

RELEASE FOR CONSTRUCTION AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 05/19/2025

RE: P250360-01 Roof - BY Lot 2050

Site Information:

Customer:Clayton PropertiesProject Name:P250360-01Lot/Block:2050Model:Heather TransitionalAddress:1432 Silo St.Subdivision:Bailey FarmsCity:Lee's SummitState:MO

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014 Wind Code: ASCE 7-16 Roof Load: 45.0 psf Design Program: MiTek 20/20 8.6 Wind Speed: 115 mph Floor Load: N/A psf

This package includes 23 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	168913058	A1	10/16/2024	21	168913078	V5	10/16/2024
2	168913059	A2	10/16/2024	22	l68913079	V6	10/16/2024
3	168913060	A3	10/16/2024	23	168913080	V7	10/16/2024
4	168913061	A4	10/16/2024				
5	168913062	A5	10/16/2024				
6	168913063	A6	10/16/2024				
7	168913064	A7	10/16/2024				
8	168913065	A8	10/16/2024				
9	168913066	B1	10/16/2024				
10	168913067	B2	10/16/2024				
11	168913068	B3	10/16/2024				
12	168913069	B4	10/16/2024				
13	168913070	C1	10/16/2024				
14	168913071	C2	10/16/2024				
15	168913072	PB1	10/16/2024				
16	168913073	PB2	10/16/2024				
17	168913074	V1	10/16/2024				
18	168913075	V2	10/16/2024				
19	168913076	V3	10/16/2024				
20	168913077	V4	10/16/2024				

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc under my direct supervision based on the parameters provided by . Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of Missouri is December 31, 2025. Missouri COA: 001193

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek. Any project specific information included is for MiTek customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



October 16, 2024

Sevier, Scott

MiTek, Inc. 16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200

										RELEASE	FOR CONS	TRUCTION	
Job	Tru	JSS	Truss Type		Qty	Ply	Roof - B	Y Lot 2050		AS NOTE	D FOR PLA	N REVIEW	l
P250360-01			Piggyback Base 9	Structural Gable	1	1					1689130	SOURI	
Dramier Building	Supply (Springhill, K	S) Spring Hills KS 66082	T Iggyback Base C		2024 Dr	int: 9 630 6 6	Job Refe	rence (opt	ional				l
Premier Building	Supply (Springhill, K	5), Spring Hills, KS - 66083,		ID:jRjVuSRmtuG2P	nuKUZ?E	DXKyg?FV-Rf	26 2024 Mill C?PsB70Hq3NS	gPqnL8w3u	TXbG	KWrCDorr 4250?f	9/2	025	
	-0-10-8	0.5.10	14 2 4 19 9 0	22 7 12	2070		26.1.9		12 7	· o	10 7 9	50-6-0	
	0-10-8	9-5-10	4-9-10 4-4-12	4-11-12	4-11-12	2	7-6-0		7-6	-0	6-0-0	0-10-8	
				5x8 II 4x4=		5x8 II							
τ τ		12	44 12) 21	~~						
		612	4x4 = 13				22	4×4					
		6x6 ≠	108011-		Ra		2381	4×4≈					
0-0-9		^{4x6} = 9		3×8 1 6	3			2526	4x6 ≈				
- <u>3-6</u> 7.		6 ^{7 °}	3x8 m	3	x 8 n	×			2728				
10				85			7		R.	29 6x6 ≈	12,		
	3	79.7		3x8 II			73 3	6 u		30 31 ₈	2		
	. 2		xo II 3x8 1		300	⁵ W	3x6 II			78	32	330.00	
									~~~~	4/		50°34 0° ⊑ 0°_⊥	
0	4x6= 62	2 8361 60 5958 5	56845554 5352	561650 49 48 4	17 46	6 45 4	4 4342 47	4075	76	39 3837 3685	5 35	4x6=	
			3x6 II 3x6=	4x8=		3x8= 3	(6∥ 3x6=	3x6=		3x6 II			
		9-5-10	^{0x0=} 18-6-12 14-2-0 14-3-4	28-8-1	2		36-1-8		13-6	43-7-8	10-7-8		
		9-5-10	4-8-6 0-1-4 4-3-8	10-2-0	)		7-4-12		7-4-	12 0-1-4	6-0-0		
Scale = 1:90													
Plate Offsets (2	X, Y): [8:0-3-0,0-4	4-8], [14:0-4-4,0-2-8], [21	:0-4-4,0-2-8], [30:0-4-1	2,0-3-8], [40:0-2-8,0-1	-8], [55:0	0-2-8,0-3-0]							-
Loading	(ns)	) Spacing	1-11-4	CSI			in (loc	l/defl	I /d		GRIP		-
TCLL (roof)	(ps 25.	0 Plate Grip DOL	1.15	TC	0.76	/ert(LL)	-0.01 39-40	>999	240	MT20	197/144		
TCDL	10.	0 Lumber DOL	1.15	BC	0.55	/ert(CT)	-0.02 39-40	>999	180				
BCLL	0. 10	0 Rep Stress Incr	YES IRC2018/TPI2014	WB Matrix-S	0.79	Horz(CT)	0.04 37	n/a	n/a	Weight: 346 lb	FT = 20%		
	10.		FORCES			Acrimentes				Wolght. O TO IS	11-20%	,	-
TOP CHORD	2x6 SPF No.2		FURGES	Tension	pression	/waximum							
BOT CHORD	2x4 SP No.2		TOP CHORD	1-2=0/12, 2-3=-943/7	/98, 3-4=	-975/850,							
WEBS	3S 2x3 SPF No.2 4-5=-995/911, 5-7=-1015/975, ERS 2x3 SPE No.2 7-8=-1050/1016, 8-9=-1410/1356.												
BRACING	2.0 011 10.2			9-10=-1485/1463, 10	)-11=-14	50/1460,							
TOP CHORD	Structural wood	sheathing directly applie	d or	11-12=-1159/1177, 12-13=-981/1053, 13-14=-899/1004_14-15=-809/909									
	5-1-4 oc purlins, 2-0-0 oc purlins	except (8-2-4 max ): 14-21		15-16=-804/904, 16-17=-804/904,									
BOT CHORD	Rigid ceiling dire	ectly applied or 5-0-4 oc		17-18=-804/904, 18-	19=-391	/615,							
WEBS	bracing.	14-51 21-45 15 66		21-22=-468/704, 22-2	23=-542	/730,							
JOINTS	1 Brace at Jt(s):	63,		23-24=-593/722, 24-2	25=-595	/673,							
	64, 65, 67, 69, 7 72, 73, 76, 77	1,	23-20=-490/030, 20-27=-404/390, 27-29=-499/541, 29-30=-539/570,										

72, 73, 76, 77 30-31=-957/846, 31-32=-992/857, **REACTIONS** (size) 37=0-3-8, 38=29-5-8, 39=29-5-8, 32-33=-960/806, 33-34=0/12 40=29-5-8, 41=29-5-8, 43=29-5-8, BOT CHORD 2-62=-731/977, 61-62=-731/977, 44=29-5-8, 45=29-5-8, 46=29-5-8, 60-61=-731/977, 59-60=-731/977, 47=29-5-8, 48=29-5-8, 49=29-5-8, 58-59=-731/977, 57-58=-731/977, 52=29-5-8, 54=29-5-8, 55=29-5-8 56-57=-731/977, 55-56=-731/977, Max Horiz 55=-179 (LC 13) 54-55=-1328/1655, 52-54=-1328/1655, Max Uplift 37=-504 (LC 9), 38=-386 (LC 26), 51-52=-1328/1655, 49-51=-738/985, 39=-111 (LC 13), 40=-35 (LC 25), 48-49=-738/985, 47-48=-738/985, 41=-36 (LC 13), 43=-56 (LC 13), 46-47=-738/985, 45-46=-738/985, 44=-56 (LC 13), 45=-28 (LC 9), 44-45=-489/652, 43-44=-489/652, 46=-35 (LC 9), 47=-20 (LC 9), 41-43=-489/652, 40-41=-489/652, 48=-9 (LC 8), 49=-47 (LC 8), 39-40=-777/995, 38-39=-777/995, 52=-69 (LC 12), 54=-81 (LC 12), 37-38=-777/995, 36-37=-755/973, 55=-296 (LC 12) 35-36=-755/973, 33-35=-755/973 37=1037 (LC 26), 38=200 (LC 9), Max Grav 39=326 (LC 26), 30=200 (LC 9), 39=326 (LC 26), 40=215 (LC 26), 41=77 (LC 26), 43=161 (LC 1), 44=226 (LC 1), 45=296 (LC 26), 46=218 (LC 1), 47=130 (LC 26), 48=118 (LC 25), 49=332 (LC 25), 52=306 (LC 1), 54=317 (LC 25),

55=1416 (LC 25)

October 16,2024

16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200/MTek-US.com

Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

						RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type	Qty	Ply	Roof - BY Lot 2050	
P250360-01	A1	Piggyback Base Structural Gable	1	1	Job Reference (optional	LEE'S SUMMIT, MISSOURI
Premier Building Supply (Springh	ill, KS), Spring Hills, KS - 66083,	Run: 8.63 S Sep 26 2 ID:jRjVuSRmtuG2Pnu	2024 Print: 8 JKUZ?DXKy	.630 S Sep 2 g?FV-RfC?Ps	6 2024 MiTek Industries, Inc. sB70Hq3NSgPqnL8w3uITXbG	ue Oct 10 17:51 9/2025

WEBS	$\begin{array}{l} 8-58=-204/151, 8-70=-897/1040,\\ 69-70=-838/970, 55-69=-867/1003,\\ 14-51=-338/294, 21-45=-369/231,\\ 45-72=-230/270, 72-73=-226/266,\\ 73-74=-227/266, 25-74=-236/278,\\ 25-40=-298/122, 40-75=-369/461,\\ 75-76=-353/445, 30-78=-365/460,\\ 30-37=-524/440, 18-63=-407/358,\\ 63-71=-376/331, 45-71=-372/327,\\ 51-66=-477/541, 65-66=-397/480,\\ 64-65=-420/500, 18-64=-458/548,\\ 19-63=-94/37, 47-63=-89/44, 17-64=-78/34,\\ 48-64=-93/32, 16-65=-235/153,\\ 49-65=-261/176, 15-66=-99/71,\\ 13-67=-235/164, 52-67=-227/158,\\ 12-68=-433/392, 54-68=-421/380,\\ 10-69=-123/55, 56-69=-82/38,\\ 9-70=-183/116, 57-70=-85/61, 7-59=-103/67,\\ 5-60=-67/51, 4-61=-67/48, 3-62=-88/57,\\ 20-71=-186/105, 46-71=-182/102,\\ 22-72=-192/141, 44-72=-188/134,\\ 23-73=-118/94, 43-73=-116/94,\\ 24-74=-67/30, 41-74=-72/37, 26-75=-53/51,\\ 27-76=-12/12, 29-77=-215/147,\\ 39-77=-227/155, 38-78=-39/52,\\ 31-36=-12/83, 32-35=-131/77,\\ \end{array}$
	27-76=-12/12, 29-77=-215/147,
	39-77=-227/155, 38-78=-39/52,
	31-36=-122/83, 32-35=-131/77,
	11-55=-600/528, 11-68=-873/900, 67-68=-862/890, 51-67=-855/883
NOTES	

### N

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 4-1-8, Interior (1) 4-1-8 to 18-8-0, Exterior(2R) 18-8-0 to 23-7-12, Interior (1) 23-7-12 to 28-7-8, Exterior(2R) 28-7-8 to 33-7-8, Interior (1) 33-7-8 to 50-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Provide adequate drainage to prevent water ponding. 4) All plates are 3x4 MT20 unless otherwise indicated. 5)
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads. 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 55, 28 lb uplift at joint 45, 35 lb uplift at joint 40, 20 Ib uplift at joint 47, 9 lb uplift at joint 48, 47 lb uplift at joint 49, 69 lb uplift at joint 52, 81 lb uplift at joint 54, 35 Ib uplift at joint 46, 56 lb uplift at joint 44, 56 lb uplift at joint 43, 36 lb uplift at joint 41, 111 lb uplift at joint 39, 386 lb uplift at joint 38 and 504 lb uplift at joint 37.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value of use only with with with every connectors, this design is based only upon parameters shown, and is for an individual building domponent, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses and truss systems, see **ANSI/TPI Quality** Criteria, and **DS-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



													RELEAS	E FOR CO	NSTRUCTION
Job		Truss		Trus	s Type			Qty	Ply	Roof - B	Y Lot 2050	0		ED FOR P	AN REVIEW
P250360-01		A2		Pig	gyback Base			6	1	Job Refe	I68913059 Job Reference (optional LEE'S SUMMIT, MISSOUR				
Premier Building St	⁻ remier Building Supply (Springhill, KS), Spring Hills, KS - 66083, ID:jRjVuSRmtuG2PnuKUZ?DXKyg?FV-RfC?PsB70Hq3NSgPqnL8w3ulTXbC											s, Inc. Tue Oo uITXbGKWrC		19/	2025
	-0-10-8	7-4-10	14	4-4-12	18-8-0	2	3-7-12	28-7-8	3	36-1-8		43-7-8		49-7-8	50-6-0
	0-10-8	7-4-10	' 7	7-0-2	4-3-4	4	-11-12	4-11-12	2 '	7-6-0		7-6-0		6-0-0	0-10-8
						5xo II 6	7	·	8						
0-0-0-6			6 ¹²	4x6 = 4	6x6 =					25	4x4 <b>≈</b> 9	4.0			
10-3-6 0 + 10- 0 + 7-6		23	4x4 = 3	and the second s		×			×			4x6≈ 10	6x12=	12 14 26	
2 <u></u> 2	1 2					$\mathbb{V}$						B			12 ₁₃ 0
$\top - 9_{+}$	4x6=	27	22		28 ₂₁ 20	19	18 29	9	17	16	15		14 ·	30	4x6=
			3x4 <b>I</b>		6x6=	5x8=	4x6=		3x8=	3x6=	3x8=		3x6 II		
					18×6=12								12 7 0		
		7-4-10	. 14	4-2-0	14-4-12		28-8	3-12		36-1-8		43-6-4	43-7-8	49-7-8	

Scale = 1:90

### Plate Offsets (X, Y): [6:0-4-4,0-2-8], [8:0-4-4,0-2-8], [15:0-2-8,0-1-8]

<b>Loading</b> TCLL (roof) TCDL BCLL BCDL	(psf) 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018	8/TPI2014	<b>CSI</b> TC BC WB Matrix-S	0.55 0.93 0.86	<b>DEFL</b> Vert(LL) Vert(CT) Horz(CT)	in 0.62 0.48 0.01	(loc) 17-19 17-19 14	l/defl >560 >731 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 272 lb	<b>GRIP</b> 197/144 FT = 20%			
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD	2x6 SPF No.2 2x4 SP No.2 2x3 SPF No.2 *Exce 5-21,5-19,7-17,7-19: Structural wood she 5-8-6 oc purlins, exc 2-0-0 oc purlins (6-0	ppt* 2x4 SP No.2 athing directly applied cept -0 max.): 6-8.	1) 2) d or	Unbalanced this design. Wind: ASCE Vasd=91mph Ke=1.00; Cat exterior zone Interior (1) 4- 23-7-12, Inte 28-7-8 to 33-	roof live loads have 7-16; Vult=115mph ; TCDL=6.0psf; BC . II; Exp C; Enclose and C-C Exterior(2 1-8 to 18-8-0, Exter rior (1) 23-7-12 to 2 7-8, Interior (1) 33-	been ( (3-sec DL=6.( d; MW 2E) -0-1 rior(2R) 28-7-8, 7-8 to 5	considered for ond gust) Dpsf; h=35ft; FRS (envelop 0-8 to 4-1-8, ) 18-8-0 to Exterior(2R) 50-6-0 zone;	e)								
BOT CHORD	Rigid ceiling directly bracing, Except: 2-2-0 oc bracing: 19 6-7-7 oc bracing: 15	applied or 6-0-0 oc -21 -17.	2)	cantilever left right exposed members and Lumber DOL	and right exposed d; porch left and right forces & MWFRS =1.60 plate grip DC	; end v ht expo for rea DL=1.60	vertical left and sed;C-C for ctions shown; )	ł								
WEBS	1 Row at midpt	6-19, 8-17, 9-17, 11- 5-21, 5-19, 3-21, 7-1 7-19	-15, 3) 7, 4)	This truss ha	s been designed fo d nonconcurrent w	r a 10.0 ith any	vater ponding ) psf bottom other live load	Is.								
REACTIONS	(size) 14=0-5-8, Max Horiz 21=-185 ( Max Uplift 14=-568 ( Max Grav 14=1826	21=0-5-8 LC 17) LC 9), 21=-614 (LC 8 (LC 26), 21=2948 (LC	5) 3) C 1)	All bearings a capacity of 50 Provide mech bearing plate	are assumed to be 65 psi. nanical connection capable of withstar	SP No. (by oth nding 5	2 crushing ers) of truss to 68 lb uplift at	)								
FORCES	(lb) - Maximum Com Tension	pression/Maximum	7)	This truss is o	designed in accorda	ance w	ith the 2018 R502 11 1 ar	nd								
TOP CHORD	1-2=0/12, 2-3=-853/ 5-6=-347/614, 6-7=- 8-9=-871/707, 9-11= 11-12=-1007/922, 12	838, 3-5=-1579/1588 263/504, 7-8=-662/68 1040/793, 2-13=0/12	, 85, 8)	R802.10.2 ar Graphical pu or the orienta bottom chord	referenced stand rlin representation of tion of the purlin al	lard AN does no ong the	ISI/TPI 1. ot depict the si top and/or	ze				TE OF M	AISSOL			
BOT CHORD	2-22=-610/833, 21-2 19-21=-1331/1672, 15-17=-526/834, 14- 12-14=-795/1002	22=-610/833, 17-19=-146/452, -15=-873/1068,	LC	AD CASE(S)	Standard						R.	SCOTI SEVI	ER			
WEBS	6-19=-494/277, 8-17 9-17=-301/395, 9-15 11-15=-1581/1772, ' 5-21=-2452/2465, 5- 3-21=-829/978, 3-22 7-17=-869/653, 7-19	7=-216/68, 5=-355/156, 11-14=-1659/1330, -19=-2005/1821, 2=-497/327, 9=-1093/1074								2		PE-20010	118807 E F			
NUTES												A NA				

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling or individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

# October 16,2024



						RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type	Qtv	Plv	Roof - BY Lot 2050	AS NOTED FOR PLAN REVIEW
000			~.,	,	1001 01 2000	DEVELOPMENT SERVICES 168913060
P250360-01	A3	Piggyback Base	2	1	Job Reference (optional	LEE'S SUMMIT, MISSOURI
			-			

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. ue Oct 5 47.471 9/269:25 ID:CpRVt7ams0odYdBf7pXnXoyfzw6-RfC?PsB70Hq3NSgPqnL8w3ulTXbGf WrCDoi754259?



Scale = 1:90
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late Offsets (X, Y): [2:Edge,0-1-14], [5:0-4-4,0-2-8], [7:0-4-4,0-2-8], [10:0-5-4,0-3-8], [14:0-2-8,0-1-8]													
Loading TCLL (roof) TCDL BCLL BCDL	(psf) 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC201	8/TPI2014	CSI TC BC WB Matrix-S	0.72 0.68 0.90	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.70 0.53 0.11	(loc) 16-18 16-18 13	l/defl >740 >982 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 243 lb	<b>GRIP</b> 197/144 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD	2x6 SPF No.2 2x4 SP 2400F 2.0E 17-15,19-17:2x4 SP 2x3 SPF No.2 *Exce Structural wood she 3-0-13 oc purlins, ex 2-0-0 oc purlins (4-1 Bioid ocilian directly	*Except* 1650F 1.5E pt* 14-10:2x4 SP No athing directly applie ccept 1.14 max.): 5-7.	2) 5.2 ed or	Wind: ASCE Vasd=91mph Ke=1.00; Ca exterior zone Interior (1) 4- 23-7-12, Inte 28-7-8 to 33- cantilever lef right exposed and forces &	7-16; Vult=115mpl n; TCDL=6.0psf; B0 t. II; Exp C; Encloss and C-C Exterior( 1-8 to 18-8-0, Exterior (1) 23-7-12 to 3 7-8, Interior (1) 33- t and right exposed d; porch right exposed d; porch right exposed	n (3-sec CDL=6.0 ed; MW 2E) -0-1 rrior(2R 28-7-8, 7-8 to 5 I; end v sed;C-C ons sho	oond gust) Opsf; h=35ft; FRS (envelop 0-8 to 4-1-8, ) 18-8-0 to Exterior(2R) i0-6-0 zone; ertical left and f for members wn; Lumber	be) d					
WEBS	bracing. 1 Row at midpt	3-18, 5-18, 7-16, 8-1 10-14, 6-16, 6-18	16, 3) 4)	DOL=1.60 pl Provide adeo This truss ha	ate grip DOL=1.60 quate drainage to p s been designed fo	revent or a 10.0	water ponding ) psf bottom						
REACTIONS	(size) 2=0-5-8, 1 Max Horiz 2=-185 (L Max Uplift 2=-523 (L Max Grav 2=1978 (L (lb) - Maximum Com	13=0-5-8 C 17) C 9), 13=-750 (LC 9 _C 1), 13=2606 (LC pression/Maximum	5) ) 6) 1)	chord live loa All bearings a capacity of 8 Provide mec bearing plate joint 2 and 75	ad nonconcurrent w are assumed to be 05 psi. hanical connection capable of withsta 50 lb uplift at joint 1	vith any SP 240 (by oth inding 5 3.	other live load 0F 2.0E crush ers) of truss to 23 lb uplift at	ds. ning D					
TOP CHORD	Tension 1-2=0/12, 2-3=-3380 5-6=-2088/2438, 6-7 7-8=-2221/2432, 8-1 10-11=-994/911, 11-	0/3435, 3-5=-2507/26 '=-1864/2237, 0=-2139/2250, -12=0/12	7) 570, 8)	This truss is International R802.10.2 a Graphical pu or the orienta	designed in accord Residential Code s nd referenced stan rlin representation ation of the purlin a	ance w sections dard AN does no long the	ith the 2018 R502.11.1 a ISI/TPI 1. ot depict the si top and/or	nd ize					
BOT CHORD	2-20=-2892/2890, 18-20=-2892/2890, 16-18=-1937/2054, 14-16=-1828/1816, 13-14=-902/1103, 11-13=-785/991 2-20=-2892/2890, 18-20=-2892/2890, bottom chord. LOAD CASE(S) Standard												
VVEBO	5-18=-944/567 7-16	5=-918/1001, 5=-840/533									A	S SCOTI	M. EN

### NOTES

 Unbalanced roof live loads have been considered for this design.

8-16=-147/215, 8-14=-642/407,

10-14=-2870/2822, 10-13=-2430/2279, 6-16=-543/337, 6-18=-221/190



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITPTI Quality Criteria**, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



							RELEASE FOR CONSTRUCTION
Job	Truce				DIV	Poof RV Lot 2050	AS NOTED FOR PLAN REVIEW
300	11035	11035 1 ypc		QUY	ту	K001 - BT L01 2050	DEVELOPMENT SERVICES
P250360-01 A4		Piggyback Base		1	Job Reference (optional	LEE'S SUMMIT, MISSOURI	
Premier Building Supply (Springh	nill, KS), Spring Hills, KS - 66083,	Run: 8	3.63 S Sep 26 20	)24 Print: 8.	630 S Sep 2	6 2024 MiTek Industries, Inc.	ue Oct 1515710/29925

ID:iRkjCnRMbKBn0lyG89lvmGyfzti-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWCDoi7J420C+7719/26925

#### 50-6-0 0-10-8 28-7-8 9-6-5 18-8-0 23-7-12 36-1-8 43-7-8 49-7-8 9-6-5 9-1-11 4-11-12 4-11-12 7-6-0 7-6-0 6-0-0 5x8 u 4x4= 5x8 II 5 6 7 6¹² 4x6 ≠ 22 23 4x4 10-0-0 7-6-0 4x4 🤹 8 4x6**≈** 3 10-3-6 9 7x8≈ 12 14 21 10 24 ¹¹129 7 9⊥ 2-6-0 2-6-0 ÷ × 25 20 26 19 18 17 27 16 15 14 28 4x6= 6x6= 13 3x4 II 4x6= 3x8= 3x6= 3x8= 3x6= 3x8= 3x6 II 43-7-8 9-6-5 18-6-12 28-8-12 36-1-8 43-6-4 49-7-8 9-6-5 9-0-7 10-2-0 7-4-12 7-4-12 6-0-0 0-1-4

Scale =	1:90
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Plate Offsets (X, Y): [2:Edge,0-1-14], [5:0-4-4,0-2-8], [7:0-4-4,0-2-8], [10:0-5-4,0-3-8], [14:0-2-8,0-1-8]

Loading TCLL (roof) TCDL BCLL BCDL LUMBER	(psf) 25.0 10.0 0.0 10.0 2x6 SPE No 2	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018 2)	8/TPI2014 Wind: ASCE Vasd=91mph	CSI TC BC WB Matrix-S 7-16; Vult=115mj ; TCDL=6.0psf; E	0.73 0.68 0.90 ph (3-sec 3CDL=6.1	DEFL Vert(LL) Vert(CT) Horz(CT) ond gust) Dpsf; h=35ft;	in 0.70 0.53 0.11	(loc) 16-18 16-18 13	l/defl >737 >978 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 243 lb	<b>GRIP</b> 197/144 FT = 20%	
BOT CHORD	2x4 SP 2400F 2.0E 17-15,19-17:2x4 SP	*Except* 1650F 1.5E		Ke=1.00; Car exterior zone	t. II; Exp C; Enclo and C-C Exterior	sed; MW r(2E) -0-1	FRS (envelop 0-8 to 4-1-8,	be)						
WEBS	2x3 SPF No.2 *Exce	ept* 14-10:2x4 SP No	0.2	Interior (1) 4-	-1-8 to 18-8-0, EX	terior(2R)	18-8-0 to							
BRACING TOP CHORD	Structural wood she 3-0-5 oc purlins, exc 2-0-0 oc purlins (5-0	I wood sheathing directly applied or purlins, except       28-7-8 to 33-7-8, Interior (1) 33-7-8 to 50-6-0 zone; cantilever left and right exposed; end vertical left and right exposed; porch right exposed; C-C for members and forces & MWEPS for reactions shown: Lumber												
BOT CHORD	Rigid ceiling directly bracing.	applied or 3-6-11 oc	;	DOL=1.60 pl	ate grip DOL=1.6	tions sho 0	wn; Lumber							
WEBS	1 Row at midpt	5-18, 7-16, 8-16, 10 3-18, 6-16, 6-18	-14, 3) 4)	Provide adec This truss ha	uate drainage to is been designed	for a 10.0	vater ponding ) psf bottom	).						
REACTIONS	(size) 2=0-5-8, ² Max Horiz 2=-185 (L Max Uplift 2=-523 (L Max Grav 2=1978 (L	13=0-5-8 .C 13) .C 9), 13=-750 (LC 9 .C 1), 13=2606 (LC ⁻	5) ) 6) 1)	All bearings a capacity of 8 Provide mech bearing plate	ad nonconcurrent are assumed to be 05 psi. hanical connectio e capable of withsi	with any e SP 240 n (by oth tanding 5	other live load 0F 2.0E crush ers) of truss to 23 lb uplift at	as. hing o						
FORCES	(lb) - Maximum Com Tension	pression/Maximum	7)	joint 2 and 75 This truss is	50 lb uplift at joint designed in accor	13. dance w	th the 2018							
TOP CHORD	1-2=0/12, 2-3=-3376 5-6=-2088/2438, 6-7 7-8=-2221/2432, 8-1 10-11=-994/911, 11-	5/3430, 3-5=-2506/26 7=-1864/2238, 10=-2139/2249, -12=0/12	571, ⁷ 8)	International R802.10.2 ar Graphical pu or the orienta	Residential Code nd referenced star rlin representation ation of the purlin	e sections ndard AN n does no along the	R502.11.1 at SI/TPI 1. ot depict the s	nd ize						
BOT CHORD	2-20=-2887/2886, 18 16-18=-1937/2054, 1 13-14=-902/1103, 1	8-20=-2887/2886, 14-16=-1828/1816, 1-13=-785/990	LC	bottom chord DAD CASE(S)	d. Standard	0	·					TE OF M	AISSO	
WEBS	5-18=-946/571, 7-16 8-16=-147/216, 8-14 10-14=-2870/2822, 3-18=-917/1000, 3-2 6-16=-543/337, 6-18	6=-840/534, 4=-643/407, 10-13=-2430/2279, 20=-571/385, 3=-220/190										SCOTI SEVI	M. ER	2
NOTES										2		Joll I	Contes	
<ol> <li>Unbalanc</li> <li>this dosig</li> </ol>	ed roof live loads have	been considered for								-	87	PE-20010	18807 54	

this design.





October 16,2024

SSIONAL ET

Job P250360-01 Premier Building Supply (Springt	Truss A5 hill, KS), Spring Hills, KS - 66083,	Truss Type Piggyback Base Run: 8.63 S Sep 26: ID:yFuqM6NfS_usm7	Qty 1 2024 Print: 8 L39qByOVy	Ply 1 3.630 S Sep 2 /fzsV-RfC?PsI	Roof - BY Lot 2050 Job Reference (optional 6 2024 MiTek Industries, Inc. 370Hq3NSgPqnL8w3uITXbGi	RELEASE FOR CONSTRUCTION AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES 168913062 LEE'S SUMMIT, MISSOURI UNE Oct 0157:1719/2025
-0-10-8   0-10-8	9-5-10 9-5-10	18-8-0   23-7- 9-2-6 4-11- 5x8 ⊪	12 12 4x4=	28-7-8 4-11-12	36-0-4 7-4-12 5x8 II	<u>43-7-8</u> 7-7-4
9°°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -0°-0 -5×5 -5×5 -5×5 -5×5 -5×5 -5×5 -5×5 -5	6 3 18 18 1 9-5-10 9-5-10	12     4x8 #       12     19       4x4 #     4       4x4 #     4       10     10       17     16       15     5x4 #       18-6-12       9.1-2	28-8-1 10-2-0	2	20 20 14 13 3x8= 3x6= 36-0-4 7,3-8	$4x4_{s}$ 8 21 9 $6x6_{s}$ 10 9 11 12 3x4= 4x8= 43-7-8 7.7.4

Scale = 1:78.6

Plate Offsets (	ate Offsets (X, Y): [2:Edge,0-1-7], [5:0-4-4,0-2-8], [7:0-4-4,0-2-8], [11:Edge,0-1-8], [12:0-2-8,0-2-0]												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.69	Vert(LL)	-0.24	14-15	>999	240	MT20	197/144	
TCDL	10.0	Lumber DOL	1.15	BC	0.72	Vert(CT)	-0.54	14-15	>957	180			
BCLL	0.0	Rep Stress Incr	YES	WB	0.90	Horz(CT)	0.14	11	n/a	n/a			
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S							Weight: 217 lb	FT = 20%	
LUMBER TOP CHORD	2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; bcDL=6.0psf; h=35ft;												

BOT CHORD	2x4 SP 1650F 1.5E *Except* 13-11:2x4 SP No.2
WEBS	2x3 SPF No.2 *Except* 11-10:2x4 SP No.2
BRACING	
TOP CHORD	Structural wood sheathing directly applied or
	2-10-13 oc purlins, except end verticals, and
	2-0-0 oc purlins (4-10-10 max.): 5-7.
BOT CHORD	Rigid ceiling directly applied or 9-6-11 oc
	bracing.
WEBS	1 Row at midpt 8-14, 3-15, 6-14, 6-15
REACTIONS	(size) 2=0-5-8, 11=0-5-8
	Max Horiz 2=203 (LC 9)
	Max Uplift 2=-299 (LC 12), 11=-232 (LC 13)
	Max Grav 2=2025 (LC 1), 11=1945 (LC 1)
FORCES	(Ib) - Maximum Compression/Maximum
	Tension
TOP CHORD	1-2=0/12, 2-3=-3474/566, 3-5=-2606/539,
	5-6=-2177/547, 6-7=-2000/522,
	7-8=-2373/517, 8-10=-2459/453,
	10-11=-1871/369
BOT CHORD	2-17=-554/2973, 15-17=-554/2973,
	14-15=-345/2166, 12-14=-366/2108,
	11-12=-44/96
WEBS	5-15=-45/610, 7-14=-59/580, 8-14=-205/224,
	8-12=-449/185, 10-12=-335/2102,
	3-17=0/383, 3-15=-914/366, 6-14=-523/170,
	6-15=-235/181

- NOTES
- 1) Unbalanced roof live loads have been considered for this design.

- Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 4-1-8, Interior (1) 4-1-8 to 18-8-0, Exterior(2R) 18-8-0 to 23-7-12, Interior (1) 23-7-12 to 28-7-8, Exterior(2R) 28-7-8 to 33-7-8, Interior (1) 33-7-8 to 43-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Provide adequate drainage to prevent water ponding. 3) 4) This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads. Bearings are assumed to be: Joint 2 SP 1650F 1.5E 5)
- crushing capacity of 565 psi, Joint 11 SP No.2 crushing capacity of 565 psi. 6) Provide mechanical connection (by others) of truss to
- bearing plate capable of withstanding 299 lb uplift at joint 2 and 232 lb uplift at joint 11.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not beigh valid for use only with with with with the contractions. This design is based only door plantaters shown, and is for an individual dualing component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria**, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



						RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type	Qty	Ply	Roof - BY Lot 2050	AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES
P250360-01	A6	Piggyback Base	8	1	Job Reference (optional	I68913063 LEE'S SUMMIT, MISSOURI
Premier Building Supply (Springl	nill, KS), Spring Hills, KS - 66083,	Run: 8.63 S Sep 2 ID:46ptwcZ7N5Ey\	6 2024 Print VBruvPxxM/	: 8.630 S Sep 2 Ayfzqz-RfC?PsE	16 2024 MiTek Industries, Inc. 1 370Hq3NSgPqnL8w3uITXbGK	ue Oct 0157/19/2925
0.10.8			- / -			
-0-10-8 	<u>9-5-10</u> 9-5-10	+ 18-8-0 23- 9-2-6 4-1	<u>/-12</u> 1-12	<u>28-7-8</u> 4-11-12	<u> </u>	<u>43-7-10</u> 7-7-5
9 <del>.</del> -0-0-0 -0-0-0 -0-0-0 -0-0-0 -0-0 -0-0	6 3 18 1 9-5-10 9-5-10	5x8 II 5 4x8 = 5 4x4 = 4 7 7 7 16 15 5x4 II 4x6 = 3x8 = 1 18-6-12 9-1-2	4xi 6 28-8- 10-2	4= 	5x8 II 7 20 20 5x8 14 14 13 3x8 3x6 3x6 - 36-0-5 7-3-9	$4x4_{s}$ $8$ $4x8_{s}$ $21 \text{ g}$ $6x6_{s}$ $10$ $11^{1}$ $12$ $3x4=$ $43-7-8$ $43-7-10$ $7-7-3$ $0-0-2$

Scale =	1:78.6
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Plate Offsets (	ate Offsets (X, Y): [2:Edge,0-1-7], [5:0-4-4,0-2-8], [7:0-4-4,0-2-8], [11:Edge,0-1-8], [12:0-2-8,0-2-0]												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	25.0	Plate Grip DOL	1.15	тс	0.69	Vert(LL)	-0.24	14-15	>999	240	MT20	197/144	
TCDL	10.0	Lumber DOL	1.15	BC	0.72	Vert(CT)	-0.54	14-15	>957	180			
BCLL	0.0	Rep Stress Incr	YES	WB	0.90	Horz(CT)	0.14	11	n/a	n/a			
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S							Weight: 217 lb	FT = 20%	
LUMBER	2) Wind: ASCE 7-16; Vult=115mph (3-second gust) 2x6 SPE No 2 Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; t=35ft;												

BOT CHORD	2x4 SP 1650F 1.5E *Except* 13-11:2x4 SP No.2
WEBS	2x3 SPF No.2 *Except* 11-10:2x4 SP No.2
BRACING	
TOP CHORD	Structural wood sheathing directly applied or
	2-10-13 oc purlins, except end verticals, and
	2-0-0 oc purlins (4-10-10 max.): 5-7.
BOT CHORD	Rigid ceiling directly applied or 9-6-11 oc bracing.
WEBS	1 Row at midpt 3-15, 8-14, 6-14, 6-15
REACTIONS	(size) 2=0-5-8, 11=0-5-8
	Max Horiz 2=202 (LC 9)
	Max Uplift 2=-299 (LC 12), 11=-232 (LC 13)
	Max Grav 2=2025 (LC 1), 11=1946 (LC 1)
FORCES	(lb) - Maximum Compression/Maximum
	Tension
TOP CHORD	1-2=0/12, 2-3=-3475/567, 3-5=-2607/539,
	5-6=-2178/548, 6-7=-2001/523,
	7-8=-2374/517, 8-10=-2463/453,
	10-11=-1872/369
BOT CHORD	2-17=-554/2974, 15-17=-554/2974,
	14-15=-345/2167, 12-14=-366/2111,
	11-12=-44/96
WEBS	3-17=0/383, 3-15=-914/366, 5-15=-45/610,
	7-14=-59/580, 8-14=-207/225,
	8-12=-448/184, 10-12=-336/2104,
	6-14=-522/170, 6-15=-235/181
NOTES	

1) Unbalanced roof live loads have been considered for

this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf, h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 4-1-8, Interior (1) 4-1-8 to 18-8-0, Exterior(2R) 18-8-0 to 23-7-12, Interior (1) 23-7-12 to 28-7-8, Exterior(2R) 28-7-8 to 33-7-8, Interior (1) 33-7-8 to 43-5-14 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
   Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearings are assumed to be: Joint 2 SP 1650F 1.5E crushing capacity of 565 psi, Joint 11 SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 299 lb uplift at joint 2 and 232 lb uplift at joint 11.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITPTI Quality Criteria**, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



										RELE	ASE FOR CONSTRUCTION
Job		Truss		Truss Type		Qty	Ply	Roof - BY L	ot 2050	AS N	OTED FOR PLAN REVIEW
P250360-0	1	A7		Piggyback Base S	Supported Gable	1	1	Job Refere	nce (option	alj LEI	I68913064 E'S SUMMIT, MISSOURI
Premier Buildin	g Supply (Sprir	ighill, KS), S	Spring Hills, KS - 66083,	•	Run: 8.63 S Sep 26	2024 Print	8.630 S Sep 2	26 2024 MiTek	ndustries, Inc	c. ue Oct 15 1 47	19/2025
					ID:0f61Y0b5thaOB1	TXH_Pexqy	/flxL-RfC?PsB	70Hq3NSgPqnl	.8w3ulTXbGł	۲CDoi7J4zJC۹۲	10/2020
	-0-10-8		18.	8-0		28-7	-8			43-7-10	
	0-10-8		18-	8-0		9-11-	-8			15-0-2	
					4x8=	15 10	47	4x8=			
ТТ					13 14			20			
				10	1				21	2	
			6	2 9 <b>1</b>						23	
မှ ဝှ			٤	3						24	53 ^{4x8} ≈
10-3			4x8 = 7				× Þ	⊠			25
		4	525								28
		3									15
-9											
-		51 5	50 49 48	47 46 45	4 <b>4</b> 3 42 41 4	40 39	38	37 36	35 34	33 32	31 30
	4x6	= 1	.5x4 II 1.5x4 II	1.5x4 u	1.5x4 II 1.5x4 II	1.5x	(4 II	1.5x4 <b>။</b>	1.5x	4 <b>n</b> 1.5x4	n 1.5x4 n
		1.5x4 <b>I</b>	1.5x4 II	1.5x4 II 1.5x4 II	3x6= 1	.5x4 <b>II</b>	1.5x4 🛛	5x5=	1.5x4 <b>II</b>	1.5x4 🛛	1.5x4 <b>II</b>
	H				43	-7-10					
Scale = 1:78.6											
Plate Offsets	(X, Y): [13:0	-4-0,0-3-8	], [19:0-4-0,0-3-8], [29	9:Edge,0-2-8], [37:0-2	-8,0-3-0]						
Loading		(psf)	Spacing	2-0-0	CSI	DE	FL	in (loc)	l/defl L/	d PLATES	GRIP
TCLL (roof)		25.0 10.0	Lumber DOL	1.15 1.15	BC	0.09 Ve 0.04 Ve	rt(LL) rt(CT)	n/a - n/a -	n/a 99 n/a 99	9 M120 9	197/144
BCLL BCDI		0.0 10.0	Rep Stress Incr	YES IRC2018/TPI2014	WB 0 Matrix-S	0.22 Ho	rz(CT) 0	.00 29	n/a n/	a Weight: 259	lb FT = 20%
			0000		Max Gray 2=170 (I C	20) 29=7	74 (I C 1)	WEBS	27-	30=-147/171 25	5-31=-140/142
TOP CHORD	2x6 SPF N	lo.2			30=189 (L0 32=180 (L0	C 26), 31=	32=-140/105, 23	B-33=-140/96,			
WEBS	2x3 SPF N	lo.2			34=180 (LC	C 1), 35=1	80 (LC 1),		20-	36=-139/0, 18-3	7=-140/32,
OTHERS BRACING	2x3 SPF N	10.2			36=178 (LC 38=184 (LC	26), 37= 225), 39=	180 (LC 1), 181 (LC 26)		17-	40=-143/74, 14-	39=-141/83, 41=-140/38,
TOP CHORD	Structural	wood she	athing directly applied	l or d	40=183 (LC 42=179 (LC	C 26), 41= C 25), 44=	:180 (LC 25) :180 (LC 1),		12- 10-	42=-139/26, 11- 45=-140/101, 9-	44=-140/106, 46=-140/96,
	2-0-0 oc p	urlins (6-0	-0 max.): 13-19.	-	45=180 (L0 47=180 (L0	C 1), 46=1 C 25), 48=	80 (LC 25), 180 (LC 25)		8-4 4-5	7=-140/96, 7-48 0=-144/137, 3-5	=-140/97, 6-49=-139/96, 1=-120/144
BUICHURD	bracing.		applied of 10-0-0 oc		49=179 (L0 51=157 (L0	C 1), 50=1	84 (LC 25),	NOTES		, 6 Kura Jana Jan Januar	
WEBS	1 Row at r	nidpt	20-36, 18-37, 17-38, 16-39, 15-40, 14-41,	FORCES	(lb) - Maximum Comp	pression/N	laximum	1) Unb this	alanced roc design.	of live loads have	e been considered for
REACTIONS	(size)	2=43-7-10	12-42 ) 29=43-7-10	TOP CHORD	Tension 1-2=0/12, 2-3=-233/14	41, 3-4=-1	178/135,				
	(0120)	30=43-7-1	10, 31=43-7-10,		4-6=-153/149, 6-7=-1 8-9=-123/249, 9-10=-	38/175, 7· 141/303,	-8=-122/202,				
		32=43-7- 34=43-7-	10, 35=43-7-10, 10, 35=43-7-10,		10-11=-161/359, 11-1	2=-181/4	16, 13				
		36=43-7-' 38=43-7-'	10, 37=43-7-10, 10, 39=43-7-10,		14-15=-169/413, 15-1	6=-169/4	13,				
		40=43-7-	10, 41=43-7-10, 10, 44=43-7-10,		16-17=-169/413, 17-1 18-19=-169/413, 19-2	8=-169/4 20=-178/4	13, 14,				
		45=43-7-	10, 46=43-7-10,		20-21=-181/417, 21-2 22-23=-141/304, 23-2	22=-161/30 24=-122/2	60, 50,			~	1111
		49=43-7-	10, 50=43-7-10,		24-25=-104/197, 25-2 27-28=-65/87, 28-29=	27=-85/143 =-61/67	3,			A 01	E MISC
	Max Horiz	2=202 (LC	C 9)	BOT CHORD	2-51=-44/66, 50-51=-	44/66, 49	-50=-44/66,		-	8 AND	and the second
	Max Uplift	2=-78 (LC 30=-78 (L	C 8), 29=-13 (LC 12), .C 13), 31=-60 (LC 13	),	45-46=-44/66, 44-45=	44/66, 42	2-44=-44/66,		E E	SCO SI	OTT M.
		32=-61 (L 34=-65 (L	C 13), 33=-61 (LC 13 C 13), 35=-69 (LC 13	),	41-42=-44/66, 40-41= 38-39=-44/66, 36-38=	=-44/66, 3 =-45/66, 3	9-40=-44/66, 5-36=-45/65,		5	*	· A. 1+
		37=-8 (LC	(20, 10), 00, 00, (20, 10), 00, 00, (20, 10), 00, 00, 00, 00, 00, 00, 00, 00, 00,	,,	34-35=-45/65, 33-34= 31-32=-45/65, 30-31=	=-45/65, 32 =-45/65, 29	2-33=-45/65, 9-30=-45/65		- A	Catt	Server
		41=-14 (L	C 9, $40=-43$ (LC 8), C 9), $42=-2$ (LC 9),	,					Y	PE-20	01018807
		44=-67 (L 46=-61 (L	C 12), 45=-64 (LC 12 C 12), 47=-61 (LC 12	), ),						S.F.C	I GTE
		48=-61 (L 50=-64 (L	.C 12), 49=-61 (LC 12 .C 12), 51=-85 (LC 12	), )						W SION	VAL EN
										Octo	her 16 2024



Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusces and truss systems, see AMSI/TP11 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

Job	Truss	Truss Type	Qty	Ply	Roof - BY Lot 2050	AS D
P250360-01	A7	Piggyback Base Supported Gable	1	1	Job Reference (optional	L
			-			

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 4-1-8, Exterior(2N) 4-1-8 to 18-8-0, Corner(3R) 18-8-0 to 23-7-10, Exterior(2N) 23-7-10 to 28-7-8, Corner(3R) 28-7-8 to 33-7-10, Exterior(2N) 33-7-10 to 43-6-6 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Provide adequate drainage to prevent water ponding.
   All plates are 3x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads.All bearings are assumed to be SP No.2 crushing
- capacity of 565 psi.
  Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 29, 78 lb uplift at joint 2, 78 lb uplift at joint 30, 60 lb uplift at joint 31, 61 lb uplift at joint 32, 61 lb uplift at joint 33, 65 lb uplift at joint 34, 69 lb uplift at joint 35, 8 lb uplift at joint 37, 41 lb uplift at joint 38, 46 lb uplift at joint 39, 43 lb uplift at joint 40, 14 lb uplift at joint 41, 2 lb uplift at joint 42, 67 lb uplift at joint 44, 64 lb uplift at joint 45, 61 lb uplift at joint 46, 61 lb uplift at joint 47, 61 lb uplift at joint 50 and 85 lb uplift at joint 51.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. ue Oct





														RELEASE	FOR CONSTRUCTION
Job		Truss		Trus	s Type			Qt	y Pl	y	Roof - B	' Lot 205	i0	AS NOTE DEVEL	D FOR PLAN REVIEW
P250360-01		A8		Pig	gyback Base			1	1		Job Refe	rence (or	otional	LEE'S	I68913065 SUMMIT, MISSOURI
Premier Building	Supply (Springh	ill, KS), S	Spring Hills, KS - 66083			Ru ID:	in: 8.63 S Si :46ptwcZ7N	ep 26 2024 5EyWBruvF	Print: 8.630 xxMAyfzqz-	S Sep 26 RfC?PsB	6 2024 MiTe 70Hq3NSgF	k Industrie PqnL8w3u	es, Inc. [:] ITXbGK	ue Oct 15 1 07:58 WrCDoi75 4239?1	19/2025
	-0-10-8   - 0-10-8		<u>9-5-10</u> 9-5-10		<u>18-8</u> 9-2	-0 ·6		<u>23-7-12</u> 4-11-12	4	<u>28-7-8</u> -11-12	3	3-1-9 -6-1	35-9	39-6-2 9-6 37-10-4 13 2-0-14 1-8-0	43-7-10 4 <u>41-7-5</u> 2-1-1 2-0-5
							5x8 II		4x4=		5x8 II				
10-3-6 			26	6 ¹² 4x4, 3 25	4x8 = 27 = 4	24	23	22		8	21	2	5x8≈ 828	4x8 ₅ 4; 9 3x4 II 1029 18 117 18 117	x4z 11 12 $6x6z$ 13 10x10= 14 15 $2x4$
	5x5:	=		1.5x4	II	4x6=	3x8=	4x6	=		3x8=		6x6=	MT18 MT1	3x4= HS 6x12 = 8HS 5x8 II
	F		9-5-10 9-5-10		<u>18-6</u> 9-1-	<u>12</u> 2			<u>28-8-12</u> 10-2-0		3	<u>3-1-9</u> -4-13	+ 3	87-11-8 39-5-0 4-9-15 1-5-8	$ \begin{array}{c} 6x12 = & 43-7-10 \\ 41-6-1^{3}43 = 7-8 \\                                    $
Scale = 1:79.6	[2:Edge	e,0-1-7],	[5:0-4-4,0-2-8], [7:0	-4-4,0-	2-8], [8:0-3-2,0	-2-4], [12	:0-4-9,0-2-:	2], [13:Ed	ge,0-2-12]	, [14:Ed	ge,0-1-8],	[17:Edge	,0-3-12	2], [18:0-5-0,Edge]	0-1-4 0-0-2 ], [19:Edge,0-2-8],
	X, Y): [20:0-2·	-8,0-3-0	Creating	2.01		661			DEEL				1 /4		
TCLL (roof) TCDL BCLL BCDL		25.0 10.0 0.0 10.0	Plate Grip DOL Lumber DOL Rep Stress Incr Code	1.15 1.15 YES IRC:	2018/TPI2014	TC BC WB Mat	rix-S	0.84 0.87 0.90	Vert(LL) Vert(CT) Horz(CT	-0. -0. ) 0.	33 19 74 21-23 80 14	>999 >705 n/a	240 180 n/a	MT20 MT18HS Weight: 247 lb	197/144 197/144 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS	2x6 SPF No. 2.0E 2x4 SP 1650 No.2, 18-12: 2x3 SPF No.	2 *Exce 0F 1.5E 2x4 SP 2 *Exce	ept* 9-13:2x6 SP 24( *Except* 19-10:2x3 2400F 2.0E ept* 14-13:2x4 SP 16	00F SPF 650F	WEBS	3-25=0 7-21=- 13-15: 6-23=- 8-18=- 18-20: 11-17:	)/382, 3-23 ·109/690, 1 =-89/229, 6 ·237/182, 8 ·646/3406, =-403/2467 =-16/200	3=-914/36 11-18=-13 6-21=-527 8-21=-278 8-20=-12 7, 12-15=-	6, 5-23=-4 51/289, /169, /217, 57/255, 873/182,	7/613,					
BRACING TOP CHORD	Structural wo	ood she	athing directly applie	ed or	NOTES 1) Unbalan	ced roof li	ve loads ha	ave been	considered	d for					
BOT CHORD	2-0-0 oc pur 2-0-0 oc pur Rigid ceiling bracing, Ex 9-6-11 oc bra 8-0-10 oc bra	lins (4-1 directly cept: acing: 2	-25,23-25		2) Wind: AS Vasd=91 Ke=1.00 exterior 2	CE 7-16; mph; TCI Cat. II; E cone and (	Vult=115n )L=6.0psf; xp C; Encl C-C Exterio	nph (3-se BCDL=6. osed; MW or(2E) -0- xterior(2R	cond gust) 0psf; h=35 /FRS (enve 10-8 to 4-1	ift; elope) -8,					
WEBS	1 Row at mic	dpt	3-23, 6-21, 6-23, 8- 8-20	21,	23-7-12, 28-7-8 to	Interior (1 33-7-8, 1	) 23-7-12 t nterior (1) (	to 28-7-8, 33-7-8 to	Exterior(2)	, R) ine;					
REACTIONS	(size) 2= Max Horiz 2= Max Uplift 2= Max Grav 2=	=0-5-8, ^ =202 (L0 =-299 (L =2025 (L	14=0-5-8 C 9) C 12), 14=-232 (LC _C 1), 14=1946 (LC	13) 1)	cantileve right exp for react DOL=1.6	r left and osed;C-C ons show 0	right exposion for member n; Lumber	sed ; end ers and fo DOL=1.6	vertical left rces & MW ) plate grip	and /FRS					
FORCES	(lb) - Maximu Tension	um Com	pression/Maximum	,	<ol> <li>Provide a</li> <li>All plates</li> </ol>	adequate are MT2	drainage to 0 plates ur	o prevent nless othe	water pono rwise indic	ding. ated.					all.
TOP CHORD	CHORD 1-2=0/12, 2-3=-3475/567, 3-5=-2607/539, 5-6=-2178/547, 6-7=-1994/518, 7-8=-2320/537, 8-10=-5182/1020, 10-11=-5299/975, 11-12=-6117/1113, 12-13=-870/213, 13-14=-1600/279 2-25=-554/2974, 23-25=-554/2974, 21-23=-346/2168, 20-21=-343/2116, 19-20=-3/34, 18-19=0/94, 10-18=0/122, 17-18=-1062/5779, 12-17=-1010/5524, 16-17=-74/434, 15-16=-52/259, 14-15=-50/109					<ul> <li>5) The Fabrication Tolerance at joint 18 = 12%</li> <li>6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</li> <li>7) All bearings are assumed to be SP 1650F 1.5E crushing capacity of 565 psi.</li> <li>8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 299 lb uplift at joint 2 and 232 lb uplift at joint 14.</li> <li>9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.</li> <li>10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or</li> </ul>							MISSOLUTION		
					LOAD CASE	(S) Star	ndard							and and	STATES -

October 16,2024

16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.12007 MITeK-US.com

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													RELEASE	FOR CONSTRUCTION	
Job		Truss		Truss	Гуре		Qty		Ply	Roof - BY	Lot 2050	)	AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES 168913066		
P250360-0	1	B1		Comm	non Supporte	d Gable	1		1	Job Refer	ence (op	tional	LEE'S	SUMMIT, MISSOURI	
Premier Building	g Supply (Spring	ghill, KS), Sp	ring Hills, KS - 66083,			Run: 8.63 S Sep ID:QVJbkrrd9q5Z	26 2024 F a6?NSBm	Print: 8.0 hKl2yflx	630 S Sep 2 1-RfC?PsB7	6 2024 MiTel 0Hq3NSgPq	α Industries nL8w3uIT>	s, Inc. KbGKV	ue Oct 15 147:58 CDoi7J42JC4	19/2025	
		-p-′	10-8		10-4-0						20-8-0	<b>ה</b>		21-6-8	
		0-1	0-8		10-4-0						10-4-0	)	0-10-8		
								4x4 =							
	8														
5-11-3		1	3x4 = 4 3 2 3x4 = 25	5	6 5 2423		2		9		0	11	12 12 16	3x4 <b>x</b> 13 14 15 3x4 II	
Soolo - 1:44 1					3x4 =		20-	-8-0							
Plate Offsets	(X, Y): [2:0-2	-1,0-0-5], [	14:0-2-1,0-0-5]												
Loading TCLL (roof) TCDL BCLL BCDL		(psf) 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	1-11-4 1.15 1.15 YES IRC201	8/TPI2014	CSI TC BC WB Matrix-S	0.06 0.04 0.09	DEFL Vert(L Vert(C Horz(	.L) r CT) r CT) 0.	in (loc) n/a - n/a - 00 14	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 95 lb	<b>GRIP</b> 197/144 FT = 20%	
LUMBER TOP CHORD BOT CHORD OTHERS SLIDER BRACING	Instruction														
BOT CHORD	6-0-0 oc pu Rigid ceilin bracing. (size)	2=20-8-0, 1 2=20-8-0, 1 17=20-8-0, 20=20-8-0, 24=20-8-0,	applied or 10-0-0 oc 14=20-8-0, 16=20-8 18=20-8-0, 19=20- 21=20-8-0, 22=20- 25=20-8-0 12)	8-0, 8-0, -8-0, -8-0,	Vasd=91mpl Ke=1.00; Ca exterior zone Exterior(2N) 15-4-0, Exte left and right exposed;C-C reactions sh	t, TCDL=6.0psf; B tt. II; Exp C; Enclos and C-C Corner(3 4-4-0 to 10-4-0, C rior(2N) 15-4-0 to 2 exposed ; end ver for members and own: Lumber DOL	CDL=6.0 cod; MWF 3E) -0-10 corner(3R 21-6-8 zc tical left forces & =1.60 pla	PSf; h= RS (e -8 to 4 ) 10-4- one; ca and rig MWF te grip	=35ft; nvelope) -4-0, -0 to ntilever ht RS for						
	Max Uplift	2=-25 (LC 16=-83 (LC 18=-62 (LC 21=-60 (LC 24=-53 (LC 2=174 (LC 16=194 (LC 18=174 (LC	13), 14=-3 (LC 9), 13), 17=-54 (LC 1; 13), 19=-59 (LC 1; 12), 22=-61 (LC 1; 12), 25=-91 (LC 1; 1), 14=174 (LC 1), 26), 17=170 (LC 2) 26), 17=183 (LC 2)	3), 3) 3), 2), 4) 1), 5) 5), 6)	<ul> <li>DOL=1.60</li> <li>Truss desigr only. For stu see Standar or consult qu</li> <li>All plates are</li> <li>Gable requir</li> <li>Gable studs</li> </ul>	ned for wind loads Jds exposed to win d Industry Gable E Jalified building des e 1.5x4 MT20 unles es continuous bott spaced at 2-0-0 oc	in the pla id (norma nd Detail signer as ss otherw om choro c.	ine of t al to the ls as a per Al vise ind beari	he truss e face), pplicable, NSI/TPI 1. dicated. ng.					200	
	-	20=155 (LC 22=174 (LC	C 22), 21=183 (LC 2 C 22), 21=183 (LC 2 C 1) 24=170 (LC 1)	25), 7)	) This truss ha	as been designed f ad nonconcurrent v	or a 10.0 vith anv o	psf bo other li	ottom ve loads.				E OF I	MISSON	
FORCES	(lh) - Mavir	25=194 (LC	C 25)	8	All bearings capacity of 5	are assumed to be 65 psi.	SP No.2	2 crush	ing			Å	SCOT	M T M T	
TOP CHORD	<ul> <li>(b) - Maximum Compression/Maximum</li> <li>(c) - Maximum Compression/Maximum Compression/Maximum</li> <li>(c) - Maximum Compression/Maximum Compression/Max</li></ul>									Singer					
BOT CHORD	CHORD         2-25=-22/116, 24-25=-22/116, 22-24=-22/116, 22-24=-22/116, 21-22=-22/116, 20-21=-22/116, 18-19=-22/116, 17-18=-22/116, 16-17=-22/116, 14-16=-22/116         at Joint 17 and 05 to Upin at Joint 10.           10         This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.         10           14-16=-22/116         LOAD CASE(S)         Standard								PE-2001	018807 5 4 L ENGL 1 ENO					

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												RELEASE	FOR CONSTRUCTION	
Job		Truss		Truss Type		Qty	, I	Ply	Roof - E	3Y Lot 205	0	AS NOTE	D FOR PLAN REVIEW	1
P250360-01		B2		Common		8	.   ·	1	Job Ref	erence (or	otional	LEE'S	I68913067 SUMMIT, MISSOURI	
Premier Building	Supply (Springh	ill, KS), S	pring Hills, KS - 66083,		Run: 8.63 S Sep 26 ID:0BAuge0Ps8saG0	2024 F 333G7	Print: 8.63 OcK?yflw	30 S Sep 26 p-RfC?PsB	6 2024 Mi 70Hq3NS	Fek Industrie	s, Inc. ITXbGł	ue Oct 15 1 77:58 WrCDoi794z36?4	19/2025	-
			-ρ-10-8 <u>-</u>	j-3-4	10-4-0		I	15-4-	12	i	_	20-8-0	21-6-8	
			0-10-8	j-3-4	5-0-12			5-0-1	2			5-3-4	0-10-8	
						4	4x4 =							
						5	5							
Т	-			,1 <u>2</u>	/									
				61		$\square$	$\parallel >$	$\searrow$						
				1.5x4 💊					$\geq$	1.5x4	11			
ကု	0		3	4	~ //		/			6	14	2×4		
5-11	5-10		3								$\leq$	5X4 <b>x</b>		
			3		、  //				,			$\searrow$		
			2	j v	$\langle \langle \rangle //$				$\langle   \rangle$			- CB-	< 8	
	8-0	- 1							$\mathcal{W}$				9	
1	&_	_			12 11				10					
			4x4 II		12 11				2014				4x4 II	
					3x4 = 3x4 =				3x4 =					
			1	6-11-8	1	13-	8-8		I.		20-8	8-0	1	
Scale - 1:46 1				6-11-8		6-9	9-0		I		6-1	1-8		
														_
Loading TCLL (roof)		(psf) 25.0	Spacing Plate Grip DOL	2-0-0 1.15	TC CSI	).42	DEFL Vert(LL	_) -0.(	in (lo 06 2-1	c) l/defl 2 >999	L/d 240	MT20	GRIP 197/144	
TCDL		10.0	Lumber DOL	1.15	BC 0	).51	Vert(C	T) -0.1	13 2-1	2 >999	180			
BCLL		0.0	Rep Stress Incr	YES	WB ( Matrix-S	).16	Horz(C	T) 0.0	04	8 n/a	n/a	Weight: 92 lb	FT - 20%	
		10.0	Code									Weight. 32 lb	11 - 2076	-
TOP CHORD	2x4 SP No.2			capacity of 5	65 psi.	² NO.2	2 Crushir	ng						
BOT CHORD	2x4 SP No.2	2		<ol> <li>Provide mech</li> <li>bearing plate</li> </ol>	hanical connection (by	y othe	ers) of tr	uss to lift at						
SLIDER	Left 2x4 SPF No.	.2 No.2 2	-11-0, Right 2x4 SP	joint 2 and 16	63 lb uplift at joint 8.	ing n		int at						
	No.2 2-11-	0		<ol> <li>This truss is International</li> </ol>	designed in accordan Residential Code sec	ce wi	th the 20 R502 1	018 1.1 and						
TOP CHORD	Structural wo	ood shea	athing directly applied	or R802.10.2 ar	nd referenced standar	rd AN	SI/TPI 1							
BOT CHORD	4-5-0 oc pur Rigid ceiling bracing.	lins. directly	applied or 10-0-0 oc	LOAD CASE(S)	Standard									
REACTIONS	(size) 2=	=0-3-8, 8	=0-3-8											
	Max Horiz 2= Max Uplift 2=	=106 (LC =-163 (L(	; 16) C 12), 8=-163 (LC 13	)										
I	Max Grav 2=	=991 (LC	51), 8=991 (LC 1)											
FORCES	(lb) - Maximu Tension	um Comj	pression/Maximum											
TOP CHORD	1-2=0/6, 2-4	=-1495/3	366, 4-5=-1316/378,											
BOT CHORD	2-12=-249/1 8-10=-242/1	≀ο, ο-ο≕ 244, 10- 244	- 1490/300, 8-9=0/6 12=-87/876,											
WEBS	5-10=-113/4	66, 6-10 66 4-12	=-298/211, =-298/211											
NOTES			_00,2											

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 4-1-8, Interior (1) 4-1-8 to 10-4-0, Exterior(2R) 10-4-0 to 15-5-11, Interior (1) 15-5-11 to 21-6-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

nt with any other live loads.





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												REL	EASE FOR CONSTRUCTION	
Job		Truss		Truss Ty	ре		Qty	/ Ply	R	oof - BY	Lot 2050	AS D	NOTED FOR PLAN REVIEW EVELOPMENT SERVICES	
P250360-07	1	B3		Commo	n		1	1	J	b Refere	ence (option	nali L	I68913068 EE'S SUMMIT, MISSOURI	
Premier Building	g Supply (Springh	iill, KS), Sp	oring Hills, KS - 66083,			Run: 8.63 S Se ID:UfFhSoEidgi	p 26 2024 70Q1RXKvI	Print: 8.630 \$ _q2oyflwX-R	S Sep 26 20 fC?PsB70F	)24 MiTek Iq3NSgPq	Industries, Ir nL8w3uITXb	nc. Tue Oct 15 1 GK VrCDoi7342JC	5/19/2025	
			5-3	-4		10-4-0		1	5-4-12		1	20-8-0	21-6-8	
			5-3	-4	I	5-0-12	I	5	5-0-12		I	5-3-4	0-10-8	
							4x4 4	=						
	5-11-3 5-10-0		3x4 - 2 1 4x4 II	6-11-8	612 1.5x4 x 3	11 10 3x4= 3x4=	-	3	9 3x	4=	.5x4 # 5 12	3x4 ±	7 8 4x4 II	
Scale = 1:46.1				6-11-8			6-9-0				6-1	1-8	———————————————————————————————————————	
Loading TCLL (roof) TCDL BCLL BCDL		(psf) 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018	/TPI2014	CSI TC BC WB Matrix-S	0.42 0.51 0.16	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.06 -0.14 0.04	(loc) 1-11 1-11 7	l/defl l >999 2 >999 1 n/a r	/d <b>PLATES</b> 40 MT20 80 n/a Weight: 91	<b>GRIP</b> 244/190 I Ib FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS SLIDER	2x4 SP No.2 2x4 SP No.2 2x3 SPF No Left 2x4 SP No.2 2-11	.2 No.2 2 -0	-11-0, Right 2x4 SP	4) 5) 6)	All bearings a capacity of 50 Provide mech bearing plate joint 1 and 10 This truss is o International	are assumed to b 65 psi. hanical connectio capable of withs 63 lb uplift at joint designed in acco	e SP No. on (by othe tanding 1 7. rdance wi sections	2 crushing ers) of truss 40 lb uplift th the 2018 R502.11.1	s to at 3 and					
TOP CHORD	Structural w 4-4-14 oc pu Rigid ceiling	ood shea Irlins. directly a	athing directly applied	or LO	R802.10.2 ar AD CASE(S)	nd referenced sta Standard	indard AN	SI/TPI 1.						
PEACTIONS	bracing.	0 2 9 7	0.2.8											
REACTIONS	Max Horiz 1= Max Uplift 1= Max Grav 1=	=0-3-8, 7 =-107 (LC =-140 (LC =929 (LC	=0-3-8 C 13) C 12), 7=-163 (LC 13) C 1), 7=993 (LC 1)	)										
FORCES	(lb) - Maxim	um Comp	pression/Maximum											
TOP CHORD	1-3=-1501/3	74, 3-4=-	-1323/387,											
BOT CHORD	4-5=-1319/3 1-11=-252/1	oz, 5-7=- 252, 9-11	1=-88/879,											
WEBS	7-9=-246/12 4-9=-113/46 3-11=-302/2	46 5, 5-9=-2 13	298/211, 4-11=-115/4	72,										
NOTES												-	and the second s	

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-2-5, Interior (1) 5-2-5 to 10-4-0, Exterior(2R) 10-4-0 to 15-5-11, Interior (1) 15-5-11 to 21-6-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.



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								_
							RELEASE FOR CONSTRUCTION	
Job	Truss	Truss Type		Qty	Ply	Roof - BY Lot 2050	AS NOTED FOR PLAN REVIEW	
P250360-01	B4	Common		2	1	Job Reference (optional	LEE'S SUMMIT, MISSOURI	
Premier Building Supply (Springh	hill, KS), Spring Hills, KS - 66083,		Run: 8.63 S Sep 26 2 ID:jOI5LsLLVQGk?Po	2024 Print: 8. IGMI?xwhyfl	630 S Sep 2 wO-RfC?PsB	6 2024 MiTek Industries, Inc. 370Hq3NSgPqnL8w3uITXbGK	ue Oct 015:#19/2025	
	5-3-4	ι Ι	10-4-0	1	15-4-12	20	0-8-0	
	5-3-4	t I	5-0-12	1	5-0-12	5	j- <b>3</b> -4	
				4x4 =				
				4				
T		1 <u>2</u> 6				1.5x4 <i>u</i>		
5-10-0	3x4 = 2	3				5	3x4 <b>2</b>	
	Test S						760	

<u>e</u>	1				7
⊥ %⊥	<b>4</b> x4 µ	10 3x4 =	9 3x4=	8 3x4=	4x4 II

	6-11-8	13-8-8	20-8-0	
	6-11-8	6-9-0	6-11-8	
Scale = 1:45.7				

0.41

0.51

0.16

DEFL

Vert(LL)

Vert(CT)

Horz(CT)

in

-0.06

-0.14

0.04

(loc)

1-10

1-10

7

l/defl

>999

>999

n/a n/a

L/d

240

180

PLATES

Weight: 89 lb

MT20

GRIP

244/190

FT = 20%

Loading		(psf)	Spacing	2-0-0	)	
TCLL (roof)		25.0	Plate Grip DOL	1.15		
TCDL		10.0	Lumber DOL	1.15		
BCLL		0.0	Rep Stress Incr	YES		
BCDL		10.0	Code	IRC2	2018	/TPI2014
					-	
LUMBER	0 4 0 D N				4)	All bearin
TOP CHORD	2x4 SP N	0.2			E)	Drovido
BOLCHOKD	2X4 SP N	0.2			5)	bearing r
WEBS	2X3 SPF I					ioint 1 an
SLIDER	No 2 2-	0F NU.2 2 11_0	-11-0, Right 2x4 SP		6)	This trus
	110.2 2-	11-0			•)	Internatio
	Structural	wood abo	thing directly opplied	d or		R802.10
TOP CHORD		nurline	atting directly applied	101	LO	AD CASE
	Rigid ceili	na directly	applied or 10-0-0 oc			
BOT ONORD	bracing.	ing directly				
REACTIONS	(size)	1=0-3-8.7	'=0-3-8			
	Max Horiz	1=104 (LC	2 16)			
	Max Uplift	1=-140 (L	C 12), 7=-140 (LC 13	5)		
	Max Grav	1=930 (LC	C 1), 7=930 (LC 1)	,		
FORCES	(lb) - Max	imum Com	pression/Maximum			
	Tension					
TOP CHORD	1-3=-1504	4/375, 3-4=	-1325/388,			
	4-5=-1325	5/388, 5-7=	-1504/375			
BOT CHORD	1-10=-260	0/1254, 8-1	0=-95/881,			
	7-8=-252/	1254				
WEBS	4-8=-115/	471, 5-8=-3	302/213, 4-10=-115/4	171,		
	3-10=-302	2/213				

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0 to 5-2-5, Interior (1) 5-2-5 to 10-4-0, Exterior(2R) 10-4-0 to 15-5-11, Interior (1) 15-5-11 to 20-8-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at
- joint 1 and 140 lb uplift at joint 7.

CSI

тс

BC

WB

Matrix-S

6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

OAD CASE(S) Standard



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International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

### LOAD CASE(S) Standard

10=226 (LC 26), 11=103 (LC 1),

12=226 (LC 25)

1-2=0/7, 2-4=-97/72, 4-5=-83/179, 5-6=-83/177, 6-8=-97/64, 8-9=0/7 2-12=-1/50, 11-12=-1/50, 10-11=-1/50,

Unbalanced roof live loads have been considered for

Wind: ASCE 7-16; Vult=115mph (3-second gust)

Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-11-0 to 4-1-0,

Exterior(2N) 4-1-0 to 4-6-0, Corner(3R) 4-6-0 to 9-6-0, Exterior(2N) 9-6-0 to 9-11-0 zone; cantilever left and

right exposed ; end vertical left and right exposed;C-C

Lumber DOL=1.60 plate grip DOL=1.60

for members and forces & MWFRS for reactions shown;

Tension

8-10=-1/50

(Ib) - Maximum Compression/Maximum

5-11=-71/0, 4-12=-172/240, 6-10=-172/239

FORCES

TOP CHORD

BOT CHORD

this design

WEBS

NOTES

1)

2)



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								RELEASE FOR CONSTRUCTION
Job	Truss		Truss Type		Qty	Ply	Roof - BY Lot 2050	AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES
P250360-01	C2		Common		3	1	Job Reference (optic	I68913071 LEE'S SUMMIT, MISSOURI
Premier Building Supply (Springl	hill, KS), S	oring Hills, KS - 66083,		Run: 8.63 S Sep 26 ID:w4iDMZp9S4bl6N	2024 Print MF5GO7dil	: 8.630 S Sep : SyVCU4-RfC?F	26 2024 MiTek Industries, l PsB70Hq3NSgPqnL8w3ulT	Inc. Tue Oct 16 177:491 9/219:25
			-0-11-0 0-11-0	4-6-0 4-6-0			<u>9-0-0</u> 4-6-0	9-11-0
				12 6 Г	10	4x4 = 4 ↓ 11		
	3-0-7	2-11-0	1 9 2	3x4 = 3			3x4≈ 5	6 12 7
			3x4 II			8 1.5x4 ॥		3x4 II
Scale = 1:32.3	1005	IC:0.2.4.0.0.51		<u>4-6-0</u> 4-6-0			9-0-0 4-6-0	
Plate Offsets (X, Y): [2:0-2-	1,0-0-5],	[6:0-2-1,0-0-5]						
Loading TCLL (roof)	(psf) 25.0	Spacing Plate Grip DOL	2-0-0 1.15	TC	0.39   DE	E <b>FL</b> rt(LL) -C	in (loc) l/defl ).01 6-8 >999 2	L/d PLATES GRIP 240 MT20 197/144

Loading	(pst)	Spacing	2-0-0	CSI		DEFL	ın	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.39	Vert(LL)	-0.01	6-8	>999	240	MT20	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.25	Vert(CT)	-0.03	2-8	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.01	6	n/a	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S							Weight: 41 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER	2x4 SP No.2 2x4 SP No.2 2x3 SPF No.2 Left 2x4 SP No.2 2 No.2 2-5-12	2-5-12, Right 2x4 SP	5) Provide me bearing plat 2 and 84 lb 6) This truss is Internationa R802.10.2 a	chanical connection te capable of withstar uplift at joint 6. s designed in accorda al Residential Code s and referenced stanc ) Standard	(by oth nding 8 ance wi ections dard AN	ers) of truss t 4 lb uplift at j ith the 2018 R502.11.1 a ISI/TPI 1.	o oint nd					
		othing disectly opplie.	LOAD CASE(S	) Stanuaru								
I OP CHORD	6-0-0 oc purlins	aming directly applied	u or									
BOT CHORD	Rigid ceiling directly bracing.	applied or 10-0-0 oc										
REACTIONS	(size) 2=0-5-8, 6	6=0-5-8										
	Max Horiz 2=-51 (LC	317)										
	Max Uplift 2=-84 (LC	2 12), 6=-84 (LC 13)										
	Max Grav 2=469 (LC	C 1), 6=469 (LC 1)										
FORCES	(lb) - Maximum Com	pression/Maximum										
	Tension											
TOP CHORD	1-2=0/7, 2-4=-504/2 6-7=0/7	19, 4-6=-504/219,										
BOT CHORD	2-8=-78/358, 6-8=-7	8/358										
WEBS	4-8=0/223											
NOTES												
1) Unbalance	d roof live loads have	been considered for									000	The
this design											S OF M	Alson
2) Wind: ASC	E 7-16; Vult=115mph	(3-second gust)									A SE	-0.0 M
VaS0=9111 Ko-1.00.0	at II: Exp.C: Enclose	d. MW/FRS (envelope	e)							A	AN AGOM	New
exterior zor	ne and C-C Exterior(2	PF) -0-11-0 to 4-1-0	<i>c</i> ,							A	SCOT	M. YOY
										u	/ SEVI	ER V V

Interior (1) 4-1-0 to 4-6-0, Exterior(2R) 4-6-0 to 9-6-0, Interior (1) 9-6-0 to 9-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown;

Lumber DOL=1.60 plate grip DOL=1.60 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) All bearings are assumed to be SP No.2 crushing 4)

capacity of 565 psi.

NUMBER PE-20 PH-SSIONAL EN PE-2001018807 G

October 16,2024



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										_ [	RELEASE	E FOR CONSTRUCTION
Job		Truss		Truss Type		Qty	Ply	Roof - BY	' Lot 2050	)	AS NOTE DEVEL	ED FOR PLAN REVIEW
P250360-01	l	PB1		Piggyback		2	1	Job Refe	rence (opt	tional	LEE'S	I68913072 SUMMIT, MISSOURI
Premier Building	Supply (Spring	nill, KS), S	pring Hills, KS - 66083,		Run: 8.63 S Sep 26	2024 Pri	nt: 8.630 S Sep	26 2024 MiTe	k Industries	s, Inc.	ue Oct 15 1 07:59	19/2025
					D.DVPINJSKIIIe?E92	axiquui	JYERRERIC ? P	SB70Hq3N3gP	qricowouri	ADGR	VICD01734230-1-	
				-0-10-13 0-10-13	4-0-7 4-0-7				<u>8-0-14</u> 4-0-7		8-11- 0-10-	<u>11</u> 13
					12 6 ┌		4x4 = 4					
					1.5x4 u				1.5x4 µ			
	2-5-14	2-4-6		3	3				5		6	
			0-4-3									7
				3x4 =	10		9 1 5×4 #		8 1 5×4 #	~~~~	3x4 =	
				I	1.004		8-0-14		1.074		I	
Scale = 1:26.3												
Loading TCLL (roof) TCDL BCLL BCDL		(psf) 25.0 10.0 0.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr	1-11-4 1.15 1.15 YES	CSI TC C BC C WB C	0.07 V 0.03 V 0.05 H	DEFL /ert(LL) /ert(CT) łorz(CT)	in (loc) n/a - n/a - 0.00 6	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 197/144
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x4 SP No.2 2x3 SPF No Structural w 6-0-0 cc puu Rigid ceiling bracing. (size) 2: 9 Max Uplift 2: 8 Max Uplift 2: 8 Max Grav 2: (1	2 2 2 3 3 4 directly =8-0-14, =8-0-14, =42 (LC =-19 (LC =-73 (LC =-73 (LC =-123 (LC C 1), 9=	athing directly applied applied or 10-0-0 oc 6=8-0-14, 8=8-0-14, 10=8-0-14 12), 6=-27 (LC 13), 13), 10=-73 (LC 12); 1), 6=123 (LC 1), 8= 127 (LC 1), 10=206 (	<ul> <li>5) Gable studs</li> <li>6) This truss ha chord live loa</li> <li>7) All bearings capacity of 5</li> <li>8) Provide meco bearing plate</li> <li>2, 27 lb uplift uplift at joint</li> <li>9) This truss is International R802.10.2 a</li> <li>10) See Standar Detail for Co consult quali</li> <li>206</li> <li>LOAD CASE(S)</li> </ul>	spaced at 2-0-0 oc. Is been designed for a ad nonconcurrent with are assumed to be SF 65 psi. hanical connection (b) a capable of withstand at joint 6, 73 lb uplift 8. designed in accordan Residential Code sec nd referenced standard d Industry Piggyback nnection to base truss fied building designer. Standard	a 10.0 p any oth No.2 c y others ing 19 I at joint ce with tions R d ANSI Truss C a as app	asf bottom her live loads crushing s) of truss to b) uplift at joir 10 and 73 lb the 2018 502.11.1 and //TPI 1. Connection blicable, or	It				
FORCES	(lb) - Maxim	um Com	pression/Maximum									
TOP CHORD	1-2=0/16, 2- 4-5=-52/96,	3=-53/37 5-6=-43/	7, 3-4=-52/91, 26, 6-7=0/16									
BOT CHORD	2-10=-13/47 6-8=-13/47	, 9-10=-′	13/47, 8-9=-13/47,	_								
WEBS NOTES	4-9=-91/12,	3-10=-16	51/207, 5-8=-161/167	<i>,</i>								aan
<ol> <li>Unbalance this design</li> <li>Wind: ASC Vasd=91m Ke=1.00; C</li> <li>exterior zo and right e exposed; C</li> <li>reactions s</li> </ol>	ed roof live loa CE 7-16; Vult= ph; TCDL=6.0 Cat. II; Exp C; one and C-C E exposed ; end C-C for member shown; Lumber	ds have 115mph Dpsf; BCI Enclosed xterior(2) vertical le ers and fo rr DOL=1	been considered for (3-second gust) DL=6.0psf; h=35ft; d; MWFRS (envelope E) zone; cantilever le eft and right prces & MWFRS for .60 plate grip	ə) ft							STATE OF I SCOT SEVI	MISSOLA T.M. HER Service

DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

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October 16,2024

PE-200-

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Scale =	1:28.7
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Scale = 1:28.7														
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	25.0	Plate Grip DOL	1.15		тс	0.35	Vert(LL)	n/a	-	n/a	999	MT20	197/144	
TCDL	10.0	Lumber DOL	1.15		BC	0.16	Vert(CT)	n/a	-	n/a	999			
BCLL	0.0	Rep Stress Incr	YES		WB	0.05	Horz(CT)	0.00	4	n/a	n/a			
BCDL	10.0	Code	IRC201	8/TPI2014	Matrix-P							Weight: 30 lb	FT = 20%	
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x3 SPF No.2 Structural wood she	athing directly appli	7) 8) ed or ⁹⁾	All bearings capacity of s Provide med bearing plat 2, 75 lb uplit This truss is Internationa	are assumed 565 psi. chanical conne e capable of w t at joint 4 and designed in a I Residential C	to be SP No. ection (by oth rithstanding 6 I 8 Ib uplift at ccordance w code sections	2 crushing ers) of truss t 7 lb uplift at j joint 6. th the 2018 R502.11.1 a	to joint and						
BOT CHORD	Rigid ceiling directly bracing. (size) 2=8-0-14,	applied or 10-0-0 o , 4=8-0-14, 6=8-0-14	c 10	R802.10.2 a ) See Standa Detail for Co consult gual	nd referenced rd Industry Pig onnection to ba ified building d	standard AN gyback Trus ase truss as a lesigner.	SI/TPI 1. Connection applicable, or							

8-0-14

Max Horiz 2=44 (LC 12) Max Uplift 2=-67 (LC 12), 4=-75 (LC 13), 6=-8 (LC 12) 2=234 (LC 1), 4=234 (LC 1), 6=341 Max Grav

### (LC 1) FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/17, 2-3=-92/76, 3-4=-92/87, 4-5=0/17 BOT CHORD 2-6=0/46, 4-6=0/46 3-6=-240/180 WEBS

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) 2) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing. 4)

5) Gable studs spaced at 4-0-0 oc.

This truss has been designed for a 10.0 psf bottom 6) chord live load nonconcurrent with any other live loads.

# OF MISS SCOTT M. SEVIER VI IMPRER PE-2001018807 C SSIONAL E October 16,2024



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LOAD CASE(S) Standard



						-							
Loading TCLL (roof) TCDL BCLL BCDL	(psf) 25.0 10.0 0.0 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018	8/TPI2014	CSI TC BC WB Matrix-S	0.35 0.21 0.06	<b>DEFL</b> Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 32 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x4 SP No.2 2x3 SPF No.2 Structural wood shi 6-0-0 oc purlins. Rigid ceiling directly bracing. (size) 1=10-4-4 Max Horiz 1=43 (LC Max Uplift 1=-46 (L 4=-35 (L Max Grav 1=195 (L 4=438 (L	eathing directly applie y applied or 10-0-0 or , 3=10-4-4, 4=10-4-4 ; 16) C 12), 3=-54 (LC 13), C 12) C 25), 3=195 (LC 26) C 1)	7) 8) ed or 9) 2 LC	All bearings capacity of 5 Provide mec bearing plate 1, 54 lb upliff This truss is International R802.10.2 ar DAD CASE(S)	are assumed to b 65 psi. hanical connectio e capable of withs a t joint 3 and 35 designed in acco Residential Code nd referenced sta Standard	e SP No. on (by oth tanding 4 Ib uplift a rdance w e sections indard AN	2 crushing ers) of truss to 6 lb uplift at jo t joint 4. ith the 2018 R502.11.1 a ISI/TPI 1.	o pint nd					
FORCES TOP CHORD BOT CHORD WEBS	(lb) - Maximum Cor Tension 1-2=-119/74, 2-3=- 1-4=-3/50, 3-4=-3/5 2-4=-299/210	npression/Maximum 119/81 0											
<b>NOTES</b> 1) Unbalance	ed roof live loads have	e been considered for	r										

- this design. Wind: ASCE 7-16; Vult=115mph (3-second gust) 2) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing. 4) 5)
- Gable studs spaced at 4-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

## OF MISSO P SCOTT M. SEVIER NUMBER ro, PE-2001018807 HESSIONAL E

October 16,2024



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 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to dury with with where outputs into design is based only door parameters shown, and is for an individual building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH1 Quality Criteria**, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)





6-4-4

Scale = 1:25.8

Loading		(psf)	Spacing	2-0-0	)	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)		25.0	Plate Grip DOL	1.15		TC	0.15	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL		10.0	Lumber DOL	1.15		BC	0.07	Vert(TL)	n/a	-	n/a	999		
BCLL		0.0	Rep Stress Incr	YES		WB	0.03	Horiz(TL)	0.00	3	n/a	n/a		
BCDL		10.0	Code	IRC2	018/TPI2014	Matrix-P							Weight: 19 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP N 2x4 SP N 2x3 SPF Structura 6-0-0 oc Rigid ceil bracing. (size) Max Horiz Max Uplift Max Grav	lo.2 lo.2 No.2 l wood she purlins. ing directly 1=6-4-4, 5 1=25 (LC 1=-32 (LC (LC 12) 1=122 (LC (LC 1)	athing directly applied applied or 10-0-0 oc 3=6-4-4, 4=6-4-4 12) C 12), 3=-36 (LC 13), C 1), 3=122 (LC 1), 4	d or 4=-8 =223	<ol> <li>All bearings capacity of 5</li> <li>Provide mec bearing plat 1, 36 lb uplif</li> <li>This truss is Internationa R802.10.2 a</li> <li>LOAD CASE(S)</li> </ol>	are assumed 565 psi. chanical conne e capable of w it at joint 3 and designed in a I Residential C und referenced Standard	to be SP No. ection (by oth /ithstanding 3 4 8 lb uplift at cccordance w Code sections I standard AN	2 crushing ers) of truss i2 lb uplift at joint 4. ith the 2018 is R502.11.1 iSI/TPI 1.	to joint and					
FORCES	(lb) - Max Tension	kimum Corr	pression/Maximum											
TOP CHORD	1-2=-59/4	47, 2-3=-59	/54											

BOT CHORD 1-4=-1/26, 3-4=-1/26 2-4=-158/139 WEBS

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) 2) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing. 4)
- 5) Gable studs spaced at 4-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.



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 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not beigh valid for use only with with with with the contractions. This design is based only door plantaters shown, and is for an individual dualing component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria**, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)

										_ [	RELEASE	E FOR CONSTRUCTION	
Job	Truss		Truss Type	Truss Type Qty Ply Roof - BY Lot 20				Lot 2050 AS NOTED FOR PLAN			ED FOR PLAN REVIEW	]	
P250360-0	1 V3		Valley	1 1 Job Reference (optic				tional	I68913076 Dinal LEE'S SUMMIT, MISSOURI				
Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,				Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Oct 1915;191 9/2025 ID:15stfKS3rish2YgxkdfWe5yfluy-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJe7									)
					4-1-15	i			-				
				1.5x4 u									
	1-4-14		4 T	1				2	3		1-4-14		
				3x4 =				1.	.5x4 u				
Scale = 1:18.8					4-1-15	<u>.</u>							
Loading TCLL (roof) TCDL BCLL BCDL	(psf) 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018/TPI2014	CSI TC BC WB Matrix-P	0.21 0.12 0.00	<b>DEFL</b> Vert(LL) Vert(TL) Horiz(TL)	ii n/a n/a 0.00	n (loc) a - a - 0 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 12 lb	<b>GRIP</b> 244/190 FT = 20%	_
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x3 SPF No.2 Structural wood she	eathing directly applied	8) This truss is International R802.10.2 a LOAD CASE(S)	designed in accorda Residential Code se nd referenced standa Standard	Ince witections ard AN	h the 2018 R502.11.1 SI/TPI 1.	and						_
BOT CHORD	4-2-11 oc purlins, e Rigid ceiling directly	except end verticals. applied or 10-0-0 oc											
REACTIONS	(size) 1=4-1-15 Max Horiz 1=51 (LC Max Uplift 1=-27 (L1 Max Grav 1=143 (L	5, 3=4-1-15 5 9) C 8), 3=-35 (LC 12) C 1), 3=143 (LC 1)											
FORCES	(lb) - Maximum Cor Tension 1-2=-72/44, 2-3=-1 ⁻	npression/Maximum											
BOT CHORD NOTES 1) Wind: ASI Vasd=91r Ke=1.00;	1-3=-22/24 CE 7-16; Vult=115mpl nph; TCDL=6.0psf; B( Cat. II; Exp C; Enclos	n (3-second gust) DDL=6.0psf; h=35ft; ed; MWFRS (envelope	<b>)</b>										

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 4-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.6) All bearings are assumed to be SP No.2 crushing

 All bearings are assumed to be SP No.2 crusning capacity of 565 psi.
 Description of the second second

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 1 and 35 lb uplift at joint 3.



EFORE USE. nent, not ne overall inent bracing a n Truss Plate Institute (www.tpinst.org) .com) **Mittek** 16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200 / Mittek-US.com

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											RELEASI	FOR CONSTRUCTION
Job	Truss		Truss Type		Qt	y Ply	/	Roof - BY	Lot 2050	0	AS NOT	ED FOR PLAN REVIEW
P250360-01	V4		Valley		1	1		Job Refere	ence (op	tional	LEE'S	SUMMIT, MISSOURI
Premier Building S	Supply (Springhill, KS), S	•	Run: 8.63 S Sep 2 ID:RgX?HLUx8dE	26 2024 Gv0OW	Print: 8.630 S QICDGjyfluv-	S Sep 26 : RfC?PsB	2024 MiTek 70Hq3NSgF	Industrie PqnL8w3u	s, Inc. [†] ıITXbG	ue Oct 15 147:59 (WrCDoly 4230 f	19/2025	
			ŀ		3-4-6	i		_				
								1.5x4 ॥				
							2	1				
				6 T				e				
		1-8-7		1						1-8-1		
					<u> </u>			3				
		0						×				
				3x4 ዾ				1.5x4 🛛				
			-		3-4-6	i		_				
Scale = 1:19.9		1				r						
Loading	(psf)	Spacing	2-0-0	CSI	0.15	DEFL	ii n/i	n (loc)	l/defl	L/d	PLATES	GRIP
ICEL (1001) ICEL	25.0 10.0	Lumber DOL	1.15	BC	0.15	Vert(LL)	n/a n/a	a -	n/a n/a	999 999	WI120	244/190
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	) 3	n/a	n/a		
SCDL	10.0	Code	IRC2018/TPI2014	Matrix-P							Weight: 11 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD	2x4 SP No.2 2x4 SP No.2		8) This truss is International R802.10.2 a	designed in accord Residential Code s nd referenced stand	ance w ections lard AN	ith the 2018 R502.11.1 ISI/TPI 1.	8 I and					
BRACING	2x3 SPF INU.2		LUAD CASE(S)	SIGNUARU								

TOP CHORD Structural wood sheathing directly applied or 3-4-14 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. REACTIONS (size) 1=3-4-6, 3=3-4-6 Max Horiz 1=60 (LC 9) Max Uplift 1=-19 (LC 12), 3=-34 (LC 12) Max Grav 1=120 (LC 1), 3=120 (LC 1) FORCES (lb) - Maximum Compression/Maximum Tension

#### TOP CHORD 1-2=-84/57, 2-3=-94/121 BOT CHORD 1-3=-28/30

### NOTES

- Wind: ASCE 7-16; Vult=115mph (3-second gust) 1) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing. 3)
- Gable studs spaced at 4-0-0 oc. 4)
- This truss has been designed for a 10.0 psf bottom 5) chord live load nonconcurrent with any other live loads.

All bearings are assumed to be SP No.2 crushing 6) capacity of 565 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1 and 34 lb uplift at joint 3.

OF MISSOL TE SCOTT M. SEVIER UMBER OFFESSIONAL ET PE-2001018807

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						RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type	Qty	Ply	Roof - BY Lot 2050	AS NOTED FOR PLAN REVIEW
P250360-01	V5	Valley	1	1	Job Reference (optional	LEE'S SUMMIT, MISSOURI
Premier Building Supply (Springh	ill, KS), Spring Hills, KS - 66083,	R	tun: 8.63 S Sep 26 2024 Prin D:_15GiA9FkqWtQZlfziGqc7z	t: 8.630 S Sep 2 aioo-RfC?PsB70	6 2024 MiTek Industries, Inc. Hq3NSgPqnL8w3uITXbGKW	ue Oct 0.5%19/2025
		0-1-15 5-	8-7		11-2-15	11-4-14
		-	<u> </u>		500	



11-4-14

											1	
(psf) 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018	8/TPI2014	CSI TC BC WB Matrix-S	0.56 0.32 0.09	<b>DEFL</b> Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 38 lb	<b>GRIP</b> 197/144 FT = 20%
2x4 SP No.2 2x4 SP No.2 2x3 SPF No.2 Structural wood shea 6-0-0 oc purlins. Rigid ceiling directly bracing. (size) 1=11-4-14 Max Horiz 1=-54 (LC Max Uplift 1=-57 (LC 4=-43 (LC Max Grav 1=242 (LC 4=543 (LC	athing directly appli applied or 10-0-0 o I, 3=11-4-14, 4=11- 17) 12), 3=-67 (LC 13) 12) 22), 3=242 (LC 26 21)	6) 7) ed or 9) 4-14 1, 5),	This truss ha chord live lo All bearings capacity of 5 Provide mec bearing plat 1, 67 lb uplif This truss is Internationa R802.10.2 a DAD CASE(S)	as been desigr ad nonconcurr are assumed 1 565 psi. chanical conne e capable of w t at joint 3 and designed in a Residential C nd referenced Standard	ned for a 10.1 rent with any to be SP No. action (by oth ithstanding 5 43 lb uplift a ccordance w ode sections standard AN	) psf bottom other live loa 2 crushing ers) of truss 7 lb uplift at 1 joint 4. ith the 2018 R502.11.1 a ISI/TPI 1.	ads. to joint					
(lb) - Maximum Com Tension 1-2=-149/110, 2-3=- 1-4=-4/63, 3-4=-4/63 2-4=-371/245 ed roof live loads have n. CE 7-16; Vult=115mph	pression/Maximum 149/99 been considered fo (3-second gust)	or										
	(psf) 25.0 10.0 0.0 10.0 2x4 SP No.2 2x4 SP No.2 2x3 SPF No.2 Structural wood shead 6-0-0 oc purlins. Rigid ceiling directly bracing. (size) 1=11-4-14 Max Horiz 1=-54 (LC Max Uplift 1=-57 (LC 4=-43 (LC Max Grav 1=242 (LC 4=-543 (LC (lb) - Maximum Com Tension 1-2=-149/110, 2-3=- 1-4=-4/63, 3-4=-4/63 2-4=-371/245 ed roof live loads have n. CE 7-16; Vult=115mph	(psf)         Spacing           25.0         Plate Grip DOL           10.0         Lumber DOL           0.0         Rep Stress Incr           10.0         Code	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	(psf) $25.0$ $10.0$ Spacing Plate Grip DOL $1.15$ 2-0-0 $1.15$ CSI TC TC BC WB Matrix-S $10.0$ Lumber DOL Rep Stress Incr Code $1.15$ RC2018/TPI2014BC WB Matrix-S $2x4$ SP No.2 $2x3$ SPF No.2 $Code$ IRC2018/TPI2014Matrix-S $2x4$ SP No.2 $2x3$ SPF No.2 $6$ )This truss has been design chord live load nonconcurr $7$ )All bearings are assumed capacity of 565 psi. $8$ Structural wood sheathing directly applied or $6-0-0$ oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. $6$ )This truss has been design chord live load nonconcurr $7$ )All bearings are assumed capacity of 565 psi. $8$ $8$ $9$ This truss is designed in a liternational Residential conne bearing plate capable of w $1, 67$ Ib uplift at joint 3 and $9$ ) $(size)$ $1=11-4-14, 3=11-4-14, 4=11-4-14$ Max Horiz $1=-54$ (LC 17) Max Uplift $1=-57$ (LC 12), $3=-67$ (LC 13), $4=-43$ (LC 1) $802.10.2$ and referenced LOAD CASE(S) $Max mum Compression/MaximumTension1-2=-149/110, 2-3=-149/991-4=-4/63, 3-4=-4/632-4=-371/2452-4=-371/245ed roof live loads have been considered forn.CE 7-16; Vult=115mph (3-second gust)2-0-01-2=-16 (Vult=115mph (3-second gust)$	(psf) $25.0$ $10.0$ Spacing Plate Grip DOL $1.15$ 2-0-0 $TC$ CSI $TC$ $0.0$ $10.0$ Plate Grip DOL Lumber DOL $1.15$ $TC$ $0.56$ BC $0.0$ $10.0$ Rep Stress Incr CodeYES (RC2018/TPI2014)WB $0.09$ Matrix-S $2x4$ SP No.2 $2x3$ SPF No.2 $Code$ IRC2018/TPI2014Matrix-S $2x4$ SP No.2 $2x3$ SPF No.2 $6$ This truss has been designed for a 10.0 chord live load nonconcurrent with any $7$ $2x4$ SP No.2 $2x3$ SPF No.2 $6$ This truss has been designed for a 10.0 chord live load nonconcurrent with any $7$ $3II$ bearings are assumed to be SP No. $2x3$ SPF No.2 $6$ This truss in a seeigned in accordance w International Residential Code sections R802.10.2 and referenced standard AN LOAD CASE(S) $Structural wood sheathing directly applied ortracing.(size) 1=11-4-14, 3=11-4-14, 4=11-4-14Max Horiz 1=-54 (LC 17)Max Uplift 1=-57 (LC 12), 3=-67 (LC 13),4=-43 (LC 12)Max Grav 1=242 (LC 25), 3=-242 (LC 26),4=543 (LC 1)A=43 (LC 1)(lb) - Maximum Compression/MaximumTension1-2=-149/110, 2-3=-149/991-4=-4/63, 3-4=-4/632-4=-371/245ed roof live loads have been considered forn.CE 7-16; Vult=115mph (3-second gust)$	(psf) $25.0$ $10.0$ Spacing Plate Grip DOL $1.15$ 2-0-0 $1.5$ CSI $TC$ $0.56$ DEFL Vert(LL) Vert(LL) Vert(TL) Horiz(TL) $0.0$ $0.0$ $0.0$ $10.0$ $0.0$ $10.0$ $0.0$ Code $1.15$ $Rep Stress Incr(YES)TC0.0Matrix-SDEFLVert(LL)Vert(LL)Vert(TL)Horiz(TL)2x4 SP No.22x4 SP No.2CodeRC2018/TPI2014Matrix-S2x4 SP No.22x3 SPF No.26This truss has been designed for a 10.0 psf bottomchord live load nonconcurrent with any other live loadchord live load nonconcurrent with any other live loadrapacity of 565 psi.6-0- 0 cc purlins.Rigid ceiling directly applied or 10-0-0 ccbracing.6(size)1=11-4-14, 3=11-4-14, 4=11-4-14Max Horiz1=-57 (LC 12), 3=-67 (LC 13),4=-43 (LC 17)802.10.2 and referenced standard ANSI/TPI 1.LOAD CASE(S)Max Grav1-2e-149/110, 2-3=-149/991-4=-4/63, 3-4=-4/632-4=-371/245Codeed roof live loads have been considered forn.CE 7-16; Vult=115mph (3-second gust)Code$	(psf) 25.0 10.0Spacing Plate Grip DOL Lumber DOL Lumber DOL Code2-0-0 1.15 Rep Stress Incr YES CodeCSI TC O.66 BC O.32 WB Matrix-SDEFL Vert(LL) n/a Horiz(TL)in Vert(LL) n/a Horiz(TL)2x4 SP No.2 2x3 SPF No.26)This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.2x4 SP No.2 2x3 SPF No.26)This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.3Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.6)(size)1=11-4-14, 3=11-4-14, 4=11-4-14 Max Horiz 1=-54 (LC 17) Max Grav6)Max Grav1=242 (LC 25), 3=242 (LC 26), 4=543 (LC 1)9)(ib) - Maximum Compression/Maximum Tension 1-2=-149/110, 2-3=-149/99 1-4=-4/63, 3-4=-4/63 2-4=-371/24510ed roof live loads have been considered for n. CE 7-16; Vult=115mph (3-second gust)Co-0	(psf) $25.0$ $1.00$ $1.00$ Spacing Plate Grip DOL $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.15$ $1.1$	(psf) $25.0$ $10.0$ Spacing Plate Grip DOL $1.15$ 2-0-0 $1.15$ CSI TC $C$ DEFL $Vert(LL)$ $n'a$ in $(loc)$ //defl Vert(TL) $n'a$ $0.0$ $0.0$ $0.0$ Rep Stress Incr CodeYES IRC2018/TPI2014BC WB Matrix-S $0.09$ Vert(TL) Vert(TL) $n'a$ $n'a$ $2x4$ SP No.2 $2x3$ SPF No.2 $0.09$ Atrix-S $0.09$ Horiz(TL) $0.00$ $0.00$ $0.00$ $2x4$ SP No.2 $2x3$ SPF No.2 $0.09$ $1.15$ Code $0.09$ $1.15$ $0.00$ $0.00$ $0.00$ $3x$ SPF No.2 $2x3$ SPF No.2 $0.09$ $1.12$ $0.00$ $0.00$ $0.00$ $0.00$ $3x$ SPF No.2 $2x3$ SPF No.2 $0.09$ $0.09$ $0.09$ $0.00$ $3x$ SPF No.2 $2x3$ SPF No.2 $0.20$ $2x3$ SPF No.2 $0.09$ $0.00$ $0.00$ $3x$ SPF No.2 $2x3$ SPF No.2 $0.09$ $0.00$ $0.00$ $0.00$ $3x$ SPF No.2 $2x3$ SPF No.2 $0.09$ $0.00$ $0.00$ $0.00$ $3x$ SPF No.2 $2x3$ SPF No.2 $0.00$ $0.00$ $0.00$ $0.00$ $3x$ SPF No.2 $2x3$ SPF No.2 $0.000$ $0.000$ $0.000$ $3x$ SPF No.2 $2x3$ SPF No.2 $0.000$ $0.000$ $0.000$ $3x$ SPF No.2 $2x3$ $0.0000$ $0.0000$ $0.0000$ $3x$ SPF No.2 $2x3$ $0.00000000000000000000000000000000000$	$ \begin{array}{ c c c c c } \hline (psf) \\ 25.0 \\ Plate Grip DOL 1.15 \\ Nep Stress Incr YES \\ 0.0 \\ 10.0 \\ 0.0 \\ 10.0 \\ \hline (b) \\ Code \\ \hline (RC2018/TPI2014 \\ \hline (b) \\ (b) \\ Code \\ \hline (RC2018/TPI2014 \\ \hline (b) \\ (b) \\ (b) \\ (c) \\ (RC2018/TPI2014 \\ \hline (RC2018/TPI201$	(psf)         Spacing         2-0-0         CSI         DEFL         in         (loc)         //deft         L/d           10.0         Lumber DOL         1.15         TC         0.58         Vert(LL)         n/a         -         n/a         999         MT20           0.0         Rep Stress Incr         YES         WB         0.09         Horiz(TL)         0.00         3         n/a         n/a         999         MT20           2x4 SP No.2         Code         IRC2018/TPI2014         Matrix-S         WB         0.00 pd horiz(TL)         0.00 3         n/a         n/a         Weight: 38 lb           2x4 SP No.2         Structural wood sheathing directly applied or 6-0-0 cc purlins.         6)         This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.         7)         All bearings are assumed to be SP No.2 crushing capacity of 565 psi.         9 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1.         9         Phistruss is designed in accordance with the 2018 Intermational Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP11.         LOAD CASE(S)         Standard           (size)         1=11-4-14, 4, 3=11-4-14, 4=11-4-14         Max Grav         1=242 (LC 25), 3=242 (LC 26), 4=543 (LC 1)         1.2 and referenced standard ANSI/TP11.

- 2) Wind: ASCE 7-16; Vult=115mpn (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-1-15 to 4-10-1, Interior (1) 4-10-1 to 5-6-8, Exterior(2R) 5-6-8 to 10-6-8, Interior (1) 10-6-8 to 11-2-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
   Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 4-0-0 oc.



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LUMBER		
TOP CHORD	2x4 SP N	0.2
BOT CHORD	2x4 SP N	0.2
OTHERS	2x3 SPF I	No.2
BRACING		
TOP CHORD	Structura	wood sheathing directly applied or
	6-0-0 oc p	ourlins.
BOT CHORD	Rigid ceil	ing directly applied or 10-0-0 oc
	bracing.	
REACTIONS	(size)	1=8-8-8, 3=8-8-8, 4=8-8-8
	Max Horiz	1=36 (LC 12)
	Max Uplift	1=-46 (LC 12), 3=-53 (LC 13),
		4=-11 (LC 12)
	Max Grav	1=177 (LC 1), 3=177 (LC 1), 4=324
		(LC 1)
FORCES	(lb) - Max	imum Compression/Maximum
	Tension	-

#### TOP CHORD 1-2=-85/62, 2-3=-85/70 BOT CHORD 1-4=-1/38, 3-4=-1/38 2-4=-230/176 WEBS

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) 2) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. Gable requires continuous bottom chord bearing. 4)
- 5) Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom 6)
- chord live load nonconcurrent with any other live loads.

7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 8)

Provide mechanical connection (by others) of truss to

- bearing plate capable of withstanding 46 lb uplift at joint
- 1, 53 lb uplift at joint 3 and 11 lb uplift at joint 4. This truss is designed in accordance with the 2018
- 9) International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



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										- [	RELEASE	FOR CONSTRUCTION
Job	Truss		Truss Type		Qty	/ Ply	R	loof - BY	Lot 205	0	AS NOTE	ED FOR PLAN REVIEW
P250360-01	V7		Valley		1	1	J	ob Refere	ence (or	otional	LEE'S	I68913080 SUMMIT, MISSOURI
Premier Building Supply (\$	Springhill, KS), S	Spring Hills, KS - 66083,		Run: 8.63 S Sep ID:_15GiA9FkqV	o 26 2024 VtQZlfziGo	Print: 8.630 S c7zaioo-RfC	Sep 26 2 PsB70Ho	024 MiTek I3NSgPqnL	Industrie _8w3uITX	s, Inc. (bGKW)	ue Oct 15 157:59 CDoi7J429C	19/2025
					<u>2-4</u> 2-4	-4		<u>4-1-9</u> 1-9-5		4-8-8 0-6-1	3	
							3x4 =					
		1-2-6 0-10-10	- - -	6 T 6 T			2			3	8	
					∞∞∞∞ 3x4 ≠	~~~~~	******	~~~~~	3x4	*	2	
Scale = 1:21.9							4-8-8				-	
Plate Offsets (X, Y): [2	2:0-2-0,Edge]	-		_								
Loading TCLL (roof) TCDL BCLL	(psf) 25.0 10.0 0.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI TC BC WB	0.09 0.15 0.00	<b>DEFL</b> Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-P		(-=)					Weight: 13 lb	FT = 20%
LUMBER			8) Provide me	chanical connection	n (by oth	ers) of truss	s to					

BOT CHORD 2x4 SP No.2 BRACING TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

 REACTIONS
 (size)
 1=4-8-8, 3=4-8-8

 Max Horiz
 1=-17 (LC 13)

 Max Uplift
 1=-24 (LC 12), 3=-24 (LC 13)

 Max Grav
 1=159 (LC 1), 3=159 (LC 1)

 FORCES
 (lb) - Maximum Compression/Maximum Tension

 TOP CHORD
 1-2=-160/143, 2-3=-160/151

BOT CHORD 1-3=-98/123

TOP CHORD 2x4 SP No.2

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) All bearings are assumed to be SP No.2 crushing
- capacity of 565 psi.

- bearing plate capable of withstanding 24 lb uplift at joint 1 and 24 lb uplift at joint 3.
  9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and
- R802.10.2 and referenced standard ANSI/TPI 1.
- LOAD CASE(S) Standard



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