

### STORM WATER DRAINAGE REPORT

## **GOPPERT ACRES – 2<sup>ND</sup> PLAT**

LEE'S SUMMIT, MISSOURI

PREPARED FOR DUSTY GOPPERT

PREPARED BY

HG CONSULT, INC.

May, 2019

Revised June, 2019



6/17/19



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Drainage Area Map: Existing Condition Drainage Area Map Post Developed Drainage Area Map NCS Soil Survey HydroCAD Drainage report



#### 3. Project Overview

The proposed project is a 5 lot, 10.18 acre residential subdivision developed in southwest Lee's Summit, Jackson County, Missouri. There is an existing street that will be extended onto the site from the east. There is an existing house on this project that is located on proposed Lot 7. The existing storm water flows from the site from the east in a north west and south west direction. There are three major existing drainage areas for this project. Drainage Area 1, located in the north east corner of property drains in a north direction to an existing residential subdivision. Drainage Area 2 drains in a south western direction to a residential subdivision, via a field inlet. Drainage Area 3 drains in a south western direction and in to an existing 3.3 acre pond. At the initial review stage, City staff concurred that the existing pond would function as the detention for the additional runoff from the development.

#### 4. Drainage Assessment of the Project Site

This site after development will divert some of the existing storm flow from two of the existing drainage areas to the existing pond (utilized as the storm water detention area). Drainage Area 1 directs storm water off site to the east by the roadway to an off-site public storm sewer system on the south side of Southwest 15<sup>th</sup> Street. Drainage Area 2 directs storm water to the existing pond through a 15" pipe at the cul-de-sac. Drainage Area 3 directs storm water to the north to a residential subdivision. Drainage Area 4 directs storm water to the south west into an existing residential subdivision being captured by an off-site area inlet. Drainage Area 5 is unchanged in direction of storm water flow. This storm water discharges into the existing pond.

Туре	Area (ac)	CN
DA-1	1.52	46
DA-2	2.42	46
DA-3	3.09	46
Total	7.03	46

#### **Existing Condition Curve Number Calculations**

#### Post Developed Curve Number Calculations

Туре	Area (ac)	CN
DA-1	0.12	69
DA-2	0.73	67
DA-3	1.20	46
DA-4	1.96	46
DA-5	3.03	46
Total	7.03	46



Curve Numbers are based on the SCS/NRSCS Chart for various site conditions. Time of concentration was considered using TR-55; however, due to the small size of the drainage basin and the amount of impervious area on the site that will just be conveying sheet flow, a time of concentration of 5 minutes was assumed. This is the minimum time of concentration per APWA 5600.

The existing and proposed drainage areas are 7.03 acres and flows to the same single point of interest where the proposed existing pond is located. Therefore, no off-site drainage will be bypassing the detention pond.

Drainage Area	Area (ac)	Q10 (cfs)	Q100 (cfs)
DA-1	1.52	1.14	4.25
DA-2	2.42	1.82	6.77
DA-3	3.09	2.32	8.65
Total	7.03	5.28	19.67

#### Discharge rates for Post Developed Condition

Drainage Area	Area (ac)	Qwq (cfs)	Q10 (cfs)	Q100 (cfs)
DA-1	0.12	0.08	0.47	0.74
DA-2	0.73	0.51	2.64	5.08
DA-3	1.20	0.84	0.90	3.36
DA-4	1.96	1.37	1.47	5.48
DA-5	3.03	2.11	1.86	6.91
Total	7.03	4.91	7.34	21.57

#### 5. Rip Rap Design

As requested, the rip rap at the end of the 15" flared section at the pond's edge design has been calculated and sized based on the City's unpublished riprap pad sizing chart for a 15" diameter pipe. The dimensions of the pad and nominal rock size are shown on the plans and the City chart is included in this report.



#### 6. Temporary Erosion and Sediment Control

During construction and prior to paving, it will be necessary to control erosion and sediment from the site during storms with in the construction timeframe. To insure that sediment does not enter the existing storm system or runs off to the existing street, perimeter containment is controlled by silt fence installation, inlet protection and an engineered detention release structure. To keep construction traffic from tracking mud onto the adjacent city street, a stabilized rock construction entrance will need to be installed. These erosion control devices, and their maintenance throughout the construction timeframe, are required by ordinance and the details for them are referenced by the City's Design and Construction Manual and shown on Detail Sheets 9 and 10.

#### 7. Conclusion

The proposed project is a proposed residential subdivision. The report has been prepared to evaluate the storm water discharge. Even though there is an increase in impervious coverage due to the street improvements and houses, it has been shown that with the majority of this changed coverage is directed to the pond and therefore very minimal impact or increase in sheet flow to downstream areas, especially to the north and east of the project, from roof drainage only. In regard to the water quality component for the project, the existing pond will receive the majority of the flow and provide the settling of suspended solids and particles.

#### 8. Design Calculations and Exhibits

See the attached for drainage area calculations, flows, rip rap, pipe and inlet sizing for the project.







ILET		T Pipe Size	Pipe Slope	Din e Turre	Rough	PIPE Design	DESIGN Depth Flow	Full	Full	NO. BY CK/APP	PY AND MAY CONTAIN NOT APPLY TO ANY COPIES
ACITY cfs] <u>1.5</u>	CAPACITY 80% [cfs] 9.2	[in] 15	[%] 2.90	Pipe Type	Coeff [MANNG] 0.011	Velocity [fps] 5.85	(inches) Q[100] 6.3	Velocity [fps] 5.9	Flow [cfs] 13.0		SIGNATURE IN BLUE INK, THE PLAN IS A COP TION CONTAINED ON THIS DOCUMENT SHALL N
										DATE REVISION	IF THIS IS NOT A BLUE INK SEAL AND THE UNAUTHORIZED ALTERATIONS. THE CERTIFICA
										5/1	R. KEVIN STERRERT, MO E-26440
			Know w Call	/hat's below before yo	bu dig.						engineers planners
						30'	45' 60'				
	55			•	D.	RAINAGE FLON	' DIRECTION				
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										DRAINAGE AR	CRES, 2ND PLAT
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										X-4 4225 DRAI 1 FEBRUA JC	REF NO. 59_TOPO WING NO. 8028 DATE RY 7, 2019 DB NO. 8028
										<b>7</b> s	HEET <b>11</b>



**Conservation Service** 

		MA	AP LEGEND			MAP INFORMATION
Area of Int	erest (AOI)	~	A-2-4		A-7	The soil surveys that comprise your AOI were mapped at
	Area of Interest (AOI)	-	A-2-5		A-7-5	1.24,000.
Soils Soil Pati	ng Polygons	-	A-2-6		A-7-6	Warning: Soil Map may not be valid at this scale.
	A-1		A-2-7		A-8	Enlargement of maps beyond the scale of mapping can cause
	A-1-a	-	A-3		Not rated or not available	line placement. The maps do not show the small areas of
	A-1-b	العريبين ا	A-4	Water Fea	atures	contrasting soils that could have been shown at a more detailed scale
	A-2	-	A-5	$\sim$	Streams and Canals	
	A-2-4	-	A-6	Transport	tation Poile	Please rely on the bar scale on each map sheet for map measurements.
	A-2-5	-	A-7		Interstate Highways	Source of Map: Natural Resources Conservation Service
	A-2-6	-	A-7-5	~	LIS Routes	Web Soil Survey URL:
	A-2-7	-	A-7-6	~	Maior Roads	Mans from the Web Soil Survey are based on the Web Marcater
	A-3	-	A-8	~	Local Roads	projection, which preserves direction and shape but distorts
	A-4	100	Not rated or not available	Backgrou	Ind	distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more
	A-5	Soil Rat	ing Points	- Ing I	Aerial Photography	accurate calculations of distance or area are required.
	A-6		A-1			This product is generated from the USDA-NRCS certified data
	A-7		A-1-a			
	A-7-5		A-1-b			Soll Survey Area: Jackson County, Missouri Survey Area Data: Version 19, Sep 13, 2018
	A-7-6		A-2			Soil map units are labeled (as space allows) for map scales
	A-8		A-2-4			1:50,000 or larger.
	Not rated or not available		A-2-5			Date(s) aerial images were photographed: Jun 11, 2017—Sep
Soil Rati	ng Lines		A-2-6			
~	A-1		A-2-7			compiled and digitized probably differs from the background
$\sim$	A-1-a		A-3			imagery displayed on these maps. As a result, some minor shifting of man unit boundaries may be evident
~	A-1-b		A-4			sinting of map unit boundaries may be evident.
~	A-2		A-5			
			A-6			

## AASHTO Group Classification (Surface)

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
10024	Greenton-Urban land complex, 5 to 9 percent slopes	A-6	7.3	39.4%
10082	Arisburg-Urban land complex, 1 to 5 percent slopes	A-6	7.0	37.5%
10181	Udarents-Urban land- Sampsel complex, 5 to 9 percent slopes	A-6	4.3	23.1%
Totals for Area of Intere	est	18.5	100.0%	

## Description

AASHTO group classification is a system that classifies soils specifically for geotechnical engineering purposes that are related to highway and airfield construction. It is based on particle-size distribution and Atterberg limits, such as liquid limit and plasticity index. This classification system is covered in AASHTO Standard No. M 145-82. The classification is based on that portion of the soil that is smaller than 3 inches in diameter.

The AASHTO classification system has two general classifications: (i) granular materials having 35 percent or less, by weight, particles smaller than 0.074 mm in diameter and (ii) silt-clay materials having more than 35 percent, by weight, particles smaller than 0.074 mm in diameter. These two divisions are further subdivided into seven main group classifications, plus eight subgroups, for a total of fifteen for mineral soils. Another class for organic soils is used.

For each soil horizon in the database one or more AASHTO Group Classifications may be listed. One is marked as the representative or most commonly occurring. The representative classification is shown here for the surface layer of the soil.

## **Rating Options**

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Lower Layer Options (Horizon Aggregation Method): Surface Layer (Not applicable)

This report was prepared with the free HydroCAD SAMPLER, which is licensed for evaluation and educational use ONLY. For actual design or modeling applications you MUST use a full version of HydroCAD which may be purchased at www.hydrocad.net. Full programs also include complete technical support,training materials, and additional features which are essential for actual design work.

Event	Rainfall (inches)	Runoff (cfs)	Volume (acre-feet)	Depth (inches)
1-Year	3.00	0.01	0.003	0.02
2-Year	3.50	0.02	0.010	0.08
5-Year	4.60	0.50	0.038	0.30
10-Year	5.30	1.14	0.064	0.50
25-Year	6.20	2.18	0.104	0.82
50-Year	6.90	3.10	0.141	1.11
100-Year	7.70	4.25	0.188	1.48
Custom	3.00	0.00	0.000	0.00

### Events for Subcatchment 1S: DRAINAGE AREA 1

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Event	Rainfall (inches)	Runoff (cfs)	Volume (acre-feet)	Depth (inches)
1-Year	3.00	0.01	0.004	0.02
2-Year	3.50	0.03	0.015	0.08
5-Year	4.60	0.79	0.060	0.30
10-Year	5.30	1.82	0.101	0.50
25-Year	6.20	3.47	0.166	0.82
50-Year	6.90	4.94	0.225	1.11
100-Year	7.70	6.77	0.299	1.48
Custom	3.00	0.00	0.000	0.00

### Events for Subcatchment 2S: DRAINAGE AREA 2

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Event	Rainfall (inches)	Runoff (cfs)	Volume (acre-feet)	Depth (inches)
1-Year	3.00	0.01	0.005	0.02
2-Year	3.50	0.04	0.020	0.08
5-Year	4.60	1.01	0.077	0.30
10-Year	5.30	2.32	0.129	0.50
25-Year	6.20	4.42	0.212	0.82
50-Year	6.90	6.31	0.287	1.11
100-Year	7.70	8.65	0.382	1.48
Custom	3.00	0.00	0.000	0.00

### Events for Subcatchment 3S: DRAINAGE AREA 3

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Event	Rainfall (inches)	Runoff (cfs)	Volume (acre-feet)	Depth (inches)
1-Year	3.00	0.01	0.005	0.02
2-Year	3.50	0.04	0.020	0.08
5-Year	4.60	1.01	0.077	0.30
10-Year	5.30	2.32	0.129	0.50
25-Year	6.20	4.42	0.212	0.82
50-Year	6.90	6.31	0.287	1.11
100-Year	7.70	8.65	0.382	1.48
Custom	3.00	0.00	0.000	0.00

### Events for Subcatchment 3S: DRAINAGE AREA 3

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Event	Rainfall (inches)	Runoff (cfs)	Volume (acre-feet)	Depth (inches)
1-Year	3.00	0.13	0.006	0.59
2-Year	3.50	0.20	0.009	0.85
5-Year	4.60	0.36	0.015	1.52
10-Year	5.30	0.47	0.020	1.99
25-Year	6.20	0.62	0.026	2.63
50-Year	6.90	0.74	0.032	3.16
100-Year	7.70	0.88	0.038	3.78

### Events for Subcatchment 1S: DRAINAGE AREA 1

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Event	Rainfall (inches)	Runoff (cfs)	Volume (acre-feet)	Depth (inches)
1-Year	3.00	0.70	0.031	0.51
2-Year	3.50	1.06	0.046	0.76
5-Year	4.60	1.99	0.084	1.38
10-Year	5.30	2.64	0.111	1.83
25-Year	6.20	3.53	0.149	2.45
50-Year	6.90	4.25	0.180	2.96
100-Year	7.70	5.08	0.217	3.57

### Events for Subcatchment 2S: DRAINAGE AREA 2

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Event	Rainfall (inches)	Runoff (cfs)	Volume (acre-feet)	Depth (inches)
1-Year	3.00	0.00	0.002	0.02
2-Year	3.50	0.02	0.008	0.08
5-Year	4.60	0.39	0.030	0.30
10-Year	5.30	0.90	0.050	0.50
25-Year	6.20	1.72	0.082	0.82
50-Year	6.90	2.45	0.111	1.11
100-Year	7.70	3.36	0.148	1.48

### Events for Subcatchment 3S: DRAINAGE AREA 3

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Event	Rainfall (inches)	Runoff (cfs)	Volume (acre-feet)	Depth (inches)
1-Year	3.00	0.01	0.003	0.02
2-Year	3.50	0.02	0.012	0.08
5-Year	4.60	0.64	0.049	0.30
10-Year	5.30	1.47	0.082	0.50
25-Year	6.20	2.81	0.135	0.82
50-Year	6.90	4.00	0.182	1.11
100-Year	7.70	5.48	0.242	1.48

### Events for Subcatchment 4S: DRAINAGE AREA 4

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Event	Rainfall (inches)	Runoff (cfs)	Volume (acre-feet)	Depth (inches)
1-Year	3.00	0.01	0.004	0.02
2-Year	3.50	0.03	0.016	0.08
5-Year	4.60	0.81	0.062	0.30
10-Year	5.30	1.86	0.104	0.50
25-Year	6.20	3.54	0.170	0.82
50-Year	6.90	5.04	0.229	1.11
100-Year	7.70	6.91	0.305	1.48

### Events for Subcatchment 5S: DRAINAGE AREA 5

#### **Rip-Rap Apron**

Max Velocity: <= 36" 10 ft/s > 36" 12 ft/s

Outlet	Apron Dimensions		Rip-Rap	Rip-Rap	Rip-Rap	DS Channel	
Diameter	Length	Width	Thickness	Quantity	Size - D50	Weight	Width (Min.)
<u>(ln.)</u>	(Ft.)	<u>(Ft.)</u>	(Ft.)	(Cu. Yd.)	(In.)	(Lb.)	(ft)
15	12	5	1.5	3	12	50	5
18	12	5	1.5	3	12	50	5
24	14 🖌	6	1.5	5	12	50	5
30	16	8	1.5	7	12	50	6
36	18	9	2	12	15	100	8
42	21	11	2	17	-15	100	8
48	24	12	2	21	15	100	10
54	27 `	14	2.5	35	18	200	10
60	30	15	2.5	42	18	200	12
>60"	Additional Considerations For Use of Rip-Rap Apron						

Standard Construction Note (Example):

Minimum Notation needel and on plans

200 STA. 0+00.00, LINE 2 INSTALL 18" RCP FLARED END SECTION W/ TOEWALL AND AND 3 CU, YD, (12'Lx5'Wx1.5'T) STONE RIP-RAP USING A MIN. 50# (D50=12") STONE. PLACE FILTER FABRIC PRIOR TO **INSTALLATION OF RIP-RAP** N 12563.1256 E 9568.2357

Standard Detail (Place in all projects using rip-rap apron)

XRIPRAP-STONE.DWG

NOTE:

Make sure to include city standard detail for toewall. Typically, called "End Section Detail".