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ENGINEERED BUILDING SYSTEMS

evisions to technical submissions which are n

CHAEL MOORES, MO Architect #2009032812

TENANT FINIS

McBaine Driv nit, Missouri (

2736 NE Lee's Summ

04.29.2

roject Number:

SUPPLY

BUTLER

Permit Submittal

Project Name and Address:

made or approved by the licensee are prohibited

BUTLER SUPPLY

PERMIT SUBMITTAL

APRIL 29, 2025

ACCESSIBILITY NOTES:

- 1. ACCESS TO THESE FACILITIES SHALL BE PROVIDED AT PRIMARY ENTRANCES, AS REQUIRED BY ADA.
- WALKS & SIDEWALKS SHALL HAVE A CONTINUOUS COMMON SURFACE NOT INTERRUPTED BY STEPS OR BY ABRUPT CHANGES IN LEVEL EXCEEDING 1/2" AND SHALL BE A MIN. OF 36" IN WIDTH.
- AT LEAST AS SLIP RESISTANT AS THAT DESCRIBED AS A MEDIUM

SURFACES WITH A SLOPE OF LESS THAN 6% GRADIENT SHALL BE

- 4. SURFACES WITH A SLOPE OF 6% GRADIENT OR GREATER SHALL BE SLIP RESISTANT.
- 5. SURFACE CROSS SLOPES SHALL NOT EXCEED 1/4" PER FOOT.
- WALKS, SIDEWALKS & PEDESTRIAN WAYS SHALL BE FREE OF GRATING WHENEVER POSSIBLE. FOR GRATINGS LOCATED IN THE SURFACE OF ANY OF THESE AREAS, GRID OPENINGS IN THE GRATINGS SHALL BE LIMITED TO 1/2" IN THE DIRECTION OF TRAFFIC FLOW.
- WHEN THE SLOPE IN THE DIRECTION OF TRAVEL OF ANY WALK EXCEEDS 1 VERTICAL TO 20 HORIZONTAL, IT SHALL COMPLY WITH THE PROVISIONS OF A PEDESTRIAN RAMP.
- ABRUPT CHANGES IN LEVEL ALONG ANY ACCESSIBLE ROUTE SHALL NOT EXCEED 1/2". WHEN CHANGES IN LEVEL DO OCCUR, THEY SHALL BE BEVELED WITH A SLOPE NO GREATER THAN 1:2. EXCEPT THAT LEVEL CHANGES NOT EXCEEDING 1/4" MAY BE VERTICAL. WHEN CHANGES IN LEVELS GREATER THAN 1/2" ARE NECESSARY, THEY SHALL COMPLY WITH THE REQUIREMENTS FOR CURB OR PEDESTRIAN RAMPS.
- 9. EVERY REQUIRED EXIT DOORWAY SHALL BE SIZED FOR A DOOF NOT LESS THAN 3 FT. WIDE BY NOT LESS THAN 6'-8" HIGH CAPABLE OF OPENING 90° AND MOUNTED SO THAT THE CLEAR WIDTH OF THE EXIT WAY IS 32" MIN.
- 10. THRESHOLDS MAY BE A MAX. 1/2" ABOVE ADJACENT FINISH
- 11. MAXIMUM EFFORT TO OPERATE DOORS SHALL NOT EXCEED 8 1/2 LBS. FOR EXTERIOR DOORS AND 5 LBS. FOR INTERIOR DOORS, SUCH PULL OR PUSH EFFORT BEING APPLIED AT RIGHT ANGLES TO HINGED DOORS AND AT THE CENTER PLANE OF SLIDING OR FOLDING DOORS. COMPENSATING DEVICES OR AUTOMATIC DOOR OPERATORS MAY BE UTILIZED TO MEET THE ABOVE STANDARDS. WHEN FIRE DOORS ARE REQUIRED, THE MAXIMUM EFFORT TO OPERATE THE DOOR MAY BE INCREASED TO THE MAXIMUM ALLOWABLE BY THE APPROPRIATE ADMINISTRATIVE AUTHORITY, NOT TO EXCEED 15 LBS.
- 12. THE BOTTOM 10" OF ALL DOORS, EXCEPT AUTOMATIC AND

SLIDING, SHALL HAVE A SMOOTH UNINTERRUPTED SURFACE.

- 13. PROVIDE LEVER-TYPE HARDWARE, PANIC BARS, PUSH PULL **ACTIVATING BARS OR OTHER HARDWARE** DESIGNED TO PROVIDE PASSAGE WITHOUT REQUIRING TIGHT GRASPING, TIGHT PINCHING, OR TWISTING OF THE WRIST TO OPERATE THE HARDWARE. (34" TO 48" A.F.F.)
- 14. PROVIDE 17" (MIN.) OR 18" (MAX.) FROM ADJACENT WALL TO CENTERLINE OF WATER CLOSET.
- 15. PROVIDE A 30"x48" CLEAR SPACE WITHIN THE TOILET ROOM THAT DOES NOT ENCROACH INTO THE DOOR SWING.
- 16. GRAB BARS LOCATED ON EACH SIDE, OR ONE SIDE AND THE BACK OF PHYSICALLY DISABLED TOILET COMPARTMENTS SHALL BE SECURELY ATTACHED 33" MIN. AND 36" MAX. FROM THE FINISHED FLOOR TO THE TOP OF THE GRAB BAR AND PARALLEL TO THE FLOOR. THE SPACE BETWEEN WALL-MOUNTED GRAB BARS AND THE WALL SHALL BE 1 1/2". GRAB BARS AT THE SIDE SHALL BE 42 LONG, AND THE BACK END SHALL BE LOCATED 12" FROM THE BACK WALL. GRAB BARS AT THE BACK SHALL BE NOT LESS THAN 36" LONG WITH THE END CLOSEST TO THE SIDE WALL MOUNTED 12" FROM THE CENTER OF THE WATER CLOSET. THE DIAMETER OR WIDTH OF THE GRIPPING SURFACES OF A GRAB BAR SHALL BE 1 1/4" TO 1 1/2" OR THE SHAPE SHALL PROVIDE AN EQUIVALENT GRIPPING SURFACE.
- 17. WATER CLOSET HEIGHT SHALL BE 17" (MIN.) OR 19" (MAX.) MEASURED TO THE TOP OF THE TOILET SEAT TO THE FINISHED FLOOR. CONTROLS SHALL BE OPERABLE WITH ONE HAND AND SHALL NOT REQUIRE TIGHT GRASPING, PINCHING, OR TWISTING OF THE WRIST, CONTROLS FOR FLUSH VALVES SHALL BE MOUNTED ON THE WIDE SIDE OF TOILET AREAS, NO MORE THAN 44" A.F.F. THE FORCE REQUIRED TO ACTIVATE CONTROLS SHALL BE NO GREATER THAN 5 LBS. OF
- 18. URINALS SHALL BE 17" (MAX.) ABOVE THE FLOOR AND PROJECT 13 1/2" FROM THE WALL. URINALS SHALL HAVE A CLEAR SPACE OF 30"X48" IN FRONT. FI USH VAI VES SHALL BE AUTOMATIC OR MOUNTED NO MORE THAN 44" A.F.F. IF HAND-OPERATED.
- 19 IN FRONT OF LAVATORIES PROVIDE A 30"x48" CLEAR SPACE LOCATED 25" (MAX.) FROM THE LEADING EDGE OF THE LAVATORY TOWARD THE MOUNTING WALL. KNEE CLEARANCE SHALL BE 11" DEEP (MIN.) AT 9" A F.F. AND 8" DEEP (MIN.) AT 27" A F.F. BETWEEN 9" AND 27" A.F.F., THE KNEE CLEARANCE SHALL BE PERMITTED TO REDUCE AT A RATE OF 1" IN DEPTH FOR EACH 6" IN HEIGHT.
- 20. ALL ACCESSIBLE LAVATORIES SHALL BE MOUNTED WITH THE RIM OR COUNTER SURFACE NO HIGHER THAN 34" A.F.F.
- 21. HOT WATER AND DRAIN PIPES UNDER LAVATORIES SHALL BE INSULATED OR OTHERWISE COVERED. THERE SHALL BE NO SHARP OR ABRASIVE SURFACES UNDER LAVATORIES.

H2 TYP. ADA TOILET DIMENSIONS

RESTROOM

INSTALL APPLICABLE

ADA SIGNAGE ON THE

WALL ADJACENT TO

THE LATCH SIDE OF

THE DOOR.

22. FAUCET CONTROLS AND OPERATING MECHANISMS SHALL BE OPERABLE WITH ONE HAND AND SHALL NOT REQUIRE TIGHT GRASPING. PINCHING. OR TWISTING OF THE WRIST. THE FORCE REQUIRED TO ACTIVATE CONTROLS SHALL BE NO GREATER THAN 5 LBS

CONTROLLED MECHANISMS ARE EXAMPLES OF ACCEPTABLE

DESIGNS. SELF-CLOSING VALVES ARE ALLOWED IF THE FAUCET IS OPEN FOR AT LEAST 10 SECONDS. 23. LOCATE PAPER TOWEL DISPENSERS, SOAP DISPENSERS, SANITARY NAPKIN DISPENSERS, AND WASTE RECEPTACLES WITH

ALL OPERABLE PARTS BETWEEN 15" AND 48" A.F.F.

LEVER-OPERATED, PUSH-TYPE, AND ELECTRONIC

- LOCATE TISSUE DISPENSERS ON THE WALL 7" (MIN.) AND 9" (MAX.) FROM THE FRONT EDGE OF THE TOILET SEAT TO THE CENTERLINE OF DISPENSER WITH THE OUTLET BETWEEN 15" AND
- . ACCESSIBLE RESTROOMS SHALL BE PROVIDED WITH SIGNAGE DESIGNED AND LOCATED PER SECTION 703 OF THE ADA DESIGN
- 26. DOORS IN ACCESSIBLE ROUTES SHALL BE DESIGNED TO MEET CLEARANCE REQUIREMENTS PER SECTION 404 OF THE ADA DESIGN GUIDELINES.
- 27. WALKS, HALLS, CORRIDORS, PASSAGEWAYS, AISLES OR OTHER CIRCULATION SPACES SHALL HAVE 80" MINIMUM CLEAR
- 28. OBJECTS PROJECTING FROM WALLS WITH THEIR LEADING EDGES BETWEEN 27" AND 80" ABOVE THE FINISH FLOOR SHALL PROTRUDE NO MORE THAN 4" INTO WALKS, HALLS, CORRIDORS. PASSAGEWAYS OR AISLES. OBJECTS MOUNTED AT OR BELOW 27' ABOVE FINISH FLOOR MAY PROTRUDE ANY AMOUNT.
- 29. OBJECTS THAT ARE BETWEEN 27" AND 80" A.F.F. AND MOUNTED ON POSTS MAY EXTEND BEYOND THE POSTS A MAXIMUM OF 12 OBJECTS MOUNTED BETWEEN POSTS, WHERE THE SPACE BETWEEN THE POSTS IS GREATER THAN 12", THE LOWEST EDGE OF THE OBJECT SHALL BE LOCATED 27" MAX. AND 80" MIN. A.F.F.
- 30. IF CARPET OR CARPET TILE IS USED ON A GROUND OR FLOOR SURFACE IN A COMMON USE AREA. IT SHALL HAVE FIRM BACKING OR NO BACKING. THE MAXIMUM PILE HEIGHT SHALL BE 1/2". EXPOSED EDGES OF CARPET SHALL BE FASTENED TO FLOOR SURFACES AND HAVE TRIM ALONG THE EXPOSED EDGE. AND TRIM SHALL COMPLY WITH THE REQUIREMENTS FOR CHANGES IN

& Suites Lee's.

CONSTRUCTION NOTES:

- PERFORM ALL WORK IN ACCORDANCE WITH ACCEPTABLE TRADE PRACTICE TO ENSURE THE HIGHEST QUALITY FINISHED PRODUCT -EXPRESSED OR IMPLIED PERFORM ALL WORK BY SKILLED MECHANICS. IN ACCORDANCE WITH ESTABLISHED STANDARDS OF WORKMANSHIP IN EACH OF THE VARIOUS TRADES.
- 2. WHEN THE PROJECT REQUIREMENTS REQUIRE THAT THE INSTALLATION OF WORK SHALL COMPLY WITH MANUFACTURER'S INSTRUCTIONS, PERFORM THE WORK IN STRICT ACCORDANCE WITH THE MOST CURRENT WRITTEN MANUFACTURER'S INSTRUCTIONS.
- 3. ALL PRODUCTS AND EQUIPMENT SHALL BE DELIVERED IN UNDAMAGED CONDITION AND STORED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS TO AVOID DISRUPTION OF THE WORK OR DAMAGE TO THE ITEMS. REPLACE DAMAGED OR UNFIT MATERIALS, AT NO COST TO
- 4. COORDINATE BLOCKING REQUIREMENTS WITH ADJACENT OR RELATED TRADES, ACCESSORIES, EQUIPMENT AND FIXTURES. INSTALL REQUIRED BLOCKING AT NO ADDITIONAL COST TO THE CONTRACT.
- . ALL WEATHER-EXPOSED SURFACES SHALL HAVE A WEATHER-RESISTIVE BARRIER. EXTERIOR OPENINGS SHALL BE FLASHED IN SUCH A MANNER AS TO MAKE THEM WATERPROOF.
- 6. REPAIR PROPERTY DAMAGE BY THE INSTALLERS TO A LIKE NEW CONDITION, OR REPLACE DAMAGED SURFACES AND MATERIALS OF THE PREVIOUSLY INSTALLED WORK BY OTHER TRADES, INSTALLERS, AND SUBCONTRACTORS.
- 7. ALLOWABLE TOLERANCES UNLESS OTHERWISE NOTED OR INDICATED, THE FOLLOWING TOLERANCES SHALL APPLY TO ALL WORK:
- g. ALL VERTICAL SURFACES SHALL BE PLUMB OR CONSTRUCTED TO THE EXACT SLOPES OR ANGLES INDICATED.
- h. ALL HORIZONTAL SURFACES SHALL BE LEVEL OR CONSTRUCTED TO THE EXACT ANGLE INDICATED OR INTENDED.
- i. WALL AND SOFFIT INTERSECTIONS SHALL BE 90° OR THE EXACT ANGLE INDICATED OR INTENDED. ALL CORNERS AND EDGES SHALL BE STRAIGHT AND TRUI
- WITHOUT DENTS, WAVES, BULGES OR OTHER BLEMISHES. k. ALL JOINTS SHALL BE TIGHT, STRAIGHT, EVEN, AND SMOOTH. I. ALL OPERABLE ITEMS SHALL OPERATE SMOOTHLY WITHOUT STICKING OR BINDING AND WITHOUT EXCESSIVE
- 8. THE CONTRACTOR SHALL NOTIFY THE OWNER WHEN THE WORK IS SUBSTANTIALLY COMPLETE AND READY FOR INSPECTION. UPON INSPECTION PROVIDE WRITTEN OPERATION AND MAINTENANCE INSTRUCTIONS AND GUARANTEES FOR ALL EQUIPMENT AND MATERIALS INSTALLED. PROVIDE WRITTEN GUARANTEES FOR A PERIOD OF ONE (1) YEAR FROM THE DATE OF FINAL ACCEPTANCE OF THE WORK.

GENERAL NOTES:

- THE CONTRACTOR SHALL SECURE AND PAY FOR GOVERNMENT LICENSES, INSPECTIONS, TESTING, TEMPORARY UTILITIES AND PERMITS AS REQUIRED BY THE CONSTRUCTION DOCUMENTS AND/OR REGULATORY BODY HAVING AUTHORITY.
- CONTRACTORS SHALL VISIT THE SITE WHILE BIDDING AND SHALL FAMILIARIZE THEMSELVES WITH EXISTING CONDITIONS AND THE REQUIREMENTS OF THE PROJECT AND CONSTRUCTION DOCUMENTS PRIOR TO DEVELOPING THEIR BID. FABRICATION / CONSTRUCTION. AND PURCHASING. MATERIAL QUANTITIES SHALL BE BASED ON ACTUAL FIELD CONDITIONS AND MEASUREMENTS. DO NOT RELY ON SCALING DRAWINGS FOR ACCURATE DIMENSIONS. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT OR OWNER'S REPRESENTATIVE OF ANY DISCREPANCIES, CONFLICTS OR OMISSIONS DISCOVERED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CORRECTIONS AND/OR REPAIRS REQUIRED FOR FAILING TO DO SO.
- 3. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE TO PROVIDE ALL CONSTRUCTION DOCUMENTS TO THEIR SUBCONTRACTORS AS REQUIRED FOR THEM TO DEVELOP A COMPLETE BID FOR THEIR WORK AND TO HAVE A COMPLETE UNDERSTANDING OF COORDINATION NEEDED WITH OTHER SUBCONTRACTORS FOR RELATED HIDDEN OR EXPOSED WORK TO ENSURE EFFICIENT AND ORDERLY INSTALLATION.
- 4. THE ARCHITECT ASSUMES NO LIABILITY FOR THE SERVICES AND/OR CONSTRUCTION DOCUMENTS OF DESIGN SUB-CONSULTANTS COMPILED INTO THE SET OF DOCUMENTS ISSUED BY THE ARCHITECT. THESE DESIGN SERVICES MAY INCLUDE. BUT ARE NOT LIMITED TO. CIVIL LANDSCAPE, STRUCTURAL, MECHANICAL, PLUMBING, ELECTRICAL PRE-ENGINEERED METAL BUILDING DESIGN, TILT-UP DESIGN, TRUSS SYSTEM DESIGN, AUTOMATIC FIRE SPRINKLER AND/OR ALARM SYSTEMS, LOW-VOLTAGE ELECTRICAL TELECOMMUNICATION AND SECURITY SYSTEMS AND GUTTER / DOWNSPOUT DESIGN.
- UNLESS SPECIFICALLY NOTED OTHERWISE, THE CONTRACTOR SHALL PROVIDE AND PAY FOR LABOR, MATERIALS, FOUIPMENT, MACHINERY SCAFFOLDING, SHORING, TOOLS, LAYOUT, ON-SITE DIMENSIONING, TRANSPORTATION, UTILITIES, AND OTHER FACILITIES AND SERVICES NECESSARY FOR PROPER EXECUTION AND COMPLETION OF THE WO AS REQUIRED BY THE CONSTRUCTION CONTRACT DOCUMENTS. THIS SHALL ALSO INCLUDE NECESSARY CUTTING, PATCHING AND REPAIRING OF EXISTING CONSTRUCTION MATERIALS IN PLACE. ALL WORK AND MATERIAL SHALL COMPLY WITH THE APPLICABLE GOVERNING CODES
- 6. WHERE DETAILS AND DESIGN INTENT ARE NOT CLEAR, THE CONTRACTOR SHALL CONSULT THE ARCHITECT FOR CLARIFICATION PRIOR TO PROCEEDING WITH THE WORK.
- 7. THE CONTRACTOR SHALL DESIGN AND INSTALL ADEQUATE SHORING AND BRACING FOR STRUCTURAL MODIFICATIONS, INSTALLATIONS AND
- 8. CONTRACTORS SHALL TAKE CARE TO PROTECT ADJACENT AREAS FROM DUST AND DAMAGE DURING THE CONSTRUCTION PROCESS AND SHALL CLEAN UP AFTER THEMSELVES AT THE END OF EACH WORKING DAY. ANY DAMAGE DONE TO ADJACENT AREAS MUST BE REPAIRED TO MATCH ORIGINAL CONDITIONS OR TO THE OWNER'S SATISFACTION. REPAIRS ARE TO BE PAID FOR BY THE CONTRACTOR RESPONSIBLE.
- 9. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY ADDITIONAL WORK OR REVISIONS REQUIRED DUE TO SITE CONDITIONS OR ADDITIONAL REQUIREMENTS OF ANY REGULATORY BODIES HAVING
- 10. FOR THE DURATION OF THE PROJECT AND AT ALL TIMES OF EACH DAY THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR JOB SITE CONDITIONS, SECURITY AND SAFETY FOR WORKERS AND THE GENERAL PUBLIC, AS REQUIRED BY THE REGULATORY BODY HAVING AUTHORITY.
- 11. THE GENERAL CONTRACTOR SHALL PURCHASE AND MAINTAIN INSURANCE COVERAGE IN ACCORDANCE WITH THE REQUIREMENTS OF THE OWNER. VERIFY AND COORDINATE WITH THE OWNER'S REPRESENTATIVE FOR ANY ADDITIONAL REQUIREMENTS.
- 12. THE OWNER OR THE OWNER'S SUBCONTRACTORS MAY OCCUPY PORTIONS OF THE PROJECT DURING THE FINAL STAGE OF CONSTRUCTION. COORDINATE AND COOPERATE WITH THE OWNER TO MINIMIZE CONFLICT AND FACILITATE THE OWNER'S OPERATION.
- 13. THE CONTRACTOR SHALL PROVIDE SECURITY OF THE WORK, INCLUDING TOOLS AND UNINSTALLED MATERIALS. PROTECT THE WORK, STORED PRODUCTS, CONSTRUCTION EQUIPMENT, AND OWNER'S PROPERTY FROM THEFT AND VANDALISM, AND PROTECT THE PREMISES FROM ENTRY BY UNAUTHORIZED PERSONNEL UNTIL FINAL ACCEPTANCE BY
- 14. CONTRACTOR SHALL COORDINATE STAGING AREAS AS REQUIRED BY
- 15. THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF ALL EXISTING UTILITIES.
- 16. THE STRUCTURAL ENGINEER AND ARCHITECT MUST BE NOTIFIED AND MUST GIVE APPROVAL PRIOR TO ANY STRUCTURAL MEMBER(S) BEING CUT OR MODIFIED TO ACCOMMODATE THE INSTALLATION OF ANY PIPES, DUCTS OR OTHER CONSTRUCTION.
- 17. THE STRUCTURAL ENGINEER AND ARCHITECT MUST BE NOTIFIED AND MUST GIVE APPROVAL PRIOR TO ANY MODIFICATION TO THE ROOF SYSTEM OR ADDING ANY ADDITIONAL ROOF-MOUNTED EQUIPMENT.

DISCLAIMER:

THESE DRAWINGS ARE CONSIDERED A "BUILDER'S SET" AND BY BEGINNING CONSTRUCTION, THE CONTRACTOR GUARANTEES TO THE ARCHITECT, THAT THE CONTRACTOR HAS THE COMPETENCE AND SKILL IN CONSTRUCTION NECESSARY TO BUILD THE PROJECT WITH THESE DRAWINGS. THE CONTRACTOR WILL BE REQUIRED TO ADAPT THE DRAWINGS TO ACTUAL FIELD CONDITIONS AND MAKE LOGICAL ADJUSTMENTS IN FIT, FORM, DIMENSION AND QUANTITY. IN THE EVENT ADDITIONAL DETAIL OR GUIDANCE IS NEEDED. THE CONTRACTOR, SHALL IMMEDIATELY NOTIFY THE ARCHITECT. FAILURE TO GIVE NOTICE SHALL RELIEVE THE ARCHITECT OF RESPONSIBILITY FOR ANY RESULTANT EXPENSES, REPAIRS OR ADDITIONAL WORK. IT IS UNDERSTOOD AND AGREED THAT IF THE ARCHITECT IS NOT HIRED TO DO CONSTRUCTION OBSERVATION OR ANY OTHER CONSTRUCTION PHASE SERVICES, THAT THE ENTITY HIRED TO PERFORM SUCH SERVICES ASSUMES ALL RESPONSIBILITY FOR THESE SERVICES, AND THE CLIENT WAIVES ANY CLAIMS AGAINST THE ARCHITECT THAT MAY BE IN ANY WAY CONNECTED THERETO.

ABBREVIATIONS:

*NOTE: THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY ABBREVIATIONS

@ ACT	AT ACOUSTIC CEILING TILE	JT	JOINT
ADJ	ADJUSTABLE	KS	KNEE SPACE
AFF ALUM	ABOVE FINISHED FLOOR ALUMINUM	L	LONG
ANOD	ANODIZED	LB (#)	POUND
ATT	ATTENUATION	LVL	LAMINATED VENEER LUMBER
BD	BOARD	MAX	MAXIMUM
BET	BETWEEN	MDO	MEDIUM DENSITY OVERLAY
3F	BARRIER FREE	MECH	MECHANICAL
BIT	BITUMINOUS	MFR	MANUFACTURER
BLDG	BUILDING	MICRO	MICROWAVE
30	BOTTOM OF	MIN	MINIMUM
3TM	BOTTOM	MO	MASONRY OPENING
ODT	OADDET	MR	MOISTURE RESISTANT
CPT	CARPET	MTD	MOUNTED
CT CJ	CERAMIC TILE	MTL	METAL
SJ CL	CONTROL JOINT CENTER LINE	NIC	NOT IN CONTRACT
OL CLG	CEILING	NO	NUMBER
CLR	CLEAR	NOM	NOMINAL
CMU	CONCRETE MASONRY UNIT	NOW	NOMINAL
COMP	COMPRESSIBLE	O.C.	ON CENTER
CONC	CONCRETE	O.D.	OUTSIDE DIAMETER
ONT	CONTINUOUS	O.H.	OVERHEAD or OPPOSITE HAND
<i>/</i> O1 1 1	00111110000	OSB	ORIENTED STRAND BOARD
)	DRYER	OZ	OUNCE
EG	DEGREE	5-	· · · -
EMO	DEMOLITION	PREFAB	PREFABRICATED
)F	DRINKING FOUNTAIN	PLAM	_
DΗ	DOUBLE-HUNG	PLYWD	PLYWOOD
OIA	DIAMETER	PR	PAIR
ON	DOWN	PT	PRESSURE TREATED
OP	DEEP	PNT	PAINT
DS	DOWN SPOUT	PEMB	PRE-ENGINEERED MTL BLDG
OW	DISHWASHER		
		QTY	QUANTITY
ĒΑ	EACH		
ΞJ	EXPANSION JOINT	R	RISER
EQ.	EQUAL	RCP	REFLECTED CEILING PLAN
ETR	EXISTING TO REMAIN	REF	REFRIGERATOR, REFERENCE
XG	EXISTING	REINF	REINFORCED
EXP	EXPOSED TO STRUCTURE	REQD	REQUIRED

SIMILAR SQUARE STAINLESS STEEL STAIN TO BE DETERMINED TOP OF TYPICAL

UNLESS NOTED OTHERWISE

ROUGH OPENING

RUBBER COVE BASE

SEALED CONCRETE

SQUARE FEET

RCB

VINYL COMPOSITION TILE VERTICAL WASHER, WIDE WITH WOOD WATER HEATER

HOLLOW METAL HOUR

WALK-IN CLOSET WELDED WIRE FABRIC

INSUL INSULATION

FLOOR DRAIN

FINISHED FLOOR

FIRE RETARDANT

FIELD VERIFY

GALVANIZED

INTERRUPTER

GYPSUM BOARD

HOSE BIB

HARDWARE

HEIGHT

HRDWD HARDWOOD

FLOOR

FRP

GALV

HM

FURNISH AND INSTALL

GENERAL CONTRACTOR

GROUND FAULT CIRCUIT

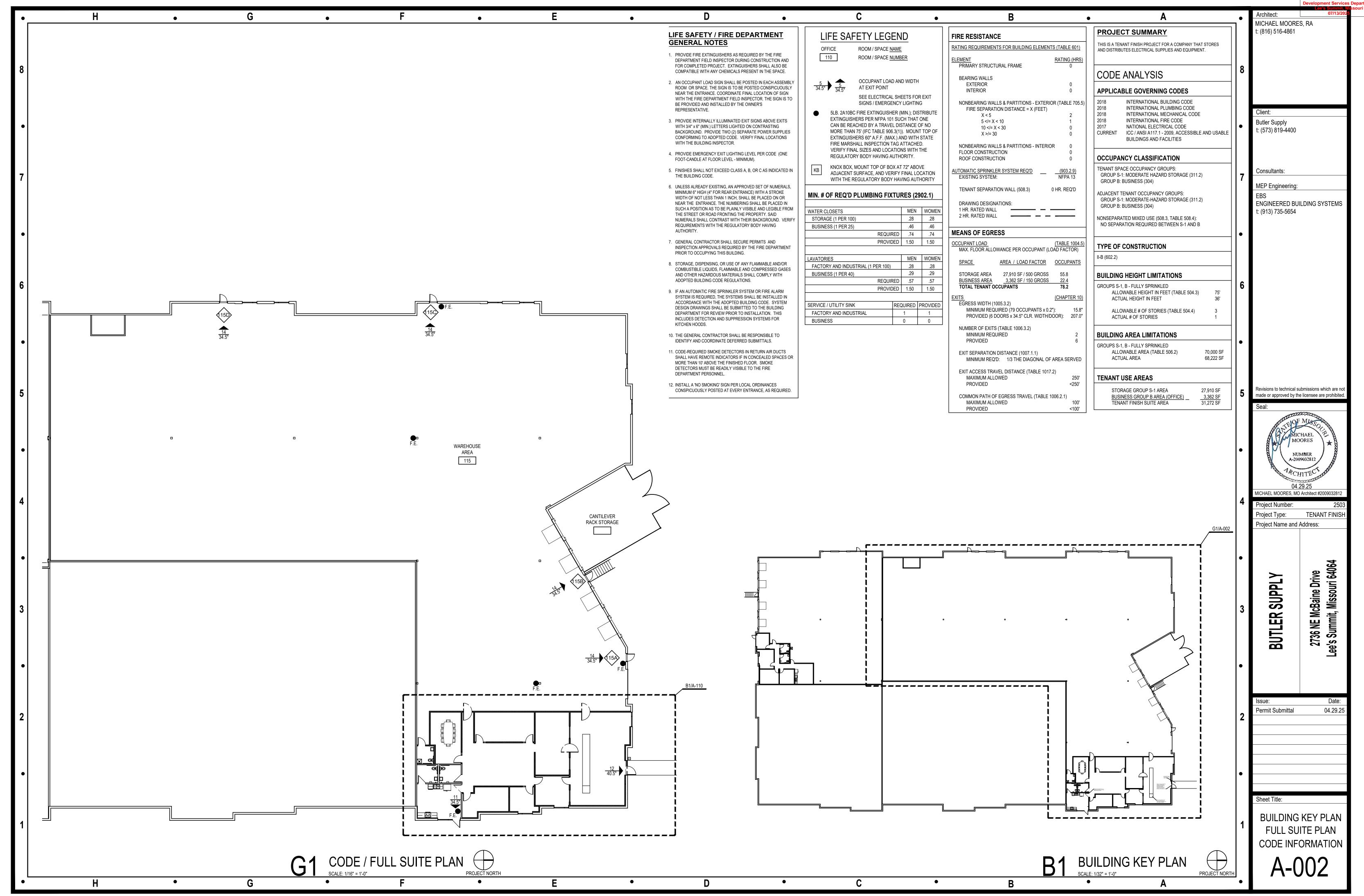
FIRE EXTINGUISHER, FINISHED

FIBER-REINFORCED PLASTIC

Sheet Title: COVER / MAP **GENERAL NOTES** ADA NOTES

Suites By Marriott.

RELEASED FOR CONSTRUCTION As Noted on Plans Review



RCB

2. PAINT AND INSTALL RCB AT THE OUTSIDE WALLS OF THE OFFICE.

115 WAREHOUSE AREA

COMMENTS

R.T. 2x4 SUB FRAME - AS REQUIRED -

HOLLOW METAL FRAME DETAILS

PNT

1. WALLS WITHIN 2' OF SERVICE SINKS, URINALS, AND WATER CLOSETS MUST BE PAINTED WITH EPOXY-BASED PAINT UP TO A MINIMUM OF 4' A.F.F.

D

EXP

1, 2.

EXIT DOORS SHALL SWING IN THE DIRECTION OF EXIT TRAVEL WHEN SERVING 50 OR MORE

COORDINATE ALL DOOR HARDWARE WITH THE

FUNCTIONS, MODELS, AND KEYING SYSTEMS MEET

OWNER TO ENSURE THE MANUFACTURER,

THE OWNER'S STANDARD REQUIREMENTS.

Permit Submittal

Sheet Title:

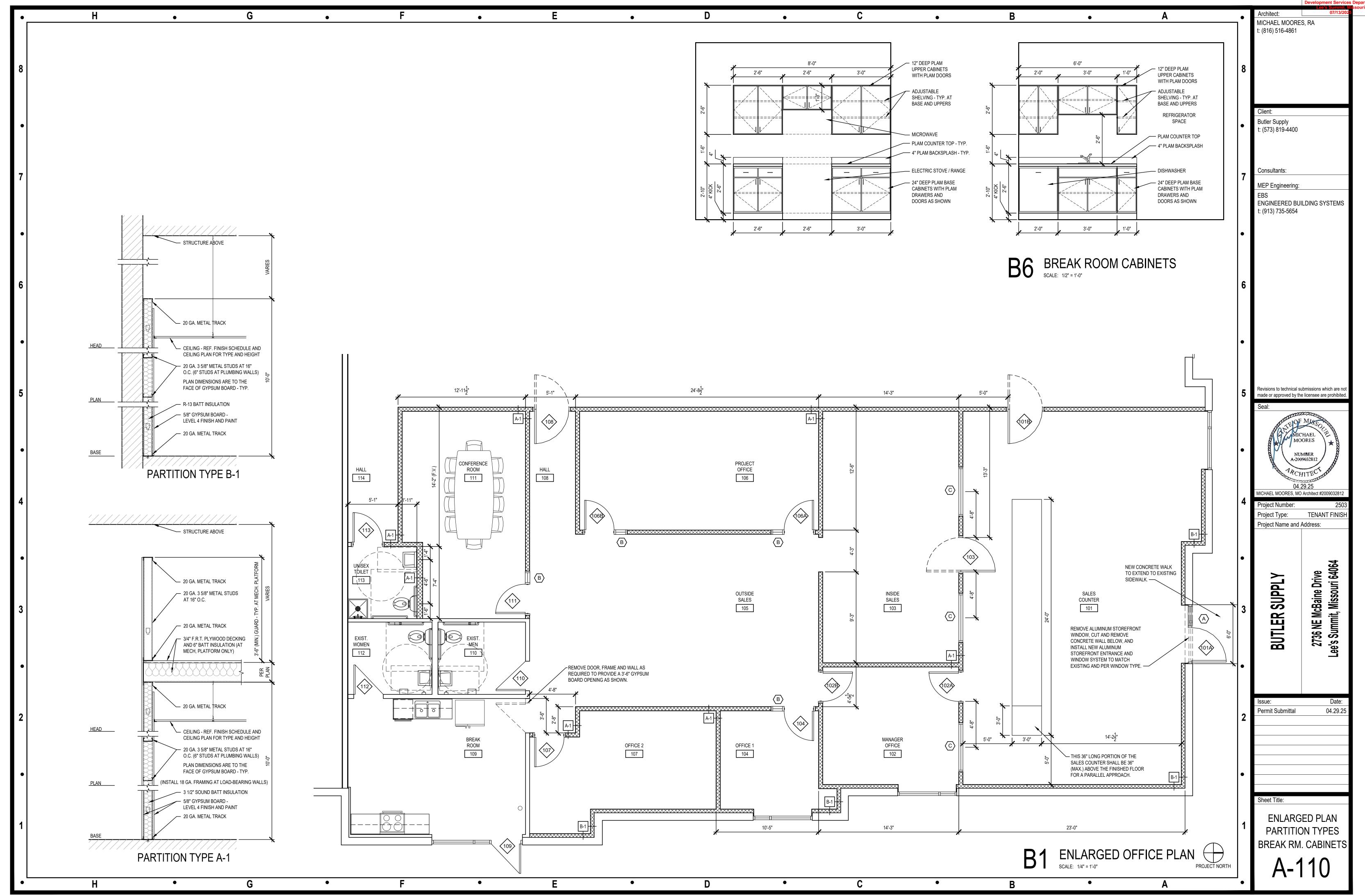
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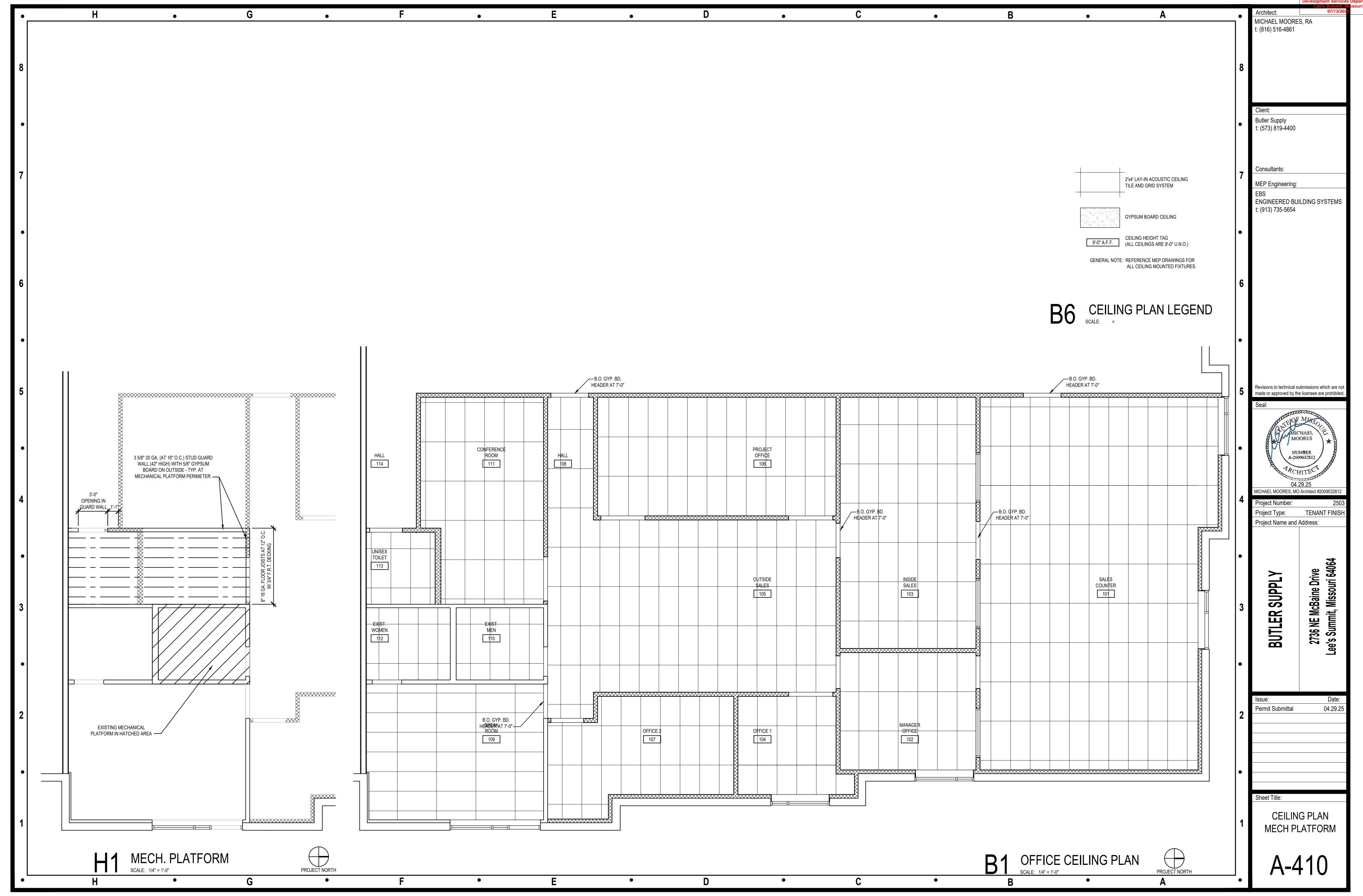
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AND NOTES

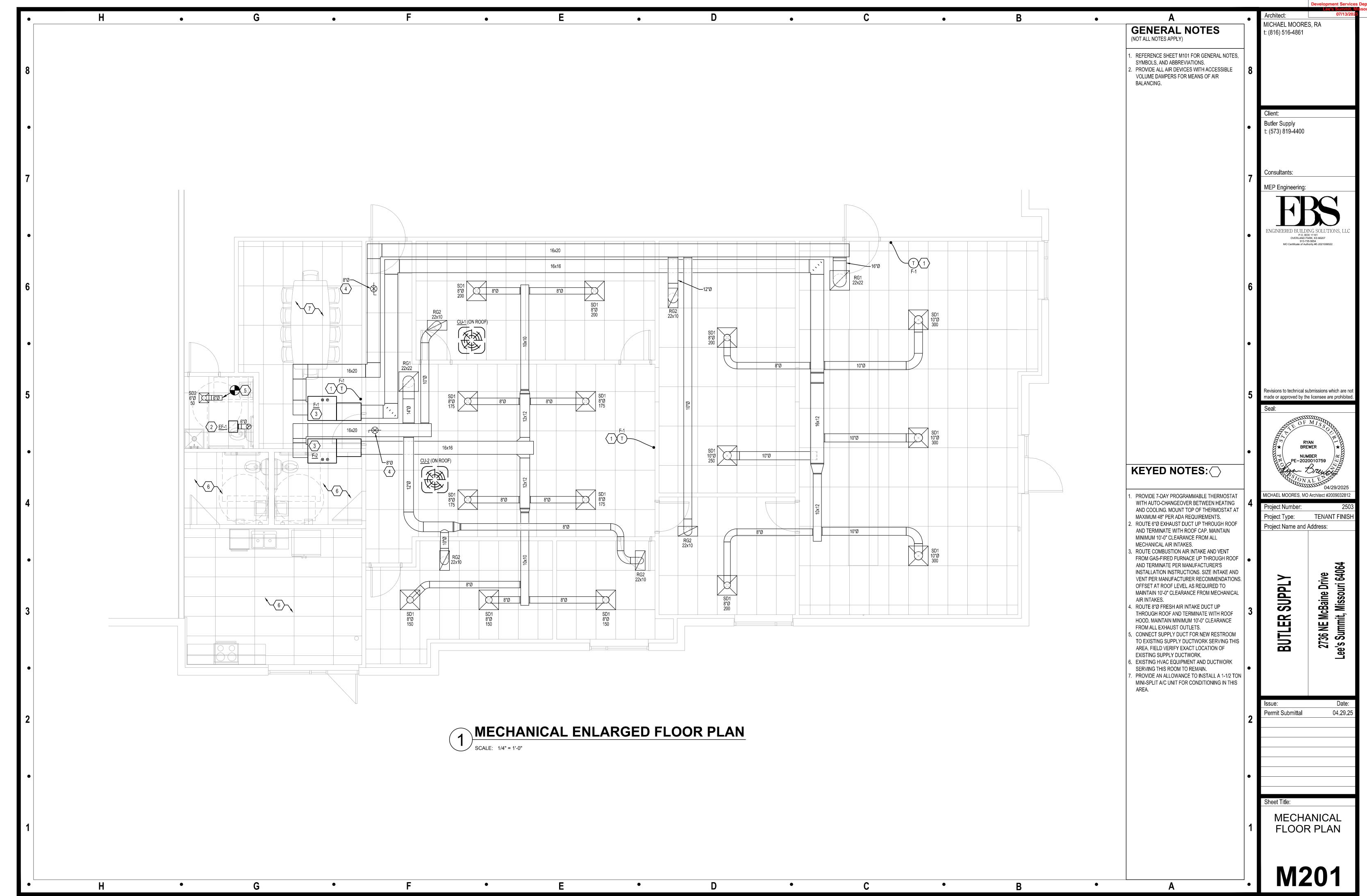
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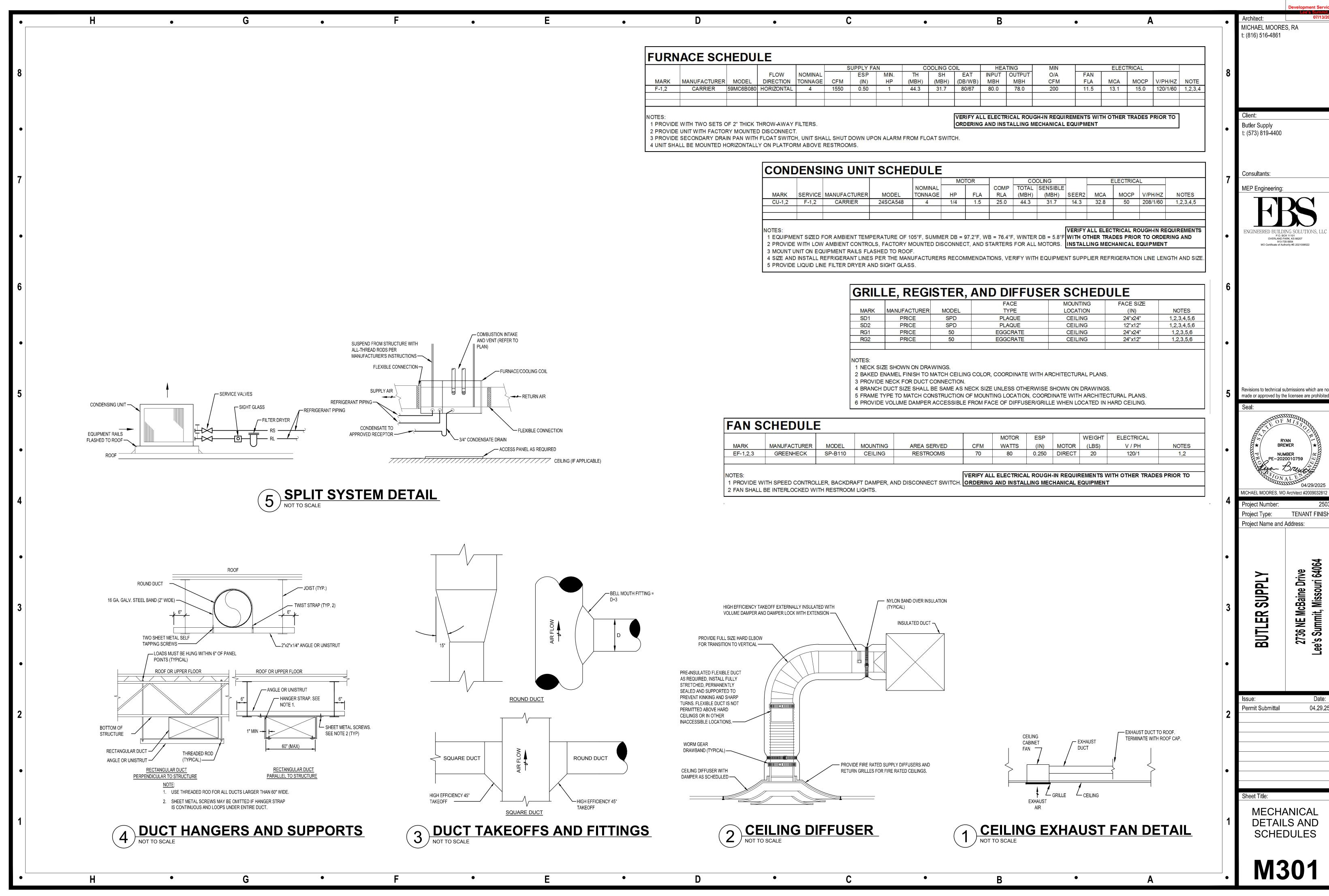
OCCUPANTS.

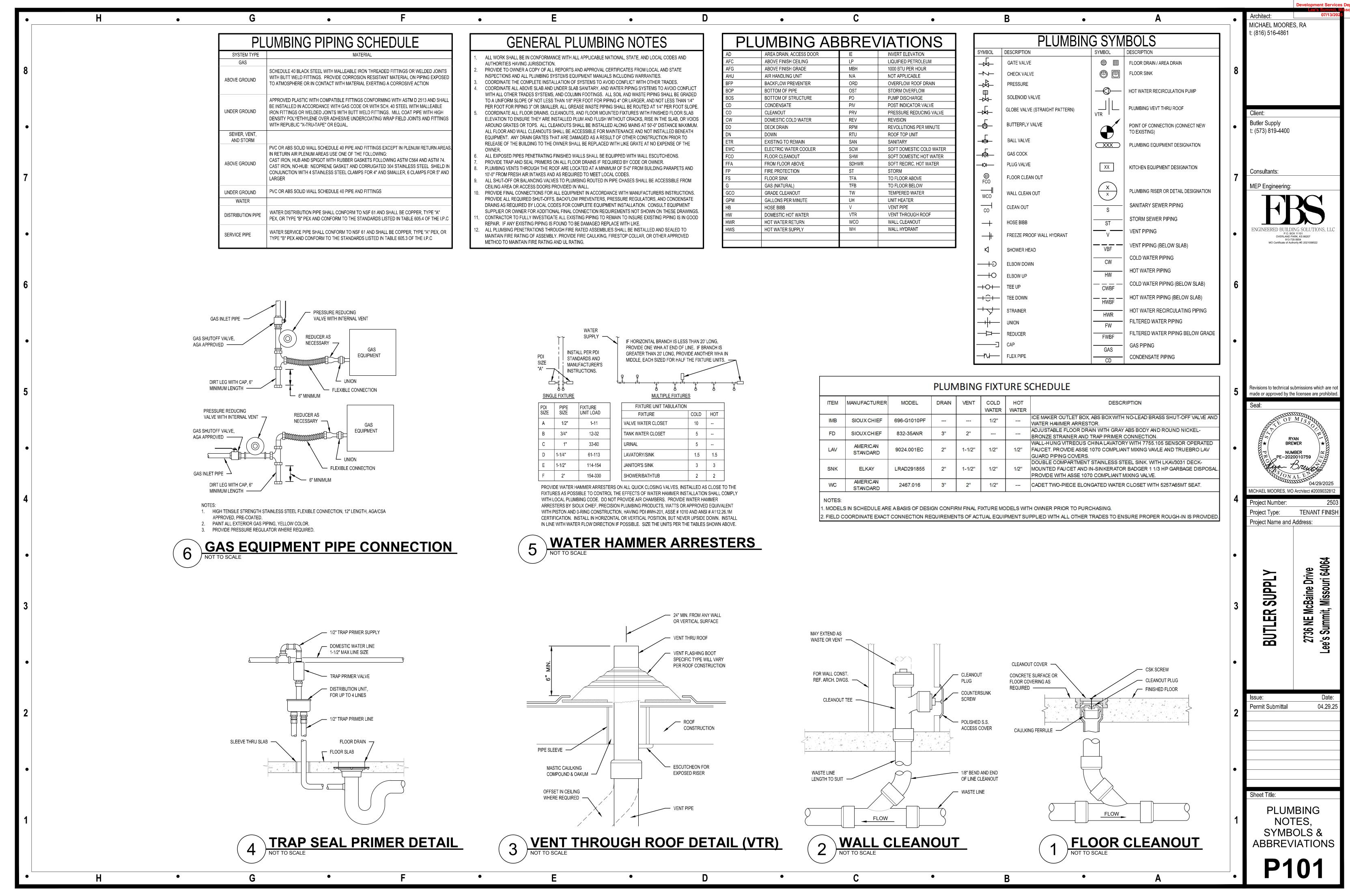


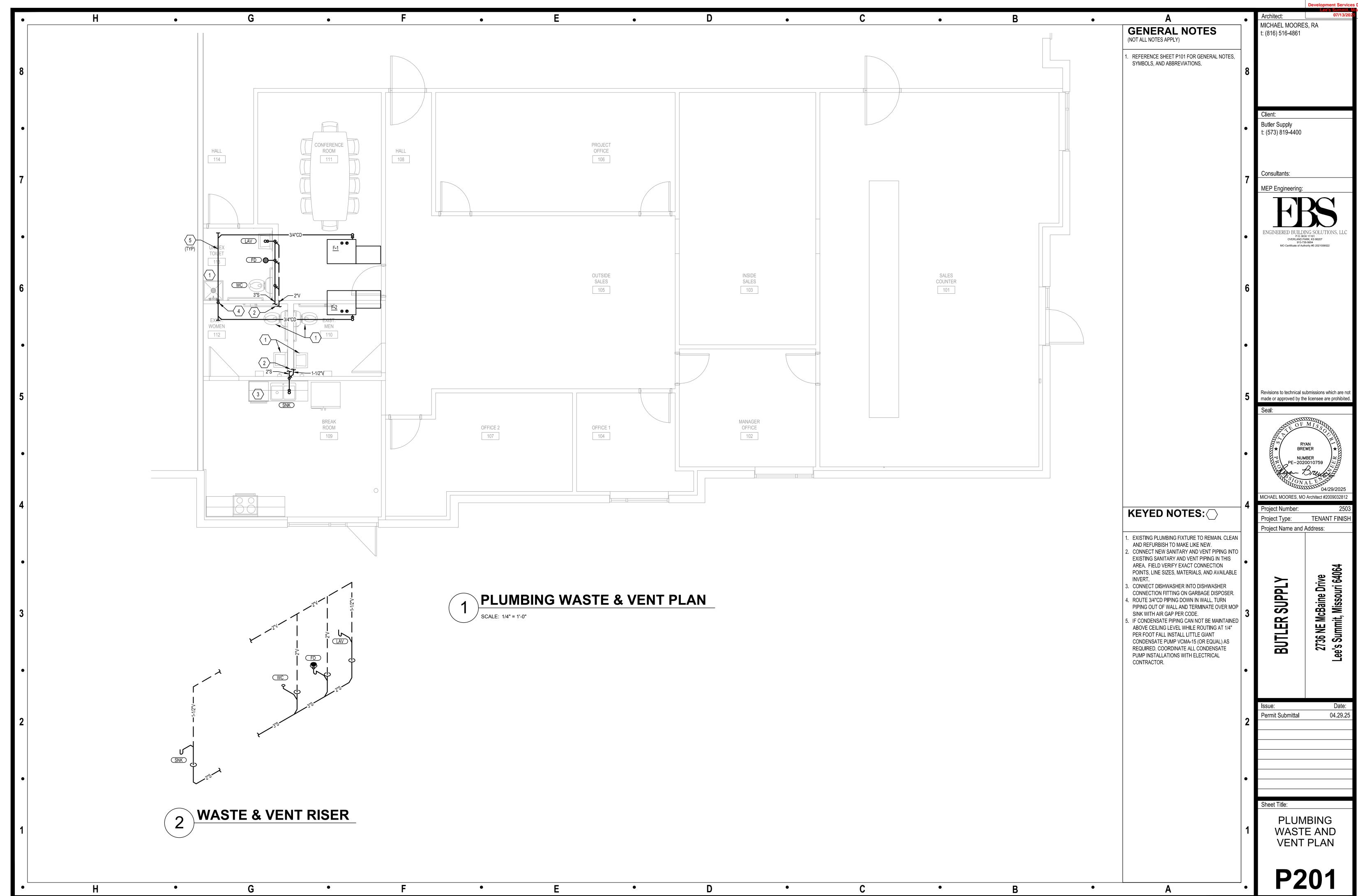


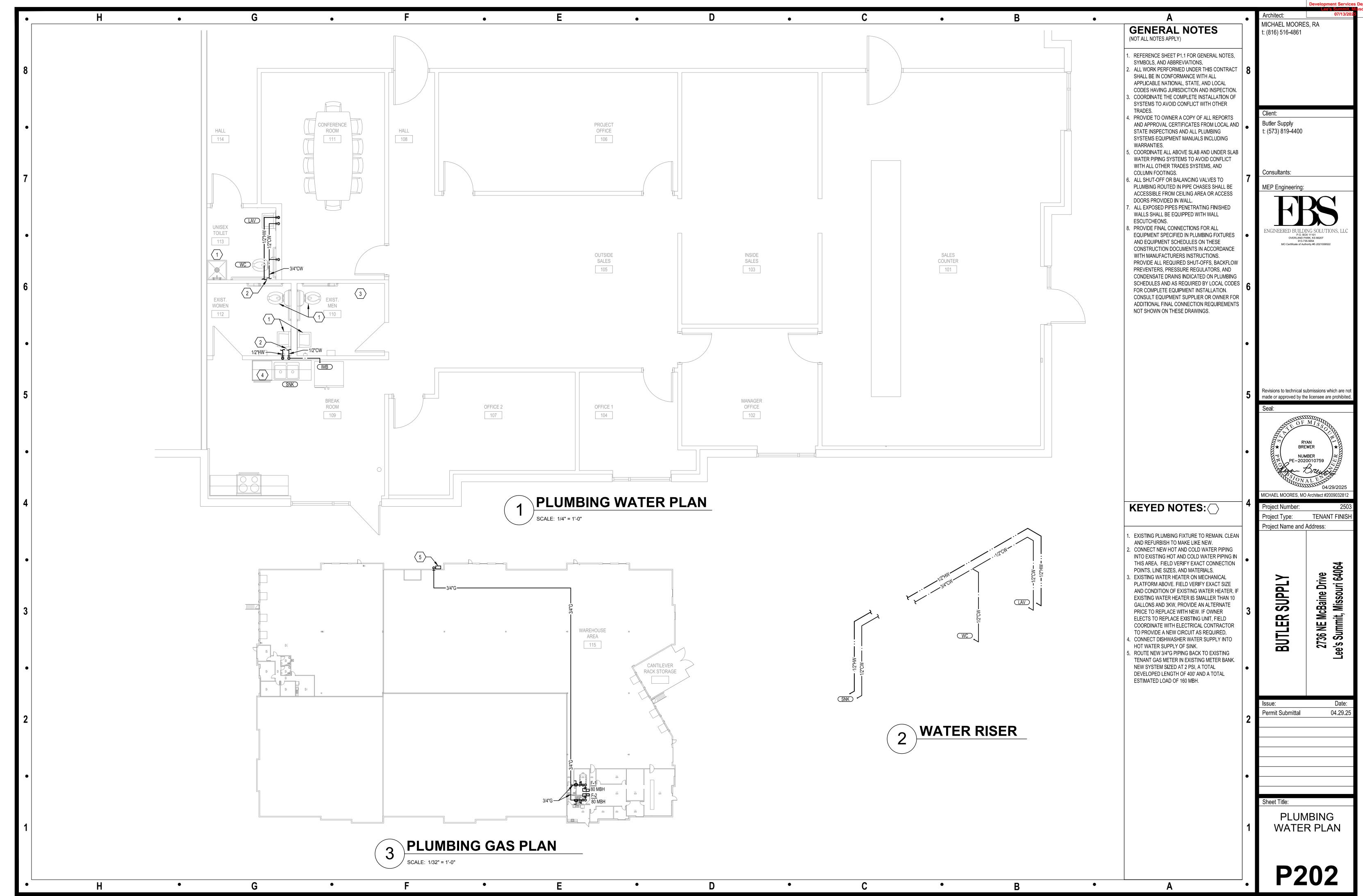
Architect:





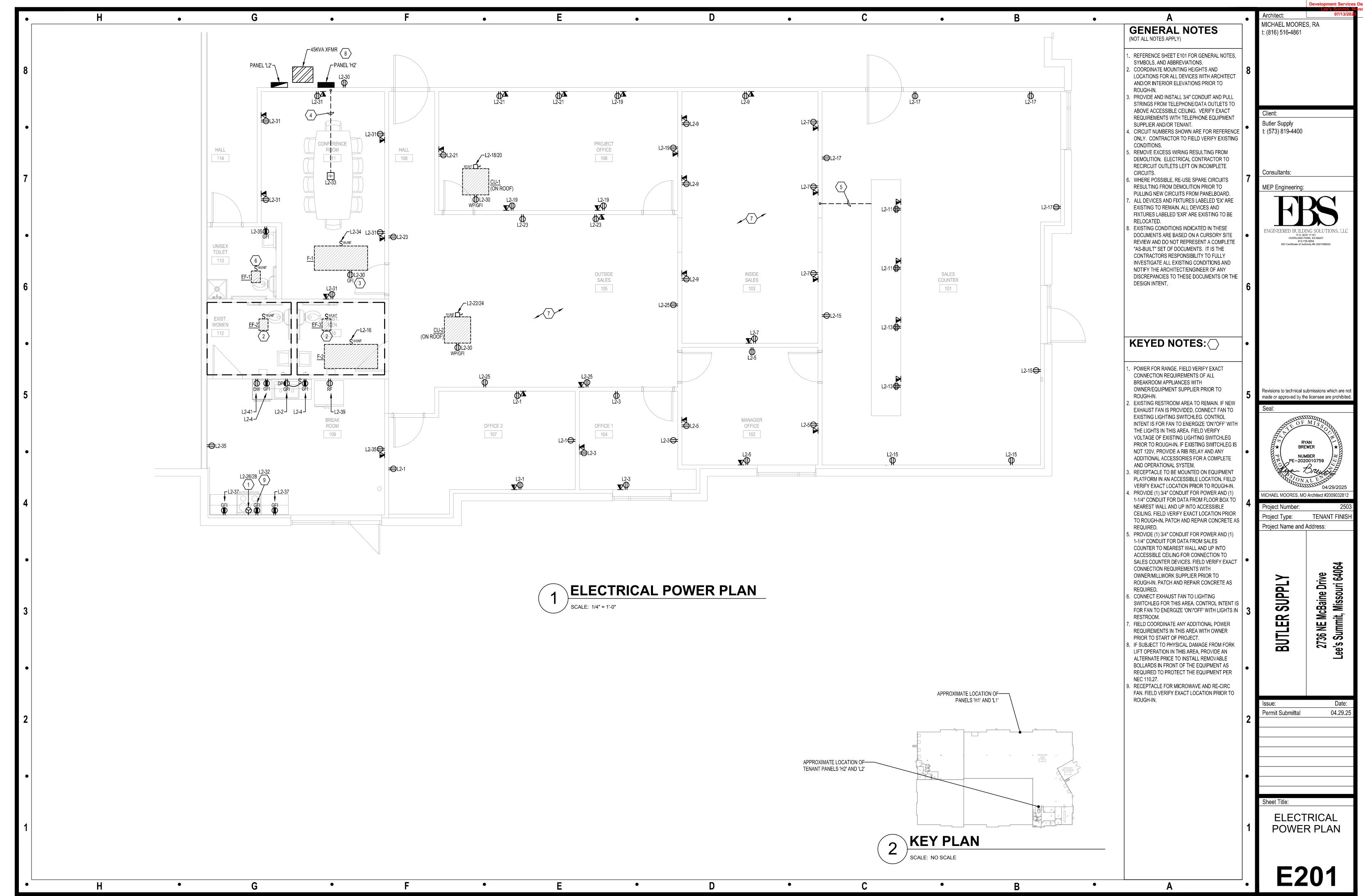


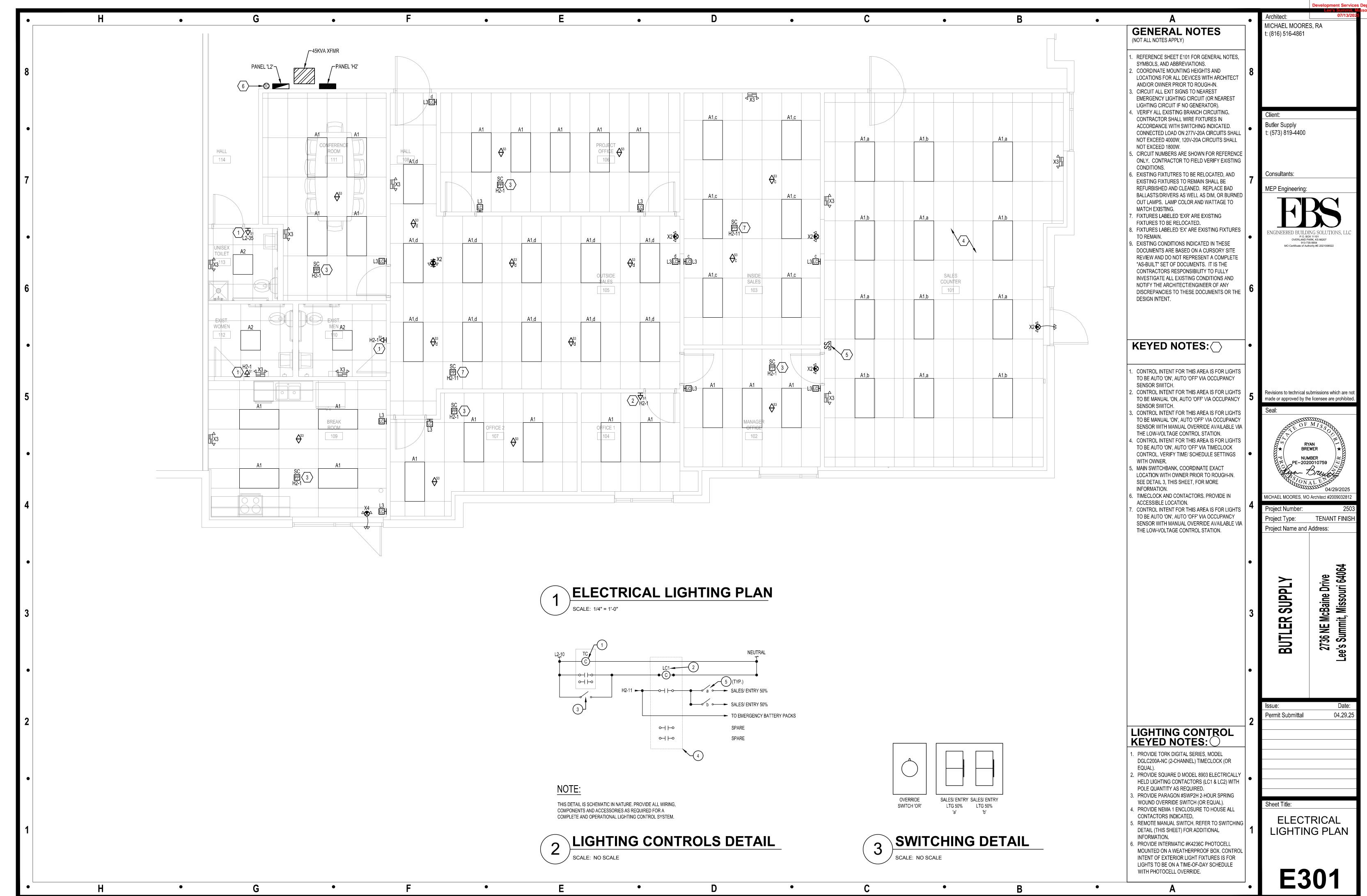




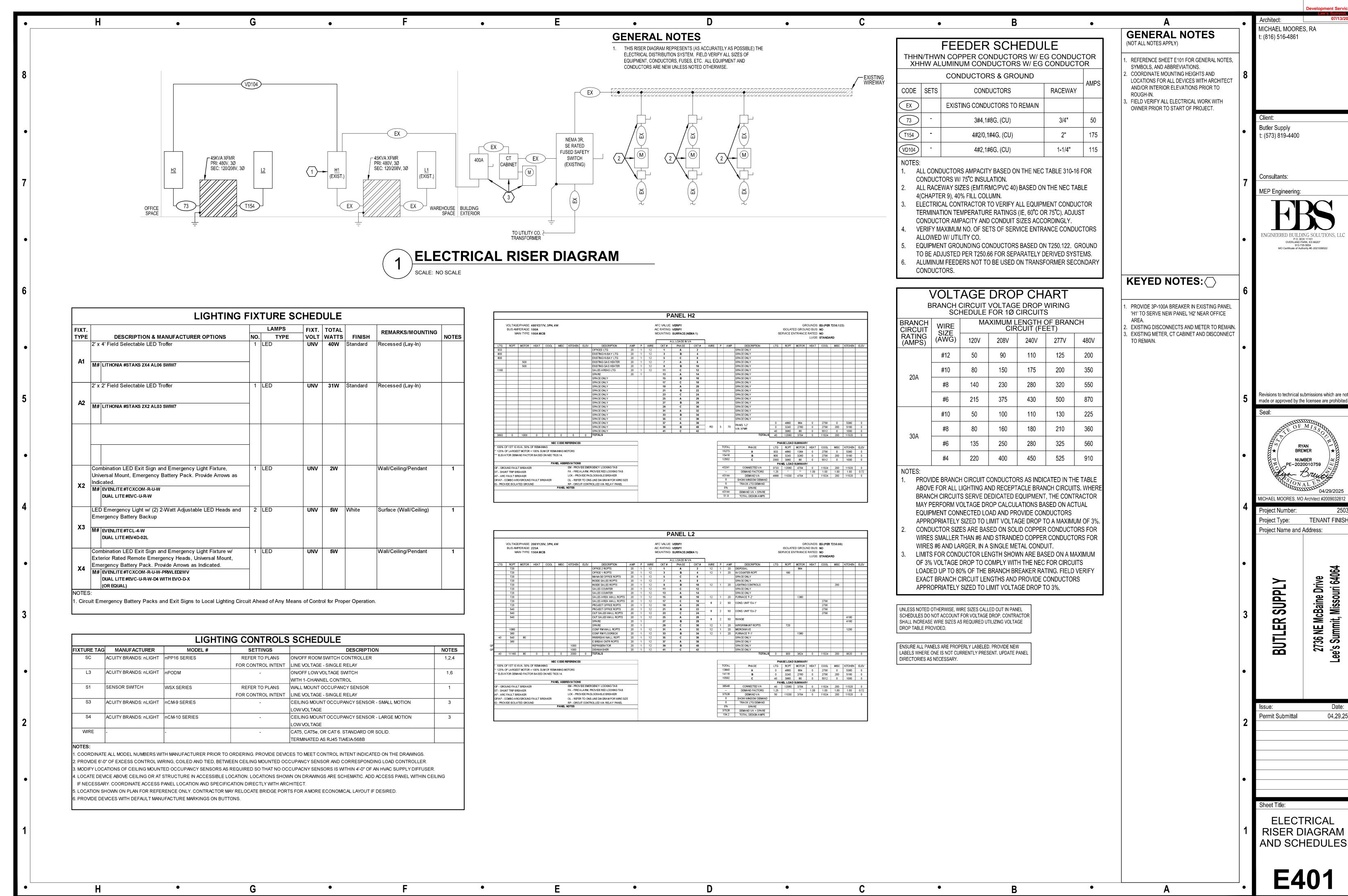
				_			Development Service Lee's Summit, II 07/13/202
•	H • G	• F	• E •	D •	C • B	• A	Architect: 07/13/202 MICHAEL MOORES, RA
	15400 - PLUMBING WORK	ACCESS DOORS INSTALL AS REQUIRED FOR MAINTENANCE, ADJUSTMENT, REMOVAL AND REPAIR	ESCUTCHEONS PROVIDE FOR ALL PIPING THROUGH WALLS, FLOORS AND CEILING WERE PIPING IS	INSERTS: IN CONCRETE, GRINNELL MODEL NO. 285 OR APPROVED EQUAL, HAVING ADJUSTMENT FROM 3/4 INCH THROUGH 1-1/4 INCH. IN METAL DECKS READHEAD	INSTALLATION DRAIN SHALL BE INSTALLED WHERE SHOWN ON THE DRAWINGS: ACCESSIBLE AND	DOMESTIC WATER HEATING	t: (816) 516-4861
	DESCRIPTION ALL PLUMBING AND ASSOCIATED WORK IN DIVISION 15 IS GOVERNED BY THIS	OF VALVES, EQUIPMENT AND LIKE ITEMS. DOORS SHALL BE CONVENIENTLY LOCATED AND OF SUFFICIENT SIZE.	EXPOSED TO VIEW IN FINISHED AREA. ESCUTCHEONS SHALL BE CHROMIUM PLATED, TWO PIECE, HINGED WITH SET SCREW.	SD1 OR APPROVED EQUAL. POWDER PROPELLED PERMITTED IN NEW CONSTRUCTION WHERE TYPE AND LOCATION ARE APPROVED PRIOR TO	LOCATED TO SUIT EQUIPMENT APPROVED FOR INSTALLATION. WHERE FLUSH VALVES ARE SPECIFIED WITH FIXTURES. THE SUPPLY TO THE VALVE IN EACH	DESCRIPTION PROVIDE DOMESTIC WATER HEATING EQUIPMENT WHERE SHOWN ON DRAWINGS	
	SECTION. PROVIDE LABOR AND MATERIALS NECESSARY TO PROVIDE THE WORK			INSTALLATION. IN EXISTING CONSTRUCTION, START SLUGIN NO. 6800 SERIES OR	ROOM SHALL BE AT THE SAME HEIGHT FOR THE TYPE OF FIXTURE AND THE VALVE		
8	AS SHOWN ON THE DRAWINGS AND SPECIFIED HEREIN. REFER TO OTHER DIVISIONS FOR CONTINUATION OF EXTERIOR AND ALLIED WORK.	PIPING PROVISIONS FOR FIXTURES AND EQUIPMENT SPECIFIED IN OTHER SECTIONS OR FURNISHED BY THE OWNER	UNIONS PROVIDE GROUND JOINT BRASS UNIONS OR FLANGES ON EACH PIPING	APPROVED EQUAL.	SHALL BE SET IN PLACE SO THAT THE CENTER LINE OF THE VALVE DISCHARGE IS DIRECTLY ABOVE THE CENTER LINE OF FIXTURE STUD. BENDING OF NIPPLE	DISCHARGE PIPE	8
	QUALITY ASSURANCE	ROUGH IN LOCATIONS SHALL BE DETERMINED FOR SERVICES. PROVIDE ALL NECESSARY PLUMBING SERVICES, ACCESSIBLE VALVES ON PLUMBING BRANCHES	CONNECTION TO EQUIPMENT.	SIDE BEAM CLAMPS: PROVIDE WHEN SUPPORTING FROM STRUCTURAL STEEL MEMBERS, GRINNELL, MODEL 225 OR APPROVED EQUAL.	BETWEEN THE VALVE AND THE STUD TO ACHIEVE CONNECTION WILL NOT BE PERMITTED.	RELIEF VALVE DISCHARGE SHALL BE COPPER WATER TUBE, TYPE M.	
	OBTAIN AND PAY FOR ALL PERMITS, INSPECTIONS AND CONNECTION FEES	AND MAKE ALL FINAL CONNECTIONS.	PROVIDE DIELECTRIC UNIONS BETWEEN COPPER AND STEEL PIPING CONNECTION			INSTALLATION	
	REQUIRED BY GOVERNING BODIES IN CONNECTION WITH THE WORK. DELIVER CERTIFICATES OF INSPECTION TO THE OWNER'S REPRESENTATIVE. ALL WORK	PLUMBING OPERATION AND MAINTENANCE MANUALS	TO EQUIPMENT.	OTHER SUPPORTS: OBTAIN OWNER'S REPRENTATIVE APPROVAL FOR OTHER METHODS OF SUPPORT.	CHROME PLATED PIPING REQUIRING THE USE OF WRENCH SHALL BE PROTECTED FROM DAMAGE.	WATER HEATER SHALL BE INSTALLED WHERE SHOWN ON THE DRAWINGS.	
	SHALL COMPLY WITH GOVERNING CODES, ORDINANCES, AND REGULATIONS OF CITY, COUNTY AND STATE.	DESCRIPTION	VACUUM BREAKERS SHALL CONFORM TO THE REQUIREMENTS OF THE REFERENCED PLUMBING CODE	SPACING OF HANGERS	BOLT WATER CLOSET CARRIER TO FLOOR.	DISCHARGE PIPE SHALL HAVE TERMINATING END CUT AT 45 DEGREE ANGLE.	Client:
•	,	FURNISH TWO COPIES OF COMPLETE OPERATION AND MAINTENANCE MANUALS	AND SHALL BE PROVIDED FOR HOSE BIBBS, FLUSHOMETERS AND ANY FIXTURE OR	PROVIDE HANGER AT EACH CHANGE OF DIRECTION.		TERMINATE RELIEF VALVE DRAIN AS SHOWN ON THE DRAWINGS.	Butler Supply
	SUBMITTALS SHOP DRAWINGS: SUBMIT MATERIALS, PRODUCTS, EQUIPMENT AND SYSTEMS AS	TO THE OWNER'S REPRESENTATIVE, FOR APPROVAL AND FOR THE OWNER, ON PLUMBING EQUIPMENT AND SPECIALTIES. THE MANUAL SHALL BE BOUND IN	EQUIPMENT WATER SUPPLY HAVING A THREADED OUTLET.	SPACE HANGERS AND SUPPORTS TO PREVENT SAGGING AND REDUCE STRAIN ON	GAS PIPING		t: (573) 819-4400
	SPECIFIED UNDER EACH PLUMBING SECTION IN THIS DIVISION IN ACCORDANCE	HARD-BACK, THREE-RING LOOSE-LEAF BINDERS.	FLASHING	VALVES AND SPECIALTIES WITH SPACING NO GREATER AND ROD NO SMALLER	PIPING		
	WITH THE GENERAL CONDITIONS. SHOW PIPE SIZES, LOCATION, SLOPES OF HORIZONTAL RUNS, FITTINGS, VALVES, METERS, GAGES AND CONNECTIONS.	MANUAL CONTENTS	VENT FLASHING SHALL COMPLY WITH ROOFING MANUFACTURER'S WRITTEN SPECIFICATIONS	THAN SHOWN ON THE FOLLOWING TABLE. HANGERS SHALL ALLOW FOR EXPANSION AND CONTRACTION.	SHALL COMPLY WITH THE REQUIREMENTS OF NFPA NO. 54 AND THE LOCAL GAS COMPANY.		
	PRODUCT DATA: SUBMIT ON MATERIALS, FIXTURES, AND EQUIPMENT UNLESS	TITLE SHEET WITH JOB NAME, AND THE NAMES, ADDRESSES AND PHONE NUMBERS OF THE CONTRACTOR, SUBCONTRACTOR, CONTROL SUBCONTRACTOR.	CLEANOUTS	FERROUS PIPING AND COPPER TUBING:	PIPE SHALL BE SCHEDULE 40 BLACK STEEL PIPE WITH MALLEABLE FITTINGS		Consultants:
7	OTHERWISE SPECIFIED OR ACKNOWLEDGED IN WRITING.	RELATED CONTRACTOR AND MATERIAL AND EQUIPMENT SUPPLIERS.	CLEANOUTS ON NO-HUB PIPE SHALL BE STANDARD NO-HUB FITTINGS.	DIAMETER OF PIPE MAXIMUM SPACING ROD SIZE			MEP Engineering:
	SAMPLES: SUBMIT WHEN SPECIFIED OR REQUESTED.	INDEX OF CONTENTS	CLEANOUTS ON CAST IRON HUB AND SPIGOT PIPING, SHALL BE CADMIUM PLATED. APPROVED MANUFACTURERS: ZURN, JOSAM OR JONESPEC.	1/2" THROUGH 1-1/2" 6 FT. 3/8" 2" THROUGH 3" 10 FT. 1/2"	INSTALLATION PIPING SHALL COMPLY WITH THE REQUIREMENTS OF NFPA NO. 54 AND THE LOCAL		WLF Liighteering.
	RECORD DOCUMENTS	TYPEWRITTEN OPERATING INSTRUCTIONS FOR THE OWNER'S PERSONNEL DESCRIBING HOW TO OPERATE EACH PIECE OF EQUIPMENT, AND CAUTION AND	TRAP PRIMERS	4" THROUGH 5" 12 FT. 5/8" 6" AND LARGER 16 FT. 3/4" D.	GAS COMPANY.		
	REFER TO GENERAL CONDITIONS FOR REQUIREMENTS CONCERNING RECORD	WARNING NOTICES.	PROVIDE WHERE INDICATED ON DRAWINGS. PRECISION PRODUCTS WITH	CAST IRON PIPING:	INSTALL GAS SHUT-OFF AND GAS MANIFOLDS AS INDICATED OR REQUIRED.		
	DOCUMENTS. ADDITIONAL REQUIREMENTS MAY BE SPECIFIED IN DIVISION 1. UNLESS SEPIAS OF THE DRAWINGS ARE TO BE FURNISHED BY THE	APPROVED SHOP DRAWINGS, PRODUCT DATA AND PARTS AND MAINTENANCE	DISTRIBUTION UNIT OR APPROVED EQUAL.	DIAMETER OF PIPE MAXIMUM SPACING ROD SIZE 2" AND 3" EACH JOINT 3/8"	DOMESTIC HOT AND COLD WATER		ENGINEERED BUILDING SOLUTIONS LLC
•	ARCHITECT-ENGINEER FOR PREPARATION OF RECORD DRAWINGS, FURNISH OWNER'S REPRESENTATIVE WITH TWO SETS OF ACCURATELY MARKED COPIES OF	BOOKLET FOR EACH ITEM OF PLUMBING, EQUIPMENT SPECIFIED IN DIVISION 15.	PIPE SLEEVES 1. EXTEND SLEEVE 1/4 INCH BEYOND FINISHED SURFACE.	4" AND 5" EACH JOINT 1/2" 6" AND 8" EACH JOINT 3/4"	DESCRIPTION	•	P.O. BOX 11101 OVERLAND PARK, KS 66207 913-735-5654
	THE DRAWINGS, INSTEAD OF ONE SET AS REQUIRED BY THE GENERAL	COPIES OF CERTIFICATES OF INSPECTION, WHERE INSPECTION IS REQUIRED.	2. SET SLEEVE BEFORE POURING CONCRETE.	10" THROUGH 15" EACH JOINT 3/4" (TWO HANGERS)	THE WORK INCLUDES FURNISHING AND INSTALLING HOT AND COLD WATER PIPING		MO Certificate of Authority #E-2021006522
	CONDITIONS, INDICATING ALL CHANGES FROM ORIGINAL DRAWINGS AS INSTALLED.	GUARANTEES, INCLUDING EXTENDED GUARANTEES.	3. PROVIDE CLEARANCE BETWEEN SLEEVE AND PIPE OR BETWEEN SLEEVE AND INSULATION TO ALLOW FOR PIPE MOVEMENT DUE TO EXPANSION AND	RISER CLAMPS	AS SHOWN ON THE DRAWINGS AND SPECIFIED HEREIN.		
	PRODUCT HANDLING	DELIVERY DELIVER THE MANUALS TO THE OWNER'S REPRESENTATIVE PRIOR TO	CONTRACTION. 4. INSULATION SHALL PASS CONTINUOUS THROUGH THE SLEEVE.	INSTALL AT EACH LEVEL BELOW THE FLOOR. SUSPEND FROM TWO HANGER RODS	PIPING HOT AND COLD WATER PIPING SHALL BE COPPER WATER TUBE HARD TEMPER,		
6	PIPE, FIXTURES AND ACCESSORIES SHALL BE PROTECTED FROM DAMAGE IN	SUBMITTING APPLICATION FOR FINAL PAYMENT.	5. CAULK BETWEEN SLEEVE AND PIPE OR SLEEVE AND INSULATION.	AND INSERTS WHERE THE INSTALLATION OF ESCUTCHEON PLATES IS REQUIRED.	TYPE "L" WITH WROUGHT SOLDER FITTINGS ABOVE FLOOR AND SOFT TEMPER		6
	SHIPMENT, HANDLING, STORAGE AND INSTALLATION: FROM MOISTURE, DIRT AND DEBRIS. PIPE, CLEANOUT AND FLOOR DRAIN OPENINGS SHALL BE TEMPORARILY	PLUMBING PIPING	PREFABRICATED, PRE-INSULATED, "PIPE SHIELDS" WILL BE ACCEPTABLE FOR PIPES PASSING THROUGH FLOORS, EXTERIOR WALLS, FIRE WALLS AND	TESTING OF PLUMBING PIPING	TYPE "K" WITH WROUGHT SOLDER FITTINGS BELOW GRADE.		
	PLUGGED WITH TEST PLUGS UNTIL FINAL CONNECTIONS ARE MADE.	DESCRIPTION DESCRIPTION	FIRE RESISTIVE WALLS AND PARTIITONS. 6. ESCUTCHEONS: FIT AROUND INSULATION WHERE PRESENT. PROVIDE DEEP	DESCRIPTION	GATE VALVES SPECIFIED IN SECTION, PLUMBING VALVES .		
	GUARANTEE AND SERVICE	FURNISH AND INSTALL PLUMBING PIPING WHERE SHOWN ON DRAWINGS AND AS	ESCUTCHEON PLATES WHERE PIPE SLEEVES EXTEND ABOVE FLOORS.	CONDUCT ALL TESTS AFTER PIPING IS INSTALLED AND BEFORE PIPING IS CONCEALED OR COVERED.	,		
	REFER TO GENERAL CONDITIONS FOR GUARANTEE. WHERE EXTENDED GUARANTEES ARE CALLED FOR, FURNISH THREE COPIES TO BE INSERTED INTO	SPECIFIED.	 WATER HAMMER ARRESTERS: INSTALL WHERE SHOWN ON DRAWINGS. CLEANOUTS: INSTALL WHERE SHOWN ON DRAWINGS AND AT BASE OF ALL 	PROVIDE ALL NECESSARY TEMPORARY PIPING CLOSURES.	INSTALLATION NOTCHING OF PIPE FOR CONNECTION NOT PERMITTED.		
•	OPERATION AND MAINTENANCE MANUALS.	PIPING MATERIALS OPTIONS	RISERS. PROVIDE ADDITIONAL CLEANOUTS WHERE REQUIRED BY LOCAL CODES AND FOR CONVENIENCE OF TESTING AND ERECTION AT	PROVIDE ALL TESTING EQUIPMENT, MATERIALS AND SUPPLIES.	WHERE POSSIBILITY OF BACKFLOW FROM THE DRAIN TO THE SUPPLY FITTING		, [
	GENERAL	1. CAST IRON HUBLESS SANITARY PIPE AND FITTINGS: CISPI STD. 301.	CONTRACTOR'S OPTION.	·	EXISTS, INSTALL VACUUM BREAKERS.		
	PLUMBING SYSTEMS SHALL BE PROVIDED COMPLETE. SHOULD A SYSTEM, OR ANY PART THEREOF FAIL TO MEET PERFORMANCE REQUIREMENTS, NECESSARY	 CAST IRON SOIL PIPE AND FITTINGS, SERVICE WEIGHT: ASTM A 74. CAST IRON SOIL PIPE AND FITTINGS, EXTRA HEAVY WEIGHT: ASTM A 74. 	 FRAMES AND COVERS SHALL BE FLUSH WITH ADJOINING ARCHITECTURAL FINISH. 	SYSTEMS SHALL REMAIN UNDER TEST FOR SUFFICIENT LENGTH OF TIME TO PROVE TIGHTNESS THEREOF AND FOR ADEQUATE OBSERVATION BY THE	NOT MORE THAN ONE LAVATORY, SINK, OR SIMILAR FIXTURE SHALL BE SUPPLIED		
	REPLACEMENTS, ALTERATIONS OR REPAIRS, AS REQUIRED BY THE OWNER'S REPRESENTATIVE, SHALL BE MADE TO BRING PERFORMANCE UP TO SPECIFIED	 STEEL PIPE: ASTM A 53. MALLEABLE IRON FITTINGS, 150 LB.: ASTM A 197. 	PLUMBING VALVES	ARCHITECT-ENGINEER.	BY A 1/2 INCH BRANCH. LINEAR DIMENSIONS NOT TO EXCEED 5 FEET.		
	REQUIREMENTS AND ALL BUILDING CONSTRUCTION AND FINISHES DAMAGED OR	6. PIPE THREADS: ANSI B2.1.		MATERIALS OTHER THAN THOSE SPECIFIED FOR JOINING WILL NOT BE PERMITTED			Revisions to technical submissions which are not
5	MARRED BY SUCH REPLACEMENTS, ALTERATIONS OR REPAIRS SHALL BE RESTORED TO PRIOR CONDITION, AT NO ADDITIONAL COST TO THE OWNER.	 NIPPLES, PIPE (THREADED): FED SPEC. WW-N-351. COPPER WATER TUBE: ASTM B 88. 	DESCRIPTION INSTALL IN ACCESSIBLE LOCATION.	IN THE PIPING SYSTEMS FOR THE PURPOSE OF STOPPING LEAKS.	OR SPECIFIED HEREIN.		made or approved by the licensee are prohibited.
	WHERE MULTIPLE ITEMS OF EQUIPMENT OR MATERIALS ARE REQUIRED THEY	9. WROUGHT COPPER AND BRONZE SOLDER-JOINT PRESSURE FITTINGS: ANSI B16.29.	VALVES SHALL NOT BE INSTALLED WITH THE STEMS BELOW THE HORIZONTAL POSITION.	ALL LEAKS DISCLOSED BY THE TESTING PROCEDURES SHALL BE REPAIRED AND TESTING REPEATED UNTIL THE SYSTEM IS PROVEN TIGHT.	HOT WATER BRANCH CONNECTIONS TO DISTRIBUTION MAINS SHALL BE TOP TAKE-OFF, SWING JOINT TYPE.		Seal:
	SHALL BE THE PRODUCT OF A SINGLE MANUFACTURER.	10. WROUGHT COPPER AND WROUGHT COPPER ALLOY SOLDER-JOINT					OF MISS
	INSERTS, PIPE SLEEVES, HANGERS, SUPPORTS, FIXTURES, TRIM DRAINS AND	DRAINAGE FITTINGS: ANSI BL6.29. 11. CAULKING LEAD: FED. SPEC. QQ-C-40 (2).	VALVES, GATE, 125# UNION BONNET. RISING STEM 3 INCH AND SMALLER:	TESTING REQUIREMENTS ARE MINIMUM AND ARE NOT INTENDED TO BE LIMITING WHERE ADDITIONAL TESTING METHODS ARE REQUIRED BY THE AUTHORITY	ALL PIPING INSTALLED BELOW GROUND SHALL RECEIVE TWO COATS OF KOPPERS NO. 50 OR APPROVED EQUAL.		
	ANCHORAGE OF PLUMBING SHALL BE PROVIDED AS SPECIFIED HEREIN. WHERE SUCH ITEMS ARE TO BE SET OR EMBEDDED IN CONCRETE, MASONRY OR SIMILAR	12. SHEET LEAD: FED. SPEC. QQ-L-201. 13. SHEET COPPER: ASTM B 152.	 SCREWED: ITT GRINNELL #3080 OR APPROVED EQUAL. SOLDER JOINT: ITT GRINNELL #3080 SJ OR APPROVED EQUAL. 	HAVING JURISDICTION.	PLUMBING INSULATION		RYAN BREWER
•	WORK, THE ITEMS SHALL BE FURNISHED AND LAYOUT MADE AT THE PROPER TIME	14. NO-HUB STAINLESS STEEL COUPLING AND GASKETS: CISPI STD. S-301.		SUBMITTALS			NUMBER 2020010759 20 20 20 20 20 20 20 20 20 20 20 20 20
	FOR THE SETTING OR EMBEDMENT THEREOF SO AS TO CAUSE NO DELAY IN THE WORK.	 WHERE ACCEPTABLE BY LOCAL AUTHORITY HAVING JURISDICTION SOLID WALL ABS PIPING MAY BE USED FOR WASTE PIPING. 	VALVES, BALL (MAY BE USED IN LIEU OF GATE VALVES UP TO 2"): 2" AND SMALLER NIBCO #T580; TWO PIECE BRONZE BODY, WITH SCREEWED ENDS, CHROME	STERILIZATION: PROVIDE A DATED LETTER TO THE ARCHITECT-ENGINEER'S REPRESENTATIVE STATING THAT PIPING SYSTEM HAS BEEN STERILIZED AND	DESCRIPTION INSULATION SHALL NOT BE INSTALLED UNTIL TESTING PROCEDURES HAVE BEEN		Rye-Brend
	MANUFACTURER'S NAMES AND CATALOG NUMBERS	15.A. PVC/ABS PIPING CANNOT BE USED IN RETURN AIR PLENUM APPLICATION.	PLATED BRONZE BALL WITH CONVENTIONAL PORT, 400 PSI, BLOW OUT PROOF STEM.	FLUSHED AS SPECIFIED.	COMPLIED WITH AND ALL SURFACES HAVE BEEN CLEANED AND FREE OF DIRT, GREASE AND COMPLETELY DRIED.		NONAL ENGINEER
	SPECIFIED REFERENCES HAVE BEEN MADE TO ONE OR MORE MANUFACTURER'S			PIPING TEST			04/29/2025 MICHAEL MOORES, MO Architect #2009032812
4	NAMES AND MODEL OR CATALOG NUMBERS. THIS DOES NOT NECESSARILY INDICATE THAT THE MATERIAL AND EQUIPMENT SPECIFIED IS AN "OFF THE SHELF"	JOINTS AND CONNECTIONS OPTIONS	VALVES, GLOBE 150# TEFLON DISC. UNION BONNET 3 INCH OR SMALLER:	DOMESTIC HOT AND COLD WATER PIPING SHALL BE FILLED, THEN TESTED TO A HYDROSTATIC PRESSURE OF 150 PSIG. MAINTAIN TEST PRESSURE FOR A MINIMUM	MATERIALS SHALL COMPLY WITH UL 723, FLAME SPREAD RATING, HOT SURFACE TEST PERFORMANCE, AND SMOKE DEVELOPED RATING.		Project Number: 2503
	ITEM. REQUIREMENTS FOR SPECIFIC FINISHES, MATERIAL OR OTHER MODIFICATIONS MAY INTRODUCE VARIANCES FROM MANUFACTURERS'	CAST IRON, HUB AND SPIGOT: PACKED WITH OAKUM AND FINISHED WITH LEAD NOT LESS THAN 1 INCH DEEP: WELL CAULKED.	 SCREWED: ITT GRINELL #3240 OR APPROVED EQUAL. SOLDER JOINT: ITT GRINELL #3240 SJ OR APPROVED EQUAL. 	OF ONE HOUR.	SUBMITTALS		Project Type: TENANT FINISH
	STANDARDS. MODIFICATIONS SHALL BE FULLY CONSIDERED.	2. CAST IRON, NO-HUB: NEOPRENE GASKET AND CORRUGATED 304 STAINLESS		SANITARY PIPING, PREVIOUS TO CONNECTION OF FIXTURES, SHALL BE FILLED	SAMPLES AND MANUFACTURER'S PRODUCT DATA: SUBMIT SAMPLES OF		Project Name and Address:
	CHARTS AND TAGS	STEEL SHIELD IN CONJUNCTION WITH 4 STAINLESS STEEL CLAMPS FOR 4" AND SMALLER, 6 CLAMPS FOR 5" AND LARGER.	VALVES, CHECK 125# REMOVABLE REGRINDABLE DISC A. 3 INCH AND SMALLER, HORIZONTAL:	WITH WATER TO THE TOP OF THE SYSTEM AND PROVEN TIGHT. WHEN TESTING THE SYSTEM BY SECTIONS THE MINIMUM HEIGHT OF THE WATER COLUMN SHALL	INSULATION AND ADHESIVE AND PRODUCT DATA LISTING RECOMMENDATIONS FOR USE AND COMPLIANCE WITH NFPA 90.		
	IN AREAS HAVING VALVES, PROVIDE SINGLE LINE DIAGRAMS FRAMED UNDER GLASS AND MOUNTED ON EQUIPMENT ROOM WALL. THE DIAGRAMS SHALL GIVE	3. BETWEEN LEAD AND BRASS: FERRULES OR SOLDERING NIPPLES WITH WIPED JOINTS 3/8" THICK AND 3/4" EACH SIDE OF JOINTS.	 SCREWED: ITT GRINELL #3300 OR APPROVED EQUAL. SOLDER JOINT: ITT GRINELL #3300 SJ OR APPROVED EQUAL. 	BE 10 FEET. EXAMINE ALL JOINTS FOR LEAKS.	INSULATION		
•	NAME, NUMBER DESIGNATION AND LOCATION OF VALVE.	4. SCREWED JOINTS: AMERICAN NATIONAL STANDARD WITH PIPE FREE FROM		NEW FIRE STANDPIPE SYSTEM SHALL BE TESTED TO A HYDROSTATIC PRESSURE	INSULATION FOR HOT AND COLD WATER PIPING, SHALL BE SECTIONAL GLASS	•	•
	VALVES SHALL BE IDENTIFIED WITH 1/16 INCH THICK WHITE LAMINATED PLASTIC	CUTTING AND BURRS. THREE THREADS EXPOSED MAXIMUM. 5. SOLDERED JOINTS: 95-5 TIN-ANTIMONY SOLDER, SLIP JOINTS: USE FOR	3 INCH AND SMALLER, VERTICAL: 1. FOR SCREWED AND SOLDER JOINT INSTALLATION, SAME AS SECTION A OR	OF 200 PSIG. MAINTAIN TEST PRESSURE FOR A MINIMUM OF TWO HOURS.	FIBER AS MANUFACTURED BY OWENS CORNING FIBERGLASS TYPE ASJ/SSLII OR APPROVED EQUAL, WITH FACTORY APPLIED, ALL PURPOSE, FIRE RETARDANT		, e 4064
	NAMEPLATES WITH 3/16 INCH HIGH BLACK LAMINATED LETTERS. THE NAMEPLATE	PLUMBING TRAP SEALS ON INLET SIDE ONLY.	APPROVED EQUAL. PROVIDE ADAPTERS FOR SOLDER JOINT CONNECTION. 2.05	GAS PIPING SHALL BE TESTED WITH NITROGEN TO 50 PSIG. PRESSURE SHALL BE MEASURED WITH A MANOMETER. MAINTAIN TEST PRESSURE FOR A MINIMUM OF	JACKET.		JLY Drive
	IDENTIFICATION SHALL COINCIDE WITH ITEMS APPEARING ON DIAGRAMS. ATTACH NAMEPLATES TO VALVES WITH NON-CORROSIVE CHAIN OR WIRE.	 BETWEEN COPPER AND FERROUS MATERIALS: INSULATING DIELECTRIC UNION. 	HOSE BIBBS A. SEE FIXTURE SCHEDULE ON DRAWINGS. B. PLUG COCKS, 125# BRONZE COCKS. TWO (2) INCH AND SMALLER SHALL BE CRANE NO. 250 OR	MEASURED WITH A MANOMETER. MAINTAIN TEST PRESSURE FOR A MINIMUM OF 30 MINUTES.	INSULATION FOR EXPOSED HOT AND COLD WATER PIPING SHALL BE SECTIONAL		UPPI aine D
	ACCESS DOORS	 FLANGED JOINTS: FURNISH WITH COMPANION FLANGE AND CLOTH INSERTED RUBBER GASKET. 	APPROVED EQUAL.	STERILIZATION	GLASS FIBER AS MANUFACTURED BY OWENS CORNING FIBERGLASS TYPE ASJ/SSLII OR APPROVED EQUAL, WITH FACTORY APPLIED, .016 EMBOSSED		SU Sair Su Miss
3	PROVIDE ACCESS DOORS AS INDICATED AND SPECIFIED IN DRAWINGS	8. FLANGED BOLTS: ASTM A 354, MINIMUM GRADE BD, ALLOY STEEL WITH HEX	INSTALLATION	AFTER TESTS ARE COMPLETED ALL WATER SUPPLY SYSTEMS SHALL BE FILLED	ALUMINUM JACKET.		
	INSTALLATION AND WORKMANSHIP	NUTS IN COMPLIANCE WITH ANSI B18.22 AND STANDARD ROLLED STEEL WASHERS.	INSTALL VALVES WHERE SHOWN ON DRAWINGS.	WITH A SOLUTION CONTAINING 100 PPM OF AVAILABLE CHLORINE AND ALLOWED TO STAND FOR A PERIOD TO TWO HOURS BEFORE BEING FLUSHED WITH CLEAN	ADHESIVE SHALL BE BENJAMIN FOSTER 30-36, OR APPROVED EQUAL, WHITE		
	THE WORK SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. ANY MATERIAL, APPARATUS OR EQUIPMENT WHICH, IN THE	 ASSEMBLY FOR HUBLESS PIPING: AS RECOMMENDED BY THE MANUFACTURER. 	PLUMBING HANGERS AND SUPPORTS	WATER.	INSULATION LAGGING ADHESIVE.		BUT 2736 e's Su
	OPINION OF THE OWNER'S REPRESENTATIVE, IS IMPROPERLY INSTALLED SHALL BE REMOVED AND REINSTALLED IN AN APPROVED MANNER AT NO ADDITIONAL	10. CHANGES IN PIPE SIZE SHALL BE MADE WITH REDUCERS, INCREASERS OR REDUCING FITTINGS. BUSHINGS WILL NOT BE PERMITTED.	DESCRIPTION PROVIDE HANGERS FOR ALL PIPING NOT INDICATED BELOW GRADE. USE	PLUMBING, FIXTURES, TRIM AND DRAINS	VAPOR BARRIER MASTIC SHALL BE BENJAMIN FOSTER NO. 82-07, WHITE, OR APPROVED EQUAL.		BU 27 -ee's
	COST TO THE OWNER.		HANGERS CAPABLE OF ADJUSTMENT.	MANUFACTURER			, [
	THE LOCATION OF PLUMBING PIPING SHALL BE COORDINATED TO ENSURE THAT IT	INSTALLATION BEFORE INSTALLING PIPE IN ANY PART OF THE SYSTEM, THE PIPE SHALL BE	HANGERS AND SUPPORTS	MANUFACTURER SHALL BE AS SCHEDULED OR BY APPROVED EQUAL.	INSTALLATION HOT AND COLD WATER PIPING: SHALL BE INSULATED WITH 1/2 INCH THICK GLASS		
	CLEARS OPENINGS AND STRUCTURAL MEMBERS; THAT PIPING INDICATED AS CONCEALED CAN BE PROPERLY CONCEALED IN WALLS OR PARTITIONS AND THAT	CLEANED INSIDE AND MADE FREE OF OIL, DIRT, AND FOREIGN MATTER. PROPERLY ALIGN AND INSTALL IN NEAT ARRANGEMENT. TRUE TO THE LINES OF	HANGERS FOR BLACK OR GALVANIZED STEEL PIPE SHALL BE GRINNELL, MODEL NO. 65 OR APPROVED EQUAL.	PIPING PIPING TO SERVE FIXTURES AND EQUIPMENT AND EXPOSED TO VIEW IN FINISHED	FIBER INSULATION HAVING A FACTORY APPLIED, ALL PURPOSE, FIRE RETARDANT JACKET WITH A MINIMUM R-4.0 PER INCH. CONCEALED AND EXPOSED PIPING		
	IT DOES NOT INTERFERE WITH LIGHTS, DUCTWORK OR EQUIPMENT HAVING FIXED	THE BUILDING. PITCH LINE AT A CONSTANT SLOPE FOR PROPER DRAINAGE.		AREAS SHALL BE BRASS, CHROMIUM PLATED.	SHALL HAVE THE INSULATION APPLIED WITH SIDE AND END JOINTS BUTTED		Issue: Date:
	LOCATIONS. MAKE NECESSARY HORIZONTAL OR VERTICAL OFFSETS WITH PIPE FITTINGS TO INSTALL THE SYSTEM IN THE AVAILABLE SPACE. CONCEAL OR	EXCEPT AS NOTED OTHERWISE ON DRAWINGS, PIPING SHALL BE HELD AS HIGH AS	HANGERS FOR CAST IRON PIPE SHALL BE GRINNELL, MODEL NO. 260 OR APPROVED EQUAL.	SUPPORTS	TIGHTLY. SEAL JACKET LEGS AND BUTT JOINT STRIPS WITH ADHESIVE.		Issue: Date: Permit Submittal 04.29.25
2	INSTALL TIGHT TO STRUCTURE (IF EXPOSED) UNLESS OTHERWISE NOTED.	POSSIBLE, BETWEEN STRUCTURES AND THROUGH JOIST WEBBING, WITH DUE REGARD TO CONFLICTS WITH OTHER SYSTEMS AND THEIR REQUIREMENTS FOR	HANGERS FOR COPPER TUBING SHALL BE GRINNELL, MODEL NO. 97 C OR	PROVIDE ALL BRACKETS, PLATES, ANCHORS AND FASTENING DEVICES REQUIRED FOR ANCHORING THE FIXTURES RIGIDLY IN PLACE. RISERS TO SHOWER HEADS	INSULATE FITTINGS FOR PIPING UP TO 3 INCHES IPS WITH MOLDED GLASS FIBER. INSULATE FITTINGS FOR PIPING LARGER THAN 3 INCHES WITH MOLDED FITTINGS		2
	PIPING SHALL BE EXPOSED IN FINISHED AREAS ONLY WHERE INDICATED OR WITH	SPACE.	APPROVED EQUAL.	SHALL BE ANCHORED TO THE WALL CONSTRUCTION TO PREVENT MOVEMENT.	OR SEGMENTED SECTIONS, WIRED IN PLACE TO THE SAME THICKNESS AS		
	THE APPROVAL OF THE OWNER'S REPRESENTATIVE.	PIPING, INCLUDING NO-HUB PIPING, SHALL BE INSTALLED STRAIGHT AND TRUE TO	TRAPEZE HANGERS OF A TYPE APPROVED BY THE OWNER'S REPRESENTATIVE	FIXTURES	ADJACENT INSULATION. EXPOSED INSULATED PIPING AND FITTINGS SHALL BE JACKETED WITH 6 OUNCE CANVAS PIPING INCLUDING THE FITTING CHANGE FROM		
	WHERE DRAIN OR WATER CONNECTIONS NECESSARY TO THE OPERATION OF FIXTURES OR EQUIPMENT ARE NOT SPECIFICALLY SHOWN ON DIAGRAMS. EXTEND	VERTICAL AND HORIZONTAL LINES. DEFLECTION SHALL NOT EXCEED ONE DEGREE. WHEN NECESSARY TO ACHIEVE THIS ALIGNMENT PROVIDE ADDITIONAL	MAY BE USED WHERE PIPES ARE DESIGNED TO RUN PARALLEL AT THE SAME ELEVATION.	PROVIDE PLUMBING FIXTURES AS SCHEDULED ON DRAWINGS, AMERICAN STANDARD, KOHLER, ELJER OR APPROVED EQUAL.	HORIZONTAL TO VERTICAL. CONCEALED AND EXPOSED PIPING SHALL HAVE THE INSULATION APPLIED WITH SIDE AND END JOINTS BUTTED TIGHTLY. SEAL OFF		
	NECESSARY BRANCHES TO THE CLOSEST INDICATED BRANCH OR MAIN, AT NO	HANGERS OR BRACING.			ENDS OF INSULATION WITH VAPOR BARRIER MASTIC AT EACH FITTING AND AT 21		
•	ADDITIONAL COST TO THE OWNER.	APPLY LUBRICANT TO SCREW JOINT MALE THREADS.	PROVIDE ISOLATION HANGER WITH PROTECTIVE SHIELD, GRINNELL, MODEL NO. 300 103 OR APPROVED EQUAL, FOR ALL INSULATED PIPING. AT HANGER POINTS,	PLUMBING DRAINS FURNISH WITH SEEPAGE FLANGE WHERE INSTALLED WITH PANS OR FLASHING,	FOOT INTERVALS ON CONTINUOUS RUNS.		
	EACH FIXTURE, EQUIPMENT DRAIN OR FLOOR DRAIN SHALL BE SEPARATELY TRAPPED UNLESS OTHERWISE INDICATED OR SPECIFIED.	METAL TO BE SOLDERED SHALL BE CLEANED AND FLUXED AS SUITABLE FOR THE	PROVIDE 6 INCH LONG SECTION OF 1/2 INCH THICK CALCIUM SILICATE SECTIONAL PIPE INSULATION WITH FACTORY LONGITUDINAL LAP. SEAL BUTT JOINTS WITH	FURNISH CLAMPING RING.	INSTALL THE FACTORY APPLIED FIRE RETARDANT JACKET VAPOR BARRIER SO THAT IT WILL LAP SMOOTHLY AND SECURELY AT THE LONGITUDINAL LAP AND		
		SOLDER USED.	INSULATING CEMENT.	ALL DRAINS SHALL BE OF THE SAME MANUFACTURER.	ADHERE IT WITH VAPOR BARRIER MASTIC. ADHERE 3 INCH WIDE BUTT STRIPS X SMOOTHLY OVER ALL END JOINTS WITH THE VAPOR BARRIER MASTIC TO ASSURE		Sheet Title:
	PLUMBING PIPING AND EQUIPMENT SHALL NOT BE FIELD PAINTED, OR PRIMED BEYOND THE DEGREE OF APPLICATION FROM THE FACTORY SOURCE, OR EXCEPT	NOTCHING OF COPPER TUBING OR PLASTIC PIPING FOR CONNECTIONS WILL NOT	STRAP HANGERS: NOT PERMITTED.	FURNISH FLOOR DRAINS WITH PRIMER CONNECTIONS WHERE INDICATED ON THE	A CONTINUOUS VAPOR BARRIER - NO STAPLES ALLOWED. INSULATE DRAIN		PLUMBING
1	AS REQUIRED BY APPLICABLE CODES AND AUTHORITIES HAVING JURISDICATION.	BE PERMITTED.	RISER CLAMPS: PROVIDE RISER CLAMPS FOR VERTICAL PIPING AT EACH LEVEL,	DRAWINGS. IN LIEU OF CAST-IN PRIMER CONNECTIONS ON THE DRAIN BODY, A TEE BETWEEN THE DRAIN BODY AND THE TRAP, TO RECEIVE THE PRIMER	BODIES AND FITTINGS WITH METERED SEGMENTS OF PIPE INSULATION, OVERSIZED PIPE INSULATION OR MOLDED FITTINGS. COAT WITH TWO, 1/8 INCH		1 SPECIFICATIONS
	WATERPROOFING DO NOT CUT OR PENETRATE WATERPROOFED SURFACES OR WATERPROOFING	PLUMBING SPECIALITES	GRINNELL MODEL NO. 261	DISCHARGE WILL BE ACCEPTABLE.	COATS OF VAPOR BARRIER MASTIC REINFORCED WITH GLASS FABRIC EXTENDING 2 INCHES ONTO ADJACENT PIPES. EXPOSED INSULATED PIPING AND FITTINGS		
	MEMBRANES WITHOUT FIRST MAKING ARRANGEMENTS FOR REPAIR BY A METHOD			PROVIDE FLOOR DRAINS WITH 4 INCH DEEP SEAL TRAPS.	SHALL BE JACKETED WITH 8 OUNCE CANVAS. TERMINATE INSULATION NEATLY AT		
	APPROVED BY THE OWNER'S REPRESENTATIVE.	SCHEDULE 40 BLACK STEEL, GALVANIZED 26 GAGE STEEL, PROVIDE FOR ALL PIPES THROUGH WALLS AND FLOORS.		PROVIDE ALL DRAINS AS SCHEDULED ON DRAWINGS OR APPROVED EQUAL.	CLEANOUTS ON STORM AND COLD DRAIN PIPING. DO NOT COVER CLEANOUTS.		
							.I P301
•	H • G	• F	• E •	D •	• B	• A	

				ТГОТ		ADDDE\/I	ATIONIC			CTDIC	VI CVI		
			AC		TERNATING CURRENT	ABBREVI	ATIONS					MBOLS POWER EQUIPMENT/DEVICE)EQ
			AHU A. OR	OR AMPS. AM	R HANDLING UNIT			SYMBOL	LIGHTING FIXTURES/DEVIO	MOUNTING		DESCRIPTION	MOUNTING
			AFCI AFCI	CI AR	BOVE FINISH COUNTER RC FAULT CIRCUIT INTERRUP BOVE FINISHED FLOOR	TER		OA OA	DOWNLIGHT (LETTER INDICATES FIXTURE TYPE)	CEILING		SWITCHBOARD OR DISTRIBUTION PANEL	MOONTING
			AIC ATS	AM	MPERES INTERRUPTING CAPA JTOMATIC TRANSFER SWITC				REFER TO LIGHT FIXTURE SCHEDULE DIRECTIONAL DOWNLIGHT (LETTER INDICATES FIXTURE TYPE)			REFER TO PANEL SCHEDULES DRY-TYPE TRANSFORMER	
			BTC C	CO	ONDUIT ("E.C." IS EMPTY CON	NT AND CONNECT EQUIPMENT NDUIT)		ÔA ————————————————————————————————————	REFER TO LIGHT FIXTURE SCHEDULE	CEILING		REFER TO PLANS FOR KVA RATING	
			CF CM	CO	EILING FAN OFFEE MAKER OOKTOP			Юа	WALL MOUNTED LIGHT FIXTURE (LETTER INDICATES FIXTURE TYP REFER TO LIGHT FIXTURE SCHEDULE	E) WALL		120/208V, 3Ø, 4W PANELBOARD REFER TO PANEL SCHEDULES	
			D DCO	DEI	EDICATED CIRCUIT UPLEX CONVIENCE OUTLET			A	LINEAR LIGHT FIXTURE (LETTER INDICATES FIXTURE TYPE) REFER TO LIGHT FIXTURE SCHEDULE	CEILING OR SUSPENDED		277/480V, 3Ø, 4W PANELBOARD REFER TO PANEL SCHEDULES	
			DP DW	DIS	SPOSAL SHWASHER			A	2X4 LIGHT FIXTURE (LETTER INDICATES FIXTURE TYPE) REFER TO LIGHT FIXTURE SCHEDULE	CEILING	<u>Q</u>	JUNCTION BOX	WALL OR CEILING
			EMT FF	T ELE	RYER LETRICAL METALLIC TUBING KHAUST FAN			A	2X2 LIGHT FIXTURE (LETTER INDICATES FIXTURE TYPE) REFER TO LIGHT FIXTURE SCHEDULE	CEILING	 	FUSED SAFETY SWITCH (E.G. 30/20/3 INDICATES A 30A, 3-POLE SWITCH WITH 20A FUSES)	
			EWC EX	C ELE		ATER-COOLED DRINKING FOUNTAIN)	J)		HATCHING ON FIXTURE INDICATES FIXTURE TO HAVE		30/20/3 🗖	NON-FUSED SAFETY SWITCH	
			FCU GFI/G	/GFCI GR	AN COIL UNIT ROUND FAULT CIRCUIT INTEF				EMERGENCY BACK-UP TWO HEAD EMERGENCY LIGHT FIXTURE (LETTER INDICATES FIXTU	RE WALL OR	30/NF/3C	(E.G. 30/NF/3 INDICATES A 30A, 3-POLE SWITCH WITHOUT FUSES)	
			GFIP GRD	O GR	ROUND FAULT INTERRUPTER ROUND ORIZONTAL MOUNT (RECEPT.			X3	TYPE) REFER TO LIGHT FIXTURE SCHEDULE	CEILING	S ^M	MOTOR RATED SWITCH	
			HD HP	VEI	ENTILATION HOOD ORSEPOWER	(CLE)		<u></u> \$ x₁ 191	SHADING INDICATES FACE (LETTER INDICATES FIXTURE TYPE) REFER TO LIGHT FIXTURE SCHEDULE	WALL OR CEILING	Ø	MOTOR	
			HT HVAC		EAT TRACE POWER (PROVIDE EATING, VENTILATING, & AIR	,		S	SINGLE POLE SWITCH 20A (120/277V)	1	Ю	NEMA 5-20R SIMPLEX RECEPTACLE	WALL - 18" AFF
			HZ IG	ISC	ERTZ OLATED GROUND (DUPLEX R HOUSAND CIRCULAR MILLS	ECEPTS NEMA 5-20RIG)		S ₃	THREE WAY SWITCH 20A (120/277V)	1	₽	NEMA 5-20R DUPLEX RECEPTACLE	WALL - 18" AFF
			KVA KW	A KIL	LOVOLT-AMPERES (1000 VOL LOWATTS (1000 WATTS)	T-AMPERES)		S,	FOUR WAY SWITCH 20A (120/277V)	(1)	⊨	NEMA 5-20R DUPLEX RECEPTACLE MOUNTED ABOVE COUNTER	<u>(1)</u>
			MLO MCB	D MA B MA	AIN LUGS ONLY AIN CIRCUIT BREAKER			H.	WALL BOX DIMMER SWITCH			NEMA 5-20R QUAD-PLEX RECEPTACLE	WALL - 18" AFF
			MW NIC	NO	ICROWAVE (COORD MTG HT) OT IN CONTRACT ATIONAL ELECTRICAL CODE				CEILING OR WALL MOUNTED OCCUPANCY SENSOR	WALL OR	₩	NEMA 5-20R QUAD-PLEX RECEPTABLE NEMA 5-20R SPLIT RECEPTABLE. TOP OUTLET WIRED HOT.	
			NF OFCI	NO	ATIONAL ELECTRICAL CODE OT FUSED WNER FURNISHED CONTRAC				(LETTER INDICATES SENSOR TYPE) REFER TO LIGHTING CONTROLS SCHEDULE LOW-VOLTAGE CONTROL STATION	CEILING	+	BOTTOM OUTLET SWITCHED.	WALL - 18" AFF
			PNL PH O	- PAI	ANEL HASE			⊬ Cx	(LETTER INDICATES CONTROL STATION (LETTER INDICATES CONTROL STATION TYPE) REFER TO LIGHTING CONTROLS SCHEDULE	WALL - 48" AFF TO TOP OF JUNCTION BOX	\ \	SPECIAL PURPOSE RECEPTACLE REFER TO PLANS FOR NEMA CONFIGURATION	WALL - 18" AFF OR CEILING
			P PVC		DLYVINYL CHLORIDE			PCX	PHOTOCELL SENSOR (LETTER INDICATES SENSOR TYPE) REFER TO LIGHTING CONTROLS SCHEDULE	FIELD VERIFY	₩USB	NEMA 5-20R - DUPLEX RECEPTACLE WITH USB PORTS SIMILAR TO HUBBELL #USB20AC5W	WALL - 18" AFF
			RF RG SPD	RAI	EFRIGERATOR ANGE JRGE PROTECTIVE DEVICE			PP	POWERPACK (LETTER INDICATES POWERPACK TYPE)	ACCESSIBLE CEILING	- •	NEMA 5-20R DUPLEX RECEPTACLE MOUNTED ON CEILING	CEILING - FLUSH
			T TTB	TAN	AMPERPROOF RECEPTACLE ELEPHONE TERMINAL BOARD			COMM	REFER TO LIGHTING CONTROLS SCHEDULE 1UNICATION/LOW-VOLTAGE		FB1	HUBBELL CFB4 SERIES FLOOR BOX (OR EQUAL) WITH (2) DUPLEX RECEPTACLES AND DATA/COMMUNICATION	FLOOR - FLUSH
			TV UC	UN	ELEVISION RECEPTACLE NDERCOUNTER REFRIGERAT	,		SYMBOL	DESCRIPTION	MOUNTING		CONNECTION CAPABILITY HUBBELL B24 SERIES FLOOR BOX (OR EQUAL)	
			UL U.N.C	.O. UN	NDERWRITERS LABORATORI NLESS NOTED OTHERWISE OI TS	. <u>S</u>		CR	CARD READER (VERIFY EXACT REQUIREMENTS)		FB2	WITH (1) DUPLEX RECEPTACLE AND DATA/COMMUNICATION CONNECTION CAPABILITY HUBBELL B24 SERIES FLOOR BOX (OR EQUAL)	FLOOR - FLUSH
			VA VD	VO	OLT-AMPERES ENDING MACHINE (24"AFF)				DATA, TELEPHONE, OR COMBO TELE/DATA OUTLET	WALL - 18" AFF	FB3	FOR POWER AND DATA CONNECTIONS TO PRE-WIRED FURNITURE VERIFY EXACT CONNECTION WITH FURNITURE VENDOR	FLOOR - FLUSH
			VFD W	WA	ARIABLE FREQUENCY DRIVE				PROVIDE PULLSTRING IN CONDUIT TO ACCESSIBLE CEILING DATA, TELEPHONE, OR COMBO TELE/DATA OUTLET	FLOOR OR		HUBBELL S1PT SERIES 4" POKE-THRU (OR EQUAL) WITH (2) DUPLEX RECEPTACLES AND DATA/COMMUNICATION CONNECTION CAPABILITY	FLOOR - FLUSH
			WA WD	WA	ASHER ARMING DRAWER ALL OVEN				PROVIDE PULLSTRING IN CONDUIT TO ACCESSIBLE CEILING	CEILING		HUBBELL S1PTFF SERIES 4" POKE-THRU (OR EQUAL) FOR POWER AND DATA CONNECTIONS TO PRE-WIRED FURNITURE VERIFY EXACT CONNECTION WITH FURNITURE VENDOR	FLOOR - FLUSH
			WP WP/W	WE	EATHERPROOF EATHERPROOF/WEATHER RE	ESISTANT			TELEVISION OUTLET	WALL OR CEILING		HUBBELL S1R6 SERIES 6" POKE-THRU (OR EQUAL) WITH (2) DUPLEX RECEPTACLES AND DATA/COMMUNICATION AND	FLOOR - FLUSH
			W/UN	JNIT DIS	SCONNECT IS SUPPLIED WIT	H THE UNIT			SPEAKER OUTLET	FIELD VERIFY		A/V CONNECTION CAPABILITY CONDUIT IN OR UNDER FLOOR/GRADE	
			GI	ENEF	RAL ELE	CTRICAL	NOTES	'TTB'	TELEPHONE TERMINAL BOARD	WALL			
					LATION SHALL BE IN ACCORI F THE AHJ AND ALL LOCAL &	DANCE WITH THE NATIONAL ELECTR	RICAL CODE,		SECURITY CAMERA OUTLET	FIELD VERIFY		CONDUCTOR HOME RUN - () HOT, () NUETRAL, () EQUIPMENT GROUND, & () ISOLATED GROUND	
			2. D 3. R	DO NOT SCALE FRO REFER TO ARCHITE	OM THESE DRAWINGS. ECTURAL DRAWINGS AND RE	EFLECTED CEILING PLANS FOR EXAC	CT LOCATIONS OF ALL	•	PUSH BUTTON			EQUIPMENT CONNECTION	
			4. A 5. A	ALL EMPTY CONDUI	KES SHALL HAVE A COVER.	TH PULL STRINGS AND BUSHINGS.		모 	, 33, 33 , 33			CONDUIT IN CEILING OR WALL	
			7. A		UITS WITHOUT A CONDUCTO	ION(S) W/ ACTUAL CEILING TO BE FUI R & CONDUIT INDICATED SHALL BE F		NOTE: NOT ALL SYN					
			В	BE SMALLER THAN 3	3/4"C. UNLESS SPECIFICALL	OT BE SMALLER THAN #12 AWG AND A OY NOTED OTHERWISE. BE PROVIDED WITH DEDICATED NEUT		FINISHED CO IF NOT MOUN	ABOVE A COUNTER, DEVICE TO BE WALL MOUNTED 6" ABOVE UNTER OR 44" TO TOP OF JUNCTION BOX (WHICHEVER IS LOWER). ITED ABOVE A COUNTER, DEVICE TO BE WALL MOUNTED AT				
			C P	OTHERWISE. WHER PROVIDED WITH 2P	RE NEUTRALS ARE INDICATE POR 3P BREAKERS AS REQU	ED TO BE SHARED, MULTIWIRE BRAN IIRED PER NEC210.4.	NCH CIRCUITS SHALL BE		OP OF JUNCTION BOX AS REQUIRED TO MEET ADA REQUIREMENTS. RCHITECTURAL DRAWINGS FOR EXACT LOCATIONS/ELEVATIONS.				
			O G	CONDUCTOR SIZED GROUND.	D IN ACCORDANCE WITH THE	SE PROVIDED WITH AN INSULATED EC ENEC. THE RACEWAY SHALL NOT BE	E USED AN EQUIPMENT						
			V	WITH 12 GAUGE WIF	IRE CONNECTED TO STRUCT	ACH CORNER (INDEPENDENT OF THE TURAL SYSTEM OF BUILDING. THE INS LOCAL AND NATIONAL CODES.							
			Р	ELECTRICAL DEVICE PANELBOARDS SWITCHES		D: TOP OF PANEL TOP OF JUNCTION BOX							
			R T	RECEPTACLES TELE/DATA OUTLET:	18" AFF TO C TS 18" AFF TO C	CENTER OF RECEPTACLE CENTER OF RECEPTACLE	LOCAL INSPECTORY						
			13. E S	SCALE ON THE FLOO	PMENT (PANELBOARDS, TRAN DOR PLANS.	17.1 REQUIREMENTS (VERIFY WITH L NSFORMERS, DISTRIBUTION EQUIPM	MENT, ETC.) IS SHOWN TO						
			S	SPACES SHOWN ON	N THE PLANS AND COMPLYIN	LE FOR PROVIDING EQUIPMENT THA NG WITH ALL CODE REQUIRED CLEAF CES (RECEPTACLES, SWITCHES, PAN	ARANCES.						
			D A	DISCONNECTS, ETC AND SIMILAR DEVIC	C.) WITH CIRCUIT NUMBER AN CES TO HAVE PRE-PRINTED,	ND PÀNELBOARD DÉSIGNATION. RE	ECEPTACLES, SWITCHES,						
			L	LAMINATED ACRYLIC	IC LABEL (BLACK W/ WHITE L		· ·						





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CHAFL MOORES MO Architect #2009032812

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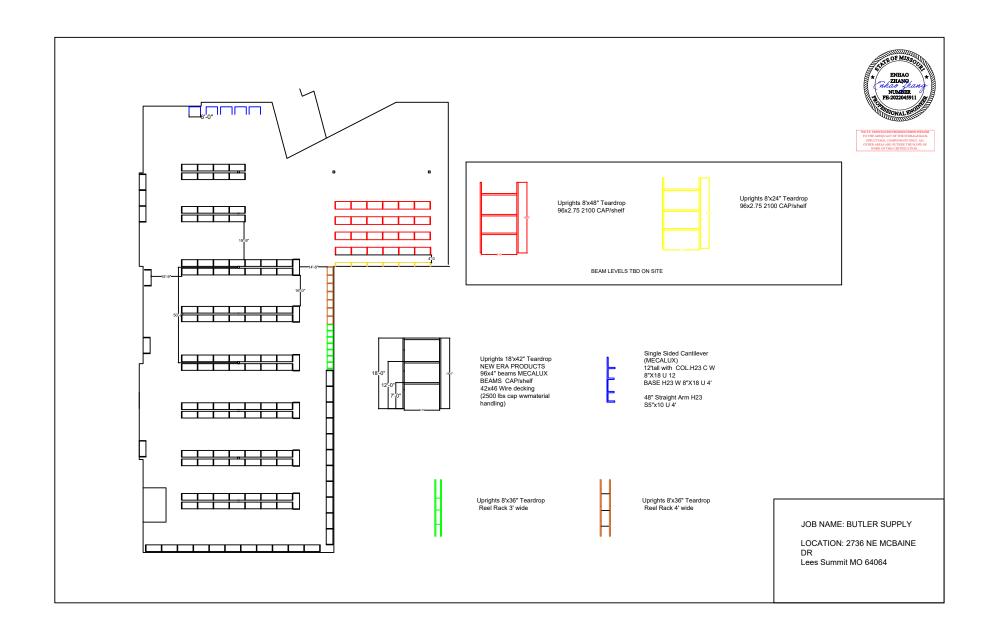
Development Services Department Lee's Summit, Missouri

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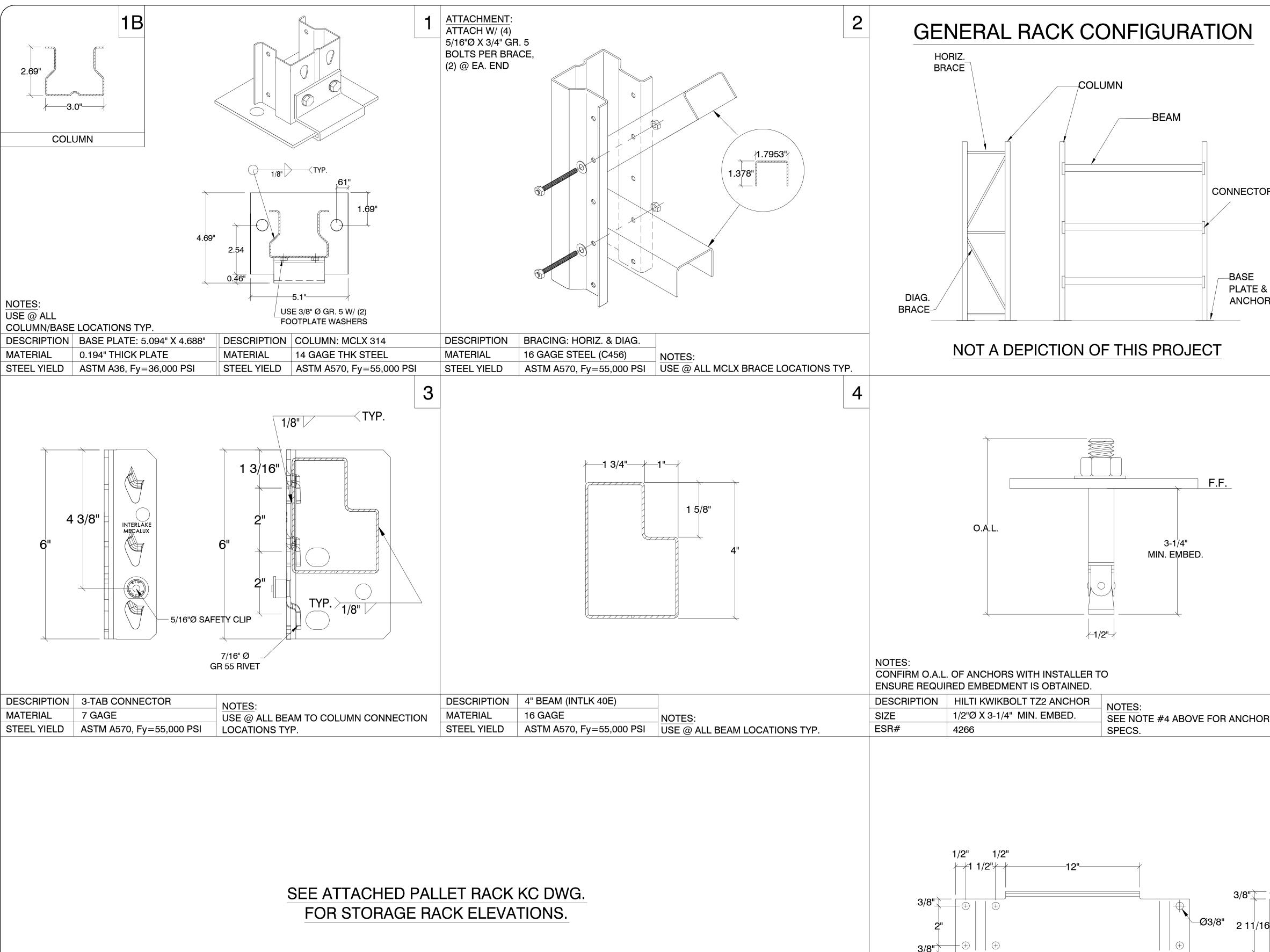
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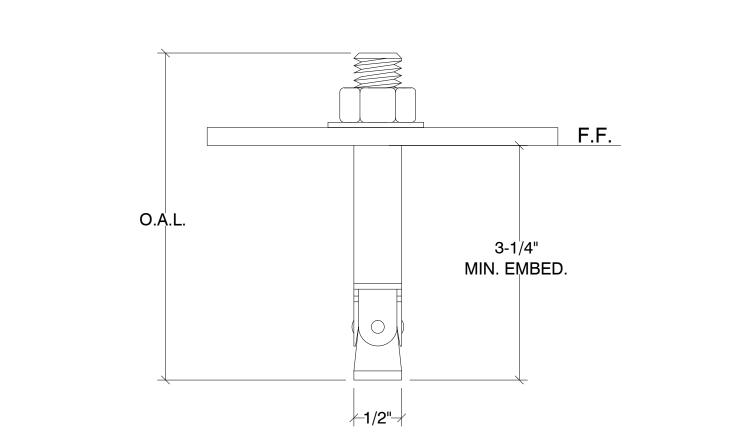


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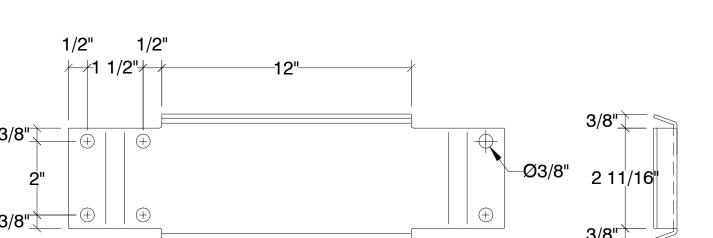


DESCRIPTION | STORAGE RACK ELEVATIONS

CONNECTOR PLATE & ANCHOR



DESCRIPTION	HILTI KWIKBOLT TZ2 ANCHOR	NOTES:
SIZE	1/2"Ø X 3-1/4" MIN. EMBED.	SEE NOTE #4 ABOVE FOR ANCHOR
ESR#	4266	SPECS.



USES (3) ROW SPACERS FOR DOUBLE ROW UNITS

(0)	0.7.02.00.00.00.00.00.00.00.00.00.00.00.00.	
DESCRIPTION	STD ROW SPACER	NOTES:
MATERIAL	14 GAGE	ATTACH WITH (4) 5/16"Ø GR. 5 BOLTS,
STEEL YIELD	ASTM A570. Fv=55.000 PSI	(2) @ EACH END

GENERAL PROJECT NOTES

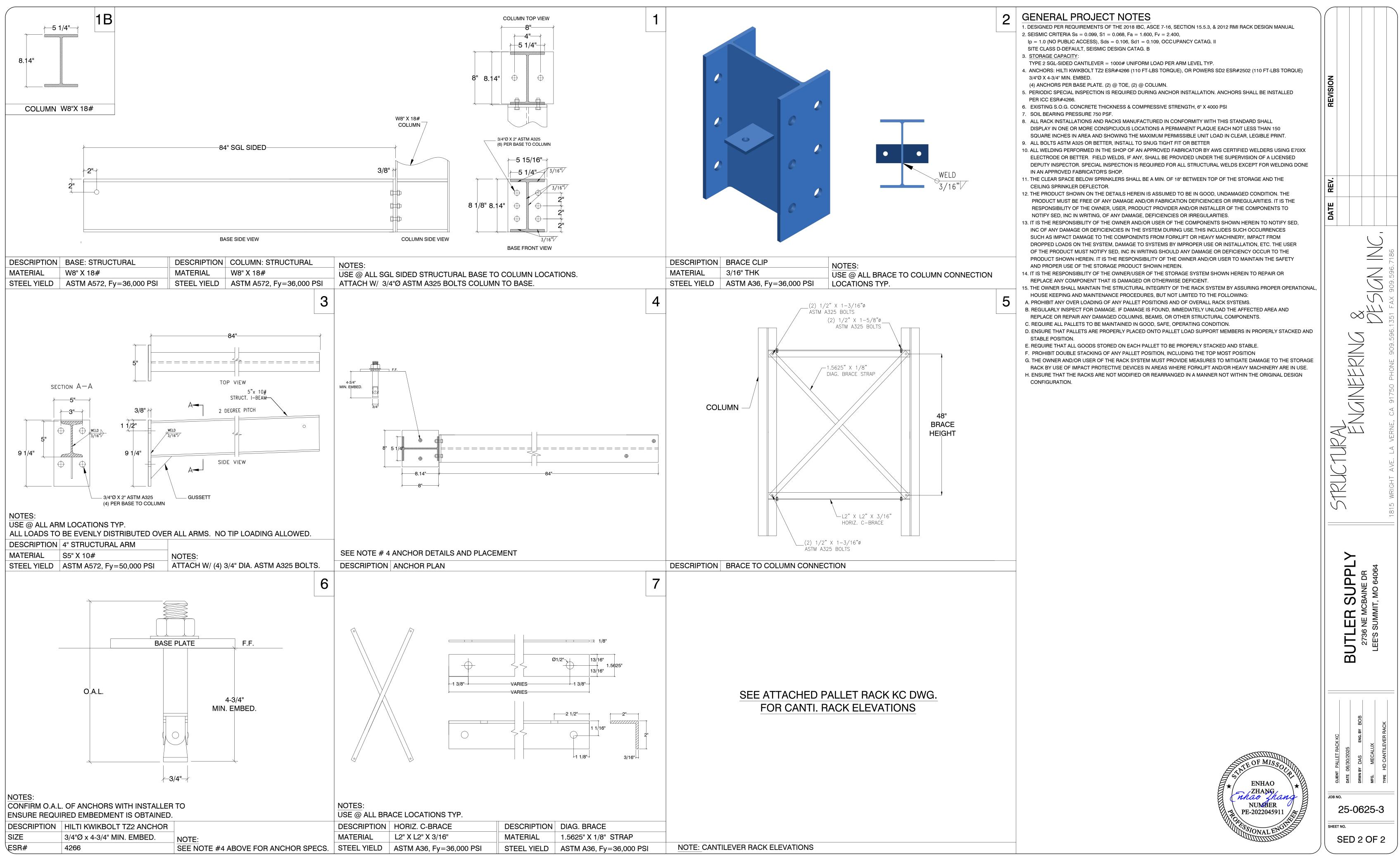
- 1. DESIGNED PER REQUIREMENTS OF THE 2018 IBC, ASCE 7-16 AND THE 2012 RMI RACK DESIGN MANUAL 2. SEISMIC CRITERIA Ss = 0.099, S1 = 0.068, Fa = 1.600, Fv = 2.400
- Ip = 1.0 (NO PUBLIC ACCESS), Sds = 0.106, Sd1 = 0.109, OCC. CATAG. II,
- SITE CLASS D-DEFAULT, SEISMIC DESIGN CATAG. B
- 3. STORAGE CAPACITY: TYPE 1 SELECTIVE RACK = 3000 LBS PER LEVEL TYP.
- 4. ANCHORS: HILTI KWIKBOLT TZ2 ESR #4266 (50 FT-LBS TORQUE), OR POWERS SD2 ESR#2502 (40 FT-LBS TORQUE) 1/2" Ø x 3-1/4" MIN. EMBEDT. (1) ANCHOR PER BASE PLATE
- 5. PERIODIC SPECIAL INSPECTION IS REQUIRED DURING ANCHOR INSTALLATION. ANCHORS SHALL BE INSTALLED PER ICC ESR#4266.
- 6. EXISTING S.O.G. CONCRETE THICKNESS & COMPRESSIVE STRENGTH, 6" x 4000 PSI
- 7. SOIL BEARING PRESSURE 750 PSF.
- 8. ALL RACK INSTALLATIONS AND RACKS MANUFACTURED IN CONFORMITY WITH THIS STANDARD SHALL DISPLAY IN ONE OR MORE CONSPICUOUS LOCATIONS A PERMANENT PLAQUE EACH NOT LESS THAN 50 SQUARE
- INCHES IN AREA AND SHOWING THE MAXIMUM PERMISSIBLE UNIT LOAD IN CLEAR, LEGIBLE PRINT.
- 9. ALL BOLTS GR. 5 OR BETTER, INSTALL TO SNUG TIGHT FIT OR BETTER
- 10. ALL WELDING PERFORMED IN THE SHOP OF AN APPROVED FABRICATOR BY AWS CERTIFIED WELDERS USING E70XX ELECTRODE OR BETTER. NO FIELD WELDING PERFORMED. SPECIAL INSPECTION IS REQUIRED ONLY FOR ANY FIELD
- 11. THE CLEAR SPACE BELOW SPRINKLERS SHALL BE A MIN. OF 18" BETWEEN TOP OF THE STORAGE AND THE CEILING SPRINKLER DEFLECTOR.
- 12. THE PRODUCT SHOWN ON THE DETAILS HEREIN IS ASSUMED TO BE IN GOOD, UNDAMAGED CONDITION. THE PRODUCT MUST BE FREE OF ANY DAMAGE AND/OR FABRICATION DEFICIENCIES OR IRREGULARITIES. IT IS THE RESPONSIBILITY OF THE OWNER, USER, PRODUCT PROVIDER AND/OR INSTALLER OF THE COMPONENTS TO NOTIFY SED, INC IN WRITING, OF ANY DAMAGE, DEFICIENCIES OR IRREGULARITIES.
- 13. IT IS THE RESPONSIBILITY OF THE OWNER AND/OR USER OF THE COMPONENTS SHOWN HEREIN TO NOTIFY SED, INC OF ANY DAMAGE OR DEFICIENCIES IN THE SYSTEM DURING USE. THIS INCLUDES SUCH OCCURRENCES SUCH AS IMPACT DAMAGE TO THE COMPONENTS FROM FORKLIFT OR HEAVY MACHINERY, IMPACT FROM DROPPED LOADS ON THE SYSTEM, DAMAGE TO SYSTEMS BY IMPROPER USE OR INSTALLATION, ETC. THE USER OF THE PRODUCT MUST NOTIFY SED, INC IN WRITING SHOULD ANY DAMAGE OR DEFICIENCY OCCUR TO THE PRODUCT SHOWN HEREIN. IT IS THE RESPONSIBILITY OF THE OWNER AND/OR USER TO MAINTAIN THE SAFETY AND PROPER USE OF THE STORAGE PRODUCT SHOWN HEREIN.
- 14. IT IS THE RESPONSIBILITY OF THE OWNER/USER OF THE STORAGE SYSTEM SHOWN HEREIN TO REPAIR OR
- REPLACE ANY COMPONENT THAT IS DAMAGED OR OTHERWISE DEFICIENT. 15.THE OWNER SHALL MAINTAIN THE STRUCTURAL INTEGRITY OF THE RACK SYSTEM BY ASSURING PROPER OPERATIONAL, HOUSE KEEPING AND MAINTENANCE PROCEDURES, BUT NOT LIMITED TO THE FOLLOWING:
- a. PROHIBIT ANY OVER LOADING OF ANY PALLET POSITIONS AND OF OVERALL RACK SYSTEMS. b. REGULARLY INSPECT FOR DAMAGE. IF DAMAGE IS FOUND, IMMEDIATELY UNLOAD THE AFFECTED AREA AND REPLACE OR REPAIR ANY DAMAGED COLUMNS, BEAMS, OR OTHER STRUCTURAL COMPONENTS. c. REQUIRE ALL PALLETS TO BE MAINTAINED IN GOOD, SAFE, OPERATING CONDITION.
- d. ENSURE THAT PALLETS ARE PROPERLY PLACED ONTO PALLET LOAD SUPPORT MEMBERS IN PROPERLY STACKED AND STABLE POSITION.
- e. REQUIRE THAT ALL GOODS STORED ON EACH PALLET TO BE PROPERLY STACKED AND STABLE. f. PROHIBIT DOUBLE STACKING OF ANY PALLET POSITION, INCLUDING THE TOP MOST POSITION, UNLESS THE RACK SYSTEM IS SPECIFICALLY DESIGNED FOR SUCH LOADING.
- g. THE OWNER AND/OR USER OF THE RACK SYSTEM MUST PROVIDE MEASURES TO MITIGATE DAMAGE TO THE STORAGE RACK BY USE OF IMPACT PROTECTIVE DEVICES IN AREAS WHERE FORKLIFT AND/OR HEAVY MACHINERY ARE IN USE.

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Development Services Department Lee's Summit, Missouri 07/13/2025

Structural Engineering & Design, Inc.

1815 Wright Ave La Verne, CA 91750 Phone: 909.596.1351 Fax: 909.596.7186

Project Name: BUTLER SUPPLY

Project Number: 25-0625-3

Date: 07/01/25

Street Address: 2736 NE MCBAINE DR City/State: LEES SUMMIT, MO 64064

Scope of Work: STORAGE RACK



Engineering & Design Inc.



Development Services Department Lee's Summit, Missouri 07/13/2025

1815 Wright Ave La Verne, CA 91750 Tel; 909,596,1351 Fax: 909,596,7186 By: Bob S Project: **Butler Supply** Project #: 25-0625-3 **TABLE OF CONTENTS** Title Page 1 Table of Contents..... 2 Design Data and Definition of Components 3 Critical Configuration Seismic Loads 5 to 6 7 Column Beam and Connector 8 to 9 Bracing 10 Anchors 11 Base Plate 12 Slab on Grade 13 14 to 24 Other Configurations

CONSTRUCTION As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

Engineering & Design Inc.

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By: Bob S Project: **Butler Supply** Project #: 25-0625-3

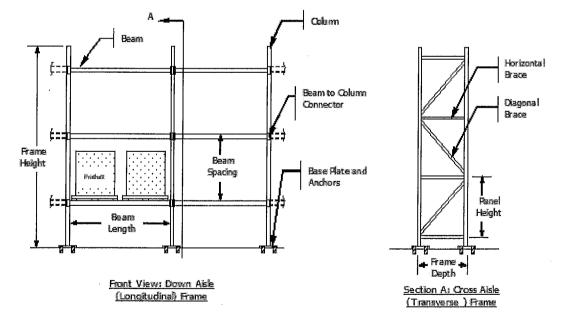
Design Data

1) The analyses herein conforms to the requirements of the: 2021 IBC Section 2209

ANSI MH 16.1-2012 Specifications for the Design of Industrial Steel Storage Racks "2012 RMI Rack Design Manual" ASCE 7-16, section 15.5.3

- 2) Transverse braced frame steel conforms to ASTM A570, Gr.55, with minimum strength, Fy=55 ksi Longitudinal frame beam and connector steel conforms to ASTM A570, Gr.55, with minimum yield, Fy=55 ksi All other steel conforms to ASTM A36, Gr. 36 with minimum yield, Fy= 36 ksi
- 3) Anchor bolts shall be provided by installer per ICC reference on plans and calculations herein.
- 4) All welds shall conform to AWS procedures, utilizing E70xx electrodes or similar. All such welds shall be performed in shop, with no field welding allowed other than those supervised by a licensed deputy inspector.
- 5) The existing slab on grade is 6" thick with minimum 4000 psi compressive strength. Allowable Soil bearing capacity is 750 psf. The design of the existing slab is by others.
- 6) Load combinations for rack components correspond to 2012 RMI Section 2.1 for ASD level load criteria

Definition of Components



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As Noted on Plans Review

Development Services Department Lee's Summit, Missouri

07/13/2025

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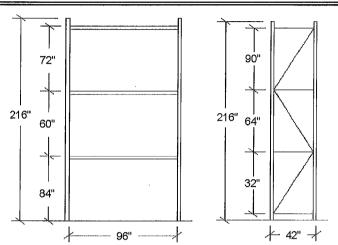
1815 Wright Ave La Verne, CA 91750 Tel: 909.596.1351 Fax: 909.596.7186

By: Bob S

Project: Butler Supply

Project #: 25-0625-3

Configuration & Summary: TYPE 1 SELECTIVE RACK



**RACK COLUMN REACTIONS

UNFACTORED LOAD

AXIAL D= 75 |b

AXIAL P= 4,500 |b

AXIAL S= |b

SEISMIC AXIAL PS=+/- 441 |b

BASE MOMENT= 0 in-lb

Seismic Criteria	# Bm Lvls	Frame Depth	Frame Height	# Diagonals	Beam Length	Frame Type
Ss=0.099, Fa=1.6	3	42 in	216.0 in	3	96 in	Single Row

Compo	nent	Description							STRESS
Colur	nn	Fy=55 ksi	Mecal	ux 314 3.0"x2.69	14 3.0"x2.69"x0.070" P=45			75 lb, M=4415 in-lb	
Column &	Backer	None		None					N/A
Bear	n	Fy≕55 ksi	Intlk 4	Intlk 40E 4Hx2.75Wx0.059"Thk Lu=96 in				5164 lb/pr	0.58-OK
Beam Cor	nector	Fy=55 ksi	Lvl 1:	Lvl 1: 3 Tab OK Mconn=3521 in-lb Mcap=8828 in-lb			0,4-OK		
Brace-Hor	izontal	Fy=50 ksi		McIx C456 Sgl 1.7953x1.378x16ga(U31x)					0.04-OK
Brace-Dia	agonal	Fy=50 ksi		Mclx C456 Sgl 1.7953x1.378x16ga(U31x)					0.39-OK
Base P	late	Fy=36 ksi		5.09x4.66x0.194 Fixity= 0 in-lb					0.52-OK
Anch	or	1 per Base	0.5" x 3.2	25" Embed HILTI	TZ2 ESR 4266 Ir	nspection Reqd	(Net Seismic U	olift=0 lb)	0.025-OK
Slab &	Soil		6" thk >	x 4000 psi slab o	n grade. 750 psf	Soil Bearing Pro	essure		0.27-OK
Level	Load**			Story Force	Story Force	Column	Column	Conn.	Beam
	Per Level	Beam Spcg	Brace	Transv	Longit.	Axial	Moment	Moment	Connector
1	3,000 lb	84.0 in	32.0 in	31 lb	21 lb	4,575 lb	4,415 "#	3,521 "#	3 Tab OK
2	3,000 lb	60.0 in	64.0 in	53 lb	35 lb	3,050 lb	1,326 "#	2,310 "#	3 Tab OK
3	3,000 lb	72.0 in	90.0 in	79 lb	53 lb	1,525 lb	954 "#	1,846 "#	3 Tab OK

* Load defined as product weight per pair of beams	Total:	163 lb	109 lb		
Notes				 	
W-1-1-1					

CONSTRUCTION

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

Engineering & Design Inc.

Bv:	Bob S	Vright Ave L Project:	· · · · · · · · · · · · · · · · · · ·			Project #: 2	5-0625-3
Seismic Forces	Configuration: T			/ 		FTOJECU #. 2	3-0020-3
	ormed with regard to the r		10.0	4U 16 1 2012 Con 2 6	9. ACCE 7 16 per 15 F 2		C- 0.000
			e 2012 KMI ANSI I	ALL 10:1-5015 Sec 5:0	& ASCE 7-16 SEC 15,5.3		Ss= 0.099
	ss Aisle) Seismic Lo V= Cs*Ip*Ws=Cs*		-f - D - A 26\		Vŧ Vŧ		S1= 0.068
	-	Tb(n.e)b	/H+D+0.25)				Fa= 1.600
Cs	s1= Sds/R	*	G . W.	0.0004			Fv= 2.400
0.	= 0.0264		Cs-max * Ip=			Sds=2/3*Ss*	
CS	2= 0.044*Sds	Ecc n.		0.015		Sd1=2/3*S1*	FV= 0.109
C	= 0.0046	EIT B	ase Shear=Cs=	0.0264 (0.67*PL _{RF1} * PL)	Transverse Elevation		
Cs	3= 0.5*S1/R				TDLT0.25 (Transverse, Braced Frame Dir	
Co. mr	= 0.0085 ax = 0.0264			6,180 lb	b + 6030 lb + 0.2		Ip= 1.0
Base Shear Coeff=			Etransverse=	_	D + 0030 ID + 0.2		RF1= 1.0
base Shear Coen=	us= 0.0264		1	105 ID vel Transverse seism	in ahans nas unvisibit	Pallet Height=	
Loval	PRODUCT LOAD P	P*0.67*P _{RF1}				DL per Beam L	
Level .	3,000 lb	2,010 lb	DL 50 lb	hi 84 in	wi*hi 173.040	Fi 30,8 lb	Fi*(hi+hp/2)
2	3,000 lb	2,010 lb	50 lb	84 m 144 in	173,040 296,640	52.9 lb	2,587-# 7,618-#
3	3,000 lb	2,010 lb	50 lb	216 in	444,960	79.3 lb	7,010-# 17,129-#
Ų.	3,000 ID	Σ'ΩΤΩ ID	di 0	210 111	444, 960 0	7 3.3 10	1/,125-#
			0 lb		0		
			0 lb		0		•
			0 lb		0		
			0 lb		0		
			0 lb		.0		
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			0 lb		0		
			0 lb		0		
			0 lb		0		
			0 lb		0		
SU	m: P=9000 lb	6,030 lb	150 lb	W=6180 lb	914,640	163 lb	Σ=27,334
	wnaisle) Seismic L	•			,		
	sismic loads, using R=6.0		(0.67 * PL _{RF2} *	P) + DL+0.2S	P _{RF2} = 1,	0	
Cs	1= 0.0176		6,180 lb		udinal, Unbraced Dir.) R= 6.	0	Vland
		Cs=Cs-max*Ip=	•			75 sec	Yeng
	3= 0.0057			50 lb + 6030 lb +			
		_	-		seismic shear per upright		
Level	PRODUC LOAD P		DL	hi	wi*hi	Fi	Front View
1	3,000 lb	2,010 lb	50 lb	84 in	173,040	20.6 lb	
2	3,000 lb	2,010 lb	50 lb	144 in	296,640	35.4 lb	•
3	3,000 lb	2,010 lb	50 lb	216 in	444,960	53.0 lb	
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As Noted on Plans Review

CONSTRUCTION

Development Services Department Lee's Summit, Missouri

07/13/2025

Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909.596.1351 Fax: 909.596.7186

By: Bob S

Project: **Butler Supply** Project #: 25-0625-3

Downaisle Seismic Loads

Configuration: TYPE 1 SELECTIVE RACK

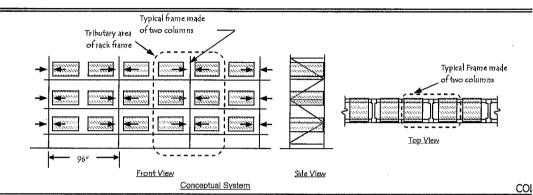
Determine the story moments by applying portal analysis. The base plate is assumed to provide no fixity.

Seismic Story Forces

Vlong= 109 lb Vcol=Vlong/2= 55 lb F1 = 21 lb

F2 = 35 lb

F3= 53 lb



h1-eff= h1 - beam clip height/2

Vcol

h2

= 81 in

Seismic Story Moments

Mbase-max= 0 in-lb

<=== Default capacity

Mbase-v= (Vcol*h1eff)/2

= 2,207 in-lb

<=== Moment going to base

Mbase-eff= Minimum of Mbase-max and Mbase-v

= 0 in-lb

PINNED BASE ASSUMED

M 1-1= [Vcol * h1eff]-Mbase-eff

= (55 lb * 81 in)-0 in-lb

= 4,415 in-lb

M 2-2= [Vcol-(F1)/2] * h2

= [55 lb - 17.7 lb]*60 in/2

= 1,326 in-lb

Mseis= (Mupper+Mlower)/2

Mseis(1-1)= (4415 in-lb + 1326 in-lb)/2

Mseis(2-2)= (1326 in-lb + 954 in-lb)/2

= 2.870 in-lb

= 1.140 in-lh

rho= 1.0000

Beam to Column Elevation

	- 2,070 HI-ID			מורווו טדבי.		1110-	1.0000		
	Summary of Forces								
LEVEL	hi	Axial Load	Column Moment**	Mseismic**	Mend-fixity	Mconn**	Beam Connector		
1	84 in	4,575 lb	4,415 in-lb	2,870 in-lb	2,160 in-lb	3,521 in-lb	3 Tab OK		
2	60 in	3,050 lb	1,326 in-lb	1,140 in-lb	2,160 in-lb	2,310 in-lb	3 Tab OK		
3	72 in	1,525 lb	954 in-lb	477 in-lb	2,160 in-lb	1,846 in-lb	3 Tab OK		

Mconn= (Mseismic + Mend-fixity)*0.70*rho

Mconn-allow(3 Pin)= 8,828 in-lb

**all moments based on limit states level loading

CONSTRUCTION

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Development Services Department Lee's Summit, Missouri 07/13/2025

Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909.596,1351 Fax: 909.596,7186

Bv: Bob S Project: **Butler Supply**

Project #: 25-0625-3

Column (Longitudinal Loads)

Configuration: TYPE 1 SELECTIVE RACK

Section Properties

Section: Mecalux 314 3.0"x2.69"x	0.070"			3,000 in ->
Aeff = 0.538 in^2	$Iy = 0.464 in^4$	Kx = 1.7		[
$Ix = 0.765 in^4$	$Sy = 0.307 \text{ in}^3$	Lx = 82.0 in	1	
$Sx = 0.510 \text{ in}^3$	ry = 0.928 in	Ky = 1.0		0.070 in
rx = 1.190 in	Fy= 55 ksi	Ly = 32.0 in	2.690 in	
$\Omega f= 1.67$	Cmx= 0.85	Cb= 1.0	1	しっし
E= 29,500 ksi				1 [
			1	

Considers loads at level 1

COLUMN DL= 75 lb Critical load cases are: RMI Sec 2.1 COLUMN PL= 4,500 lb Load Case 5: : (1+0.105*Sds)D + 0.75*(1.4+0.14Sds)*B*P + 0.75*(0.7*rho*E)<= 1.0, ASD Method Mcol= 4,414 in-lb axial load coeff: 0.7427616 * P seismic moment coeff: 0.5625 * Mcol Sds= 0.1056 Load Case 6: : (1+0.104*Sds)D + (0.85+0.14Sds)*B*P + (0.7*rho*E)<= 1.0, ASD Method 1+0.105*Sds= 1.0111 axial load coeff: 0.60535 seismic moment coeff: 0.7 * Mcol

By analysis, Load case 5 governs utilizing loads as such

0.85+0.14*Sds= 0.8648 Axial Load=Pax= 1.011088*75 lb + 0.75 * 1.414784 * 0.7 * 4500 lb Moment=Mx= 0.75*0.7*rho*Mcol B = 0.7000= 3,418 lb= 0.525*4414 in-lb rho= 1,0000 = 2,317 in-lb

Axial Analysis

1.4+0.14Sds= 1.4148

1+0.14Sds= 1.0148

KxLx/rx = 1.7*82"/1.19"KyLy/ry = 1*32"/0.9284"Fe < Fy/2 = 117.1= 34.5Fn= Fe = $\pi^2E/(KL/r)$ max^2 Fe= $\pi^2E/(KL/r)$ max^2 Fy/2 = 27.5 ksi= 21.2 ksi = 21.2ksi Pa= Pn/ΩcPn= Aeff*Fn $\Omega c = 1.92$ = 11413 lb/1.92= 11,413 lb= 5,944 lb

Bending Analysis

P/Pa=

0.58

Check: $Pax/Pa + (Cmx*Mx)/(Max*ux) \le 1.0$

 $P/Pao + Mx/Max \le 1.0$

Pno= Ae*Fy Pao= Pno/Ωc Myield=My= Sx*Fy = 29585lb/1.92 = 0.538 in^2 *55000 psi = 0.51 in^3 * 55000 psi

= 29,585 lb= 15,409 lb= 28,050 in-lb

 $Max = My/\Omega f$ Pcr= $\pi^2EI/(KL)$ max^2

= 28050 in-lb/1.67 $= \pi^2*29500 \text{ ksi}/(1.7*82 \text{ in})^2$

= 16,796 in-lb= 11,462 lb

 $\mu x = \{1/[1-(\Omega c^*P/Pcr)]\}^{-1}$ $= \{1/[1-(1.92*3418 | b/11462 | b)]\}^{-1}$

> 0.15

= 0.43**Combined Stresses**

> (3418 lb/5944 lb) + (0.85*2317 in-lb)/(16796 in-lb*0.43) =0.85 < 1.0, OK(EQ C5-1) (3418 lb/15409 lb) + (2317 in-lb/16796 in-lb) =0.36 < 1.0, OK(EQ C5-2)

** For comparison, total column stress computed for load case 6 is: 77.0% izing loads 2800.1784 lb Axial and M= 3089 in-lb

CONSTRUCTION

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

1.625 in

0.059 in

Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909.596,1351 Fax: 909.596,7186

Butler Supply By: Bob S Project: Project #: 25-0625-3

BEAM

Configuration: TYPE 1 SELECTIVE RACK

DETERMINE ALLOWABLE MOMENT CAPACITY

A) Check compression flange for local buckling (B2.1)

w = c - 2*t - 2*r

= 1.75 in - 2*0.059 in - 2*0.059 in

= 1.514 in

w/t = 25.66

 $l=lambda= [1.052/(k)^0.5] * (w/t) * (Fy/E)^0.5$

= [1.052/(4)^0.5] * 25.66 * (55/29500)^0.5

= 0.583

< 0.673, Flange is fully effective

Eq. B2.1-4

Ea. B2.1-1

OK

Eq B2.3-2

B) check web for local buckling per section b2.3

f1(comp) = Fy*(y3/y2) =

f2(tension) = Fy*(y1/y2) =101.99 ksi

Y = f2/f1

Eq. B2.3-5

= -2.03

 $k = 4 + 2*(1-Y)^3 + 2*(1-Y)$ Eq. B2.3-4

= 65.70

flat depth=w= y1+y3

= 3.764 in

w/t= 63.79661017

 $l=lambda= [1.052/(k)^0.5] * (w/t) * (f1/E)^0.5$

= [1.052/(65.7)^0.5] * 3.764 * (50.23/29500)^0.5

< 0.673

= 0.342

be=w= 3.764 in

b2 = be/2

b1 = be(3-Y)

= 1.88 in

= 0.748

Lcorner=Lc= (p/2) * (r + t/2)

b1+b2=2.628 in > 1.242 in, Web is fully effective

Determine effect of cold working on steel yield point (Fya) per section A7.2

Fya= C*Fyc + (1-C)*Fy

0.139 in

C = 2*Lc/(Lf+2*Lc)

Lflange-top=Lf= 1.514 in

= 0.155 in

m = 0.192*(Fu/Fy) - 0.068

(EQ A7.2-4)

(EQ A7.2-1)

= 0.1590

Bc= 3.69*(Fu/Fy) - 0.819*(Fu/Fy)^2 - 1.79

= 1.427

since fu/Fv= 1.18

< 1.2

and r/t=1

< 7 OK

then Fyc= Bc * Fy/(R/t) m

(EQ A7.2-2)

= 78.485 ksi

Thus, Fya-top= 58.64 ksi

(tension stress at top)

Fya-bottom= Fya*Ycg/(depth -Ycg)

= 113.84 ksi

(tension stress at bottom)

Check allowable tension stress for bottom flange

Lflange-bot=Lfb= Lbottom - 2*r*-2*t

= 2.514 in

Cbottom=Cb= 2*Lc/(Lfb+2*Lc)

= 0.100

Fy-bottom=Fyb= Cb*Fyc + (1-Cb)*Fyf

= 57.34 ksi

Fya= (Fya-top)*(Fyb/Fya-bottom)

= 29.54 ksi

if F= 0.95

Then F*Mn=F*Fya*Sx=

Beam= Intlk 40E 4Hx2.75Wx0.059"Thk Ix= 1.667 in^4 Sx= 0.783 in^3

2.75 in

1.75 in

4.000 in

Ycg= 2.640 in 0.059 in t=

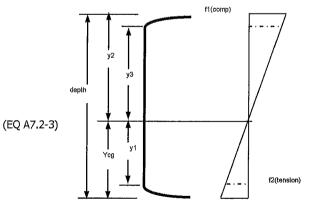
Bend Radius=r= 0.059 in

Fy=Fyv= 55.00 ksi Fu=Fuv= 65.00 ksi

E= 29500 ksi top flange=b= 1.750 in

bottom flange= 2.750 in

Web depth= 4.000 in



v1= Yca-t-r= 2.522 in

y2= depth-Ycg= 1.360 in

y3= y2-t-r= 1.242 in

Development Services Department Lee's Summit, Missouri 07/13/2025

Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909.596.1351 Fax: 909.596.7186

By: Bob S Project: Butler Supply Project #: 25-0625-3

Configuration: TYPE 1 SELECTIVE RACK

RMI Section 5.2, PT II

Section

BEAM

Beam= Intlk 40E 4Hx2,75Wx0.059"Thk

Ix=Ib= 1.667 in^4 Sx= 0.783 in^3

t= 0.059 in Fy=Fyv= 55 ksi

> P=Product Load= 3,000 lb/pair D=Dead Load= 50 lb/pair

E= 29500 ksi

1. Check Bending Stress Allowable Loads

Mcenter=F*Mn= W*L*W*Rm/8

W= 1.599

Rm= 1 - [(2*F*L)/(6*E*Ib + 3*F*L)] 1 - (2*225*96 in)/[(6*29500 ksi*1.667 in^3)+(3*225*96 in)] = 0.88

= 0.88 if F = 0.95

Then F*Mn=F*Fya*Sx= 43.59 in-k

Thus, allowable load

per beam pair=W= F*Mn*8*(# of beams)/(L*Rm*W)

= 43.59 in-k * 8 * 2/(96in * 0.88 * 1.599)

= 5,164 lb/pair allowable load based on bending stress

Mend= W*L*(1-Rm)/8

= (5164 lb/2) * 96 in * (1-0.88)/8

= 3,718 in-lb

@ 5164 lb max allowable load

= 2,160 in-lb

@ 3000 lb imposed product load

2. Check Deflection Stress Allowable Loads

Dmax= Dss*Rd

Rd = 1 - (4*F*L)/(5*F*L + 10*E*Ib)

= 1 - (4*225*96 in)/[(5*225*96 in)+(10*29500 ksi*1.667 in^4)]

= 0.856 in

if Dmax= L/180

Based on L/180 Deflection Criteria

and Dss= 5*W*L^3/(384*E*Ib)

 $L/180 = 5*W*L^3*Rd/(384*E*Ib*# of beams)$

solving for W yields,

W= 384*E*I*2/(180*5*L^2*Rd)

 $= 384*1.667 in^4*2/[180*5*(96 in)^2*0.856)$

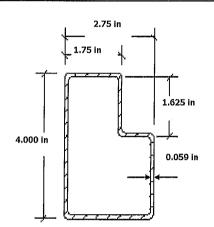
= 5,319 lb/pair allowable load based on deflection limits

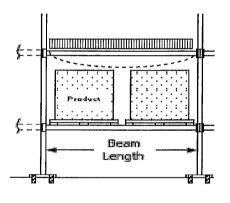
Thus, based on the least capacity of item 1 and 2 above:

Allowable load= 5,164 lb/pair Imposed Product Load= 3,000 lb/pair

Beam Stress= 0.58

Beam at Level 1





Allowable Deflection= L/180

Deflection at imposed Load= 0.310 in

= 0.533 in

Engineering & Design Inc.

RELEASED FOR CONSTRUCTION

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

1815 Wright Ave La Verne, CA 91750 Tel: 909.596,1351 Fax: 909.596,7186

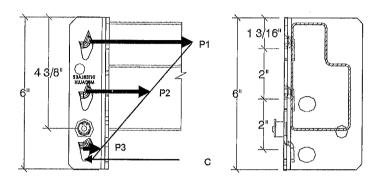
By: Bob S Project: Butler Supply Project #: 25-0625-3

3 Tab Beam to Column Connection

Configuration: TYPE 1 SELECTIVE RACK

Mconn max= (Mseismic + Mend-fixity)*0.70*Rho = 3,521 in-lb Load at level 1

Connector Type= 3 Tab



Shear Capacity of Tab

Tab Length= 0.50 in

Fv= 55,000 psi

Ashear= 0.5 in * 0.135 in = 0.0675 in^2

Pshear= 0.4 * Fy * Ashear

= 0.4 * 55000 psi * 0.0675in^2

= 1,485 lb

Bearing Capacity of Tab

tcol= 0.070 in

Fu= 65,000 psi

Bearing Length= 0.5000 in

Omega= 2.22

a = 2.22

Pbearing= alpha * Fu * tab length * tcol/Omega

= 2.22 * 65000 psi * 0.5 in * 0.07 in/2.22

= 2,275 lb

> 1485 lb

Moment Capacity of Bracket

Edge Distance=E= 1,00 in

Tab Spacing= 2.0 in

Fy= 55,000 psi

C= P1+P2+P3

= P1+P1*(2.5"/4.5")+P1*(0.5"/4.5")

tclip= 0.135 in

Sclip= 0.183 in^3

= P1+P1*(2.5")= 1.667 * P1

Mcap= Sclip * Fbending

C*d = Mcap = 1.667

d = E/2= 0.50 in

 $= 0.1832 \text{ in}^3 * 0.66 * \text{Fy}$ = 6,650 in-lb

Pclip= Mcap/(1.667 * d)

= 6650.16 in-lb/(1.667 * 0.5 in)

Thus, P1= 1,485 lb

= 7,979 lb

Mconn-allow= [P1*4.5"+P1*(2.5"/4.5")*2.5"+P1*(0.5"/4.5")*0.5"]

= 1485 LB*[4.5"+(2.5"/4.5")*2.5"+(0.5"/4.5")*0.5"]

= 8,828 in-lb

> Mconn max, OK

Stress= 0.4

CONSTRUCTION

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Development Services Department Lee's Summit, Missouri 07/13/2025

Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909.596.1351 Fax: 909.596.7186

By: Bob S

Project: **Butler Supply** Project #: 25-0625-3

Transverse Brace

Configuration: TYPE 1 SELECTIVE RACK

Section Properties

Diagonal Member = Mclx C456 Sgl 1.7953x1.378x16ga(U31x)

Area = 0.259 in^2 r min= 0.449 in

Fy= 50,000 psi

K = 1.0 $\Omega c = 1.92$

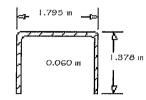
1.378 m 0.060 in

Horizontal Member= Mclx C456 Sgl 1.7953x1.378x16ga(U31x)

Area = 0.259 in^2

r min= 0.449 in Fy= 50,000 psi

K = 1.0



Frame Dimensions

Bottom Panel Height=H= 90.0 in

Frame Depth=D= 42.0 in

Column Width=B= 2.7 in

Clear Depth=D-B*2= 36.6 in

X Brace= NO

rho= 1,30

Diagonal Member

Load Case 6: : (1+0.104*Sds)D + [(0.85+0.14Sds)*B*P + [0.7*rho*E]<= 1.0, ASD Method

Vtransverse= 163 lb

Vb=Vtransv*0.7*rho= 163 lb * 0.7 * 1.3

= 148 lb

Ldiag= $[(D-B*2)^2 + (H-6")^2]^1/2$

= 91.6 in

Pmax= V*(Ldiag/D)

= 371 lb

axial load on diagonal brace member

Pn= AREA*Fn

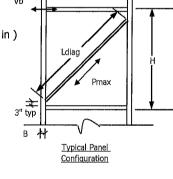
= 0.259 in^2 * 6996 psi

= 1,810 lb

(kl/r)=(k*Ldiag)/r min $= (1 \times 91.6 \text{ in } / 0.449 \text{ in })$ = 204.0 inFe= $pi^2*E/(kl/r)^2$ = 6,996 psi Since Fe<Fy/2,

Fn= Fe

= 6,996 psi



Pallow= Pn/Ω

= 1810 lb / 1.92

= 943 lb

Pn/Pallow=

0.39

<= 1.0 OK

Horizontal brace

Vb=Vtransv*0.7*rho= 148 lb

(kl/r)=(k*Lhoriz)/r min

 $= (1 \times 42 \text{ in}) / 0.449 \text{ in}$

= 93.5 in

Since Fe>Fy/2, Fn=Fy*(1-fy/4fe) = 31,233 psi

Fe= $pi^2*E/(kl/r)^2$

= 33,304 psi

Pn= AREA*Fn

= 0.259in^2*31233 psi

= 8,080 lb

Fy/2= 25,000 psi

Pallow= Pn/Ωc

= 8080 lb / 1.92

= 4,208 lb

Pn/Pallow=

0.04

<= 1.0 OK

CONSTRUCTION

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909.596.1351 Fax: 909.596.7186

By: Bob S Project: Butler Supply Project #: 25-0625-3

Single Row Frame Overturning

Configuration: TYPE 1 SELECTIVE RACK

Loads

Critical Load case(s):

1) RMI Sec 2.2, item 7: (0.9-0.2Sds)D + (0.9-0.20Sds)*B*Papp - E*rho

Vtrans=V=E=Qe= 163 lb

DEAD LOAD PER UPRIGHT=D= 150 lb PRODUCT LOAD PER UPRIGHT=P= 9,000 lb

Papp=P*0.67= 6,030 lb

Wst LC1=Wst1=(0.87888*D + 0.87888*Papp*1)= 5,431 lb

Product Load Top Level, Ptop= 3,000 lb

DL/LvI= 50 lb

Seismic Ovt based on E, $\Sigma(Fi*hi)=18,546$ in-lb

height/depth ratio= 5.1 in

Sds= 0.1056

(0.9-0.2Sds) = 0.8789

(0.9-0.2Sds) = 0.8789

B = 1.0000

rho= 1.0000

Frame Depth=Df= 42.0 in

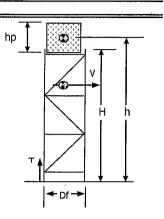
Htop-ivl=H= 216.0 in

Levels= 3

Anchors/Base= 1

hp = .0 in

h=H+hp/2= 216.0 in



SIDE ELEVATION

A) Fully Loaded Rack Load case 1:

Movt= $\Sigma(Fi*hi)*E*rho$

= 18,546 in-lb

Mst= Wst1 * Df/2

= 5431 lb * 42 in/2

= 114,051 in-lb

T= (Movt-Mst)/Df

= (18546 in-lb - 114051 in-lb)/42 in

= -2.274 lb

No Uplift

Strength Level

B) Top Level Loaded Only

Load case 1:

 \circlearrowleft V1=Vtop= Cs * Ip * Ptop >= 350 lb for H/D >6.0

= 0.0264 * 3000 lb

= 79 lb

V1eff= 79 lb

Critical Level= 3

Cs*Ip= 0.0264

V2=VDL= Cs*Ip*D

=4lb

Mst= (0.87888*D + 0.87888*Ptop*1) * 42 in/2

= 58,138 in-lb

Movt= [V1*h + V2 * H/2]*rho

Net Seismic Uplift= -4,548 lb

= 17,535 in-lb

T= (Movt-Mst)/Df

= (17535 in-lb - 58138 in-lb)/42 in

= -967 lb

No Uplift

Net Seismic Uplift= -1,934 lb

Strength Level

Anchor

Check (1) 0.5" x 3.25" Embed HILTI TZ2 anchor(s) per base plate.

Special inspection is required per ESR 4266.

Pullout Capacity=Tcap= 1,961 lb

L.A. City Jurisdiction? NO

Tcap*Phi= 1,961 lb Vcap*Phi= 2,517 lb

Shear Capacity=Vcap= 2,517 lb

Phi= 1 $(81 lb/2517 lb)^1 =$

0.03 <= 1.2 OK

Top Level Loaded:

Fully Loaded:

 $(-1934 \text{ lb}/1961 \text{ lb})^1 + (39 \text{ lb}/2517 \text{ lb})^1 =$

0.02

<= 1.2 OK

CONSTRUCTION

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

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Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909.596.1351 Fax: 909.596.7186

By: Bob S Project: **Butler Supply** Project #: 25-0625-3

Base Plate

Configuration: TYPE 1 SELECTIVE RACK

Section Baseplate = 5,09x4,66x0,194

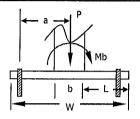
Eff Width=W = 5.09 in a = 1.55 in

Eff Depth=D = 4.66 in Anchor c.c. =2*a=d = 3.09 in Column Width=b = 3.00 in

N=# Anchor/Base= 1 Column Depth=dc = 2.69 in Fv = 36.000 psi

L = 1.05 inrho = 1

Plate Thickness=t = 0.194 in



Downaisle Elevation

Load Case 5: : (1+0.105*Sds)D + 0.75*[(1.4+0.14Sds)*B*P + 0.75*[0.7*rho*E]<= 1.0, ASD Method **Down Aisle Loads**

COLUMN DL= 75 lb Axial=P= 1.011088 * 75 lb + 0.75 * (1.414784 * 0.7 * 4500 lb)

COLUMN PL= 4,500 lb = 3,418 lb

Base Moment= 0 in-lb Mb= Base Moment*0.75*0.7*rho

1+0.105*Sds= 1.0111 = 0 in-lb * 0.75*0.7*rho

1.4+0.14Sds= 1.4148= 0 in-lb

B= 0.7000 Axial Load P = 3,418 lb Mbase=Mb=0 in-lb Effe $M1 = wL^2/2 = fa*L^2/2$

Axial stress=fa = P/A = P/(D*W)

= 144 psi = 79 in-lb

Moment Stress=fb = $M/S = 6*Mb/[(D*B^2]]$ Moment Stress=fb2 = 2 * fb * L/W= 0.0 psi= 0.0 psi

Moment Stress=fb1 = fb-fb2 $M2 = fb1*L^2)/2$ = 0.0 psi= 0 in-lb

 $M3 = (1/2)*fb2*L*(2/3)*L = (1/3)*fb2*L^2$ Mtotal = M1+M2+M3= 0 in-lb= 79 in-lb/in

S-plate = $(1)(t^2)/6$ Fb = 0.75*Fy= 0.006 in^3/in = 27,000 psi fb/Fb = Mtotal/[(S-plate)(Fb)] F'p = 0.7*F'c

0.46 οк = 2,800 psiOK Tanchor = (Mb-(PLapp*0.75*0.46)(a))/[(d)*N/2]Tallow= 1,961 lb OK

= -5,431 lbNo Tension

Critical load case RMI Sec 2.1, item 4: (1+0.11Sds)DL + (1+0.14SDS)PL*0.75+EL*0.75 <= 1.0, ASD Method **Cross Aisle Loads** Check uplift load on Baseplate

Pstatic= 3,418 lb

Pseismic= Movt/Frame Depth Movt*0.75*0.7*rho= 18,655 in-lb Frame Depth= 42.0 in = 444 lb

P=Pstatic+Pseismic= 3,862 lb

b =Column Depth= 2.69 in

L =Base Plate Depth-Col Depth= 1.05 in

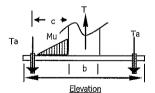
fa = P/A = P/(D*W)M= wL^2/2= fa*L^2/2 = 163 psi= 89 in-lb/in

Sbase/in = $(1)(t^2)/6$ Fbase = 0.75*Fv $= 0.006 in^3/in$ = 27,000 psi

fb/Fb = M/[(S-plate)(Fb)]OK

0.52

Check uplift forces on baseplate with 2 or more anchors per RMI 7.2.2. 'When the base plate configuration consists of two anchor bolts located on either side of the column and a net uplift force exists, the minimum base plate thickness shall be determined based on a design bending moment in the plate equal to the uplift force on one anchor times 1/2 the distance from the centerline of the anchor to the nearest edge of the rack column"



Uplift per Column= 0 lb

Qty Anchor per BP= 1

Net Tension per anchor=Ta= 0 lb

c=1.05 in

Mu=Moment on Baseplate due to uplift= Ta*c/2

= in-lb

Splate= 0.029 in^3

[fb/Fb]*0.75=0

ОК

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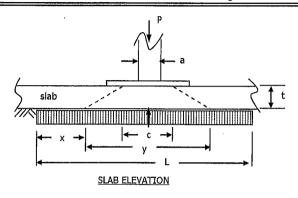
Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909.596.1351 Fax: 909.596.7186

Bob S Project: By: Butler Supply Project #: 25-0625-3

Slab on Grade

Configuration: TYPE 1 SELECTIVE RACK



slab Cross Aisle Down Aisle Baseplate Plan View

f'c= 4,000 psi tslab=t= 6.0 in teff= 6.0 in $phi=\emptyset=0.6$ Soil fsoil= 750 psf

ACI 318-14 Sec 5.3.1

Concrete

Movt= 18,546 in-lb Frame depth= 42.0 in Sds = 0.106

0.2*Sds = 0.021

λ= 0.600 $\beta = B/D = 1.092$

Effec. Baseplate width=B= 5.09 in Effec. Baseplate Depth=D= 4.66 in

width=a= 3.00 in depth=b= 2.69 in

> midway dist face of column to edge of plate=c= 4.05 in midway dist face of column to edge of plate=e= 3,68 in

 $F'c^0.5 = 63.2$

Column Loads

DEAD LOAD=D= 75 lb per column

Base Plate

unfactored ASD load

PRODUCT LOAD=P= 4,500 lb per column

unfactored ASD load

Papp= 3,015 lb per column

P-seismic=E= (Movt/Frame depth)

= 441 lb per column

unfactored Limit State load

B= 0.7000

rho= 1.3000

Sds= 0.1056

1.2 + 0.2*Sds = 1.2211

0.9 - 0.20Sds= 0.8789

Load Case 1) (1.2+0.2Sds)D + (1.2+0.2Sds)*B*P+ rho*E

= 1.22112 * 75 lb + 1.22112 * 0.7 * 4500 lb + 1.3 * 441 lb

= 4,511 lb

Load Case 2) (0.9-0.2Sds)D + (0.9-0.2Sds)*B*Papp + rho*E RMI SEC 2.2 EQTN 7

= 0.87888 * 75 lb + 0.87888 * 0.7 * 3015 lb + 1.3 * 441 lb

= 2,494 lb

Load Case 3) 1.2*D + 1.4*P

RMI SEC 2.2 FOTN 1.2

= 1.2*75 lb + 1.4*4500 lb

= 6,390 lb

Load Case 4) 1.2*D + 1.0*P + 1.0E

= 5,031 lbEqtn 5.3.1e

Effective Column Load=Pu= 6,390 lb per column

Puncture

Apunct= [(c+t)+(e+t)]*2*t

= 236.64 in^2

Fpunct1= $[(4/3 + 8/(3*\beta)] * \lambda *(F'c^0.5)$

= 143.1 psi

Fpunct2= $2.66 * \lambda * (F'c^0.5)$

= 100.9 psi

Fpunct eff= 100.9 psi

fv/Fv= Pu/(Apunct*Fpunct)

0.268

< 1 OK

Slab Bending

Pse=DL+PL+E= 6,390 lb

Asoil= (Pse*144)/(fsoil)

 $= 1,227 in^2$

x=(L-y)/2

= 9.6 in

 $Fb = 5*(phi)*(f'c)^0.5$

= 189.74 psi

 $L= (Asoil)^0.5$

= 35.03 in

 $M = w*x^2/2$

 $= (fsoil*x^2)/(144*2)$

= 239.3 in-lb

 $y=(c*e)^0.5 + 2*t$ = 15.9 in

S-slab= 1*teff^2/6

= 6.0 in^3

fb/Fb= M/(S-slab*Fb)

0.210 < 1, OK

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Development Services Department Lee's Summit, Missouri 07/13/2025

Cantilever Rack

Analysis

1815 Wright Ave., La Verne, CA 91750 Tel: 909.596.1351 Fax: 909.593.8561

CONSTRUCTION

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel; 909.596,1351 Fax; 909.596,7186 By: Bob S **Butler Supply** Project #: 25-0625-3 Design Data Configuration: TYPE 2 SGL-SIDED CANTILEVER RACK

- 1) The analysis presented herein conforms to the requirements of the 2021 IBC and ASCE 7-16
- 2) W & S Shape steel conforms to ASTM A572, Gr. 50, with minimum yield, Fy= 50 ksi Formed steel conforms to ASTM A570, Gr. 50, with minimum yield, Fy= 50 ksj Other steel conforms to ASTM A36, Gr. 36, with minimum yield, Fy= 36 ksi
- 3) Bolts shall conform to ASTM A325-N unless noted otherwise on the plans or calculations.
- 4) Anchor bolts shall be provided by installer per ICC reference on plans and calculations herein. Installer must provide any special inspection as called out on plans or calculations, or as required by the ICC report indicated herein.
- 5) All welds shall conform to AWS procedures, utilizing E70xx electrodes or similar. All such welds shall be performed in shop, with no field welding allowed other than those supervised by a licensed deputy inspector.
- 6) 6 in Thk x 4000 psi slab with 750 psf soil bearing pressure

Seismic Design Coefficient

Ss= 0.0990

S1 = 0.0680

Fa= 1.6000

Fv= 2.4000

Sds= 0.1057

Sd1 = 0.1089

CONSTRUCTION

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909.596.1351 Fax: 909.596.7186

by. Bob S	Project: Butier Supply		Project #	25-0625-3		
Summary of Results	Configuration: TYPE 2 SGL-SIDED CANTILEVER RACK					
144 IN H			Rack Dimensions & Loads			
48 IN ARM AND BASE	ENCTH	Arm	Elev	Unif. Arm Load		
48 IN BRACE HEIGHT	LLINGTTI	h1	36 in	2,400 lb		
48 IN COLUMN SPACI		h2	72 in	2,400 lb		
40 IN COLUMN SPACI	NG	h3	108 in	2,400 lb		

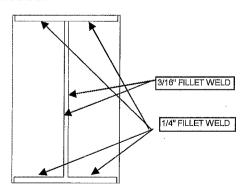
Seismic Coeff Ss= 0.099 Seismic Coeff Fa= 1.600

		Summary of Components & Results		
Column	Fy=50 ksi	W8x18	0.46	ОК
Base	Fy=50 ksi	W8x18	0.38	ОК
Arm	Fy=50 ksi	S5x10	0.40	ОК
Arm Connection		(4) 3/4" Diam Bolts, Grade 5 or better	0.44	ОК
Brace	Fy=36 ksi	X Brace: 1.575x0.984x105 Strut L2x2x3/16	0.02	ОК
X Brace Connection	Bolted	(1) 0.5 in diam Bolt per end	0.03	ОК
Anchors	ESR 4266	0.75 in diam x 4.75 in embed HILTI TZ2 with inspection 4 per base	0.09	ОК
Slab & Soil		6 in thick x 4000 psi with 750 psf soil at grade	0.22	ОК

Notes

ALL LOADS SHOWN ARE PER ARM, UNIFORM LOAD, NO TIP LOADS, NO DROPPED LOADS, NO IMPACT LOADS

OMEGA = 2.0



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Structural

Engineering & Design Inc.

Development Services Department Lee's Summit, Missouri 07/13/2025

1815 Wright Ave La Verne, CA 91750 Tel: 909.596,1351 Fax: 909.596.7186

By: Bob S Project: Butler Supply Project #: 25-0625-3

Seismic Loads Configuration: TYPE 2 SGL-SIDED CANTILEVER RACK

Seismic Eqtn:	Transverse	Longitudinal	
Cs1=	Sds*I/R		
=	0.042	0.033	
Cs2=	0.044*Sds*I		
=	0.005	0.005	
Cs3=	0.5*S1*I/R		
=	0.014	0.010	

Rtrans= 2.5 Rlong= 3.25 # Sides=n= 1 # Arms= 3

Ss= 0.099
S1= 0.068
Fa= 1.600
Fv= 2.400
Sds= 0.106
Sd1= 0.109
I= 1.0
phi= 0.67
** DL per arm= 127.0 lb

Vtrans= 0.0423

Vlong= 0.0325

Σ Arm LL= 7,200 lb

 Σ Arm LL*phi= 4,824 lb

Σ Arm DL= 381 lb

W1=W_{arm,gravity}= 7,581 lb

W2=W_{arm,seismic}= 5,205 lb

<u>Side Elevation</u>

203 10

** weight of arm plus trib weight of column per level

V-transv= 0.0423 * 5205 lb

= 220 lb/frame

tranv shear per upright

V-long= 0.0325 * 5205 lb = 169 lb/frame

longit shear per upright

	Transverse Force						
Level	W1 (DL+LL)*n	W2 (DL+LL*phi)	hx	wxhx	Fi	Fi * hi	Fi
1	2,527 lb	1,735 lb	36 in	62460	37 lb	1,332 in-lb	28 lb
2	2,527 lb	1,735 lb	72 in	124920	73 lb	5,279 in-lb	56 lb
3	2,527 lb	1,735 lb	108 in	187380	110 lb	11,880 in-lb	85 lb

 Σ = 374,760 220 lb 18,491 in-lb

Summary of Frame Loads

Mseismic= 18,491 in-lb

Mwind= 24,019 in-lb

Pcol= 7,581 lb

DL+LL

Marm-max= (LL-max+DL) * (Arm Length/2)

= (2400 lb + 31 lb) * (48 in/2)

= 58,344 in-lb

 $\mathsf{Marm\text{-}total} = \Sigma \; \mathsf{Marm}$

= 175,032 in-lb

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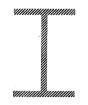
1815 Wright Ave La Verne, CA 91750 Tel: 909,596,1351 Fax: 909,596,7186

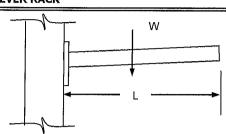
By: Bob S Project: Butler Supply Project #: 25-0625-3

Arm Configuration: TYPE 2 SGL-SIDED CANTILEVER RACK

Analysis is based on a uniformly loaded arm with no tip or impact loads considered.

Arm Type= \$5x10 Ix= 12.3000 in^4 \$x= 4.920 in^3 Fy= 50,000 psi L= 48 in





Check Arm Bending

LL-max= 2,400 lb

DL= 31 lb

M = W * L/2= 58,344 in-lb

fb= M/Sy = 11,859 psi

Fb= 0.6 * Fy 30,000 psi

fb/Fb=

0.40

OK

Check Arm Deflection

E= 29,000,000 psi

D= $w * L^4/(8 * E * Ix)$ = 0.0973 in

Dallow= L/180

0.27 in OK

Check Bolts For Imposed Arm Loads

. Spacing=d= 4.0 in

Bolt diameter= 0.75 in

Ft= 44,000 psi

Fv= 21,000 psi

tmin= 0.188 in

Fu= 58,000 psi

Bolts in shear=Ns= 4

Bolts in tension=Nt= 2

M= 58,344 in-lb

T= M/d

= 14,586 lb

W= LL+DL

= 2,431 lb

W/Bolt= 608 lb

T/Bolt= 7,293 lb

Shear Capacity= Bolt Area * Fv

= [(0.75 in)^2 * pi/4] * 21000 psi

= 9,278 lb

Bearing Capacity= Bolt Diam * tmin * Fu * 1.2

= 9,788 lb

Tension Capacity= Bolt Area * Ft

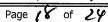
= 19,439 lb

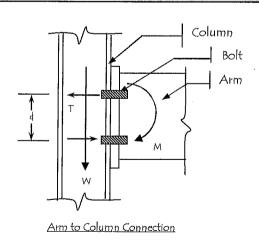
Combined stresses: (

(608 lb/9278 lb) + (7293 lb/19439 lb)=

0.44

ОК





CONSTRUCTION

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

Structural

Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909.596,1351 Fax: 909.596,7186

By: Bob S Project: **Butler Supply** Project #: 25-0625-3 Column Load Case: DL+LL Configuration: TYPE 2 SGL-SIDED CANTILEVER RACK

Section

Section= W8x18

Area = 5.26 in^2

Ix= 61.90 in^4

Sx= 15.20 in^3 rx = 3.43 in

Iy= 7.97 in^4 Sy= 3.04 in^3

ry= 1.23 in

Kx = 2.4Ky = 1.0

Fy= 50,000 psi

Lx= 108 in Ly= 48 in Cm= 1.00

Loads (Single Face Rack-Full Static Arm Loads)

Check Load Case 2 ASCE 2.4.1: : DL + LL <= 1.0, ASD Method

Pmax= 7,581 lb

Mstatic= 175,032 in-lb

COLUMN DL= 381 lb

(kl/r)max= 75.57

COLUMN PL= 7,200 lb

Mstatic= 175,032 in-lb

Combined Sress

(kl/r)x = (2.4*108 in/3.43 in)

= 75.57

(kl/r)y = (1*48 in/1.23 in)

= 39.02

 $Cc = (2\pi^2E/Fy)^0.5$

= 107.0

SINCE (KL/r)max < Cc, USE EQTN E2-1

Fa = $[1-((kl/r)^2/2Cc^2)]Fv$

 $5/3 + 3(kl/r)/8Cc - (kl/r)^3/8Cc^3$

= 19,884 psi

fa= P/AREA

= 1,441 psi

0.07 < 0.15

fbx= M/S

Fbx = 0.6*Fy

= 11,515 psi

= 26,150 psi

= 30,000 psi

 $F'ex = (12*\pi^2*E)/(23*(KL/r)^2)$

fb/Fb = 0.38

(1-fa/F'e)>0=1.00

(H1-3):

fa/Fa + fb/Fb=

0.46

<= 1 OK

Check Base Bending

Section= W8x18

Fy= 50,000 psi

Sx= 15.20 in^3

Fbx = 0.6*Fy

= 30,000 psi

Mbase-static= 175,032 in-lb

fb/Fb = 0.38

OK

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Engineering & Design Inc.

Development Services Department Lee's Summit, Missouri 07/13/2025

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TATO WITIONT AVE	ia verne	CA 91750	Tel: 909	596 1351	FAY 909 596 / 186

By: Bob S Project: **Butler Supply** Project #: 25-0625-3

Column Load Case: DL+LL+ Lateral Load Configuration: TYPE 2 SGL-SIDED CANTILEVER RACK

Section

Structural

Section= W8x18

Area = 5.26 in^2

Ix= 61.90 in^4

Sx= 15.20 in^3

rx= 3.43 in

ry= 1.23 in

Iy= 7.97 in^4

Sy= 3.04 in^3

Stress Increase = 1.00

Cm= 1.00

Ly= 48 in

Lx= 108 in

Kx= 2.4

Ky = 1.0

Fy= 50,000 psi

Loads (Single Face Rack-Full Arm Loads plus lateral loads)

Check Load Case 6b ASCE 12.4.2.3: : (1+0.10*Sds)D + 0.75L + 0.525E<= 1.0, ASD Method

Pmax = 1.01 * 381 lb + 0.75 * 7200 lb

= 5,785 lb

M= 0.75*175032 in-lb + 18491 in-lb * 0.525

= 140,982 in-lb (Mstatic + Mwind) COLUMN DL= 381 lb COLUMN PL= 7,200 lb

Mstatic= 175,032 in-lb

Mseismic= 18,491 in-lb

Sds= 0.1057 1+0.10*Sds= 1.0100

Combined Sress

(kl/r)x = (2.4*108 in/3.43 in)

= 75.57

 $Cc = (2\pi^2E/Fy)^0.5$

= 107.0

(k/r)y = (1*48 in/1.23 in)

= 39.02

(kl/r)max= 75.57

SINCE (KL/r)max < Cc, USE EQTN E2-1

Fa = $[1-((kl/r)^2/2Cc^2)]Fy$

 $5/3 + 3(kl/r)/8Cc - (kl/r)^3/8Cc^3$

= 19,884 psi

fa= P/AREA

= 1,100 psi

0.06

< 0.15

fbx = M/S

Fbx = 0.6*Fy

= 9,275 pši

= 26,150 psi

= 30,000 psi

 $F'ex= (12*\pi^2*E)/(23*(KL/r)^2)$

fb/Fb= 0.31

(1-fa/F'e) = 1.00

(H1-3):

fa/Fa + fb/Fb=

0.36

<= 1 OK

Check Base Bending

Section= W8x18

Fy= 50,000 psi

Sx= 15.20 in^3

Fbx = 0.6*Fy

= 30,000 psi

Mbase-lateral= 140,982 in-lb

fb/Fb = 0.31

OK

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909,596,1351 Fax: 909.596,7186

By: Bob S Project:

Project #: 25-0625-3

Anchors

Configuration: TYPE 2 SGL-SIDED CANTILEVER RACK

Butler Supply

Check Load Case 7 ASCE 12.4.2.3: : (0.9-0.20*Sds)DL + (0.9-0.20*Sds)Papp + E

Overturning Forces

Seismic Forces

Seismic Ovt Moment=Mseismic= 18,491 in-lb

Rack DL= 381 lb

load per upright

LLeff=Total LL*0.67=Papp= 4,824 lb

load per upright

Mstabilizing=Mst= [0.879*(381 lb+4824 lb)]*56.14 in/2

= 128,426 in- lb

T-seismic= [Mseismic - Mst]/d * Omega

= 0 lb

net seismic uplift

Seismic shear= 220 lb/frame

Seismic Tension per anchor= 0 lb

Seismic Shear per anchor= 55 lb

Side Elevation Base depth= 48.0 in Column depth= 8.1 in d = 56.1 in0.9-0.20*Sds= 0.879

Check Anchors

Anchor Type= 0.75 in diam x 4.75 embed HILTI TZ2 Per ICBO ESR 4266 Anchor Inspection Required? Yes

Anchor/base= 4

Tallow= 1,961 lb

Stress Increase= 1

Anchor/end= 2

Vallow= 2,517 lb

Eqtn Exponent= 1

	Tension/anchr	Shear/anchr	Tallow	Vallow	Interactn Eq.	Status
Seismic	0 lb	55 lb	1,961 lb	2,517 lb	0.022	ок

CONSTRUCTION

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

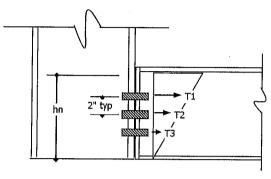
Engineering & Design Inc.

1815 Wright Ave La Verne, CA 91750 Tel: 909,596,1351 Fax: 909,596,7186

Bob S Project: Butler Supply Project #: 25-0625-3

Base to Column Connection

Configuration: TYPE 2 SGL-SIDED CANTILEVER RACK



h1 = 6.0 inh2 = 4.0 inh3 = 2.0 in

M_D= 2,232 in-lb M_P= 172,800 in-lb M_E= 18,491 in-lb $\beta = 0.7$ p = 1.3Sds = 0.106Omega= 2

Bolt Diam= 0.750 in

Ft= 19.40 kip

(Grade 5 or better)

Base to Column Elevation

T1 = Ft * 2= 38.80 kip T2 = T1 * h2/h1

= 25.87 kip

T3= T1 * h3/h1 = 12.93 kip

Moment Capacity=Mcap= T1 * h1 + T2*h2 + T3*h3 + T4*h4 + T5*h5 + T6*h6 = 362 in-kip

Load Case 1 (ANSI MH16.3-2016 Sec 2.1)

D+P= 175,032 in-lb

Load Case 5

 $(1+0.105*Sds)*D+0.75*[1.4+0.14*Sds)*\beta*P+L+S+0.7*p*E]=143,227 in-lb$

Load Case 6

 $(1+0.14*Sds)*D+(0.85+0.14*Sds)*\beta*P+0.7*p*E= 123,697 in-lb$

Load Case 7

(0.6-0.14*Sds)*D+(0.6-0.14*Sds)*Papp+0.7*E*Omega= -51,167 in-lb

Check Longitudinal Brace

M demand= 175.0 in-kip

ΟK

Brace Type= X Brace

Brace= 1.575x0.984x105

Area=An= 0.337 in^2

V-long= 169 lb T= 0.525*Vlong*Ldiag/Lh

= 126 lb

Ft = 0.6 * Fy= 21,600 psi

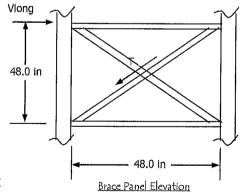
ft= T/An

= 374 psi

ft/Ft=

0.02

< 1.0, OK



Bay Width=Lh= 48.0 in Brace Panel Ht=Lv= 48.0 in Ldiag= 68.0 in Fy= 36,000 psi

Check Brace Connection

Connection= Bolted

Fv-bolt= 21,000 psi

Bolt Capacity= Bolt Area * Fv-boit = 4,123 lb

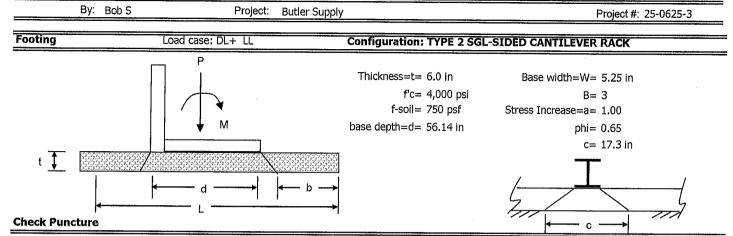
OK

Bolts= 1 Bolt Diam= 0.5

Engineering & Design Inc.

Development Services Department Lee's Summit, Missouri 07/13/2025

_1815 Wright Ave La Verne, CA 91750 Tel: 909.596.1351 Fax: 909.596.7186



Pgravity= 7,581 lb

Mbase= 175,032 in-lb

Povt= Mbase/(d*0.66)

= 4,724 lb

P= 1.4 * (Pgravity + Povt)

= 17,227 lb

 $Fp = [(4/3) + (8/3)/B] * (f'c)^0.5 < 2.66 * (f'c)^0.5 * a$

= 140.5 psi

Punct Area=A= (d + t) + (W + t) * 2 * t

= 880.7 in^2

Puncture Stress= (P/A)/Fp

= 0.14

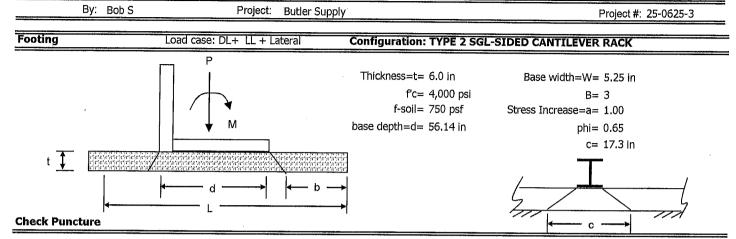
ОК

Check Bending

Engineering & Design Inc.

Development Services Department Lee's Summit, Missouri 07/13/2025

1815 Wright Ave La Verne, CA 91750 Tel: 909,596,1351 Fax: 909,596,7186



Pgravity= 5,785 lb

Mbase= 140,982 in-lb

Povt= Mbase/(d*0.66)= 3,805 lb P= 1.2*Pgravity + Povt = 10,351 lb

Fp= $[(4/3) + (8/3)/B] * (f'c)^0.5 < 2.66 * (f'c)^0.5 * a$ = 140.5 psi

Punct Area=A= (d + t) + (W + t) * 2 * t= 880.7 in^2

Puncture Stress= (P/A)/Fp

= 0.08

ОК

Check Bending

Areq= F = 1	P/fsoil 987 in^2			b= 115.2 in - (56 = 47.06 in	5.14 in + 2 * 6 in) >= 0
=	115.2 in	х	17.3 in		
Mslab= f	soil * c * b^2/(2*144)		S= c * t/	^2/6	
= 6	91 in-lb		103.5	in^3	•
Fb= 5	* phi * (f'c)^0.5 * a		fb/Fb= (Msla	b/S)/Fb	
= 2	05 . 5 psi		= 0.03	ок	

As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 07/13/2025

MICHAEL MOORES, RA (816) 516-4861

Development Services CITY OF LEE'S SUMMIT, Missouri 220 SE Green Street Lee's Summit, MO 64063

Re:

Butler Supply

2736 NE McBaine Drive Lee's Summit, MO 64064 Permit No.: PRCOM20251795

June 30, 2025

Dear Plans Examiner:

According to the bar joist manufacturer, the existing bar joists are capable of supporting up to 100 lbs. of concentrated load without installing additional web members. The two new condensing units each weigh 182 lbs. The condensers shall be installed on the roof equidistant from the adjacent bar joists, and the two units shall be separated by at least one bay so that each bar joist does not support more than 100 lbs. of concentrated load (see sketch below).

Sincerely,



MICHAEL MOORES, RA

