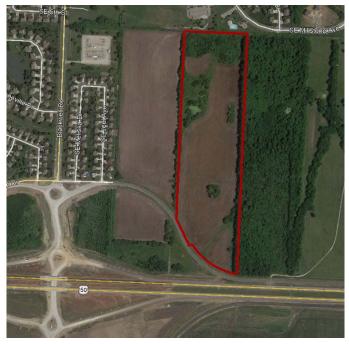


5/3/2018

City of Lee's Summit Development Services 220 SE Green Lee's Summit, MO 64063 816-980-1200

Re: Sanitary Sewer Impact
Case Properties
Artisan Point (Robbins Site)

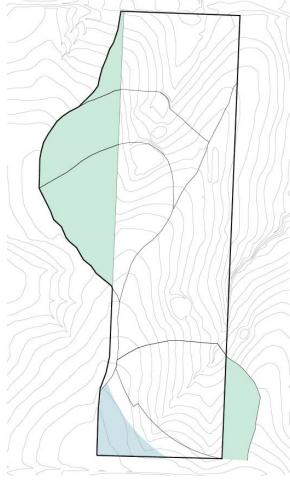
The purpose of this study is to evaluate the potential impact of the multi-family housing development proposed for a 35.36 Acre site located in the NW/4 of Section 11, T-47-N, R-31-W, within the jurisdiction of the City of Lee's Summit. The property is lies north of Shenandoah Drive approximately 1,600 feet east of Blackwell, within the upper reaches of the South Prairie Lee Watershed.



**Subject Property** 

The subject property, together with additional offsite areas, currently drains to the north east corner of the site. The downstream receiving sanitary sewer will be an 8-inch main located within the Summit Mill Addition. The system then flows 3,200 feet north through single-family developments to a pump station located on the north side of East Langford Road.





**Downstream Analysis** 

**Contributing Area** 

Sanitary sewer loads contributing to the Langford Road pump station may be broken down into:

- 83 Existing Homes in the Summit Mill Subdivision(s)
- Existing Highland Park Elementary School (500 Enrollment)
- Proposed 600 apartment units connecting at Summit Mill
- Probable Commercial development south of Shenandoah (600 seats, restaurant).
- 40 Probable single-family homes adjacent to the subject property.

The proposed multi-family development will consist of 600 dwelling units constructed in two phases. Each phase will have approximately equal numbers of one bedroom and two-bedroom units.

Case Properties owns and operates 44 apartment complexes over a 5-state region. Recent analysis of water usage in comparable projects identifies an average of 2,500 gallons per month per unit. 300 units generate a water demand of approximately 750,000 gallons per month.

A small area of commercial property may be served by sanitary sewer main extensions to the south side of Shenandoah. The largest load for the small triangular area would be due to restaurant development. We estimate that two 300-seat restaurants could be constructed on the east side of the ridge.

Approximately 11 acres west and southeast of the site may be served by sanitary sewer extensions connecting to the main lines constructed by the apartment development. Given a density of 3.7 du/acre, approximately 40 residential lots could contribute load to the existing system.

The following table itemizes the assumptions of future loads:

							Average			Peak	Peak
				Persons		Average	Daily Total	Peaking	Peak Flow	Flow	Flow
Area Label	Description		DU	per DU	Population	Total GPD	(GPD)	Factor	(GPD)	(GPM)	(CFS)
	Single Family Downstream		83	3.7	299.7	100	29,970	4	119,880	83	0.185
	Elmentary School			•	500	14	7,000	4	28,000	19	0.043

					Persons		Average	Average Daily Total		Peak Flow	Peak Flow	Peak Flow
Area Label	Description	Acres	DU/Ac	DU	per DU	Population		,	Factor			
	1 Bedroom	30.00	10	300	2	600	100	60,000	4	240,000	167	0.370
	2 Bedroom	30.00	10	300	3	900	100	90,000	4	360,000	250	0.556

Area Label	Description	Acres	Seats	GPD per Patron	Seatings/D	 Average Daily Total (GPD)	Peaking Factor	Peak Flow (GPD)	I	Peak Flow (CFS)
	Commercial Lot	1.66				, ,		, , ,		
	300 Seat Restaurant		300	6	8	14,400	4	57,600	40	0.089
	300 Seat Restaurant		300	6	8	14,400	4	57,600	40	0.089

								Average			Peak	Peak
					Persons		Average	Daily Total	Peaking	Peak Flow	Flow	Flow
Area Label	Description	Acres	DU/Ac	DU	per DU	Population	Total GPD	(GPD)	Factor	(GPD)	(GPM)	(CFS)
DA 7	North West Area	0.87	4	3.5	3.7	12.9	100	1,288	4	5,150	4	0.008
DA 5	West Central Area	3.19	4	12.8	3.7	47.2	100	4,721	4	18,885	13	0.029
DA 3	South West Area	4.60	4	18.4	3.7	68.1	100	6,808	4	27,232	19	0.042

Peak Totals 41,617	914,347	635	1.411
Branch Load to Pump Station	Non Pea	k CFS	0.353

## **Analysis**

The capacity of the existing sanitary sewer gravity mains affected by the proposed apartment project have been analyzed by developing a hydraulic profile extending from the Langford Road Pump Station for 5200 feet upstream to the uppermost commercial development area.

## Exhibit 1 is the profile of the existing system:

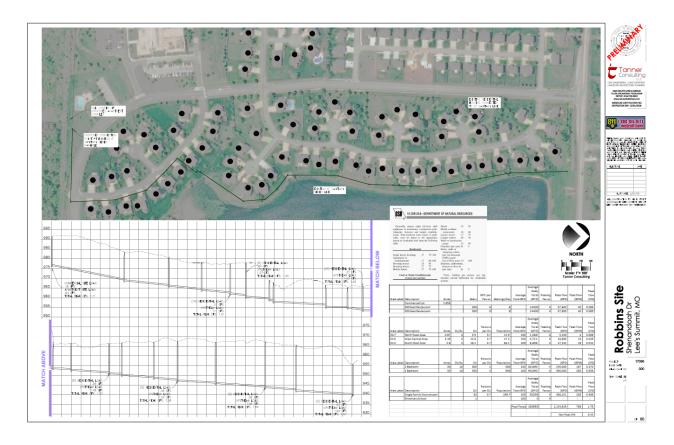
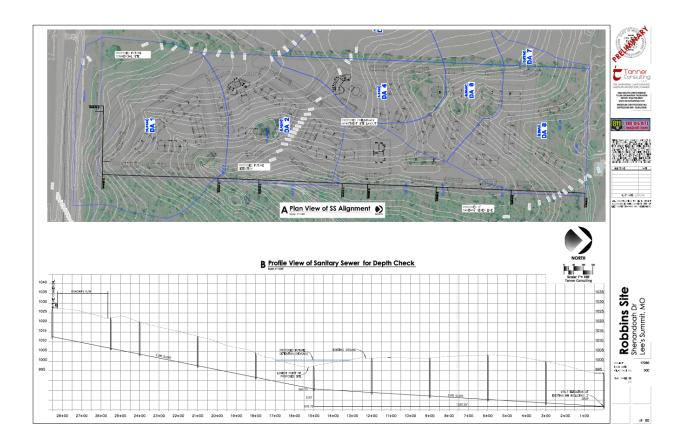
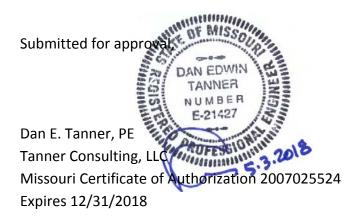


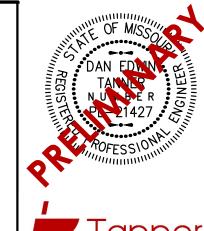
Exhibit 2 is the profile of the proposed system.



## **Summary of Findings**

The hydraulic capacity of an 8-inch PVC pipe laid on the minimum slope of 0.004 will convey the peak load of .353 CFS at half full. Applying a peaking factor of 4, an 8-inch pipe on minimum grade may surcharge up to 2.94 feet.







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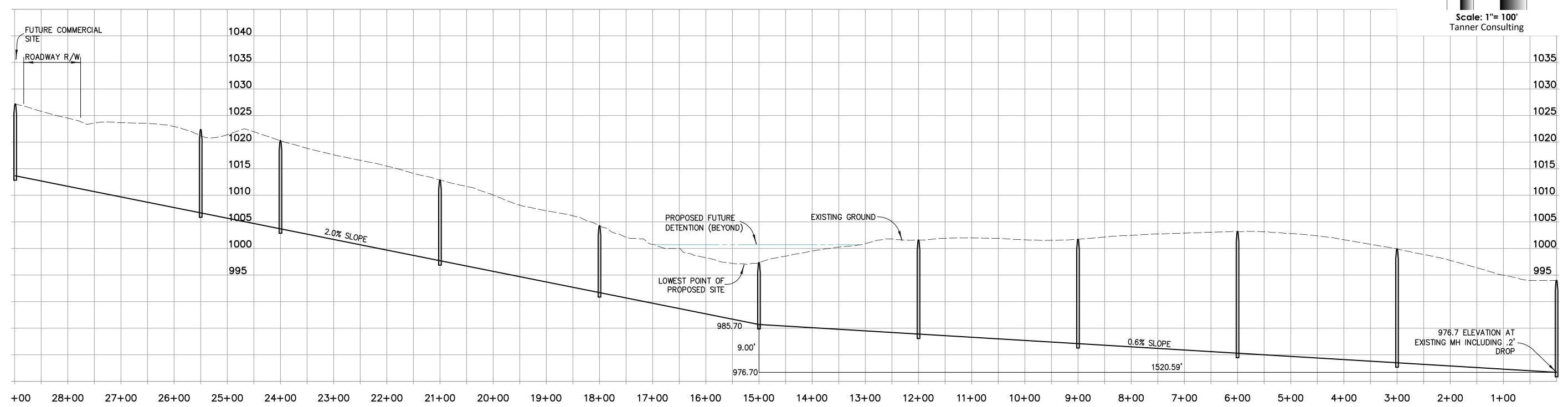
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B Profile View of Sanitary Sewer for Depth Check



(5) RESIDENTIAL LOAD: -

.03 CFS

UPSTREAM LOAD: 1.64 CFS TOTAL PEAK CFS: 1.67

1.52 CFS

TOTAL PEAK CFS: 1.58

.06 CFS

1.58 CFS

UPSTREAM LOAD:

TOTAL PEAK CFS: 1.64



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												0.353

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