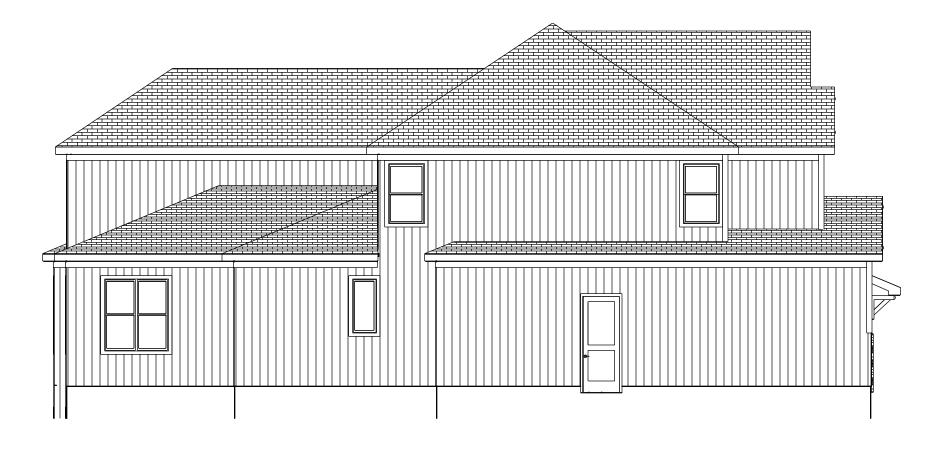
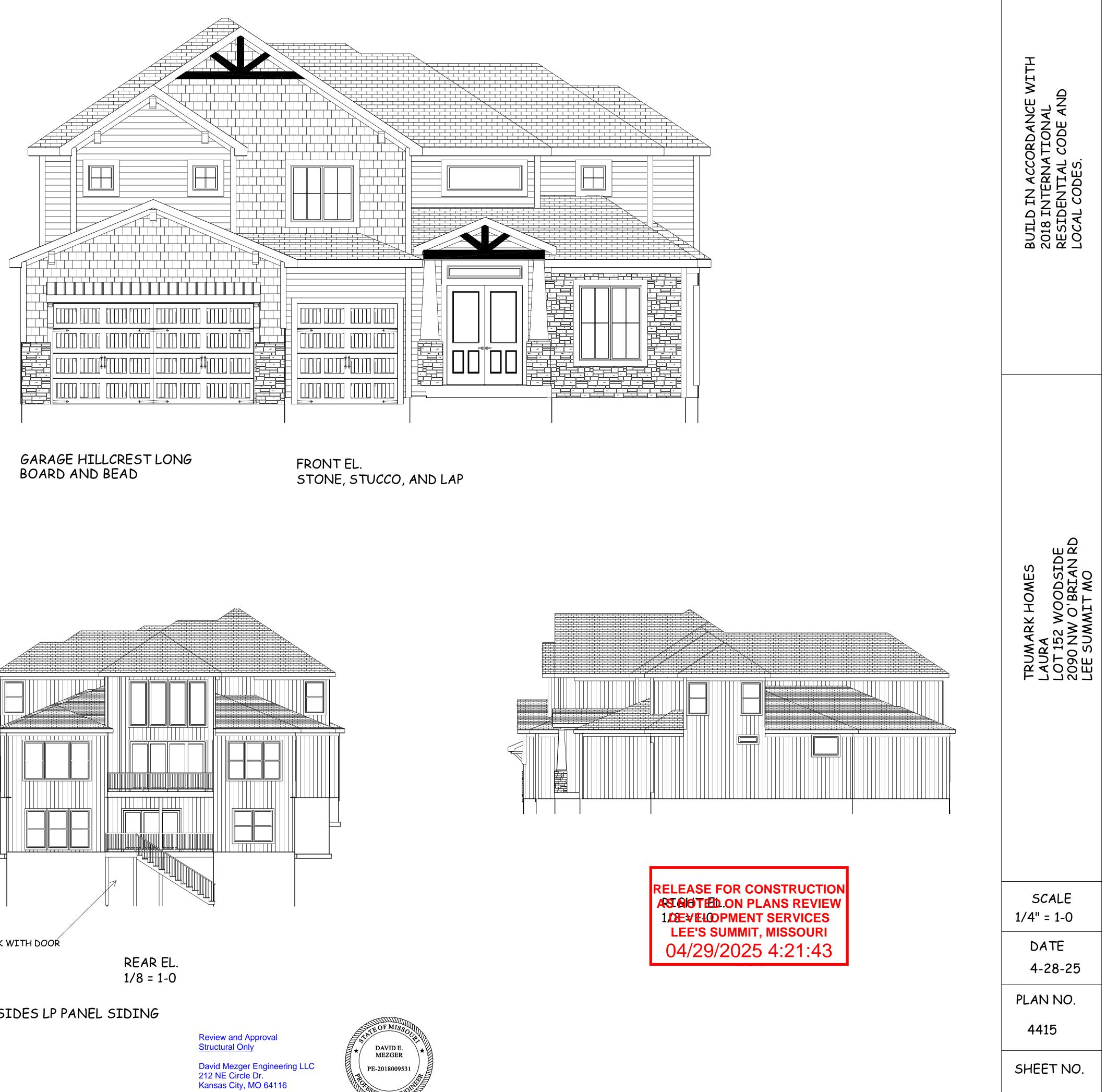


ROOF PLAN 1/8 = 1-0 ROOF PITCHES 5/12 MAIN FLOOR U.N.O. ROOF PITCHES 8/12 SECOND FLOOR U.N.O. RAFTERS 2 X 6 DF NO 2 @ 16" OC TYP. HIPS AND RIDGES 2 X 8 DF NO 2 TYP.



LEFT EL. 1/8 = 1-0

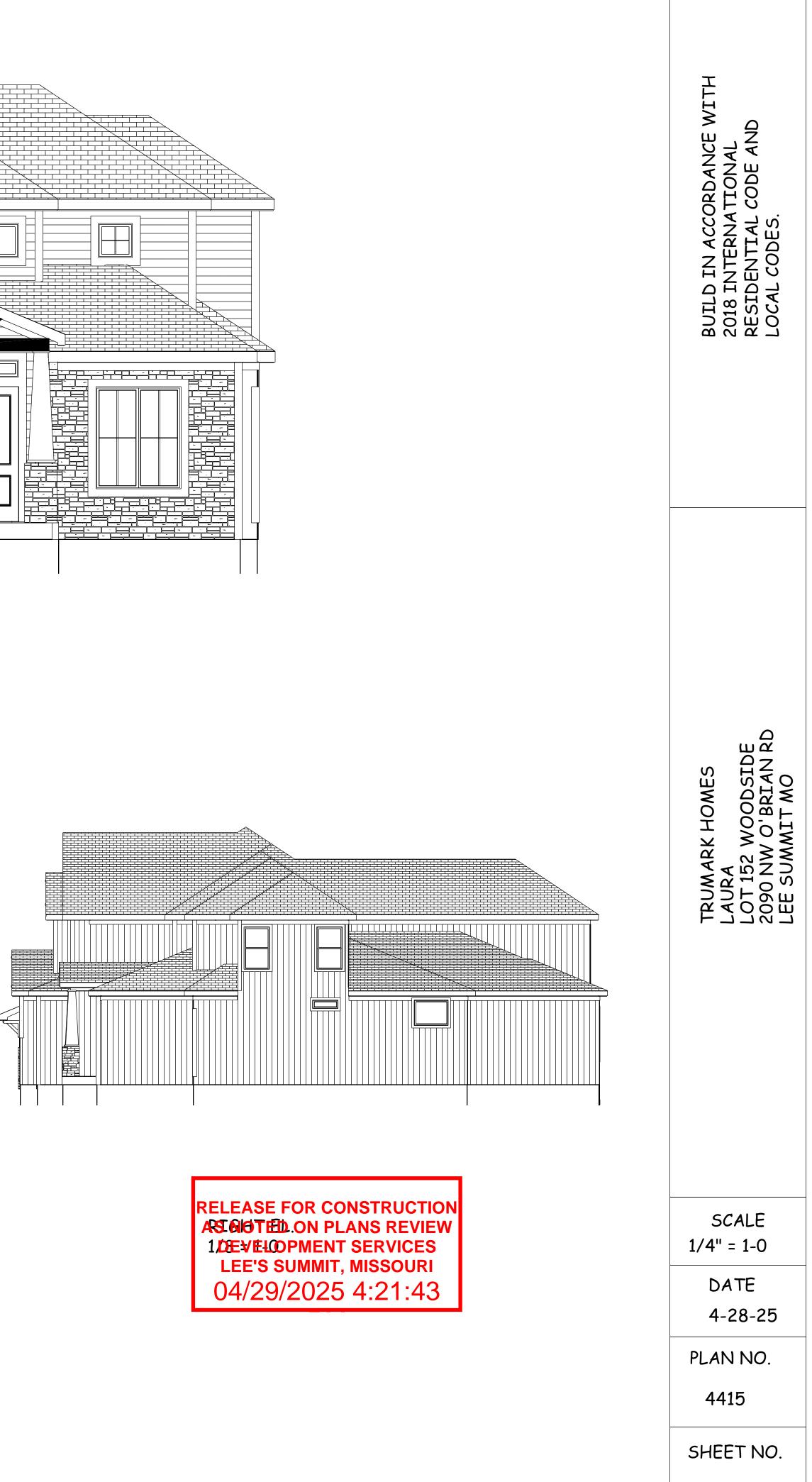
NOTE: ENCLOSED LOWER DECK WITH DOOR

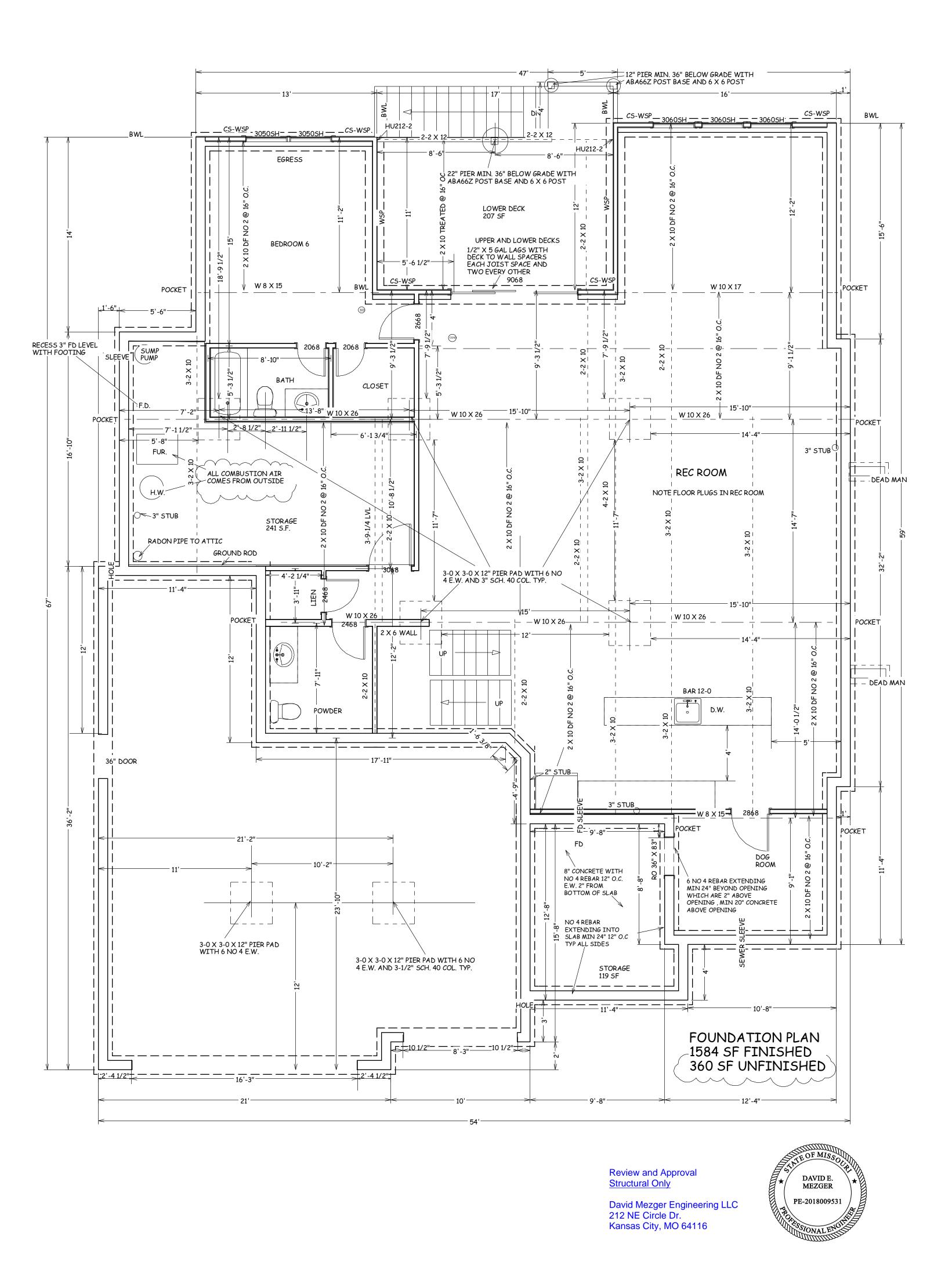




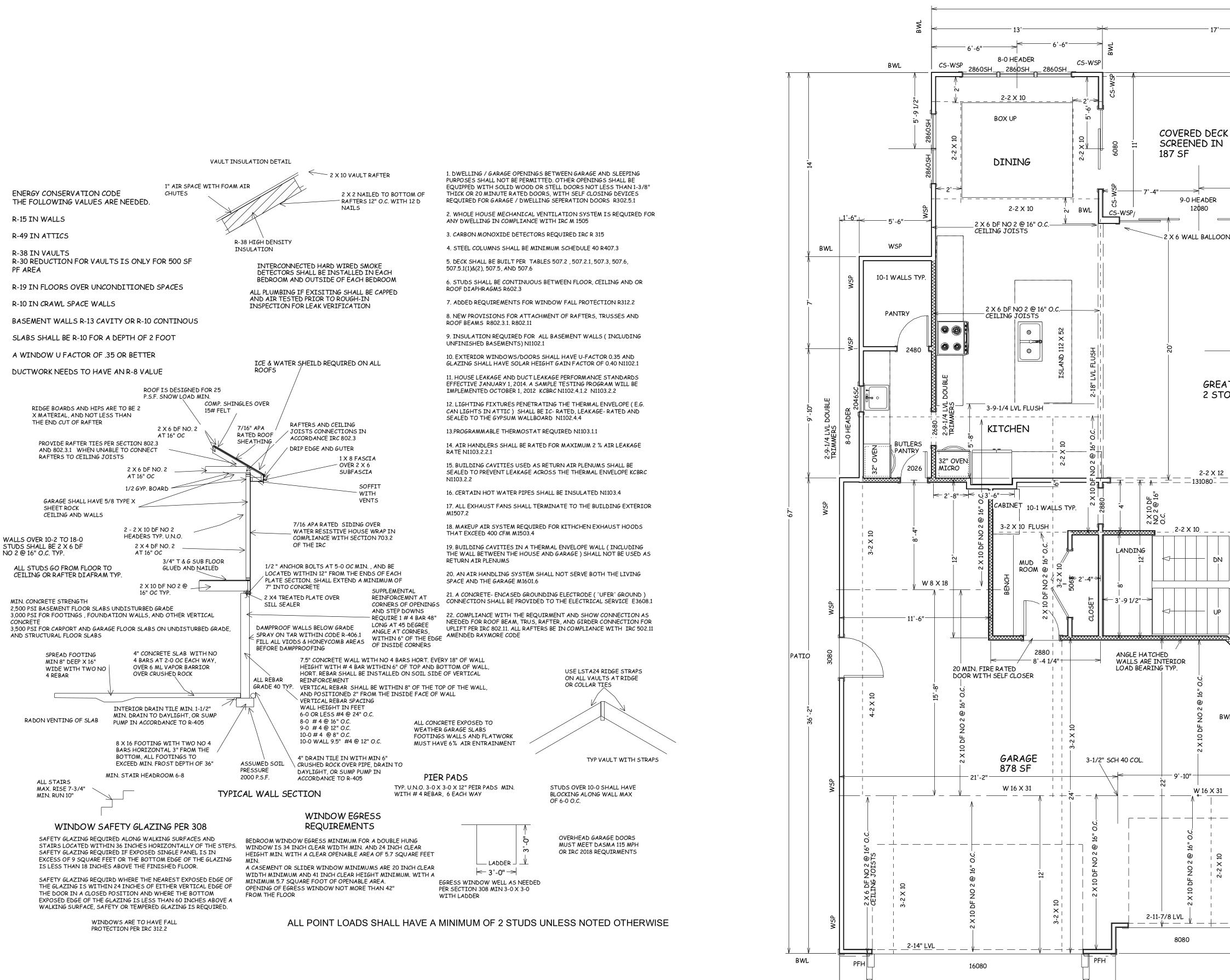
3 SIDES LP PANEL SIDING







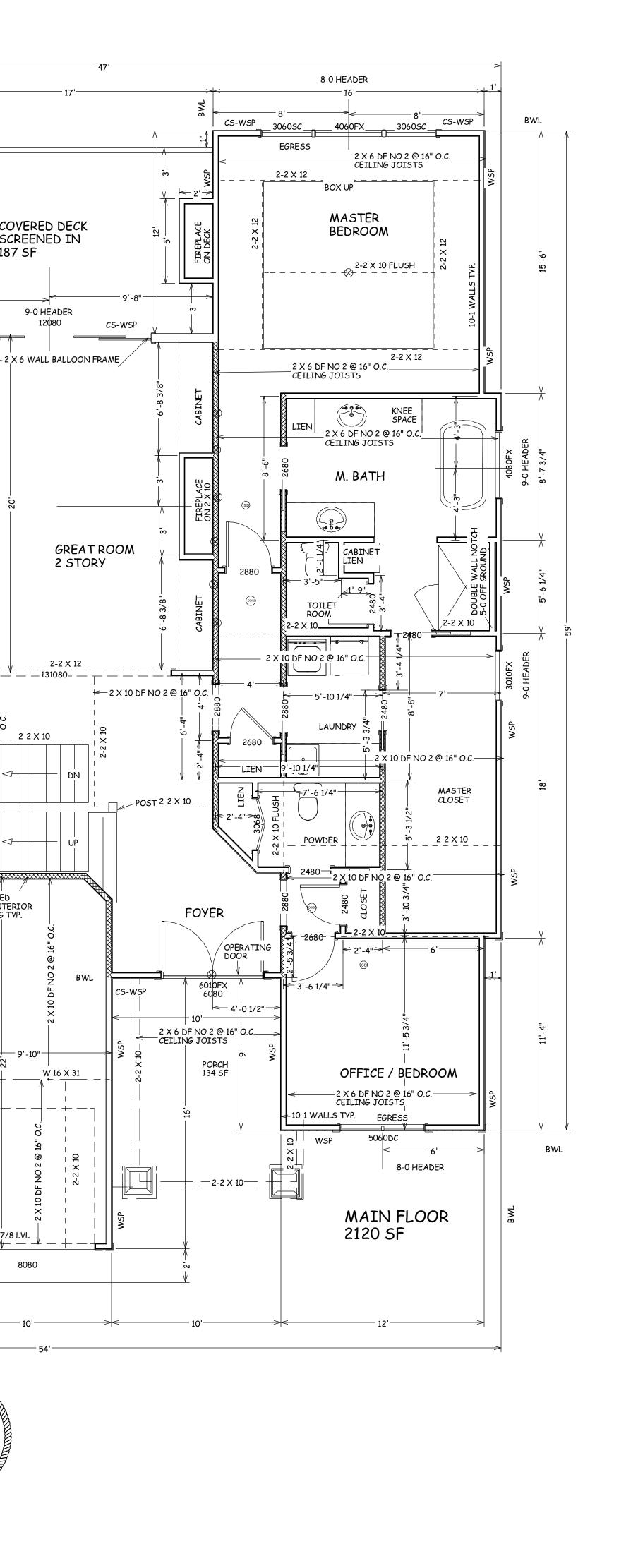
BUILD IN ACCORDANCE WITH 2018 INTERNATIONAL RESIDENTIAL CODE AND LOCAL CODES.	
TRUMARK HOMES LAURA LOT 152 WOODSIDE 2090 NW O'BRIAN RD LEE SUMMIT MO	
SCALE 1/4" = 1-0 DATE 4-28-25 PLAN NO. 4415 SHEET NO. 2 DE 5 FOR COM AS NOTED ON PL/ DEVELOPMENT LEE'S SUMMIT, 04/29/2025	SERVICES MISSOURI



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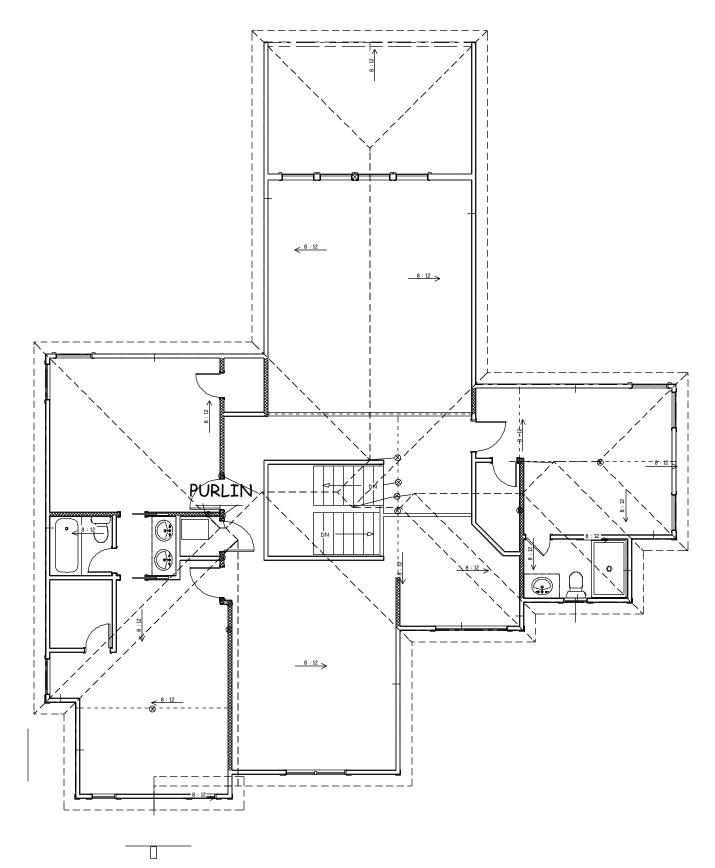




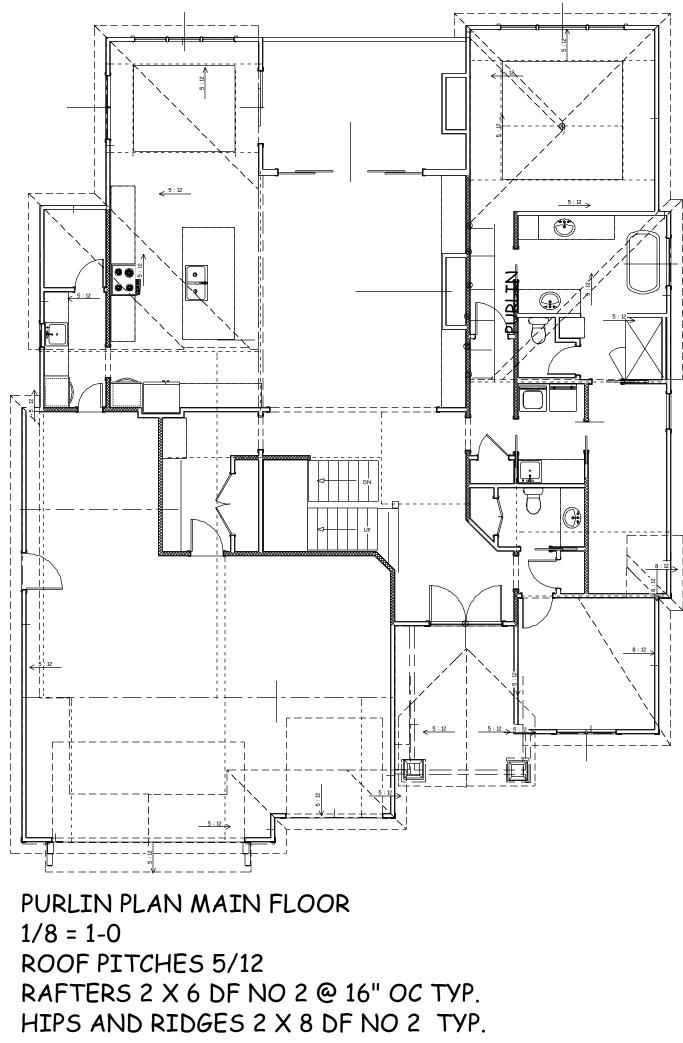


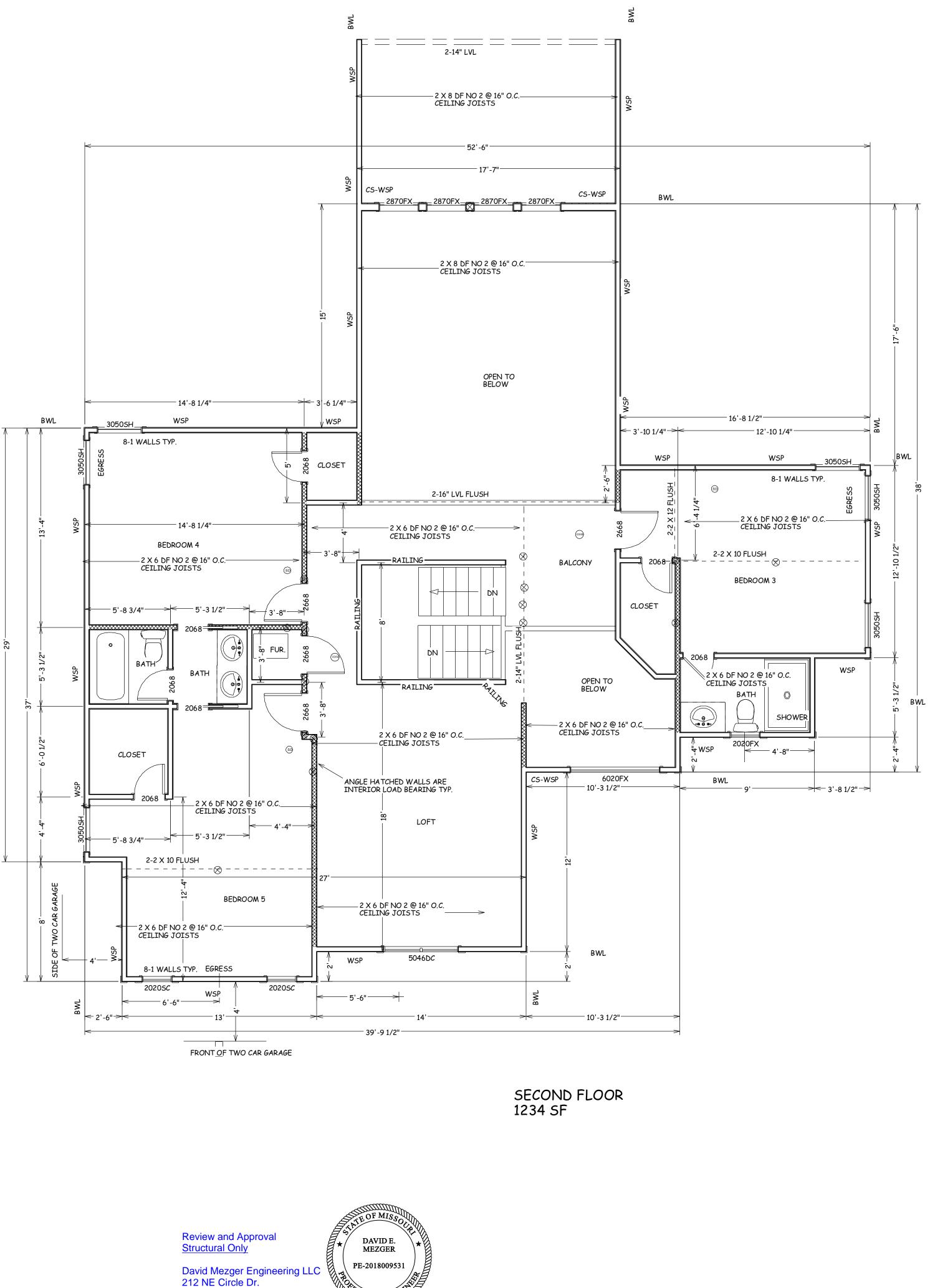
DEVELOPMENT SERVICES

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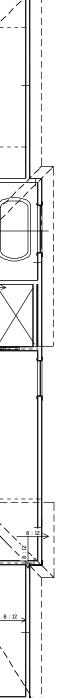
PURLIN PLAN SECOND FLOOR 1/8 = 1-0 ROOF PITCHES 8/12 RAFTERS 2 X 6 DF NO 2 @ 16" OC TYP. HIPS AND RIDGES 2 X 8 DF NO 2 TYP.







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 \square IDE N R[S TRUMARK HOMES LAURA LOT 152 WOODSI 2090 NW O'BRIAN LEE SUMMIT MO

SCALE 1/4" = 1-0

> DATE 4-28-25

PLAN NO.

4415

SHEET NO.

4 OF 5 RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 04/29/2025 4:21:43

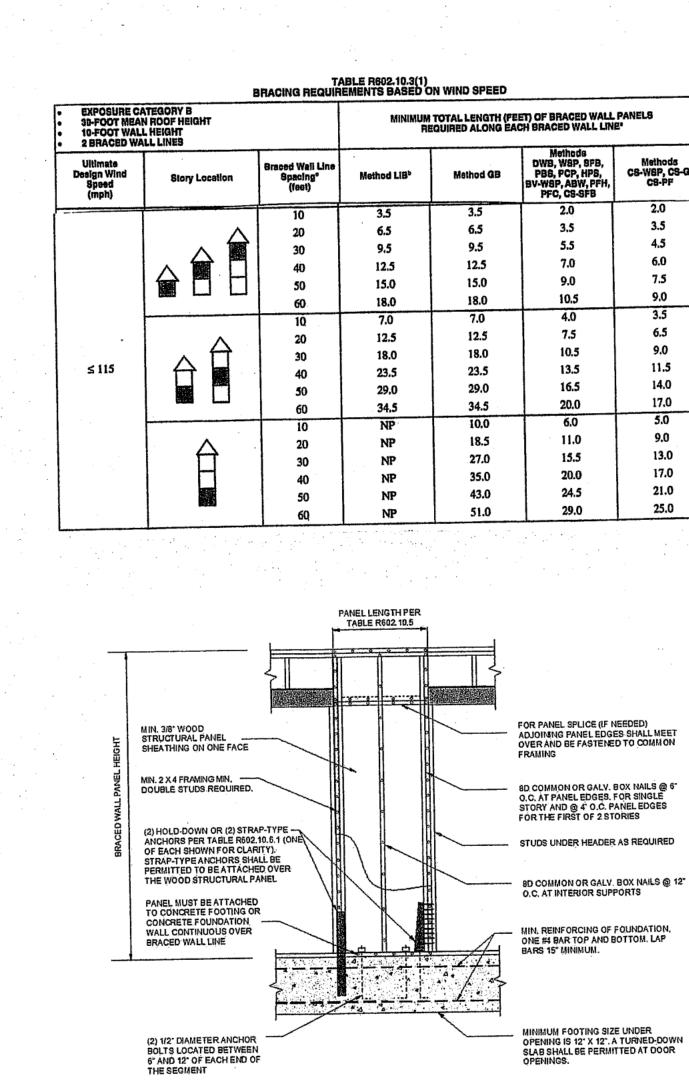
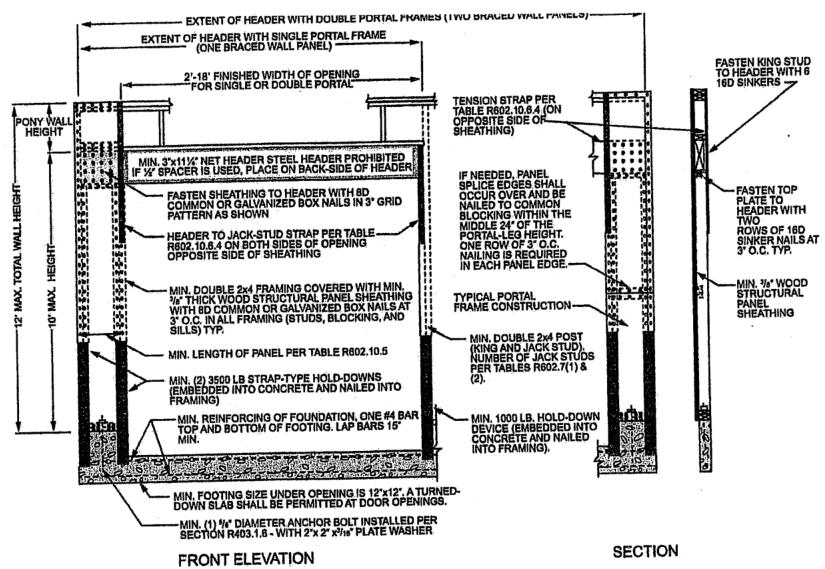


FIGURE R602.10.6.1 METHOD ABW---ALTERNATE BRACED WALL PANEL



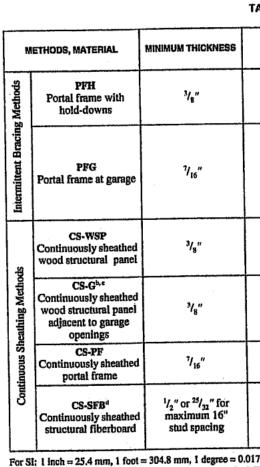
4 mm, 1 foot = 304.8 mm.

25.4 mm.

FIGURE R602.10.6.2 METHOD PFH---PORTAL FRAME WITH HOLD-DOWNS

D WALL PANELS ALL LINE'				
8 SFB, HPS, Y, PFH, SFB	Methods CS-W8P, CS-G, CS-PF			
1	2.0			
	3.5			
	4.5			
	6.0			
	7.5			
	9,0			
	3.5			
	6.5			
	9.0			
	11.5			
	14.0			
	17.0			
	5.0			
	9.0			
	13.0			
	17.0			
	21.0			
	25.0			

			TABLE R602.10 BRACING METHO	DDS		
	T	CONNECTION CRITERIA"			A" '	
MET	HODS, MATERIAL	TERIAL MINIMUM THICKNESS FIGURE Fasteners		Fasteners	Spacing	
	LIB	1×4 wood or approved metal straps at 45° to 60° angles for	NIIIIIIII		Wood: per stud and top and bottom plates	
	Let-in-bracing	maximum 16" stud spacing		Metal strap: per manufacturer	Metal: per manufacturer	
	DWB Diagonal wood boards	³ / ₄ " (1" nominal) for maximum 24" stud spacing		2-8d $(2^{1}/_{2}" \log \times 0.113" \text{ dia.})$ nails or 2 - $1^{3}/_{4}" \log \text{ staples}$	Per stud	
ł	WSP Wood			Exterior sheathing per Table R602.3(3)	6" edges 12" field	
	structural panel (See Section R604)	3/ ₈ "	³ / ₈ " Interior sheathing per Table R602.3(1) or R602.3(2)		Varies by fastener	
ethods	BV-WSP" Wood structural panels with stone or masonry veneer (See Section R602, 10.6.5)	7/ ₁₆ ″	$7'_{15}"$ See Figure R602.10.6.58d common $(2^{1}/_{2}" \times 0.131)$ nails $1'_{15}"$ $1'_{2}"$ or $2^{2}/_{32}"$ for maximum 16" stud spacing $1'_{2}"$ long $\times 0.12"$ dia. (for $1'_{2}"$ thick sheathing) $1^{3}/_{4}"$ long $\times 0.12"$ dia. (for $2^{2}/_{32}"$ thick sheathing) galvanized roofing nails		4" at panel edges 12" at intermediate supports 4" at braced wall panel end posts	
Intermittent Bracing Methods	SFB Structural fiberboard sheathing	maximum 16"			3" edges 6" field	
Intermitten	GB Gypsum board	1/2"		Nails or screws per Table R602.3(1) for exterior locations Nails or screws per Table R702.3.5 for interior locations	For all braced wall panel locations: 7" edges (including top and bottom plates) 7"	
	PBS Particleboard sheathing (See Section R605)	³ / ₈ " or ¹ /2" for maximum 16" stud spacing		For $\frac{3}{8}$, 6d common (2" long × 0.113" dia.) nails For $\frac{1}{2}$, 8d common (2'/ ₂ " long × 0.131" dia.) nails		
	PCP Portiand cement plaster	See Section R703.7 for maximum 16" stud spacing		$1^{1}/_{2}$ " long, 11 gage, $7^{1}/_{16}$ " dia. head nails or $7^{1}/_{8}$ " long, 16 gage staples	members	
	HPS Hardboard panel siding	⁷ / ₁₆ " for maximum 16" stud spacing		0.092" dia., 0.225" dia. head nails with length to accommodate 1 ¹ / ₂ " penetration into studs	4" edges 8" field	
	ABW Alternate braced wall	3/ ₈ "		See Section R602.10.6.1	See Section R602.10.6.1	



a. Adhesive attachment of wall sheathing, including Method GB
b. Applies to panels next to garage door opening where support Design Categories D₀, D₁ and D₂ roof covering dead load shal
c. Garage openings adjacent to a Method CS-G panel shall be pr be permitted adjacent to a Method CS-G panel.

d. Method CS-SFB does not apply in Seismic Design Categorie e. Method applies to detached one- and two-family dwellings

ADJOINING PANEL EDGES SHALL MEET OVER AND BE FASTENED TO COMMON

STUDS UNDER HEADER AS REQUIRED

8D COMMON OR GALV, BOX NAILS @ 12"

MIN, REINFORCING OF FOUNDATION

METHOD (See Table R602.10.4) DWB, WSP, SFB, PBS, PCP, HPS, BV-V GB LIB ABW ABW SDC A, B and C, design wind speed < 14 SDC D ₀ , D ₁ and D ₂ design wind speed < 14 SDC D ₀ , D ₁ and D ₂ design wind speed < 14 SDC C D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC C D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design Wind speed < 14 SDC D ₀ , D ₁ and D ₂ design (112) 112 116 120 124 128 132	48 55 ultimate 28 0 mph , ultimate 32 0 mph 24 ing height	9 feet 48 48 62 32 32 32 27 27 27 27 27 27 27 29 30 32 35 37 41 44	Wall Helg 10 feet 48 48 69 34 34 34 30 30 30 30 30 30 30 30 30 30	nt 11 feet 53 53 NP 38 NP 33 33 33 33 33 33 33 33 33 3	12 feet 58 58 NP 42 NP 36 36 36 36 36 36 36 36 36 36 36 36 36	(Inches) Actual ^b Double sided = Actua Single sided = 0.5 × Act Actual ^b 48 Actual ^b	
GB LIB SDC A, B and C, design wind speed < 14	VSP 48 48 48 55 ultimate 0 mph , ultimate 32 40 mph 24 ing height 24 26 27 30 32 35 38 43 43	48 48 62 32 32 32 27 27 27 27 27 27 27 30 32 30 32 35 37 41 44	48 48 69 34 34 30 30 30 30 30 30 30 30 30 30 30 30 30	53 53 NP 38 NP 33 33 33 33 33 33 33 33 33 33 33 33 33	58 58 NP 42 NP 36 36 36 36 36 36 36 36 36 36 36 36 36	Double sided = Actua Single sided = 0.5 × Act Actual ^b 48	
GB LIB SDC A, B and C, design wind speed < 14	48 55 ultimate 0 mph , ultimate 32 40 mph 24 ing height 24 26 27 30 30 32 35 38 43 48 	48 62 32 32 27 27 27 27 27 27 27 27 27 29 30 32 35 37 41 41 44	48 69 34 34 30 30 30 30 30 30 30 30 30 30 30 32 33 35 38	53 NP 38 NP 33 33 33 33 33 33 33 33 33 33 33 33 33	58 NP 42 NP 36 36 36 36 36 36 36 36 36 36 36 36 36	Double sided = Actua Single sided = 0.5 × Act Actual ^b 48	
$LIB \\ SDC A, B and C, design wind speed < 14 \\ SDC D_0, D_1 and D_2 design wind speed < 14 \\ CS-G \\ Adjacent clear open (inches) \\ \leq 64 \\ 68 \\ 72 \\ 76 \\ 80 \\ 84 \\ 88 \\ 92 \\ 96 \\ CS-WSP, CS-SFB \\ 100 \\ 104 \\ 108 \\ 112 \\ 116 \\ 120 \\ 124 \\ 128 \\ 1$	55 ultimate 28 0 mph 28 0 mph 32 40 mph 24 ing height 24 26 27 30 32 35 38 43 43 48 	62 32 32 27 27 27 27 27 27 27 27 29 30 32 35 37 41 41	69 34 34 30 30 30 30 30 30 30 30 30 30 30 30 30	NP 38 NP 33 35 36	NP 42 NP 36 36 36 36 36 36 36 36 36 36 36 36 36	Single sided = 0.5 × Act Actual ^b 48	
$ABW = \begin{bmatrix} SDC A, B and C, \\ design \\ wind speed < 14 \\ SDC D_0, D_1 and D_2 \\ design \\ wind speed < 14 \\ \hline CS-G \\ \hline Adjacent clear open \\ (inches) \\ \hline \leq 64 \\ \hline 68 \\ \hline 72 \\ \hline 76 \\ \hline 80 \\ \hline 84 \\ \hline 88 \\ \hline 92 \\ \hline 96 \\ \hline CS-WSP, CS-SFB \\ \hline 100 \\ \hline 104 \\ \hline 108 \\ \hline 112 \\ \hline 116 \\ \hline 120 \\ \hline 124 \\ \hline 128 \\ \hline \end{bmatrix}$	ultimate 0 mph , ultimate 32 0 mph 24 ing height 24 26 27 30 32 35 38 43 43 48 	32 32 27 27 27 27 27 27 27 29 30 32 35 37 41 41	34 34 30 30 30 30 30 30 30 30 30 30 30 32 33 35 38	38 NP 33 33 33 33 33 33 33 33 33 33 33 33 33	42 NP 36 36 36 36 36 36 36 36 36 36 36 36 36	- 48	
$ABW = \begin{bmatrix} SDC A, B and C, \\ design \\ wind speed < 14 \\ SDC D_0, D_1 and D_2 \\ design \\ wind speed < 14 \\ \hline CS-G \\ \hline Adjacent clear open \\ (inches) \\ \hline \leq 64 \\ \hline 68 \\ \hline 72 \\ \hline 76 \\ \hline 80 \\ \hline 84 \\ \hline 88 \\ \hline 92 \\ \hline 96 \\ \hline CS-WSP, CS-SFB \\ \hline 100 \\ \hline 104 \\ \hline 108 \\ \hline 112 \\ \hline 116 \\ \hline 120 \\ \hline 124 \\ \hline 128 \\ \hline \end{bmatrix}$	28 0 mph , ultimate 32 40 mph 24 ing height 24 26 27 30 32 35 38 43 48	32 27 27 27 27 27 29 30 32 35 37 41 41	34 30 30 30 30 30 30 30 30 30 32 33 35 38	NP 33 33 33 33 33 33 33 33 33 33 33 35 36	NP 36 36 36 36 36 36 36 36 36 36 36 36 36		
CS-WSP, CS-SFB	32 40 mph 24 ing height 24 26 27 30 32 35 38 43 48	27 27 27 27 29 30 32 35 37 41 41	30 30 30 30 30 30 30 30 30 32 33 35 38	33 33 33 33 33 33 33 33 33 33 33 35 36	36 36 36 36 36 36 36 36 36 36 36 36	Actual ^b	
CS-G Adjacent clear open (inches)	24 ing height 24 26 27 30 32 35 38 43 43 48 	27 27 27 29 30 32 35 37 41 41 44	30 30 30 30 30 30 32 33 33 35 38	33 33 33 33 33 33 33 33 33 35 36	36 36 36 36 36 36 36 36 36 36 36	Actual [®]	
(inches) ≤ 64 68 72 76 80 84 88 92 96 CS-WSP, CS-SFB 100 104 108 112 116 120 124 128	24 26 27 30 32 35 38 43 43 48 	27 27 29 30 32 35 37 41 41 44	30 30 30 30 30 32 33 35 38	33 33 33 33 33 33 33 33 35 36	36 36 36 36 36 36 36 36 36 36		
CS-WSP, CS-SFB	26 27 30 32 35 38 43 43 48 	27 27 29 30 32 35 37 41 41 44	30 30 30 30 30 32 33 35 38	33 33 33 33 33 33 33 33 35 36	36 36 36 36 36 36 36 36 36 36		
72 76 80 84 92 96 CS-WSP, CS-SFB 100 104 108 112 116 120 124 128	27 30 32 35 38 43 43 48 	27 29 30 32 35 37 41 44	30 30 30 32 33 33 35 38	33 33 33 33 33 33 33 35 36	36 36 36 36 36 36 36 36		
76 80 84 92 96 CS-WSP, CS-SFB 100 104 108 112 116 120 124 128	30 32 35 38 43 48 	29 30 32 35 37 41 44	30 30 32 33 35 38	33 33 33 33 33 35 36	36 36 36 36 36 36 36		
80 84 88 92 96 100 104 108 112 116 120 124 128	32 35 38 43 48 —	30 32 35 37 41 44	30 32 33 35 38	33 33 33 33 35 36	36 36 36 36 36 36		
84 88 92 96 100 104 108 112 116 120 124 128	35 38 43 48 	32 35 37 41 44	32 33 35 38	33 33 35 36	36 36 36 36		
CS-WSP, CS-SFB	38 43 48 ——	35 37 41 44	33 35 38	33 35 36	36 36 36	-	
92 96 CS-WSP, CS-SFB 100 104 108 112 116 120 124 128	43 48 —	37 41 44	35 38	35 36	36 36	-	
CS-WSP, CS-SFB 96 100 104 108 112 116 120 124 128	48	41	38	36	36	-	
CS-WSP, CS-SFB 100 104 108 112 116 120 124 128		44				-	
104 108 112 116 120 124 128				1 22	38	-	
108 112 116 120 124 128		49	40	40	39	Actual ^b	
112 116 120 124 128		54	46	43	41	-	
116 120 124 128			50	45	43	-	
120 124 128		<u> </u>	55	48	45		
124 128			60	52	48	-	
128		<u> </u>		56	51	-	
				61	54	-	
				66	58	-	
136			+=		62		
140					66		
144					72	-	
METHOD			Portal head	er helght	elght		
(See Table R602,10.4)	8 fe	et 9 fee		10 feet 11 feet			
Supporting ro				Note c			
PFH Supporting one st				Note c			
PFG	24			Note d	the second s		
SDC A, B				Note e		the second se	
CS-PF SDC D_0 , D_1 SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mi			20	Note e	Note e	e Actual ⁶	

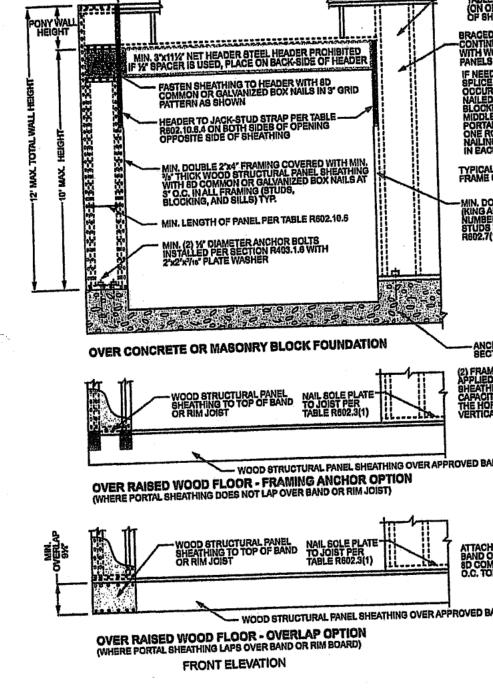
c. Maximum header height for PFH is 10 feet in accordance with Figure R602.10.6.2, but wall height shall be permitted to be increased to 12 feet with pony wall.
d. Maximum header height for PFG is 10 feet in accordance with Figure R602.10.6.3, but wall height shall be permitted to be increased to 12 feet with pony wall.
e. Maximum header height for CS-PF is 10 feet in accordance with Figure R602.10.6.4, but wall height shall be permitted to be increased to 12 feet with pony wall.

BRACE WALL DETAILS WIND SPEED 115 MPH WIND EXPOSURE A SEISMIC DESIGN CAEGORY A

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For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

FIGURE R602.10.6.4 METHOD CS-PF-CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION

					•
		TABLE R602.10.4con BRACING METHOD	tinued DS		
			CONNECTION	CRITERIA	
TERIAL	MINIMUM THICKNESS	FIGURE	Fastenera	Specing	
7 H ame with downs	3/8"		See Section R602.10.6.2	See Section R602.10.6.2	
FG ne at garage	7/ ₁₆ "		See Section R602.10.6.3	See Section R602.10.6.3	
WSP			Exterior sheathing per Table R602.3(3)	6" edges 12" field	
sly sheathed ctural panel	3/ ₈ "		Interior sheathing per Table R602.3(1) or R602.3(2)	Varies by fastener	
-G ^{b, c} sly sheathed ctural panel to garage mings	3/g″		See Method CS-WSP	See Method CS-WSP	
S-PF sly sheathed I frame	7/ ₁₆ "		See Section R602.10.6.4	See Section R602.10.6.4	
-SFB ^d isly sheathed i fiberboard	¹ / ₂ " or ²⁵ / ₃₂ " for maximum 16" stud spacing		$1^{1}/_{2}^{"}$ long × 0.12" dia. (for $1^{1}/_{2}$ " thick sheathing) $1^{3}/_{4}^{"}$ long × 0.12" dia. (for $2^{2}/_{32}$ " thick sheathing) galvanized roofing nails	3" edges 6" field	
es D ₀ , D ₁ and i s adjacent to a acent to a Met does not appl	D ₂ roof covering dead loa Method CS-G panel shall hod CS-G panel. y in Seismic Design Cate	d shall not exceed 3 pst. be provided with a header in	no of load only. Shall only be used on a accordance with Table R602.7(1). A pries D_0 through D_2 only.		
			•		-
				CONTINUOUGLY 8HEA WALL PANEL MEETING LENGTH REQUIREMEN R802,10.5	THED BRACED MINIUM VT9 OF TABLE
EXTENT OF	XTENT OF HEADER WITH F HEADER WITH SINGLE I (ONE BRACED WALL PAY	DOUBLE FORTAL FRAMES (TWO BRACED WALL PANELS		
	-18' FINISHED WIDTH OF FOR SINGLE OR DOUBLE				FASTEN KING STUD TO HEADER WITH 6 16D SINKERS
	FOR SINGLE OR DOUBLE		TENSION STRAP P TABLE 602,10.8.4 (ON OPPOSITE SIL OF SHEATHING)		
		ADER PROHIBITED	BRACED WALL LINE CONTINUOUSLY SHI WITH WOOD STRUC		
IF 1/2 SPACE	Y NET HEADER STEEL HI R IS USED, PLACE ON BA	CK-SIDE OF HEADER		Alter Alter Alter Alter Alter	FASTEN TOP PLATE TO
COM PATT HEAT RB02 OPPI	EN SHEATHING TO HEAD MON OR GALVANIZED BC ERN AS SHOWN DER TO JACK-STUD STRV 10.8,4 ON BOTH SIDES C OSITE SIDE OF SHEATHIN	AP PER TABLE	IF NEEDED, PANEL SPLICE EDGES BHA OCCUR OVER AND NAILED TO COMMO BLOCKING WITHIN MIDDLE 24° OF THE PORTAL-LEG HEIG ONE ROW OF 3° O. NAILING IS REQUIR IN EACH PANEL ED		HEADER WITH TWO ROWS OF 16D SINKER NAILS AT 3" O.C. TYP.
	DOUBLE 2'x4' FRAMING HICK WOOD STRUCTUR		IN EACH PANELED TYPICAL PORTAL FRAME CONSTRUCT		MIN. 1/4" WOOD STRUCTURAL PANEL SHEATHING

-MIN. DOUBLE 2x4 POST (KING AND JACK STUD) NUMBER OF JACK STUDS PER TABLES R602.7(1) & (2) 0.00 ANCHOR BOLTS PER -- WOOD STRUCTURAL PANEL SHEATHING OVER APPROVED BAND OR RIM J

SECTION

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 \square IDE N RI S DDSJ BRIA T MO **ARK HOME** MI TRUMARK LAURA LOT 152 \ 2090 NW LEE SUM/

SCALE 1/4" = 1-0

> DATE 4-28-25

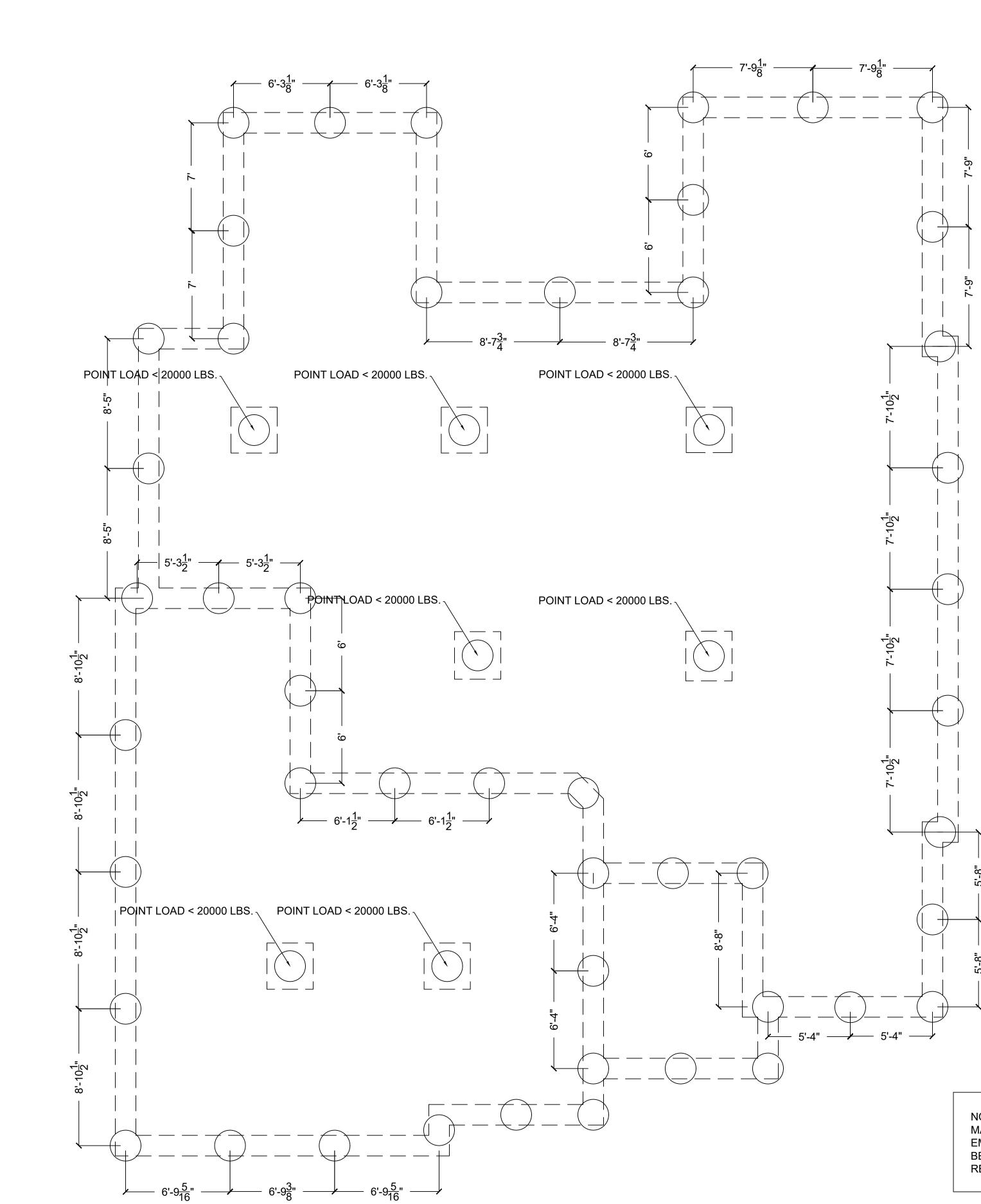
PLAN NO.

4415

SHEET NO.

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PILE CAPACITY (Qallow) CALCULATIONS (USING PRESUMPTIVE LOAD BEARING VALUES IN LIEU OF SOIL TEST DATA) FROM IRC 2018 TABLE R403.1(1), FOOTING CAPACITY MUST BE A MINIMUM OF 2333 PLF (14" WIDE X 2000 PSF). ASSUMPTIONS:

SOIL TYPE - GC (CLAYEY GRAVEL) FROM IBC 2018 TABLE 1802.2, BEARING PRESSURE CAPACITY AT THE SURFACE IS 2000 PSF WEIGHT OF SOIL 100 PCF

LATERAL BEARING PRESSURE 150 PSF/FT BELOW THE SURFACE

COEFFICIENT OF FRICTION (TO CALCULATE PILE SKIN FRICTION RESISTANCE) .25 SKIN FRICTION RESISTANCE FACTOR OF SAFETY (FOS) 2.0 DESIGN PILE SHALL BE 24" (2.0') IN DIAMETER X 20' DEPTH (OR TO ROCK PRIOR TO 20')

CALCULATE END BEARING END BEARING CAPACITY = PILE CROSS SECTIONAL AREA X SOIL BEARING CAPACITY AT DEPTH OF PILE TIP END BEARING CAPACITY = $(PI(2/2)^2)(2000 + 20(100)) = 12566 LBS$

SKIN FRICTION BEARING CAPACITY

THE AVERAGE SKIN FRICTION CAPACITY = $\frac{1}{2}$ THE DEPTH X LATERAL BEARING PRESSURE AT THAT DEPTH X THE SURFACE AREA OF THE PILE X FRICTION COEFFICIENT ALL DIVIDED BY THE SKIN FRICTION FOS (WITH A MAXIMUM OF 15 TIMES THE LATERAL PRESSURE VALUE, SEE ABOVE, THEREFORE USE A DEPTH OF $\frac{15}{2}$ OR 7.5')

SKIN FRICTION BEARING CAPACITY = (7.5(150) X PI(2)(20) X .25)/2 = 17671 LBS

Q_{allow} = END BEARING + SKIN FRICTION - WEIGHT OF THE PILE WEIGHT OF PILE = PI(2/2)² X 20 X 150 (PCF) =9425 LBS Q_{allow} = 12566 + 17671 -9425 = 20812 LBS

MAXIMUM PILE SPACING = PILE CAPACITY / CODE SPECIFIED FOOTING CAPACITY (PLF) EXCEPT WHERE POINT LOADS FROM BEAMS AND HEADERS ARE IDENTIFIED.

MAXIMUM PILE SPACING = 20812 / 2333 = 8.92' (8'-11")

PILE CONCRETE SHALL BE A MINIMUM 3000 PSI DESIGN MIX REQUIRED AXIAL CAPACITY IS $P_u = Q_{allow} = 20812 LBS$. PER IBC 2018 1810.3.2.6, THE AXIAL CONCRETE COMPRESSIVE STRENGTH $P_n = .3(f_c)(A_a = .3(3000)(452 \text{ IN}^2) = 406800 \text{ LBS}. >> P_u$ BY IBC 2018 1810.3.9.2, NO AXIAL REINFORCEMENT IS REQUIRED SINCE NO BENDING MOMENT IS ASSUMED.

2018 International Building Code (IBC)

CHAPTER 18 SOILS AND FOUNDATIONS

TABLE 1806.2 PRESUMPTIVE LOAD-BEARING VALUES

CLASS OF MATERIALS	VERTICAL FOUNDATION PRESSURE (psf)	LATERAL BEARING PRESSURE (psf/ft below natural grade)	LATERAL SLID RESISTANCE Coefficient of frictiona
1. Crystalline bedrock	12,000	1,200	0.70
2. Sedimentary and foliated rock	4,000	400	0.35
3. Sandy gravel and gravel (GW and GP)	3,000	200	0.35
4. Sand, silty sand, clayey sand, silty gravel and clayey gravel (SW, SP, SM, SC, GM and GC)	2,000	150	0.25
5. Clay, sandy clay, silty clay, clayey silt, silt and sandy silt (CL, ML, MH and CH)	1,500	100	_

For SI: 1 pound per square foot = 0.0479kPa, 1 pound per square foot per foot = 0.157 kPa/m.

a.Coefficient to be multiplied by the dead load.

b.Cohesion value to be multiplied by the contact area, as limited by Section 1806.3.2.

2018 International Building Code (IBC)

CHAPTER 18 SOILS AND FOUNDATIONS

1806.3.3 Increase for depth.

The lateral bearing pressures specified in Table 1806.2 shall be permitted to be increased by the tabular value for each additional foot (305 mm) of depth to a value that is not greater than 15 times the tabular value.

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FOOTING PIER PLAN WITH PIER SIZING AND DESIGN CALCULATIONS

NOTE: ALL PIERS 24" DIAMETER X 20' DEEP MAXIMUM OR TO ROCK U.N.O. EMBED (2) #4 BARS 5' INTO TOP OF PIER AND BEND OVER 24" TO LAP ONTO FOOTING REINFORCEMENT.

IDING Cohesio (psf) ^b 130

> NOTE: THIS DRAWING TO BE WORKED WITH PLAN #4415LA SHEET 2 OF 5 OF THE ARCHITECTURAL SET.

USE SHEET 2 FOR REFERENCE TO PLACE PIERS. DIMENSIONS SHOWN ARE FOR VERIFICATION OF TRIBUTARY SPAN ONLY.

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Engineers seal is for <u>STRUCTURAL</u> design and review only.
0 PERMIT ISSUE 4/17/25
NO. REVISION / ISSUE DATE
BUILDER / ARCHITECT
TRUMARK HOMES
PROJECT NAME / ADDRESS
LAURA
LOT 152 WOODSIDE
2090 NW O'BRIAN RD LEE SUMMIT MO
PROJECT #: 25041
DATE: 4/17/2025
SCALE: 1/4" =1'-0"
SHEET: S-0.1