

### DRAWING INDEX

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A1.72G	GARAGE SECOND FLOOR PLAN
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<b>STRUCT</b> S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G	<b>'URAL</b> GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN
<b>STRUCT</b> S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G S1.75G	<b>TURAL</b> GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN
<b>STRUCT</b> S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G S1.75G S1.76G	<b>TURAL</b> GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN
<b>STRUCT</b> S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G S1.75G S1.76G S3.00	<b>TURAL</b> GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN TYPICAL FOUNDATION DETAILS
<b>STRUCT</b> S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G S1.75G S1.76G S3.00 S3.01	<b>TURAL</b> GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN GARAGE FIFTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN TYPICAL FOUNDATION DETAILS GARAGE FOUNDATION DETAILS
<b>STRUCT</b> S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G S1.75G S1.76G S3.00 S3.01 S3.02	<b>TURAL</b> GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN GARAGE FIFTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN TYPICAL FOUNDATION DETAILS GARAGE FOUNDATION DETAILS PILE & PODIUM FOUNDATION DETAILS
<b>STRUCT</b> S0.01 S0.05 S0.10 S1.71G S1.72G S1.73G S1.74G S1.75G S1.76G S3.00 S3.01 S3.02 S3.50	GENERAL NOTES CMU DETAILS CONCRETE SCHEDULE GARAGE FOUNDATION PLAN GARAGE SECOND FLOOR FRAMING PLAN GARAGE THIRD FLOOR FRAMING PLAN GARAGE FOURTH FLOOR FRAMING PLAN GARAGE FIFTH FLOOR FRAMING PLAN GARAGE SNOW LOADING PLAN TYPICAL FOUNDATION DETAILS GARAGE FOUNDATION DETAILS PILE & PODIUM FOUNDATION DETAILS PRECAST GARAGE FRAMING DETAILS

MECHA	NICAL
ME-000	COVER
ME-001	SITE P
МЕСНА	NICAL
MP1.71G	GARA
MP1.72G	GARA
MP1.73G	GARA
MP1.74G	GARA
MP1.75G	GARA
MP2.01	MECH
ELECTF	RICAL
E1.71G	GARA
E1.72G	GARAG

E1./ IG	GARA
E1.72G	GARA
E1.73G	GARA
E1.74G	GARA
E1.75G	GARA
E3.01	ELEC
E3.02	ELEC

/ER SHEET E PLAN

AGE FIRST FLOOR- PLUMBING AGE SECOND FLOOR- PLUMBING AGE THIRD FLOOR- PLUMBING AGE FOURTH FLOOR- PLUMBING AGE FIFTH FLOOR- PLUMBING HANICAL DETAILS/SCHEDULES

AGE FIRST FLOOR PLAN- ELECTRICAL RAGE SECOND FLOOR PLAN- ELECTRICAL RAGE THIRD FLOOR PLAN- ELECTRICAL RAGE FOURTH FLOOR PLAN- ELECTRICAL RAGE FIFTH FLOOR PLAN- ELECTRICAL ECTRICAL DETAILS/ SCHEDULES ECTRICAL SCHEDULES

### **PROJECT** TEAM

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LANDSCAPE TBD

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**MECHANICAL** LS&A, P.A. 8625 College Blvd., Suite 102 Overland Park, Kansas 66210 PH. 785.233.0647 F. 785.233.0647

**PLUMBING** LS&A, P.A. 8625 College Blvd., Suite 102 Overland Park, Kansas 66210 PH. 785.233.0647 F. 785.233.0647

ELECTRICAL LS&A, P.A. 8625 College Blvd., Suite 102 Overland Park, Kansas 66210 PH. 785.233.0647 F. 785.233.0647

FIRE PROTECTION TBD



CONSTRUCTION As Noted on Plans Review Lee's Summit, Misse 11/25/2024







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PERM						
		S AND SPACES				EXISTING CONSTRUCTION T
SIGNS W		IANENT ROOMS AND SPA	CES SHALL COMPLY WITH			NEW CONSTRUCTION
	GENCY	4.30.5, 4.30.6			• XX	WALL TYPE DESIGNATION - REFERENCE FLOOR PLAN(S
ALARN SOUND I OPE	A WILL IF DOOR NED	EXIT STAIR			X	ROOM NAME AND NUMBER - REFERENCE FLOOR PLAN(S REFERENCE FINISH SCHEDU
TYP	PE 'A' TYPE 'B'	TYPE 'C'			XX	DOOR AND FRAME DESIGNA REFERENCE FLOOR PLAN(S REFERENCE DOOR AND FRA
	RESERVE	<u>ה</u>				I SYMBOL
œ.	PARKING				X.	REFERENCE ELE REFERENCE SHE
~		PARKING ST REQUIREME REQUIREME JURISDICTIC	ALL SIGNAGE NTS, INCLUDING FINE NTS, WITH LOCAL N		SECTION S	YMBOL
	\$250 Fine				X.	REFERENCE SHE
INTE	RIOR SIG	SNAGE SC	HEDULE GA	RAGE		DETAIL / ENLARGED PLAN SYN
<b>DOOR N</b> 03.A	O. ROOM NAM	NE TYPE 'B'	SIGN TYPE		X	
05.A 06.A 06.B	STAIR STAIR STAIR	TYPE 'A' TYPE 'C' TYPE 'C'				
06.A 06.A 06.A	STAIR STAIR STAIR	TYPE 'C' TYPE 'C' TYPE 'C'			INTERIOR E	ELEVATION SYMBOL
1. <u>TA</u>	CTILE & BRAILE CHARAC	CTERS BE RAISED MINIMUM 1/2	2"		• X/X•	REFERENCE ELE REFERENCE SHE
b. 2. <u>TY</u> I	CHARATERS SHALL E	BE ACCOMPANIED BY GR	ADE 2 BRAILLE		FINISH DES	IGNATION SYMBOL
a. b. 3. pir	CHARACTERS SHALL CHARACTERS SHALL	BE UPPER CASE & SANS BE A MINIMUM OF 5/8" H	SERIF OR SERIF TYPESTYLE IGH AND MAXIMUM 2" HIGH		XX-1 XX-1	<ul> <li>WALL FINISH DESIGNATION</li> <li>BASE FINISH DESIGNATION</li> <li>LIMITS OF WALL AND BASE F</li> </ul>
з. <u>гю</u> а. b.	PICTOGRAMS SHALL PLACED DIRECTLY B THE BORDER DIMEN	BE ACCOMPANIED BY THE PICTORGRAM	HE EQUIVALENT VERBAL DESCR AS INDICATED. AM SHALL BE 6" MIN. IN HEIGHT	IPTION		REFERENCE FLOOR PLAN(S REFERENCE FINISH SCHEDU
4. <u>MA</u> a.	TERIAL AND FINISH CHARACTERS AND B GLARE FINISH AS RE	ACKGROUND SHALL BE	EGGSHELL, MATTE OR OTHER N GN MANUFACTURER	ON-	(XX-#)	FLOOR FINISH DESIGNATION REFERENCE FLOOR PLAN(S REFERENCE FINISH SCHEDI
b. c.	BACKGROUND SHALL "GRIZZLE GREY" CHARACTERS AND S	L CONSIST OF 1/4" ACRY	LIC, COLOR TO MATCH SW 7068		×	REVISION NOTE
5. <u>MC</u> a. h	DUNTING LOCATION AND MOUNT AT 60" ABOVI MOUNT ON WALL AD	<u>) HEIGHT</u> E FINISH FLOOR TO THE JACENT TO THE LATOH S	CENTER OF SIGN		XX	CONSTRUCTION NOTE
р. С.	IF NO WALL SPACE E LEAF DOORS, MOUN	XISTS ON THE LATCH SI T ON THE NEAREST ADJ	DE OF THE DOOR, INCLUDING DO	DUBLE	<b>(#</b> )	DEMOLITION NOTE
б. <u>FIR</u> а.	KE SPRINKLER ROOM INCLUDE 4" HIGH VIN ROOM" APPLIED TO E	IYL WHITE LETTERS W/ M EXTERIOR SIDE OF DOOF	IN. 0.5" STROKE READING "SPRI 8, AS REQUIERD BY LOCAL FIRE	NKLER DEPT.	CJ——	GYPSUM BOARD CONTROL REFERENCE FLOOR PLAN(S REFERENCE DETAIL <b>1,2/A7.0</b>
DIRE					CMJ ———	CONCRETE MASONRY CONT REFERENCE FLOOR PLAN(S REFERENCE DETAIL 2/44 (2)
FUNCTIC	NAL SPACES OF THE BUS: 4.30.1, 4.30.2, 3.30.3, 4	UILDING SHALL COMPLY 4.30.5	WITH ADAAG			WALL MOUNTED FIRE EXTIN
		HE TWO-WAY COMMUNIC	E SIGNAGE	FOR	FE	COMPANY, WWW.LARSENMI BRACKET, REFERENCE FLO CENTERLINE OF EXTINGUISI
DIRECTIC SUMMON IDENTIFIC COMMUN MOUNTIN	ONS FOR THE USE OF TI NING ASSISTANCE CIA T CATION OF THE LOCATION NICATION SYSTEM. EACI NG LOCATION OF SIGNA	HE TWO-WAY COMMUNIC HE TWO-WAY COMMUNIC ON SHALL BE POSTED AI H SIGN SHALL COMPLY V GE AND DEVICE SHALL E	CATION SYSTEM, INSTRUCTIONS CATION SYSTEM AND WRITTEN DJACENT TO EACH TWO-WAY /ITH ICC A117.1 FOR VISUAL CHA E PER DRAWING BELOW.	NRACTERS.	FEC	SEMI-RECESSED FIRE EXTIN COMPANY, WWW.LARSENM ARCHITECTURAL SERIES, M (2 ½" PROTRUSION FROM W RECESSED HANDLE, ENGRA "FIRE EXTINGUISHER" ON DO
						FIRE EXTINGUISHER AND MA BRACKET. MOUNT SO CENT

IBOLS LEGEND		GENERAL NOTES			
TO REMAIN	1.	ALL CONSTRUCTION SHALL CONFORM TO THE MINIMUM STANDARDS OF THE APPLICABLE CODE INDICATED IN THE BUILDING SUMMARY COLUMN AND ALL LOCAL CODES PRESENTLY IN FEEECT UNLESS MORE STRINGENT REQUIREMENTS ARE INDICATED	THIS DISC DISCLAIM PROJECT		
TO BE DEMOLISHED	2.	ALL NEW CONSTRUCTION SHALL COMPLY W/THE AMERICANS WITH DISABILITIES ACT ACCESSIBILITY GUIDELINES (ADAAG) AND CHAPTER 11 OF THE INTERNATIONAL BUILDING CODE (INCLUDES ICC A117 1 PER IBC)	18017,1905 THE UNDE		
S) FOR LOCATIONS.	3.	THE GENERAL CONTRACTOR AND SUBCONTRACTORS SHALL OBTAIN AND PAY FOR ALL REQUIRED PERMITS, LICENSES, AND ALL UTILITY CHARGES, AND ARRANGE FOR ALL REQUIRED INSPECTIONS	PREPARA		
- S) FOR LOCATIONS. DULE FOR FINISHES.	4.	THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING BUILDING & SITE UTILITIES BETWEEN CIVIL & MEP DRAWINGS. THE CONTRACTOR SHALL ALSO CONTACT ALL APPLICABLE LITHUTY COMPANIES & PROVIDE CONDUIT & OTHER FACILITIES AS REQUIRED	A0.010 A0.03		
ATION -	5.	THE GENERAL CONTRACTOR AND ALL SUBCONTRACTORS SHALL VERIFY ALL DIMENSIONS & CONDITIONS ON THE JOB SITE PRIOR TO THE BIDDING OF THE CONTRACT DOCUMENTS.	A1.700 A1.710 A1.720		
S) FOR LOCATIONS. RAME SCHEDULE FOR REQUIREMENTS.		THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES.	A1.720 A1.730 A1.740		
EVATION		CONTRACTOR SHALL, IN WRITING, CALL TO THE ATTENTION OF THE ARCHITECT ANY DISCREPANCIES BETWEEN SPECIFICATIONS, PLANS, DETAILS OR SCHEDULES. THE ARCHITECT WILL THEN INFORM THE CONTRACTOR, IN WRITING, WHICH DOCUMENT TAKES PRECEDENCE. THERE SHALL BE NO ADJUSTMENT TO THE COST OR TIME OF THE WORK RESULTING FROM CLARIFICATION OF SUCH DISCREPANCIES.	A1.750 A1.760 A1.770 A1.780 A1.780 A1.790		
IEET	6.	DIMENSIONS ON DRAWINGS ARE SHOWN TO FINISHED FACE OF WALLS AND PARTITIONS OF EXISTING OR NEW CONSTRUCTION UNLESS OTHERWISE NOTED. CEILING HEIGHT DIMENSIONS AND ALL OTHER VERTICAL DIMENSIONS ARE TO THE FINISHED FLOOR SURFACE UNLESS OTHERWISE NOTED.	A8.010		
ECTION IEET	7.	CONTRACTOR TO FOLLOW ALL PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS FOR ALL BULIDING PRODUCTS. IN THE EVENT OF A CONFLICT BETWEEN INFORMATION SHOWN ON THE CONTRACT DOCUMENTS AND PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS, PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS SHALL GOVERN. CONTRACTOR SHALL NOTIFY ARCHITECT OF AN CONFLICTS BETWEEN PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS AND THE CONTRACT DOCUMENTS PRIOR TO INSTALLATION.	THE U ALL O ESTIM PART GEOT		
<u>(MBOL</u> ETAIL IEET	8.	THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING SHOP DRAWINGS, PRODUCT DATA, OR SAMPLES FOR CASEWORK, FINISHES, DOORS, FRAMES, HARDWARE, MECHANICAL, ELECTRICAL, AND PLUMBING FIXTURES, AND OTHER ITEMS REQUIRING ARCHITECT'S REVIEW FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS, AND FOR ALL ITEMS WHICH ALLOWED CONTRACTOR OPTIONS. PRIOR TO FORWARDING TO THE ARCHITECT FOR REVIEW. THESE SUBMITTALS MUST BE REVIEWED BY THE CONTRACTOR FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATIONS OF CONSTRUCTION AND SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO, ALL OF WHICH ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL AFFIX A STAMP TO SUBMITTAL INDICATING HIS REVIEW. SUBMITTALS FORWARDED WITHOUT A STAMP WILL BE RETURNED. ALL SUBMITTALS MUST BE REVIEWED BY THE ARCHITECT PRIOR TO CONSTRUCTION.	THIS MARCH		
EVATION	9.	CONTRACTOR SHALL GUARANTEE ALL WORK AGAINST FAULT OF ANY MATERIAL OR WORKMANSHIP FOR A PERIOD OF NOT LESS THAN ONE YEAR AFTER COMPLETION OR ACCEPTANCE. FAULTY WORK SHALL BE REPLACED OR REPAIRED AS REQUIRED AT NO COST TO THE OWNER.	ARCH		
	10.	CONTRACTOR SHALL COORDINATE WITH OWNER ALL ITEMS TO BE SALVAGED PRIOR TO SUBMISSION OF BIDS AND START OF CONSTRUCTION. OWNER SHALL HAVE SALVAGE RIGHTS TO RETAIN ALL REMOVED ITEMS.			
FINISHES	11.	ALL CHANGES PROPOSED DURING CONSTRUCTION WHICH RESULT IN A CHANGE TO THE CONTRACT TIME AND/OR SUM SHALL BE SUBMITTED TO THE ARCHITECT IN WRITING AND APPROVED BY THE ARCHITECT AND OWNER BEFORE SUCH WORK SHALL COMMENCE.			
S) FOR LOCATIONS. DULE FOR DESCRIPTIONS.	12.	CONTRACTOR SHALL COORDINATE CLEAR OPENINGS FOR ALL APPLIANCES PRIOR TO CONSTRUCTION OF CASEWORK.			
N - S) FOR LOCATIONS. DULE FOR DESCRIPTIONS.	13.	CONTRACTOR SHALL FURNISH AND INSTALL CONCEALED FIRE-RETARDANT TREATED WOOD BLOCKING BEHIND ALL CABINETS, TOILET ACCESSORIES, PLUMBING FIXTURES, AND OTHER WALL MOUNTED ITEMS AS REQUIRED FOR ADEQUATE SUPPORT.			
	14.	CONTRACTOR SHALL COORDINATE ALL LOCK AND LATCH SETS AND FINAL KEYING WITH OWNER. DOUBLE KEYED LOCKS ARE NOT PERMITTED ON ANY REQUIRED OR MARKED EXIT. MATCH EXISTING KEYING SYSTEM IF ONE IS EXISTING.			
	15.	ALL DOOR HARDWARE ON EXIT DOORS SHALL BE READILY OPERABLE FROM THE EGRESS SIDE WITHOUT THE USE OF A KEY, SPECIAL KNOWLEDGE, OR EFFORT.			
JOINT -	16.	CONTRACTOR SHALL PREPARE ALL NEW AND EXISTING SURFACES SCHEDULED TO RECEIVE NEW FINISHES IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR THE SUBSTRATE & FINISH BEING APPLIED.			
S) FOR LOCATIONS. 01 FOR CONSTRUCTION REQUIREMENTS.	17.	CONTRACTOR SHALL COORDINATE FINAL QUANTITY AND LOCATIONS OF FIRE EXTINGUISHERS WITH THE FIRE DEPARTMENT AND/OR BUILDING DEPARTMENT. SEE SYMBOLS LEGEND FOR TYPE OF EXTINGUISHER.			
ITROL JOINT - S) FOR LOCATIONS. DE FOR CONSTRUCTION REQUIREMENTS.	18.	ALL CONSTRUCTION MATERIALS EXPOSED WITHIN PLENUMS SHALL BE NON-COMBUSTIBLE OR SHALL HAVE A MAXIMUM FLAME SPREAD RATING OF 25 AND MAXIMUM SMOKE DEVELOPED RATING OF 50.			
NGUISHER BY LARSEN'S MANUFACTURING /FG.COM, MODEL MP10 W/B2 MOUNTING DOR PLAN(S) FOR LOCATIONS MOUNT SO	19.	ALL PIPING, LOW VOLTAGE WIRE AND CABLE, OPTICAL FIBER, PNEUMATIC TUBING, AND ALL DUCT AND DUCT COVERINGS, LININGS AND CONNECTORS INSTALLED WITHIN PLENUMS MUST BE RATED FOR PLENUM USE.			
SHER IS 46" A.F.F.	20.	TENANT SHALL BE RESPONSIBLE FOR COORDINATION AND INSTALLATION OF VOICE AND DATA CABLING AND EQUIPMENT.			
AFG.COM OR APPROVED EQUAL: ADDEL # AL-2409-6R. ALUMINUM, SEMI-RECESSED VALL WITH ROLLED EDGES), SOLID DOOR WITH	21.	CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE AUTOMATIC SPRINKLER SYSTEM. THE DESIGN SHALL BE PER NFPA REQUIREMENTS.			
AVED VERTICAL LETTERS WITH NO BACKFILL DOOR. CABINET TO BE PROVIDED WITH MP10 IANUFACTURER'S STANDARD MOUNTING TERLINE OF CABINET HANDLE IS 46" A F F	22.	ALL NEW GLASS AND GLAZING LOCATED IN HAZARDOUS LOCATIONS AS DEFINED IN IBC SECTION 2406.3 SHALL MEET THE REQUIREMENTS FOR SAFETY GLAZING AS DEFINED IN IBC SECTION 2406.			
	23.	IF THE CONTRACTOR FAILS TO SUBMIT A MATERIAL FOR APPROVAL, THE MATERIAL MAY BE REQUIRED TO BE REMOVED BY THE CONTRACTOR EITHER BY DIRECTION OF THE OWNER OR ARCHITECT.			
	24.	ALL HIGH-PILED STORAGE SHALL COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE APPLICABLE EDITION OF THE INTERNATIONAL FIRE CODE.			
	25.	THE CONTRACTOR IS TO PROVIDE AS BUILT DRAWINGS IN HARD COPY & AN ELECTRONIC AUTOCAD FILE TO THE OWNER AT THE CONCLUSION OF THE PROJECT.			
	26.	INSTALL ELASTOMERIC JOINT SEALER AROUND ALL PIPES, DUCTWORK, & STRUCTURE PASSING THRU INTERIOR NON-RATED CONCRETE AND MASONRY WALLS, GYPSUM BOARD PARTITIONS, AND CONCRETE FLOOR/ROOF SLABS. FOR FIRE RATED INTERIOR CONCRETE AND MASONRY WALLS, GYPSUM BOARD PARTITIONS, AND CONCRETE FLOOR/ROOF SLABS SEAL ALL PIPES, DUCTWORK, AND STRUCTURE. INSTALL FIRESTOP MATERIALS IN ALL GAPS PRIOR TO SEALANT APPLICATION. INSTALL SEALER ACCORDING TO MANUFACTURER'S WRITTEN INSTRUCTIONS.			
	27.	CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTION OF ALL EXISTING CONSTRUCTION INDICATED TO REMAIN AND SHALL REPAIR AND/OR REPLACE ALL AREAS AND /OR MATERIAL DAMAGED DURING CONSTRUCTION AT A MINIMUM TO THE CONDITION WHICH EXISTED PRIOR TO CONSTRUCTION.			
	28.	CONTRACTOR SHALL BE RESPONSIBLE FOR PRICING RADIO COVERAGE AMPLIFIER FOR EMERGENCY RESPONDERS AS AN ALTERNATE. PRIOR TO CONSTRUCTION COMPLETION, AMPLIFIER SHALL BE PROVIDED ONLY IF REQUIRED BY AHJ .			

# FESSIONAL SERVICES DISCLAIMER

SCLAMER SERVES NOTICE OF ACCEPTANCE OF RESPONSIBILITY AND MER OF RESPONSIBILITY AS TO THE CONTRACT DOCUMENTS PREPARED FOR NUMBER:

050.07,19050.08, PARAGON STAR NORTH VILLAGE BY FINKLE + WILLIAMS, INC. DERSIGNED ARCHITECT, AND FINKLE + WILLIAMS, INC., ARE RESPONSIBLE FOR RATION OF ONLY THE NOTED CONSTRUCTION DRAWINGS BELOW:

TITLE	DATE
LEGENDS & GEN. NOTES	10.18.24
CODE	10.18.24
GARAGE FIRST FLOOR PLAN AT GRADE	10.18.24
GARAGE FIRST FLOOR PLAN	10.18.24
GARAGE SECOND FLOOR PLAN	10.18.24
GARAGE THIRD FLOOR PLAN	10.18.24
GARAGE FOURTH FLOOR PLAN	10.18.24
GARAGE FIFTH FLOOR PLAN	10.18.24
GARAGE SECTIONS	10.18.24
GARAGE STAIRS	10.18.24
GARAGE STAIRS AND SECTIONS	10.18.24
GARAGE DETAILS	10.18.24
DOOR SCHEDULE AND DETAILS	10.18.24

UNDERSIGNED ARCHITECT AND FINKLE + WILLIAMS DISCLAIM RESPONSIBILITY FOR OTHER CONSTRUCTION DOCUMENTS, AND ANY OTHER SPECIFICATIONS, REPORTS, MATES, SHOP DRAWINGS, ETC. RELATING TO OR INTENDED TO BE USED FOR ANY RT OF THE ARCHITECTURAL OR ENGINEERING PROJECT, INCLUDING ANY TECHNICAL ENGINEERING SERVICES, OR ENVIRONMENTAL REPORTS.

S NOTICE IS EXECUTED BY THE UNDERSIGNED AND AUTHENTICATED BY THE CHITECTURAL SEAL OF THE PERSON PREPARING THS NOTICE.



HITECT: DAVID A. WILLIAMS

# **BUILDING SUMMARY**

GENERAL BUILDING INFORMATION

PROJECT NAME: PARAGON STAR NORTH VILLAGE ADDRESS: 3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

PROPOSED USE: MIXED USE MULTI-FAMILY RESIDENTIAL AND RETAIL

### APPLICABLE CODES

INTERNATIONAL BUILDING CODE (IBC)	2018 EDITION
INTERNATIONAL MECHANICAL CODE (IMC)	2018 EDITION
INTERNATIONAL PLUMBING CODE (IPC)	2018 EDITION
NATIONAL ELECTRIC CODE (NEC)	2017 EDITION
INTERNATIONAL FIRE CODE (IFC)	2018 EDITION
INTERNATIONAL FUEL GAS CODE (IFGC)	2018 EDITION

Retail Building	(SEPA	ARATE F
Parking Garage		
Apartment Building	А	(SEPAR
Apartment Building	В	(SEPAR
Apartment Building	С	(SEPAR
Apartment Building	D	<b>SEPAR</b>
Apartment Building	E	<b><i>(SEPAR</i></b> )

vertically and horizontally.

Apartment Building F (SEPARATE PERMIT) Parking Garage

RELEASED FOR

Construction Type: Allowable Height: Allowable Area: Max. Travel Distance:

Occupancy:







CONSTRUCTION As Noted on Plans Review





# RELEASED FOR CONSTRUCTION As Noted on Plans Review evelopment Services Departme Lee's Summit, Missouri 11/25/2024 F5 paragon star PARAGON STAR NORTH VILLAGE 3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081 Project No.: 18017,19050.17,19050.08 10.18.24 Date: Issued For: GARAGE PERMIT REVISIONS Descriptio Date \_\_\_\_\_

_	REGISTRATION
	DAVID ALAN WILLIAMS NUMBER A-5540

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PROJECT TEAM			
	ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE	
	CIVIL	GBA ENGINEERS	
	LANDSCAPE	LAND 3	
	STRUCTURAL	BOB D. CAMPBELL	
	PLUMBING	LATIMER SOMMERS	
	MECHANICAL	LATIMER SOMMERS	
	ELECTRICAL	LATIMER SOMMERS	
	FIRE PROTECTION	LATIMER SOMMERS	
	CONTRACTOR	BRINKMANN CONSTRUCTORS	

![](_page_3_Picture_7.jpeg)

![](_page_4_Figure_0.jpeg)

![](_page_5_Figure_0.jpeg)

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	RELEASED FOR CONSTRUCTION As Noted on Plans Review
D	evelopment Services Departme Lee's Summit, Missouri 11/25/2024
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PARAGO NORTH	ON STAR VILLAGE
3200 NW PA LEE'S SUMM	RAGON PKWY /IIT, MO 64081
Project No.: 18017 Date: 10.18.	,19050.17,19050.08 24
Issued For: GARA	GE PERMIT
No.         Date	ISIONS Description
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PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
	LATIMER SOMMERS BRINKMANN
CONTRACTOR	CONSTRUCTORS

![](_page_6_Picture_3.jpeg)

SHEET NUMBER

A1.73G

![](_page_7_Figure_0.jpeg)

![](_page_8_Figure_0.jpeg)

30://Paragon Star - Multifamily R21/Paragon Star\_Multifamily\_R21 (REVIT SERVER)\_ellenlfoster.

![](_page_9_Figure_0.jpeg)

![](_page_9_Figure_1.jpeg)

![](_page_10_Figure_0.jpeg)

60://Paragon Star - Multifamily R21/Paragon Star\_Multifamily\_R21 (REVIT SERVER)\_ellenlfoster

![](_page_11_Figure_0.jpeg)

![](_page_12_Figure_0.jpeg)

![](_page_12_Picture_5.jpeg)

![](_page_13_Figure_0.jpeg)

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				OR			FR/	AME			DETAILS		_	
	).  -	W.	H.	Th.	TYPE	MAT.	TYPE	MAT.	RATING	JAMB	HEAD	SILL	HARDWARE	RE
103	.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	2	НМ	-	1/A8.01G	2/A8.01G	-	EL.2	
105	.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	3/A8.01G	4/A8.01G	-	EL.1 1	(2)
105	.В	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	5/A8.01G SIM	6/A8.01G SIM	9/A8.01G	EL.4	(2)
106	.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	3/A8.01G	4/A8.01G	-	EL.4	(2)
106	.В	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	1/A8.07	2/A8.07	-	EL.1	(2)
200	.A	24' - 6"	7' - 0"	MFR.	O1	STL.	MFR.	MFR.	-	10/A8.01G	11/A8.01G	-	MFR.	(1)
201	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
201	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1 1	
202	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
202	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1 1	
203	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
203	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
204	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
204	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
206	.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	3/A8.01G	4/A8.01G	-	EL.2	(2)
206	.В	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	3/A8.01G	4/A8.01G	-	EL.4	(2)
301	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
301	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
302	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
302	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
303	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
303	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	1	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
304	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
304	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	1	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
306	.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	3/A8.01G	4/A8.01G	-	EL.2	(2)
401	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
401	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
402	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
402	.В	3' - 0"	7' - 0"	1 3/4"	H2	HM	3	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
403	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
403	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	1	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
404	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
404	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	1	MFR.	90 MIN.	5/A8.01G	6/A8.01G	9/A8.01G	EL.1	
405	.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	2	НМ	-	1/A8.01G	2/A8.01G	-	EL.2	
406	.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	3	НМ	90 MIN.	3/A8.01G	4/A8.01G	-	EL.2	(2)
501	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	6/A1.79G	12/A8.01G	AL.1	
501	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	1	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
502	.A	3' - 0"	7' - 0"	1 3/4"	A1	AL	1	MFR.	-	MFR.	4/A1.74G	12/A8.01G	AL.1	
502	.В	3' - 0"	7' - 0"	1 3/4"	H2	НМ	1	MFR.	90 MIN.	7/A8.01G	8/A8.01G	9/A8.01G	EL.1	
505	.A	3' - 0"	7' - 0"	1 3/4"	H1	НМ	2	НМ	-	1/A8.01G	2/A8.01G	9/A8.01G	EL.5	
500	-												-	1

### **GENERAL NOTES - STRUCTURAL** General Information A. The contractor shall verify dimensions and conditions before construction and notify the engineer of any discrepancies, inconsistencies, or difficulties affecting the work before proceeding. B. The contractor shall coordinate all disciplines, verifying size and location of all openings, whether shown on structural drawings or not, as called for on architectural, mechanical, or electrical drawings. In the case of work in an existing building the contractor shall scan existing structure to locate all rebar in the area of the new core/opening using ground penetrating radar and notify the engineer of record for review prior to coring/cutting. Conflicts, inconsistencies, or other difficulties affecting structural work shall be called to the architect or engineer's attention for direction before proceeding. All design and construction work for this project shall conform to the requirements of the following governing design codes: 1. International Building Code (IBC 2018) as amended by the city of Lee's Summit, Minimum Design Loads for Buildings and Other Structures (ASCE7-16) Specification for Structural Steel Buildings (AISC 360-16) Member Design Basis is Allowable Stress Design (ASD) Connection Design Basis is Allowable Stress Design (ASD) 4. Structural Welding Code (AWS D1.4-17) 5. Building Code Requirements for Structural Concrete (ACI 318-14) Building Code Requirements for Masonry Structures (TMS 402-16) . North American Specification for the Design of Cold-Formed Steel Structural Members (AISI S100-16) 8. National Design Specification (NDS) for Wood Constriction with 2018 Supplements (ANSI/AWC NDS-2018) 9. Special Design Provisions for Wind and Seismic (AWC SDPWS-2015) D. These drawings are for this specific project and no other use is authorized. 2. Structural Load Design Criteria A. Dead Load: = 35 psf Floor, Apartment = 15 psf Floor, Balony Floor, Corridor (Above Podium) = 25 psf Garage Collateral = 5 psf = 25 psf Stair, Metal Pan = 60 psf Stair, Wood = 25 psf B. Live Load: Floor, Apartment = 40 psf = 60 psf Floor, Balcony Floor, Corridor (Serving Apartment) = 40 psf Floor, Corridor )Serving Public) = 100 psf Floor, Public (Clubhouse) = 100 psf Floor, Storage = 125 psf = 40 psfGarage = 20 psf Roof, MEP Equipment Zone = 45 psf = 100 psf C. Snow: Pg = 20psf, Ce = 1.0 Pf = 14 psf (Apt) & 16.8 psf (Garage), Pm = 20 psf ls = 1.0, Cs = 1.0, Ct = 1.0 (Apt) & 1.2 (Garage) Drift & unbalanced snow loads per ASCE/SEI 7-16 D. Lateral Loads: 1.) Wind V(ult) = 109 mph, exposure C. lw=1.0 GCpi=+/-0.18 Design wind pressures to be used for the design of exterior component and cladding materials on the designated zones of wall and roof surfaces shall be per section 30.7 and Table 30.7-2 of ASCE/SEI 7-16. Tabulated pressures shall be multiplied by effective area reduction factors, exposure adjustment factors, and topographic factors where applicable 2.) Seismic: Ss = 0.099, S1 = 0.068, le=1.0, Site Classification D Seismic Design Category B Basic Seismic Force-resisting System: At Podium A.2 - Ordinary Reinforced Concrete Shear Walls R = 4, Omega = 4, Cd = 2 1/2, V = 0.053W At Apartments Above Podium: A.17 - Light-Framed Walls with Shear Panels of All Other Materials R = 2, Omega = 2 1/2, Cd = 2, V = 0.053W At Precast Garage: A.6 - Ordinary Precast Shear Walls (N/S Direction) R = 3, Omega = 2 1/2, Cd = 3, V = 0.035W B.9 - Ordinary Precast Shear Walls (E/W Direction) R = 4, Omega = 2 1/2, Cd = 4, V = 0.026W E. This project is designed to resist the most critical effects resulting from the load combinations of section 1605.3 of the International Building Code. 3. Concrete A. All concrete for foundations (walls, grade beams, footings and piers) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic vard of concrete regardless of strengths obtained, not over 6 gallons of water per 100 pounds of cement and not over 4 inches of slump. B. All concrete for interior flatwork (without floor covering) shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 525 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.75 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only). All concrete for interior flatwork (with floor covering) shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 540 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.40 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only). All concrete for exterior flatwork shall have a minimum design compressive strength of 4500 psi in 28 days, with not less than 560 pounds of cement per cubic yard of concrete, not over 5 gallons of water per 100 pounds of cement, with 6% +/- 1% air entrainment, and a maximum of 4 inches of slump. All concrete for columns shall develop a minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 560 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5 gallons of water per 100 pounds of cement and not over 4 inches of slump. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's dosage rates for improved workability. G. The preceding minimum mix requirements may have up to 15% maximum of the cement content replaced with an approved ASTM C618 Class C fly ash, provided the total minimum cementitious content is not reduced. H. Combined aggregate (coarse plus fine) for all concrete shall be well graded from coarsest to finest with no more than 18 percent and not less than 8 percent retained on an individual sieve, except that less than 8 percent may be retained on coarsest sieve and on No. 50 and finer sieves. Submit this gradation report with the concrete mix design shop drawings. All interior concrete slabs on grade shall be placed over 15 mil, Class A Vapor Barrier per ASTM E1745 with less than 0.01 perms, tested after mandatory conditioning. All joints shall be lapped and sealed per manufacturer's recommendations. All penetrations, as well as damaged vapor barrier material shall also be sealed per manufacturer's recommendation prior to concrete placement. Install barrier per manufacturer recommended details at all discontinuous edges (at interior columns, exterior edge of slab, etc.) to ensure terms of warranty are followed. The vapor barrier shall be placed over freedraining granular material as prescribed by the project soils report. Basement foundation walls shall be braced at the base and top of wall by the contractor until the slab on grade at the base and the floor framing/slab at the top of wall is complete and the concrete has achieved 75% of the design strength. The contractor is responsible for engineering and design of the wall bracing, if required. K. All concrete is reinforced concrete unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any details not shown shall be detailed per ACI 315 and meet requirements of ACI 318, current editions. Control joints in dirt formed slab to be as shown on plans. Where not shown, limit controlled areas to not more than 144 square feet, or 12 feet on any side. Slab panel side ratio shall not exceed 1 1/2 to 1. M. Contractor shall verify that all concrete inserts, reinforcing and embedded items are correctly located and rigidly secured prior to concrete placement. N. Construction joints in beams, slabs, and grade beams shall occur at midspan (middle third) unless noted otherwise. Provide 2 x 4 horizontal keys at construction joints for shear transfer. O. No aluminum items shall be embedded in any concrete. 4. Reinforcing Steel A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel. Welded plain wire fabric shall be supplied in sheets and conform to the requirements of ASTM A185. B. Clear minimum coverage of concrete over reinforcing steel shall be as follows: Concrete placed against earth: 3" Formed concrete against earth: 2" Slabs: 1-1/2" 4. Beams or Columns: Other All coverage shall be nominal bar diameter minimum. All dowels shall be the same size and spacing as adjoining main bars (splice lap 48 bar diameters or 24" minimum unless noted otherwise). . At corners of all walls, beams, and grade beams supply corner bars (minimum 2'-0" in each direction or 48 bar diameters) in outside face of wall, matching size and spacing of horizontal bars. Where there are no vertical bars in outside face of wall, supply 3 - #4 vertical support bars for corner bars. Bars marked continuous and all vertical steel shall be lapped 48 bar diameters (2'-0" minimum) at splices and embedments, unless shown otherwise. Splice top bars near midspan and splice bottom bars over supports, unless noted otherwise. At all holes in concrete walls and slabs, add 2 - #5 bars (opening dimension plus 96 diameters long) at each of four sides and add 2 - #5 x 5'-0" diagonally at each of four corners of hole. Openings in 8" thick walls are reinforced similar, but with 1 - #

5 instead of 2 - #5, respectively. G. Unless otherwise covered on architectural plans or specifications, vertical control joints in concrete wall shall be spaced at a maximum of 20'-0" on center and coordinated with the architect. Every other horizontal wall reinforcing bar shall be discontinuous at control joints except heavy top and bottom bars unless noted otherwise. Provide base seal waterstop style number 772 (by Greenstreak Inc. or approved equal) on dirt face side of wall at all walls below grade.

- H. Accessories shall be as specified in latest edition of the ACI Detailing Handbook and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet.
- I. All slabs and stairs not shown otherwise shall be 6" thick with #4 bars at 12" on center each way. All exterior porches and stoops not otherwise detailed may be constructed in any standard manner, solid or hollow, but must be reinforced with #4 bars at 12" on center each way minimum. Porches shall be doweled to adjacent walls or grade beams with #4 bars at 12" on center, hooked or embedded 48 diameters into both members. Slope porches 1/8" per foot for drainage unless noted otherwise.
- J. Allow 2 tons of reinforcing bars #4 or larger to be used as directed in the field for special conditions by the engineer of record (labor for placing same to be included).
- 5. Structural Steel
- A. All structural steel beams and columns shall be ASTM A992, grade 50 steel and all miscellaneous steel shall be ASTM A36 grade steel (except at moment connections where plates shall be ASTM A572, grade 50). Hollow Structural Sections (HSS) shall be ASTM A500, grade C. Fabrication and erection shall be in accordance with AISC 303-05 "Code of Standard Practice for Steel Buildings and Bridges" in the 13th Edition
- of the AISC Steel Construction Manual. B. All welding shall conform to the recommendations of the AWS. C. All exterior steel and connections, and brick relief angles shall be hot-dip galvanized. D. All bolts not otherwise specified shall be 3/4" diameter high strength (ASTM A325-N). All bolts shall be fully pretensioned. All beam connections shall be designed per the AISC Manual of Steel Construction "Framed Beam Connections" for the indicated reactions or at least 0.4 x beam total shear capacity, Vn/Omega, shown in the maximum total uniform load tables, whichever is greater; and, shall account for eccentricity when the bolt line is more than 2" from the center of the support. All connections must be two bolt minimum. Additional connection elements may not be specifically shown in the conceptual details in this set but may be required by the final connection design, such as stiffener plates, doubler plates, supplement/reinforcing plates or other connection material. Connection design and shop drawing preparation shall be completed under the direct supervision of a professional engineer licensed in the state the project is located and shop drawings and connection calculations shall
- bear his/her seal. E. All anchor bolts shall be 3/4" diameter, ASTM F1554, Grade 36 unless noted otherwise. Washers of minimum size and thickness for the given anchor diameter in Table 14-2 of the AISC Steel Construction Manual shall be provided at every column anchor bolt. Washers shall have a standard size hole for the anchor bolt. At braced frames washers shall be welded all around to the column base plate with 3/16" fillet weld.
- F. Allow 2.0 tons structural steel to be used as directed in field for special conditions by the engineer of record. Cost for shop drawings, fabrication, delivery, detailing, and erection to be included. 50% of structural steel allowance shall be bid as miscellaneous galvanized angle and plate.
- 6. Post Installed Anchors
- A. Post-installed anchors shall be used only where specified on the drawings unless approved in writing by the engineer of record. See drawings for anchor diameter, spacing and embedment. Performance values of the anchors shall be obtained for specified products using appropriate design procedures and/or standards as required by the governing building code. Anchors installed in concrete shall have an ICC-ES Evaluation Service Report. Special inspection is required for all post installed anchors. The contractor shall coordinate an on-site meeting with the post installed anchor manufacturer field representative to educate the construction team on the anchor
- installation guidelines and requirements B. Mechanical anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ACI 355.2 and ICC-ES AC193. All anchors
- shall be installed per the anchor manufacturer's written instructions. C. Adhesive anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions.
- D. Mechanical anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC01. All anchors shall be installed per the anchor manufacturer's written instructions. E. Adhesive anchors used in solid grouted masonry shall have been tested and qualified
- for use in accordance with ICC-ES AC58. All anchors shall be installed per the anchor manufacturer's written instructions. F. Anchors used in hollow concrete masonry shall have been tested and qualified in accordance with ICC-ES AC106 or ICC-ES AC58 as appropriate. All anchors shall be installed per the anchor manufacturer's written instructions with appropriate screen tubes used for adhesives
- 7. Foundations
- A. The soil investigation was prepared by Terracon, the report number is 02215169 and the telephone number is 913-492-777 B. Structural foundations consist of a network of stright shaft auger pressure grouted piles established on moderatly weathered shale capable of safely supporting 40ksf end
- bearing. Each pile shall penetrate 5'-0" minimum into the moderately weathered shale. Spread footing and shallow foundations for ancillary structures are designed to bear on engineered fill or undisturbed soil capable of safely supporting 1,500 psf. Retaining walls are designed for an active lateral load of 50 pcf equivalent fluid pressure. D. Basement walls are designed for an at rest lateral load of 70 pcf equivalent fluid
- pressure. See General Note 3.J for wall bracing requirements. E. Contractor shall provide for dewatering at excavations from either surface water or seepage
- F. All foundation excavations shall be inspected by a qualified soil engineer, approved by the architect and/or structural engineer, prior to placement of steel or concrete. This inspection shall be at the owner's expense. G. All concrete in the structural portion retaining the backfill shall have attained its design
- strength prior to being backfilled. H. Moisture content in soils beneath building locations should not be allowed to change after footing excavations and after grading for slabs on grade are completed. If subgrade materials become desiccated or softened by water or other conditions, recompact materials to the density and water content specified for engineered fill. Do not place concrete on frozen ground.

### 8. Drilled Auger Pressure-Grouted Piers

- A. Piers not otherwise indicated shall be 30" diameter. B. All piers shall have (4) #7x6'-0" hooked dowels unless otherwise indicated.
- C. Pier dowels shall extend 40 diameters above top of pier. Driving dowels into concrete after initial set is not allowed.
- D. Refer to the specifications (sections for excavation and concrete) for other detailed requirements E. Pier concrete to have 6" slump.
- 9. Concrete Masonry Units
- A. Concrete block used in exterior walls or load bearing walls shall meet the requirements of ASTM C90 and have a minimum net compressive strength of 2650 psi and laid up using type N mortar such that f'm equals 2000 psi. Mortar shall be volume proportion based cement lime mortar. Proportioning shall be completed by box measure. Any
- block in contact with earth shall be normal weight units, laid using type "S" mortar and arouted solid. B. The contractor shall provide adequate temporary bracing for all masonry walls during
- construction. C. All concrete block shall have 9 gage (or larger) horizontal joint reinforcing (ladder or truss) per architectural drawings and specifications (16" maximum vertical spacing).
- D. Cavity wall construction shall be reinforced as designed for specific concrete block used. The horizontal joint reinforcing shall be of the ladder or truss style per
- specification and continuous between brick and block, as prescribed by the architectural drawings. E. Concrete block shall be reinforced as follows in 6", 8", 10", and 12" walls: . Vertical reinforcing shall be a minimum of 1 - #4 bar in 6" and 8" walls and 2 - #4 bars in 10" and 12" walls at 4'-0" on center, at each corner, at each door and window jamb, each side of control joints and in the end void of each length of wall. Lap splices for masonry vertical reinforcing shall be 48 bar diameters or 24"
- minimum. 2. Horizontal reinforcing:
- A. Horizontal joint reinforcing as noted above. B. Continuous horizontal bars shall be included per section or detail in bond beam or optional running bond beam where noted. Where bond beams are continuous at corners of walls, supply corner bars matching size of horizontal bars (minimum 2'-0" or 40 bar diameters in each direction).
- F. Grout, where noted above, shall have a minimum design ultimate compressive strength of 2500 psi at 28 day test and 3/8" maximum aggregate size.
- G. Non-load bearing concrete block walls shall be isolated from adjacent structural elements with vertical 3/8" control joints and at the top of the wall with 1" air space or compressible material and support per architectural detail. H. Unless otherwise covered on architectural plans or specifications, vertical control joints
- in masonry construction shall be 3/8" wide, full height of wall. Joints shall be spaced at a maximum of 24'-0" on center and coordinated with the architect. All horizontal joint reinforcing shall be discontinuous at control joints in masonry. All bond beam horizontal
- reinforcing shall be continuous through control joints. I. Lintels over all openings up to 8'-0" wide in new and existing masonry walls not otherwise covered shall be one 6x3 1/2x5/16 angle for each 4" width of masonry. All exterior lintels to be galvanized.
- J. Walls shall be anchored top and bottom by dowels matching wall vertical reinforcing(unless noted otherwise) from floor slab bottom and bracing angles at the top, per details on the drawings.
- 10. Light Gage Metal Structural Framing
- A. All load bearing, light gage structural studs, track, and bridging shall be of the type, size, gage, and spacing as shown on the plans, minimum.
- B. All materials shall be 33,000 psi minimum yield, except studs of 16 gage or heavier shall have a minimum yield of 50,000 psi.
- C. All properties, fabrication, and erection shall be in accordance with latest editions of D. All framing components shall be cut squarely or at an angle to fit squarely
- the AISI "Specifications for the Design of Cold-Formed Structural Members." against abutting members. Splicing of axially loaded members is not permitted. Members shall be held firmly in place until properly fastened. Attachments of similar components shall be by welding, screw attachment, or bolting. Wire
- tying of components is not permitted. E. Tracks shall be securely anchored to floor and overhead members. Special anchorage requirements required for wind bracing shall be as shown on the plans. F. Prior to fabrication and/or erection, the contractor shall submit shop drawings complete with detail of erection, fabrication, attachments, anchorages, lintels, etc., for review by the architect/engineer.

### 11. Timber and Wood Framing

- A. Quality and construction of wood framing members and their fasteners for load supporting purposes not otherwise indicated on the drawings shall be in accordance with the 2018
- International Building Code. B. All studs and top and bottom plates shall be Douglas Fir No. 2 grade visually graded lumber, with an allowable fiber stress in bending of 900 psi minimum and an elastic modulus of 1,600,000psi unless noted otherwise. All joist, truss members, and headers to be No. 2
- grade (min.) unless noted otherwise. All lumber for exterior decks and balconies shall be treated Southern Yellow Pine No. 2 grade. C. Bridging of stud bearing walls and shear walls shall be solid, matching sheathing joints. D. Joist blocking and bridging shall be solid wood or cross bridging of either wood or metal
- straps. Spacing, in any case, shall not exceed 8'-0". E. Wood members and sheathing shall be fastened with number and size of fasteners not less than that set forth in Table 2304.9.1 of the 2018 International Building Code. Floor sheathing shall be APA rated tongue and groove Sturd-I-Floor, exposure 1, glued and nailed with 10d nails or # 10 screws at 12" on center field. Sheathing of shear walls or roof diaphragms shall be edge nailed with 8d common nails at 6" on center and nailed to intermediate framing and/or blocking members with 8d common nails at 12" on center unless otherwise noted on the drawings.
- F. Sill plates shall be bolted to concrete slabs with 1/2" diameter bolts at 32" on center (UNO, re: shearwall schedule.) Provide plate washers at sill plate anchors for shearwalls per shearwall schedule. Plates in direct contact with concrete or masonry shall be preservative-
- G. All hangers, ties and connections shown are based on Simpson Strong Tie as the basis of design, provide Simpson Strong Tie or an approved equal. Joist hangers shall be equal to "LUS" for wood application and "LB" for steel weld-on application. Roof truss ties shall be equal to "H2.5A" and tie the roof truss to the top plate (provide (2) "H2.5A" diagonally across from each other when uplift load shown in truss shop submittal exceeds 600lbs). Roof girder ties shall be equal to a "LGT2", "LTG3" or "LGT4" tie (dependent on number of plies) and tie the truss girder to the top plate. Provide "H2.5A" at the top of each stud to top track when the top track has roof truss attached.
- H. Service condition dry with moisture content at or below 19% in service. I. Laminated strand lumber (LSL) shall have an allowable flexural stress (Fb) of 1,700 psi (reduced by size factor) and an elastic modulus (E) of 1,300,000 psi.
- J. Laminated veneer lumber (LVL) shall have an allowable flexural stress (Fb) of 2.600 psi (reduced by size factor) and an elastic modulus (E) of 2.000.000 psi.
- K. Parallel Strand Lumber (PSL) shall have an allowable flexural stress (Fb) of 2,900 psi (reduced by size factor) and an elastic modulus (E) of 2,000,000 psi. ((E) = 2,200,000 psi for members > 18"). L. Glulams shall be 24F-V8 or better with an allowable flexural stress (Fb) of 2,400 psi and an
- elastic modulus (E) of 1,800,000 psi. Exterior glulams shall be moisture-resistant treated. M. Pre-engineered wood trusses shall be designed in accordance with the Truss Plate Institute's national design standard for metal-plate connected wood truss construction (ANSI/TPI-1 latest edition). Trusses shall be designed and manufactured by an authorized member of the Wood Truss Council of America (WTCA). Truss design shall conform to specified codes, allowable stress increases, deflection limitations and other applicable
- criteria of the governing code. N. Truss shop drawings showing complete erection and fabrication details and calculations (including connections) shall be submitted to the project architect/engineer for review prior to fabrication and/or erection. Calculations and layout plan shall bear the seal of a professional engineer, registered in the state of the project location. Layout plan shall incldue truss locations, spacing and all hanger designations used to support trusses to beams or other trusses. Calculations shall indicate max reactions in all directions, number of plies for the truss and dead, live and total load deflections along with a list or key of all standard and nonstandard utilized load combination. Shop drawings shall also be submitted to the local
- government controlling agency when requested by that agency. O. All trusses shall be securely braced both during erection and permanently, as indicated on the approved truss design drawings and in accordance with TPI's commentary and recommendations for handling, installing and bracing metal-plate connected wood trusses (HIB-91, booklet) and the latest edition of ANSI/TPI-1.
- P. The truss manufacturer shall supply all hardware and fasteners for joining truss members together and fastening truss members to their supports. Metal connector plates shall be manufactured by a member of the Wood Truss Council of America (WTCA) and shall be 20 gauge minimum. Connector plates shall meet or exceed ASTM A653, grade 33, with ASTM A924 galvanized coating designation G60. Q. Provide truss space directly above and centered over HVAC closets. Refer to Architectural
- and MEP drawings for exact locations. R. Shipment, handling, and erection of trusses shall be by experienced, qualified persons and shall be performed in a manner so as not to endanger life or property. Apparent truss damage shall be reported to the truss manufacturer for evaluation prior to erection. Cutting or alteration of trusses is not permitted.
- S. Pre-Engineered Floor Trusses: Top Chord Dead Load = 30 psf Top Chord Live Load = Per General Note 5B Bottom Chord Dead Load = 10 psf Live Load Deflection = L/480; (1/2" max) Total Load Deflection = L/360
- Roof Truss Design Criteria: Top Chord Dead Load Top Chord Live Load Top Chord Snow Load Bottom Chord Dead Load Bottom Chord Live Load
- Live Load Deflection = L/360 Total Load Deflection = L/300 U. Roof trusses shall be designed per IBC 2018 for net uplift resulting from wind loading as calculated using components and cladding loading. Top and bottom chord dead load used in
- V. Construction bracing shall be provided by the contractor as required to keep the building and studs plumb. W. Structural members shall not be cut for pipes, etc., unless specifically detailed. Nothing and boring of studs and top of plates shall conform to the provisions of section 2308.9.10 and 2308.9.11 of the IBC. Where top plates or sole plates are cut for pipes, a metal tension tie
- with minimum 0.058 inches thick and 1 1/2" inches wide shall be fastened to each plate across and to each side of the opening with not less than (6) 16d nails, in accordance with section 2308.9.8 of the IBC. X. All fasteners for wood to wood connections and wood connectors shall be as indicated in
- structural drawings or manufacturer literature to achieve full capacity of connector. Alternate fasteners may be submitted as a substitution request. Submittal must show that alternative fasteners will not reduce the capacity of the connection.

### 12. Precast Concrete Members

- A. The contractor/supplier is responsible for the design of all the precast members and connection between them and other structural members. Submit design calculations, sealed by an engineer licensed in the state of the project location, for review by the architect/engineer of record.
- B. All precast members are to be designed in accordance with ACI 318-11, 2012 IBC and other applicable codes, standards (see specs) and design criteria shown on design documents.
- C. Precast concrete members shall conform to the 2012 IBC for the required fire ratings (refer to architects documents). D. All wall panels should be designed for building wind loads, seismic loads, gravity loads,
- and transmit these loads to the foundation through properly designed connections. E. Provide blockouts and openings for mechanical/electrical equipment. Refer to mechanical/electrical documents.
- F. Shop drawings shall be complete and shall include a layout plan, fabrication details, estimated camber, connection and anchorage details and member identification marks. Identification marks shall appear on manufactured units to facilitate correct field placement.

### 13. Deferred Submittal and Shop Drawing

- A. Bob D. Campbell and Company, Inc. will review the General Contractor's (GC) shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to be a properly functioning integral element of the overall structural system designed by Bob D. Campbell and Company, Inc.
- B. Deferred submittals shall be submitted to the architect of record for review who shall forward to the building official for review and approval. Design calculations for deferred sub mittals shall be submitted at the same time as the shop drawings for review. Design calculations shall be prepared and sealed by a Professional Engineer licensed in the state of the project. The deferred submittal items shall not be installed until the deferred submittal documents have been approved by the building official.
- Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall: 1. Review each submission for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs incidental thereto, all of which are the sole responsibility of the GC.
- 2. Review and approve each submission. . Stamp each submission as approved. D. Bob D. Campbell and Company, Inc. shall assume that no submission comprises a
- variation unless the GC advises Bob D. Campbell and Company, Inc. with written documentation
- E. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has met the above requirements. Bob D. Campbell and Company, Inc. shall return without comment unrequired material or submissions without GC approval stamp.
- F. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify the GC. 1. Concrete mix designs and material certificates including admixtures and
- compounds applied to the concrete after placement. Reinforcing steel shop drawings including erection drawings and bending details.Bar list will not be reviewed for correct quantities.
- 3. Elevations of all reinforced concrete masonry walls at a scale no smaller than 3/8" = 1'-0" showing all required reinforcing.
- 4. Grout mix designs (for CMU). 5. Construction and control joint plans and/or elevations.
- 6. Structural steel shop drawings including erection drawings and piece details. Include joist, decking and connector submittals. Include miscellaneous framing specified on the structural drawings, but do not submit framing specified on nonstructural drawings for Bob D. Campbell and Company, Inc. review. Defferred Submittal: Railings and guardrails with sealed calculations
- 8. Defferred Submittal: Metal stair framing with sealed calculations 9. Defferred Submittal: Exterior cold-formed metal framing
- 10. Defferred Submittal: Exterior curtain wall 11. Deferred Submittal: Structural steel connection design calculations submitted
- concurrently with structural steel shop drawings. 12. Miscellaneous anchors shown on the structural drawings. 13. Deferred Submittal: Wood truss design calculations and detailed erection and
- fabrication drawings. Standard stick framing shop drawings need not be submitted. 14. Standard details and bridging information for light gage metal framing. Erection plans and details for light gage metal joists and lintels spanning more than 6'-0" shall be submitted. Standard wall framing
- need not be submitted. 15. Deferred Submittal: Augured pile foundation plans and details.
- 16. Deferred Submittal: Precast concrete shop drawings including erection drawings and connection details. 17. Deferred Submittal: Precast concrete connection design calculations.
- 18. Deferred Submittal: Cold-Formed metal framing for exterior walls.

14 04	stangent of Chrystynel Chariel Inchastions					STRUCTURAL SHEET LIST		
14. Sta	The structural design for this project is based on completion of special inspections				Sheet	Sheet Name	Current	C Re
Α.	during construction in accordance with section 1704 of the International Building Code. The owner shall employ one or more qualified special inspectors to provide				S0.01	GENERAL NOTES	4	9.2
В.	the required special inspections. The special inspector shall furnish inspection reports to the building official, owner, architect and structural engineer, and any other designated person.				S0.02	WOOD SCHEDULES & TYPICAL DETAILS	3	7.2
C	All discrepancies shall be brought to the immediate attention of the contractor for correction, then, if uncorrected, to the proper design authority, building official and structural engineer.				S0.03	STEEL SCHEDULES	3	7.2
D.	The special inspector shall submit a final signed report stating that the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with				S0.05 S0.06	WOOD SHRINKAGE & MOVEMENT	2	7.1
E.	the building code. The following inspections and tests are required with the frequency (continuous or				S0.10	CONCRETE SCHEDULE	2	7.1
	periodic) as defined within the referenced section or standard listed below. The General Contractor shall provide notification to the inspector when items requiring inspection are ready to be inspected and provide access for those inspections.				S1.02	SECOND FLOOR FRAMING PLAN		
	<ol> <li>Shop Fabrication – structural steel per Section 1704.2.5 unless AISC certified shop</li> <li>Shop Fabrication – pre-engineered wood trusses per Section 1704.2.5 unless</li> </ol>				S1.03 S1.04	FOURTH FLOOR FRAMING PLAN		
	<ol> <li>Shop Pablication – pre-engineered wood trusses per Section 1704.2.5 unless</li> <li>TPI certified shop</li> <li>Shop Fabrication – precast concrete per Section 1704.2.5 unless PC certified</li> </ol>				S1.05 S1.06	FIFTH FLOOR FRAMING PLAN ROOF FRAMING PLAN		
	<ul> <li>shop</li> <li>Steel Construction per Section 1705.2 and the quality assurance requirements of AISC 341 Chapter J (as referenced by AISC 360)</li> </ul>				S1.11A	BUILDING A FOUNDATION PLAN	3	7.2
	<ol> <li>Concrete Construction per Section 1705.3 and Table 1705.3</li> <li>a. Reinforcing Steel Placement</li> <li>b. Reinforcing Steel Welding</li> </ol>				S1.12A S1.13A	BUILDING A SECOND FLOOR FRAMING PLAN         BUILDING A THIRD FLOOR FRAMING PLAN	3	7.2
	c. Cast in Place Anchors d. Post Installed Anchors				S1.14A S1.15A	BUILDING A FOURTH FLOOR FRAMING PLAN         BUILDING A ROOF FRAMING PLAN	3	7.2
	e. Design Mix Verification f. Concrete Sampling and Testing g. Concrete Placement				S1.16A	BUILDING A SHEARWALL PLAN	3	7.2
	<ul> <li>h. Concrete Curing</li> <li>i. Prestressed Concrete Stressing and Grouting</li> <li>i. Erection of Precast</li> </ul>				S1.21B	BUILDING B SECOND FLOOR FRAMING PLAN	4	9.2
	<ul> <li>k. Verification of In-situ Concrete Strength Prior to Stressing Post-Tensioned Concrete</li> </ul>				S1.23B S1.24B	BUILDING B THIRD FLOOR FRAMING PLAN         BUILDING B FOURTH FLOOR FRAMING PLAN	4	9.2
	<ol> <li>Pornwork Shape, Eccation and Dimensions</li> <li>Masonry Construction per Section 1705.4 and the quality assurance requirements of TMS 402/ACI530/ASCE5 and TMS602/A530.1/ASCE6 Level B</li> </ol>				S1.25B S1.26B	BUILDING B ROOF FRAMING PLAN BUILDING B SHEARWALL PLAN	4	9.2
	<ol> <li>Verification of Soils per Table 1705.6</li> <li>Inspections and Tests of Cast-In-Place Deep Foundation per Table 1705.8</li> <li>Wood Lateral System (periodic)</li> </ol>				S1.31C	BUILDING C FOUNDATION PLAN	4	9.2
	<ul> <li>a. Wood shearwalls (include sheathing, rim board and bottom plate attachments)</li> <li>b. Dartel frames</li> </ul>				S1.32C S1.33C	BUILDING C SECOND FLOOR FRAMING PLAN         BUILDING C THIRD FLOOR FRAMING PLAN	4	9.2
	<ul><li>c. Shear wall and portal frame holdowns</li><li>d. Shear wall tension rod system</li></ul>				S1.34C	BUILDING C FOURTH FLOOR FRAMING PLAN BUILDING C ROOF FRAMING PLAN	4	9.2
	<ol> <li>Wood Gravity Framing and Placement (adjust frequency of random sampling where indicated as required)         <ul> <li>Heavy timber/SCI (glulam beams and supports (periodic))</li> </ul> </li> </ol>				S1.36C		4	9.2
	<ul> <li>b. Headers and jambs (random sampling)</li> <li>c. Bearing walls (random sampling)</li> <li>d. Connector/bardware installation (random compling)</li> </ul>				S1.41D S1.42Da	BUILDING D PODIUM SLAB REINFORCEMENT PLAN	4	9.2
	e. Floor and roof trusses (random sampling)				S1.42Db S1.43D	BUILDING D PODIUI DTLID RAL & LAE GEOMETRYPLAN BUILDING D THIRD FLOOR FRAMING PLAN	3	7.2
15. Co	pyright and Disclaimer				S1.44D		4	9.2
Α.	All drawings in the structural set (S-series drawings) are the copyrighted work of Bob D. Campbell and company, Inc. These drawings may not be photographed,				S1.45D S1.46D	BUILDING D FIFTN BLOOF FRAMING PLAN       BUILDING D ROOF FRAMING PLAN	4	9.2
	traced, or copies in any manner without the written permission of Bob D. Campbell and Company, Inc. Exception: Original drawings may be printed for distribution to the owner, architect, and general contractor for coordination, bidding, and				S1.47D S1.51E	BUILDING D SHEARWALL PLAN BUILDING E FOUNDATION PLAN	3	7.2
в	construction. Subcontractors may not reproduce these drawings for any purpose or in any manner. L Christopher A. Beverlin, P.F., registered engineer and a representative of Bob				S1.52Ea	BUILDING E PODIUM SLAB REINFORCEMENT PLAN	2	7.1
D.	D. Campbell and Company, Inc., do hereby accept professional responsibility as required by the professional registration laws of this state for the structural design				S1.52ED S1.53E	BUILDING E FODIOM STOD RAIL & SLAB GEOMETRY PLAN         BUILDING E THIRD FLOOR FRAMING PLAN	4	9.2
	drawings consisting of S-series drawings. I hereby disclaim responsibility for all other drawings in the construction document package, they being the responsibility of other design professionals whose seals and signed statements may appear				S1.54E S1.55E	BUILDING E FOURTH FLOOR FRAMING PLANBUILDING E FIFTH FLOOR FRAMING PLAN	4	9.2
	elsewhere in the construction document package.				S1.56E	BUILDING E ROOF FRAMING PLAN	4	9.2
	STRUCTURAL ABBREVIATIONS				S1.57E S1.61F	BUILDING F FOUNDATION PLAN	4	9.2
) i	AT FLR FLOOR AND FS FAR SIDE ROUND DIAMETER ETG EQOTING	PERP PL PL F	PERPE PLATE POUNT		S1.62Fa S1.62Fb	BUILDING F PODIUM SLAB REINFORCEMENT PLANBUILDING F PODIUM STUD RAIL & SLAB GEOMETRY PLAN	2 3	7.1
DTL FF	ADDITIONAL FV FIELD VERIFY ABOVE FINISHED FLOOR GA GAGE	PJP PSF	PARTI/ POUNE	AL JOINT PENETRATION DS PER SQUARE FOOT	S1.63F	BUILDING F THIRD FLOOR FRAMING PLAN	2	7.1
LT RCH LDG	ALTERNATE GALV GALVANIZE(D) ARCHITECTURAL GEN GENERAL BUILDING GR GRADE	PSI QTY RAD	POUNE QUANT RADIU:	DS PER SQUARE INCH FITY S	S1.65F	BUILDING F FIFTH FLOOR FRAMING PLAN	2	7.1
/ M OTT	BOTTOM OF     GRBM     GRADE BEAM       BEAM     HORIZ     HORIZONTAL       BOTTOM     HSS     HORIZONTAL	RD-# REF	ROOF REFER	DECK TYPE ENCE	S1.66F S1.67F	BUILDING F ROOF FRAMING PLAN         BUILDING F SHEARWALL PLAN	3	7.2
RG	BEARING IF INSIDE FACE CAMBER INFO INFORMATION	REQD	REQUI	RED ON	S1.71G	GARAGE FOUNDATION PLAN	4	9.2
D-# J JP	CONCRETE DECK TYPE INT INTERIOR CONSTRUCTION/CONTROL JOINT JST JOIST COMPLETE JOINT PENETRATION JT JOINT	RLL RTU SC	ROOF ROOF SLIP C	LIVE LOAD TOP UNIT RITICAL	S1.73G	GARAGE THIRD FLOOR FRAMING PLAN	1	6.0
L MU OI	CENTERLINE K KIPS (1000 LBS) CONCRETE MASONRY UNIT KSF KIPS PER SQUARE FOOT COLUMN KSI KIPS PER SQUARE INCH	SCHED SECT SHT	SCHED SECTIO SHEFT	DULE(D) DN	S1.74G S1.75G	GARAGE FOURTH FLOOR FRAMING PLAN GARAGE FIFTH FLOOR FRAMING PLAN	1	6.0
ONC ONN	CONCRETE LBS, # POUNDS CONNECTION Ld DEVELOPMENT LENGTH	SIM SJ	SIMILA SAW J		S1.76G	GARAGE SNOW LOADING PLAN STAIR FRAMING - BUILDING A	3	7.2
OORD OV, CVR	CONTINUOUSLLLIVE LOADCOORDINATELLHLONG LEG HORIZONTALCOVERLLVLONG LEG VERTICAL	SL SOG SOG-#	SLAB-0 SLAB-0	DN-GRADE DN-GRADE TYPE	S2.01	STAIR FRAMING - BUILDING B	4	9.2
BL ET	DOUBLELONGLONGITUDINALDETAILLSLTLONG-SLOTTED HOLE TRANSVERSEDIAMETERLTWTLIGHTWEIGHT	SPCG SPEC SPRT	SPACI SPECI	NG FICATION	S2.02 S2.03	STAIR & ELEVATOR FRAMING - BUILDING C STAIR & ELEVATOR FRAMING - BUILDING D	2	7.1
IM L	DIMENSION M MOMENT FORCE DEAD LOAD MAX MAXIMUM	SQ SS	SQUAF	RE LESS STEEL	S2.04	STAIR & ELEVATOR FRAMING - BUILDING E	2	7.1
WG A F	DRAWING MECH MECHANICAL EACH MFGR MANUFACTURER EACH FACE MIN MINIMUM	SSLT STD STIFF	SHOR1 STAND STIFFE	-SLOTTED HOLE TRANSVERSE ARD NER	S2.10	STAIR FRAMING DETAILS	2	7.1
J L, ELEV MBED	EXPANSION JOINT MISC MISCELLANEOUS ELEVATION MSRY MASONRY EMBEDMENT EMBEDDED MTL METAL	STIR STL STRUCT	STIRRI STEEL		S2.11 S2.20	BALCONY FRAMING PLANS	3	7.2
NGR OD	ENGINEER NF NEAR FACE EDGE OF DECK NS NEAR SIDE	T/ THRU	TOP OI THROU	JGH	S2.21	BALCONY FRAMING PLANS	3	7.2
OR OS Q	ENGINEER OF RECORD       NTS       NOT TO SCALE         EDGE OF SLAB       NW       NORMAL WEIGHT         EQUAL       OC       ON CENTER	TOS TRANS TYP	TOP O TRANS TYPICA	F STEEL, TOP OF SLAB VERSE AL	S2.30	BALCONY FRAMING DETAILS	2	7.1
QUIP W XP	EQUIPMENT     OF     OUTSIDE FACE       EACH WAY     OPNG     OPENING       EXPANSION     OPP     OPPOSITE	UNO V VERT	UNLES SHEAR	S NOTED OTHERWISE	S3.00 S3.01	GARAGE FOUNDATION DETAILS	2	7.1
XT XTG, EXIST	EXTERIOR OVS OVERSIZED HOLE EXISTING P AXIAL FORCE	W/ W/0	WITH		S3.02	PILE & PODIUM FOUNDATION DETAILS APARTMENT FOUNDATION DETAILS	<b>4</b>	9.2
D-# DN F	FLOOR DECK TYPEPAFPOWDER ACTUATED FASTENERFOUNDATIONPCPRECAST / PILE CAPFAR FACEPCFPOUNDS PER CUBIC FOOT	WF WL WP	WIDE F WIND L WORK	OAD POINT	S3.04	FOUNDATION DETAILS	4	9.2
IN	FINISH PEMB PRE-ENGINEERED METAL BUILDING	WWF	WELDE	ED WIRE FABRIC	S3.05	FOUNDATION DETAILS	3	9.2
STR	UCTURAL DECK & SLAB SCHEDULE				S3.07 S3.08	FOUNDATION DETAILS FOUNDATION DETAILS	3	7.2
MARK	DESCRIPTION 1" TO 1 1/4" GYPCRETE ATOP 23/32" APA RATED T&G STURD-I-FLOOR,				S3.11 S3.12	CONCRETE FRAMING DETAILS	4	9.2
	EXP 1 SHEATHING. SHEATHING SHALL BE GLUED AND NAILED W/ 8d RING SHANK NAILS OR #10 SCREWS AT 6"o.c. AT EDGES & 12"o.c. AT FIELD.				S3.15	SHEAR RAIL DETAILS	2	7.1
CD-1	3" NORMAL WEIGHT CONC. SLAB (4500psi, AIR-ENTRAINED) REINFORCE WITH CELLULOSE FIBER AT 1.5 LBS/CU. YD. ATOP WATERPROOFING				S3.30 S3.31	WOOD FLOOR FRAMING DETAILS WOOD FLOOR FRAMING DETAILS	3	1.2
	(SLOPE PER ARCH.) RE: NOTE 5				S3.41	WOOD ROOF FRAMING DETAILS	3	7.2
CD-2	3" NORMAL WEIGHT CONC. SLAB (4500psi, AIR-ENTRAINED) REINFORCE WITH CELLULOSE FIBER AT 1.5lb/cu. yd. ATOP PRECAST STAIR LANDING				S3.45	WOOD FIREWALL DETAILS	3	7.2
CD-3	3" NORMAL WEIGHT CONC. SLAB (4500psi, AIR-ENTRAINED) REINEORCE WITH CELLULOSE EIBER AT 1.5b/cu. vd. ATOR 2" RIGID INSULATION				S3.50 S3.51	PRECAST GARAGE FRAMING DETAILS PRECAST GARAGE FRAMING DETAILS	4	9.2
CD-3	ATOP WATERPROOFING PER ARCH ATOP PODIUM SLAB. SLOPE TO DRAIN PER ARCH. RE: NOTE 5		<u>LEG</u>	END:	S3.60	CFMF DETAILS	3	7.2
SOG-1	4" CONC. SLAB (4000psi) REINFORCE WITH 6x6-W2.9xW2.9 WWF ATOP 15 MIL VAPOR BARRIER ATOP 4" OF			SPAN DIRECTION OF DECI	K - DECK TYPE	PER SCHEULE ON S0.01		
	3/4" CLEAN GRANULAR LEVELING COURSE ATOP SUITABLE SUBGRADE MATERIAL PER GEOTECH SPECIFICATIONS. T/SLAB EL. = PER PLAN, SLOPE TO DRAIN			BASE PLATE MARK - SEE SCHEDU	JLE ON SHEET	S0.04		
SOG-2	4" CONC. SLAB (4500psi, AIR-ENTRAINED) REINFORCE WITH 6x6-W2.9xW2.9 WWF ATOP 4" OF 3/4" CLEAN GRANULAR		(A#-#)	BEAM OR HEADER PER SC	CHEDULE ON S	60.02		
<b>__</b>	SPECIFICATIONS T/SLAB EL. = PER PLAN, SLOPE TO DRAIN		(A#-#u)			LE ON \$0.02		
SOG-3	8" CONC. SLAB (4500psi, AIR-ENTRAINED) REINFORCE WITH #4 @ 12"oc EACH WAY BOTTOM ATOP 4" OF 3/4" CLEAN GRANULAR LEVELING COURSE, ATOP SUITABLE SUBGRADE MATERIAL PER		(A)	SHEARWALL HOLDDOWN	TYPE PER SCH	HEDULE ON S0.03		
	GEOTECH SPECIFICATIONS. T/SLAB EL. = PER PLAN, SLOPE TO DRAIN 6" CONC. SLAB (4500psi. AIR-FNTRAINED)		#	NUMBER OF WALL STUDS	S IN STUD PACK	ζ		
SOG-4	REINFORCE WITH #4 @ 12"o.c. EACH WAY ATOP 4" OF 3/4" CLEAN GRANULAR LEVELING COURSE, ATOP SUITABLE SUBGRADE MATERIAL PER GEOTECH		#/#		/KING STUDS IN	N STUD PACK		
RD-1	19/32" APA RATED, EXP 1 SHEATHING ATTACHED WITH #10 SCREWS AT		A SW#	SHEARWALL TYPE PER SC	CHEDULE ON S	60.03		
	FIREWALLS - REFER TO ARCH DRAWINGS FOR LOCATION AND EXTENTS)		CSW#	CONCRETE SHEARWALL T	TYPE PER SCH	ED ON S0.10		
RD-2	23/32 APA KATED T&G STURD-I-FLOOR, EXP 1 SHEATHING. SHEATHING SHALL BE GLUED AND NAILED W/ 10d RING SHANK NAILS OR #10 SCREWS AT 4"a c. AT EDGES & 12"a c. AT FIELD WITH ALL EDGES BLOCKED		\ <u>3/4</u> "	AMOUNT OF UPWARD POS	SITIVE CAMBER	R		

14 State	mont of Structural Special In	sportion	<b>~</b>				STRUCTURAL SHEET LIST				
	structural design for this project is be	spection	S			Sheet	Sheet Name	Current			
A. The duri Coc	ng construction in accordance with se le. The owner shall employ one or mo	ection 1704 pre qualified	of the International Building special inspectors to provide			S0.01	GENERAL NOTES	4			
the B. The arcl	required special inspections. special inspector shall furnish inspective and structural engineer, and any	tion reports	to the building official, owner,			S0.02	WOOD SCHEDULES & TYPICAL DETAILS	3			
C. All c	discrepancies shall be brought to the i rection, then, if uncorrected, to the pro-	mmediate a	authority, building official and			S0.03 S0.04	WOOD SCHEDULES & TYPICAL DETAILS STEEL SCHEDULES	3			
stru D. The	ctural engineer. special inspector shall submit a final cial inspection was, to the best of the	signed repo	ort stating that the work requiring			S0.05		2			
the	approved plans and specifications an building code.	d the applic	able workmanship provisions of			S0.10	CONCRETE SCHEDULE	2			
E. The peri Ger	following inspections and tests are re odic) as defined within the referenced neral Contractor shall provide notificat	equired with I section or : ion to the in	the frequency (continuous or standard listed below. The spector when items requiring			S1.01 S1.02	FOUNDATION PLAN SECOND FLOOR FRAMING PLAN				
insp 1.	bection are ready to be inspected and Shop Fabrication – structural steel pe	provide acc er Section 1	ess for those inspections. 704.2.5 unless AISC certified			S1.03	THIRD FLOOR FRAMING PLAN				
2.	shop Shop Fabrication – pre-engineered w TPI certified shop	ood trusses	s per Section 1704.2.5 unless			S1.04 S1.05	FOURTH FLOOR FRAMING PLAN FIFTH FLOOR FRAMING PLAN				
3.	Shop Fabrication – precast concrete shop	per Section	1704.2.5 unless PC certified			S1.06	ROOF FRAMING PLAN				
4. 5	Steel Construction per Section 1705. of AISC 341 Chapter J (as reference Concrete Construction per Section 1	2 and the q d by AISC 3 705 3 and T	uality assurance requirements 360) Fable 1705 3			S1.11A S1.12A	BUILDING A FOUNDATION PLAN BUILDING A SECOND FLOOR FRAMING PLAN	3			
0.	<ul><li>a. Reinforcing Steel Placement</li><li>b. Reinforcing Steel Welding</li></ul>					S1.13A	BUILDING A THIRD FLOOR FRAMING PLAN	3			
	<ul> <li>c. Cast in Place Anchors</li> <li>d. Post Installed Anchors</li> <li>e. Design Mix Verification</li> </ul>					S1.15A	BUILDING A ROOF FRAMING PLAN	4			
	f. Concrete Sampling and Testing g. Concrete Placement					S1.16A S1.21B	BUILDING A SHEARWALL PLAN BUILDING B FOUNDATION PLAN	3			
	<ul> <li>i. Prestressed Concrete Stressing</li> <li>j. Erection of Precast</li> </ul>	and Groutir	ng			S1.22B	BUILDING B SECOND FLOOR FRAMING PLAN	4			
	k. Verification of In-situ Concrete S Concrete	trength Prio	r to Stressing Post-Tensioned			S1.23B S1.24B	BUILDING B FOURTH FLOOR FRAMING PLAN	4			
6.	Masonry Construction per Section 17 requirements of TMS 402/ACI530/AS	705.4 and th SCE5 and T	e quality assurance MS602/A530.1/ASCE6 Level B			S1.25B	BUILDING B ROOF FRAMING PLAN	4			
7. 8.	Verification of Soils per Table 1705.6 Inspections and Tests of Cast-In-Plac	; ce Deep Fo	undation per Table 1705.8			S1.31C	BUILDING C FOUNDATION PLAN	4			
9.	a. Wood shearwalls (include sheath attachments)	hing, rim bo	ard and bottom plate			S1.32C S1.33C	BUILDING C SECOND FLOOR FRAMING PLAN BUILDING C THIRD FLOOR FRAMING PLAN	4			
	<ul><li>b. Portal frames</li><li>c. Shear wall and portal frame hold</li><li>d. Shear wall tension rod system</li></ul>	owns				S1.34C	BUILDING C FOURTH FLOOR FRAMING PLAN	4			
10.	Wood Gravity Framing and Placeme where indicated as required)	nt (adjust fre	equency of random sampling			S1.35C S1.36C		4			
	<ul> <li>a. Heavy timber/SCL/glulam beams</li> <li>b. Headers and jambs (random sampling)</li> <li>c. Bearing walls (random sampling)</li> </ul>	s and suppc npling) \	orts (periodic)			S1.41D	BUILDING D FOUNDATION PLAN	4			
	<ul> <li>d. Connector/hardware installation</li> <li>e. Floor and roof trusses (random s</li> </ul>	/ (random sa sampling)	mpling)			S1.42Db	BUILDING D PODIUI DTLID RAL & LAE GEOMETRY PLAN	3			
15 Com	right and Disclaimer					S1.43D S1.44D	BUILDING D THIRD FLO <del>UR</del> FRAMING PLAN	4			
A. Allo	drawings in the structural set (S-series	s drawings)	are the copyrighted work of			S1.45D		4			
Bob trac	D. Campbell and company, Inc. The red, or copies in any manner without the Company Inc. Exception: Original	ese drawings he written po drawings ma	s may not be photographed, ermission of Bob D. Campbell av be printed for distribution to			S1.46D S1.47D	BUILDING D ROOF FRAMING PLAN BUILDING D SHEARWALL PLAN	3			
the	owner, architect, and general contrac struction. Subcontractors may not re	tor for coord produce the	lination, bidding, and se drawings for any purpose			S1.51E	BUILDING E FOUNDATION PLAN	4			
or iı B. I, C D. (	n any manner. hristopher A. Beverlin, P.E., registere Campbell and Company, Inc., do bere	d engineer a	and a representative of Bob			S1.52Ea	BUILDING E PODIUM STUD RAIL & SLAB GEOMETRY PLAN	2			
req drav	uired by the professional registration la wings consisting of S-series drawings	aws of this s . I hereby d	isclaim responsibility for all			S1.53E	BUILDING E THIRD FLOOR FRAMING PLAN BUILDING E FOURTH FLOOR FRAMING PLAN	4			
othe of o else	er drawings in the construction docum ther design professionals whose seal where in the construction document r	ient packag s and signe package	e, they being the responsibility d statements may appear			S1.55E	BUILDING E FIFTH FLOOR FRAMING PLAN	4			
		חוודר				S1.56E S1.57E	BUILDING E ROOF FRAMING PLAN BUILDING E SHEARWALL PLAN	4			
^	<u>51RU(</u>			DEDD		S1.61F	BUILDING F FOUNDATION PLAN	4			
A R	ND OUND, DIAMETER	FS FTG	FAR SIDE FOOTING	PL PLF	PLATE POUNDS PER LINEAR FOOT	S1.62Fb	BUILDING F PODIUM STUD RAIL & SLAB GEOMETRY PLAN	3			
DTL A F A T A	DDITIONAL BOVE FINISHED FLOOR L TERNATE	FV GA GALV	FIELD VERIFY GAGE GALVANIZE(D)	PJP PSF PSI	PARTIAL JOINT PENETRATION POUNDS PER SQUARE FOOT POUNDS PER SOLVARE INCH	S1.63F S1.64F	BUILDING F THIRD FLOOR FRAMING PLAN BUILDING F FOURTH FLOOR FRAMING PLAN	2			
RCH A .DG B	RCHITECTURAL UILDING	GEN GR	GENERAL GRADE	QTY RAD	QUANTITY RADIUS	S1.65F	BUILDING F FIFTH FLOOR FRAMING PLAN	2			
B M B DTT B	OTTOM OF EAM OTTOM	GRBM HORIZ HSS	GRADE BEAM HORIZONTAL HOLLOW STRUCTURAL SECTION	RD-# REF REINF	ROOF DECK TYPE REFERENCE REINFORCEMENT	S1.66F S1.67F	BUILDING F ROOF FRAMING PLAN BUILDING F SHEARWALL PLAN	3			
RG B C	EARING AMBER	IF INFO	INSIDE FACE INFORMATION	REQD REV	REQUIRED REVISION	S1.71G	GARAGE FOUNDATION PLAN	4			
D-# C	ONCRETE DECK TYPE ONSTRUCTION/CONTROL JOINT	INT JST JT	INTERIOR JOIST JOINT	RLL RTU SC	ROOF LIVE LOAD ROOF TOP UNIT SUP CRITICAL	S1.72G	GARAGE SECOND FLOOR FRAMING FLAN	1			
	ENTERLINE ONCRETE MASONRY UNIT	K KSF	KIPS (1000 LBS) KIPS PER SQUARE FOOT	SCHED SECT	SCHEDULE(D) SECTION	S1.74G S1 75G	GARAGE FOURTH FLOOR FRAMING PLAN	1			
DL C DNC C DNN C	OLUMN ONCRETE ONNECTION	KSI LBS, # Ld	KIPS PER SQUARE INCH POUNDS DEVELOPMENT LENGTH	SHT SIM SJ	SHEET SIMILAR SAW JOINT	S1.76G	GARAGE SNOW LOADING PLAN				
ONT CORD C	ONTINUOUS OORDINATE	LL LLH	LIVE LOAD LONG LEG HORIZONTAL	SL SOG	SNOW LOAD SLAB-ON-GRADE	S2.00 S2.01	STAIR FRAMING - BUILDING A STAIR FRAMING - BUILDING B	4			
BL D ET D	OVER OUBLE ETAIL	LLV LONG LSLT	LONG LEG VERTICAL LONGITUDINAL LONG-SLOTTED HOLE TRANSVERSE	SDG-# SPCG SPEC	SLAB-ON-GRADE TYPE SPACING SPECIFICATION	S2.02	STAIR & ELEVATOR FRAMING - BUILDING C	2			
A D M D	IAMETER IMENSION	LTWT M	LIGHTWEIGHT MOMENT FORCE	SPRT SQ	SUPPORT SQUARE	S2.03	STAIR & ELEVATOR FRAMING - BUILDING E	2			
VG D	RAWING ACH	MAX MECH MFGR	MAXIMUM MECHANICAL MANUFACTURER	SS SSLT STD	STAINLESS STEEL SHORT-SLOTTED HOLE TRANSVERSE STANDARD	S2.05 S2.10	STAIR FRAMING - BUILDING F STAIR FRAMING DETAILS	2			
	ACH FACE XPANSION JOINT	MIN MISC MSRV	MINIMUM MISCELLANEOUS MASONRY	STIFF STIR STI	STIFFENER STIRRUP STEEL	S2.11	ELEVATOR FRAMING DETAILS				
IBED E	MBEDMENT, EMBEDDED NGINEER	MTL NF	METAL NEAR FACE	STRUCT T/	STRUCTURE, STRUCTURAL TOP OF	S2.20 S2.21	BALCONY FRAMING PLANS BALCONY FRAMING PLANS	3			
DD E DR E DS F	DGE OF DECK NGINEER OF RECORD DGE OF SLAB	NS NTS NW	NEAR SIDE NOT TO SCALE NORMAL WEIGHT	THRU TOS TRANS	THROUGH TOP OF STEEL, TOP OF SLAB TRANSVERSE	S2.22	BALCONY FRAMING PLANS BALCONY FRAMING DETAILS	3			
Q E QUIP E	QUAL	OC OF	ON CENTER OUTSIDE FACE	TYP UNO	TYPICAL UNLESS NOTED OTHERWISE	S3.00	TYPICAL FOUNDATION DETAILS	2			
V E (P E (T E	ACH WAY XPANSION XTERIOR	OPNG OPP OVS	OPENING OPPOSITE OVERSIZED HOLE	V VERT W/	SHEAR FORCE VERTICAL WITH	S3.01 S3.02	GARAGE FOUNDATION DETAILS PILE & PODIUM FOUNDATION DETAILS	2 4			
(TG, EXIST E )-# F	XISTING LOOR DECK TYPE	P PAF	AXIAL FORCE POWDER ACTUATED FASTENER	W/0 WF	WITHOUT WIDE FLANGE	S3.03	APARTMENT FOUNDATION DETAILS	3			
F N F	AR FACE INISH	PCF PEMB	POUNDS PER CUBIC FOOT PRE-ENGINEERED METAL BUILDING	WP WWF	WORK POINT WELDED WIRE FABRIC	S3.04	FOUNDATION DETAILS	4			
ייחדס				]		S3.06 S3.07	FOUNDATION DETAILS FOUNDATION DETAILS	3			
MARK	DESCRIPTION	SLAB	SUNEDULE	-		S3.08	FOUNDATION DETAILS	3			
FD-1	1" TO 1 1/4" GYPCRETE ATOP 23	/32" APA R				S3.11 S3.12	CONCRETE FRAMING DETAILS	3			
	SHANK NAILS OR #10 SCREWS A	AT 6"o.c. AT	EDGES & 12"o.c. AT FIELD.			S3.15	SHEAR RAIL DETAILS WOOD FLOOR FRAMING DETAILS	2			
CD-1	3" NORMAL WEIGHT CONC. SLA REINFORCE WITH CELLULOSE F MEMBRANE (REVARCH) ATOP 1	B (4500psi, FIBER AT 1. 5/32" F X T F	AIR-ENTRAINED) 5 LBS/CU. YD. ATOP WATERPROOFING RIOR GRADE PI YWOOD SHEATHING			S3.31	WOOD FLOOR FRAMING DETAILS	0			
	(SLOPE PER ARCH.) RE: NOTE 5					S3.41 S3.42	WOOD ROOF FRAMING DETAILS WOOD ROOF FRAMING DETAILS	3			
CD-2	3 NORMAL WEIGHT CONC. SLAI REINFORCE WITH CELLULOSE F STAIR LANDING	⊟ (4500psi, FIBER AT 1.	AIR-EN I RAINED) .5lb/cu. yd. ATOP PRECAST			S3.45	WOOD FIREWALL DETAILS	3			
CD-3	3" NORMAL WEIGHT CONC. SLA	B (4500psi, -IBFR Δ⊤ 1	AIR-ENTRAINED) 51b/cu, vd. ATOP 2" RIGID INSULATION			S3.50 S3.51	PRECAST GARAGE FRAMING DETAILS PRECAST GARAGE FRAMING DETAILS	4			
	ATOP WATERPROOFING PER AF	RCH ATOP	PODIUM SLAB. SLOPE TO DRAIN PER		LEGEND:	S3.60	CFMF DETAILS	3			
SOG-1	4" CONC. SLAB (4000psi) REINFORCE WITH 6x6-W2.9xW2.	9 WWF AT	OP 15 MIL VAPOR BARRIER ATOP 4" OF		HSS 6x6x1/4 COLUMN SIZE	K - DECK TYPE	PER SCHEULE ON S0.01				
<b></b>	3/4" CLEAN GRANULAR LEVELIN PER GEOTECH SPECIFICATIONS	G COURSE 6. T/SLAB E	E ATOP SUITABLE SUBGRADE MATERIAL EL. = PER PLAN, SLOPE TO DRAIN		BASE PLATE MARK - SEE SCHEDU	JLE ON SHEET S	S0.04				
SOG-2	4" CONC. SLAB (4500psi, AIR-ENT REINFORCE WITH 6x6-W2.9xW2.	RAINED) 9 WWF AT	OP 4" OF 3/4" CLEAN GRANULAR		A#-# BEAM OR HEADER PER SC	CHEDULE ON S	0.02				
•	LEVELING COURSE, ATOP SUITA SPECIFICATIONS T/SLAB EL. = P	ABLE SUBO ER PLAN, S	SLOPE TO DRAIN	(A#-#u) UPSET BEAM OR HEADER PER SCHEDULE ON S0.02							
SOG-3	8" CONC. SLAB (4500psi, AIR-ENT REINFORCE WITH #4 @ 12"oc EA	RAINED) CH WAY B	SOTTOM ATOP 4" OF 3/4" CLEAN	A SHEARWALL HOLDDOWN TYPE PER SCHEDULE ON S0.02							
•	GEOTECH SPECIFICATIONS. T/S	ELAB EL. = I	PER PLAN, SLOPE TO DRAIN								
SOG-4	6" CONC. SLAB (4500psi, AIR-ENT REINFORCE WITH #4 @ 12"o.c. E LEVELING COURSE ATOP SUIT	I RAINED) ACH WAY ABLE SUBC	ATOP 4" OF 3/4" CLEAN GRANULAR GRADE MATERIAL PER GEOTECH		#/# NUMBER OF JACK STUDS/	KING STUDS IN	I STUD PACK				
	SPECIFICATIONS T/SLAB EL. = P	ER PLAN, S		-		LE ON S0.02	0.02				
RD-1	6"o.c. AT EDGES & 12"o.c. AT FIE FIREWALLS - REFER TO ARCH D	LD. (PROVI RAWINGS	IDE FRT TREATED PLYWOOD AT FOR LOCATION AND EXTENTS)		SW# SHEARWALL TYPE PER SC	CHEDULE ON S	0.03 ED ON S0.10				
RD-2	23/32" APA RATED T&G STURD- SHALL BE GLUED AND NAUED M	I-FLOOR, E	XP 1 SHEATHING. SHEATHING SHANK NAILS OR #10 SOREWS	1	3/4" AMOUNT OF UPWARD POS	SITIVE CAMBER	R				
	AT 4"o.c. AT EDGES & 12"o.c. AT	FIELD WITH	H ALL EDGES BLOCKED	B PILE CAP SIZE PER SCHEDULE ON S3.02							
1. $FD = FLC$ 2. $CD = COI$	OOR DECK TYPE. NCRETE DECK TYP.				A SHEAR RAIL TYPE PER SC	HEDULE ON S3	3.15				

- SOG = SLAB-ON-GRADE TYP. 4. RD = ROOF DECK TYP.
- 5. PROVIDE 1" DEEP TOOLED CONTROL JOINT (TRANSVERSE DIRECTION ) @ MID-SPAN OF BALCONY (8'-0" MAX SPACING) FILL JOINT w/ SEALANT.

= 20 psf or 14 psf plus Drift = 10 psf = 5 psf combination with wind uplift shall be 5psf for each chord.

= 15 psf (TPO Roof)

= 20 psf (Plus Rooftop Equipment)

![](_page_14_Picture_127.jpeg)

SHEET NUMBER

### GENERAL NOTES

SHEET TITLE

BELL & CO. Since 1957	816.531.4144 /ww.bdc-engrs.com
BOB D. CAMP	4338 Belleview Ave. Kansas City, MO 64111  w

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMER
MECHANICAL	LATIMER SOMMER
ELECTRICAL	LATIMER SOMMER
FIRE PROTECTION	LATIMER SOMMER
CONTRACTOR	BRINKMANN CONSTRUCTORS

PROJECT TEAM

![](_page_14_Picture_134.jpeg)

REGISTRATION

Project No.: 18017,19050.07,19050.08 10.18.24 Issued For: GARAGE PERMIT REVISIONS Description Date 6.08.22 Permit Response 7.11.22 ADDENDUM 1 7.20.22 ADDENDUM 2 4 9.28.22 ASI 1 \_\_\_\_\_ \_\_\_\_ \_\_\_\_ \_\_\_\_ \_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_

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RELEASED FOR CONSTRUCTION As Noted on Plans Review

Lee's Summit, Missouri

# PARAGON STAR NORTH VILLAGE 3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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Current

Revision

![](_page_15_Figure_0.jpeg)

 $A \underline{CMU WALL ELEVATION}_{1.1/2" = 1'-0"}$ 

MASONRY VERTICAL REINFORCING SCHEDULE FOR LOAD BEARING MASONRY (CMU) WALLS									
WALL THICKNESS	LOCATION	VERTICAL REINF. (IN GROUTED CELLS)	SPACING						
8"	GARAGE ATOP PRECAST	1- #5	48"oc						
8"	EXT WALLS BELOW PODIUM	1- #5	32"oc						

1- #5

BE SPACED AT A MAXIMUM OF 24'-0" ON CENTER AND NOT LESS THAN 2'-0" FROM THE EDGE OF ANY OPENING. ALL HORIZONTAL JOINT REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS. ALL BOND BEAM HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL JOINTS. CONTRACTOR SHALL COORDINATE AND VERIFY ALL CONTROL JOINT LOCATIONS.

DETAILED ON REINFORCING STEEL SHOP DRAWING ELEVATIONS. VERTICAL CONTROL JOINTS IN MASONRY WALLS SHALL BE 3/8" WIDE, FULL HEIGHT OF WALL. JOINTS SHALL

1. VERTICAL REINFORCING BARS SHALL BE DOWELED TO FOUNDATION WITH A DOWEL OF MATCHING SIZE

ELEVATOR

S0.05

3

VERT. WALL -

CONTROL JT.

(FULL HEIGHT)

2'-0" MIN.

4

в

S0.05

AND SPACING.

GENERAL CRITERIA: (SECTION A CONTINUED):

TYPICAL CMU WALL REINFORCING AT OPENINGS

- HORIZONTAL BOND BEAM

**REINFORCING CONTINUOUS** 

THROUGH CONTROL JOINTS

2

TOP OF WALL

S0.05

LINE WHERE

INDICATED

1 FULL HEIGHT VERTICAL BARS AS JAMB REINFORCING IN FIRST 2 CELLS ADJACENT TO OPENING. REINFORCE EACH CELL WITH SIZE & QUANTITY OF BAR TO MATCH WALL REINFORCING (1 BAR TYPICAL IN 8" WALLS AND 2 BARS TYPICAL IN 12" WALLS). 2 LINTEL REINFORCING PER SECTION C. EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL). 3 2-#5 CONTINUOUS HORIZONTAL BARS AS SILL REINFORCING IN 8" COURSE BELOW OPENING (U.N.O.). EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).

![](_page_15_Picture_17.jpeg)

NOT USED

32"oc

![](_page_15_Picture_19.jpeg)

NOT USED

![](_page_15_Figure_21.jpeg)

![](_page_15_Figure_22.jpeg)

![](_page_15_Figure_23.jpeg)

![](_page_15_Picture_24.jpeg)

RELEASED FOR CONSTRUCTION

BEAM	SCHE	DULES							E	BEAN	M S	CHE	JUT	JLES						<u></u>			URAL) ON SHEET S	S001.	
MARK SIZE SHAPE OR b d section Quantity Size total MK	GITUDINAL TOP PLACED	STEEL BOT. REMARKS	NO. S	TIRRUPS HAPE SPACING		MARK -	SIZE <sup>S'</sup> b d <sub>SE</sub>	OR OR ECTION QUAN	NTITY SIZE	LC TOTAL LENGTH MI	ongitue /k top	PINAL Placed	STEEI Bot.	L Remarks		STIRRUPS NO. SHAPE SPACING	3			2	2. PODIUM SLA COLUMN CEN CENTERLINE FOR BOTTOM	B IS 15" THICK REIN ITERLINE OF COLUI OF MID-STRIPS) BC I MAT EXTENDING E ERS AT 4'-0" o c	NFORCED WITH A MN STRIPS AND 24 DTTOM MAT OF #6 EAST/WEST THAT S	CONTINUOUS ( 4" LAP AT COLL @ 12" EACH W SHALL BE SUPI	(60" LAP AT JMN /AY. SEE PLAI 'PORTED ON 1
		<ul> <li>♀ OF SUPPORT</li> <li>1/4 PT. OF SPAN</li> <li>FACE OF SUPPORT</li> <li>● 1/4 PT. OF ADJ. SPAN</li> </ul>												<ul> <li>Q OF SUPPORT</li> <li>1/4 PT. OF SPAN</li> <li>FACE OF SUPPORT</li> <li>1/4 PT. OF ADJ. SPAN</li> </ul>						3	$\begin{array}{c} \text{SLAD BOLUTE}\\ \text{3. TOP REINFOL}\\ 14A & 5 \\ \hline \end{array}$	RCING BARS PLACI 19'-9" TOTAL	NG SEQUENCE: L LENGTH OF BAR	IN FEET AND I	INCHES
B1 36" 36" 4 #8 32-0 6 #8 12-0 6 #8 27-0	•		#4 <b>[</b>	18 @ 4"oc RIGHT END RMDR @ 12"oc		B32	24" 36"		4 #7 2 #7 4 #7	26-0 20-0 22-0	•		1 - • •	ALT HOOK DIRECTION		#4 @ 12"oo	c				TOTAL	SIZE OF BAR AND NUMBER OF EXTRA	) LOCATION IN SLA A BARS IN STRIP D VITH 1" CLEAR COV	AB AS NOTED B EFINED ON PL/ VER BOTTOM.	ELOW AN (PLACE
B2 36" 36" 4 #8 34-0 6 #8 12-0 6 #8 27-0			#4	10 @ 4"oc, 12" @ 8"oc LEFT END RMDR @ 12"oc		B33	24" 36"		4 #7 4 #7 4 #7	22-0 20-0 22-0	•			ALT HOOK DIRECTION		#4 @ 12"00	c				WITH T "B" #6 EXTR WITH 1 ( PERPEN "C" #7 TOP I	A BOTTOM BARS W 3/4" CLEAR COVER IDICULAR (1" CLEAF BARS WITH 1 7/8" C	VITH 1 3/4" CLEAR ( BOTTOM MAT BAR R COVER) BOTTOM LEAR COVER WHE	COVER BOTTO RS.) PLACE ON M MAT AND "A" FRE TWO LAYE	M. (PLACE I TOP OF BARS. FRS OF BARS
B3 48" 36" 6 #8 32-0 8 #8 36-0 12 #8 46-0		<ul> <li>PROJECT 4'-0" PAST GRID F.2</li> <li>6" CLR FROM BOTT</li> </ul>	(2) #4	@ 10"oc		B34	24" 36"		4 #7 4 #8 4 #8	29-0 26-0 27-0	• [*			ALT HOOK DIRECTION		#4 @ 12"oo	c				OCCUR ON IHC "D" #7 TOP I BARS W	AND 1" CLEAR COV @4'-0" o.c. AND #5 S BARS WITH 1" CLEA 'HERE THEY OCCUF	VER WHERE ONE L SUPPORT BARS @ AR COVER TOP. PL R OR OTHERWISE	AYER OF BARS 4'-0"o.c. LACE ON TOP ( PLACE ON IHC	3 OCCUR OF "C" C AT 4'-0"o.c.
B4 48" 36" 6 #8 40-0 12 #8 24-0 8 #8 14-0		PROJECT 4'-0" PAST 12" WALL	(2) #4	@ 10"oc													]				AND #5 "E" #6 TOP I OCCUR "F" #5 TOP I	SUPPORT BARS AT BARS WITH 1" CLEA ON IHC AT 4'-0" o.c. BARS WITH 1 7/8" Cl	<sup>4</sup> '-0" o.c. AR COVER WHERE AND #5 SUPPORT LEAR COVER WHE	ONE LAYER O BARS AT 4'-0"d ERE TWO LAYE	F BARS o.c. ERS OF BARS
B5 36" 36" 4 #8 32-0 4 #8 36-0 10 #8 46-0		<ul> <li>PROJECT 4'-0" PAST GRID F.2</li> <li>6" CLR FROM BOTT</li> </ul>	#4	@ 10"oc		ТҮРЕ	THICKNESS	VER1 REINFOR	TICAL RCEMENT	HORIZON		COLUMN @	LATERA	AL LOAD ALONG WALL R PILE DESIGN (kif)	UPLI	FT AT EACH END OF WALL OR PILE DESIGN (kips)					IHC AT 4 "G" #5 TOP I WHERE SUPPOF	AND T CLEAR COV I'-0" o.c. AND #5 SUF BARS WITH 1" CLEA THEY OCCUR, OTH RT BARS AT 4'-0"o.c.	PPORT BARS AT 4 R COVER TOP, PL ERWISE PLACE O	-0"o.c. ACE ON TOP C N IHC AT 4'-0" c	)F "F" BARS o.c. AND #5
B6 36" 36" 4 #8 40-0 10 #8 24-0 6 #8 14-0		PROJECT 4'-0" PAST 12" WALL	#4	@ 10"oc		CSW1	8"	#5 @ 12"oc (	CTR IN WALL	#5 @ 12"oc CTF	TR IN WALL	8"x32"		- ( )						4	4. REINFORCIN REINFORCIN EQUAL CON PROVIDE RE	IG SHALL BE SPLAY IG SHALL BE CUT A TINUOUS BARS ADI EINFORCING PER GI	/ED AROUND OPEI T OPENINGS GREA DED 0NE-HALF EAG ENERAL NOTE 7F A	NINGS LESS TH ATER THAN 18' CH SIDE OF OF AT ALL OPENIN	HAN 18" WIDE " WIDE WITH PENING. NGS LARGER
B7 36" 32" 36" 32" 4 #8 42-0 2 #8 16-0 3 #8 22-0	•		#4	@ 12"oc		CSW2	8"	#5 @ 12"oc (	CTR IN WALL	#5 @ 12"oc CTF	TR IN WALL	8"x32"					-			5	THAN 8". 5. STRIP LINES UNLESS NO <sup>-</sup> 6. SEE DETAIL	ARE LOCATED AT TED ON PLAN OTHE 2/S3.10 FOR PLACIN	1/4 POINTS BETWE ERWISE. NG PATTERN FOR	EEN COLUMN ( TOP REINFOR(	CENTERLINES
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	• • • • • • • • • • • • • • • • • • •	•			-	CSW3	8"	#5 @ 12"oc (		#5 @ 12"oc CTF		8"x32"					-			7	7. TOP BARS S PLACED; TH STRIP LINE, 3. BOTTOM BA	HOWN STAGGEREI IE END OF EVERY C UNLESS NOTED OF RS ARE SHOWN TH	D ON PLAN SHALL DTHER BAR TO BE N PLAN. IUS	BE STAGGERE PLACED AT RE _ — —	ED WHEN ELATIVE
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	• • • • • • •	•	#4		-	CSW4 CSW5	8"	#5 @ 12 0c 0	CTR IN WALL	#5 @ 12"oc CTF		8"x32"								ç	TOP BARS A TOP BARS S ACI 90 DEG. 9. UNLESS SHO	RE SHOWN THUS HOWN ON PLAN TH HOOK. DWN ON "S" SERIES	IUS SHA	 ALL HAVE A STA HOLES LARGEF	ANDARD R THAN TEN
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	•		#4	@ 12"0c	-	CSW6	8"	#5 @ 12"oc (	CTR IN WALL	#5 @ 12"oc CTF	TR IN WALL	8"x32"									INCH DIAME SIX TO EIGH OR THREE T WITHIN 20" C	TER SHALL BE PLAC T INCH DIAMETER H WO INCH DIAMETE DF THE FACE OF TH	CED THROUGH SL HOLES, OR TWO F R OR SMALLER HO IE COLUMNS.	AB. NOT MORE OUR INCH DIAN DLES SHALL BE	E THAN ONE, METER HOLES E PLACE
B10 36" 32" 4 #8 16-0 4 #8 22-0 8 #8 34-0		•	#4	RIGHT END RMDR @ 12"oc		CSW7	8"	#5 @ 12"oc (	CTR IN WALL	#5 @ 12"oc CTF	TR IN WALL	8"x32"								1	10. CAMBER ALI OF SUPPOR (I.E., 3/8 AT N THAN 24'-0" I 11 AT TERMINA	L SPANS BETWEEN TS) FOR L/600 MININ AIDSPAN FOR 18'-0" FOR L/480 (I.E., 3/4" TION OF COLUMN 5	MUM AT MIDSPAN SPAN. CAMBER A AT MIDSPAN FOR	CENTERLINE 1 (WITH L = SPAN ALL SPANS LON 30'-0" SPAN.)	N IN INCHES) NGER
B11 36" 32" 4 #8 42-0 4 #8 16-0 2 #8 22-0			#4	@ 12"oc		CSW8	12"	#5 @ 12"00		#6 @ 12"oc E		12"x24"					-				STANDARD / CENTERLINE	ACI HOOK EACH EN E PER 6/S3.10	ID AT (4) BOTTOM	BARS NEARES	T TO COLUM
B12 36" 32" 36" 32" 4 #8 34-0 2 #8 18-0 2 #8 12-0	• • •	• 	#4	@ 8"oc		CSW9 CSW10	12"	#5 @ 12"oo #5 @ 12"oo	DC EA FACE	#6 @ 12"oc E #6 @ 12"oc E	EA FACE	12"x24" 12"x24"					-			BEAM SCH	<u>EDULE PI</u>	_ACING NOT	<u>ES</u>		
6       #8       16-0         B13       36"       32"       6       #8       28-0				@ 3"oc BTWN GRID		CSW11	8"	#5 @ 12"oc (	CTR IN WALL	#5 @ 12"oc CTF	TR IN WALL	8"x32"					-			<ol> <li>See General N</li> <li>Orientation of b the plan sheet.</li> </ol>	otes (Structural) beams in schedul	on sheet S0.01. le are as seen from th	ne bottom or right of		
DIS     0     1     2     #8     24-0       6     #8     27-0       12     #8     32-0		ALT HOOK DIRECTION		16.2 & GA RMDR @ 8"oc	_	CSW12	8"	#5 @ 12"oc (	CTR IN WALL	#5 @ 12"oc CTF	TR IN WALL	8"x32"								<ol> <li>Center group o on centerline of 55% of longers</li> <li>Top bars sched</li> </ol>	f top bars indicat f support. Stagg span. duled thus	ed thus ————————————————————————————————————	"placed" schedule span. Bar length = face of exterior		
B14 48" 32" 12 #8 26-0 18 #8 32-0 8 #8 32-0		6" CLR FROM BOTT     ALT HOOK DIRECTION     ALT HOOK DIRECTION	(2) #4	@ 12"oc	_	CSW13	12"	#5 @ 12"oo	DC EA FACE	#6 @ 12"oc E	EA FACE	12"x24"					-			5. All lapped top b diameters. Per 66 bar diamters	span past 1/4 po bars shall have a rimeter beams sh s. dicated thus <del>□</del> →	int of span. minimum of lap of 2'- nall have two bars lap ──┼── in "placed sch	-6" or 48 bar ped a minimum of		
B15 42" 32" 8 #8 26-0 14 #8 32-0 8 #8 40-0		6" CLR FROM BOTT     ALT HOOK DIRECTION	#4	@ 8"oc	_	CSW14 CSW15	8"	#5 @ 12"oo	CTR IN WALL	#6 @ 12"oc E	EA FACE	12"x24"								<ul> <li>scheduled bars minimum 2 bar</li> <li>7. Bottom bars sc span.</li> </ul>	s extend 12" past s with a 66 bar d cheduled thus     -	iameter lap.	hin 1/8 point of		
B16 B16R 36" 40" 40" 40" 40" 40" 40" 40" 40" 40" 40		1/2 EA FACE @ 10"oc FROM BOTT ●	#4	@4"oc AT CANTILEVER RMDR @12"oc		CSW16	8"	#5 @ 12"oc (	CTR IN WALL	#5 @ 12"oc CTF	TR IN WALL	8"x32"								<ol> <li>Start stirrups 2'</li> <li>All bars shown within 2" of external 10. No holes, sleev</li> </ol>	" from face of sup thusl to erior face. /es, or conduit la	oport each end unless have standard ACI h rger than 1" diameter	s noted. look. Extend to round shall be put		
B17 36" 40"	•	1/2 AT EA COLUMN 1/2 EA FACE @ 10"oc FROM BOTT	#4	@ 12"oc		CSW17	8"	#5 @ 12"oc (	CTR IN WALL	#5 @ 12"oc CTF	TR IN WALL	8"x32"								through beams conduit shall be 11. Splice length at two bar diamete	e without written a e PVC (non meta t bars of different ers.	authorization from the lic). : sizes shall be based	engineer. All on the larger of the		
B1/R 4 #8 42-0 4 #8 48-0 4 #8 36-0		• • —			_	CSW18	12"	#5 @ 12"oo	DC EA FACE	#6 @ 12"oc E	EA FACE	12"x24"					-			BEAM PL	ACEME	ENT SCHE	EDULE		
B18 36" 40" 40" 40" 40" 40" 40" 40" 400 400 40		1/2 EA FACE @ 10"oc FROM BOTT	#4	@ 12"oc		CSW19 CSW20	8"	#5 @ 12"oo	CTR IN WALL	#6 @ 12"oc E	EA FACE	12"x24"							+				•	© SPAN 1/4 POINT SF	- PAN
4     #8     24-0       4     #8     24-0       4     #8     24-0       4     #8     24-0	• • • • • • • • • • • • • • • • • • •	1/2 EA FACE @ 10"oc FROM BOTT			-	CSW21	12"	#5 @ 12"oo	DC EA FACE	#6 @ 12"oc E	EA FACE	12"x24"					<u>_</u>	M SCHEDULE NOTE #		5% SI	PAN 1	55% LONGER :			JJ SPAN
B19 36" 40" 2 #8 17-0 4 #8 24-0 4 #8 40-0		•	#4	@ 12"oc	_	CSW22	12"	#5 @ 12"oo	DC EA FACE	#6 @ 12"oc E	EA FACE	12"x24"						TOP BEAM		5% OF SP PER 2'-6"	AN				
B20 24" 40" 40" 40" 40" 40" 40" 40" 40" 40" 4			#4	@ 12"oc		CSW23	12"	#5 @ 12"oo	DC EA FACE	#6 @ 12"oc E	EA FACE	12"x24"						BOTTOM BEAM     N(       7     BOTTOM BEAM       YPICAL BOTTOM	OTE 1/8 SPAN				SPAN		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		1/2 FA FACE @ 10"oc FROM BOTT	#4	@ 12"oc		1) PROVIDE BOTTOM 2) VERTICA	: (2)#7 CONT T( BARS 5'-0" AN L FOUNDATIO AL AP INTO WA	OP AND BOTTO D TOP BARS 6 N DOWELS TO	OM OF EACH W 5'-0" WITH 90 DE 9 MATCH VERTI	WALL WITHIN 4" O EG HOOKS AT DIS TCAL WALL REINF	OF THE TOP AND ISTONT ENDS IFORCEMENT SIZ	BOTTOM. SPL	LICE IG WITH					PICAL TOP		PER NOTE 2'-6" MIN					
4     #8     28-0       8     #8     38-0       4     #5     40.0	•			(20) @ 6"oc FA	-	3) TERMINA	TE TOP OF VE	ERTICAL BARS	WITH 90 DEG F	HOOK INTO TOP	OF SLAB WITH (	3" OF CLEAR CC	OVER.					2" EDGE	, 30% OF S WHICHE\	PAN OR CANT DIST. /ER IS GREATER					
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$			#4	END RMDR @ 12"oc	-								00100555	001111110750											
$\begin{array}{c c c c c c c c c c c c c c c c c c c $		1/2 EA FACE @ 10"oc FROM BOTT	#4	@ 12"oc		AN SIZE					REINEORCI	MENT	<ol> <li>CONCRETE</li> <li>PROVIDI</li> <li>ALL COL</li> <li>PROVID</li> <li>VERTICA</li> </ol>	COLUMN NOTES: E (4) SETS OF TIES AT 3 LUMNS TO CENTER ON ( DE VERTICAL FOUNDATION AL REINFORCEMENT WI	"oc TOP & BOT GRIDLINE AND ON DOWELS M	TOM OF EACH COLUMN PIER/FOUNDATION U.N.O. ATCH SIZE AND QUANITY OF									
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		1/2 EA FACE @ 10"oc FROM BOTT	#4	@ 12"oc	82	(32	(8) #7 VE (2) #3 TIE	ERTICAL S @ 8"oc	16	6X32	(10) #8 VEF (2) #3 TIES (	RTICAL @ 16"oc	4) PROVIDE LAP INT	OOK INTO BOTTOM OF F E VERTICAL SLAB DOWE O COLUMN AND 90 DEG	OOTING. ELS AT TOP OF HOOK INTO TO	COLUMN WITH 48 BAR Ø OP OF SLAB ABOVE.									
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		•			12	X24	(8) #7 VE (2) #3 TIEs	ERTICAL S @ 12"oc	24	24X24	(12) #8 VEF (3) #3 TIES (	RTICAL @ 16"oc	·												
B25 36" 40" 40" 40" 40" 40" 40" 40" 40" 40" 40		1/2 EA FACE @ 10"oc FROM BOTT ● ●	#4	@ 12"oc	14.	X34	(10) #8 VE (3) #3 TIE5	ERTICAL S @ 14"oc		24Ø	(10) #8 VEF #3 TIES @	RTICAL 12"oc				REBAR D	DEVELOPME	NT LENGT	H AND L	AP SPLICE	E SCHE	DULE			
B26 36" 40" 40" 8 #8 38-0 40" 40" 40" 40" 400 400 400 400 400 400 400 400 400		<ul> <li>1/2 AT EA COLUMN</li> <li>1/2 EA FACE @ 10"oc FROM BOTT</li> <li>•</li> </ul>	#4	@ 12"oc		•••	3/4" ( (TYF	CHAMFER ?)					СС	ONCRETE S	TRENG	iTH = 5000 psi	CONCF	RETE STRE	NGTH =	4000 psi	CON	NCRETE S	STRENGT	H = 350	)0 psi
B27 36" 40" 40" 6 4 5 2 8 12-0 2 8 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-0 12-		1/2 EA FACE @ 10"oc FROM BOTT      ●	#4	@ 12"oc		<u> </u>	1 1/2" CVR (	CLR TYP)					CASE	E DEVELOP LENGTH CLASS A	MENT I OR . LAP	CLASS B LAP	CASE D	EVELOPMENT LENGTH OR CLASS A LAP	CLA	SS B LAP	CASE	DEVELOF LENGT CLASS	PMENT H OR A LAP	CLASS	BLAP
4     #8     16-0       B28     36"     40"     2     #8     30-0       4     #5     24-0       4     #8     18-0			#4	(18) @ 4"oc RIGHT END		└── VER (2) #3 TIES 32" COLUN	₹T REINF <u>/IN</u> 3/4"		<u> </u>		- 3/4" CHA (TYP)	MFER	BAR SIZE	TOP C BARS	DTHER BARS	TOP OTHER BARS BARS	BAR SIZE B/	OP OTHE ARS BARS	R TOP B BARS	OTHER BARS	BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHEF BARS
B29 36" 40" 7 6 4 #8 18-0		I/2 EA FACE @ 10"oc FROM BOTT		(20) @ 4"oc LEFT END		•	(TYP 1 1/2" CVR (	°) CLR TYP)		(3) #3 TIFS	UVERT RE	) INF	#3 #4 #5	24 24 28	24 24 24	24     24       29     24       36     28	#3 #4 #5	24     24       25     24       31     24	24 33 41	24 25 31	#3 #4 #5	24 27 33	24 24 26	26 35 43	24 27 33
2     #8     12-0       4     #8     16-0       4     #8     36-0	• <b>Г</b>			RMDR @12"oc           12"oc AT		(2) #3 TIES	VERT REINF		<u>14"x34</u>	4" COLUMN	/- 3/4" CHAMFER (TYP)	र	#6 #7	34 49	26 38	43         34           63         49	#6 #7	37         29           54         42	49 71	37 54	#6	40 58	31 45	52 75	40 58
B30   48"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"   40"		<ul> <li>1/2 EA FACE @ 10"oc FROM BOTT</li> <li>6" CLR FROM BOTT</li> </ul>	(2) #4	CANTILEVER (10) @ 6"oc EA. END BTWN COLS BMDD @ 10"-c	8"x32" &	12"x24" CO	<u>PLUMN</u>	N R			1 1/2" CLR 0 CVR (TYP)		#8 #9	56 63	43 48	72     56       81     63	#8 #9	62 48 70 54	81	62 70	#8 #9	66 75	51 58	86 97	66 75
16     #8     36-0       48"     40"     8     #8     36-0       48"     40"     4     45     35-0		ALI HOOK DIRECTION     ALT HOOK DIRECTION     1/2 EA FACE @ 10"oc FROM BOTT	(2) #4	(10) @ 6"oc EA END EMDE @ 40"		• *	CVR (T	YP)					#10 #11	71 78	54 60	92     70       102     78	#10 #11	79     61       87     67	102 113	79 87	#10 #11	84 93	65 72	109 121	84 93
10     #8     32-0       16     #8     36-0		<ul> <li>6" CLR FROM BOTT</li> <li>ALT HOOK DIRECTION</li> </ul>		ССС КМDR @12"ос				E EINF		(3) #3 TIFS	RT REINF		NOTES: 1. UNLES			ISE, USE THE MINIMUM LENGTI	TH FOR A CLASS B LAP S	PLICE OR THE MINIMU	JM DEVELOPMEN	T LENGTH INDICATE	D IN THE TABL	ES ABOVE MULTIPL	LIED BY THE		

**24"Ø COLUMN 24"x24" COLUMN 7 CONCRETE COLUMN DETAILS 3**/4" = 1'-0"

APPLICABLE FACTOR(S) LISTED BELOW.
 WHERE THE CLEAR SPACING BETWEEN BARS LAP SPLICED OR EMBEDDED AT ANY SECTION IS LESS THAN 2 BAR DIAMETERS, OR WHERE THE BAR COVER IS LESS THAN OR EQUAL TO THE BAR DIAMETER. INCREASE THE INDICATED BAR SPLICE OR DEVELOPMENT LENGTH BY 50%.
 TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
 MECHANICAL COUPLERS MAY BE SUBSTITUTED FOR TENSION LAP SPLICED BARS PROVIDED THAT THEY MEET THE REQUIREMENTS OF ACI 318-11, 12.14.
 AT LOCATIONS WHERE REINFORCING WITHIN A STRUCTURAL ELEMENT WILL BE SPLICED, ALTERNATING SPLICES SHALL BE STAGGERED A MINIMUM OF THE CLASS B SPLICE LENGTH UNLESS INDICATED OTHERWISE.

![](_page_16_Picture_11.jpeg)

![](_page_16_Figure_12.jpeg)

SHEET TITLE

![](_page_16_Figure_14.jpeg)

CIVIL GBA ENGINEERS	;
LANDSCAPE LAND 3	
STRUCTURAL BOB D. CAMPBEI	
PLUMBING LATIMER SOMME	ĒF
MECHANICAL LATIMER SOMME	ΞF
ELECTRICAL LATIMER SOMME	ΞF
FIRE PROTECTION LATIMER SOMME	ΞF
CONTRACTOR BRINKMANN CONSTRUCTORS	5

OF MISS BEVERLIN NIMBE

PROJECT TEAM

ARCHITECT

FINKLE+WILLIAMS ARCHITECTURE

![](_page_16_Picture_17.jpeg)

\_\_\_\_\_ \_\_\_\_

Project No.: 18017,19050.07,19050.08 Date: 10.18.24 Issued For: GARAGE PERMIT REVISIONS Description No. Date 2 7.11.22 ADDENDUM 1 \_\_\_\_\_ \_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ · \_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_

# PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

STO STO paragon star

RELEASED FOR

CONSTRUCTION As Noted on Plans Review Development Services Department Lee's Summit, Missouri 11/25/2024

) psi B LAP OTHER BARS 24 27 33 40 58 66 75 84 93

![](_page_17_Figure_0.jpeg)

![](_page_18_Figure_0.jpeg)

![](_page_19_Figure_0.jpeg)

![](_page_20_Figure_0.jpeg)

![](_page_21_Figure_0.jpeg)

![](_page_22_Figure_0.jpeg)

![](_page_23_Figure_0.jpeg)

![](_page_23_Figure_1.jpeg)

![](_page_24_Figure_0.jpeg)

![](_page_24_Picture_7.jpeg)

![](_page_25_Figure_0.jpeg)

PILE CAP SCHEDULE										
PILE CA 300 @ 18"Ø	APACTY (KIPS): 9 PILE / 600 @ 14"Ø	CONCRETE 3500	(PSI):	REBAR (KSI): 60						
TYPE	FOOTING S THICKNES	SIZE (FT.) SS (IN.)	QTY/SIZE OF BARS EACH WAY							
3.5	3'-6" x 3'-6" x 30"	w/ 14"Ø PILE	#5 @ 6"oc BOTTOM / #5 @ 12"oc TOP							
9x4.5	9'-0" x 4'-6'	" x 36"	#6 @ 6"oc BOTTOM / #6 @ 12"oc TOP							
9	9'-0" x 9'-0'	" x 36"	#7 @ 6"oc BOTTOM / #7 @ 12"oc TOP							
9x13.5	9'-0" x 13'-6	60" x 60"	#7 @ 6"oc BOTTOM / #7 @ 12"oc TOP							
12.5x13.5	12'-6" x 13'-	6" x 54"	#7 @ 6"oc BOTTOM / #7 @ 12"oc TOP							

![](_page_25_Picture_6.jpeg)

![](_page_25_Picture_7.jpeg)

EL. TOP OF PILE CAP PER PLAN

- 18"Ø AUGER CAST PRESSURE GROUTED PILES (RE: 1/S3.02)

![](_page_25_Picture_10.jpeg)

SHEET NUMBER S3.02

PILE & PODIUM

FOUNDATION

DETAILS

![](_page_26_Figure_0.jpeg)

-WI Projects\FWI2101 - Paragon Star\FWI2101 Dwgs\FWI2101 - S21 - Paragon Star\_jlf.rvt

![](_page_27_Figure_0.jpeg)

![](_page_27_Figure_2.jpeg)

TYPICAL BOTTOM CMU TO PRECAST

9 <u>SECTION</u> 3/4" = 1'-0"

T/WALL PER ARCH

HSS PER PLAN

HOUSING

- PL5/16"x18"xCONT.

- OVERHEAD DOOR PER ARCH

![](_page_27_Picture_8.jpeg)

NFPA SYMBOLS LEGEND		
Ø	SMOKE DETECTOR	
Øs	SMOKE DETECTOR WITH SOUNDER BASE	
() ISO	SMOKE DETECTOR WITH ISOLATOR BASE	
	HEAT DETECTOR	
$\square$	DUCT DETECTOR	
·	ADDRESSABLE MANUAL PULL STATION	
Ŷ	DOOR HOLDER	
۶Å	FLOW DETECTOR/SWITCH	
۶ç	TAMPER DETECTOR	
[]	TEST STATION	
R MR101/C SHUTDOWN RELAY, SPDT W/RED		
$\boxtimes\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!$	A/V (WALL MOUNTED) 24 VDC	
×	STROBE	
Ŷ	BELL ANNUNCIATOR	
Ŋ	HORN/SPEAKER	
[FCP]	FIRE ALARM CONTROL PANEL	
C	FIREMAN'S PHONE	
[ARA]	AREA RESCUE CALL STATION	
[ARA] M	AREA RESCUE MASTER STATION	
ZAM S	SIGNAL ZAM	
[ZAM] <sub>C</sub>	CONTROL ZAM	
[ZAM] DET	DETECTOR ZAM	
IAM	MONITOR MODULE	
[IAM] R	RELAY IAM	
PC	GRAPHIC COMMAND CENTER	
[FAA]	REMOTE FIRE ALARM AUDIO	
[FSA]	REMOTE ANNUNCIATOR WITH AUDIO	
ANN	ANNUNCIATOR	
-[FS]-	FIRE SMOKE DAMPER	
NAC	NAC POWER EXTENDER	

SYMBOL	DESCRIPTION	REMARKS
D201/A	TELECOMMUNICATIONS OUTLET WITH ROOM AND TYPE IDENTIFIER	1
W	TELECOMMUNICATIONS OUTLET WALL PHONE PLATE	2
<b>V</b> AV	AUDIO/VISUAL OUTLET	3
TV	TELEVISION OUTLET	1
	EMT CONDUIT BY E/C (1 1/4" UNLESS NOTED OTHERWISE)	4
-()	EMT SLEEVE BY E/C (2" UNLESS NOTED OTHERWISE)	4
AFF	ABOVE FINISHED FLOOR	
T/C	TELECOMMUNICATIONS CONTRACTOR	
E/C	ELECTRICAL CONTRACTOR	
G/C	GENERAL CONTRACTOR	
AC	DEVICE LOCATED ABOVE COUNTER	
TMGB	TELECOMMUNICATIONS MAIN GROUNDING BUSBAR	
TGB	TELECOMMUNICATIONS GROUNDING BUSBAR	
	TELECOMMUNICATIONS CABLING	5
FACP	FIRE ALARM CONTROL PANEL	
SM	SINGLEMODE FIBER	
ММ	MULTIMODE FIBER	
WAP	WIRELESS ACCESS POINT	6
TELECON	MUNICATIONS CABLING IDENTIFIER	

AND 1 1/4" CONDUIT TO ABOVE ACCESSIBLE CEILING AS INDICATED ON DRAWINGS BY E/C.
2 - 2x4 BACKBOX WITH 3/4" CONDUIT TO ABOVE ACCESSIBLE CEILING.
3 - 4x4 STEEL CITY BACKBOX, MODEL NUMBER 72171-1-1/4 WITH DOUBLE GANG PLASTER RING BY E/C. CONDUITS AS INDICATED ON PLANS.
4 - E/C TO PROVIDE CONDUIT BUSHING ON CONDUIT PRIOR TO T/C INSTALLING CABLING.
5 - CABLING SHALL BE SUPPORTED WITH J-HOOKS AT 48" O.C. WHERE NOT IN CONDUIT.
6 - 2x4 SURFACE MOUNT BACKBOX LOCATED ABOVE ACCESSIBLE CEILING.

	WATER CLOSET & TYPE (TYP. FOR ALL PLUMBING FIXTURES)	— снs —	CHILLED HOT SUPPLY
-+	WASTE LINE ABOVE EARTH (W.)	— CHR—	CHILLED HOT RETURN
	WASTE LINE IN EARTH (W.)		UNION
<b>-II</b> co	CLEAN OUT		FLEXIBLE PIPE CONNECTION
00 <b>0</b>	FLUSH FLOOR CLEAN OUT		MANUAL DAMPER
co <b>o</b>	FLUSH GRADE CLEAN OUT		BACKDRAFT DAMPER
1 <u>}</u> ₽	FLOOR DRAIN AND TYPE		AUTOMATIC DAMPER
RD —	ROOF DRAIN		FIRE DAMPER
DRD —	OVERFLOW ROOF DRAIN	FS	FIRE/SMOKE DAMPER
	ROOF DRAIN AND TYPE		SMOKE DAMPER
	VENT LINE (V.)	6x6 A 🛛	GRILLE, REGISTER OR DIFFUSER, SIZE, TYPE & CFM
	DOMESTIC COLD WATER SUPPLY (DCW)		VOLUME EXTRACTOR AND TURNING VANES
	DOMESTIC HOT WATER SUPPLY (DHW)		RETURN, EXHAUST OR FRESH AIR DUCT SECTION UP & DOWN
	DOMESTIC HOT WATER RETURN (DHWR)	$\boxtimes$	SUPPLY AIR DUCT SECTION UP AND DOWN
• HB/36"	HOSE BIBB AND MOUNTING HEIGHT		FLEXIBLE DUCT CONNECTION
EI WH	WALL HYDRANT		ROUND OR RECTANGULAR DUCT
- F ——	FIRE LINE/STANDPIPE		FLEXIBLE DUCT
D —	DRAIN LINE	φ	THERMOSTAT
- G —	NATURAL GAS LINE	— R —	REFRIGERANT LIQUID/SUCTION
안 년	RISE & DROP IN PIPE WITH CUT-OFF VALVE	AD	ACCESS DOOR
<b>→</b>	REDUCER	AFF	ABOVE FINISHED FLOOR
	CHECK VALVE	EA	EXHAUST AIR
⋈—	STOP VALVE	OA	OUTSIDE AIR
肉——	BALANCING VALVE/AUTOFLOW VALVE	RA	RETURN AIR
₩—	PLUG VALVE	SA	SUPPLY AIR
逸——	2-WAY CONTROL VALVE OR SOLENOID VALVE	VBS	VENT BELOW SLAB
&—	3-WAY CONTROL VALVE OR SOLENOID VALVE	VTR	VENT THRU ROOF
<u> </u>	PRESSURE REDUCING VALVE	•	CONNECT NEW TO EXISTING
	STRAINER		LOCKABLE GUARD
cws—	CHILLED WATER SUPPLY	VFD	VARIABLE FREQUENCY DRIVE
WR—	CHILLED WATER RETURN		
-ws—	HOT WATER SUPPLY		
-IWR-	HOT WATER RETURN		

ELECTRICAL SYMBOLS LEGEND				
$\frown$	CONDUIT CONCEALED IN CEILING OR WALL. 2 HOT + GROUND.	φ	THERMOSTAT	
$\langle - \rangle$	CONDUIT CONCEALED IN FLOOR SLAB		LOCKABLE GUARD	
	EXPOSED CONDUIT		JUNCTION BOX	
	HOMERUN - ARROW INDICATES CKT., LINES INDICATE WIRES	\$	SWITCH - SINGLE POLE	
	GROUNDING ROD	\$	SWITCH - 3-WAY	
φ	SINGLE RECEPTACLE	\$ 4	SWITCH - 4-WAY	
ø	DUPLEX RECEPTACLE (20 AMP UNLESS NOTED)		LIGHT FIXTURE AND TYPE	
<b>φ</b> υ	DUPLEX RECEPTACLE WITH USB OUTLETS	X	EMERGENCY LIGHT FIXTURE WITH BATTERY PACK	
<b>þ</b> sw	SWITCHED DUPLEX RECEPTACLE	$\square \bullet$	FIXTURE ON LIFE SAFETY BRANCH OF EMERGENCY SYSTEM	
#	FOURPLEX RECEPTACLE	<b>с</b> н П	LIGHT FIXTURE (WALL MOUNTED)	
ф	208 OR 240 VOLT RECEPTACLE (20 AMP UNLESS NOTED)	$\otimes$ ଚ୍ୟ	EXIT LIGHT (CEILING OR WALL MOUNTED)	
Ø	GROUND FAULT INTERRUPTER (GFI) DUPLEX RECEPTACLE		FLUSH PANELBOARD (LIGHT & RECEPTACLES)	
▼	TELE/DATA OUTLET *		SURFACE PANELBOARD (LIGHT & RECEPTACLES)	
Б	PUSHBUTTON		DISTRIBUTION PANEL OR SWITCHBOARD	
[VFD]	VARIABLE FREQUENCY DRIVE	AC	DEVICE LOCATED ABOVE COUNTER	
ORT	OVERRIDE TIMER	AFF	ABOVE FINISHED FLOOR	
[PC]	PHOTOCELL	D	DIMMER	
6	MOTOR	E	INDICATES EXISTING DEVICE	
\$	FUSIBLE SWITCH (BUSSMAN SSU)	EDF	ELECTRIC DRINKING FOUNTAIN	
-	DISCONNECT SWITCH (D.S.)	NL	NIGHTLIGHT FIXTURE, WIRED HOT	
4⊠	COMBINATION MOTOR STARTER (CMS)	WP	WEATHERPROOF	
[R]	RELAY	AFCI	ARC FAULT CIRCUIT INTERRUPTER	
00	FLOOR BOX	•	CONNECT NEW TO EXISTING	
DTES: ALL SYMBOLS SHOWN ABOVE REFER TO MECHANICAL SYMBOLS LEGEND FOR MECHANICAL * 4x4 BACKBOX WITH SINGLE GANG PLASTER RING AND A SYMBOLS THAT MAY BE SHOWN ON FLECTRICAL PLANS 11/4" CONDUIT TO ABOVE ACCESSIBLE CEUING				

![](_page_28_Picture_10.jpeg)

RELEASED FOR CONSTRUCTION As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 11/25/2024

\_\_\_\_\_

### 3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.:		18017,19050.07,19050.08
Date:		10.18.2024
Issued For:		GARAGE PERMIT SET
		REVISIONS
No.	Date	Description

REGISTRATION

![](_page_28_Picture_14.jpeg)

PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

![](_page_28_Picture_16.jpeg)

COVER SHEET

SHEET NUMBER

ME-000

SHEET TITLE

![](_page_29_Figure_1.jpeg)

NORTH

NOTES	δ:
1.	COORDINATE WITH CIVIL PLANS FOR EXACT LOCATIONS OF UTILITIES, SITE FEATURES AND GRADE.
2.	COORDINATE WITH UTILITY PROVIDERS, INCLUDE ALL REQUIREMENTS AND FEES WITHIN THE BID WORK. IF NO FEE IS AVAILABLE, PROVIDE AN ALLOWANCE AND LIST ON BID SUBMISSION.
3.	TRANSFORMER LOCATIONS ARE AS SUGGESTED AND PREFERRED. POWER COMPANY WILL MAKE FINAL DETERMINATION.
4.	FOR ANY GAS SERVICES, PROVIDE STEEL RISER, SHUT- OFF AND APPROPRIATE PRV AT APPLIANCES IF APPROPRIATE.

ALL SITE LIGHTING IS #10 WIRE IN 1" MIN. PVC CONDUIT AND ROUTE THROUGH PHOTOCELL/TIMER.

 $\langle 2 \rangle$  water service entrance - see civil.

 $\langle 3 \rangle$  SANITARY SEWER EXIT POINT - SEE CIVIL.

![](_page_29_Picture_9.jpeg)

![](_page_30_Figure_0.jpeg)

![](_page_30_Figure_1.jpeg)

![](_page_30_Figure_2.jpeg)

![](_page_31_Figure_0.jpeg)

![](_page_32_Figure_0.jpeg)

![](_page_33_Figure_0.jpeg)

![](_page_34_Figure_0.jpeg)

![](_page_35_Figure_0.jpeg)

![](_page_35_Figure_3.jpeg)

![](_page_35_Figure_4.jpeg)

![](_page_36_Figure_0.jpeg)

![](_page_36_Figure_2.jpeg)

NORTH

NOTE	S:
1.	COORDINATE WITH OTHER SUB-CONTRACTORS FO PLACEMENT OF WORK PRIOR TO INSTALLATION BEGINNING.
2.	SEE FIRE SUPPRESSION SUBMITTALS AND INCORPO ALL DEVICES INTO FIRE ALARM.
3.	EXIT LIGHTS SHALL BE VISIBLE TO OCCUPANTS.
4.	PROVIDE NEC CLEARANCES FOR ALL PANELS AND ELECTRICAL EQUIPMENT.
5.	LABEL ALL JUNCTION BOXES AS TO THE PANEL AND CIRCUIT NUMBER SERVED.
6.	PANEL DIRECTORIES SHALL BE SPECIFIC TO THE ROOMS/EQUIPMENT SERVED.
LEGE	ND:

![](_page_36_Picture_4.jpeg)

CONSTRUCTION As Noted on Plans Review
Development Services Department Lee's Summit, Missouri 11/25/2024

RELEASED FOR

# PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.:		18017,19050.07,19050.08
Date:		10.18.2024
Issued For:		GARAGE PERMIT SET
		REVISIONS
No.	Date	Description

REGISTRATION

![](_page_36_Picture_10.jpeg)

PROJECT TEAM			
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE		
CIVIL	GBA ENGINEERS		
LANDSCAPE	LAND 3		
STRUCTURAL	BOB D. CAMPBELL		
PLUMBING	LATIMER SOMMERS		
MECHANICAL	LATIMER SOMMERS		
ELECTRICAL	LATIMER SOMMERS		
FIRE PROTECTION	LATIMER SOMMERS		
CONTRACTOR	BRINKMANN CONSTRUCTORS		

![](_page_36_Picture_12.jpeg)

![](_page_36_Picture_13.jpeg)

SHEET NUMBER

E1.71G

![](_page_37_Figure_0.jpeg)

![](_page_37_Figure_2.jpeg)

![](_page_37_Picture_3.jpeg)

NOR	TH VILLAGE
3200 N LEE'S	W PARAGON PKWY SUMMIT, MO 64081
Project No.: Date: Issued For:	18017,19050.07,19050.08 10.18.2024 GARAGE PERMIT SET
No. Date	REVISIONS Description
10,	REGISTRATION OF MISSO RICHARD R BEARDMORE NUMBER SONAL ENVIRONMENT VONAL ENVIRONMENT

PARAGON STAR

**RELEASED FOR** CONSTRUCTION As Noted on Plans Review

elopment Services Depart Lee's Summit, Missouri 11/25/2024

PROJECT TEAM								
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE							
CIVIL	GBA ENGINEERS							
LANDSCAPE	LAND 3							
STRUCTURAL	BOB D. CAMPBELL							
PLUMBING	LATIMER SOMMERS							
MECHANICAL	LATIMER SOMMERS							
ELECTRICAL	LATIMER SOMMERS							
FIRE PROTECTION	LATIMER SOMMERS							
CONTRACTOR	BRINKMANN CONSTRUCTORS							

![](_page_37_Picture_6.jpeg)

![](_page_37_Picture_7.jpeg)

E1.72G

![](_page_38_Figure_0.jpeg)

NOTE	S:
1.	COORDINATE WITH OTHER SUB-CONTRACTORS FO PLACEMENT OF WORK PRIOR TO INSTALLATION BEGINNING.
2.	SEE FIRE SUPPRESSION SUBMITTALS AND INCORPO ALL DEVICES INTO FIRE ALARM.
3.	EXIT LIGHTS SHALL BE VISIBLE TO OCCUPANTS.
4.	PROVIDE NEC CLEARANCES FOR ALL PANELS AND ELECTRICAL EQUIPMENT.
5.	LABEL ALL JUNCTION BOXES AS TO THE PANEL AND CIRCUIT NUMBER SERVED.
6.	PANEL DIRECTORIES SHALL BE SPECIFIC TO THE ROOMS/EQUIPMENT SERVED.
LEGE	ND:
$\langle 1 \rangle$ F	FUTURE EV CHARGING LOCATION PROVIDE BOX AND

![](_page_38_Picture_3.jpeg)

# PARAGON STAR NORTH VILLAGE 3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081 Project No.: 18017,19050.07,19050.08 10.18.2024 Issued For: GARAGE PERMIT SET REGISTRATION 10/18/2024 PROJECT TEAM

CONSTRUCTION As Noted on Plans Review

Iopment Services Dep Lee's Summit, Missouri 11/25/2024

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
ANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

![](_page_38_Picture_6.jpeg)

![](_page_38_Picture_7.jpeg)

SHEET NUMBER

E1.73G

![](_page_39_Figure_0.jpeg)

![](_page_39_Figure_2.jpeg)

![](_page_39_Picture_3.jpeg)

		As Noted on Plans Review								
		Development Services Department Lee's Summit, Missouri 11/25/2024								
	PARAGON STAR									
PA	RAC	JUN STAR								
PA NO	RT	I VILLAGE								
PA NO	RT	I VILLAGE								
PA NO 3200		VILLAGE								
PA NO 3200 LEE	RAC RTH	ARAGON PKWY								
PA NO 3200 LEE	RAC RTH	ARAGON PKWY								
PA NO 3200 LEE	RAC RTH 0 NW F 5'S SUM	ARAGON PKWY MMIT, MO 64081								
PA NO 3200 LEE Project N Date:	RAC RTH 0 NW P 5'S SUN 10.: 180	ARAGON PKWY MMIT, MO 64081								
PA NO 3200 LEE Project N Date: Issued Fe	RAC RTH 0 NW F 2'S SUN 10.: 180 10.1 0r: GA	PARAGON PKWY MMIT, MO 64081 017,19050.07,19050.08 8.2024 RAGE PERMIT SET								
PA NO 3200 LEE Project N Date: Issued Fe	RAC RTH 0 NW F 5'S SUN 10.1 10.1 0r: GA	PARAGON PKWY MMIT, MO 64081 017,19050.07,19050.08 8.2024 RAGE PERMIT SET								
PA NO 3200 LEE Project N Date: Issued Fo	RAC RTH 0 NW F 5'S SUN 10.: 180 10.1 0r: GA RI ate	PARAGON PKWY MMIT, MO 64081 017,19050.07,19050.08 8.2024 RAGE PERMIT SET EVISIONS Description								
PA NO 3200 LEE Project N Date: Issued Fo	RAC RTH 0 NW P 2'S SUN 10.: 180 10.1 0r: GA RI ate	PARAGON PKWY MMIT, MO 64081 017,19050.07,19050.08 8.2024 RAGE PERMIT SET EVISIONS Description								
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PA NO 3200 LEE Project N Date: Issued Fo	RAC RTH 0 NW P 2'S SUM 10.1 10.1 10.1 0r: GA RI ate	PARAGON PKWY MMIT, MO 64081 017,19050.07,19050.08 8.2024 RAGE PERMIT SET EVISIONS Description								
PA NO 3200 LEE Project N Date: Issued Fo	RAC RTH 0 NW P 2'S SUM 10.1 10.1 10.1 0r: GA RI ate  	PARAGON PKWY MIT, MO 64081 017,19050.07,19050.08 8.2024 RAGE PERMIT SET EVISIONS Description								

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10/18/2024

REGISTRATION

PROJE	CT TEAM
ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS
MECHANICAL	LATIMER SOMMERS
ELECTRICAL	LATIMER SOMMERS
FIRE PROTECTION	LATIMER SOMMERS
CONTRACTOR	BRINKMANN CONSTRUCTORS

![](_page_39_Picture_7.jpeg)

![](_page_39_Picture_8.jpeg)

E1.74G

![](_page_40_Figure_0.jpeg)

![](_page_40_Figure_2.jpeg)

	RELEASED FOR CONSTRUCTION As Noted on Plans Review
	Development Services Department Lee's Summit, Missouri 11/25/2024
L	
PARAC	ONSTAR
NORTH	VILLAGE
3200 NW PA	ARAGON PKWY
LEE'S SUM	IMIT, MO 64081
Project No.: 1801	7,19050.07,19050.08
Issued For: GAR	AGE PERMIT SET
RE'	VISIONS
<u>No.</u> <u>Date</u>	Description
REGI	STRATION
	111111-1-
A Second Street Second	MISS
BEAU BEAU	
A CARACTERS	WAL FUILIN
10/18	3/2024
	ARCHITECTURE
CIVIL	GBA ENGINEERS
LANDSCAPE	LAND 3
STRUCTURAL	BOB D. CAMPBELL
PLUMBING	LATIMER SOMMERS

& Associates P.A ONSULTING ENGINEERS 3639 SW Summerfield Drive, Suite A Topeka, Kansas 6614-3974 8625 College Boulevard, Suite 102 Overland Park, Kansas 66210 Telephone: (785) 233-3232 Email: Isapa@Isapa.com LSA PROJECT NO. 2104043

FIRE PROTECTION LATIMER SOMMERS

MECHANICAL

ELECTRICAL

CONTRACTOR

LATIMER SOMMERS

LATIMER SOMMERS

BRINKMANN CONSTRUCTORS

![](_page_40_Picture_5.jpeg)

SHEET NUMBER

E1.75G

![](_page_41_Figure_0.jpeg)

![](_page_41_Figure_2.jpeg)

![](_page_41_Figure_4.jpeg)

![](_page_41_Figure_6.jpeg)

# 2 EXTERIOR LIGHTING CONTROL PANEL SCHEMATIC NOT TO SCALE

![](_page_41_Figure_8.jpeg)

![](_page_41_Picture_9.jpeg)

5/2024 3:47:04 PM PARAGON STAR NORTH VILLAGE

![](_page_42_Figure_1.jpeg)

# 2 BUILDING B/C ELECTRICAL RISER DIAGRAM NOT TO SCALE

SWITCHGEAR LOAD ANALYSIS GARAGE											
Total Units	Total Connected	NEC Building Diversity	Diversified KVA	Amps @ 208V/3PH	House Load	Retail Load	Total Demand (Amps)				
121	3,802,320 VA	0.23	875 VA	2429	560	0	2989				
	Total Units 121	Total Units     Total Connected       121     3,802,320 VA	Total Units       Total Connected       NEC Building Diversity         121       3,802,320 VA       0.23	SWITCHGEAR LOAD AN         Total Units       Total Connected       NEC Building Diversity       Diversified KVA         121       3,802,320 VA       0.23       875 VA	SWITCHGEAR LOAD ANALYSIS GARAGE         Total Units       Total Connected       NEC Building Diversity       Diversified KVA       Amps @ 208V/3PH         121       3,802,320 VA       0.23       875 VA       2429	SWITCHGEAR LOAD ANALYSIS GARAGE         Total Units       Total Connected       NEC Building Diversity       Diversified KVA       Amps @ 208V/3PH       House Load         121       3,802,320 VA       0.23       875 VA       2429       560	SWITCHGEAR LOAD ANALYSIS GARAGE         Total Units       Total Connected       NEC Building Diversity       Diversified KVA       Amps @ 208V/3PH       House Load       Retail Load         121       3,802,320 VA       0.23       875 VA       2429       560       0				

Multi-Family Building Load Analysis GARAGE									
Meter Bank Total Units Total Connected Load Total KVA NEC Building Diversity Diversified KVA Amps @ 208V/3PH House Load Total Demand -									
		Sum of Units	Total IVVV		Diversified IVVA	7(11)5 @ 2001/0111	TIOUSC LOUG	Total Domand - Amps	
Office Type	Onit Quantity								
METER BANK G2W	42	1,316,085 VA	1316	28%	369	1024	0	1024	
A-1 ADA	2	57780 VA							
B-1	6	175140 VA							
C-1	17	536180 VA							
C-1S	1	31540 VA							
C-3	9	282645 VA							
C-5	2	63530 VA							
D-3	2	65360 VA							
D-4	1	32830 VA							
D-10	2	71080 VA							
			•						
METER BANK G3W	44	1,401,565 VA	1402	27%	378	1051	0	1051	
A-1 ADA	2	57780 VA		•					
B-1	6	175140 VA							
C-1	16	504640 VA							
C-1S	2	63080 VA							
C-3	9	282645 VA							
C-5	2	63530 VA							
D-3	1	32680 VA							
D-4	2	65660 VA							
D-10	1	35540 VA							
PH-3	1	40290 VA							
PH-4	1	39690 VA							
PH-5	1	40890 VA							
METER BANK G4E	35	1,084,670 VA	1085	30%	325	904	0	904	
A-1	12	346680 VA							
C-1S	16	504640 VA							
C-12	4	127180 VA							
D-5	3	106170 VA							

	FEEDER SCHEDULE										
MARK	OCP	SIZE GRD. CU	CONDUIT								
1	100 A	1	4	#1/0	#4	1 1/2"					
2	225 A	1	4	300 KCMIL	#2	3"					
3	400 A	2	4	#4/0	#1	2"					
4	600 A	2	4	500 KCMIL	#2/0	3"					
5	1000 A	3	4	600 KCMIL	#4/0	4"					
6	1200 A	4	4	500 KCMIL	250 KCMIL	3"					
7	1600 A	5	4	600 KCMIL	350 KCMIL	4"					
9	800 A	3	4	400 KCMIL	#3/0	3"					
10	2000 A	6	4	600 KCMIL	400 KCMIL	4"					

	Garage Luminaire Schedule											
MARK	DESCRIPTION	MFGR	MODEL	MOUNTING	FINISH	LAMPS	NOTES					
Α	Garage Light	Royal Pacific	44310B-60-MS	surface	standard	60W 4000K	with 50% motion dimming					
A1	Garage Light	Royal Pacific	4431OB-60-MS	surface	standard	60W 4000K	with 50% motion dimming/batter					
A2	Surface Globe	Lithonia	LDN4CYL-40/LO4/AR/LSS/fcm	surface	white	1000 Lumen 4000K 10W						
В	Strip	Lithonia	MNSL-L46 1LL MVOLt 40k	surface	white	20W 4000K LED	with battery					
С	Double head Pole	McGraw Edison	(2) GLEON-SA2C-740-U-5WQ	jb/surface	white	15,200 Lumen 4000K 115W LED	16 ft pole					
X1	Exit	Lithonia	LQM SW3R	surface	white/red	LED						
X2	Exit - waterproof	Lithonia	WLTE-W-1R-EL	surface	white/red	LED						

Panel: GARAGE Location: GARAGE Supply From: Mounting: SURFACE Enclosure:					Volts: 120/208 Wye Phases: 3 Wires: 4					A.I.C. Rating: 10000 Mains Type: MLO Mains Rating: 225 A MCB Rating:				
Notes	:													
скт	Circuit Description	Trip	Wire Size		4		В			Wire Size	Trip	Circu	it Description	СК
1	•	· ·		1321	1820					12	20 A	Liahtina	•	2
3	GARAGE EF	15 A	12			1321	1600			12	20 A	Lighting		4
5								1321	1680	12	20 A	Lighting		6
7				1321	1752					12	20 A	Lighting		8
9	GARAGE EF	15 A	12			1321	600			12	20 A	Lighting		10
11								1321	833	12	20 A	Lighting		12
13				1321								5 5		14
15	GARAGE EF	15 A	12			1321	800			12	20 A	CO SENSO	R	16
17								1321	800	12	20 A	CO SENSO	R	18
19				0	800					12	20 A	CO SENSO	R	20
21	Power Space 1305	20 A	12			0	296			12	20 A	Receptacle		22
23								0	2704	_	05.4			24
25					2704					8	35 A	EOH-B		26
27							1995			12	20 A	Lighting ST	AIR 106	28
29														30
31														32
33														34
35														36
37														38
39														40
41														42
		Tota	I Load:	1092	9 VA	9184	4 VA	9912	2 VA					
		Total	Amps:	92	A	77	' A	84	A					
Leger	nd:		Connec		d D	mand F	actor	Fetim	natod			Panol	Totals	
HVAC			172	97 VA		65 00%	,	1124	13 VA			1 41101		
Lightin	na		865	52 VA		125 009	2 //	108	15 VA		Total	Conn. Load:	30019 VA	
Other	.9		240	0 VA		100.009	% %	240	0 VA		Total E	st. Demand:	26119 VA	
Power			50	0 VA		100.009	/o	500	) VA			Total Conn.:	83 A	
Recen	tacle		144	0 VA		100.009	%	144	0 VA		Total E	st. Demand:	72 A	
-				-					-					
Notes	:				1									

![](_page_42_Picture_8.jpeg)