

SPECIFICATIONS:

DESIGN CRITERIA: 2018 INTERNATIONAL BUILDING CODE

DESIGN LOADS:

ROOF LIVE LOAD	20 PSF
GROUND SNOW LOAD	20 PSF
SNOW EXPOSURE FACTOR, Ce	0.9
IMPORTANCE FACTOR	1.0
THERMAL FACTOR, Ct	1.0
WIND VELOCITY, Vult	115 MPH
Vasd	89 MPH
RISK CATEGORY II	
EXPOSURE C	
BUILDING IS DESIGNED AS	FULLY ENCLOSED
INTERNAL PRESSURE COEFFICIENT	+/- 0.18
BASE SHEAR (KIPS)	N/S = 16
	E/W = 8
IMPORTANCE FACTOR	1.00
RISK CATEGORY	II
MAPPED SPECTRAL RESPONSE ACC	Ss=0.100g, S1=0.068g
SPECTRAL RESPONSE COEFF	Sds=0.107g, Sd1=0.109g
SEISMIC DESIGN CATEGORY	B
SITE CLASS D (ASSUMED)	Fa=1.6, Fv=2.4
DESIGN BASE SHEAR (KIPS)	2
RESPONSE COEFF	0.018
RESPONSE MOD FACTOR,	R = 3

(SYSTEMS NOT SPECIFICALLY DESIGN FOR SEISMIC RESISTANCE)

ANALYSIS: EQUIVALENT LATERAL FORCE PROCEDURE

FOUNDATIONS:

- THE FOUNDATION DESIGN BASED ON REACTIONS PROVIDED ON SHEET E1.1
- MAXIMUM ANCHOR BOLT DIAMETER SHALL BE 5/8", MINIMUM ANCHOR BOLT DIAMETER SHALL BE 1/2"
- PROVIDE WASHERS ON ALL ANCHOR BOLTS.

STRUCTURAL STEEL:

- STRUCTURAL STEEL SHALL MEET THE LATEST AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- ASTM SPECIFICATION AND GRADE, UNLESS NOTED OTHERWISE ON THE DRAWINGS, STRUCTURAL STEEL SHALL BE AS FOLLOWS:
 - ALL TRUSS ANGLES AND RODS SHALL BE A572 GR 50
 - ALL WIND BRACING RODS SHALL BE A512 GR 50
 - ALL PLATE SHALL BE A36
 - ALL ROOF AND WALL PANELS SHALL BE 29 GA, 80 KSI MIN, PANEL-LOC PLUS TM, MINIMUM SECTION PROPERTIES:
 - Ix MIN = 0.0133 IN
 - Ix MIN = 0.0093 IN
 - Sx MIN = 0.0220 IN
 - Sx MIN = 0.0198 IN
- ALL BOLTS SHALL MEET ASTM GRADE 5 HIGH STRENGTH, WITH WASHERS AS REQUIRED, EXCEPT ANCHOR RODS WHICH SHALL MEET ASTM A36 UNO.
- WELDING SHALL CONFORM TO THE STANDARDS SET FORTH IN THE AWS PUBLICAITON "WELDING IN BUILDING CONSTRUCTION".
- ALL SHOP CONNECTIONS TO HAVE FILLET WELDS MEETING THE REQUIREMENTS SET FORTH BY AWS D1.1 UNLESS NOTED AS BOLTED CONNECTIONS.
- ALL WELDS TO BE WITH E70XX ELECTRODES.
- STEEL FRAMES ARE NOT "SELF-SUPPORTING". ADEQUATE TEMPORARY SUPPORT SHALL BE PROVIDED BY THE CONTRACTOR UNTIL REQUIRED ELEMENTS OR CONNECTIONS ARE IN PLACE.
- STRUCTURAL BOLTS AND THREADED FASTENERS:
 - J429 GRADE 5 BOLTS
 - ALL BOLTS TO BE 5/8" DIA UNO
 - ALL BOLTS TO BE TIGHTENED ACCORDING TO THE "FULLY TENSIONED JOINTS" METHOD PER RCSC SPECIFICATION SECTION 4.2 UNO.

MISC:

- WALK DOORS AND WINDOWS MAY BE FIELD LOCATED, NO WIND ROD BRACING OR COLUMN CHORD BRACING SHALL BE CUT TO ACCOMODATE WINDOWS OR DOORS.

ABBREVIATIONS:

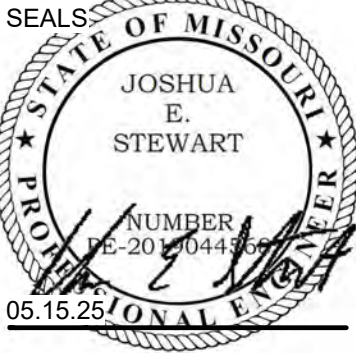
BC	= BOTTOM CHORD
C/C	= CENTER-TO-CENTER
C/L	= CENTERLINE
CB	= CHORD BRACE
CJ	= CONTROL JOINT
CONT	= CONTINUOUS
EA	= EACH
EL	= RELATIVE ELEVATION W/ RESPECT TO GROUND FLOOR
FT	= FOOT
PSF	= POUNDS PER SQUARE FOOT
SIM	= SIMILAR
ST	= STEEL
TC	= TOP CHORD
TOC	= TOP OF CONCRETE
TOF	= TOP OF FOOTING
TRC	= TRUSS COLUMN
TRG	= TRUSS GIRDER
TYP	= TYPICAL
UNO	= UNLESS NOTED OTHERWISE
WD	= WOOD

METAL STUD FRAMING:

- ALL LIGHT GAUGE FRAMING SHALL COMPLY WITH THE FOLLOWING:
 - ALL STUDS AND JOIST SHALL CONFORM TO THE REQUIREMENTS OF ASTM A653 GRADE 50 WITH A MINIMUM YIELD STRSS OF 50,000 PSI
 - GIRTS: 6"x1 5/8"x20GA
 - PURLINS: 6"x1 5/8"x20GA
- SCREWED CONNECTIONS:
 - FASTENERS SHALL BE SELF-DRILLING SCREWS BY BUILDEX, COMPASS, GRABBER OR APPROVED EQUAL WITH THE CAPACITIES NECESSARY FOR THE REQUIRED DESIGN LOADINGS FOR ALL FRAMING MEMBERS PER THE MANUFACTURERER RECOMMENDATIONS.
 - MINIMUM SCREW SIZE SHALL BE #6
 - SCREWS SHALL CONFORM TO ASTM C1513 & C954
 - A MINIMUM OF (3) EXPOSED THREADS SHALL PENETRATE THROUGH ALL JOINED MATERIALS
 - CORROSION-RESISTANT CADMIUM-PLATED SCREWS SHALL BE USED FOR SCREWS ATTACHING METAL LATH, MASONRY TIES OR OTHER EXTERIOR MATERIALS



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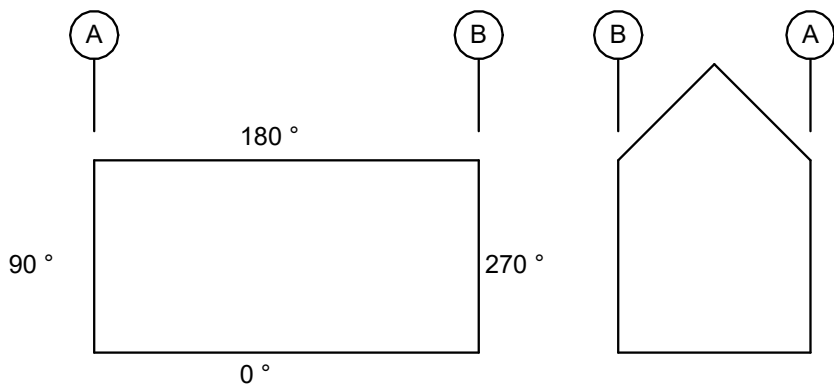
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ISSUE DATE:	MAY 15, 2025
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PROJECT NO:	91-53

SPECIFICATIONS

E1.0



NOTE:

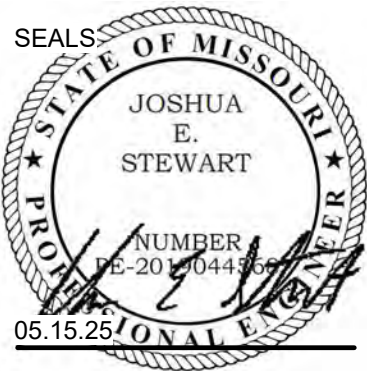
- DL, RLL AND SL ARE SERVICE LOADS (UNFACTORED), WIND LOADS ARE ULTIMATE LOADS.
- END WALL COLUMNS ARE DESIGNED AS PIN ROLLER, THEREFORE ONLY HORIZONTAL REACTIONS ARE PROVIDED.
- NEGATIVE VERTICAL REACTION (Y-AXIS) INDICATED

TRUSS FRAME COLUMN REACTIONS GL2					END WALL COLUMN REACTIONS
LOAD CASE	COL LINE B		COL LINE A		HORIZONTAL REACTION (K)
	HORIZONTAL REACTION, X-DIR(K)	VERTICAL REACTION, Y-DIR(K)	HORIZONTAL REACTION, X-DIR(K)	VERTICAL REACTION, Y-DIR(K)	
DEAD LOAD	-0.3	3.2	0.3	1.3	-
ROOF LIVE	-0.6	6.8	0.5	2.8	-
SNOW LOAD	-0.5	5.1	0.5	2.1	-
WIND 0° - POS INT	1.7	-5.8	-2.3	-4.2	-
WIND 0° - NEG INT	1.0	-2.2	-3.1	-2.8	-
WIND 90° - POS INT	-0.8	-6.0	1.4	-3.1	3.1
WIND 180° - POS INT	1.8	-12.3	2.3	-1.3	-
WIND 180° - NEG INT	2.2	-10.6	1.8	0.4	-
WIND 270° - POS INT	-0.8	-6.0	1.4	-3.1	3.1

TRUSS FRAME COLUMN REACTIONS GL6					END WALL COLUMN REACTIONS
LOAD CASE	COL LINE B		COL LINE A		HORIZONTAL REACTION (K)
	HORIZONTAL REACTION, X-DIR(K)	VERTICAL REACTION, Y-DIR(K)	HORIZONTAL REACTION, X-DIR(K)	VERTICAL REACTION, Y-DIR(K)	
DEAD LOAD	-0.3	3.3	0.3	3.3	-
ROOF LIVE	-0.3	3.4	0.3	3.4	-
SNOW LOAD	-0.2	2.6	0.2	2.5	-
LIVE LOAD	-1.1	8.4	1.1	8.4	-
WIND 0° - POS INT	-1.9	-2.6	-2.1	-4.7	-
WIND 0° - NEG INT	-1.4	-0.9	-2.6	-3.0	-
WIND 90° - POS INT	-0.5	-3.4	1.1	-3.5	2.6
WIND 180° - POS INT	2.1	-4.7	1.9	-2.6	-
WIND 180° - NEG INT	2.6	-3.0	1.4	-0.9	-
WIND 270° - POS INT	-0.5	-3.4	1.1	-3.5	2.6



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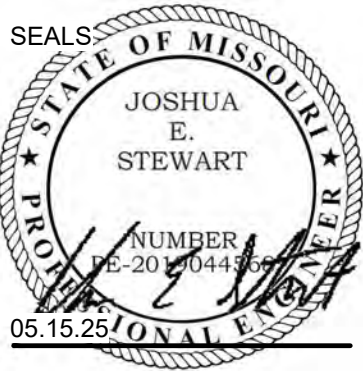
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REACTIONS

E1.1



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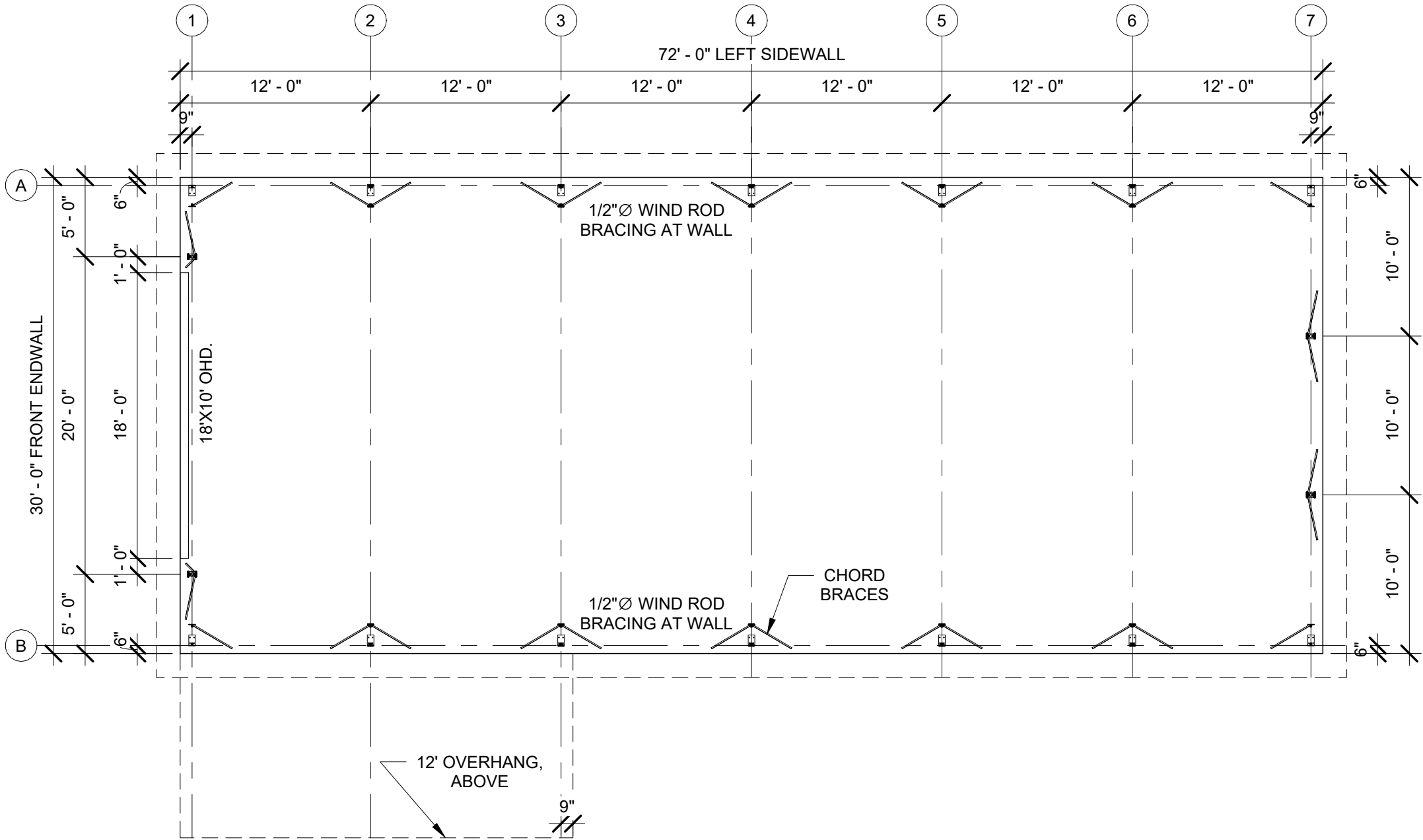
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ANCHOR BOLT
SETTING PLAN

E2.0

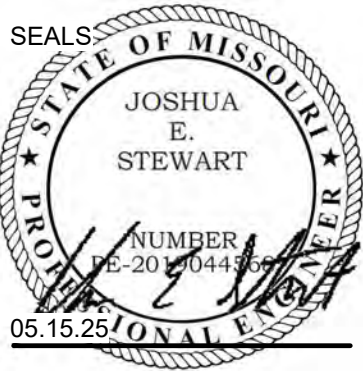


1 ANCHOR BOLT PLAN
1/8" = 1'-0"

NOTES:
FOUNDATION TO BE DESIGNED BY A
REGISTERED ENGINEER IN THE STATE THE
PROJECT IS LOCATED FOR THE FORCES
INDICATED ON THE SPECIFICATION SHEETS.



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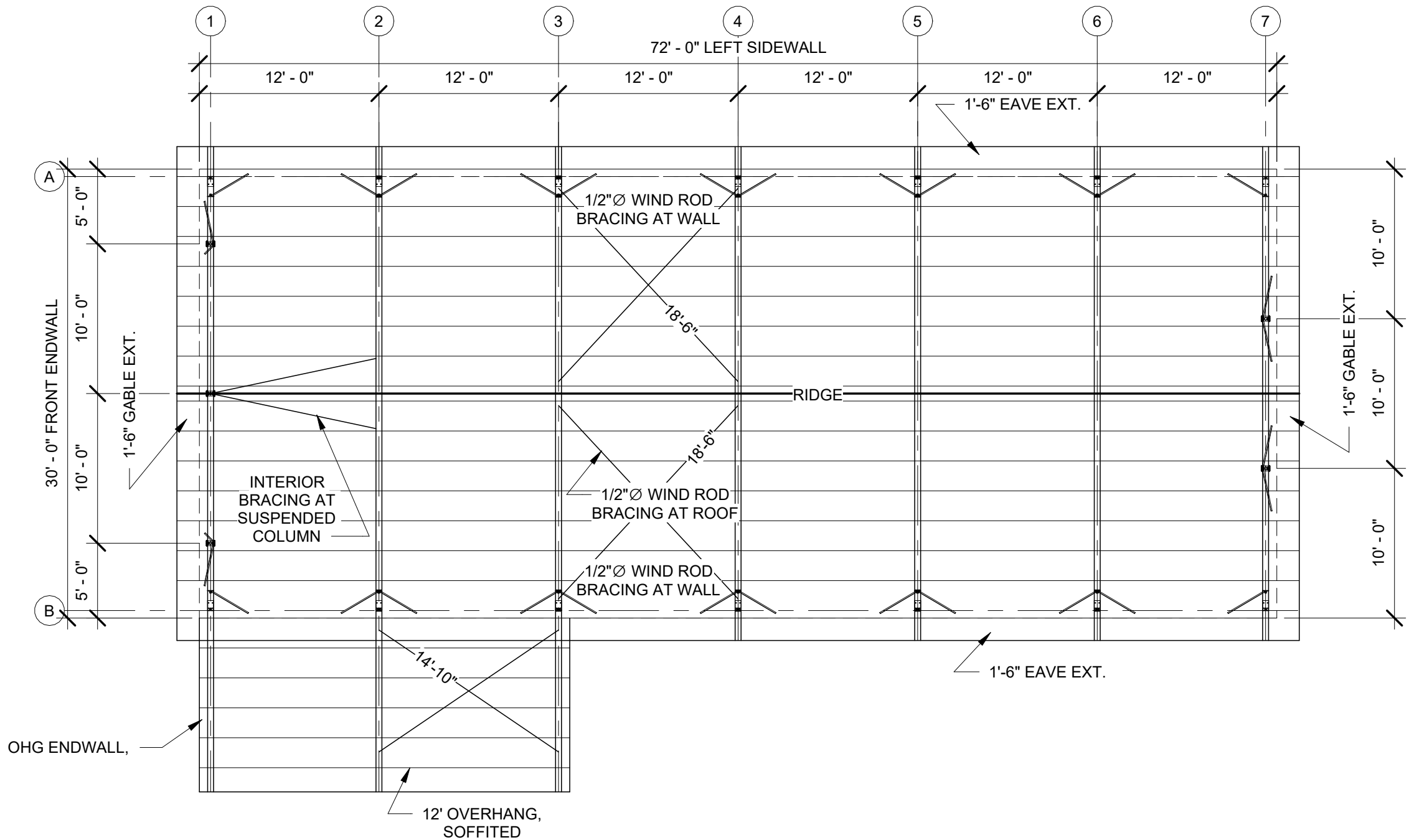
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ROOF FRAMING
PLAN

E2.1



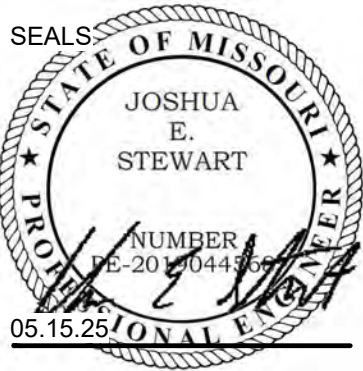
1 ROOF FRAMING PLAN
1/8" = 1'-0"

NOTES:

- GIRDER INSIDE CHORD BRACING IS VITAL TO THE STABILITY OF THE STEEL FRAMES AND SHALL NOT BE REMOVED, RELOCATED OR MODIFIED IN ANY WAY.
- CONNECTIONS ARE VITAL TO THE STABILITY OF THE STEEL FRAMES AND SHALL BE INSTALLED PER THE TYPICAL CONNECTION DETAILS.
- ROOF DECKING AND PURLINS ARE VITAL TO THE STABILITY OF THE STEEL BUILDING. THE BUILDING IS NOT STABLE UNTIL ALL DECKING IS INSTALLED.



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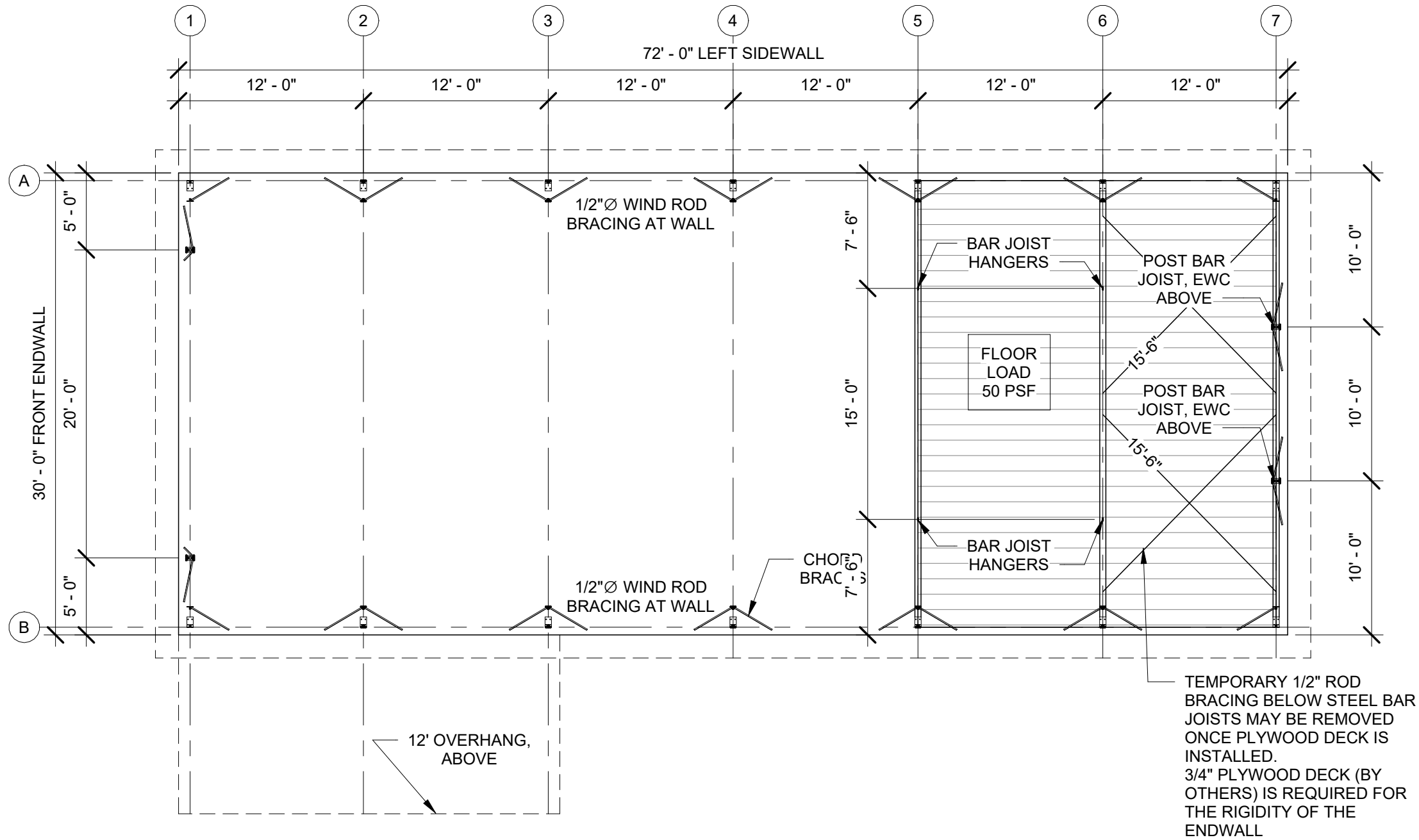
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MEZZANINE PLAN

E2.2

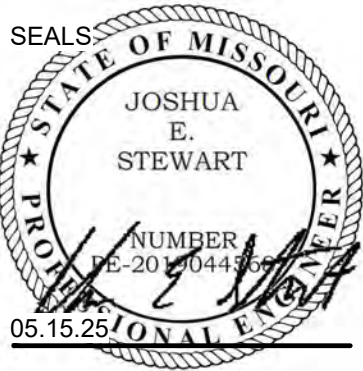


1 MEZZANINE PLAN
1/8" = 1'-0"

- NOTES:
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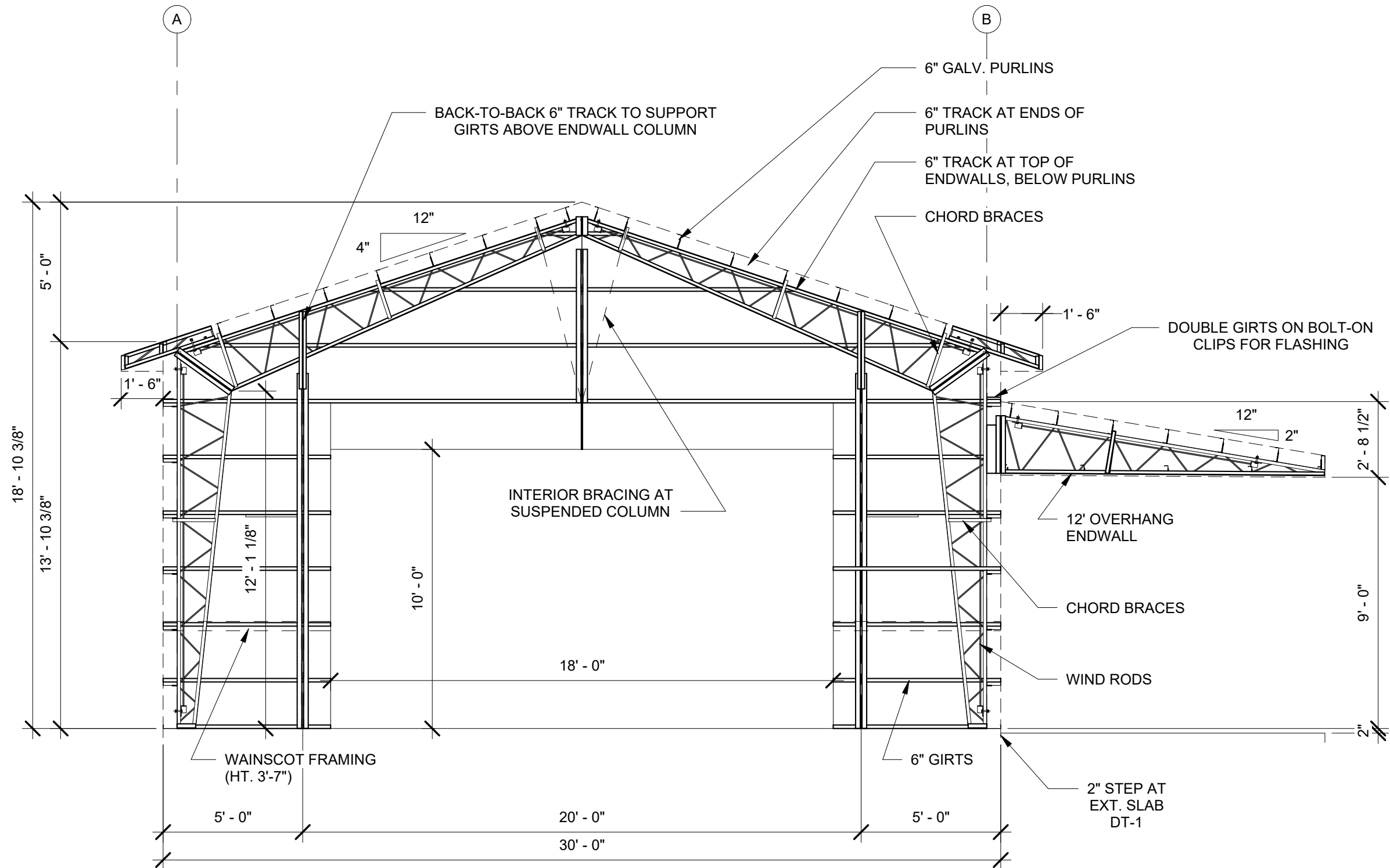
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FRONT ENDWALL

E3.0



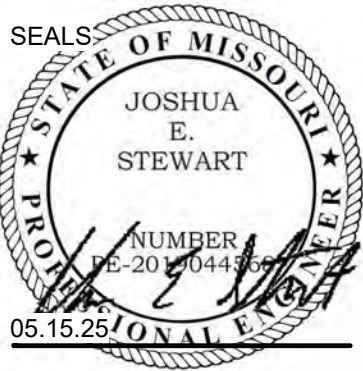
NOTES:

1 FRONT ENDWALL
1/4" = 1'-0"

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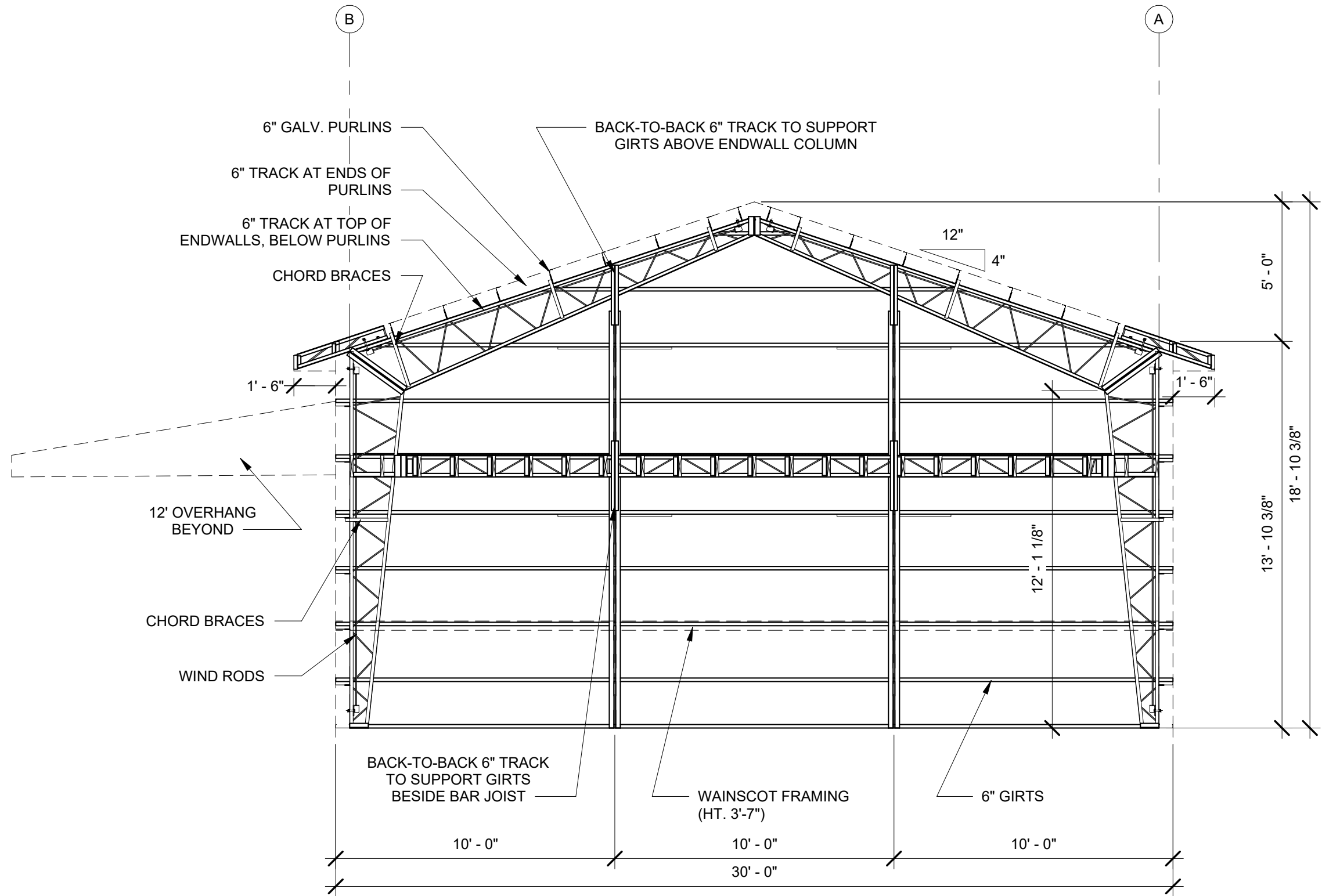
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REAR ENDWALL

E3.1



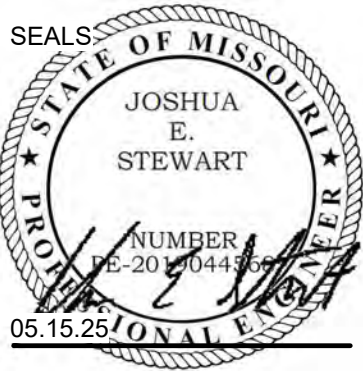
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1 REAR ENDWALL
1/4" = 1'-0"



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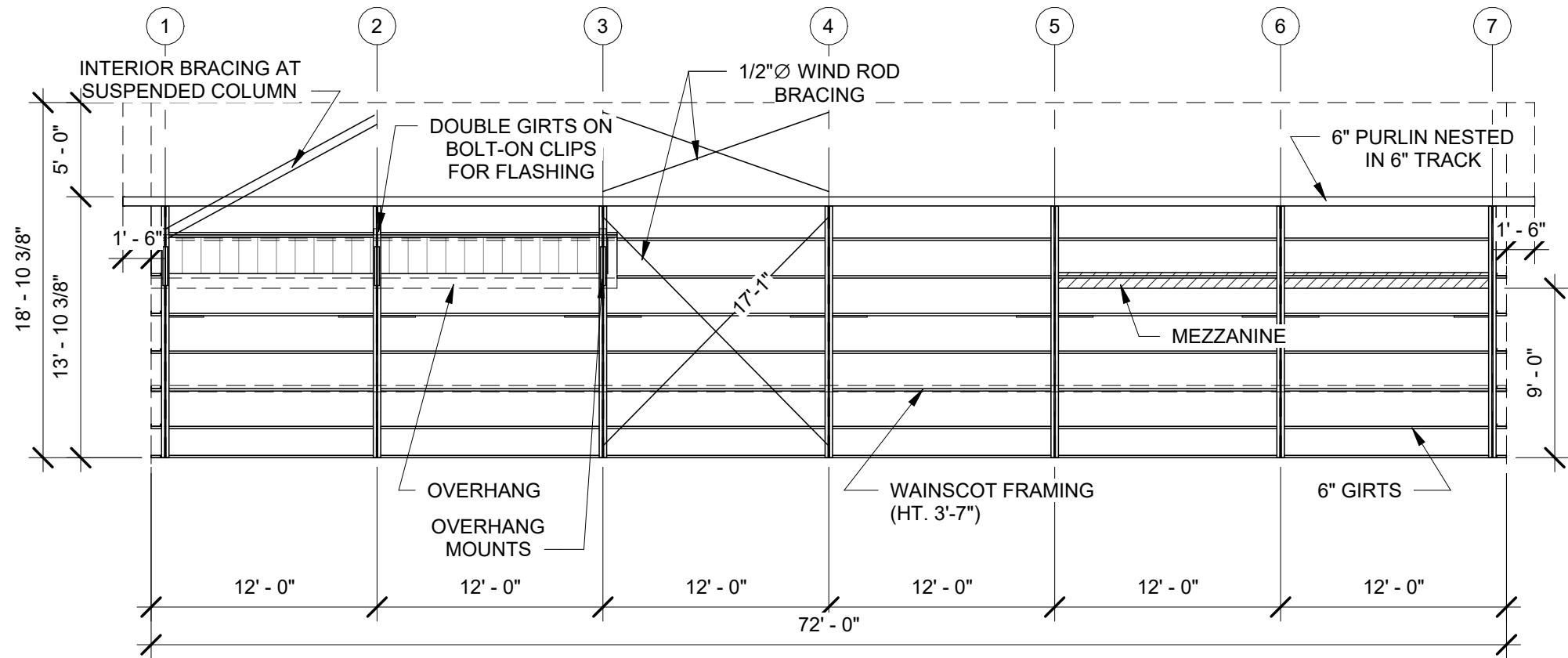
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RIGHT SIDEWALL

E3.2

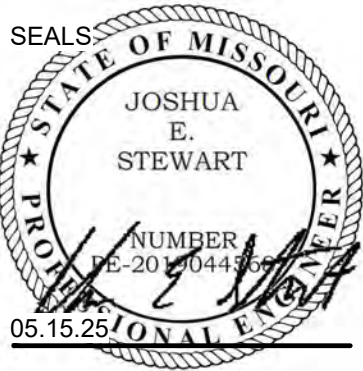


1 RIGHT SIDEWALL
1/8" = 1'-0"

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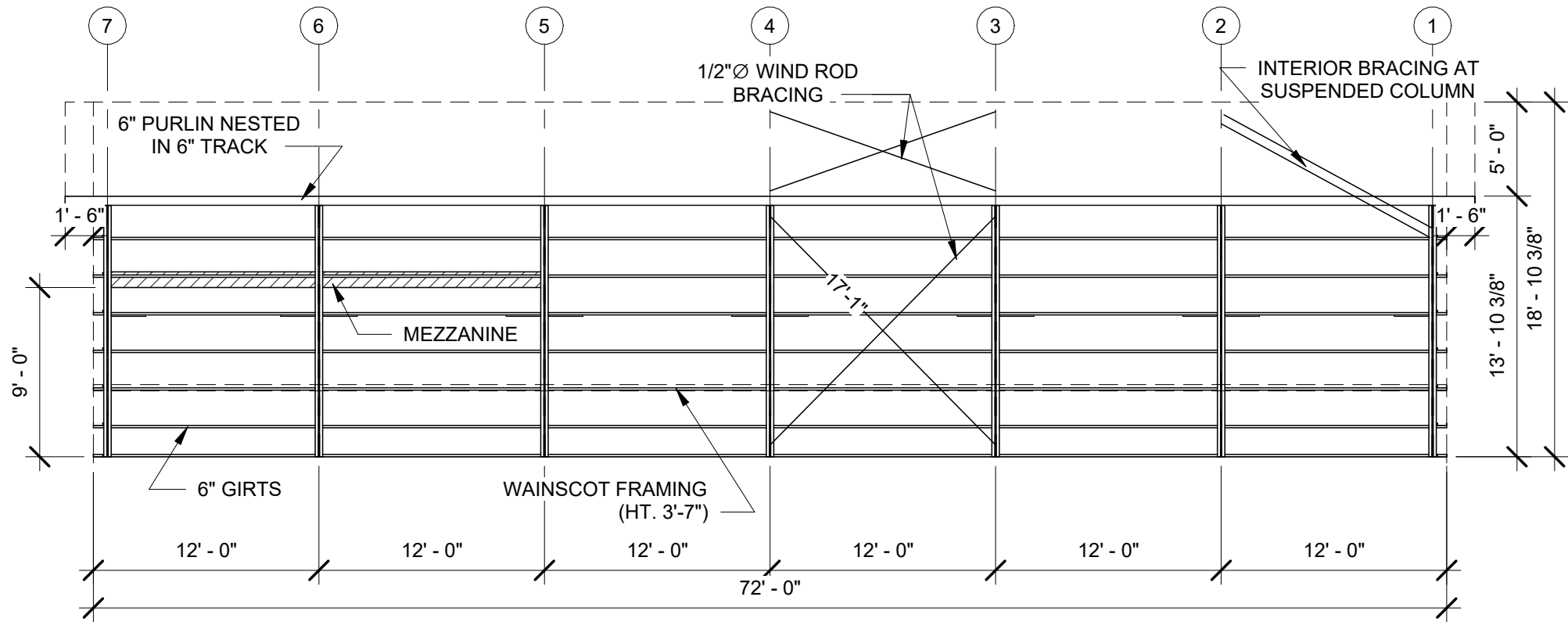
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LEFT SIDEWALL

E3.3



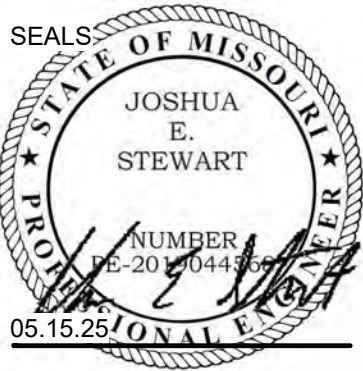
1 LEFT SIDEWALL
1/8" = 1'-0"

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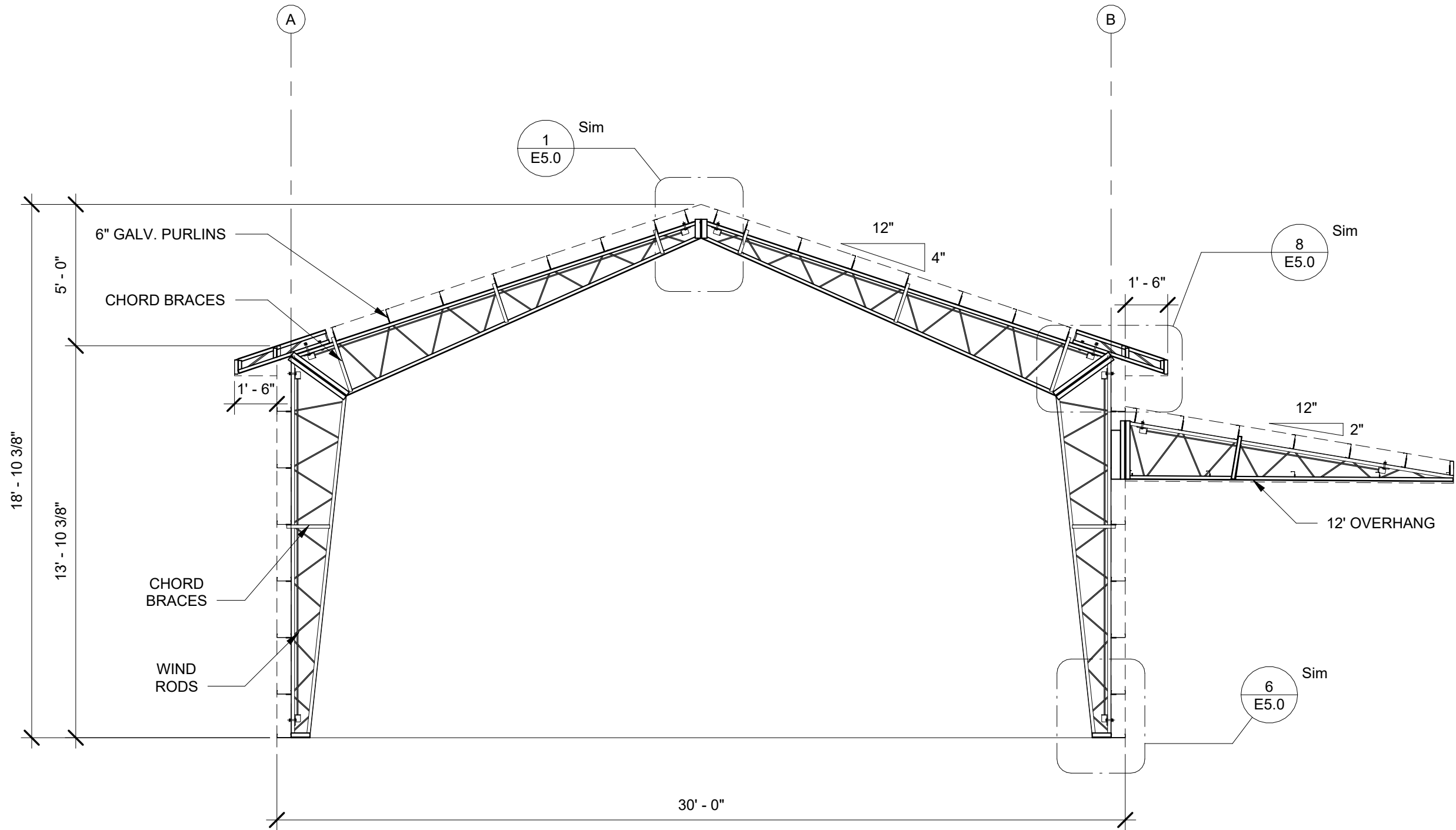
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BUILDING SECTION
(GRID 2)

E4.0



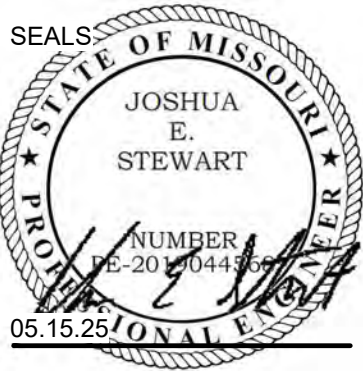
1 BUILDING SECTION (GRID 2)
1/4" = 1'-0"

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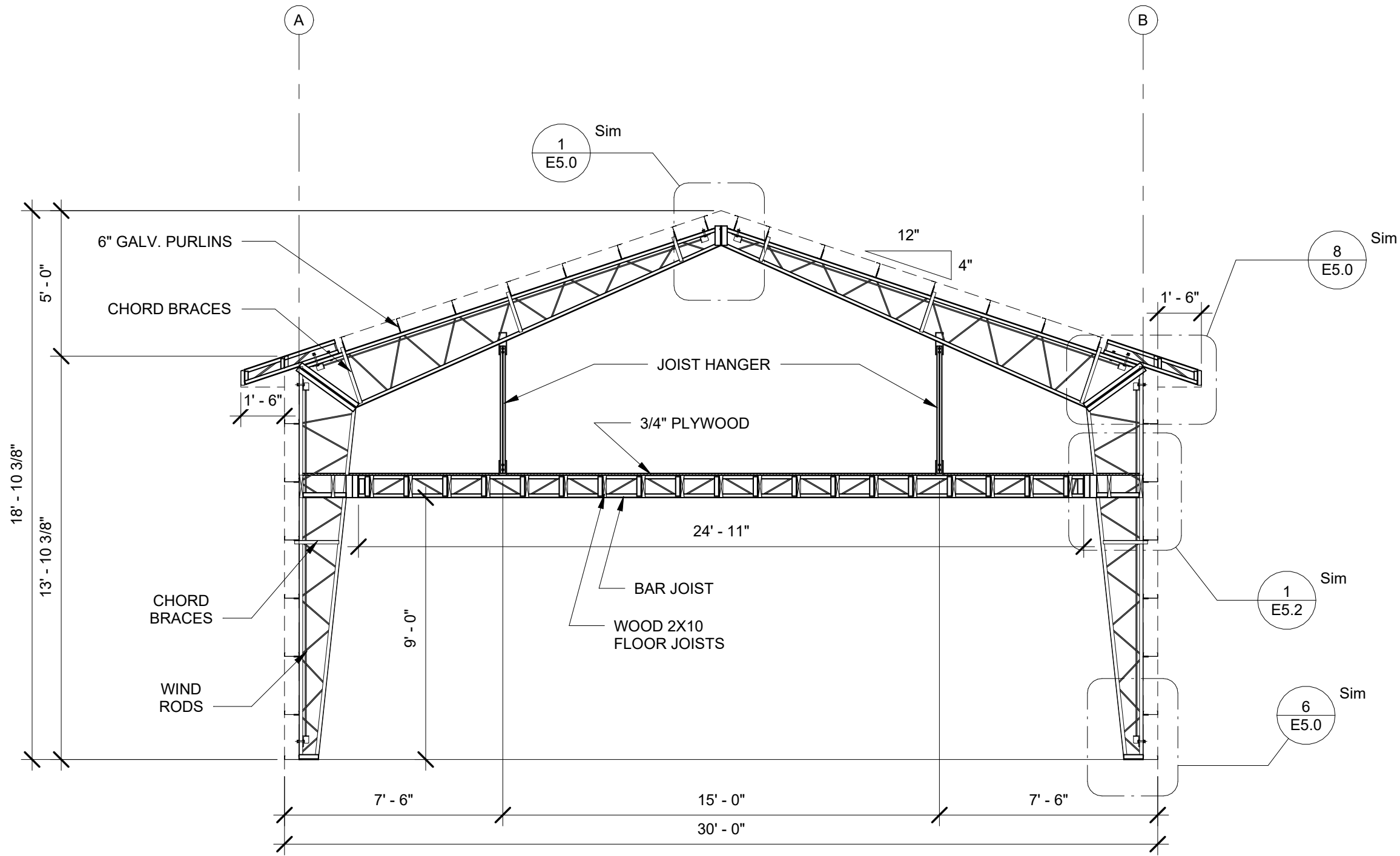
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BUILDING SECTION
(GRID 6)

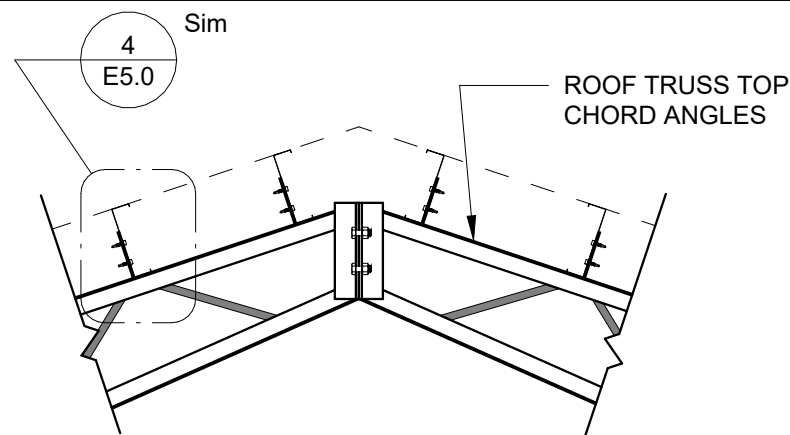
E4.1



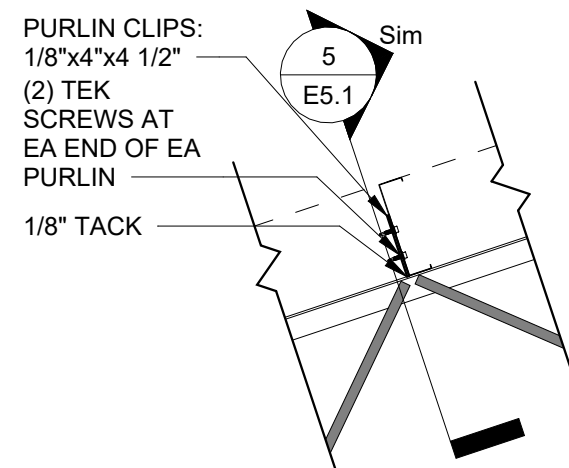
1 BUILDING SECTION (GRID 6)
1/4" = 1'-0"

NOTES:

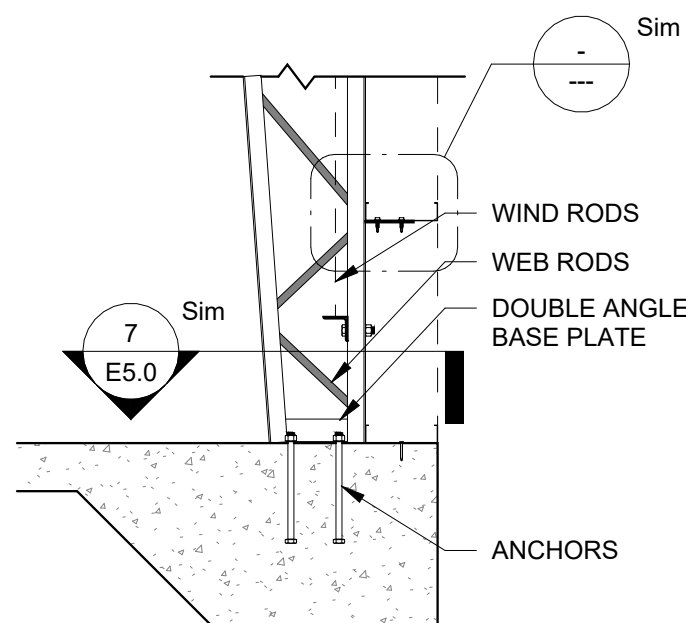
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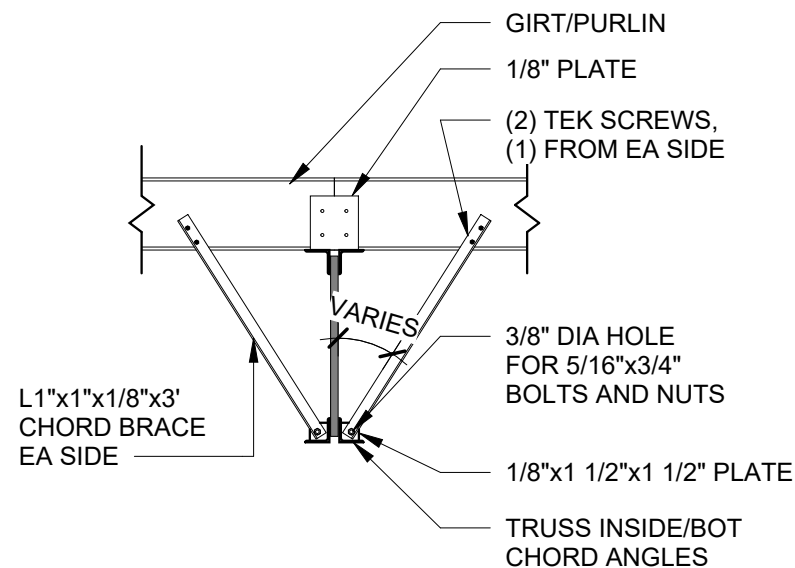
① TYP RIDGE CONNECTION
3/4" = 1'-0"



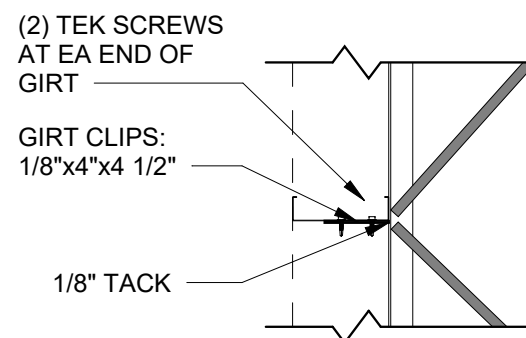
④ TYP PURLIN CONN
1" = 1'-0"



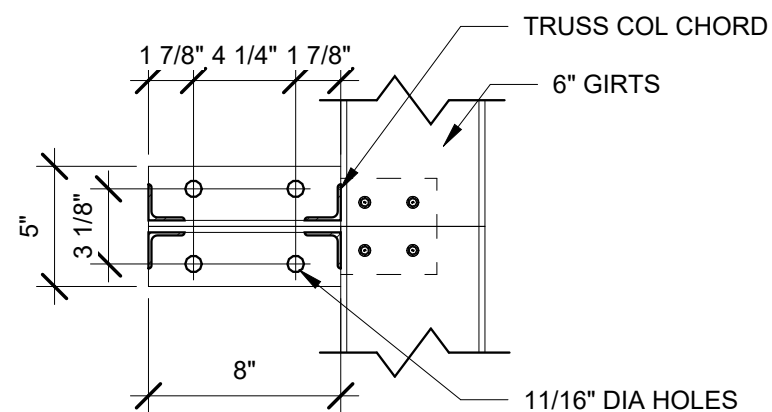
⑥ TYP COL BASE
3/4" = 1'-0"



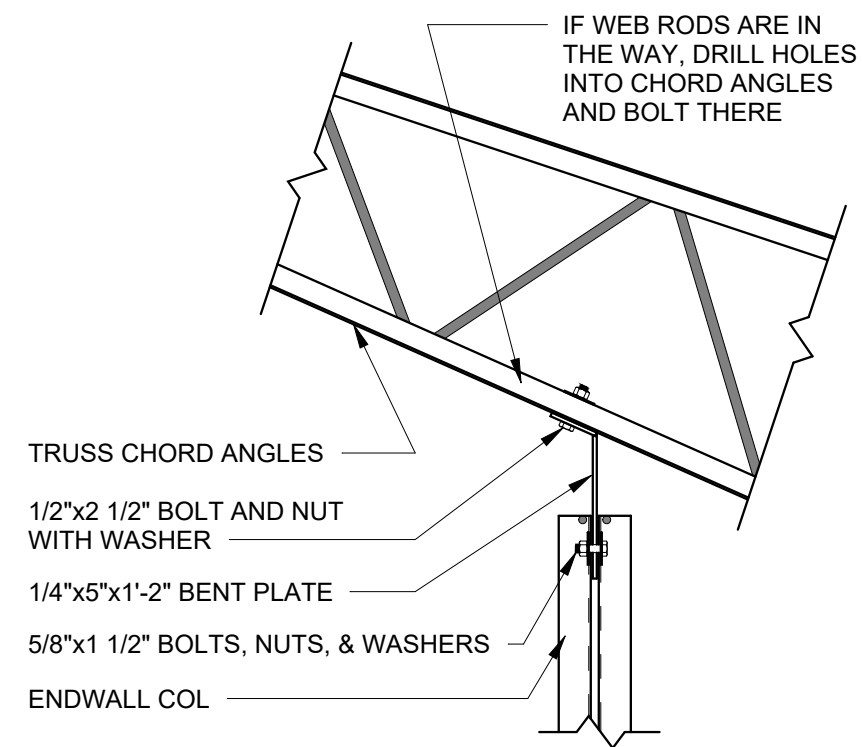
② TYP CHORD BRACE
3/4" = 1'-0"



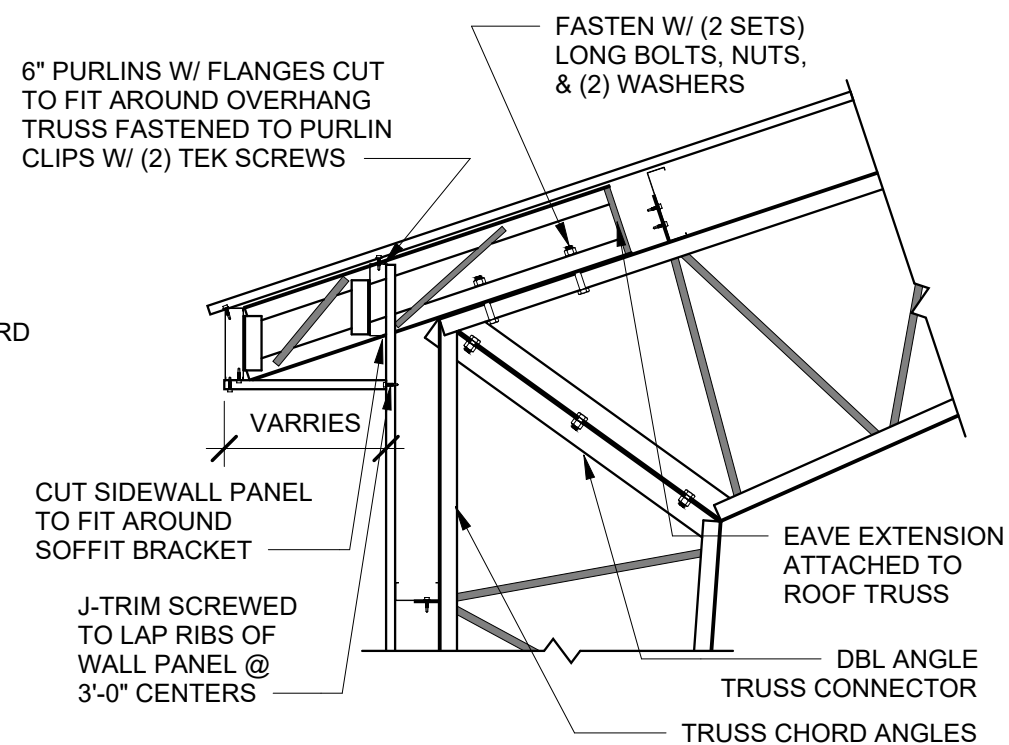
⑤ TYP GIRT CONNECTION
1" = 1'-0"



⑦ TYP SIDE WALL COL BP
1 1/2" = 1'-0"



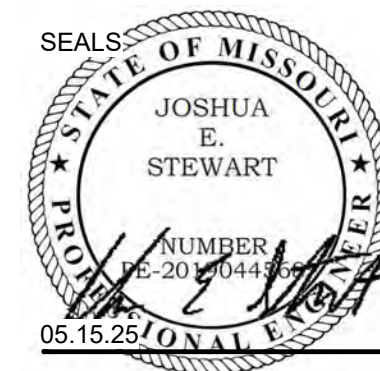
③ TYP ENDWALL COL CONN
1" = 1'-0"



⑧ EAVE EXTENSION @KNEE
3/4" = 1'-0"



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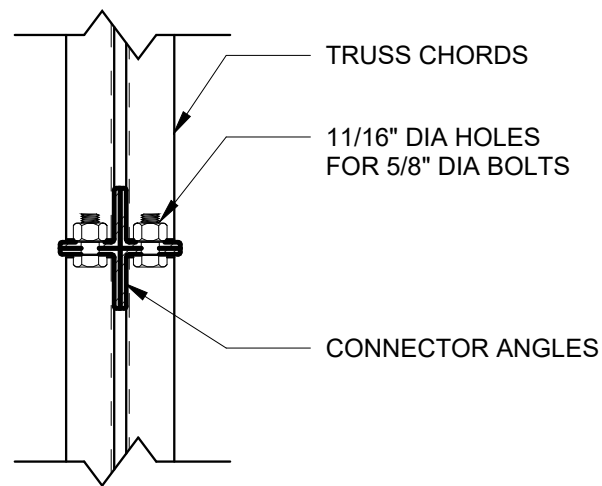
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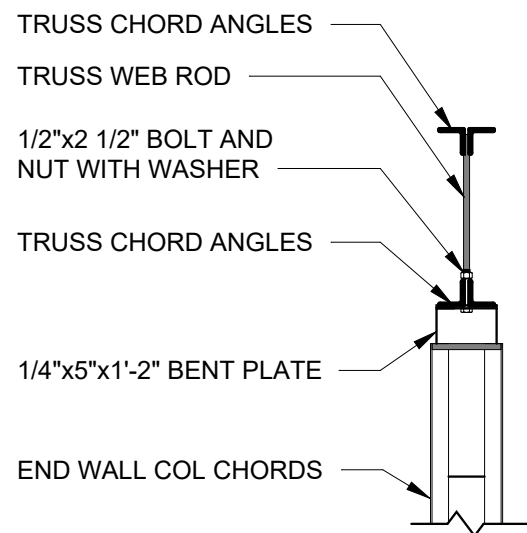
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DETAILS

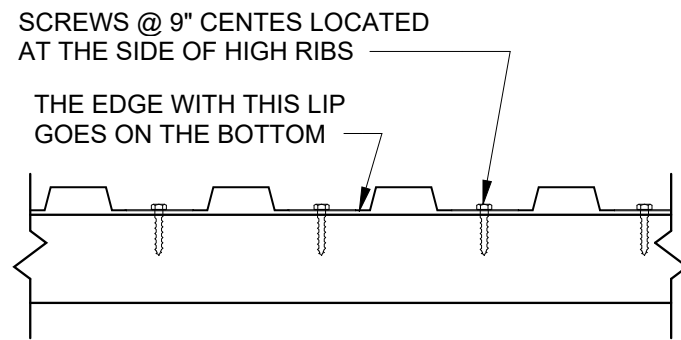
E5.0



① E5 TYP TRUSS CONNECTION
1 1/2" = 1'-0"

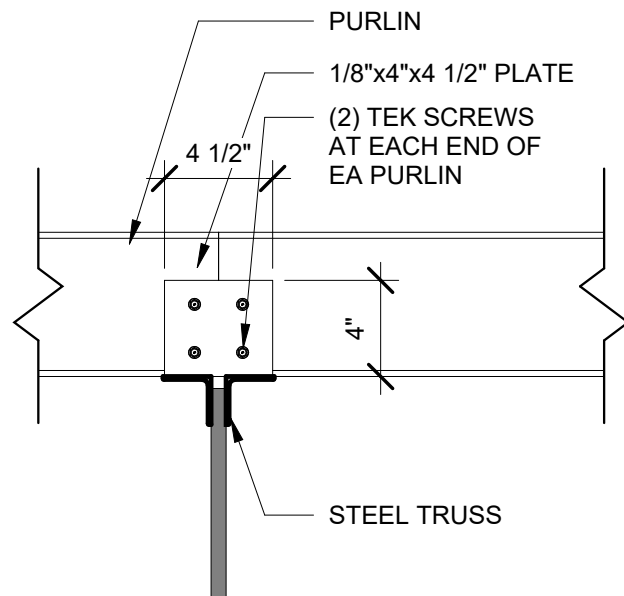


④ END WALL COL CONNECTION
3/4" = 1'-0"

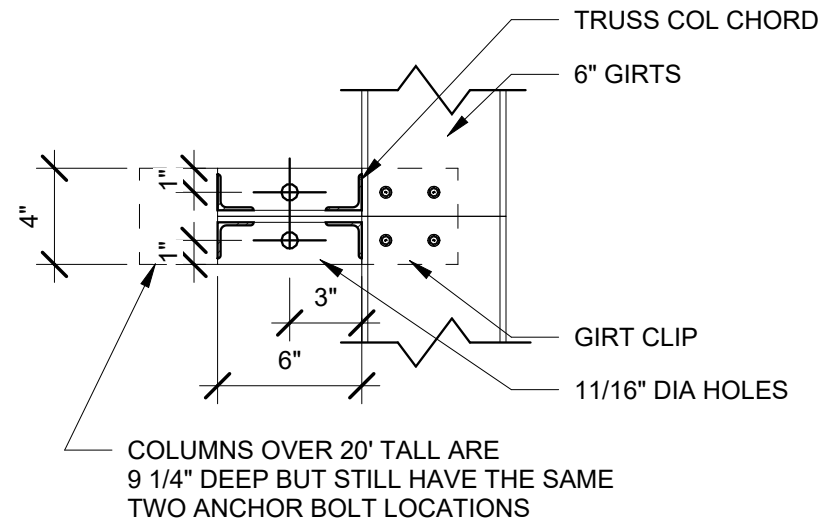


- 29 GAUGE (80 KSI) RIBBED PANEL AT ROOF AND WALLS
- ATTACH WITH #12x1 1/4" METAL-TO-METAL SELF DRILLING SCREWS WITH RUBBER WASHERS
- ALTERNATE: IF USING WOOD SUBFRAMING ATTACHED WITH #9x1 1/2" WOOD SCREWS

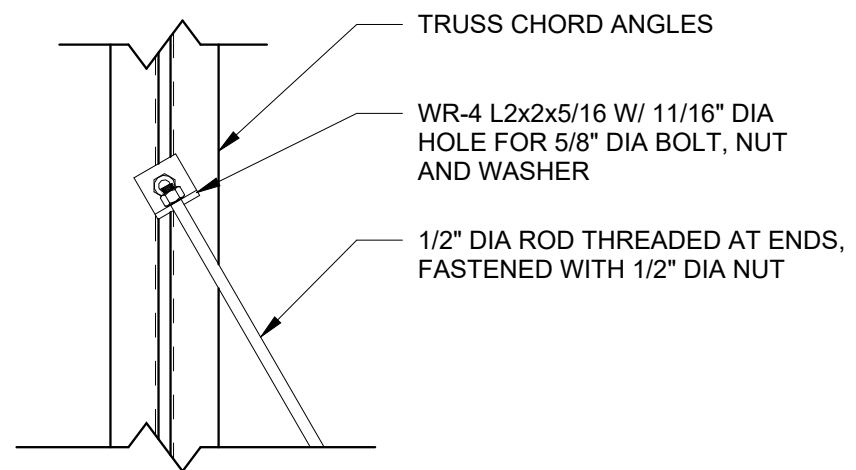
② WALL AND ROOF PANEL CONNECTION
1" = 1'-0"



⑤ TYP PURLIN CONN
1 1/2" = 1'-0"



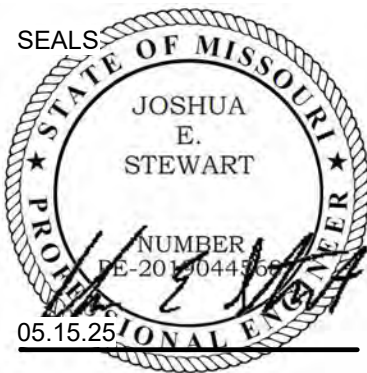
③ TYP END WALL COL BP
1 1/2" = 1'-0"



⑥ WIND BRACING CONNECTION
1 1/2" = 1'-0"



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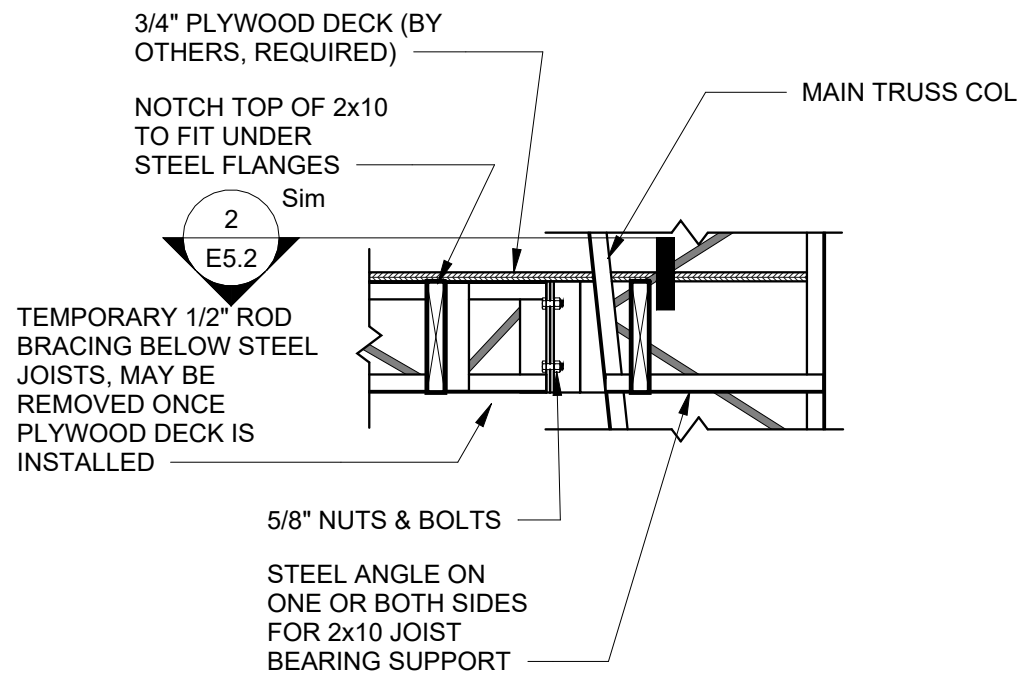
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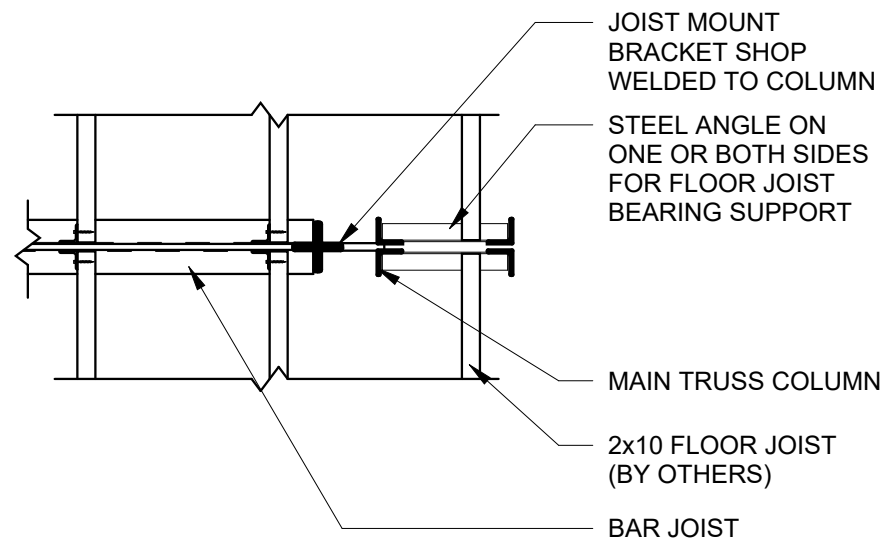
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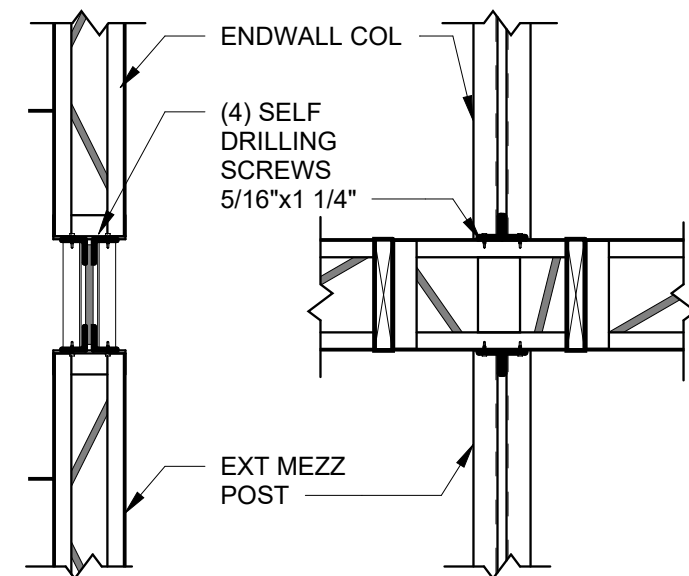
E5.1



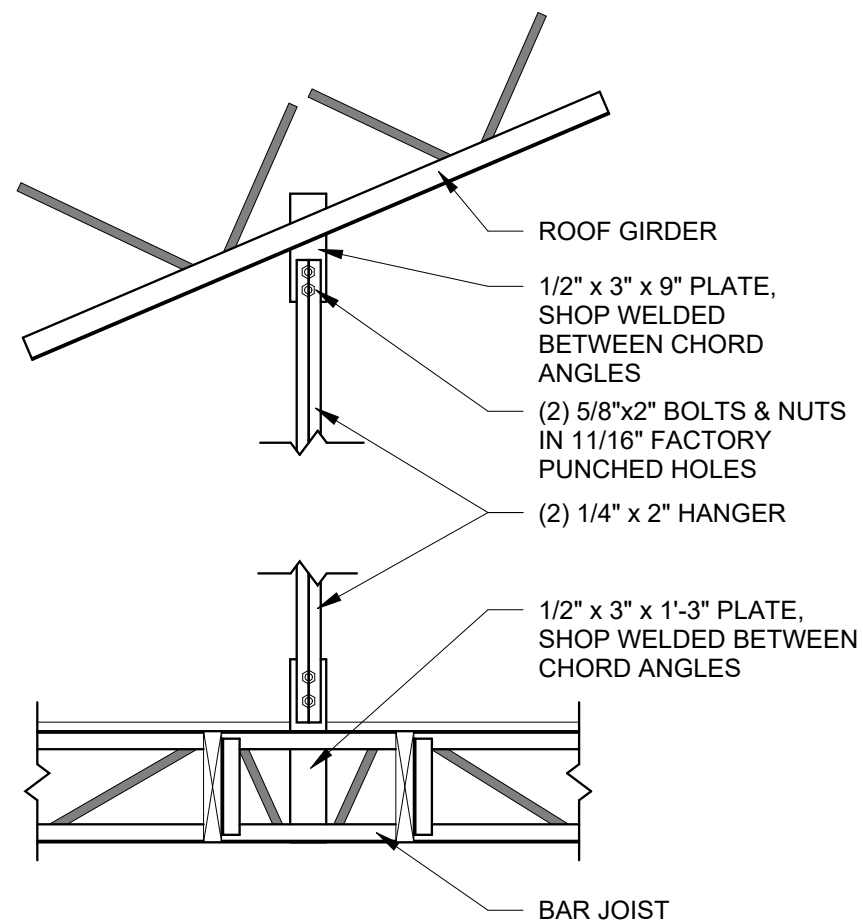
1 BAR JOIST CONNECTION
3/4" = 1'-0"



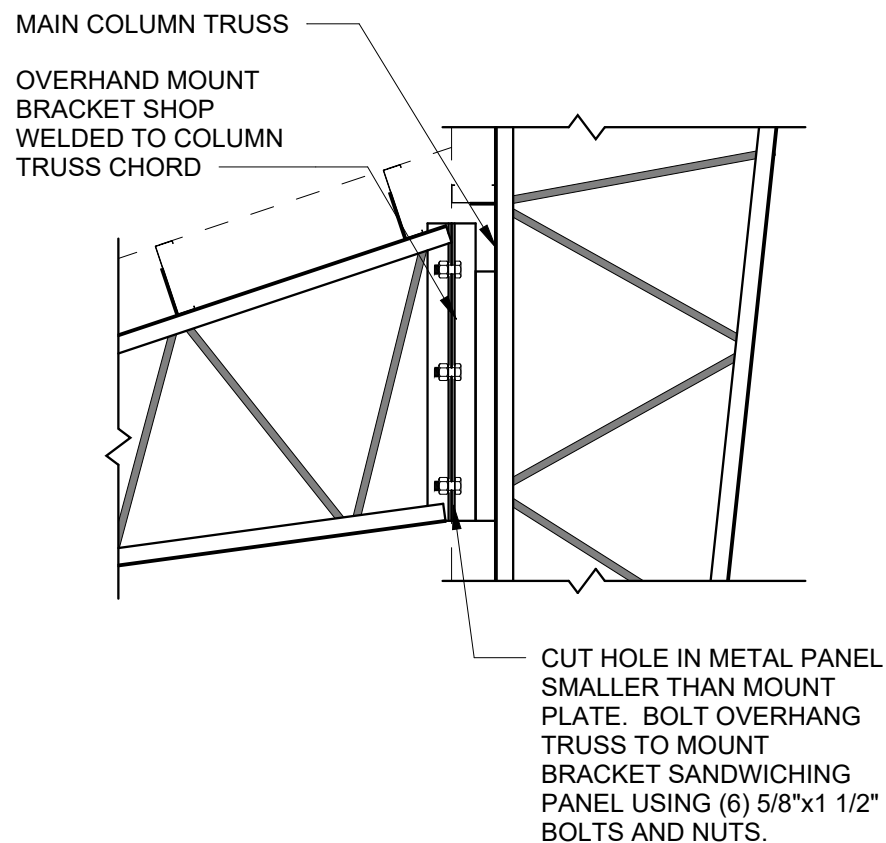
2 BAR JOIST CONNECTION (PLAN)
3/4" = 1'-0"



3 BAR JOIST POST CONN
3/4" = 1'-0"



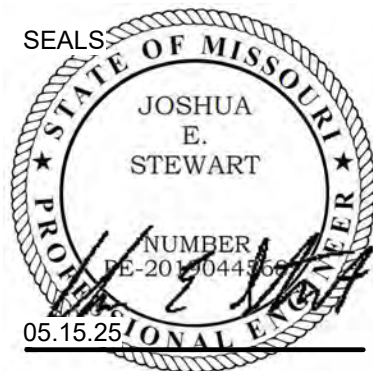
4 BAR JOIST HANGER
3/4" = 1'-0"



5 OVERHANG @ COL
3/4" = 1'-0"



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E5.2

FOUNDATIONS PLAN NOTES:

1.

CODE INFORMATION

A. 2018 VERSION OF THE INTERNATIONAL BUILDING CODE

B. REQUIREMENTS FOR STRUCTURAL CONCRETE, ACI 318
2.

CONCRETE SLAB CONSTRUCTION SHALL BE: 4" THICK CONCRETE W/ #4 BARS @ 24" OC EA WAY OR 6x6-W1.4xW1.4 WWF FLAT SHEETS OVER COMPACTED GRANULAR FILL
3.

SEE ERECTION DRAWINGS FOR ALL DIMENSIONS NOT SHOWN
4.

ALL CONCRETE SHALL BE MINIMUM 3,000 PSI
5.

ALL STEEL REINFORCEMENT BARS SHALL BE ASTM A615 GRADE 60
6.

ALL STEEL REINFORCEMENT SPLICES SHALL BE LAPPED A MINIMUM OF 24", UNLESS NOTED OTHERWISE
7.

ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED PER CRSI AND ACI STANDARDS, INCLUDING CONCRETE COVER AND BAR SUPPORTS (DESIRED METHOD OF SUPPORTING TOP BARS IN THICK MATS SHALL BE VERIFIED WITH ENGINEER.) PROVIDE CORNER BARS AT ALL FOOTINGS AND WALL INTERSECTIONS TO MATCH HORIZONTAL REINFORCING IN SIZE AND SPACING. AT INTERSECTIONS OF CONTINUOUS SPREAD FOOTINGS EXTEND ALL BARS TO THE FAR SIDE OF INTERSECTING FOOTING. LAP BARS AT ALL SPLICES, INCLUDING CORNER BARS AND DOWELS, 40 BAR DIAMETERS. LAP WWF 6" OR ONE FULL MESH, WHICHEVER IS GREATER.
8.

CONCRETE PROTECTION FOR REINFORCING: 3" AT FOOTINGS, 2" AT FORMED SURFACES LATER EXPOSED TO SOIL.
9.

ALUMINUM SHALL NOT BE EMBEDDED IN ANY CONCRETE.
10.

NO HOLES OR OPENINGS THROUGH FOUNDATION WALL AND/OR FOOTINGS WITHOUT ENGINEERS APPROVAL.
11.

ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4".
12.

CONTINUOUS FOOTINGS AND FLOOR SLABS SHALL HAVE KEYED CONSTRUCTION JOINTS SPACED AT 60'-0" MAX ON CENTER EACH WAY.
13.

PROVIDE (2) #4 X6' BARS AT MID-DEPTH OF SLAB AT RE-ENTRANT SLAB CORNERS.
14.

UNLESS SPECIFIED OTHERWISE THE CONTRACTOR SHALL PLACE SLAB JOINTS SUCH THAT CONTROL JOINTS ARE SPACED APPROXIMATELY 24 TO 36 TIMES THE SLAB THICKNESS. THIS IS BASED ON RECOMMENDATIONS PER ACI. SLAB AREAS SHALL ALSO BE LIMITED TO 450 SQUARE FEET. THE LENGTH TO WIDTH RATIO OF A JOINTED SECTION OF SLAB SHALL NOT EXCEED 1-1/2.

15.

FOUNDATION DESIGNED BASED ON FOLLOWING PROPERTIES: SOIL BEARING PRESSURE: 1,500 PSF SUBGRADE MODULUS: 100 PSF NOTE: PROOF ROLL THE EXPOSED SUBGRADE WITH A MEDIUM-WEIGHT ROLLER TO DETERMINE IF ANY POCKETS OF SOFT, UNSUITABLE MATERIAL EXIST BENEATH THE EXPOSED SUBGRADE. REMOVE ANY UNSUITABLE MATERIAL ENCOUNTERED AND REPLACE WITH A PROPERLY COMPACTED GRANULAR MATERIAL
16.

OWNER/CONTRACTOR SHALL RETAIN THE SERVICES OF A GEOTECHNICAL ENGINEER TO VERIFY THAT THE ABOVE NOTED FOUNDATION DESIGN PRESSURES AND MODULUS ARE MET
17.

OWNER/CONTRACTOR SHALL EMPLOY QUALIFIED INDEPENDENT SOIL-TESTING AGENCY TO VERIFY THAT ALL EXCAVATION AND BACKFILL OPERATIONS MEET THE CRITERIA OF THESE NOTES AND THE SPECIFICATIONS
18.

ENGINEERED FILL SHALL BE CLEAN, WELL GRADED, AND FREE DRAINING IN ITS COMPACTED STATE
19.

GRANULAR FILL MATERIAL SHALL BE A "PUT RUN GRAVEL" AS IT OCCURS IN A NATURAL STATE WITH NO LUMPS OF CLAY OR ROCKS LARGER THEN 2" IN DIAMETER. IT MUST CONFORM TO THE FOLLOWING GRADATIONS: 10-40 PERCENT SAND, 40 TO 80 PERCENT GRAVEL, AND 0 TO 15 PERCENT CLAY. OBTAIN FROM A BORROW PIT APPROVED BY THE OWNER AND THE TESTING AGENCY
20.

COMPACT ALL GRANULAR FILL BENEATH SLABS ON GRADE AND OVER FOOTINGS TO 95 PERCENT MODIFIED MAXIMUM DRY DENSITY, ASTM D-1557. INCREASE THE COMPACTION REQUIREMENTS FOR ENGINEERED FILL SUPPORTING FOOTINGS TO 97 PERCENT MODIFIED MAXIMUM DRY DENSITY, ASTM-1157. COMPACT ALL BACKFILL NOT SUPPORTING SLABS, PAVEMENT, OR FOOTINGS TO 90 PERCENT MODIFIED MAXIMUM DRY DENSITY, ASTM D-1557, PLACEMENT AND COMPACTION OF FILL SHALL BE OVERSEEN BY THE TESTING AGENCY. PLACE ALL GRANULAR FILL MATERIAL IN LAYERS NOT EXCEEDING 6" IN LOOSE THICKNESS. MECHANICALLY COMPACT EACH LAYER TO AT LEAST THE REQUIRED MINIMUM DRY DENSITY
21.

ALL BACKFILL PLACED AGAINST CONCRETE WALLS SHALL BE A WELL-GRADED, FREE-DRAINING, GRANULAR MATERIAL APPROVED BY THE TESTING AGENCY
22.

THICKENED SLAB FOUNDATION IS NOT SPECIFICALLY DESIGNED FOR FROST PROTECTION AND IS SUSCEPTIBLE TO ISSUES CAUSED BY FROST HEAVE. SUCH ISSUES INCLUDE, BUT ARE NOT LIMITED TO, DIFFERENTIAL SETTLEMENT, CRACKING, AND DOOR AND WINDOW FIT-UP ISSUES. WORLDWIDE STEEL BUILDINGS ASSUMES NO RESPONSIBILITY FOR ANY ISSUES RELATED TO FROST HEAVE. CLIENT ASSUMES ALL RESPONSIBILITY FOR CIVIL SITE WORK TO PRECLUDE WATER INFILTRATION BELOW SLAB. CLIENT UNDERSTANDS AND ACCEPTS SOLE RESPONSIBILITY FOR SLAB INTEGRITY AND SUSCEPTIBILITY TO FROST HEAVE

23.

ANCHOR BOLT OPTION(S):

•

CAST IN PLACE:

a.

MAIN BUILDING FRAME: 1/2" DIA (MIN 9" EMBEDMENT INTO FTG) F1554 GR. 55 THREADED ROD PROVIDE 3/8" PLATE WASHER AND DOUBLE NUT AT BOTTOM

b.

END WALL COLUMN: 1/2" DIA.X6" (MIN 4" EMBEDMENT) LONG F1554 GR. 36 THREADED ROD PROVIDE 3/8" PLATE WASHER AND DOUBLE NUT AT THE BOTTOM

•

POST INSTALLED:

a.

MAIN BUILDING FRAME: 1/2" DIA. (MIN 3 1/4" EMBEDMENT INTO FTG) THD50800H HEAVY-DUTY SCREW ANCHOR (OR APPROVED EQUIVALENT)

b.

END WALL COLUMNS:

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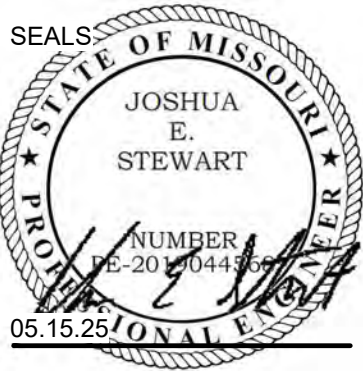
1/2"DIA.X6" LONG HILTI KWIK BOLT 3, MINIMUM 3 1/4" EMBEDMENT

•

1/2"DIA.X7" RED HEAD TRUBOLT + WEDHE ANCHOR, CARBON STEEL (MAXIMUM EMBEDMENT)
- NOTE: WHEN USING POST INSTALLED ANCHORS, HAIRPIN AND TIE BAR PLACEMENT IS CRITICAL. ANCHOR BOLTS MUST BE PLACED INSIDE (TOWARDS SLAB) OF HAIRPIN OR TIE BAR. TO ENSURE THIS, CLEAR COVER ON HAIRPINS AND TIE BARS SHALL NOT EXCEED 2" FROM SLAB EDGE
-
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-
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- | | |
|-------------|--------------|
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| CHK'D BY: | ACE |
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- FOUNDATION
SPECIFICATIONS
- F1.0



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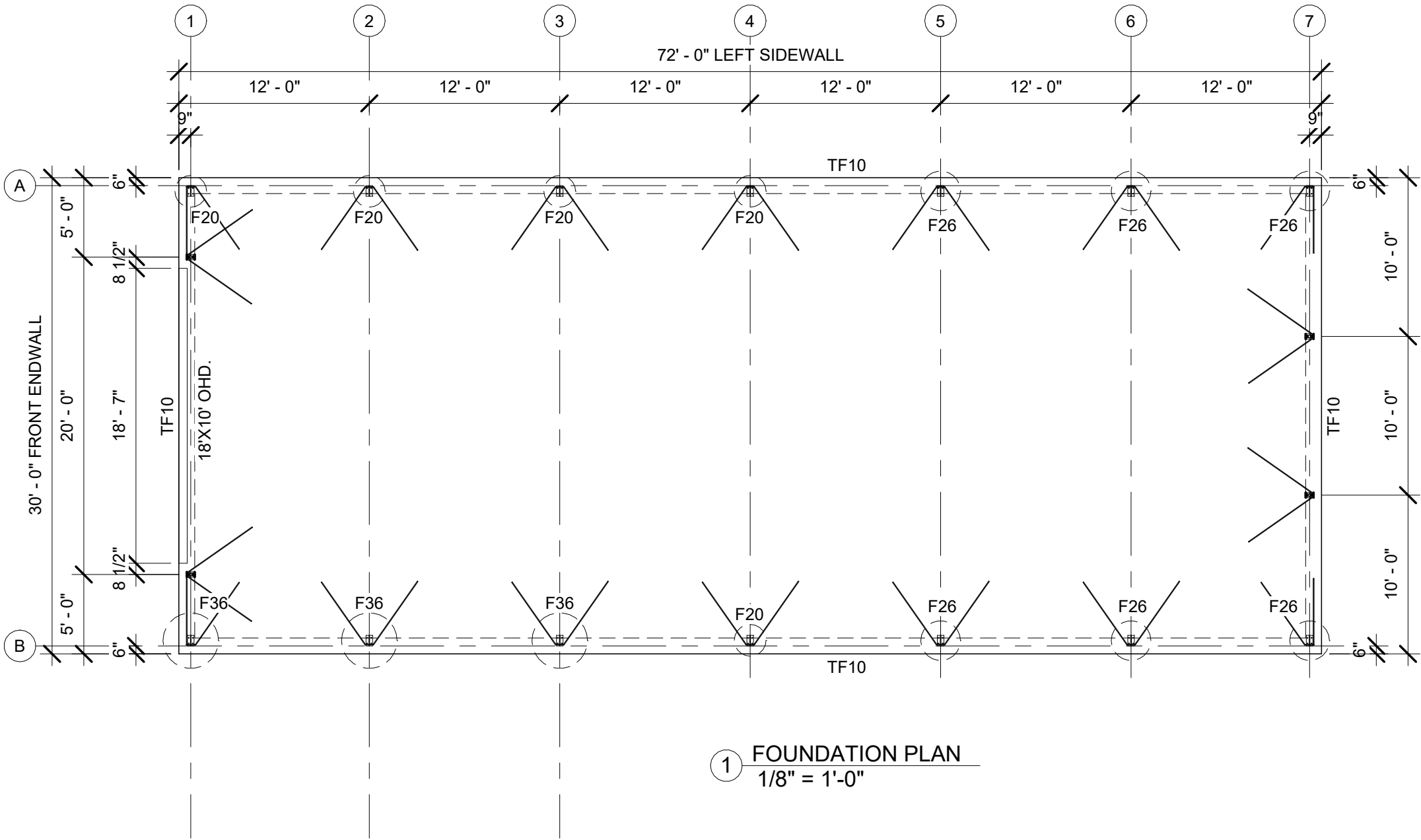
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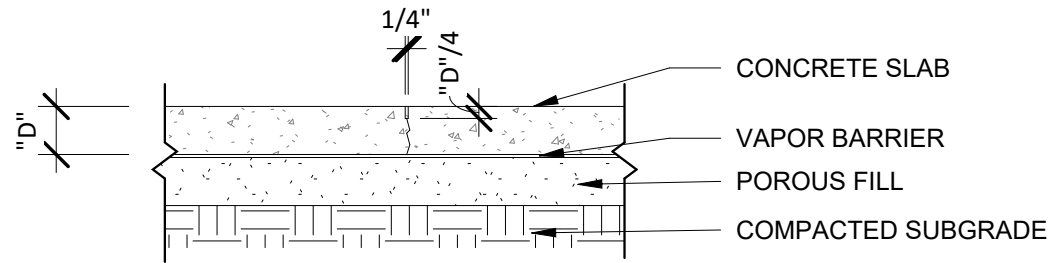
FOUNDATION PLAN

F2.0



1 FOUNDATION PLAN
1/8" = 1'-0"

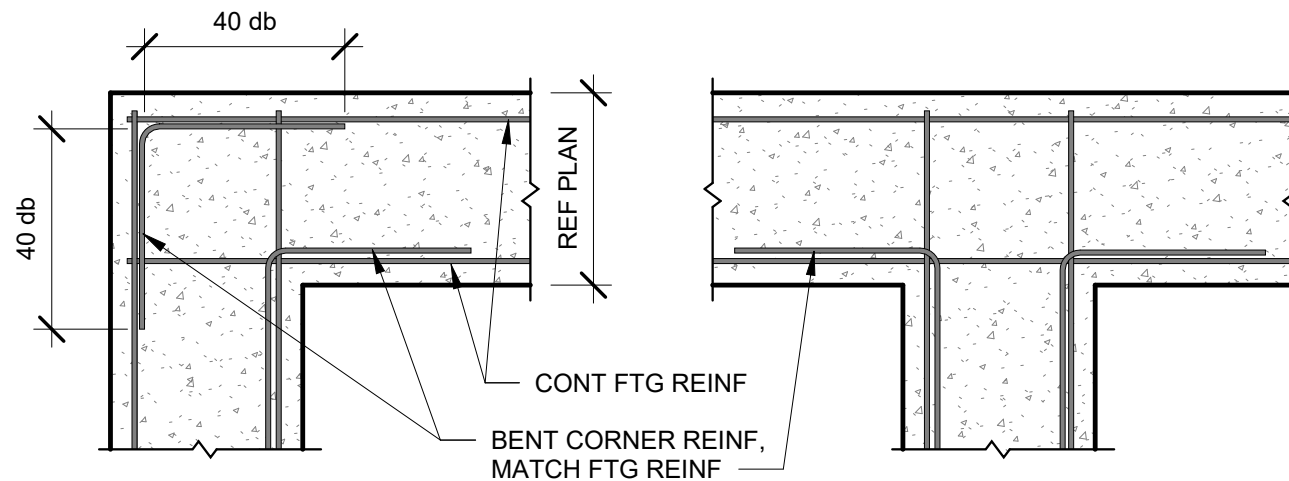
FOOTING SCHEDULE					
MARK	FOOTING SIZE	DEPTH, H	REINFORCING		COMMENT
			LONGITUDINAL	TRANSVERSE	
TF10	1'-0" x CONT	12"	2-#4	#4 @ 48" OC	TURNDOWN SLAB EDGE
F20	2'-0" Ø	24"	8-#4	#4 STIRRUPS @ 12" OC	TYP COL FTG
F26	2'-6" Ø	24"	8-#4	#4 STIRRUPS @ 12" OC	TYP COL FTG @ MEZZ
F36	3'-6" Ø	40"	8-#4	#4 STIRRUPS @ 12" OC	TYP COL FTG @ OH



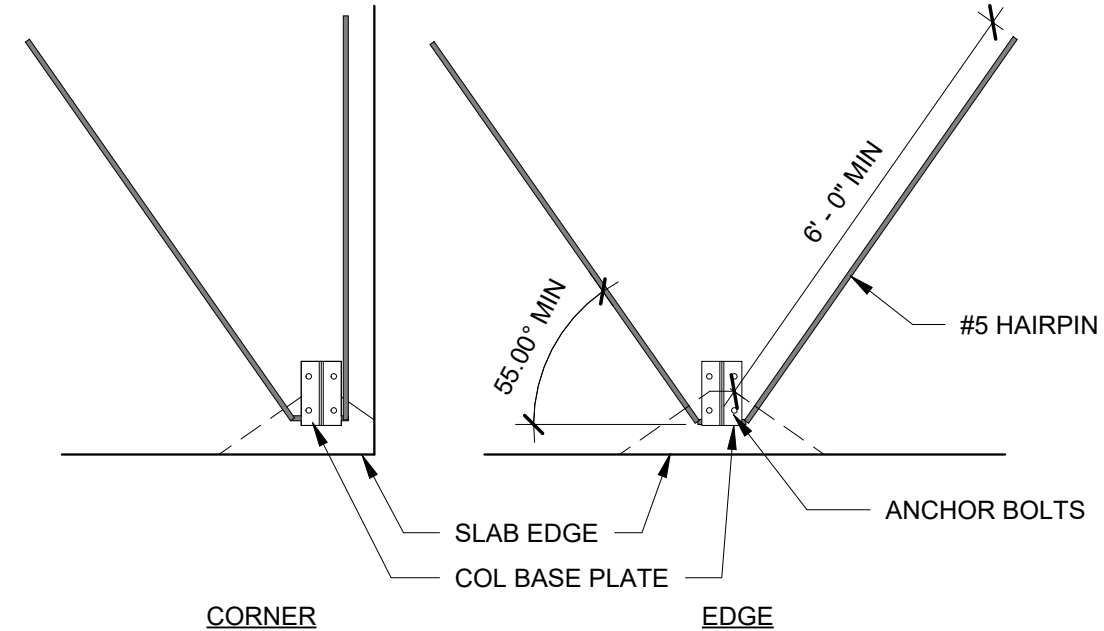
NOTES:

1. CONTRACTOR SHALL SAW-CUT CONTROL JOINTS IN CONCRETE SLABS AS SOON AS THE CONCRETE REACHES STRENGTH ADEQUATE TO PERFORM CUTS WITHOUT DAMAGING CONCRETE SURFACE.
2. A CONSTRUCTION JOINT RESULTS WHEN CONCRETE IS PLACED ON ONLY ONE SIDE OF THE DESIGNATED CONTROL JOINT TO END A DAY'S "PLACEMENT" OR TO ACCOMMODATE A DELAY IN PLACING SEQUENCE. AT CONSTRUCTION JOINTS, CONTRACTOR SHALL USE A CONTINUOUS 24 GAUGE GALVANIZED STEEL TONGUE AND GROOVE JOINT FORM. THE CONCRETE SHALL BE FINISHED TO THE TOP OF THE FORM. DO NOT EDGE/ TOOL AGAINST THE FORM. THE KEYWAY SHALL BE SUPPORTED BY 1 1/8"X1/8" THICK RIBBED STEEL STAKES AT 24" OC WITH ADDITIONAL BRACING AS REQUIRED TO HOLD THE JOINT STRAIGHT AND LEVEL. THE KEYWAY SHALL BE REMOVED AND THE PENETRATIONS THRU THE VAPOR BARRIER SHALL BE PATHCED WITH VAPOR BARRIER MANUF APPROVED TAPE/ SEALANT PRIOR TO PROCEEDING WITH THE SUBSEQUENT ADJACENT SLAB CONCRETE PLACEMENT.

1 CONTROL JOINT "CJ" DETAIL
3/4" = 1'-0"



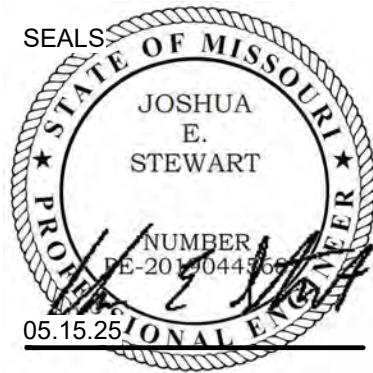
2 TYPICAL FTG CORNER REINF
1/2" = 1'-0"



3 TYPICAL HAIRPIN
1/2" = 1'-0"



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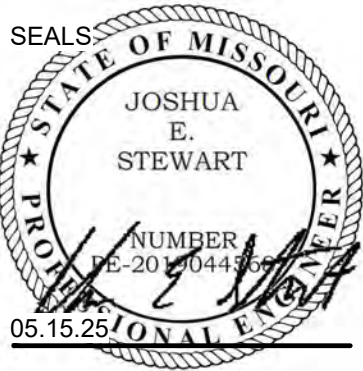
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F3.0



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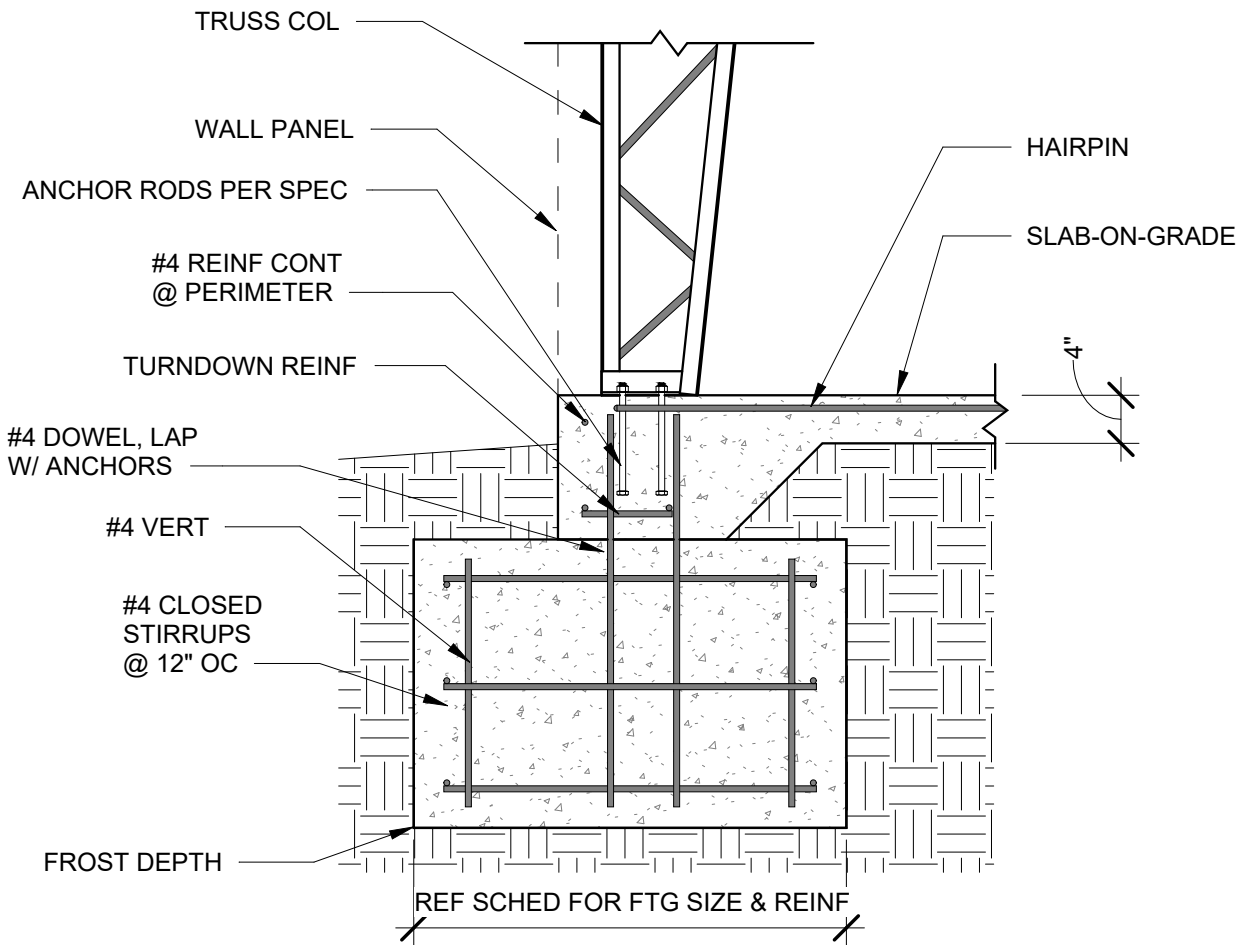
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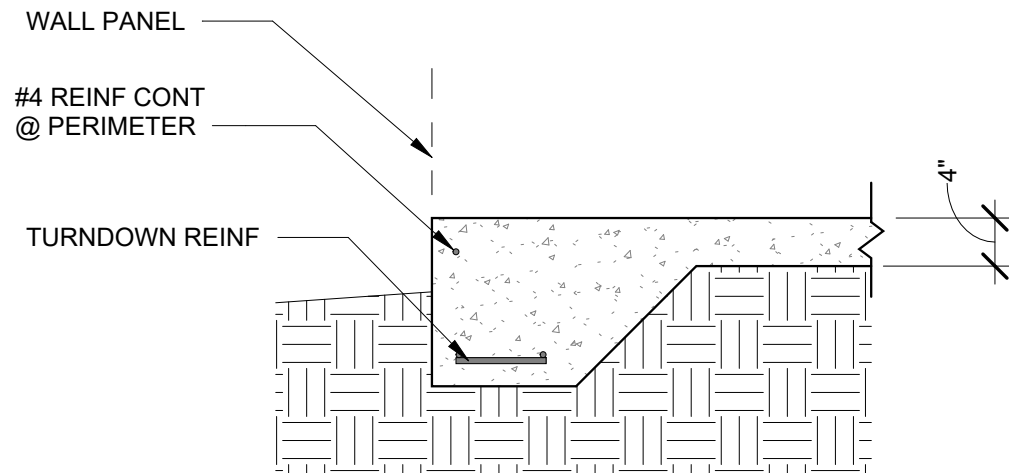
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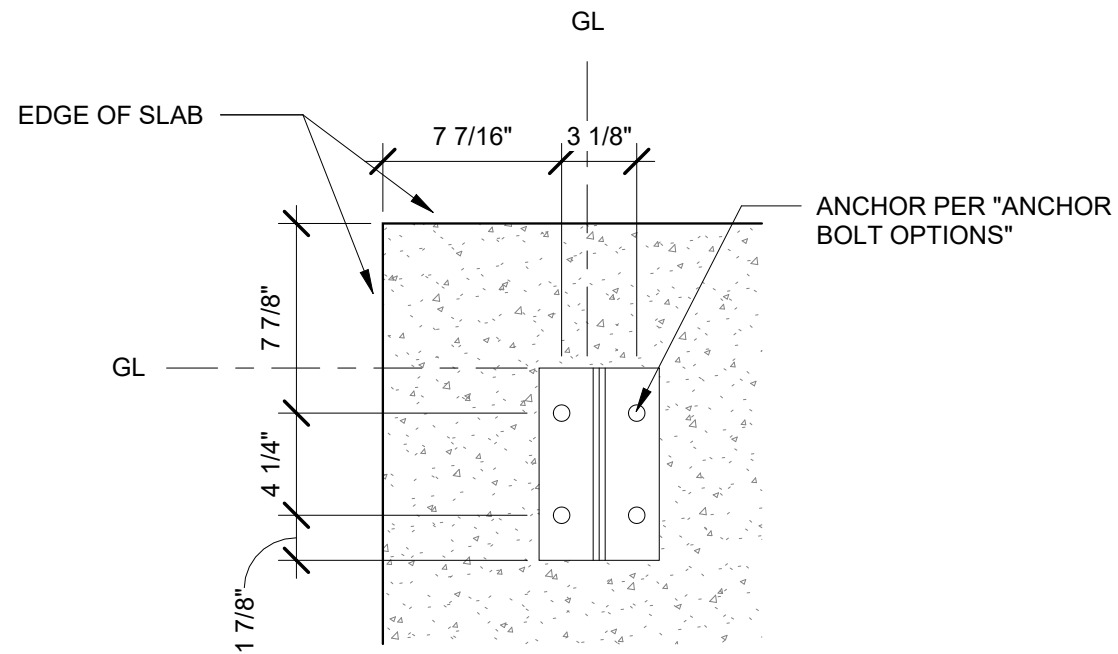
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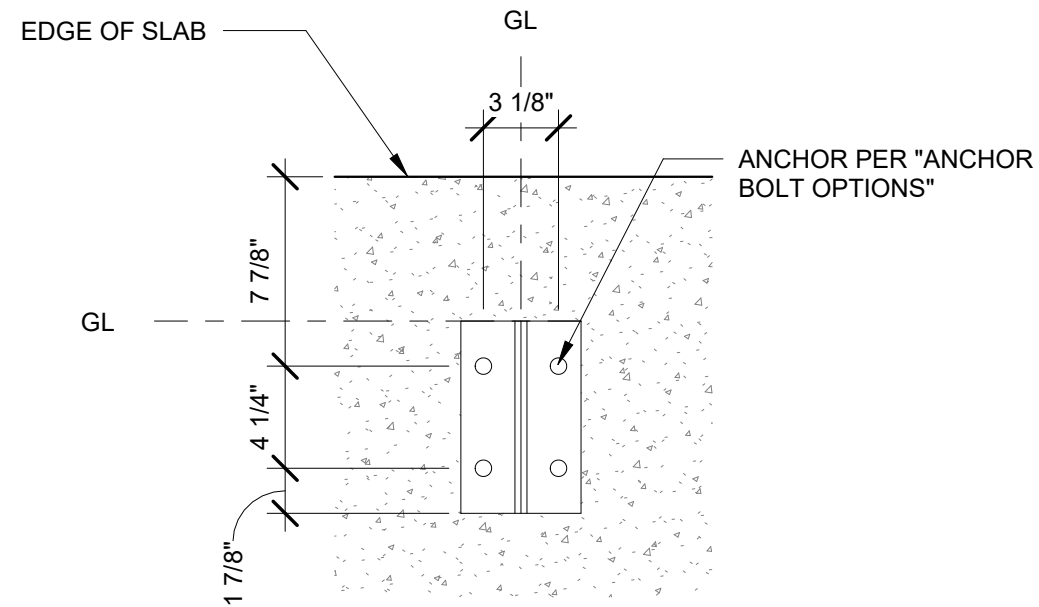
1 TYPICAL TURNDOWN
3/4" = 1'-0"



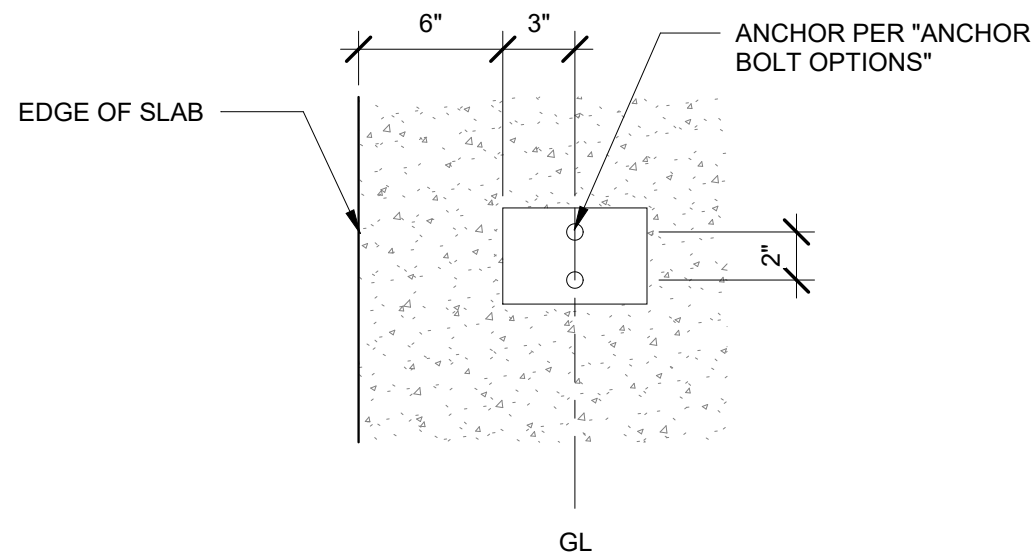
2 TYPICAL TURNDOWN
3/4" = 1'-0"



1 CORNER COL BASE PLATE
1 1/2" = 1'-0"



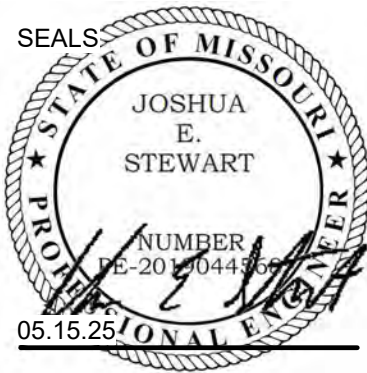
2 SIDEWALL COL BASE PLATE
1 1/2" = 1'-0"



3 ENDWALL COL BASE PLATE
1 1/2" = 1'-0"



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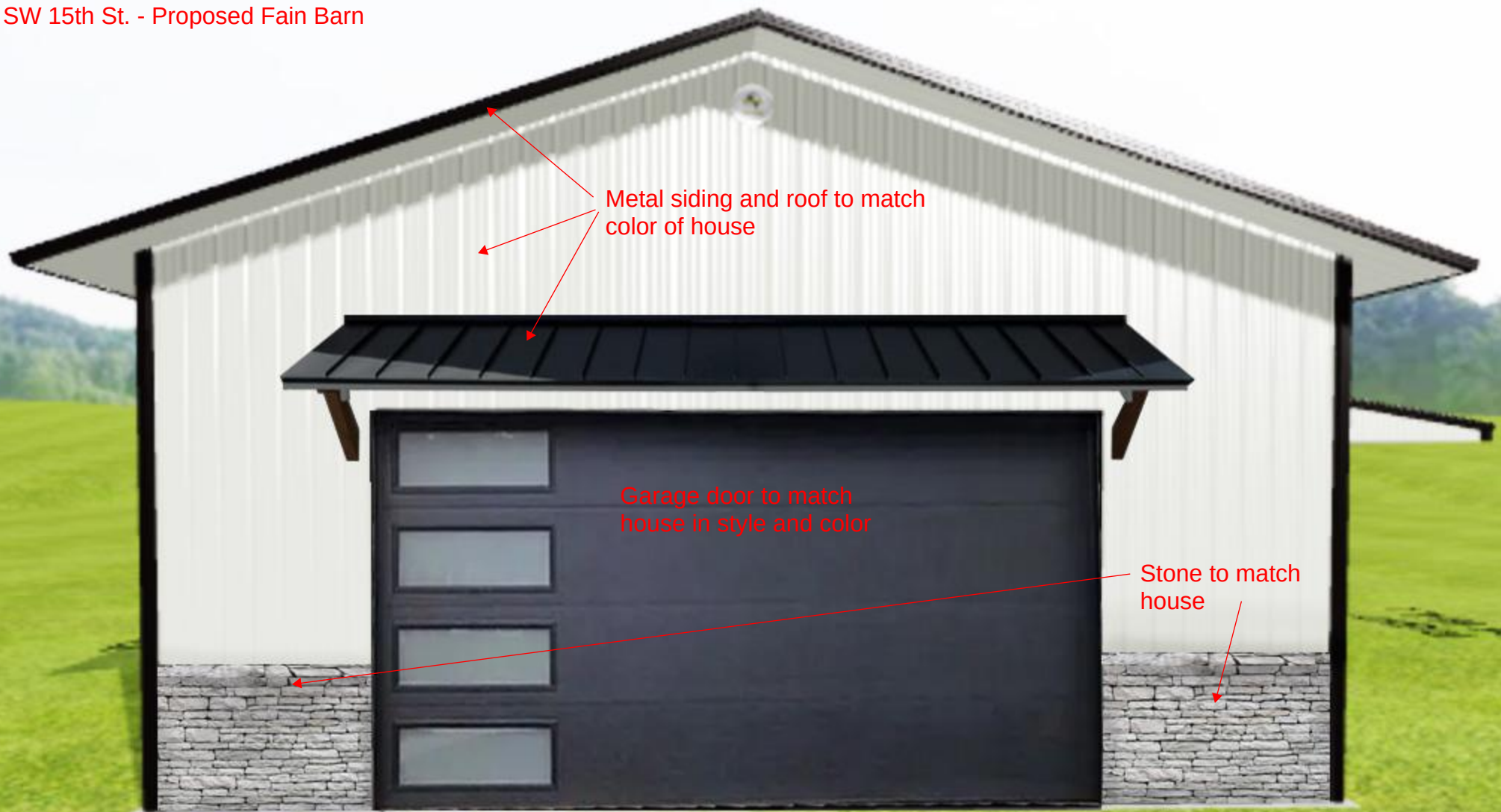
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F3.2



**FRONT
END WALL**