

# Mid-Continent Public Library East Lee's Summit Branch Preliminary Stormwater Study

BATTERY DRIVE AND SOUTHEAST BLUE PARKWAY LEE'S SUMMIT, MISSOURI

## August 17, 2018

Prepared for: Mid-Continent Public Libraries (MCPL)

### Prepared by:

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### **GENERAL INFORMATION**

This Stormwater Management Study is being submitted on behalf of the Mid-Continent Public Library (MCPL) for the proposed library facility located at the northeast corner of SE Blue Parkway and Battery Drive.

### **Project Location and Description**

The proposed site is located on Lot 2 of the Magnolia Place at Charleston Park, 1st Plat in the Northeast ¼ of Section 10, Township 47 North, Range 31 West, in Jackson County, Lee's Summit, Missouri and includes approximately 3.8 acres. The site is located at the northeast corner of the SE Blue Parkway and Battery Drive intersection and is generally bounded by Village Cooperative of Lee's Summit to the north, SE Battery Dr to the west, SE Blue Parkway to the south, and an undeveloped lot to the east (See Figure 2). The Church of Jesus Christ of Latter-Day Saints lies east of the undeveloped lot. The proposed development includes a 18,500 S.F. library facility with associated parking lots, landscaping, grading, and utilities. The entirety of the site is located outside of the 100-Year FEMA Floodplain.

### Study Purpose

The purpose of this study is to provide a Stormwater Management Plan for the proposed development in accordance with the American Public Works Association (APWA) *Standard Specifications and Design Criteria* Section 5600 "Storm Drainage Systems and Facilities", APWA Manual of Best Management Practices (BMP) for Stormwater Quality, and applicable City of Lee's Summit, Missouri guidelines.

### Soils Descriptions

Soil classifications were obtained from the Natural Resource Conservation Service's website by utilizing the Web Soil Survey feature. The site soil composition and classification are listed below:

10082 – Arisburg-Urban Land Complex, 1 to 5 percent slopes – HSG Type C.
10180 – Udarents-Urban Land-Sampsel Complex, 2 to 5 percent slopes - HSG Type C.

\*HSG – Hydrologic Soils Group

See Soils Map in Appendix B.

### **METHODOLOGY**

### **General Criteria and References**

Analytical and design criteria conform to those of Division V - Section 5600 – "Storm Drainage Systems and Facilities" of the Kansas City Metropolitan Chapter of the American Public Works Association's "Standard Specifications and Design Criteria". Based on these criteria, Post-development discharge rates for 1, 10, and 100-year storm events will be limited to provisions in section 5608.4-C1 Performance Criteria – "Comprehensive Control". Post-development discharge rates are limited to 0.5 cfs per acre for 1-year, 2.0 cfs per acre for 10-year, and 3.0 cfs per acre for 100-year storm events. Pre and post-development flows from the site are shown below and were calculated using HEC-HMS for the 1, 10 and 100-year storm events. Existing and proposed hydrographs were calculated using the 24-hour SCS Type II rainfall distribution. Existing times of concentration were determined using Inlet Time and Travel Time equations found in Section 5602.7 of APWA Section 5600. A minimum inlet time of five minutes was used when calculated times were under five minutes. Proposed times of concentration were calculated in the same manner.

### HYDROLOGIC/HYDRAULIC ANALYSES

### **Existing Conditions Analysis**

The existing site is an undeveloped parcel of land that consist of native vegetation. The site is bounded by SE Battery Drive to the west, The Village Cooperative Apartments to the north, undeveloped property to the east, and SE Blue Parkway to the South. Currently, Blue Parkway lies within MoDOT's US-50 right of way.

The existing drainage for the site is split by a ridge into two outfalls, "A" and "B". The east section drains to outfall "A" and the west section drains to outfall "B". Outfall "A" drains north into an existing storm system along SE Battery Drive. Outfall "B" drains northeast to an existing storm structure in the lot of the Village Cooperative Apartments and then east in the existing storm system.

South of the property there is an existing ditch that lies within US-50 right of way. The ditch drains run-off from Blue Parkway. There is no curb and gutter on Blue Parkway, just a graveled shoulder that allows run-off to drain into the existing ditch. There is an existing 18x24 arch culvert that drains to the ditch from under Battery Drive flows east along the north side of Blue Parkway.

As stated previously, there is an undeveloped lot to the east of the property that is owned by Richard D. Link. Mr. Link is also the person who sold Lot 2, Magnolia place to MCPL. The proposed development did not require all of the property for Lot 2. The east 81' of the property was excluded from the sale and remains the property of Richard D. Link. The proposed drive from the south will be a common access drive that will serve both the MCPL property and the future developed lot to the east. The east half of this drive will be constructed on the undeveloped lot under a mutual understanding with Mr. Link.

For the purposes of the drainage calculations moving forward this area will be included, therefore the studied area will increase from 3.8 acres to 4.5 acres.

A composite curve number was generated for the site by referencing the Web Soil Survey available in Appendix C, APWA Section 5600 and considered the following factors:

- Existing impervious area
- Existing pervious area
- Hydrologic soil group

The following tables summarize the pre-development composite curve number generation.

Sub-Area	Area (AC)	Soil Group	Curve Number
Pasture (Good)	3.0	С	74
Pasture (Good)	1.5	С	74

 Table 1: Pre-Development Curve Number Analysis

The existing peak discharge rates for the 1-year, 10-year, and 100-year storm events from the site are shown in Table 2 below:

**Table 2: Existing Peak Flows** 

Sub-Area / Outfall	Area (acres)	Q (1-Year Storm) (cfs)	Q (10-Year Storm) (cfs)	Q (100-Year Storm) (cfs)
A	3	5.2	11	19.2
В	1.5	2.6	5.5	9.6

### Proposed Conditions Analysis

The proposed Public Library will include a 18,500 SF library with associated parking lots, landscaping, grading, and utilities. A site plan has been included in Appendix A. The site will generally continue to drain in the same pattern as existing. Drainage from the site will enter into an enclosed storm sewer system that will be constructed with the development. The proposed system will collect drainage from the parking area and building. The increase in impervious area will increase runoff from the site. To mitigate the increase in runoff, the following strategy will be implemented.

Outfall A – The entire site will drain north and directed into a detention basin and water treatment facility. Due to the site design, the drainage area for this outfall will increase. However, the detention facility is designed to mitigate the increased runoff to this outfall. A control structure will limit the 1, 10, and 100-year storm events to pre-construction levels.

A proposed drainage map is included in Appendix A.

A post-development composite curve number was generated using the same methodology implemented during the pre-development curve number analysis. Table 3 below summarizes the post-development composite curve number generation.

Sub-Area	Area (AC)	Soil Group	Curve Number
Pavement, Buildings, Impervious	2.0	С	98
Turf (Good)	2.5	С	85

 Table 3: Post-Development Curve Number Analysis

A peak flow analysis of the post-development site was conducted using HEC-HMS, the composite curve number, and rainfall and distribution information acquired from APWA section 5600. Post-development peak flows to the outfall are summarized in the Table 4. Detailed reports from HEC-HMS are available in Appendix D

Table 4: Proposed Peak Flows

Sub-Area / Outfall	Tributary Area	Q (1-Year Storm)	Q (10-Year Storm)	Q (100-Year Storm)
	(acres)	(cfs)	(cfs)	(cfs)
Outfall A	4.5	12.4	21.1	33.4

The existing ditch to the south will remain essentially unchanged. The drainage area, for the ditch, lies within the existing right of way. An embankment will need to be constructed across the ditch for the southern entrance. A 30" culvert will be installed under the

entrance to maintain flow in the ditch. The 30" culvert will receive the flow from the existing 18x24 (24" equivalent) local drainage from Blue Parkway.

### Stormwater Detention Requirements

One proposed detention pond will be utilized to mitigate the increase in flow due to an increase in impervious area. The Detention Basin will be located in the northern part of the property. It will collect runoff from the 4.5-acre property and drain to Outfall A. The pond has an outlet pipe that connects to an orifice plate within a junction box structure that will be within the dam.

To meet water treatment requirements, the water quality volume (WQv) will be controlled by a conduit at the bottom of the basin. The conduit will release the water quality volume over a 40-hour period to allow pollutants to settle out of this precipitation event.

An orifice will be located above the WQv surface elevation to control the 1 and 10-year storms. Both storms have been analyzed through the control structure and will release below the pre-existing storm events. The 100-year storm event will flow into a weir placed at the top of the control structure. The dam will have an emergency spillway to control the 100-year overflow.

Table 5 provides the water surface elevations (WSE's) and peak flows for the proposed detention basin.

Table 5: Detention Basin, WSE	
Description	Detention Basin
Bottom of Basin	1007
Total Storage Volume	1.64
Top of Dam Elevation	1012
WQv Orifice	1007.5, 1 – 2"
(IE Elevation, Pipe Size)	(ft, pipe size)
Water Quality Volume	1008.5, 0.2, 0.1
WSE, Storage, Peak Outflow	(ft, ac-ft, cfs)
1-year & 10-Year Orifice	1009, 1-6"
(IE Elevation, Pipe Size)	(ft, pipe size)
10–Year Storm	1010.4, 0.8, 1.4
WSE, Storage, Peak Outflow	(ft, ac-ft, cfs)
100–Year Storm Weir	1010, 16
(Elevation, Length)	(ft, lf)
100–Year Storm	1011.2, 1.2, 9.4
WSE, Storage, Peak Outflow	(ft, ac-ft, cfs)

Table 5: Detention Basin, WSE's and Peak Flows

Table 6 shows the overall peak flow for the site pre and post-construction. Note that peak flow for post-construction has been lowered in all storm events.

Site	Q (1-Year Storm) (cfs)	Q (10-Year Storm) (cfs)	Q (100-Year Storm) (cfs)
Pre-Construction	5.2	11	19.2
Section 5600 Allowable Release Rate	2.3	9	13.5
Post Construction	1.4	3.3	9.4
% Change	-73%	-70%	-51%

### Table 6: Peak Flow Change Analysis

### STORMWATER TREATMENT REQUIREMENTS

As stated previously, the proposed detention is designed to act an extended dry bottom detention facility will be used to treat stormwater per MARC water quality standards. The orifice plate for the basin will be sized to release the water quality volume (1.37") over a 40-hour period to allow pollutants to settle from runoff before entering the public stormwater system. The maximum storage for the water quality event in the basin will be 0.2 acre-ft reaching a peak water surface of elevation 1008.5 feet.

### **CLEAN WATER ACT SECTION 404 PERMITTING REQUIREMENTS**

No jurisdictional Waters of the United States have been identified on the study site. Therefore, a Section 404 permit is not required.

### FEMA/DWR PERMIT REQUIREMENTS

No FEMA permitting or submittals will be required on this site because there are no FEMA delineated floodplains on the site. A copy of the FIRM map for this area has been included in Appendix B.

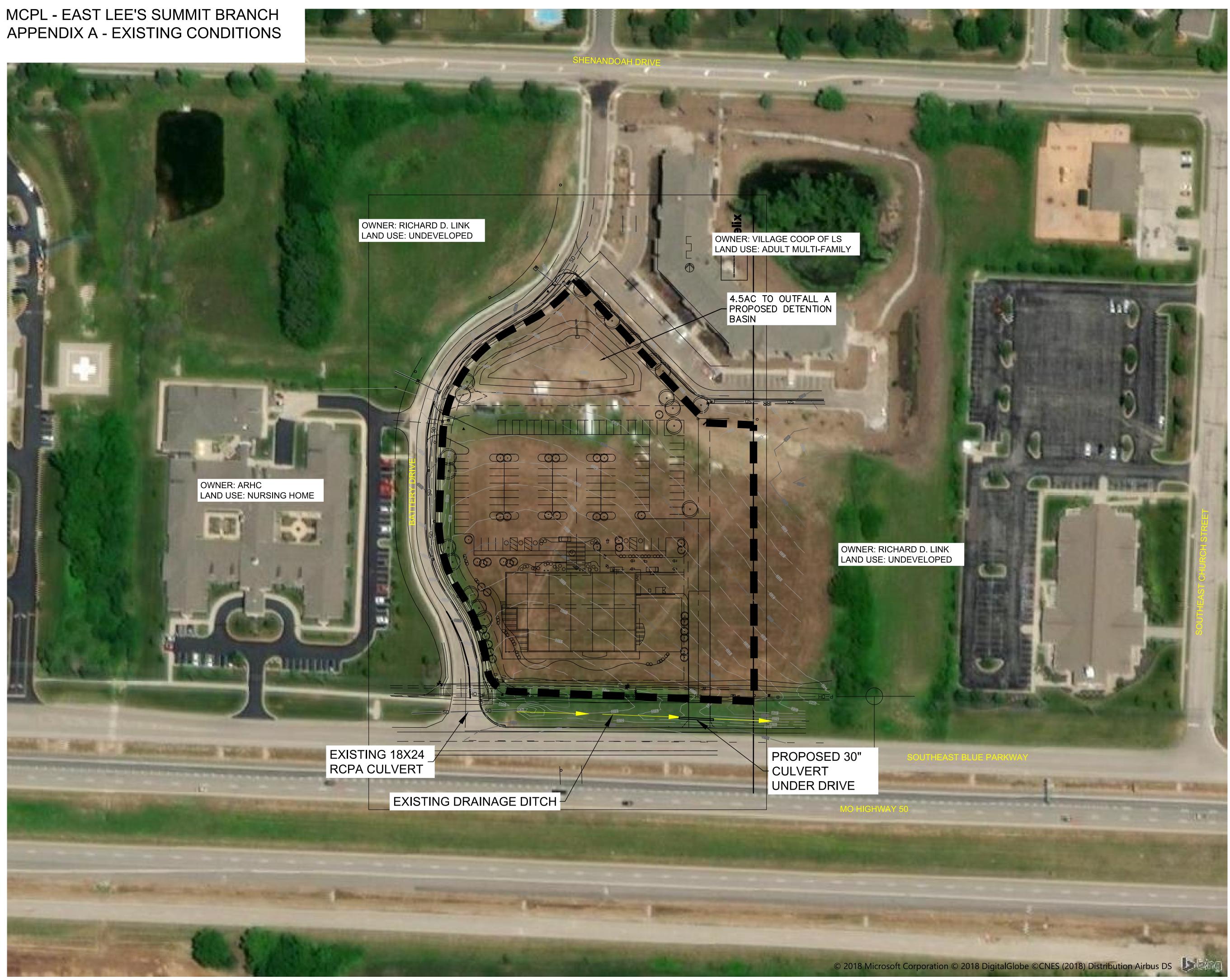
### **CONCLUSIONS AND RECOMMENDATIONS**

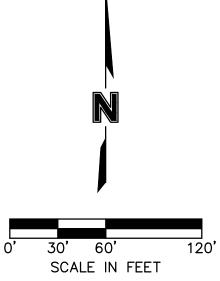
As outlined in the preceding report, increased runoff rates in the post-development conditions are mitigated by the detention basins. Drainage patterns on the site remain relatively unchanged. An extended dry detention basin has been designed to maintain or improve storm water quality. Based on these facts and other information provided herein, we request that this stormwater study be approved.

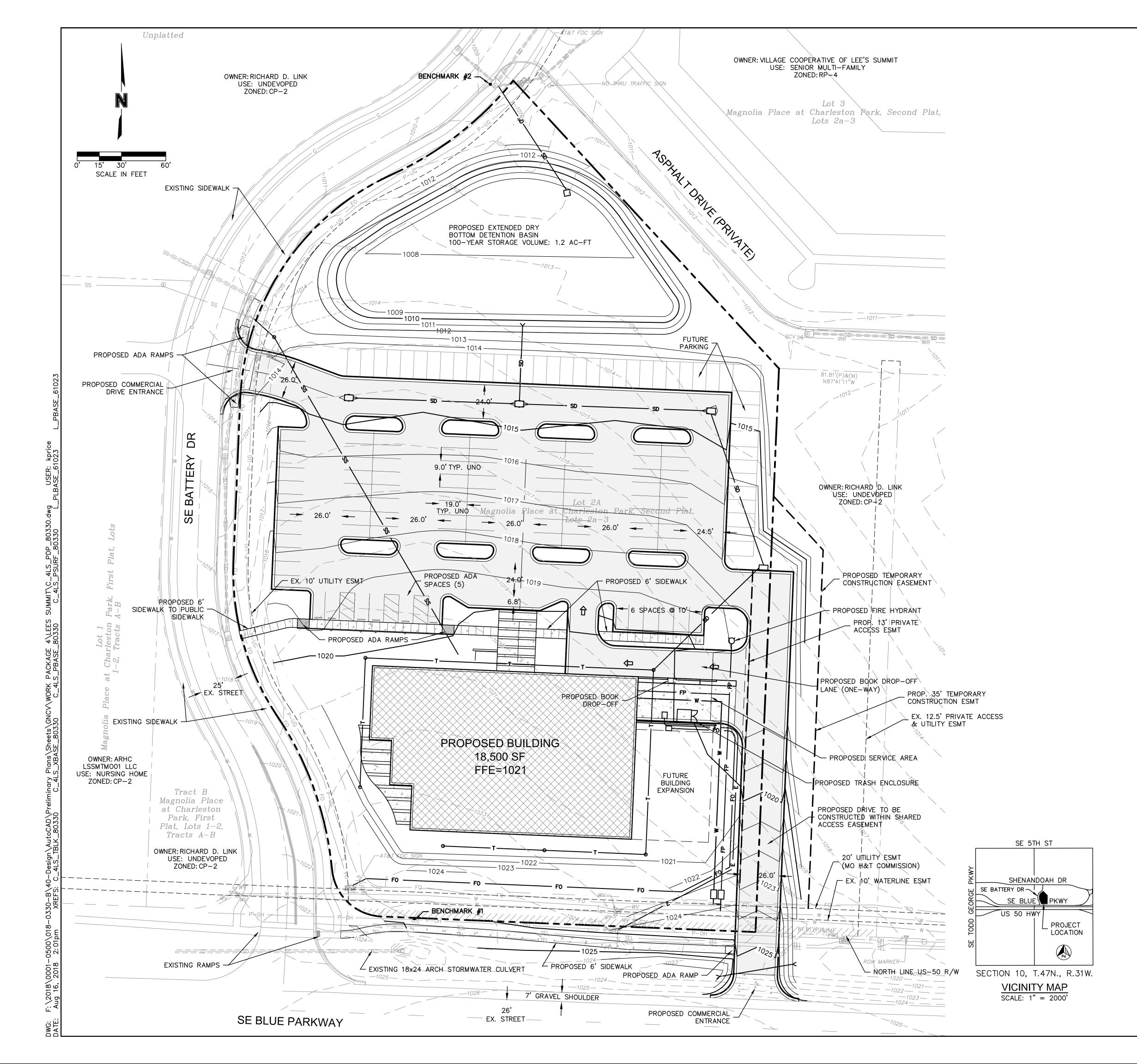
# Appendix A Map Exhibits



30' 60' SCALE IN FEET







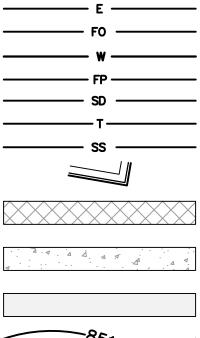
SITE DATA				
	ZO	NING & SITE	AREA	
PROPOSED USE:	Ρl	JBLIC LIBRAR	ſ	
		SITE AREA	۱.	ZONING
LOT 2 (AS DECRIBED):	3.8	BO ACRES		CP-2
IMPREVIOUS:	1.7	71 ACRES (45	%)	
PERVOUS:	2.0	09 ACRES (55	5%)	
BUILDING AREA				
BUILDING TYPE: #		# STORIES	SQ	UARE FOOTAGE
BUILDING	1			18,500 SF
FUTURE	1			4,700 SF
		PARKING		
USE	REQUIRED		D	PROVIDED
LIBRARY	4 PER 1000 SF = 74		= 74	83
ADA	3	(PER CITY T	ABLE)	5
TOTAL	7	4		88 (INCLUDING ADA)

## **EXISTING CONDITIONS LEGEND**

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PROPERTY LINES EASEMENT LINES OVERHEAD ELECTRIC UNDERGROUND ELECTRIC UNDERGROUND TELEPHONE UNDERGROUND FIBER OPTIC GAS LINE WATER LINE STORM SEWER LINE SANITARY SEWER LINE GRADE CONTOURS

## PROPOSED CONDITIONS LEGEND



PROPOSED	UNDERGROUND ELECTRIC
PROPOSED	FIBER OPTIC
PROPOSED	WATER LINE
PROPOSED	FIRE PROTECTION LINE
PROPOSED	STORM SEWER LINE
PROPOSED	TURF DRAIN LINE
PROPOSED	SANITARY SEWER SERVICE
CONCRETE	CURB & GUTTER
PROPOSED	BUILDING
PROPOSED	CONCRETE SIDEWALK
PROPOSED	LIGHT DUTY ASPHALT

PROPOSED GRADE CONTOURS

## **BENCHMARKS**:

### BENCHMARK #1: ELEVATION=1027.22'

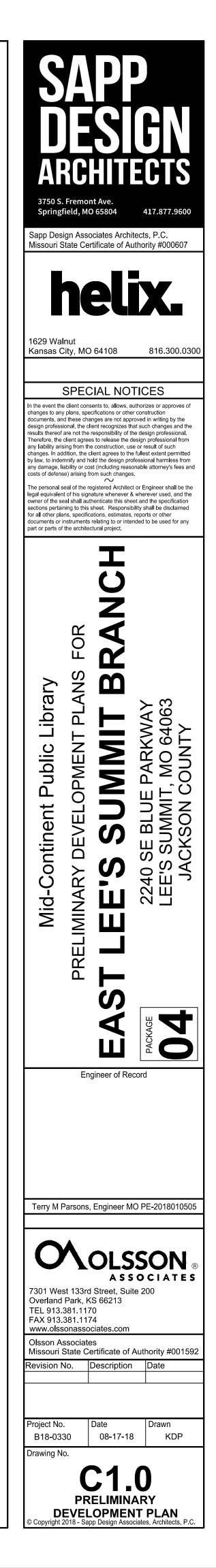
SET RAILROAD SPIKE IN THE NORTH SIDE OF A POWER POLE,  $66' \pm$  NORTH OF THE Q OF SE BLUE PARKWAY, 86' EAST OF THE Q OF SE BATTERY DRIVE.

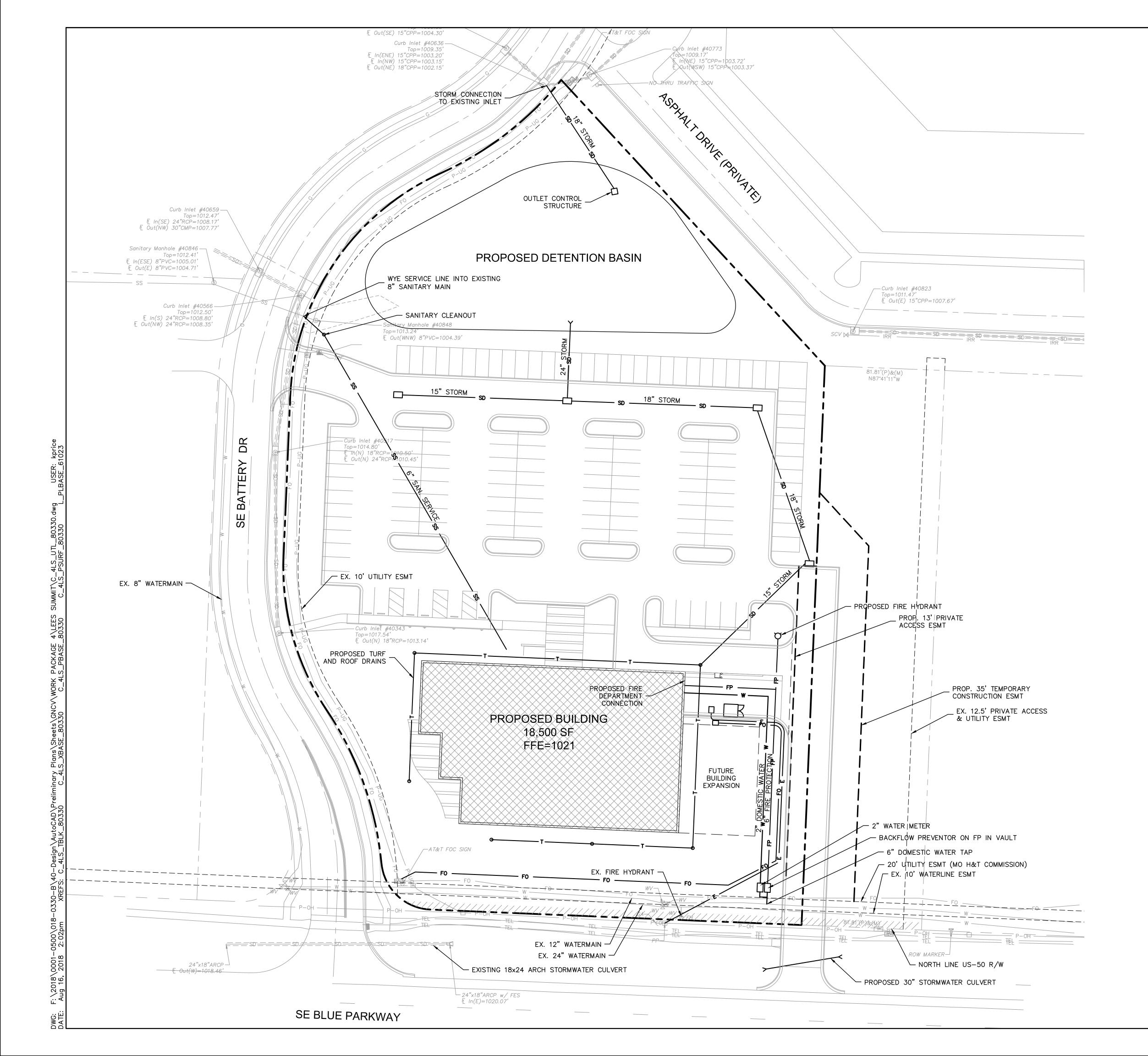
### BENCHMARK #2: ELEVATION=1009.31'

SET CHISELED "□" CUT ON THE SOUTHWESTERLY CORNER OF A CONCRETE CURB INLET #40636, 630'± NORTH OF THE € OF SE BLUE PARKWAY, 15' EAST OF THE € OF SE BATTERY DRIVE.

## LEGAL DESCRIPTION:

LOT 2A, MAGNOLIA PLACE AT CHARLESTON PARK, SECOND PLAT, LOTS 2A-3, A SUBDIVISION IN LEE'S SUMMIT, JACKSON COUNTY, MISSOURI, EXCEPT THE EAST 81.81 FEET THEREOF, CONTAINING 165,561 SQUARE FEET OR 3.8008 ACRES, MORE OR LESS.





## **EXISTING CONDITIONS LEGEND**

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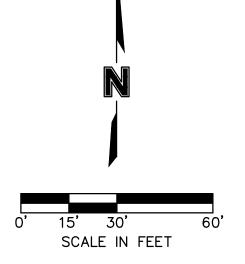
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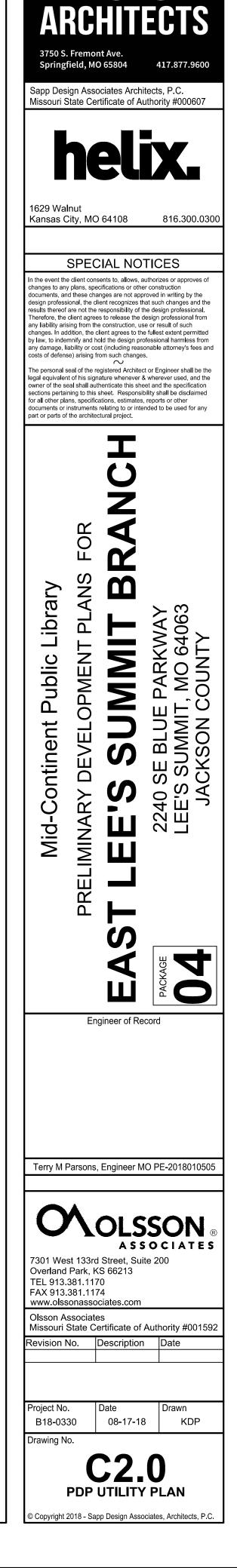
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PROPOSED UNDERGROUND ELECTRIC PROPOSED FIBER OPTIC PROPOSED WATER LINE PROPOSED FIRE PROTECTION LINE PROPOSED STORM SEWER LINE PROPOSED TURF DRAIN LINE PROPOSED SANITARY SEWER SERVICE CONCRETE CURB & GUTTER

PROPOSED BUILDING





**Appendix B** FEMA Flood Classification Firm

### NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (8FE) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Sillwater Elevations tables contained within the Flood Insurance Study (FIS) Report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation that Parents in the FIR Report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study Report for this juriscition.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control** structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study Report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Missouri State Plane West Zone (FIPS zone 2403). The **horizontal datum** was NAD 83, GRS 1980 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1998. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929, and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <u>http://www.ngs.noaa.gov</u> or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282 (301) 713-3242

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (**301) 713-3242**, or visit its website at <a href="http://www.ngs.noaa.gov">http://www.ngs.noaa.gov</a>.

Base map information shown on this FIRM was derived from the U.S.D.A Farm Service National Agriculture ImageryProgram (NAIP) dated 2014. Produced at scale of 1:24,000.

The **profile baselines** depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the **profile baseline**, in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.

Based on updated topographic information, this map reflects more detailed and up-to-date stream channel configurations and floodplain delineations than those shown on the previous FIRM for this jurisdiction. As a result he Flood Profiles and Floodway Data tables for multiple streams in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on the map. Also, the road to floodplain relationships for unrevised streams may differ from what is shown on previous maps.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

For information on available products associated with this FIRM visit the Map Service Center (MCC) website at <u>http://mscfema.gov</u>. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the MSC website.



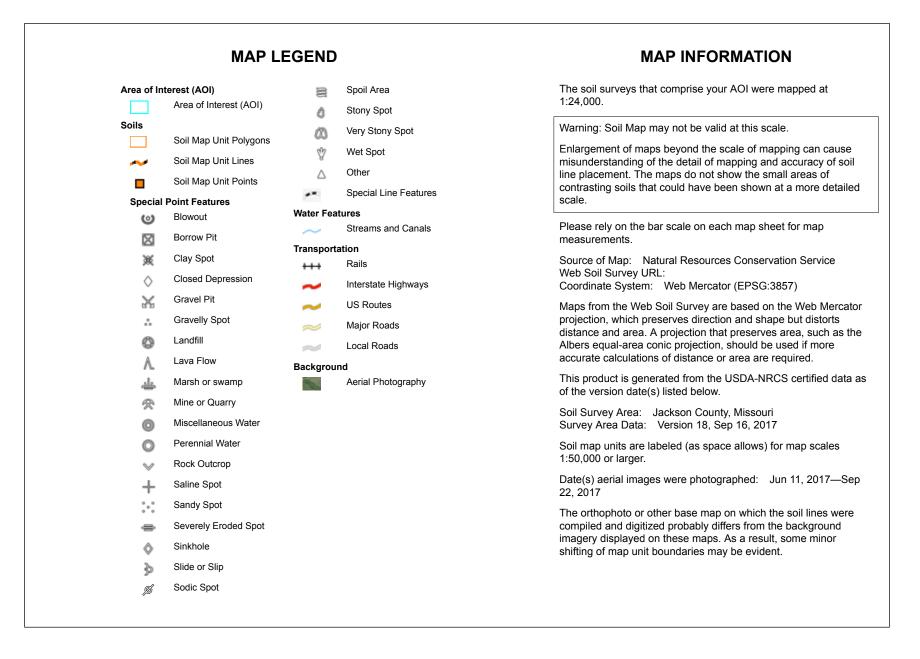
	LEGEND	
5"	SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 195 ANNUAL OHNORE FLOOD The 1% amual charen flood (100 year flood) also known as the base flood, late the a 1% charen of being equaled or exceeded in any gave year. The Special Flood haz the area subject to flooding by the 1% annual charen flood. Areas of Special Flood haz indicate Zaras A, Edr, MA, AM, AV, R9 V, and VE. The Base Flood Bearbon is the wate	rd Area is
	elevation of the 1% annual chance flood. ZONE A No Base Flood Elevations determined.	er-sundce
	ZONE AE Base Flood Elevations determined.	
4	ZONE AH Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood determined.     ZONE AO Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain);	
	depths determined. For areas of alluvial fan flooding, velocities also ZONE AR Special Flood Hazard Areas formerly protected from the 1% annual	o determined.
	flood by a flood control system that was subsequently decertified. AR indicates that the former flood control system is being restored protection from the 19% annual chance or greater flood. ZONE A99 Area to be protected from 1% annual chance flood by a Federal flo	od
	ZONE V         Coastal flood zone with velocity hazard (wave action); no Base Flood	
	determined. ZONE VE Coastal flood zone with velocity hazard (wave action); Base Flood determined.	Elevations
	FLOODWAY AREAS IN ZONE AE	
	The floodway is the channel of a stream plus any adjacent floodplain areas that must b encroachment so that the 1% annual chance flood can be carried without substantial i flood heights.	e kept free of increases in
	OTHER FLOOD AREAS ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood;	with
	average depths of less than 1 foot or with drainage areas less than 1 s mile; and areas protected by levees from 1% annual chance flood. OTHER AREAS	quare
	ZONE X Areas determined to be outside the 0.2% annual chance floodplain. ZONE D Areas in which flood hazards are undetermined, but possible.	
	COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS	;
	OTHERWISE PROTECTED AREAS (OPAS)	
	CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard /	Areas.
	1% Annual Chance Floodplain Boundary     0.2% Annual Chance Floodplain Boundary	
	Floodway boundary	
	CBRS and OPA boundary	
	Boundary dividing Special Flood Hazard Area Zones and dividing Special Flood Hazard Areas of different Base Floo flood durbs: or Flood velocities	
	flood depths, or flood velocities. 	
	(EL 987) Base Flood Elevation value where uniform within zone; feet*	elevation in
	*Referenced to the North American Vertical Datum of 1988	
	A Cross section line (23) (23) Transect line	
	Culvert	
	Bridge 45° 02' 08", 93° 02' 12" Geographic coordinates referenced to the North America	an Datum of
	1983 (NAD 83) Western Hemisphere 3100000 FT 5000-foot ticks: Missouri State Plane West Zone (FIPS Zone 2403), Transverse Mercator projection	
	DX5510 X Bench mark (see explanation in Notes to Users section of panel)	of this FIRM
	• M1.5 River Mile	
	MAP REPOSITORIES Refer to Map Repositories list on Map Index	
	EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 29, 2006	
	EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL January 20, 2017 - to change Special Flood Hazard Areas.	
	For community map revision history prior to countywide mapping, refer to the Comm Map History table located in the Flood Insurance Study report for this jurisdiction.	unity
	To determine if flood insurance is available in this community, contact your insurance or call the National Flood insurance Program at 1-800-838-6620.	e agent
ı		
	MAP SCALE 1" = 500' 250 0 500 1000 FET	
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National Cooperative Soil Survey

**Conservation Service** 

# Appendix C Soil Map



USDA

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
10082	Arisburg-Urban land complex, 1 to 5 percent slopes	3.3	79.5%
10180	Udarents-Urban land-Sampsel complex, 2 to 5 percent slopes	0.9	20.5%
Totals for Area of Interest		4.2	100.0%



## Jackson County, Missouri

### 10082—Arisburg-Urban land complex, 1 to 5 percent slopes

### Map Unit Setting

National map unit symbol: 2w7ld Elevation: 750 to 1,130 feet Mean annual precipitation: 39 to 45 inches Mean annual air temperature: 50 to 55 degrees F Frost-free period: 177 to 220 days Farmland classification: All areas are prime farmland

### Map Unit Composition

Arisburg and similar soils: 61 percent
Urban land: 30 percent
Minor components: 9 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

### **Description of Arisburg**

### Setting

Landform: Interfluves Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Convex Across-slope shape: Convex Parent material: Loess

### **Typical profile**

Ap - 0 to 6 inches: silt loam A - 6 to 13 inches: silt loam Bt - 13 to 19 inches: silty clay loam Btg - 19 to 56 inches: silty clay loam BCg - 56 to 79 inches: silty clay loam

### **Properties and qualities**

Slope: 1 to 5 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Somewhat poorly drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: About 18 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: High (about 11.5 inches)

### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e

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*Hydrologic Soil Group:* C *Ecological site:* Loess Upland Prairie (R107BY007MO) *Hydric soil rating:* No

### **Description of Urban Land**

### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydric soil rating: No

### **Minor Components**

### Sharpsburg

Percent of map unit: 3 percent Landform: Ridges Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Linear Across-slope shape: Linear Ecological site: Loess Upland Prairie (R109XY002MO) Hydric soil rating: No

### Sampsel

Percent of map unit: 3 percent Landform: Hills Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Concave Ecological site: Interbedded Sedimentary Upland Savanna (R109XY010MO) Hydric soil rating: Yes

### Greenton

Percent of map unit: 3 percent Landform: Hillslopes Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Convex Ecological site: Loess Upland Prairie (R109XY002MO) Hydric soil rating: No

## **Data Source Information**

Soil Survey Area: Jackson County, Missouri Survey Area Data: Version 18, Sep 16, 2017

## Jackson County, Missouri

# 10180—Udarents-Urban land-Sampsel complex, 2 to 5 percent slopes

### Map Unit Setting

National map unit symbol: 1n85h Elevation: 600 to 900 feet Mean annual precipitation: 33 to 43 inches Mean annual air temperature: 50 to 57 degrees F Frost-free period: 175 to 220 days Farmland classification: All areas are prime farmland

### Map Unit Composition

Udarents and similar soils: 41 percent Urban land: 39 percent Sampsel and similar soils: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

### **Description of Udarents**

### Setting

Landform position (two-dimensional): Summit Landform position (three-dimensional): Crest Down-slope shape: Convex Across-slope shape: Convex Parent material: Mine spoil or earthy fill

### **Typical profile**

C1 - 0 to 5 inches: silt loam C2 - 5 to 80 inches: silty clay loam

### **Properties and qualities**

Slope: 2 to 5 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Somewhat poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.14 to 0.57 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: Moderate (about 9.0 inches)

### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6e Hydrologic Soil Group: C Ecological site: Deep Loess Upland Prairie (R107BY002MO)

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*Other vegetative classification:* Mixed/Transitional (Mixed Native Vegetation) *Hydric soil rating:* No

### **Description of Urban Land**

### Setting

Landform: Interfluves Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Across-slope shape: Convex

### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydric soil rating: No

### **Description of Sampsel**

### Setting

Landform: Hillslopes Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Convex Parent material: Residuum weathered from shale

### **Typical profile**

*Ap - 0 to 13 inches:* silty clay loam *Bt - 13 to 80 inches:* silty clay

### **Properties and qualities**

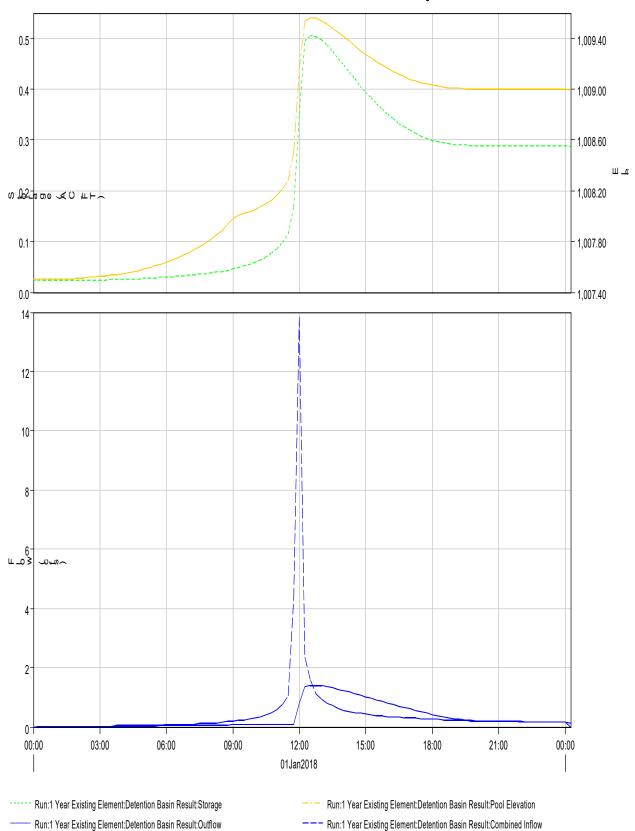
Slope: 2 to 5 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Somewhat poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 0 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: Moderate (about 8.6 inches)

### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: C/D Ecological site: Wet Footslope Prairie (R112XY041MO) Other vegetative classification: Grass/Prairie (Herbaceous Vegetation)

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**Appendix D** Drainage and Detention Calculations



Reservoir "Detention Basin" Results for Run "1 Year Existing"

Project: ELS Simulation Run: 1 Year Existing Reservoir: Detention Basin

Start of Run:01Jan2018, 00:00End of Run:02Jan2018, 00:15Compute Time:15Aug2018, 17:26:03

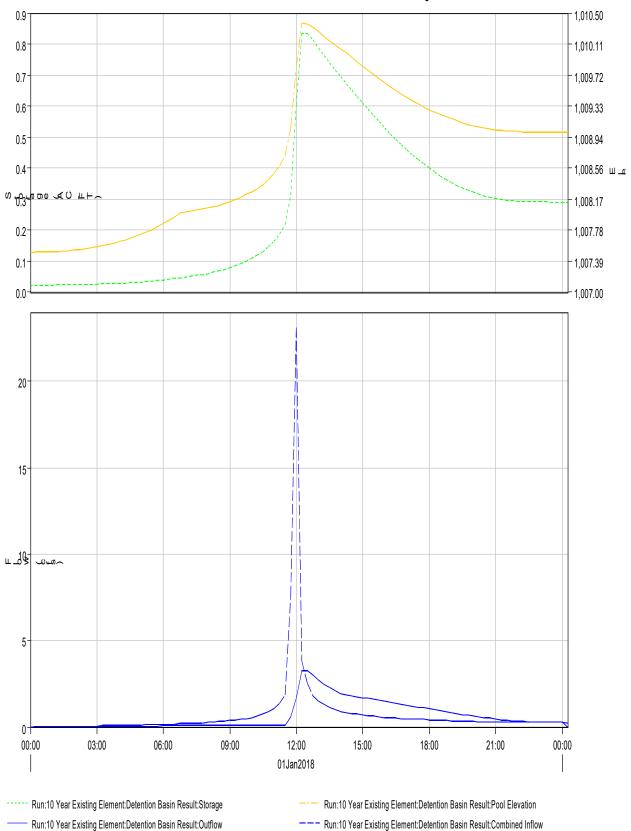
Basin Model:ProposedMeteorologic Model:1-YearControl Specifications:Control 1

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	00:00	0.0	0.0	1007.5	0.0
01Jan2018	00:15	0.0	0.0	1007.5	0.0
01Jan2018	00:30	0.0	0.0	1007.5	0.0
01Jan2018	00:45	0.0	0.0	1007.5	0.0
01Jan2018	01:00	0.0	0.0	1007.5	0.0
01Jan2018	01:15	0.0	0.0	1007.5	0.0
01Jan2018	01:30	0.0	0.0	1007.5	0.0
01Jan2018	01:45	0.0	0.0	1007.5	0.0
01Jan2018	02:00	0.0	0.0	1007.5	0.0
01Jan2018	02:15	0.0	0.0	1007.5	0.0
01Jan2018	02:30	0.0	0.0	1007.5	0.0
01Jan2018	02:45	0.0	0.0	1007.5	0.0
01Jan2018	03:00	0.0	0.0	1007.5	0.0
01Jan2018	03:15	0.0	0.0	1007.5	0.0
01Jan2018	03:30	0.0	0.0	1007.5	0.0
01Jan2018	03:45	0.0	0.0	1007.5	0.0
01Jan2018	04:00	0.0	0.0	1007.5	0.0
01Jan2018	04:15	0.0	0.0	1007.6	0.0
01Jan2018	04:30	0.0	0.0	1007.6	0.0
01Jan2018	04:45	0.0	0.0	1007.6	0.0
01Jan2018	05:00	0.1	0.0	1007.6	0.0
01Jan2018	05:15	0.1	0.0	1007.6	0.0
01Jan2018	05:30	0.1	0.0	1007.6	0.0
01Jan2018	05:45	0.1	0.0	1007.6	0.0
01Jan2018	06:00	0.1	0.0	1007.6	0.0
01Jan2018	06:15	0.1	0.0	1007.7	0.0

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	06:30	0.1	0.0	1007.7	0.0
01Jan2018	06:45	0.1	0.0	1007.7	0.0
01Jan2018	07:00	0.1	0.0	1007.7	0.0
01Jan2018	07:15	0.1	0.0	1007.7	0.1
01Jan2018	07:30	0.1	0.0	1007.8	0.1
01Jan2018	07:45	0.1	0.0	1007.8	0.1
01Jan2018	08:00	0.1	0.0	1007.8	0.1
01Jan2018	08:15	0.1	0.0	1007.8	0.1
01Jan2018	08:30	0.2	0.0	1007.9	0.1
01Jan2018	08:45	0.2	0.0	1007.9	0.1
01Jan2018	09:00	0.2	0.0	1008.0	0.1
01Jan2018	09:15	0.2	0.0	1008.0	0.1
01Jan2018	09:30	0.2	0.1	1008.0	0.1
01Jan2018	09:45	0.2	0.1	1008.0	0.1
01Jan2018	10:00	0.3	0.1	1008.1	0.1
01Jan2018	10:15	0.3	0.1	1008.1	0.1
01Jan2018	10:30	0.4	0.1	1008.1	0.1
01Jan2018	10:45	0.5	0.1	1008.1	0.1
01Jan2018	11:00	0.6	0.1	1008.2	0.1
01Jan2018	11:15	0.8	0.1	1008.2	0.1
01Jan2018	11:30	1.0	0.1	1008.3	0.1
01Jan2018	11:45	4.3	0.2	1008.5	0.1
01Jan2018	12:00	13.8	0.4	1009.2	0.8
01Jan2018	12:15	2.3	0.5	1009.5	1.4
01Jan2018	12:30	1.6	0.5	1009.6	1.4
01Jan2018	12:45	1.1	0.5	1009.6	1.4
01Jan2018	13:00	0.9	0.5	1009.5	1.4
01Jan2018	13:15	0.8	0.5	1009.5	1.4
01Jan2018	13:30	0.7	0.5	1009.5	1.3
01Jan2018	13:45	0.6	0.5	1009.4	1.3
01Jan2018	14:00	0.6	0.4	1009.4	1.2

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	14:15	0.5	0.4	1009.4	1.2
01Jan2018	14:30	0.5	0.4	1009.3	1.1
01Jan2018	14:45	0.5	0.4	1009.3	1.1
01Jan2018	15:00	0.4	0.4	1009.3	1.0
01Jan2018	15:15	0.4	0.4	1009.2	1.0
01Jan2018	15:30	0.4	0.4	1009.2	0.9
01Jan2018	15:45	0.4	0.4	1009.2	0.9
01Jan2018	16:00	0.3	0.4	1009.2	0.8
01Jan2018	16:15	0.3	0.3	1009.1	0.8
01Jan2018	16:30	0.3	0.3	1009.1	0.7
01Jan2018	16:45	0.3	0.3	1009.1	0.7
01Jan2018	17:00	0.3	0.3	1009.1	0.6
01Jan2018	17:15	0.3	0.3	1009.1	0.6
01Jan2018	17:30	0.3	0.3	1009.1	0.5
01Jan2018	17:45	0.3	0.3	1009.0	0.5
01Jan2018	18:00	0.3	0.3	1009.0	0.4
01Jan2018	18:15	0.3	0.3	1009.0	0.4
01Jan2018	18:30	0.2	0.3	1009.0	0.3
01Jan2018	18:45	0.2	0.3	1009.0	0.3
01Jan2018	19:00	0.2	0.3	1009.0	0.3
01Jan2018	19:15	0.2	0.3	1009.0	0.3
01Jan2018	19:30	0.2	0.3	1009.0	0.2
01Jan2018	19:45	0.2	0.3	1009.0	0.2
01Jan2018	20:00	0.2	0.3	1009.0	0.2
01Jan2018	20:15	0.2	0.3	1009.0	0.2
01Jan2018	20:30	0.2	0.3	1009.0	0.2
01Jan2018	20:45	0.2	0.3	1009.0	0.2
01Jan2018	21:00	0.2	0.3	1009.0	0.2
01Jan2018	21:15	0.2	0.3	1009.0	0.2
01Jan2018	21:30	0.2	0.3	1009.0	0.2
01Jan2018	21:45	0.2	0.3	1009.0	0.2

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	22:00	0.2	0.3	1009.0	0.2
01Jan2018	22:15	0.2	0.3	1009.0	0.2
01Jan2018	22:30	0.2	0.3	1009.0	0.2
01Jan2018	22:45	0.2	0.3	1009.0	0.2
01Jan2018	23:00	0.2	0.3	1009.0	0.2
01Jan2018	23:15	0.2	0.3	1009.0	0.2
01Jan2018	23:30	0.2	0.3	1009.0	0.2
01Jan2018	23:45	0.2	0.3	1009.0	0.2
02Jan2018	00:00	0.2	0.3	1009.0	0.2
02Jan2018	00:15	0.0	0.3	1009.0	0.1



Reservoir "Detention Basin" Results for Run "10 Year Existing"

Project: ELS Simulation Run: 10 Year Existing Reservoir: Detention Basin

Start of Run:01Jan2018, 00:00End of Run:02Jan2018, 00:15Compute Time:15Aug2018, 17:26:07

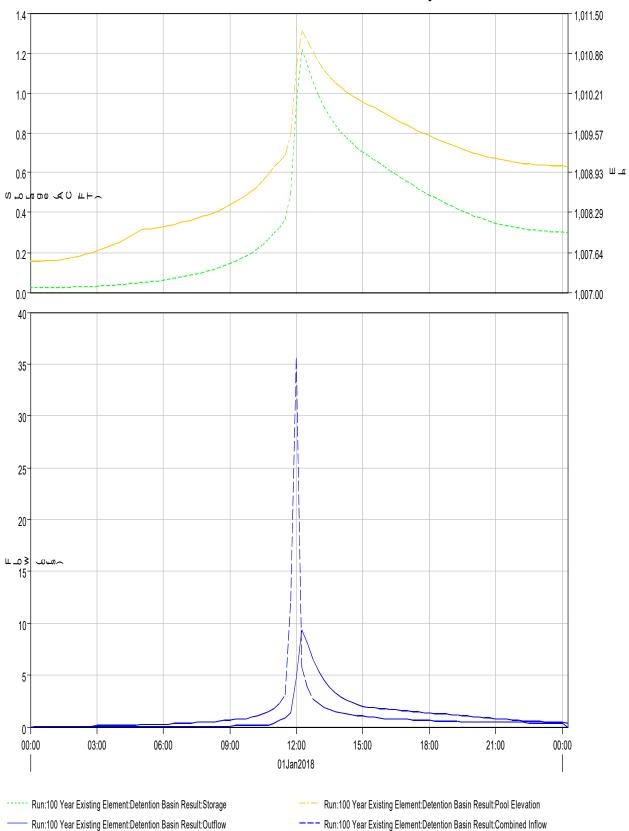
Basin Model:ProposedMeteorologic Model:10-YearControl Specifications:Control 1

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	00:00	0.0	0.0	1007.5	0.0
01Jan2018	00:15	0.0	0.0	1007.5	0.0
01Jan2018	00:30	0.0	0.0	1007.5	0.0
01Jan2018	00:45	0.0	0.0	1007.5	0.0
01Jan2018	01:00	0.0	0.0	1007.5	0.0
01Jan2018	01:15	0.0	0.0	1007.5	0.0
01Jan2018	01:30	0.0	0.0	1007.5	0.0
01Jan2018	01:45	0.0	0.0	1007.5	0.0
01Jan2018	02:00	0.0	0.0	1007.5	0.0
01Jan2018	02:15	0.0	0.0	1007.5	0.0
01Jan2018	02:30	0.0	0.0	1007.5	0.0
01Jan2018	02:45	0.1	0.0	1007.6	0.0
01Jan2018	03:00	0.1	0.0	1007.6	0.0
01Jan2018	03:15	0.1	0.0	1007.6	0.0
01Jan2018	03:30	0.1	0.0	1007.6	0.0
01Jan2018	03:45	0.1	0.0	1007.6	0.0
01Jan2018	04:00	0.1	0.0	1007.6	0.0
01Jan2018	04:15	0.1	0.0	1007.7	0.0
01Jan2018	04:30	0.1	0.0	1007.7	0.0
01Jan2018	04:45	0.1	0.0	1007.7	0.0
01Jan2018	05:00	0.1	0.0	1007.7	0.0
01Jan2018	05:15	0.1	0.0	1007.8	0.1
01Jan2018	05:30	0.1	0.0	1007.8	0.1
01Jan2018	05:45	0.1	0.0	1007.8	0.1
01Jan2018	06:00	0.2	0.0	1007.9	0.1
01Jan2018	06:15	0.2	0.0	1007.9	0.1

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	06:30	0.2	0.0	1007.9	0.1
01Jan2018	06:45	0.2	0.0	1008.0	0.1
01Jan2018	07:00	0.2	0.0	1008.0	0.1
01Jan2018	07:15	0.2	0.1	1008.0	0.1
01Jan2018	07:30	0.2	0.1	1008.0	0.1
01Jan2018	07:45	0.2	0.1	1008.0	0.1
01Jan2018	08:00	0.2	0.1	1008.1	0.1
01Jan2018	08:15	0.3	0.1	1008.1	0.1
01Jan2018	08:30	0.3	0.1	1008.1	0.1
01Jan2018	08:45	0.4	0.1	1008.1	0.1
01Jan2018	09:00	0.4	0.1	1008.1	0.1
01Jan2018	09:15	0.4	0.1	1008.2	0.1
01Jan2018	09:30	0.4	0.1	1008.2	0.1
01Jan2018	09:45	0.5	0.1	1008.2	0.1
01Jan2018	10:00	0.5	0.1	1008.3	0.1
01Jan2018	10:15	0.6	0.1	1008.3	0.1
01Jan2018	10:30	0.7	0.1	1008.4	0.1
01Jan2018	10:45	0.9	0.1	1008.4	0.1
01Jan2018	11:00	1.1	0.2	1008.5	0.1
01Jan2018	11:15	1.4	0.2	1008.6	0.1
01Jan2018	11:30	1.9	0.2	1008.7	0.1
01Jan2018	11:45	7.6	0.3	1009.1	0.6
01Jan2018	12:00	23.1	0.6	1009.8	1.7
01Jan2018	12:15	3.8	0.8	1010.4	3.3
01Jan2018	12:30	2.5	0.8	1010.4	3.2
01Jan2018	12:45	1.8	0.8	1010.3	3.0
01Jan2018	13:00	1.5	0.8	1010.3	2.7
01Jan2018	13:15	1.3	0.8	1010.2	2.5
01Jan2018	13:30	1.1	0.7	1010.2	2.3
01Jan2018	13:45	1.0	0.7	1010.1	2.1
01Jan2018	14:00	0.9	0.7	1010.0	1.9

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	14:15	0.8	0.7	1010.0	1.8
01Jan2018	14:30	0.8	0.7	1009.9	1.8
01Jan2018	14:45	0.7	0.6	1009.9	1.7
01Jan2018	15:00	0.7	0.6	1009.8	1.7
01Jan2018	15:15	0.7	0.6	1009.8	1.6
01Jan2018	15:30	0.6	0.6	1009.7	1.6
01Jan2018	15:45	0.6	0.5	1009.7	1.5
01Jan2018	16:00	0.5	0.5	1009.6	1.5
01Jan2018	16:15	0.5	0.5	1009.6	1.4
01Jan2018	16:30	0.5	0.5	1009.5	1.4
01Jan2018	16:45	0.5	0.5	1009.5	1.3
01Jan2018	17:00	0.5	0.5	1009.4	1.3
01Jan2018	17:15	0.5	0.4	1009.4	1.2
01Jan2018	17:30	0.4	0.4	1009.4	1.2
01Jan2018	17:45	0.4	0.4	1009.3	1.1
01Jan2018	18:00	0.4	0.4	1009.3	1.0
01Jan2018	18:15	0.4	0.4	1009.3	1.0
01Jan2018	18:30	0.4	0.4	1009.2	0.9
01Jan2018	18:45	0.4	0.4	1009.2	0.9
01Jan2018	19:00	0.4	0.4	1009.2	0.8
01Jan2018	19:15	0.3	0.3	1009.1	0.8
01Jan2018	19:30	0.3	0.3	1009.1	0.7
01Jan2018	19:45	0.3	0.3	1009.1	0.7
01Jan2018	20:00	0.3	0.3	1009.1	0.6
01Jan2018	20:15	0.3	0.3	1009.1	0.6
01Jan2018	20:30	0.3	0.3	1009.1	0.5
01Jan2018	20:45	0.3	0.3	1009.0	0.5
01Jan2018	21:00	0.3	0.3	1009.0	0.4
01Jan2018	21:15	0.3	0.3	1009.0	0.4
01Jan2018	21:30	0.3	0.3	1009.0	0.4
01Jan2018	21:45	0.3	0.3	1009.0	0.4

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	22:00	0.3	0.3	1009.0	0.3
01Jan2018	22:15	0.3	0.3	1009.0	0.3
01Jan2018	22:30	0.3	0.3	1009.0	0.3
01Jan2018	22:45	0.3	0.3	1009.0	0.3
01Jan2018	23:00	0.3	0.3	1009.0	0.3
01Jan2018	23:15	0.3	0.3	1009.0	0.3
01Jan2018	23:30	0.3	0.3	1009.0	0.3
01Jan2018	23:45	0.3	0.3	1009.0	0.3
02Jan2018	00:00	0.3	0.3	1009.0	0.3
02Jan2018	00:15	0.0	0.3	1009.0	0.2



Reservoir "Detention Basin" Results for Run "100 Year Existing"

Project: ELS Simulation Run: 100 Year Existing Reservoir: Detention Basin

Start of Run:01Jan2018, 00:00End of Run:02Jan2018, 00:15Compute Time:15Aug2018, 17:26:10

Basin Model:ProposedMeteorologic Model:100-YearControl Specifications:Control 1

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	00:00	0.0	0.0	1007.5	0.0
01Jan2018	00:15	0.0	0.0	1007.5	0.0
01Jan2018	00:30	0.0	0.0	1007.5	0.0
01Jan2018	00:45	0.0	0.0	1007.5	0.0
01Jan2018	01:00	0.0	0.0	1007.5	0.0
01Jan2018	01:15	0.0	0.0	1007.5	0.0
01Jan2018	01:30	0.0	0.0	1007.5	0.0
01Jan2018	01:45	0.1	0.0	1007.6	0.0
01Jan2018	02:00	0.1	0.0	1007.6	0.0
01Jan2018	02:15	0.1	0.0	1007.6	0.0
01Jan2018	02:30	0.1	0.0	1007.6	0.0
01Jan2018	02:45	0.1	0.0	1007.6	0.0
01Jan2018	03:00	0.1	0.0	1007.7	0.0
01Jan2018	03:15	0.1	0.0	1007.7	0.0
01Jan2018	03:30	0.1	0.0	1007.7	0.1
01Jan2018	03:45	0.1	0.0	1007.8	0.1
01Jan2018	04:00	0.2	0.0	1007.8	0.1
01Jan2018	04:15	0.2	0.0	1007.9	0.1
01Jan2018	04:30	0.2	0.0	1007.9	0.1
01Jan2018	04:45	0.2	0.0	1008.0	0.1
01Jan2018	05:00	0.2	0.0	1008.0	0.1
01Jan2018	05:15	0.2	0.1	1008.0	0.1
01Jan2018	05:30	0.2	0.1	1008.0	0.1
01Jan2018	05:45	0.3	0.1	1008.0	0.1
01Jan2018	06:00	0.3	0.1	1008.1	0.1
01Jan2018	06:15	0.3	0.1	1008.1	0.1

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	06:30	0.3	0.1	1008.1	0.1
01Jan2018	06:45	0.3	0.1	1008.1	0.1
01Jan2018	07:00	0.4	0.1	1008.1	0.1
01Jan2018	07:15	0.4	0.1	1008.2	0.1
01Jan2018	07:30	0.4	0.1	1008.2	0.1
01Jan2018	07:45	0.4	0.1	1008.2	0.1
01Jan2018	08:00	0.4	0.1	1008.2	0.1
01Jan2018	08:15	0.5	0.1	1008.3	0.1
01Jan2018	08:30	0.5	0.1	1008.3	0.1
01Jan2018	08:45	0.6	0.1	1008.4	0.1
01Jan2018	09:00	0.7	0.1	1008.4	0.1
01Jan2018	09:15	0.7	0.2	1008.5	0.1
01Jan2018	09:30	0.7	0.2	1008.5	0.1
01Jan2018	09:45	0.8	0.2	1008.6	0.1
01Jan2018	10:00	0.9	0.2	1008.6	0.1
01Jan2018	10:15	1.1	0.2	1008.7	0.1
01Jan2018	10:30	1.2	0.2	1008.8	0.1
01Jan2018	10:45	1.5	0.3	1008.9	0.1
01Jan2018	11:00	1.8	0.3	1009.0	0.4
01Jan2018	11:15	2.3	0.3	1009.1	0.7
01Jan2018	11:30	3.0	0.4	1009.2	0.9
01Jan2018	11:45	12.1	0.5	1009.5	1.4
01Jan2018	12:00	35.6	0.9	1010.6	4.7
01Jan2018	12:15	5.8	1.2	1011.2	9.4
01Jan2018	12:30	3.8	1.1	1011.1	8.0
01Jan2018	12:45	2.7	1.1	1010.9	6.6
01Jan2018	13:00	2.3	1.0	1010.7	5.4
01Jan2018	13:15	1.9	0.9	1010.6	4.5
01Jan2018	13:30	1.7	0.9	1010.5	3.8
01Jan2018	13:45	1.5	0.8	1010.4	3.3
01Jan2018	14:00	1.3	0.8	1010.3	2.9

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	14:15	1.2	0.8	1010.2	2.6
01Jan2018	14:30	1.1	0.7	1010.2	2.3
01Jan2018	14:45	1.1	0.7	1010.1	2.1
01Jan2018	15:00	1.0	0.7	1010.1	2.0
01Jan2018	15:15	1.0	0.7	1010.0	1.9
01Jan2018	15:30	0.9	0.7	1010.0	1.8
01Jan2018	15:45	0.9	0.6	1009.9	1.8
01Jan2018	16:00	0.8	0.6	1009.9	1.7
01Jan2018	16:15	0.8	0.6	1009.8	1.7
01Jan2018	16:30	0.8	0.6	1009.8	1.6
01Jan2018	16:45	0.7	0.6	1009.7	1.6
01Jan2018	17:00	0.7	0.6	1009.7	1.6
01Jan2018	17:15	0.7	0.5	1009.6	1.5
01Jan2018	17:30	0.7	0.5	1009.6	1.5
01Jan2018	17:45	0.6	0.5	1009.6	1.4
01Jan2018	18:00	0.6	0.5	1009.5	1.4
01Jan2018	18:15	0.6	0.5	1009.5	1.3
01Jan2018	18:30	0.6	0.5	1009.4	1.3
01Jan2018	18:45	0.6	0.4	1009.4	1.2
01Jan2018	19:00	0.5	0.4	1009.4	1.2
01Jan2018	19:15	0.5	0.4	1009.3	1.1
01Jan2018	19:30	0.5	0.4	1009.3	1.1
01Jan2018	19:45	0.5	0.4	1009.3	1.0
01Jan2018	20:00	0.5	0.4	1009.2	1.0
01Jan2018	20:15	0.4	0.4	1009.2	0.9
01Jan2018	20:30	0.4	0.4	1009.2	0.9
01Jan2018	20:45	0.4	0.4	1009.2	0.8
01Jan2018	21:00	0.4	0.3	1009.2	0.8
01Jan2018	21:15	0.4	0.3	1009.1	0.8
01Jan2018	21:30	0.4	0.3	1009.1	0.7
01Jan2018	21:45	0.4	0.3	1009.1	0.7

Date	Time	Inflow (CFS)	Storage (AC-FT)	Elevation (FT)	Outflow (CFS)
01Jan2018	22:00	0.4	0.3	1009.1	0.6
01Jan2018	22:15	0.4	0.3	1009.1	0.6
01Jan2018	22:30	0.4	0.3	1009.1	0.6
01Jan2018	22:45	0.4	0.3	1009.1	0.5
01Jan2018	23:00	0.4	0.3	1009.1	0.5
01Jan2018	23:15	0.4	0.3	1009.0	0.5
01Jan2018	23:30	0.4	0.3	1009.0	0.5
01Jan2018	23:45	0.4	0.3	1009.0	0.5
02Jan2018	00:00	0.4	0.3	1009.0	0.4
02Jan2018	00:15	0.0	0.3	1009.0	0.4