ VR calculated for final BMP only in Treatment Train

² Total treatment area cannot exceed 100 percent of the actual site area

* Blank in Redevelopment

Meets required LS (Yes/No)?

Yes

(If No, or if additional options are being tested, proceed below)

3. Proposed BMP Option Package No. 2

Cover/BMP Description	Treatment Area	VR from Table 4.4 or 4.6 ¹	Product of VR x Area
Total²:		Total:	

***Weighted VR:** **17.65** = total product/total area

¹ VR calculated for final BMP only in Treatment Train

² Total treatment area cannot exceed 100 percent of the actual site area

* Blank in Redevelopment

Meets required LS (Yes/No)?

17.65

(If No, or if additional options are being tested, move to next sheet)

Design Procedure Form: Extended Dry Detention Basin (EDDB) Main Worksheet

Designer: JGD
Checked By: MES
Company: RIC
Date: 5/1/2020
Project: 18-0251 - Sequoia
Location: Proposed NW Basin, Lee's Summit, MO
Reference: APWA/MARC BMP Manual, 8.10 EDDB, pg 8-107 thru 8-128

I. Water Quality Volume

Step 1) Tributary area to EDDC, A_T (ac)	Rational C =	0.51
Step 2) Calculate WQv using methodology in Section 6	A_T (ac) =	0.66
Step 3) Add 20 percent to account for silt and sediment deposition in the basin	WQv (ac-ft) =	0.04
	V_{design} (ac-ft) =	0.05

IIa. Pretreatment

Step 1) Set water quality outlet type	Outlet type =	1
Type 1 = single orifice		
Type 2 = perforated riser or plate		
Type 3 = v-notch weir		
Step 3) Proceed to Part IIb, IIc or IId based on water quality outlet type selected		

IIb. Water Quality Outlet, Single Orifice

Step 1) Depth of water quality volume and outlet, Z_{WQ} (ft)	Z_{WQ} (ft) =	1.50
Step 2) Average head of water quality volume over invert of orifice, H_{WQ} (ft)	H_{WQ} (ft) =	0.75
$H_{WQ} = 0.5 * Z_{WQ}$		
Step 3) Average water quality outfall rate, Q_{WQ} (cfs)	C_{WQ} (cfs) =	0.01
$Q_{WQ} = (WQ_v * 43,560) / (40 * 3,600)$		
Step 4) Set Value of orifice discharge coefficient, C_0	C_0 =	0.66
$C_0 = 0.66$ when thickness of riser/weir plate is $\leq$ orifice diameter		
$C_0 = 0.80$ when thickness of riser/weir plate is $\geq$ orifice diameter		
Step 5) Water quality outlet orifice diameter (minimum of 4 inches), D_0 (in)	D_0 (in) =	0.69
$D_0 = 12 * 2 * (Q_{WQ} / (C_0 * \pi * (2 * g * H)^{0.5}))^{0.5}$		
Step 6) To size outlet orifice for EDDB with an irregular stage-volume relationship, use the Single Orifice Worksheet		

IIc. Water Quality Outlet, Perforated Riser

Step 1) Depth at outlet above lowest perforateion, Z_{WQ} (ft)	Z_{WQ} (ft) =	1.50
Step 2) Recommended maximum outlet area per row, A_0 (in ²)	A_0 (in ²) =	0.2
$A_0 = (WQ_v) / (0.013 * Z_{WQ}^2 + 0.22 * Z_{WQ} - 0.10)$		
Step 3) Circular perforation diameter per row assuming a single column, D_1 (in)	D_1 (in) =	0.44
Step 4) Number of columns, n_c	n_c =	2.00

Design Procedure Form: Extended Dry Detention Basin (EDDB) Main Worksheet

Designer: JGD
Checked By: MES
Company: RIC
Date: 5/1/2020
Project: 18-0251 - Sequoia
Location: Proposed NW Basin, Lee's Summit, MO
Reference: APWA/MARC BMP Manual, 8.10 EDDB, pg 8-107 thru 8-128

Step 5) Design circular perforation diameter (should be between 1 and 2 inches), D_{perf} (in)	D_{perf} (in) = 0.88
Step 6) Horizontal perforation column spacing when $n_c > 1$, center to center, S_c If $D_{\text{perf}} \geq 1.0$ inch, $S_c = 4$	
Step 7) Number of rows (4" vertical spacing between perforations, center to center), n_r	n_r = 5
<u>IId. Other Pretreatment Devices</u>	
Step 1) Depth of water quality volume permanent pool, Z_{WQ} (ft)	Z_{WQ} (ft) = 1.50
Step 2) Average head of water quality pool volume over invert of v-notch, H_{WQ} (ft) $H_{WQ} = 0.5 * Z_{WQ}$	H_{WQ} (ft) = 0.75
Step 3) Average water quality pool outfall rate, Q_{WQ} (cfs) $Q_{WQ} = (WQ_v * 43,560) / (40 * 3,600)$	Q_{WQ} (cfs) = 0.01
Step 4) V-notch weir coefficient, C_v	C_v = 2.50
Step 5) V-notch weir angle, θ (deg) $\theta = 2 * (180 / \pi) * \arctan(Q_{WQ} / (C_v * H_{WQ}^{5/2}))$ V-notch angle should be at least 20 degrees. Set to 20 degrees if calculated angle is smaller.	θ (deg) = 20.0
Step 6) Top width of V-notch weir, W_v (ft) $W_v = 2 * Z_{WQ} * \tan(\theta/2)$	W_v (ft) = 0.50
Step 7) To calculate v-notch angle for EDDB with an irregular stage-volume relationship, use the V-notch Weir worksheet	

Design Procedure Form: Extended Dry Detention Basin (EDDB)
Main Worksheet

Designer: JGD

Checked By: MES

Company: RIC

Date: 5/1/2020

Project: 18-0251 - Sequoia

Location: Proposed NW Basin, Lee's Summit, MO

Reference: APWA/MARC BMP Manual, 8.10 EDDB, pg 8-107 thru 8-128

III. Flood Control

Reference APWA Specifications Section 5608

Design Procedure Form: Extended Dry Detention Basin (EDDB) Main Worksheet

Designer: JGD
Checked By: MES
Company: RIC
Date: 5/1/2020
Project: 18-0251 - Sequoia
Location: Proposed NW Basin, Lee's Summit, MO
Reference: APWA/MARC BMP Manual, 8.10 EDDB, pg 8-107 thru 8-128

IV. Trash Rock

Step 1) Total outlet area, A_0 (in²) A_{0t} (in²) = 0.7

Step 2) Depth of gravel blanket, Z_{gravel} (in)

$A_t = A_{0t} * 77 * e^{-0.124 * D}$ for single orifice outlet

$A_t = (A_{0t}/2) * 77 * e^{-0.124 * D}$ for orifice plate outlet

$A_t = 4 * A_{0t}$ for v-notch weir outlet

A_t (in²) = 50

A_t (in²) = 25

A_t (in²) = 3

V. Basin Shape

Step 1) Length to width ratio should be at least 3:1 (L:W) wherever practicable (L:W) = 4:1

Step 2) Low flow channel side lining

Concrete:

Soil/Riprap:

No low flow channel: X

Step 3) Top stage floor drainage slope (toward low flow channel), S_{ts} (%)

S_{ts} (%) = 2%

Top stage depth, D_{ts} (ft)

D_{ts} (ft) = 5

Step 4) Bottom stage volume, V_{bs} (ac-ft)

1.25 to 3ft deeper than top stage. Bottom stage shall store 10-25% of WQ_v .

V_{bs} (% of WQ_v) = 10%

V_{bs} (ac-ft) = 0.50

VI. Forebay (Optional)

Step 1) Volume should be greater than 10% of WQ_v Min Vol_{FB} (ac-ft) = 0.00

Step 2) Forebay depth, Z_{FB} (ft) Z_{FB} (ft) = 1.0

Step 3) Forebay surface area, A_{FB} (ac)

Min A_{FB} (ac) = 0.00

Min A_{FB} (ft) = 205.27

Step 4) Paved/hardbottom and sides?

Y/N? N

VI. Basin Side Slopes

Base side slopes should be at least 4:1 (H:V) Side Slope (H:V) = 3:1

VII. Dam Embankment side slopes

Dam embankment side slopes should be at least 3:1 (H:V) Dam Embankment (H:V) = 3:1

Design Procedure Form: Extended Dry Detention Basin (EDDB)
Main Worksheet

Designer: JGD
Checked By: MES
Company: RIC
Date: 5/1/2020
Project: 18-0251 - Sequoia
Location: Proposed NW Basin, Lee's Summit, MO
Reference: APWA/MARC BMP Manual, 8.10 EDDB, pg 8-107 thru 8-128

<u>IX. Vegetation</u>	
Check the method of vegetation planted in the EDDB or describe "other"	Native Grass: ☒ Irrigated Turf Grass: ☐ Other: ☐
<u>X. Inlet Protection</u>	
Indicate method of inlet protection/energy dissipation at EDDB inlet	☒ Rip Rap Rock
<u>XI. Access</u>	
Indicate that access has been provided for maintenance vehicals	☒ Yes

Design Procedure Form: Extended Dry Detention Basin (EDDB) Main Worksheet

Designer: JGD
Checked By: MES
Company: RIC
Date: 12/11/2019
Project: 18-0251 - Sequoia
Location: Proposed S Basin, Lee's Summit, MO
Reference: APWA/MARC BMP Manual, 8.10 EDDDB, pg 8-107 thru 8-128

I. Water Quality Volume

Step 1) Tributary area to EDDC, A_T (ac)	Rational C = <u>0.51</u>
Step 2) Calculate WQv using methodology in Section 6	A_T (ac) = <u>1.98</u>
Step 3) Add 20 percent to account for silt and sediment deposition in the basin	WQv (ac-ft) = <u>0.12</u>
	V_{design} (ac-ft) = <u>0.14</u>

IIa. Pretreatment

Step 1) Set water quality outlet type	Outlet type = <u>1</u>
Type 1 = single orifice	
Type 2 = perforated riser or plate	
Type 3 = v-notch weir	
Step 3) Proceed to Part IIb, IIc or IId based on water quality outlet type selected	

IIb. Water Quality Outlet, Single Orifice

Step 1) Depth of water quality volume and outlet, Z_{WQ} (ft)	Z_{WQ} (ft) = <u>0.50</u>
Step 2) Average head of water quality volume over invert of orifice, H_{WQ} (ft)	H_{WQ} (ft) = <u>0.25</u>
$H_{WQ} = 0.5 * Z_{WQ}$	
Step 3) Average water quality outfall rate, Q_{WQ} (cfs)	Q_{WQ} (cfs) = <u>0.04</u>
$Q_{WQ} = (WQ_V * 43,560) / (40 * 3,600)$	
Step 4) Set Value of orifice discharge coefficient, C_0	C_0 = <u>0.66</u>
$C_0 = 0.66$ when thickness of riser/weir plate $\leq$ orifice diameter	
$C_0 = 0.80$ when thickness of riser/weir plate $\geq$ orifice diameter	
Step 5) Water quality outlet orifice diameter (minimum of 4 inches), D_0 (in)	D_0 (in) = <u>1.57</u>
$D_0 = 12 * 2 * (Q_{WQ} / (C_0 * \pi * (2 * g * H)^{0.5}))^{0.5}$	
Step 6) To size outlet orifice for EDDDB with an irregular stage-volume relationship, use the Single Orifice Worksheet	

IIc. Water Quality Outlet, Perforated Riser

Step 1) Depth at outlet above lowest perforateion, Z_{WQ} (ft)	Z_{WQ} (ft) = <u>0.50</u>
Step 2) Recommended maximum outlet area per row, A_0 (in ²)	A_0 (in ²) = <u>8.9</u>
$A_0 = (WQ_V) / (0.013 * Z_{WQ}^2 + 0.22 * Z_{WQ} - 0.10)$	
Step 3) Circular perforation diameter per row assuming a single column, D_1 (in)	D_1 (in) = <u>3.36</u>
Step 4) Number of columns, n_c	n_c = <u>2.00</u>
Step 5) Design circular perforation diameter (should be between 1 and 2 inches), D_{perf} (in)	D_{perf} (in) = <u>6.73</u>

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Step 6) Horizontal perforation column spacing when $n_c > 1$, center to center, S_c
 If $D_{\text{perf}} \geq 1.0$ inch, $S_c = 4$

Step 7) Number of rows (4" vertical spacing between perforations, center to center), n_r

$n_r =$ 2

IId. Other Pretreatment Devices

Step 1) Depth of water quality volume permanent pool, Z_{WQ} (ft)

Z_{WQ} (ft) = 0.50

Step 2) Average head of water quality pool volume over invert of v-notch, H_{WQ} (ft)

H_{WQ} (ft) = 0.25

$$H_{WQ} = 0.5 * Z_{WQ}$$

Step 3) Average water quality pool outfall rate, Q_{WQ} (cfs)

Q_{WQ} (cfs) = 0.04

$$Q_{WQ} = (WQ_v * 43,560) / (40 * 3,600)$$

Step 4) V-notch weir coefficient, C_v

$C_v =$ 2.50

$$C_v = 0.59 - 0.57$$

Step 5) V-notch weir angle, θ (deg)

θ (deg) = 49.0

$$\theta = 2 * (180/\pi) * \arctan(Q_{WQ} / (C_v * H_{WQ}^{5/2}))$$

V-notch angle should be at least 20 degrees. Set to 20 degrees if calculated angle is smaller.

Step 6) Top width of V-notch weir, W_v (ft)

W_v (ft) = 0.43

$$W_v = 2 * Z_{WQ} * \tan(\theta/2)$$

Step 7) To calculate v-notch angle for EDDB with an irregular stage-volume relationship, use the V-notch Weir worksheet

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III. Flood Control

Reference APWA Specifications Section 5608

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IV. Trash Rock

Step 1) Total outlet area, A_0 (in²) A_{0t} (in²) = 13.3

Step 2) Depth of gravel blanket, Z_{gravel} (in)

$A_t = A_{0t} * 77 * e^{-0.124 * D}$ for single orifice outlet

A_t (in²) = 677

$A_t = (A_{0t}/2) * 77 * e^{-0.124 * D}$ for orifice plate outlet

A_t (in²) = 338

$A_t = 4 * A_{0t}$ for v-notch weir outlet

A_t (in²) = 53

V. Basin Shape

Step 1) Length to width ratio should be at least 3:1 (L:W) wherever practicable (L:W) = 4:1

Step 2) Low flow channel side lining

Concrete:

Soil/Riprap:

No low flow channel: X

Step 3) Top stage floor drainage slope (toward low flow channel), S_{ts} (%)

S_{ts} (%) = 2%

Top stage depth, D_{ts} (ft)

D_{ts} (ft) = 5

Step 4) Bottom stage volume, V_{bs} (ac-ft)

V_{bs} (% of WQ_v) = 10%

1.25 to 3ft deeper than top stage. Bottom stage shall store 10-25% of WQ_v .

V_{bs} (ac-ft) = 0.50

VI. Forebay (Optional)

Step 1) Volume should be greater than 10% of WQ_v Min Vol_{FB} (ac-ft) = 0.01

Step 2) Forebay depth, Z_{FB} (ft) Z_{FB} (ft) = 1.0

Step 3) Forebay surface area, A_{FB} (ac)

Min A_{FB} (ac) = 0.01

Min A_{FB} (ft) = 615.82

Step 4) Paved/hardbottom and sides?

Y/N? N

VI. Basin Side Slopes

Base side slopes should be at least 4:1 (H:V) Side Slope (H:V) = 4:1

VII. Dam Embankment side slopes

Dam embankment side slopes should be at least 3:1 (H:V) Dam Embankment (H:V) = 4:1

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IX. Vegetation

Check the method of vegetation planted in the EDDB or describe "other"

Native Grass: ☒
Irrigated Turf Grass: ☐
Other: ☐

X. Inlet Protection

Indicate method of inlet protection/energy dissipation at EDDB inlet

Rip Rap Rock

XI. Access

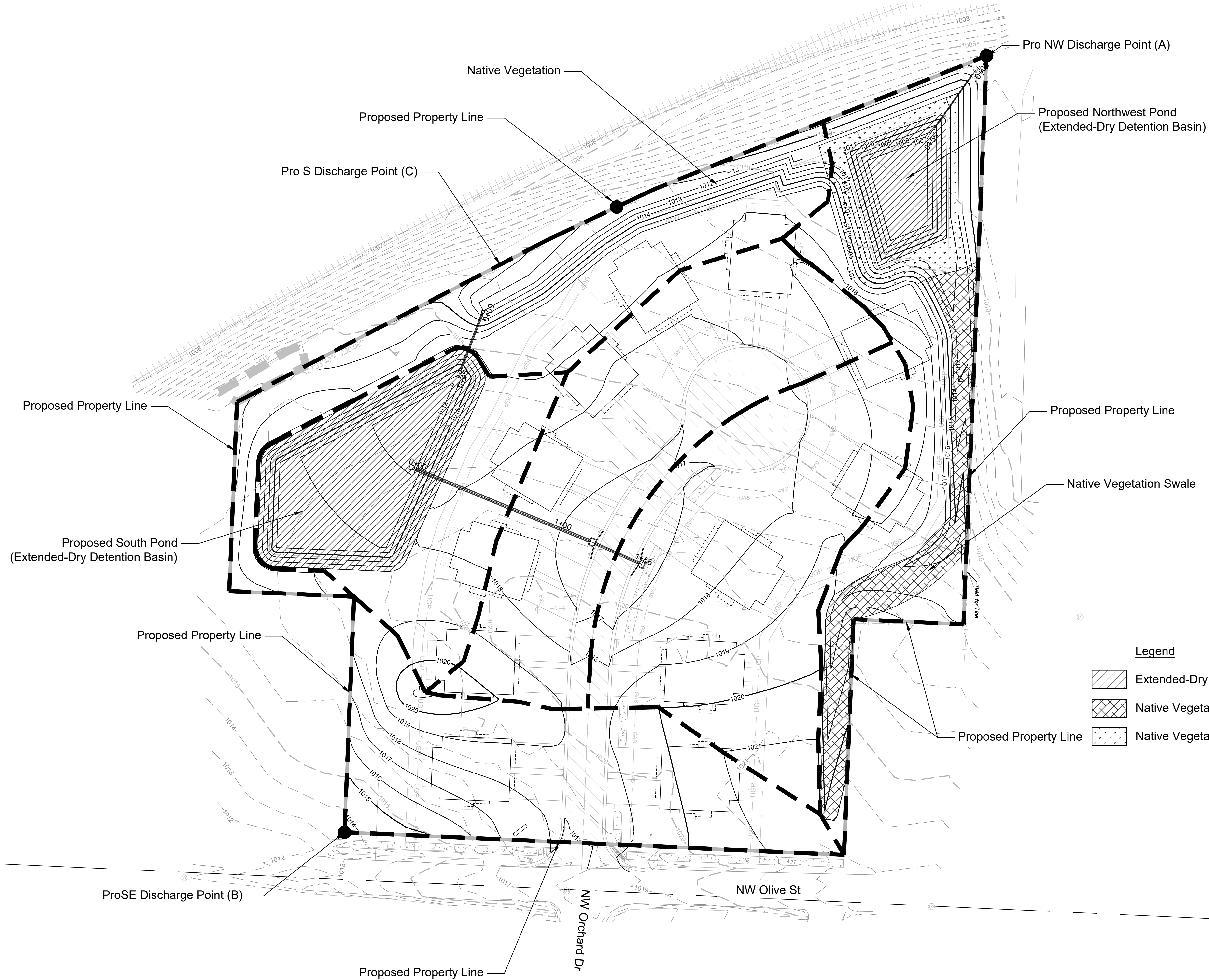
Indicate that access has been provided for maintenance vehicals

Yes

Exhibit J

Proposed BMP Location Map

May 01, 2020-10:40am
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NO.		BY	CD	DATE	ORIGINAL SUBMISSION	REVISION
1	JGD	MES		5/1/20		