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PROJECT TEAM

ARCHITECTFINKLE + WILLIAMS ARCHITECTURE 8787 Renner Blvd., Suite 100 Lenexa, Kansas 66219 PH. 913.498.1550

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LANDSCAPE

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STRUCTURAL **ELECTRICAL** BOB D. CAMPBELL & CO., INC. LS&A, P.A. 4338 Belleview Kansas City, Missouri 64111

PRECAST CONTRACTOR
CORESLAB STRUCTURES, INC. 759 S. 65th St.

8625 College Blvd., Suite 102 Overland Park, Kansas 66210 PH. 785.233.0647 F. 785.233.0647 PH. 816.531.4144 F. 816.531.8572

PLUMBING LS&A, P.A.

FIRE PROTECTION

CONTRACTOR
BRINKMANN CONSTRUCTORS

11101 Switzer Rd., Suite 310 Overland Park, Kansas 66210 PH. 913.717.9007 F. 913.717.9407

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PH. 785.233.0647 F. 785.233.0647



INTERIOR SIGNAGE SCHEDULE GARAGE

DOOR NO.	ROOM NAME	SIGN TYPE
103.A	WATER SERVICE	TYPE 'B'
105.A	STAIR	TYPE 'A'
206.A	STAIR	TYPE 'C'
206.B	STAIR	TYPE 'C'
306.A	STAIR	TYPE 'C'
406.A	STAIR	TYPE 'C'
506.A	STAIR	TYPE 'C'

- TACTILE & BRAILE CHARACTERS
 a. CHARACTERS SHALL BE RAISED
- a. CHARACTERS SHALL BE RAISED MINIMUM 1/32"b. CHARATERS SHALL BE ACCOMPANIED BY GRADE 2 BRAILLE
- 2. <u>TYPESTYLES</u> a. CHARACTER
- a. CHARACTERS SHALL BE UPPER CASE & SANS SERIF OR SERIF TYPESTYLE
 b. CHARACTERS SHALL BE A MINIMUM OF 5/8" HIGH AND MAXIMUM 2" HIGH
- 3. <u>PICTOGRAMS (SYMBOLS)</u>
- a. PICTOGRAMS SHALL BE ACCOMPANIED BY THE EQUIVALENT VERBAL DESCRIPTION PLACED DIRECTLY BELOW THE PICTORGRAM AS INDICATED.
 b. THE BORDER DIMENSION OF THE PICTORGRAM SHALL BE 6" MIN. IN HEIGHT
- b. THE BORDER DIMEN
- . <u>MATERIAL AND FINISH</u>
 a. CHARACTERS AND BACKGROUND SHALL BE EGGSHELL, MATTE OR OTHER NON-
- GLARE FINISH AS RECOMMENDED BY THE SIGN MANUFACTURER.

 b. BACKGROUND SHALL CONSIST OF 1/4" ACRYLIC, COLOR TO MATCH SW 7068
- c. CHARACTERS AND SYMBOLS SHALL BE WHITE
- a. MOUNT AT 60" ABOVE FINISH FLOOR TO THE CENTER OF SIGN
 b. MOUNT ON WALL ADJACENT TO THE LATCH SIDE OF THE DOOR
- b. MOUNT ON WALL ADJACENT TO THE LATCH SIDE OF THE DOOR
 c. IF NO WALL SPACE EXISTS ON THE LATCH SIDE OF THE DOOR, INCLUDING DOUBLE

LEAF DOORS, MOUNT ON THE NEAREST ADJACENT WALL

- 6. <u>FIRE SPRINKLER ROOM</u> a. INCLUDE 4" HIGH VINYL WHITE LETTERS W/ MIN. 0.5" STROKE READING "SPRINKLER
- ROOM" APPLIED TO EXTERIOR SIDE OF DOOR, AS REQUIERD BY LOCAL FIRE DEPT.

DIRECTIONAL INFORMATION

OTHER SIGNS WHICH PROVIDE DIRECTION TO OR INFORMATION ABOUT FUNCTIONAL SPACES OF THE BUILDING SHALL COMPLY WITH ADAAG SECTIONS: 4.30.1, 4.30.2, 3.30.3, 4.30.5

TWO-WAY COMMUNICATION DEVICE SIGNAGE

DIRECTIONS FOR THE USE OF THE TWO-WAY COMMUNICATION SYSTEM, INSTRUCTIONS FOR SUMMONING ASSISTANCE CIA THE TWO-WAY COMMUNICATION SYSTEM AND WRITTEN IDENTIFICATION OF THE LOCATION SHALL BE POSTED ADJACENT TO EACH TWO-WAY COMMUNICATION SYSTEM. EACH SIGN SHALL COMPLY WITH ICC A117.1 FOR VISUAL CHARACTERS. MOUNTING LOCATION OF SIGNAGE AND DEVICE SHALL BE PER DRAWING BELOW.

DRAWING SYMBOLS LEGEND

EXISTING CONSTRUCTION TO REMAIN

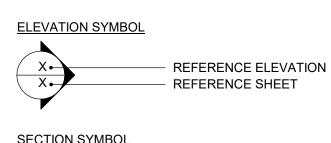
EXISTING CONSTRUCTION TO BE DEMOLISHED

NEW CONSTRUCTION

WALL TYPE DESIGNATION REFERENCE FLOOR PLAN(S) FOR LOCATIONS.

X ROOM NAME AND NUMBER REFERENCE FLOOR PLAN(S) FOR LOCATIONS.
REFERENCE FINISH SCHEDULE FOR FINISHES.

DOOR AND FRAME DESIGNATION REFERENCE FLOOR PLAN(S) FOR LOCATIONS.
REFERENCE DOOR AND FRAME SCHEDULE FOR REQUIREMENTS.



REFERENCE SECTION REFERENCE SHEET

ENLARGED DETAIL / ENLARGED PLAN SYMBOL

X
REFERENCE DETAIL
REFERENCE SHEET

INTERIOR ELEVATION SYMBOL



FINISH DESIGNATION SYMBOL

XX-1 WALL FINISH DESIGNATION
BASE FINISH DESIGNATION
LIMITS OF WALL AND BASE FINISHES

REFERENCE FLOOR PLAN(S) FOR LOCATIONS.
REFERENCE FINISH SCHEDULE FOR DESCRIPTIONS.

FLOOR FINISH DESIGNATION REFERENCE FLOOR PLAN(S) FOR LOCATIONS.
REFERENCE FINISH SCHEDULE FOR DESCRIPTIONS.

X REVISION NOTE

XX CONSTRUCTION NOTE

DEMOLITION NOTE

GYPSUM BOARD CONTROL JOINT REFERENCE FLOOR PLAN(S) FOR LOCATIONS.
REFERENCE DETAIL 1,2/A7.01 FOR CONSTRUCTION REQUIREMENTS.

CMJ — CONCRETE MASONRY CONTROL JOINT REFERENCE FLOOR PLAN(S) FOR LOCATIONS.
REFERENCE DETAIL **2/A1.10E** FOR CONSTRUCTION REQUIREMENTS.

WALL MOUNTED FIRE EXTINGUISHER BY LARSEN'S MANUFACTURING COMPANY, WWW.LARSENMFG.COM, MODEL MP10 W/B2 MOUNTING BRACKET, REFERENCE FLOOR PLAN(S) FOR LOCATIONS. MOUNT SO CENTERLINE OF EXTINGUISHER IS 46" A.F.F.

SEMI-RECESSED FIRE EXTINGUISHER BY LARSEN'S MANUFACTURING COMPANY, WWW.LARSENMFG.COM OR APPROVED EQUAL: ARCHITECTURAL SERIES, MODEL # AL-2409-6R. ALUMINUM, SEMI-RECESSED (2 ½" PROTRUSION FROM WALL WITH ROLLED EDGES), SOLID DOOR WITH RECESSED HANDLE, ENGRAVED VERTICAL LETTERS WITH NO BACKFILL "FIRE EXTINGUISHER" ON DOOR. CABINET TO BE PROVIDED WITH MP10 FIRE EXTINGUISHER AND MANUFACTURER'S STANDARD MOUNTING

BRACKET. MOUNT SO CENTERLINE OF CABINET HANDLE IS 46" A.F.F.

GENERAL NOTES

- ALL CONSTRUCTION SHALL CONFORM TO THE MINIMUM STANDARDS OF THE APPLICABLE CODE INDICATED IN THE BUILDING SUMMARY COLUMN AND ALL LOCAL CODES PRESENTLY IN EFFECT UNLESS MORE STRINGENT REQUIREMENTS ARE INDICATED.
- 2. ALL NEW CONSTRUCTION SHALL COMPLY W/THE AMERICANS WITH DISABILITIES ACT ACCESSIBILITY GUIDELINES (ADAAG) AND CHAPTER 11 OF THE INTERNATIONAL BUILDING CODE (INCLUDES ICC A117.1 PER IBC)
- 3. THE GENERAL CONTRACTOR AND SUBCONTRACTORS SHALL OBTAIN AND PAY FOR ALL REQUIRED PERMITS, LICENSES, AND ALL UTILITY CHARGES, AND ARRANGE FOR ALL REQUIRED INSPECTIONS.
- 4. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING BUILDING & SITE UTILITIES BETWEEN CIVIL & MEP DRAWINGS. THE CONTRACTOR SHALL ALSO CONTACT ALL APPLICABLE UTILITY COMPANIES & PROVIDE CONDUIT & OTHER FACILITIES AS REQUIRED.
- 5. THE GENERAL CONTRACTOR AND ALL SUBCONTRACTORS SHALL VERIFY ALL DIMENSIONS & CONDITIONS ON THE JOB SITE PRIOR TO THE BIDDING OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES.

 IN CASES OF DISCREPANCY CONCERNING DIMENSIONS, QUANTITIES AND LOCATION, THE CONTRACTOR SHALL, IN WRITING, CALL TO THE ATTENTION OF THE ARCHITECT ANY DISCREPANCIES BETWEEN SPECIFICATIONS, PLANS, DETAILS OR SCHEDULES. THE ARCHITECT WILL THEN INFORM THE CONTRACTOR, IN WRITING, WHICH DOCUMENT TAKES PRECEDENCE. THERE SHALL BE NO ADJUSTMENT TO THE COST OR TIME OF THE WORK RESULTING FROM CLARIFICATION OF SUCH DISCREPANCIES.
- 6. DIMENSIONS ON DRAWINGS ARE SHOWN TO FINISHED FACE OF WALLS AND PARTITIONS OF EXISTING OR NEW CONSTRUCTION UNLESS OTHERWISE NOTED. CEILING HEIGHT DIMENSIONS AND ALL OTHER VERTICAL DIMENSIONS ARE TO THE FINISHED FLOOR SURFACE UNLESS OTHERWISE NOTED.
- 7. CONTRACTOR TO FOLLOW ALL PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS FOR ALL BULIDING PRODUCTS. IN THE EVENT OF A CONFLICT BETWEEN INFORMATION SHOWN ON THE CONTRACT DOCUMENTS AND PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS, PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS SHALL GOVERN. CONTRACTOR SHALL NOTIFY ARCHITECT OF AN CONFLICTS BETWEEN PRODUCT MANUFACTURER INSTALLATION REQUIREMENTS AND THE CONTRACT DOCUMENTS PRIOR TO INSTALLATION.
- 8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING SHOP DRAWINGS, PRODUCT DATA, OR SAMPLES FOR CASEWORK, FINISHES, DOORS, FRAMES, HARDWARE, MECHANICAL, ELECTRICAL, AND PLUMBING FIXTURES, AND OTHER ITEMS REQUIRING ARCHITECT'S REVIEW FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS, AND FOR ALL ITEMS WHICH ALLOWED CONTRACTOR OPTIONS. PRIOR TO FORWARDING TO THE ARCHITECT FOR REVIEW. THESE SUBMITTALS MUST BE REVIEWED BY THE CONTRACTOR FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATIONS OF CONSTRUCTION AND SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO, ALL OF WHICH ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL AFFIX A STAMP TO SUBMITTAL INDICATING HIS REVIEW. SUBMITTALS FORWARDED WITHOUT A STAMP WILL BE RETURNED. ALL SUBMITTALS MUST BE REVIEWED BY THE ARCHITECT PRIOR TO CONSTRUCTION.
- 9. CONTRACTOR SHALL GUARANTEE ALL WORK AGAINST FAULT OF ANY MATERIAL OR WORKMANSHIP FOR A PERIOD OF NOT LESS THAN ONE YEAR AFTER COMPLETION OR ACCEPTANCE. FAULTY WORK SHALL BE REPLACED OR REPAIRED AS REQUIRED AT NO COST TO THE OWNER.
- 10. CONTRACTOR SHALL COORDINATE WITH OWNER ALL ITEMS TO BE SALVAGED PRIOR TO SUBMISSION OF BIDS AND START OF CONSTRUCTION. OWNER SHALL HAVE SALVAGE RIGHTS TO RETAIN ALL REMOVED ITEMS.
- 11. ALL CHANGES PROPOSED DURING CONSTRUCTION WHICH RESULT IN A CHANGE TO THE CONTRACT TIME AND/OR SUM SHALL BE SUBMITTED TO THE ARCHITECT IN WRITING AND APPROVED BY THE ARCHITECT AND OWNER BEFORE SUCH WORK SHALL COMMENCE.
- 12. CONTRACTOR SHALL COORDINATE CLEAR OPENINGS FOR ALL APPLIANCES PRIOR TO
- CONSTRUCTION OF CASEWORK.

 13. CONTRACTOR SHALL FURNISH AND INSTALL CONCEALED FIRE-RETARDANT TREATED
- WOOD BLOCKING BEHIND ALL CABINETS, TOILET ACCESSORIES, PLUMBING FIXTURES, AND OTHER WALL MOUNTED ITEMS AS REQUIRED FOR ADEQUATE SUPPORT.
- 14. CONTRACTOR SHALL COORDINATE ALL LOCK AND LATCH SETS AND FINAL KEYING WITH OWNER. DOUBLE KEYED LOCKS ARE NOT PERMITTED ON ANY REQUIRED OR MARKED EXIT. MATCH EXISTING KEYING SYSTEM IF ONE IS EXISTING.
- 15. ALL DOOR HARDWARE ON EXIT DOORS SHALL BE READILY OPERABLE FROM THE EGRESS SIDE WITHOUT THE USE OF A KEY, SPECIAL KNOWLEDGE, OR EFFORT.
- 16. CONTRACTOR SHALL PREPARE ALL NEW AND EXISTING SURFACES SCHEDULED TO RECEIVE NEW FINISHES IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR THE SUBSTRATE & FINISH BEING APPLIED.
- 17. CONTRACTOR SHALL COORDINATE FINAL QUANTITY AND LOCATIONS OF FIRE EXTINGUISHERS WITH THE FIRE DEPARTMENT AND/OR BUILDING DEPARTMENT. SEE SYMBOLS LEGEND FOR TYPE OF EXTINGUISHER.
- 18. ALL CONSTRUCTION MATERIALS EXPOSED WITHIN PLENUMS SHALL BE NON-COMBUSTIBLE OR SHALL HAVE A MAXIMUM FLAME SPREAD RATING OF 25 AND MAXIMUM SMOKE DEVELOPED RATING OF 50.
- 19. ALL PIPING, LOW VOLTAGE WIRE AND CABLE, OPTICAL FIBER, PNEUMATIC TUBING, AND ALL DUCT AND DUCT COVERINGS, LININGS AND CONNECTORS INSTALLED WITHIN PLENUMS
- 20. TENANT SHALL BE RESPONSIBLE FOR COORDINATION AND INSTALLATION OF VOICE AND DATA CABLING AND EQUIPMENT.
- 21. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE AUTOMATIC SPRINKLER SYSTEM. THE DESIGN SHALL BE PER NFPA REQUIREMENTS.
- 22. ALL NEW GLASS AND GLAZING LOCATED IN HAZARDOUS LOCATIONS AS DEFINED IN IBC SECTION 2406.3 SHALL MEET THE REQUIREMENTS FOR SAFETY GLAZING AS DEFINED IN IBC
- SECTION 2406.

 23. IF THE CONTRACTOR FAILS TO SUBMIT A MATERIAL FOR APPROVAL, THE MATERIAL MAY BE

REQUIRED TO BE REMOVED BY THE CONTRACTOR EITHER BY DIRECTION OF THE OWNER

- OR ARCHITECT.

 24. ALL HIGH-PILED STORAGE SHALL COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE
- APPLICABLE EDITION OF THE INTERNATIONAL FIRE CODE.

 25. THE CONTRACTOR IS TO PROVIDE AS BUILT DRAWINGS IN HARD COPY & AN ELECTRONIC
- AUTOCAD FILE TO THE OWNER AT THE CONCLUSION OF THE PROJECT.

 26. INSTALL ELASTOMERIC JOINT SEALER AROUND ALL PIPES, DUCTWORK, & STRUCTURE PASSING THRU INTERIOR NON-RATED CONCRETE AND MASONRY WALLS, GYPSUM BOARD PARTITIONS, AND CONCRETE FLOOR/ROOF SLABS. FOR FIRE RATED INTERIOR CONCRETE AND MASONRY WALLS, GYPSUM BOARD PARTITIONS, AND CONCRETE FLOOR/ROOF SLABS SEAL ALL PIPES. DUCTWORK, AND STRUCTURE. INSTALL FIRESTOP MATERIALS IN ALL GAPS

PRIOR TO SEALANT APPLICATION. INSTALL SEALER ACCORDING TO MANUFACTURER'S

27. CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTION OF ALL EXISTING CONSTRUCTION INDICATED TO REMAIN AND SHALL REPAIR AND/OR REPLACE ALL AREAS AND /OR MATERIAL DAMAGED DURING CONSTRUCTION AT A MINIMUM TO THE CONDITION WHICH EXISTED PRIOR TO CONSTRUCTION.

WRITTEN INSTRUCTIONS.

28. CONTRACTOR SHALL BE RESPONSIBLE FOR PRICING RADIO COVERAGE AMPLIFIER FOR EMERGENCY RESPONDERS AS AN ALTERNATE. PRIOR TO CONSTRUCTION COMPLETION, AMPLIFIER SHALL BE PROVIDED ONLY IF REQUIRED BY AHJ.

PROFESSIONAL SERVICES DISCLAIMER

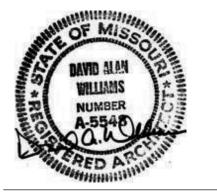
THIS DISCLAMER SERVES NOTICE OF ACCEPTANCE OF RESPONSIBILITY AND DISCLAIMER OF RESPONSIBILITY AS TO THE CONTRACT DOCUMENTS PREPARED FOR PROJECT NUMBER:

18017,19050.07,19050.08, PARAGON STAR NORTH VILLAGE BY FINKLE + WILLIAMS, INC.
THE UNDERSIGNED ARCHITECT, AND FINKLE + WILLIAMS, INC., ARE RESPONSIBLE FOR PREPARATION OF ONLY THE NOTED CONSTRUCTION DRAWINGS BELOW:

<u>NO.</u>	TITLE	<u>DATE</u>
A0.01G	LEGENDS & GEN. NOTES	10.18.24
A0.03	CODE	10.18.24
A1.70G	GARAGE FIRST FLOOR PLAN AT GRADE	10.18.24
A1.71G	GARAGE FIRST FLOOR PLAN	10.18.24
A1.72G	GARAGE SECOND FLOOR PLAN	10.18.24
A1.73G	GARAGE THIRD FLOOR PLAN	10.18.24
A1.74G	GARAGE FOURTH FLOOR PLAN	10.18.24
A1.75G	GARAGE FIFTH FLOOR PLAN	10.18.24
A1.76G	GARAGE SECTIONS	10.18.24
A1.77G	GARAGE STAIRS	10.18.24
A1.78G	GARAGE STAIRS AND SECTIONS	10.18.24
A1.79G	GARAGE DETAILS	10.18.24
A8.01G	DOOR SCHEDULE AND DETAILS	10.18.24

THE UNDERSIGNED ARCHITECT AND FINKLE + WILLIAMS DISCLAIM RESPONSIBILITY FOR ALL OTHER CONSTRUCTION DOCUMENTS, AND ANY OTHER SPECIFICATIONS, REPORTS, ESTIMATES, SHOP DRAWINGS, ETC. RELATING TO OR INTENDED TO BE USED FOR ANY PART OF THE ARCHITECTURAL OR ENGINEERING PROJECT, INCLUDING ANY GEOTECHNICAL ENGINEERING SERVICES, OR ENVIRONMENTAL REPORTS.

THIS NOTICE IS EXECUTED BY THE UNDERSIGNED AND AUTHENTICATED BY THE ARCHITECTURAL SEAL OF THE PERSON PREPARING THS NOTICE.



ARCHITECT: DAVID A. WILLIAMS

BUILDING SUMMARY

GENERAL BUILDING INFORMATION

PROJECT NAME: PARAGON STAR NORTH VILLAGE

ADDRESS: 3200 NW PARAGON PKWY

LEE'S SUMMIT, MO 64081

MIXED USE MULTI-FAMILY RESIDENTIAL AND RETAIL

APPLICABLE CODES

INTERNATIONAL BUILDING CODE (IBC)	2018 EDITIO
INTERNATIONAL MECHANICAL CODE (IMC)	2018 EDITION
INTERNATIONAL PLUMBING CODE (IPC)	2018 EDITION
NATIONAL ELECTRIC CODE (NEC)	2017 EDITION
INTERNATIONAL FIRE CODE (IFC)	2018 EDITION
INTERNATIONAL FUEL GAS CODE (IFGC)	2018 EDITION
ICC/ANSI A117.1-2009, ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES	·

GENERAL BUILDING LIMITATIONS

This development is made up of the following multiple adjacent separate and distinct buildings both vertically and horizontally.

Retail Building (SEPARATE PERMIT)
Parking Garage
Apartment Building A (SEPARATE PERMIT)
Apartment Building B (SEPARATE PERMIT)
Apartment Building C (SEPARATE PERMIT)
Apartment Building D (SEPARATE PERMIT)
Apartment Building E (SEPARATE PERMIT)

Apartment Building F

 Parking Garage
 5 Story
 33,692 GSF/Flr
 168,460 GSF

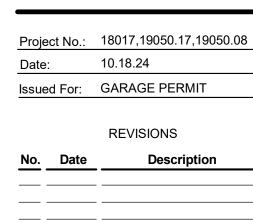
(SEPARATE PERMIT)

Occupancy: Low-hazard Stor
Construction Type: IIA (Fully Sprinkle
Allowable Height: 6 Stories – 85' A
Allowable Area: 117,000 SF/Flr
Max. Travel Distance: 100'

Low-hazard Storage Group S-2 IIA (Fully Sprinklered NFPA 13) 6 Stories – 85' Above Grade Plane paragon

PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081



REGISTRATION



ARCHITECT FINKLE+WILLIAMS ARCHITECTURE

CIVIL GBA ENGINEERS

LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL

PLUMBING LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR

<u>2 HOUR RATED PARTITION:</u>
IF INSTALLED VERTICALLY, ATTACH GYP. BD. W/ 1" LONG NO. 6 DRYWALL SCREWS TO EACH STUD @ 8"

O.C. AROUND THE PERIMETER & 12" O.C. @ INTERMEDIATE STUDS. IF INSTALLED HORIZONTALLY, ATTACH GYP. BD. W/ 1" LONG NO. 6 DRYWALL SCREWS TO EACH STUD @ 8" O.C. AT VERTICAL EDGES AND 12" O.C. @ INTERMEDIATE STUDS. ENSURE THE HORIZONTAL JOINTS ARE STAGGERED WIH THOSE ON THE OPPOSITE SIDE.

PER STRUCT.

REQUIRED

- CONCRETE DOUBLE TEE

FIRE RATED SEALANT AS

20 GA. LONG LEG RUNNER TRACK ANCHORED TO BOTTOM OF

STRUCTURE (MAINTAIN 1" CLEAR BET. UPPER & LOWER TRACK)

- CEILING, REF FINISH SCHEDULE

FOR TYPE & LOCATION, TYP.

6" BATT INSULATION,

- (2) LAYERS 5/8" TYPE

- FLOOR SLAB

- 20 GA. 6" MTL. STUDS @

Seal w/ acoustical sealant, top and bottom



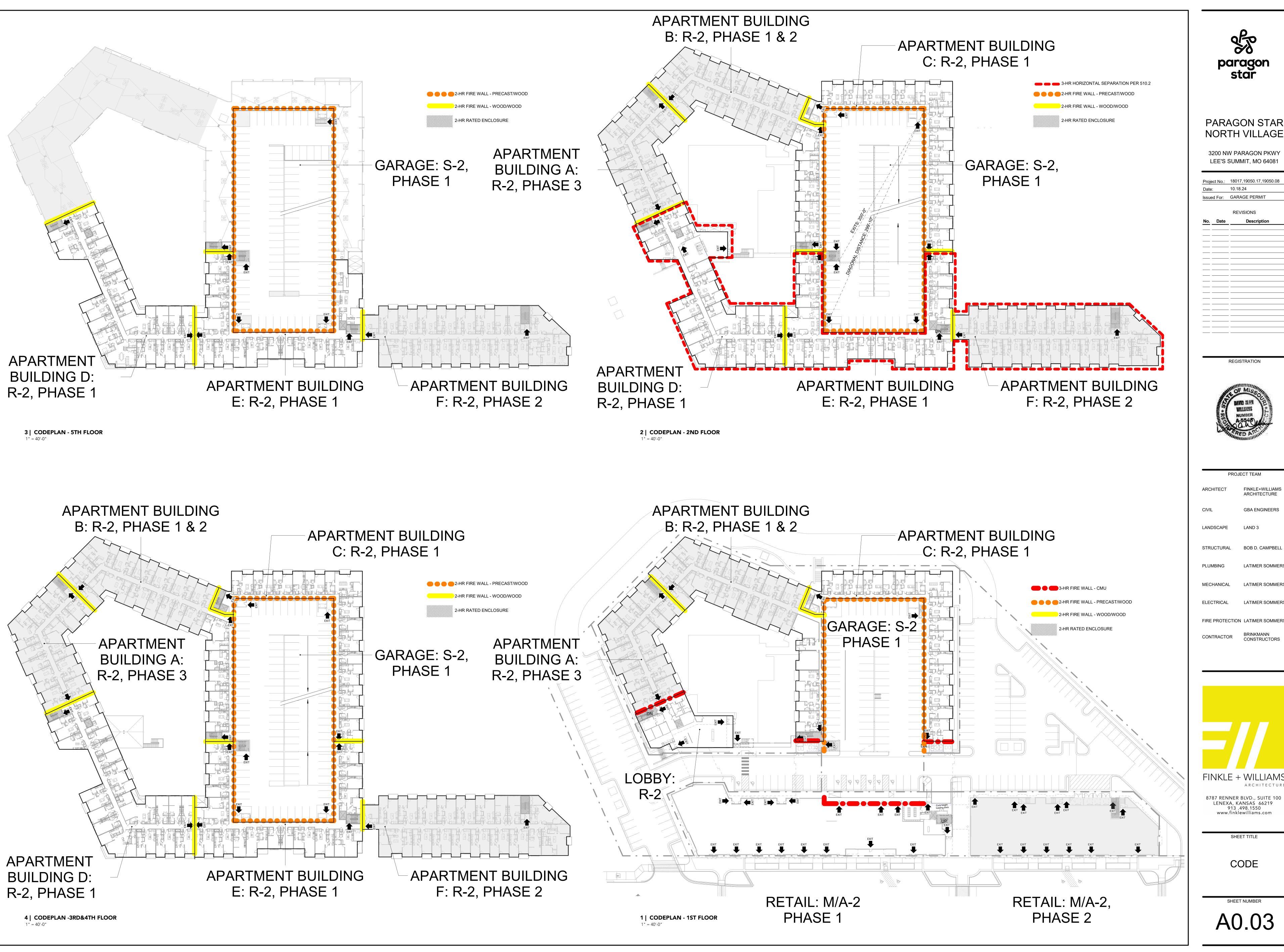
CONSTRUCTORS

SHEET TITLE

www.finklewilliams.com

LEGENDS & GEN. NOTES

A0.01G





3200 NW PARAGON PKWY



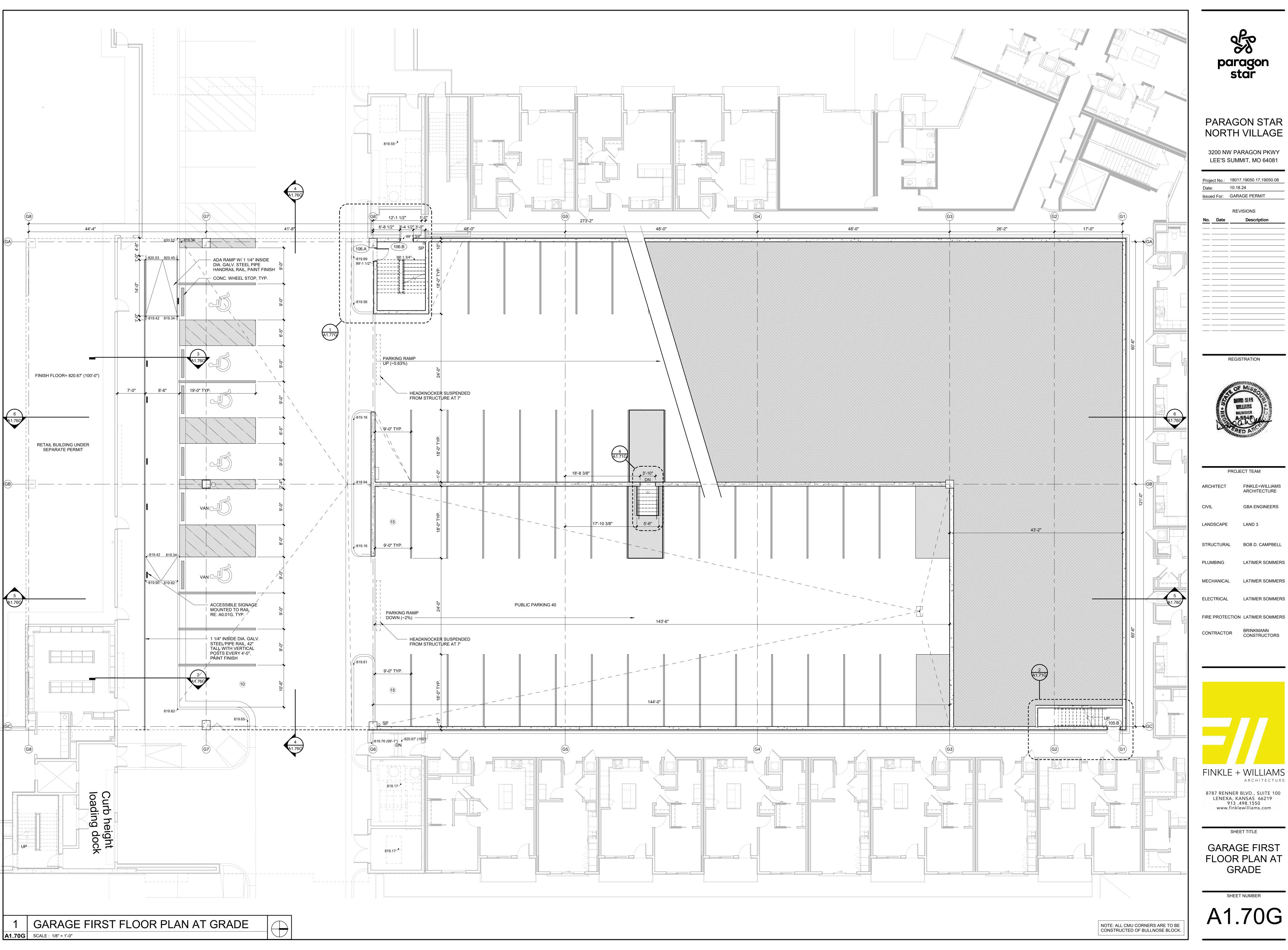
PROJECT TEAM LATIMER SOMMERS **MECHANICAL**



SHEET TITLE

CODE

SHEET NUMBER A0.03





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.17,19050.08

REVISIONS

PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE

GBA ENGINEERS LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

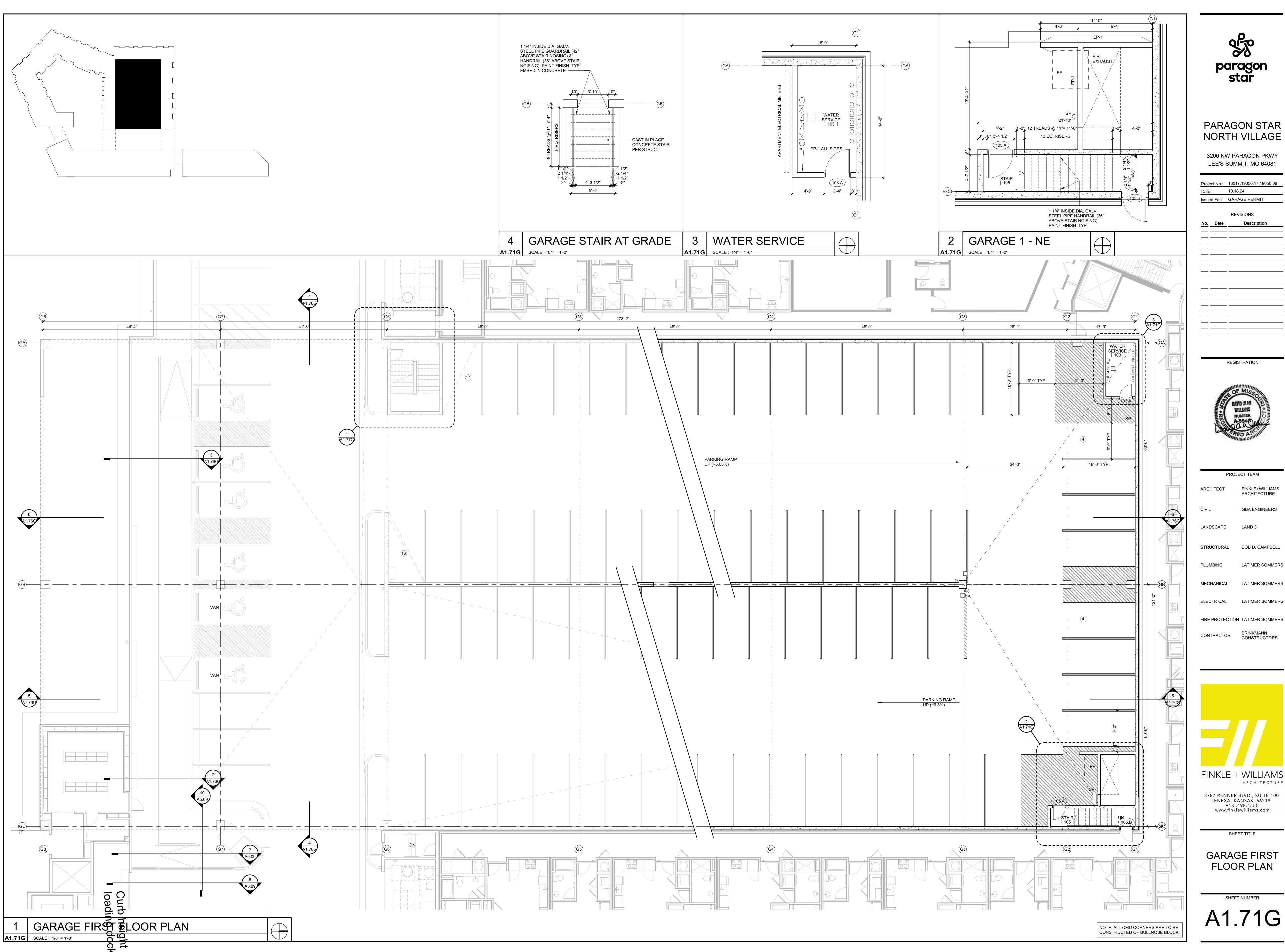
BRINKMANN CONSTRUCTORS CONTRACTOR

FINKLE + WILLIAMS ARCHITECTURE

8787 RENNER BLVD., SUITE 100 LENEXA, KANSAS 66219 913 .498.1550 www.finklewilliams.com

SHEET TITLE

GARAGE FIRST FLOOR PLAN AT GRADE





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.17,19050.08 10.18.24

Issued For: GARAGE PERMIT REVISIONS

REGISTRATION



PROJECT TEAM

FINKLE+WILLIAMS ARCHITECTURE **GBA ENGINEERS**

STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS

LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

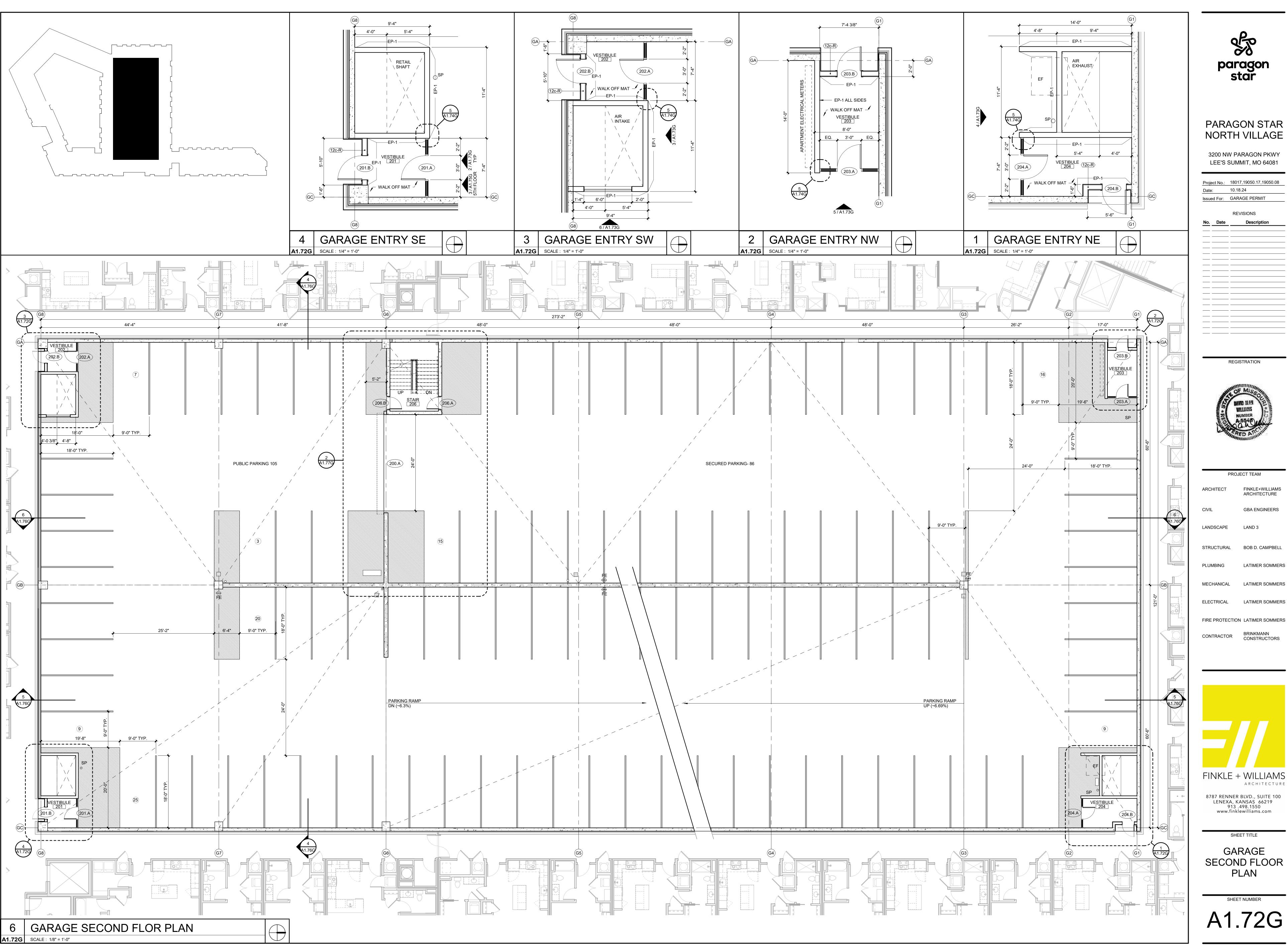
CONTRACTOR BRINKMANN CONSTRUCTORS

ARCHITECTURE

LENEXA, KANSAS 66219 913 .498.1550 www.finklewilliams.com

SHEET TITLE

GARAGE FIRST FLOOR PLAN





3200 NW PARAGON PKWY

Project No.: 18017,19050.17,19050.08 Issued For: GARAGE PERMIT

REGISTRATION

PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE

GBA ENGINEERS

LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS

LATIMER SOMMERS MECHANICAL

ELECTRICAL LATIMER SOMMERS

CONTRACTOR BRINKMANN CONSTRUCTORS

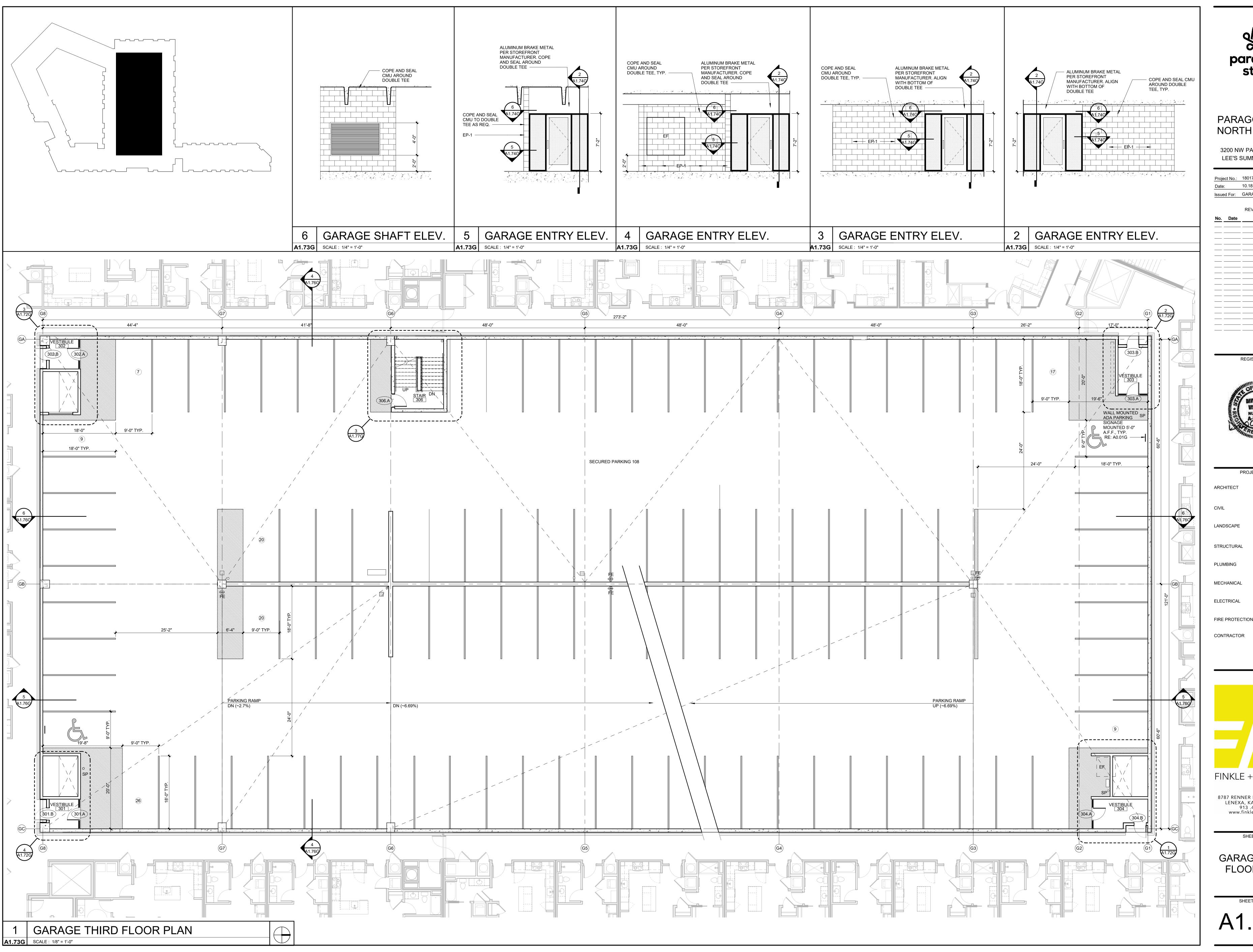
FINKLE + WILLIAMS ARCHITECTURE 8787 RENNER BLVD., SUITE 100

LENEXA, KANSAS 66219 913 .498.1550 www.finklewilliams.com

SHEET TITLE

GARAGE SECOND FLOOR PLAN

SHEET NUMBER A1.72G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.17,19050.08 Issued For: GARAGE PERMIT

REVISIONS

REGISTRATION

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LATIMER SOMMERS

LATIMER SOMMERS MECHANICAL

ELECTRICAL LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

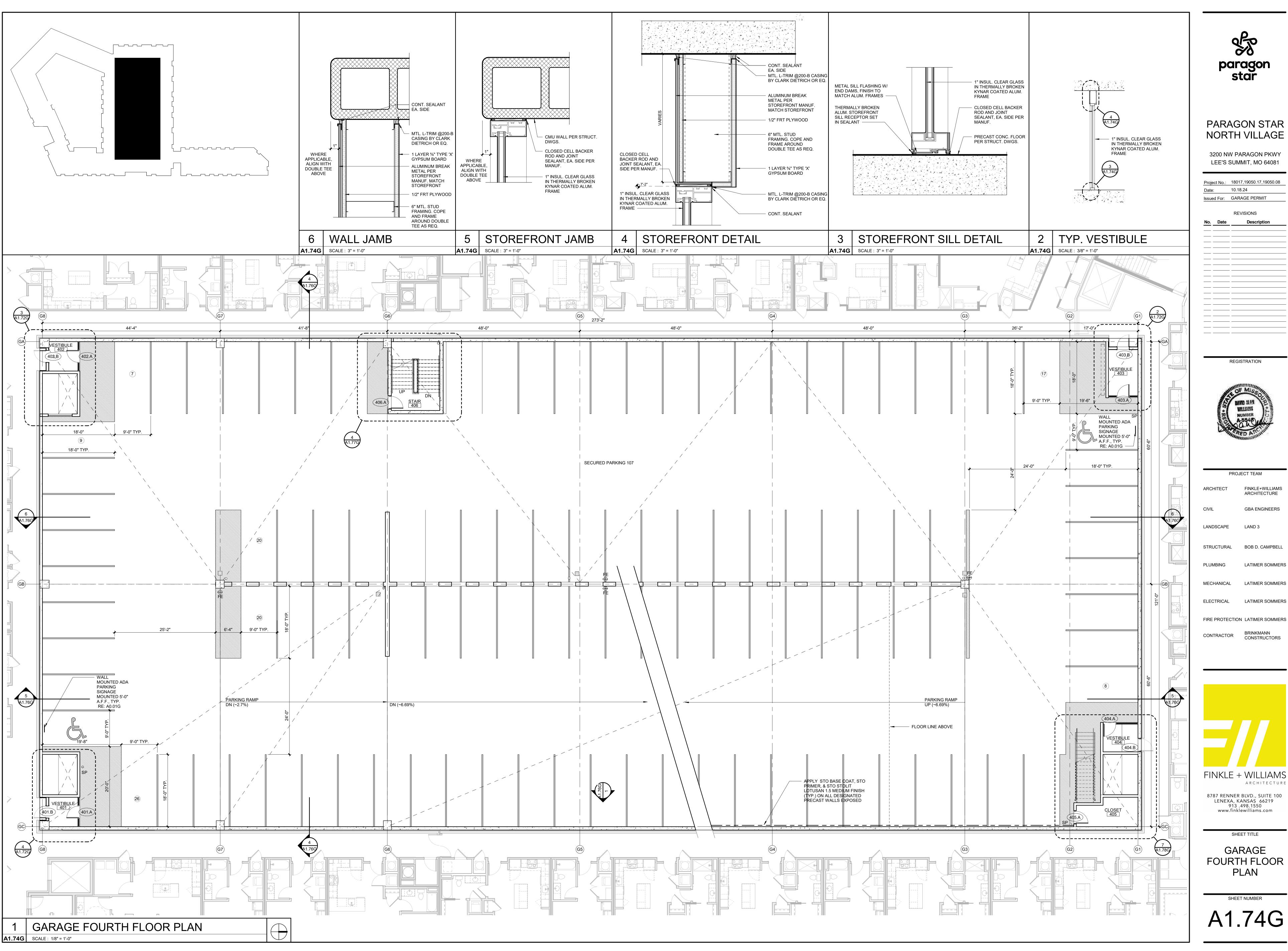
FINKLE + WILLIAMS ARCHITECTURE

8787 RENNER BLVD., SUITE 100 LENEXA, KANSAS 66219 913 .498.1550 www.finklewilliams.com

SHEET TITLE

GARAGE THIRD FLOOR PLAN

SHEET NUMBER A1.73G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.17,19050.08 10.18.24 Issued For: GARAGE PERMIT

REVISIONS

REGISTRATION



PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE

GBA ENGINEERS

STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS

LATIMER SOMMERS MECHANICAL

ELECTRICAL LATIMER SOMMERS

BRINKMANN CONSTRUCTORS CONTRACTOR

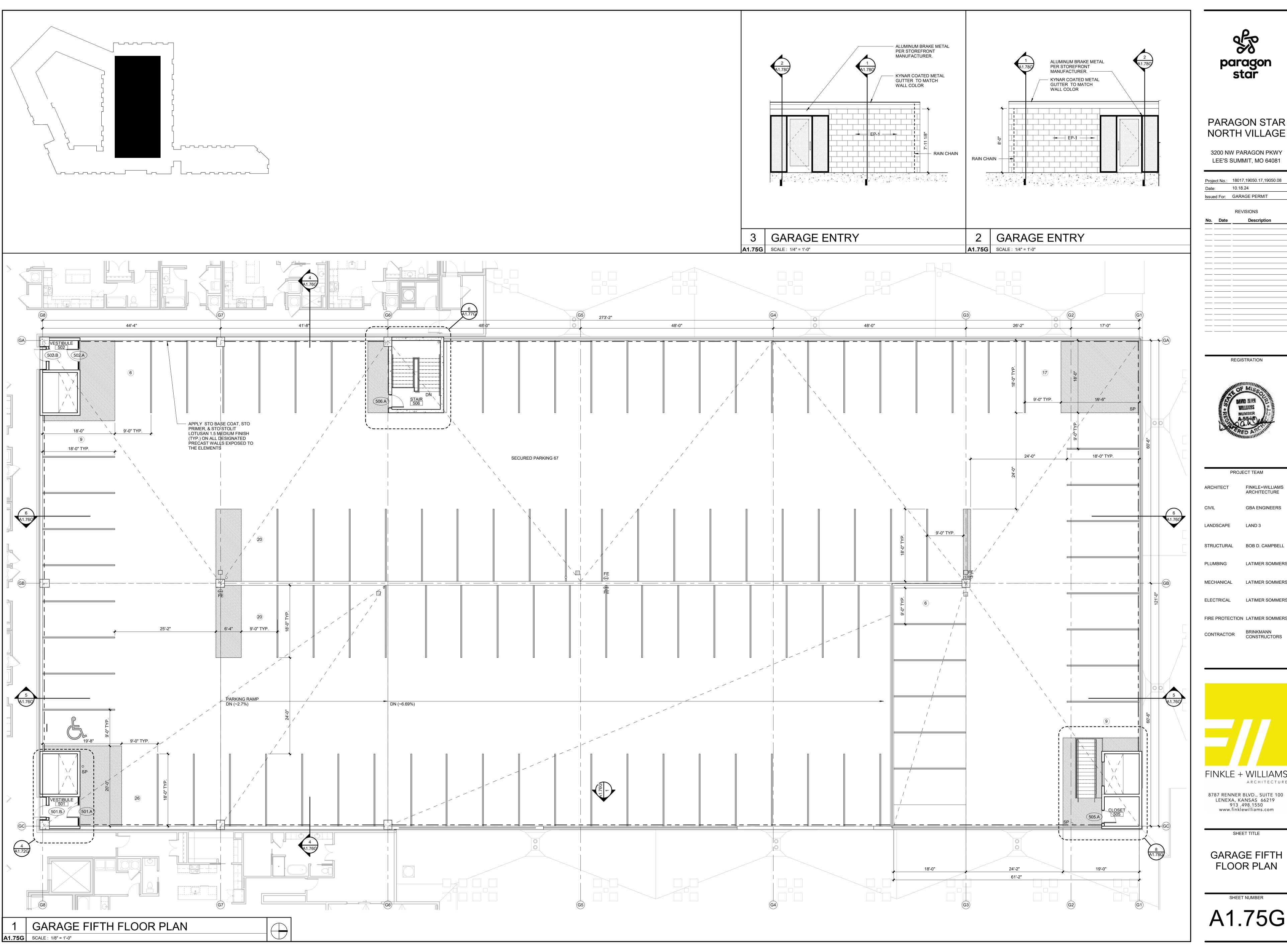
FINKLE + WILLIAMS ARCHITECTURE

LENEXA, KANSAS 66219 913 .498.1550 www.finklewilliams.com

SHEET TITLE

GARAGE FOURTH FLOOR PLAN

SHEET NUMBER A1.74G





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PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE

LANDSCAPE LAND 3

GBA ENGINEERS

STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS FIRE PROTECTION LATIMER SOMMERS

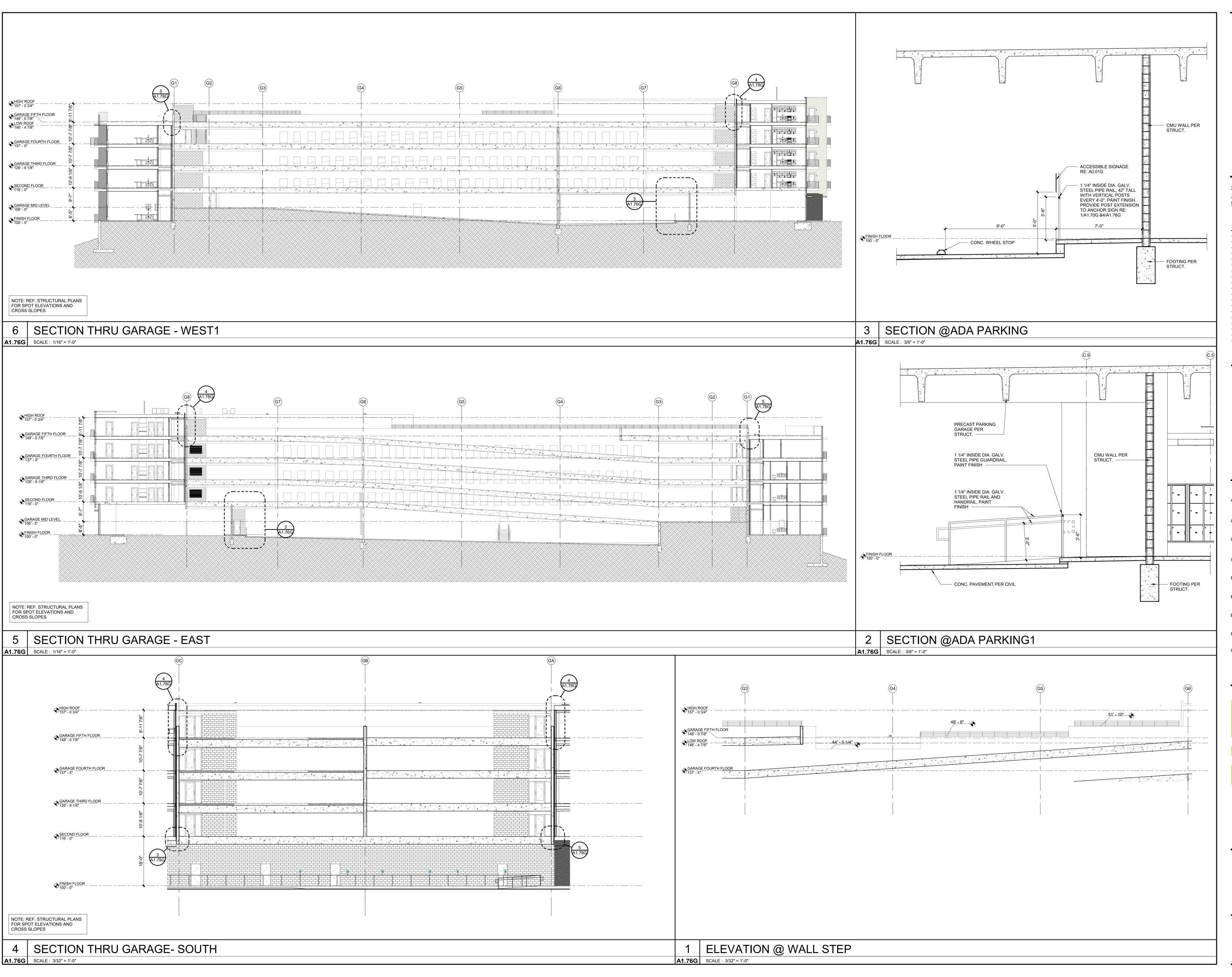
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ARCHITECTURE

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GARAGE FIFTH FLOOR PLAN

SHEET NUMBER A1.75G





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Project No.: 18017,19050.17,19050.08 Issued For: GARAGE PERMIT



PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE GBA ENGINEERS STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS LATIMER SOMMERS MECHANICAL

FIRE PROTECTION LATIMER SOMMERS

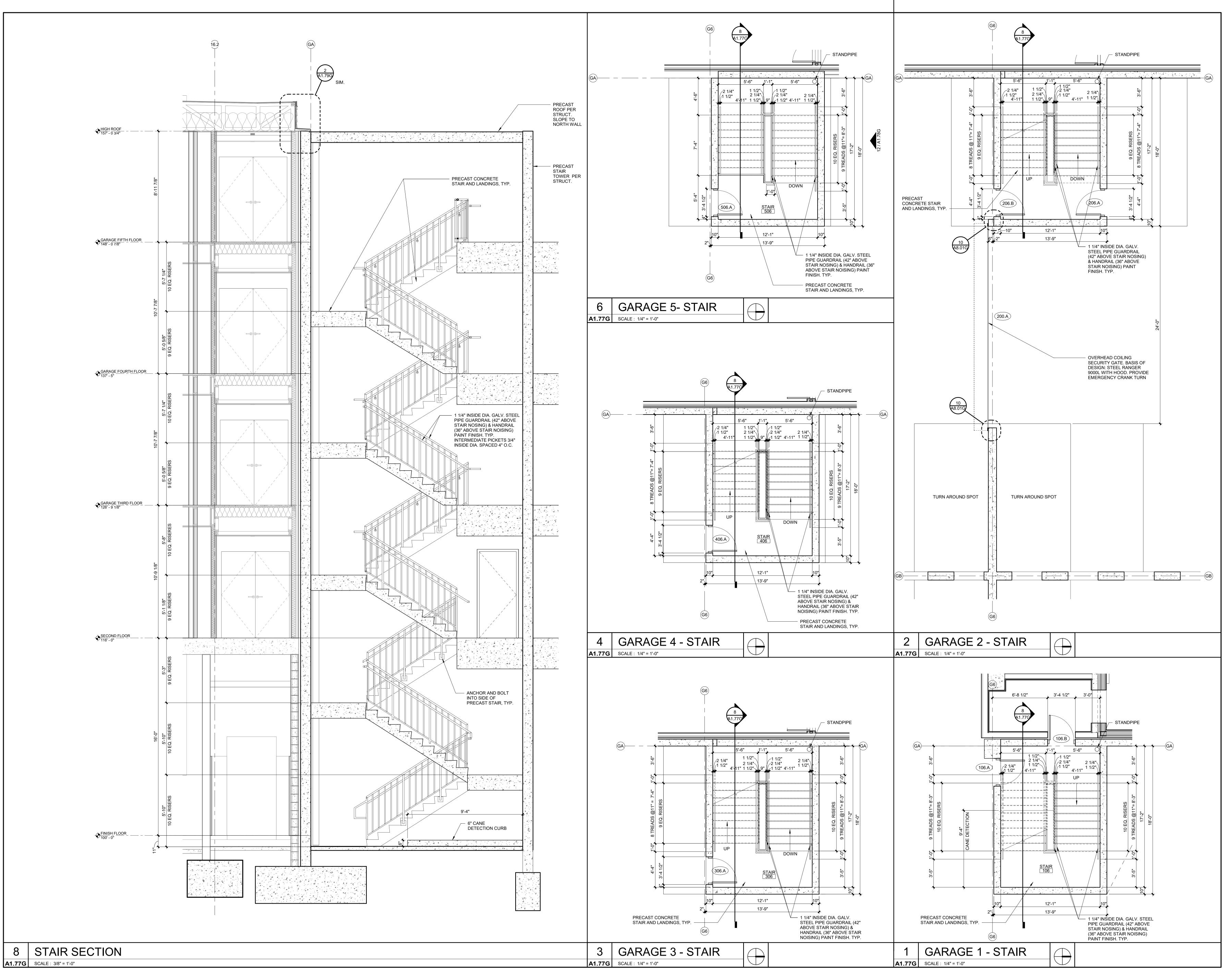
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SHEET TITLE

GARAGE SECTIONS

SHEET NUMBER A1.76G





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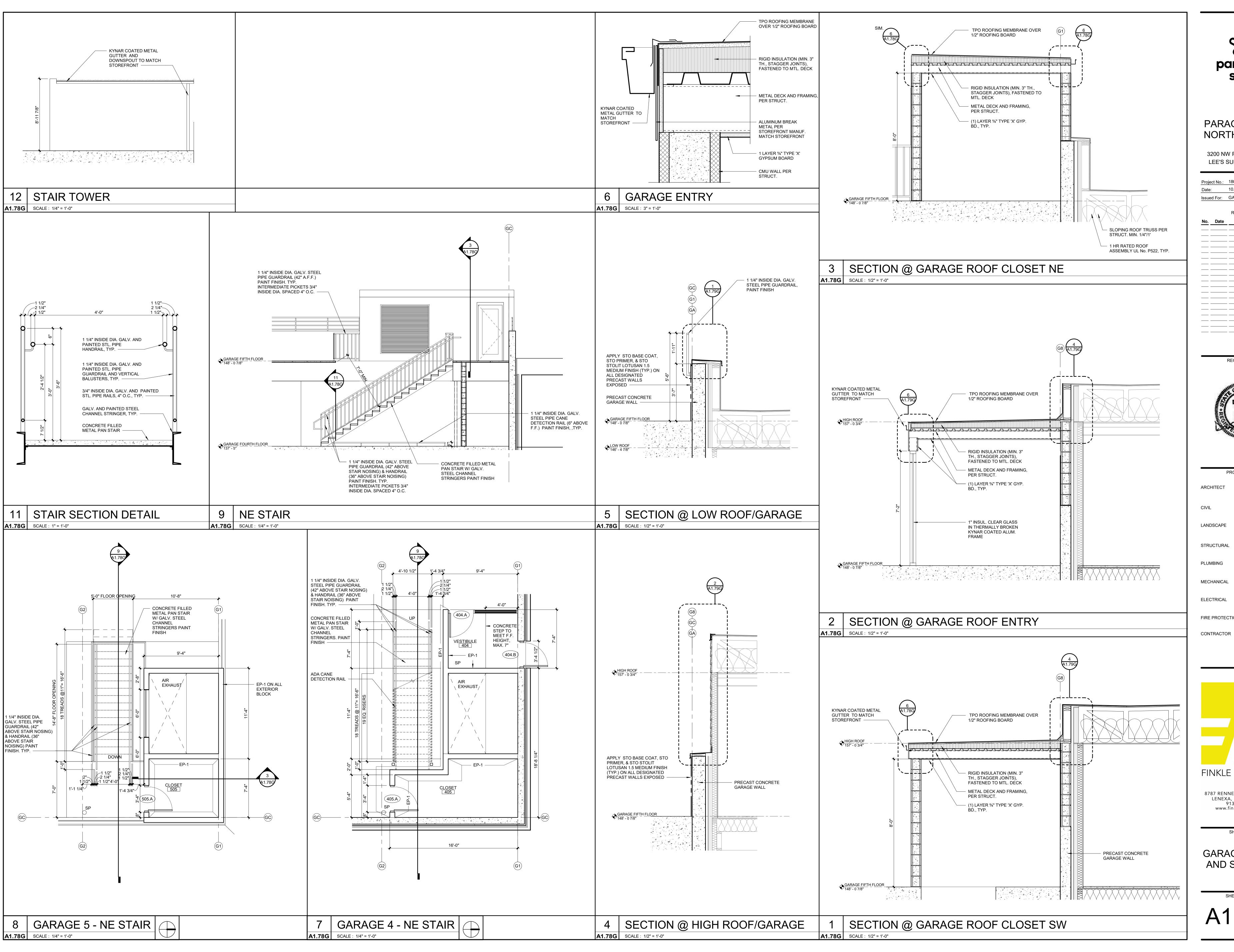
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GARAGE STAIRS

SHEET TITLE

SHEET NUMBER

A1.77G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.17,19050.08

Date: 10.18.24

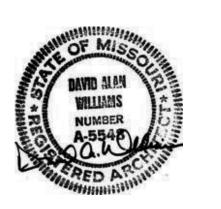
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REVISIONS

No. Date Description

Date Description

REGISTRATION



PROJECT TEAM

ARCHITECT FINKLE+WILLIAMS ARCHITECTURE

CIVIL GBA ENGINEERS

LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL

PLUMBING LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR BRINKMANN CONSTRUCTORS

FINKLE + WILLIAMS

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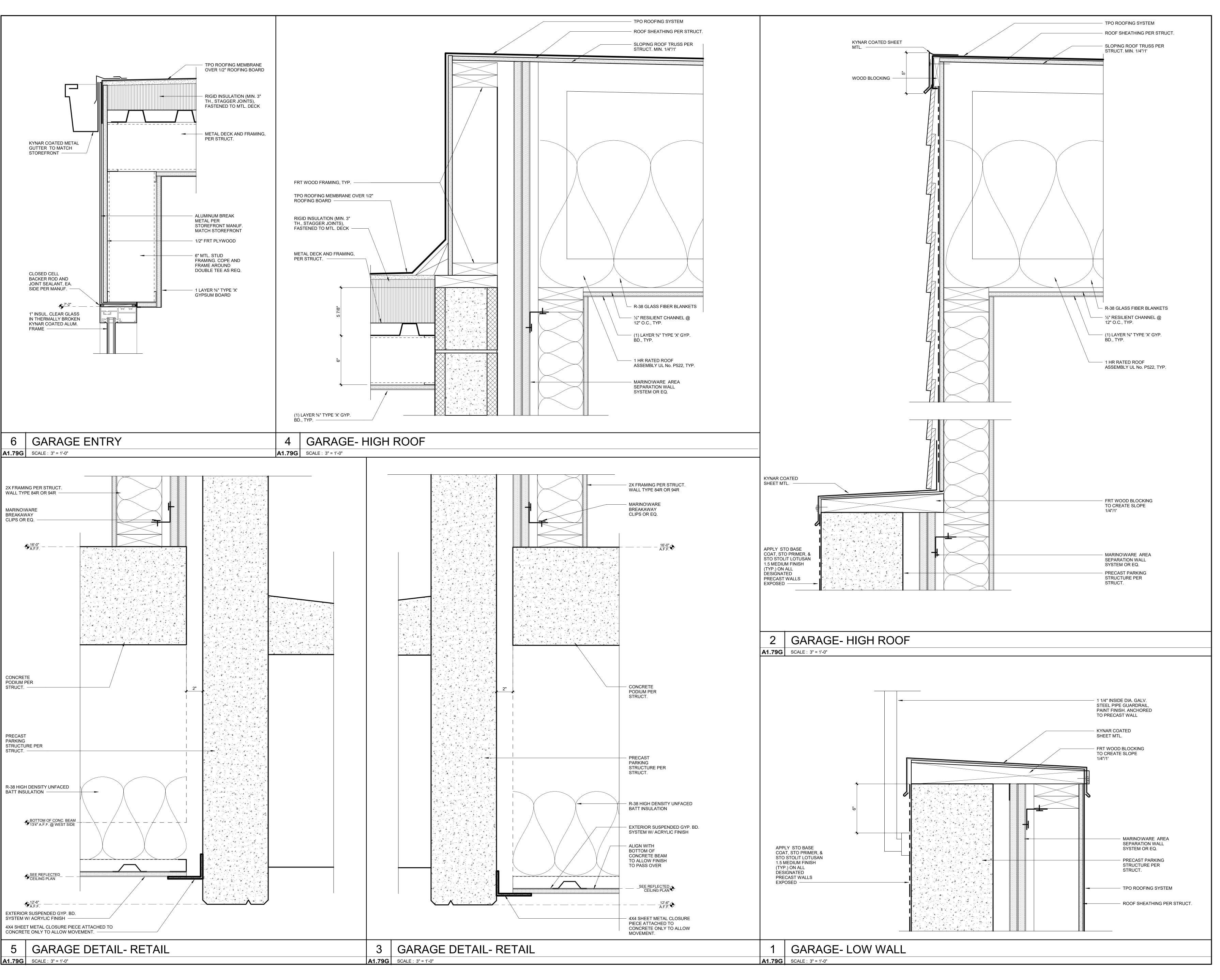
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SHEET TITLE

GARAGE STAIRS AND SECTIONS

SHEET NUMBER

A1.78G





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Project No.: 18017,19050.17,19050.08 Issued For: GARAGE PERMIT

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BRINKMANN CONSTRUCTORS

CONTRACTOR

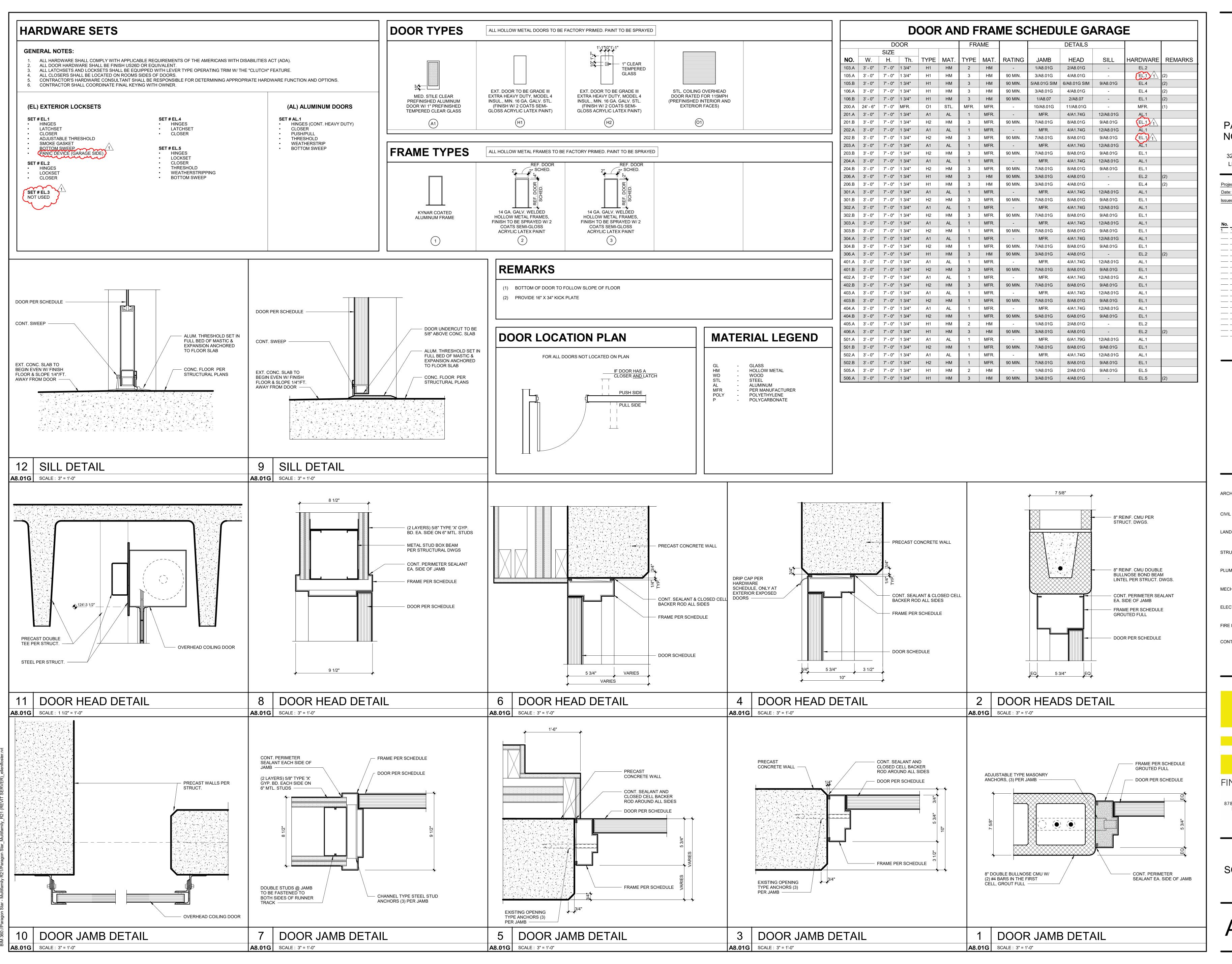
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SHEET TITLE

GARAGE **DETAILS**

SHEET NUMBER A1.79G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.17,19050.08 10.18.24 Issued For: GARAGE PERMIT

REVISIONS

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PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE GBA ENGINEERS LAND 3 LANDSCAPE STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS LATIMER SOMMERS MECHANICAL LATIMER SOMMERS ELECTRICAL FIRE PROTECTION LATIMER SOMMERS BRINKMANN CONSTRUCTORS CONTRACTOR



ARCHITECTURE

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SHEET TITLE DOOR

SCHEDULE AND DETAILS

SHEET NUMBER

A8.01G

- the engineer of any discrepancies, inconsistencies, or difficulties affecting the work B. The contractor shall coordinate all disciplines, verifying size and location of all openings, whether shown on structural drawings or not, as called for on architectural, mechanical, or electrical drawings. In the case of work in an existing building the contractor shall scan existing structure to locate all rebar in the area of the new core/opening using ground penetrating radar and notify the engineer of record for review prior to coring/cutting. Conflicts, inconsistencies, or other difficulties affecting structural work shall be called to the architect or engineer's attention for direction
- before proceeding. All design and construction work for this project shall conform to the requirements of the following governing design codes: 1. International Building Code (IBC 2018) as amended by the city of Lee's Summit,
- Minimum Design Loads for Buildings and Other Structures (ASCE7-16) Specification for Structural Steel Buildings (AISC 360-16)
- Member Design Basis is Allowable Stress Design (ASD) Connection Design Basis is Allowable Stress Design (ASD)
- 4. Structural Welding Code (AWS D1.4-17) 5. Building Code Requirements for Structural Concrete (ACI 318-14)
- Building Code Requirements for Masonry Structures (TMS 402-16) . North American Specification for the Design of Cold-Formed Steel Structural
- Members (AISI S100-16) 8. National Design Specification (NDS) for Wood Constriction with 2018 Supplements (ANSI/AWC NDS-2018)
- 9. Special Design Provisions for Wind and Seismic (AWC SDPWS-2015) D. These drawings are for this specific project and no other use is authorized.

2. Structural Load Design Criteria

Floor, Storage

Roof, MEP Equipment Zone

Garage

- A. Dead Load: = 35 psf Floor, Apartment Floor, Balony = 15 psf Floor, Corridor (Above Podium) = 25 psf Garage Collateral = 5 psf = 25 psf Stair, Metal Pan = 60 psf = 25 psf Stair, Wood B. Live Load: Floor, Apartment = 40 psf = 60 psf Floor, Balcony Floor, Corridor (Serving Apartment) = 40 psf Floor, Corridor)Serving Public) = 100 psf Floor, Public (Clubhouse) = 100 psf
- C. Snow:
- Pg = 20psf, Ce = 1.0 Pf = 14 psf (Apt) & 16.8 psf (Garage), Pm = 20 psf Is = 1.0, Cs = 1.0, Ct = 1.0 (Apt) & 1.2 (Garage) Drift & unbalanced snow loads per ASCE/SEI 7-16
- D. Lateral Loads: 1.) Wind V(ult) = 109 mph, exposure C. lw=1.0 GCpi=+/-0.18 Design wind pressures to be used for the design of exterior component and cladding materials on the designated zones of wall and roof surfaces shall be per section 30.7 and Table 30.7-2 of ASCE/SEI 7-16. Tabulated pressures shall be multiplied by effective area reduction factors, exposure adjustment factors, and topographic factors where applicable

= 125 psf

= 40 psf

= 20 psf

= 45 psf

= 100 psf

- 2.) Seismic: Ss = 0.099, S1 = 0.068, le=1.0, Site Classification D Seismic Design Category B
- Basic Seismic Force-resisting System:
- A.2 Ordinary Reinforced Concrete Shear Walls R = 4, Omega = 4, Cd = 2 1/2, V = 0.053W At Apartments Above Podium:
- A.17 Light-Framed Walls with Shear Panels of All Other Materials R = 2, Omega = 2 1/2, Cd = 2, V = 0.053W At Precast Garage:
- A.6 Ordinary Precast Shear Walls (N/S Direction) R = 3, Omega = 2 1/2, Cd = 3, V = 0.035W B.9 - Ordinary Precast Shear Walls (E/W Direction)
- R = 4. Omega = 2 1/2. Cd = 4. V = 0.026WE. This project is designed to resist the most critical effects resulting from the load combinations of section 1605.3 of the International Building Code.

3. Concrete

- A. All concrete for foundations (walls, grade beams, footings and piers) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic vard of concrete regardless of strengths obtained, not over 6 gallons of water per 100 pounds of cement and not over 4 inches of slump. B. All concrete for interior flatwork (without floor covering) shall develop minimum
- ultimate compressive design strength of 4000 psi in 28 days, but not less than 525 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.75 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only).
- All concrete for interior flatwork (with floor covering) shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 540 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.40 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only).
- All concrete for exterior flatwork shall have a minimum design compressive strength of 4500 psi in 28 days, with not less than 560 pounds of cement per cubic yard of concrete, not over 5 gallons of water per 100 pounds of cement, with 6% +/- 1% air entrainment, and a maximum of 4 inches of slump. All concrete for columns shall develop a minimum ultimate compressive design

strength of 4000 psi in 28 days, but not less than 560 pounds of cement shall be

- used per cubic yard of concrete regardless of strengths obtained, not over 5 gallons of water per 100 pounds of cement and not over 4 inches of slump. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's dosage rates for improved workability.
- G. The preceding minimum mix requirements may have up to 15% maximum of the cement content replaced with an approved ASTM C618 Class C fly ash, provided the total minimum cementitious content is not reduced.
- H. Combined aggregate (coarse plus fine) for all concrete shall be well graded from coarsest to finest with no more than 18 percent and not less than 8 percent retained on an individual sieve, except that less than 8 percent may be retained on coarsest sieve and on No. 50 and finer sieves. Submit this gradation report with the concrete mix design shop drawings. All interior concrete slabs on grade shall be placed over 15 mil, Class A Vapor
- Barrier per ASTM E1745 with less than 0.01 perms, tested after mandatory conditioning. All joints shall be lapped and sealed per manufacturer's recommendations. All penetrations, as well as damaged vapor barrier material shall also be sealed per manufacturer's recommendation prior to concrete placement. Install barrier per manufacturer recommended details at all discontinuous edges (at interior columns, exterior edge of slab, etc.) to ensure terms of warranty are followed. The vapor barrier shall be placed over freedraining granular material as prescribed by the project soils report.
- Basement foundation walls shall be braced at the base and top of wall by the contractor until the slab on grade at the base and the floor framing/slab at the top of wall is complete and the concrete has achieved 75% of the design strength. The contractor is responsible for engineering and design of the wall bracing, if
- K. All concrete is reinforced concrete unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any details not shown shall be detailed per ACI 315 and meet requirements of ACI 318, current editions. Control joints in dirt formed slab to be as shown on plans. Where not shown, limit

controlled areas to not more than 144 square feet, or 12 feet on any side. Slab

panel side ratio shall not exceed 1 1/2 to 1. M. Contractor shall verify that all concrete inserts, reinforcing and embedded items are correctly located and rigidly secured prior to concrete placement. N. Construction joints in beams, slabs, and grade beams shall occur at midspan (middle third) unless noted otherwise. Provide 2 x 4 horizontal keys at construction joints for shear transfer.

4. Reinforcing Steel

- A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel. Welded plain wire fabric shall be supplied in sheets and conform
- to the requirements of ASTM A185. B. Clear minimum coverage of concrete over reinforcing steel shall be as follows: Concrete placed against earth: 3"
- Formed concrete against earth: 2' Slabs:

O. No aluminum items shall be embedded in any concrete.

- 1-1/2" . Beams or Columns: Other
- All coverage shall be nominal bar diameter minimum.
- All dowels shall be the same size and spacing as adjoining main bars (splice lap 48 bar diameters or 24" minimum unless noted otherwise). . At corners of all walls, beams, and grade beams supply corner bars (minimum 2'-0" in each direction or 48 bar diameters) in outside face of wall, matching size and
- spacing of horizontal bars. Where there are no vertical bars in outside face of wall, supply 3 - #4 vertical support bars for corner bars. Bars marked continuous and all vertical steel shall be lapped 48 bar diameters (2'-0" minimum) at splices and embedments, unless shown otherwise. Splice top bars near midspan and splice bottom bars over supports, unless noted otherwise. At all holes in concrete walls and slabs, add 2 - #5 bars (opening dimension plus 96 diameters long) at each of four sides and add 2 - #5 x 5'-0" diagonally at each of four corners of hole. Openings in 8" thick walls are reinforced similar, but with 1 - #
- 5 instead of 2 #5, respectively. G. Unless otherwise covered on architectural plans or specifications, vertical control joints in concrete wall shall be spaced at a maximum of 20'-0" on center and coordinated with the architect. Every other horizontal wall reinforcing bar shall be discontinuous at control joints except heavy top and bottom bars unless noted otherwise. Provide base seal waterstop style number 772 (by Greenstreak Inc. or approved equal) on dirt face side of wall at all walls below grade.

- H. Accessories shall be as specified in latest edition of the ACI Detailing Handbook and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet.
- I. All slabs and stairs not shown otherwise shall be 6" thick with #4 bars at 12" on center each way. All exterior porches and stoops not otherwise detailed may be constructed in any standard manner, solid or hollow, but must be reinforced with #4 bars at 12" on center each way minimum. Porches shall be doweled to adjacent walls or grade beams with #4 bars at 12" on center, hooked or embedded 48 diameters into both members. Slope porches 1/8" per foot for drainage unless noted otherwise.
- J. Allow 2 tons of reinforcing bars #4 or larger to be used as directed in the field for special conditions by the engineer of record (labor for placing same to be included).

5. Structural Steel

A. All structural steel beams and columns shall be ASTM A992, grade 50 steel and all miscellaneous steel shall be ASTM A36 grade steel (except at moment connections where plates shall be ASTM A572, grade 50). Hollow Structural Sections (HSS) shall be ASTM A500, grade C. Fabrication and erection shall be in accordance with AISC 303-05 "Code of Standard Practice for Steel Buildings and Bridges" in the 13th Edition of the AISC Steel Construction Manual.

B. All welding shall conform to the recommendations of the AWS.

C. All exterior steel and connections, and brick relief angles shall be hot-dip galvanized. All bolts not otherwise specified shall be 3/4" diameter high strength (ASTM A325-N). All bolts shall be fully pretensioned. All beam connections shall be designed per the AISC Manual of Steel Construction "Framed Beam Connections" for the indicated reactions or at least 0.4 x beam total shear capacity, Vn/Omega, shown in the maximum total uniform load tables, whichever is greater; and, shall account for eccentricity when the bolt line is more than 2" from the center of the support. All connections must be two bolt minimum. Additional connection elements may not be specifically shown in the conceptual details in this set but may be required by the final connection design, such as stiffener plates, doubler plates, supplement/reinforcing plates or other connection material. Connection design and shop drawing preparation

shall be completed under the direct supervision of a professional engineer licensed in

the state the project is located and shop drawings and connection calculations shall

- bear his/her seal. E. All anchor bolts shall be 3/4" diameter, ASTM F1554, Grade 36 unless noted otherwise. Washers of minimum size and thickness for the given anchor diameter in Table 14-2 of the AISC Steel Construction Manual shall be provided at every column anchor bolt. Washers shall have a standard size hole for the anchor bolt. At braced frames washers
- shall be welded all around to the column base plate with 3/16" fillet weld. F. Allow 2.0 tons structural steel to be used as directed in field for special conditions by the engineer of record. Cost for shop drawings, fabrication, delivery, detailing, and erection to be included. 50% of structural steel allowance shall be bid as miscellaneous galvanized angle and plate.

6. Post Installed Anchors

- A. Post-installed anchors shall be used only where specified on the drawings unless approved in writing by the engineer of record. See drawings for anchor diameter spacing and embedment. Performance values of the anchors shall be obtained for specified products using appropriate design procedures and/or standards as required by the governing building code. Anchors installed in concrete shall have an ICC-ES Evaluation Service Report. Special inspection is required for all post installed anchors. The contractor shall coordinate an on-site meeting with the post installed anchor manufacturer field representative to educate the construction team on the anchor
- installation guidelines and requirements B. Mechanical anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ACI 355.2 and ICC-ES AC193. All anchors shall be installed per the anchor manufacturer's written instructions.
- C. Adhesive anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions.
- D. Mechanical anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC01. All anchors shall be installed per the anchor manufacturer's written instructions.
- E. Adhesive anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC58. All anchors shall be installed per the anchor
- manufacturer's written instructions. F. Anchors used in hollow concrete masonry shall have been tested and qualified in accordance with ICC-ES AC106 or ICC-ES AC58 as appropriate. All anchors shall be installed per the anchor manufacturer's written instructions with appropriate screen tubes used for adhesives

7. Foundations

- A. The soil investigation was prepared by Terracon, the report number is 02215169 and the telephone number is 913-492-7777 B. Structural foundations consist of a network of stright shaft auger pressure grouted piles established on moderatly weathered shale capable of safely supporting 40ksf end bearing. Each pile shall penetrate 5'-0" minimum into the moderately weathered shale. Spread footing and shallow foundations for ancillary structures are designed to bear on
- engineered fill or undisturbed soil capable of safely supporting 1,500 psf. Retaining walls are designed for an active lateral load of 50 pcf equivalent fluid pressure. D. Basement walls are designed for an at rest lateral load of 70 pcf equivalent fluid
- pressure. See General Note 3.J for wall bracing requirements. E. Contractor shall provide for dewatering at excavations from either surface water or
- F. All foundation excavations shall be inspected by a qualified soil engineer, approved by the architect and/or structural engineer, prior to placement of steel or concrete. This inspection shall be at the owner's expense. G. All concrete in the structural portion retaining the backfill shall have attained its design strength prior to being backfilled.
- H. Moisture content in soils beneath building locations should not be allowed to change after footing excavations and after grading for slabs on grade are completed. If subgrade materials become desiccated or softened by water or other conditions, recompact materials to the density and water content specified for engineered fill. Do not place concrete on frozen ground.

8. Drilled Auger Pressure-Grouted Piers

- A. Piers not otherwise indicated shall be 30" diameter.
- B. All piers shall have (4) #7x6'-0" hooked dowels unless otherwise indicated. C. Pier dowels shall extend 40 diameters above top of pier. Driving dowels into concrete
- after initial set is not allowed D. Refer to the specifications (sections for excavation and concrete) for other detailed
- requirements E. Pier concrete to have 6" slump.

9. Concrete Masonry Units

- A. Concrete block used in exterior walls or load bearing walls shall meet the requirements of ASTM C90 and have a minimum net compressive strength of 2650 psi and laid up using type N mortar such that f'm equals 2000 psi. Mortar shall be volume proportion based cement lime mortar. Proportioning shall be completed by box measure. Any block in contact with earth shall be normal weight units, laid using type "S" mortar and arouted solid.
- B. The contractor shall provide adequate temporary bracing for all masonry walls during construction. C. All concrete block shall have 9 gage (or larger) horizontal joint reinforcing (ladder
- or truss) per architectural drawings and specifications (16" maximum vertical spacing). D. Cavity wall construction shall be reinforced as designed for specific concrete block used. The horizontal joint reinforcing shall be of the ladder or truss style per specification and continuous between brick and block, as prescribed by the architectural drawings.
- E. Concrete block shall be reinforced as follows in 6", 8", 10", and 12" walls: . Vertical reinforcing shall be a minimum of 1 - #4 bar in 6" and 8" walls and 2 - #4 bars in 10" and 12" walls at 4'-0" on center, at each corner, at each door and window jamb, each side of control joints and in the end void of each length of wall. Lap splices for masonry vertical reinforcing shall be 48 bar diameters or 24" Horizontal reinforcing:
- A. Horizontal joint reinforcing as noted above. B. Continuous horizontal bars shall be included per section or detail in bond beam or optional running bond beam where noted. Where bond beams are continuous at corners of walls, supply corner bars matching size of horizontal bars (minimum 2'-0" or 40 bar diameters in each direction). F. Grout, where noted above, shall have a minimum design ultimate compressive strength
- of 2500 psi at 28 day test and 3/8" maximum aggregate size. G. Non-load bearing concrete block walls shall be isolated from adjacent structural elements with vertical 3/8" control joints and at the top of the wall with 1" air space or compressible material and support per architectural detail. H. Unless otherwise covered on architectural plans or specifications, vertical control joints
- in masonry construction shall be 3/8" wide, full height of wall. Joints shall be spaced at a maximum of 24'-0" on center and coordinated with the architect. All horizontal joint reinforcing shall be discontinuous at control joints in masonry. All bond beam horizontal reinforcing shall be continuous through control joints. I. Lintels over all openings up to 8'-0" wide in new and existing masonry walls not
- otherwise covered shall be one 6x3 1/2x5/16 angle for each 4" width of masonry. All exterior lintels to be galvanized. J. Walls shall be anchored top and bottom by dowels matching wall vertical reinforcing(unless noted otherwise) from floor slab bottom and bracing angles at the

10. Light Gage Metal Structural Framing

top, per details on the drawings.

- A. All load bearing, light gage structural studs, track, and bridging shall be of the type, size, gage, and spacing as shown on the plans, minimum.
- B. All materials shall be 33,000 psi minimum yield, except studs of 16 gage or heavier shall have a minimum yield of 50,000 psi. C. All properties, fabrication, and erection shall be in accordance with latest editions of
- the AISI "Specifications for the Design of Cold-Formed Structural Members." D. All framing components shall be cut squarely or at an angle to fit squarely against abutting members. Splicing of axially loaded members is not permitted. Members shall be held firmly in place until properly fastened. Attachments of similar components shall be by welding, screw attachment, or bolting. Wire
- tying of components is not permitted. E. Tracks shall be securely anchored to floor and overhead members. Special anchorage requirements required for wind bracing shall be as shown on the plans. F. Prior to fabrication and/or erection, the contractor shall submit shop drawings complete with detail of erection, fabrication, attachments, anchorages, lintels, etc., for review by the architect/engineer.

11. Timber and Wood Framing

criteria of the governing code.

- A. Quality and construction of wood framing members and their fasteners for load supporting
- purposes not otherwise indicated on the drawings shall be in accordance with the 2018 International Building Code. B. All studs and top and bottom plates shall be Douglas Fir No. 2 grade visually graded lumber, with an allowable fiber stress in bending of 900 psi minimum and an elastic modulus of 1,600,000psi unless noted otherwise. All joist, truss members, and headers to be No. 2 grade (min.) unless noted otherwise. All lumber for exterior decks and balconies shall be
- treated Southern Yellow Pine No. 2 grade. C. Bridging of stud bearing walls and shear walls shall be solid, matching sheathing joints. D. Joist blocking and bridging shall be solid wood or cross bridging of either wood or metal
- straps. Spacing, in any case, shall not exceed 8'-0". E. Wood members and sheathing shall be fastened with number and size of fasteners not less than that set forth in Table 2304.9.1 of the 2018 International Building Code. Floor sheathing shall be APA rated tongue and groove Sturd-I-Floor, exposure 1, glued and nailed with 10d nails or # 10 screws at 12" on center field. Sheathing of shear walls or roof diaphragms shall be edge nailed with 8d common nails at 6" on center and nailed to intermediate framing and/or blocking members with 8d common nails at 12" on center unless otherwise noted on
- the drawings. F. Sill plates shall be bolted to concrete slabs with 1/2" diameter bolts at 32" on center (UNO. re: shearwall schedule.) Provide plate washers at sill plate anchors for shearwalls per shearwall schedule. Plates in direct contact with concrete or masonry shall be preservative-
- G. All hangers, ties and connections shown are based on Simpson Strong Tie as the basis of design, provide Simpson Strong Tie or an approved equal. Joist hangers shall be equal to "LUS" for wood application and "LB" for steel weld-on application. Roof truss ties shall be equal to "H2.5A" and tie the roof truss to the top plate (provide (2) "H2.5A" diagonally across from each other when uplift load shown in truss shop submittal exceeds 600lbs). Roof girder ties shall be equal to a "LGT2", "LTG3" or "LGT4" tie (dependent on number of plies) and tie the truss girder to the top plate. Provide "H2.5A" at the top of each stud to top track when the top track has roof truss attached.
- H. Service condition dry with moisture content at or below 19% in service. I. Laminated strand lumber (LSL) shall have an allowable flexural stress (Fb) of 1,700 psi (reduced by size factor) and an elastic modulus (E) of 1,300,000 psi. J. Laminated veneer lumber (LVL) shall have an allowable flexural stress (Fb) of
- 2.600 psi (reduced by size factor) and an elastic modulus (E) of 2.000.000 psi. K. Parallel Strand Lumber (PSL) shall have an allowable flexural stress (Fb) of 2,900 psi (reduced by size factor) and an elastic modulus (E) of 2,000,000 psi. ((E) = 2,200,000 psi for members > 18").
- L. Glulams shall be 24F-V8 or better with an allowable flexural stress (Fb) of 2,400 psi and an elastic modulus (E) of 1,800,000 psi. Exterior glulams shall be moisture-resistant treated. M. Pre-engineered wood trusses shall be designed in accordance with the Truss Plate Institute's national design standard for metal-plate connected wood truss construction (ANSI/TPI-1 latest edition). Trusses shall be designed and manufactured by an authorized member of the Wood Truss Council of America (WTCA). Truss design shall conform to specified codes, allowable stress increases, deflection limitations and other applicable
- N. Truss shop drawings showing complete erection and fabrication details and calculations (including connections) shall be submitted to the project architect/engineer for review prior to fabrication and/or erection. Calculations and layout plan shall bear the seal of a professional engineer, registered in the state of the project location. Layout plan shall incldue truss locations, spacing and all hanger designations used to support trusses to beams or other trusses. Calculations shall indicate max reactions in all directions, number of plies for the truss and dead, live and total load deflections along with a list or key of all standard and nonstandard utilized load combination. Shop drawings shall also be submitted to the local
- government controlling agency when requested by that agency. O. All trusses shall be securely braced both during erection and permanently, as indicated on the approved truss design drawings and in accordance with TPI's commentary and recommendations for handling, installing and bracing metal-plate connected wood trusses (HIB-91, booklet) and the latest edition of ANSI/TPI-1.
- members together and fastening truss members to their supports. Metal connector plates shall be manufactured by a member of the Wood Truss Council of America (WTCA) and shall be 20 gauge minimum. Connector plates shall meet or exceed ASTM A653, grade 33, with ASTM A924 galvanized coating designation G60. Q. Provide truss space directly above and centered over HVAC closets. Refer to Architectural and MEP drawings for exact locations. R. Shipment, handling, and erection of trusses shall be by experienced, qualified persons and

P. The truss manufacturer shall supply all hardware and fasteners for joining truss

- shall be performed in a manner so as not to endanger life or property. Apparent truss damage shall be reported to the truss manufacturer for evaluation prior to erection. Cutting or alteration of trusses is not permitted. S. Pre-Engineered Floor Trusses: Top Chord Dead Load = 30 psfTop Chord Live Load = Per General Note 5B Bottom Chord Dead Load = 10 psf
- Live Load Deflection = L/480; (1/2" max) Total Load Deflection = L/360Roof Truss Design Criteria: = 15 psf (TPO Roof) Top Chord Dead Load = 20 psf (Plus Rooftop Equipment) Top Chord Live Load Top Chord Snow Load = 20 psf or 14 psf plus Drift Bottom Chord Dead Load
- Bottom Chord Live Load = 5 psf Live Load Deflection = L/360Total Load Deflection = L/300U. Roof trusses shall be designed per IBC 2018 for net uplift resulting from wind loading as calculated using components and cladding loading. Top and bottom chord dead load used in
- combination with wind uplift shall be 5psf for each chord. V. Construction bracing shall be provided by the contractor as required to keep the building and W. Structural members shall not be cut for pipes, etc., unless specifically detailed. Nothing and boring of studs and top of plates shall conform to the provisions of section 2308.9.10 and 2308.9.11 of the IBC. Where top plates or sole plates are cut for pipes, a metal tension tie with minimum 0.058 inches thick and 1 1/2" inches wide shall be fastened to each plate across and to each side of the opening with not less than (6) 16d nails, in accordance with
- section 2308.9.8 of the IBC. X. All fasteners for wood to wood connections and wood connectors shall be as indicated in structural drawings or manufacturer literature to achieve full capacity of connector. Alternate fasteners may be submitted as a substitution request. Submittal must show that alternative fasteners will not reduce the capacity of the connection.

12. Precast Concrete Members

- A. The contractor/supplier is responsible for the design of all the precast members and connection between them and other structural members. Submit design calculations, sealed by an engineer licensed in the state of the project location, for review by the
- architect/engineer of record. B. All precast members are to be designed in accordance with ACI 318-11, 2012 IBC and other applicable codes, standards (see specs) and design criteria shown on design
- C. Precast concrete members shall conform to the 2012 IBC for the required fire ratings (refer to architects documents). D. All wall panels should be designed for building wind loads, seismic loads, gravity loads, and transmit these loads to the foundation through properly designed connections.
- E. Provide blockouts and openings for mechanical/electrical equipment. Refer to mechanical/electrical documents. F. Shop drawings shall be complete and shall include a layout plan, fabrication details, estimated camber, connection and anchorage details and member identification marks. Identification marks shall appear on manufactured units to facilitate correct field

13. Deferred Submittal and Shop Drawing

4. Grout mix designs (for CMU).

need not be submitted.

- A. Bob D. Campbell and Company, Inc. will review the General Contractor's (GC) shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to be a properly functioning integral element of the overall structural system designed by Bob D. Campbell and Company, Inc.
- B. Deferred submittals shall be submitted to the architect of record for review who shall forward to the building official for review and approval. Design calculations for deferred sub mittals shall be submitted at the same time as the shop drawings for review. Design calculations shall be prepared and sealed by a Professional Engineer licensed in the state of the project. The deferred submittal items shall not be installed until the deferred
- submittal documents have been approved by the building official. Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall: 1. Review each submission for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs
- 2. Review and approve each submission. . Stamp each submission as approved. D. Bob D. Campbell and Company, Inc. shall assume that no submission comprises a

incidental thereto, all of which are the sole responsibility of the GC.

- variation unless the GC advises Bob D. Campbell and Company, Inc. with written E. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has met the above requirements. Bob D. Campbell and Company, Inc. shall return without comment unrequired material
- or submissions without GC approval stamp. F. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify the GC. 1. Concrete mix designs and material certificates including admixtures and
- compounds applied to the concrete after placement. Reinforcing steel shop drawings including erection drawings and bending details.Bar list will not be reviewed for correct quantities. 3. Elevations of all reinforced concrete masonry walls at a scale no smaller than 3/8" = 1'-0" showing all required reinforcing.
- 6. Structural steel shop drawings including erection drawings and piece details. Include joist, decking and connector submittals. Include miscellaneous framing specified on the structural drawings, but do not submit framing specified on nonstructural drawings for Bob D. Campbell and Company, Inc. review. Defferred Submittal: Railings and guardrails with sealed calculations

5. Construction and control joint plans and/or elevations.

- 8. Defferred Submittal: Metal stair framing with sealed calculations 9. Defferred Submittal: Exterior cold-formed metal framing 10. Defferred Submittal: Exterior curtain wall 11. Deferred Submittal: Structural steel connection design calculations submitted concurrently with structural steel shop drawings.
- 12. Miscellaneous anchors shown on the structural drawings. 13. Deferred Submittal: Wood truss design calculations and detailed erection and fabrication drawings. Standard stick framing shop drawings need not be submitted. 14. Standard details and bridging information for light gage metal framing. Erection plans and details for light gage metal joists and lintels spanning more than 6'-0" shall be submitted. Standard wall framing

15. Deferred Submittal: Augured pile foundation plans and details.

and connection details. 17. Deferred Submittal: Precast concrete connection design calculations. 18. Deferred Submittal: Cold-Formed metal framing for exterior walls.

16. Deferred Submittal: Precast concrete shop drawings including erection drawings

14. Statement of Structural Special Inspections

- A. The structural design for this project is based on completion of special inspections during construction in accordance with section 1704 of the International Building Code. The owner shall employ one or more qualified special inspectors to provide
- the required special inspections. B. The special inspector shall furnish inspection reports to the building official, owner. architect and structural engineer, and any other designated person.
- correction, then, if uncorrected, to the proper design authority, building official and
- the building code.
- periodic) as defined within the referenced section or standard listed below. The General Contractor shall provide notification to the inspector when items requiring inspection are ready to be inspected and provide access for those inspections. 1. Shop Fabrication – structural steel per Section 1704.2.5 unless AISC certified
 - TPI certified shop
- of AISC 341 Chapter J (as referenced by AISC 360)
- a. Reinforcing Steel Placement Reinforcing Steel Welding
- Concrete Placement
- Prestressed Concrete Stressing and Grouting Erection of Precast
- Verification of In-situ Concrete Strength Prior to Stressing Post-Tensioned
- requirements of TMS 402/ACI530/ASCE5 and TMS602/A530.1/ASCE6 Level B Verification of Soils per Table 1705.6
- Inspections and Tests of Cast-In-Place Deep Foundation per Table 1705.8 Wood Lateral System (periodic) a. Wood shearwalls (include sheathing, rim board and bottom plate
- b. Portal frames Shear wall and portal frame holdowns
- b. Headers and jambs (random sampling) Bearing walls (random sampling) Connector/hardware installation (random sampling)

e. Floor and roof trusses (random sampling)

elsewhere in the construction document package.

- 15. Copyright and Disclaimer A. All drawings in the structural set (S-series drawings) are the copyrighted work of Bob D. Campbell and company, Inc. These drawings may not be photographed, the owner, architect, and general contractor for coordination, bidding, and
- B. I, Christopher A. Beverlin, P.E., registered engineer and a representative of Bob D. Campbell and Company, Inc., do hereby accept professional responsibility as required by the professional registration laws of this state for the structural design drawings consisting of S-series drawings. I hereby disclaim responsibility for all other drawings in the construction document package, they being the responsibility

STRUCTURAL ARREVA

FF ABOVE FINISHED FLOOR GA GAGE PS RCH ALTERNATE GALV GALVANIZE(D) PS RCH ARCHITECTURAL GEN GENERAL QT LOG BUILDING GR GRADE RA BOTTOM OF GRBM GRADE BEAM RE M BEAM HORIZ HORIZONTAL RE DITT BOTTOM HSS HOLLOW STRUCTURAL SECTION RE GR GEARL INFO INFORMATION RE CAMBER INFO INFORMATION RE CONCRETE DECK TYPE INT INTERIOR RI DIT CONSTRUCTION/CONTROL JOINT JST JOINT SC L CENTERLINE K KIPS PER SQUARE FOOT SE COLUMN KSI KIPS PER SQUARE INCH SH DONC CONCRETE LBS,# POUNDS SII DONN CONNECTION Ld DEVELOPMENT LENGTH SJ DONT CONTINUOUS LL LIVE LOAD SL DOV, CVR COVER LLV LONG LEG HORIZONTAL SC DOV, CVR COVER LLV LONG LEG HORIZONTAL SC DOV, CVR COVER LLV LONG LEG HORIZONTAL SC DONBLE LONG LONG-SUIDINAL SE T DETAIL LSLT LONG-SLOTTED HOLE TRANSVERSE SF M DIMENSION M MOMENT FORCE SC M DAWING MECH MECHANICAL SC DEAD LOAD MAX MAXIMUM SS MG DRAWING MECH MECHANICAL ST JE EXPANSION JOINT MISC MISCELLANEOUS ST MSED EMBEDMENT, EMBEDDED MTL METAL SO DORD CORDINER IN MINIMUM ST MSED EMBEDMENT, EMBEDDED MTL METAL SO DORD EDGE OF DECK NS NEAR SIDE THE DORN CONCENTER OF RECORD NTS NOT TO SCALE SC DORD EDGE OF DECK NS NEAR SIDE THE DORN MINIMUM ST MSEC HOGINEER TO DORD CONCENTER OF RECORD NTS NOT TO SCALE ST DETAIL ST METAL ST MOMENT OF OWNERT OF ST MED EMBEDMENT, EMBEDDED MTL METAL ST MOMENT ON SERVENCE ST MOMENT OF OWNERT OF ST METAL ST MOMENT OF OWNERT OF ST MOMENT OWNERT OWNERT OF ST MOMENT OWNERT		STRUC	<u>۱UR/</u>	AL ABBREVIATIONS	
D-# FLOOR DECK TYPE PAF POWDER ACTUATED FASTENER W DN FOUNDATION PC PRECAST / PILE CAP W FAR FACE PCF POUNDS PER CUBIC FOOT W	DTL FF LT RCH LD ONC ONT ONC ONT OON OOV, CV BET IA WG AF J., ELEV NGR OOR OOR OOR OOR OOR OOR OOR OOR OOR O	AT AND ROUND, DIAMETER ADDITIONAL ABOVE FINISHED FLOOR ALTERNATE ARCHITECTURAL BUILDING BOTTOM OF BEAM BOTTOM BEARING CAMBER CONCRETE DECK TYPE CONSTRUCTION/CONTROL JOINT COMPLETE JOINT PENETRATION CENTERLINE CONCRETE MASONRY UNIT COLUMN CONCRETE CONNECTION CONTINUOUS COORDINATE COVER DOUBLE DETAIL DIAMETER DIMENSION DEAD LOAD DRAWING EACH EACH FACE EXPANSION JOINT ELEVATION EMBEDMENT, EMBEDDED ENGINEER EDGE OF DECK ENGINEER OF RECORD EDGE OF SLAB EQUAL EQUIPMENT EACH WAY EXPANSION EXTERIOR	FLR FS FTG GALV GEN GRBM HOSS IF INT JT KSSI LB LLH LLV LSLT MAX MEGR MIN MISC MTL NS NT N	FLOOR FAR SIDE FOOTING FIELD VERIFY GAGE GALVANIZE(D) GENERAL GRADE GRADE BEAM HORIZONTAL HOLLOW STRUCTURAL SECTION INSIDE FACE INFORMATION INTERIOR JOIST JOINT KIPS (1000 LBS) KIPS PER SQUARE FOOT KIPS PER SQUARE INCH POUNDS DEVELOPMENT LENGTH LIVE LOAD LONG LEG HORIZONTAL LONG LEG VERTICAL LONGITUDINAL LONG-SLOTTED HOLE TRANSVERSE LIGHTWEIGHT MOMENT FORCE MAXIMUM MECHANICAL MANUFACTURER MINIMUM MISCELLANEOUS MASONRY METAL NEAR FACE NEAR SIDE NOT TO SCALE NORMAL WEIGHT ON CENTER OUTSIDE FACE OPENING OPPOSITE OVERSIZED HOLE	PLP PS OF REER REER RESOLUTION OF SECULIAR REER REER RESOLUTION OF SECULIAR REER REER REER RESOLUTION OF SECULIAR REER REER REER RESOLUTION OF SECULIAR REER REER REER REER REER REER REER R
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FF FIN	FAR FACE PCI FINISH PEI	
STR	UCTURAL DECK & SL	AB SCHEDULE
MARK	DESCRIPTION	
FD-1	1" TO 1 1/4" GYPCRETE ATOP 23/32" A EXP 1 SHEATHING. SHEATHING SHAL SHANK NAILS OR #10 SCREWS AT 6"6	LL BE GLUED AND NAILED W/ 8d RING
CD-1		00psi, AIR-ENTRAINED) R AT 1.5 LBS/CU. YD. ATOP WATERPROOFING ' EXTERIOR GRADE PLYWOOD SHEATHING
CD-2	3" NORMAL WEIGHT CONC. SLAB (450 REINFORCE WITH CELLULOSE FIBER STAIR LANDING	
CD-3		00psi, AIR-ENTRAINED) R AT 1.5lb/cu. yd. ATOP 2" RIGID INSULATION ATOP PODIUM SLAB. SLOPE TO DRAIN PER
SOG-1	3/4" CLEAN GRANULAR LEVELING CO	NF ATOP 15 MIL VAPOR BARRIER ATOP 4" OF DURSE ATOP SUITABLE SUBGRADE MATERIA SLAB EL. = PER PLAN, SLOPE TO DRAIN
SOG-2		VF ÁTOP 4" OF 3/4" CLEAN GRANULAR SUBGRADE MATERIAL PER GEOTECH
SOG-3		NAÝ BOTTOM ATOP 4" OF 3/4" CLEAN P SUITABLE SUBGRADE MATERIAL PER
SOG-4		WAY ATOP 4" OF 3/4" CLEAN GRANULAR SUBGRADE MATERIAL PER GEOTECH
RD-1		G ATTACHED WITH #10 SCREWS AT PROVIDE FRT TREATED PLYWOOD AT /INGS FOR LOCATION AND EXTENTS)
DD 0	23/32" APA RATED T&G STURD-I-FLO	OOR, EXP 1 SHEATHING. SHEATHING

 FD = FLOOR DECK TYPE. 2. CD = CONCRETE DECK TYP.

4. RD = ROOF DECK TYP.

MID-SPAN OF BALCONY (8'-0" MAX SPACING) FILL JOINT w/ SEALANT.

C. All discrepancies shall be brought to the immediate attention of the contractor for

structural engineer. D. The special inspector shall submit a final signed report stating that the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with the approved plans and specifications and the applicable workmanship provisions of

E. The following inspections and tests are required with the frequency (continuous or

2. Shop Fabrication – pre-engineered wood trusses per Section 1704.2.5 unless

3. Shop Fabrication – precast concrete per Section 1704.2.5 unless PC certified 4. Steel Construction per Section 1705.2 and the quality assurance requirements

5. Concrete Construction per Section 1705.3 and Table 1705.3

c. Cast in Place Anchors d. Post Installed Anchors e. Design Mix Verification Concrete Sampling and Testing

Concrete Curing

I. Formwork Shape, Location and Dimensions 6. Masonry Construction per Section 1705.4 and the quality assurance

attachments)

d. Shear wall tension rod system 10. Wood Gravity Framing and Placement (adjust frequency of random sampling where indicated as required) a. Heavy timber/SCL/glulam beams and supports (periodic)

- traced, or copies in any manner without the written permission of Bob D. Campbell and Company, Inc. Exception: Original drawings may be printed for distribution to construction. Subcontractors may not reproduce these drawings for any purpose
- of other design professionals whose seals and signed statements may appear

<u>IATIONS</u>		
	PERP PL PLF PJP PSF PSI QTY RAD RD-# REF	PERPENDICULAR PLATE POUNDS PER LINEAR FOOT PARTIAL JOINT PENETRATION POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH QUANTITY RADIUS ROOF DECK TYPE REFERENCE
RAL SECTION	REINF REQD REV RLL RTU SC SCHED	REINFORCEMENT REQUIRED REVISION ROOF LIVE LOAD ROOF TOP UNIT SLIP CRITICAL
OOT	SECT	SECTION ` ´
NCH	SHT SIM	SHEET SIMILAR
GTH	SJ	SAW JOINT
TAL	SL SOG	SNOW LOAD SLAB-ON-GRADE
-	SOG-#	SLAB-ON-GRADE TYPE
E TRANSVERSE	SPCG SPEC	SPACING SPECIFICATION
L INANOVERSE	SPRT	SUPPORT
	SQ	SQUARE
	SS SSLT	STAINLESS STEEL SHORT-SLOTTED HOLE TRANSVER
	STD	STANDARD
	STIFF	STIFFENER
	STIR STL	STIRRUP STEEL
	STRUCT	STRUCTURE, STRUCTURAL
	T/	TOP OF
	THRU TOS	THROUGH TOP OF STEEL, TOP OF SLAB
	TRANS	TRANSVERSE
	TYP	TYPICAL
	UNO V	UNLESS NOTED OTHERWISE SHEAR FORCE
	VERT	VERTICAL
	W/	WITH
) FASTENER	W/0 WF	WITHOUT WIDE FLANGE
)	WL	WIND LOAD
FOOT	WP	WORK POINT

PC PRECAST / PILE CAP PCF POUNDS PER CUBIC FOOT PEMB PRE-ENGINEERED METAL BUILDING
ECK & SLAB SCHEDULE
ETE ATOP 23/32" APA RATED T&G STURD-I-FLOOR, SHEATHING SHALL BE GLUED AND NAILED W/ 8d RING 10 SCREWS AT 6"o.c. AT EDGES & 12"o.c. AT FIELD.
T CONC. SLAB (4500psi, AIR-ENTRAINED) CELLULOSE FIBER AT 1.5 LBS/CU. YD. ATOP WATERPROOFING RCH.) ATOP 15/32" EXTERIOR GRADE PLYWOOD SHEATHING .) RE: NOTE 5
T CONC. SLAB (4500psi, AIR-ENTRAINED) CELLULOSE FIBER AT 1.5lb/cu. yd. ATOP PRECAST
T CONC. SLAB (4500psi, AIR-ENTRAINED) CELLULOSE FIBER AT 1.5lb/cu. yd. ATOP 2" RIGID INSULATION DFING PER ARCH ATOP PODIUM SLAB. SLOPE TO DRAIN PER
00psi) 6x6-W2.9xW2.9 WWF ATOP 15 MIL VAPOR BARRIER ATOP 4" OF LAR LEVELING COURSE ATOP SUITABLE SUBGRADE MATERIA ECIFICATIONS. T/SLAB EL. = PER PLAN, SLOPE TO DRAIN
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EXP 1 SHEATHING ATTACHED WITH #10 SCREWS AT 12"o.c. AT FIELD. (PROVIDE FRT TREATED PLYWOOD AT R TO ARCH DRAWINGS FOR LOCATION AND EXTENTS)
T&G STURD-I-FLOOR, EXP 1 SHEATHING. SHEATHING

3. SOG = SLAB-ON-GRADE TYP.

5. PROVIDE 1" DEEP TOOLED CONTROL JOINT (TRANSVERSE DIRECTION) @

SHALL BE GLUED AND NAILED W/ 10d RING SHANK NAILS OR #10 SCREWS

AT 4"o.c. AT EDGES & 12"o.c. AT FIELD WITH ALL EDGES BLOCKED

1.25B BUILDING B ROOF FRAMING PLAN 1.26B BUILDING B SHEARWALL PLAN 1.31C BUILDING C FOUNDATION PLAN 1.32C BUILDING C SECOND FLOOR FRAMING PLAN 1.33C BUILDING C THIRD FLOOR FRAMING PLAN 1.34C | BUILDING C FOURTH FLOOR FRAMING PLAN 1.35C BUILDING C ROOF FRAMING PLAN 81.36C BUILDING C SHEARWALL PLAN
S1.41D BUILDING D FOUNDATION PLAN 1.42Da BUILDING D PODIUM SLAB REINFO 1.43D BUILDING D THIS 1.44D BUILDING D F 1.46D BUILDING D ROOF FRAMING PLAN 1.47D BUILDING D SHEARWALL PLAN 1.51E BUILDING E FOUNDATION PLAN 1.52Ea BUILDING E PODIUM SLAB REINFORCEMENT PLAN 1.52Eb | BUILDING E PODIUM STUD RAIL & SLAB GEOMETRY PLAN | |2 1.53E | BUILDING E THIRD FLOOR FRAMING PLAN 1.54E BUILDING E FOURTH FLOOR FRAMING PLAN 1.55E BUILDING E FIFTH FLOOR FRAMING PLAN 1.56E BUILDING E ROOF FRAMING PLAN 1.57E BUILDING E SHEARWALL PLAN 1.61F BUILDING F FOUNDATION PLAN 1.62Fa BUILDING F PODIUM SLAB REINFORCEMENT PLAN 1.62Fb | BUILDING F PODIUM STUD RAIL & SLAB GEOMETRY PLAN | | 1.63F BUILDING F THIRD FLOOR FRAMING PLAN 1.64F BUILDING F FOURTH FLOOR FRAMING PLAN 1.65F BUILDING F FIFTH FLOOR FRAMING PLAN 1.66F BUILDING F ROOF FRAMING PLAN 1.67F BUILDING F SHEARWALL PLAN S1.71G GARAGE FOUNDATION PLAN S1.72G GARAGE SECOND FLOOR FRAMING PLAN S1.73G GARAGE THIRD FLOOR FRAMING PLAN S1.74G GARAGE FOURTH FLOOR FRAMING PLAN S1.75G GARAGE FIFTH FLOOR FRAMING PLAN S1.76G GARAGE SNOW LOADING PLAN S2.00 STAIR FRAMING - BUILDING A 2.01 STAIR FRAMING - BUILDING B 2.02 STAIR & ELEVATOR FRAMING - BUILDING C 2.03 STAIR & ELEVATOR FRAMING - BUILDING D 2.04 STAIR & ELEVATOR FRAMING - BUILDING E 2.05 STAIR FRAMING - BUILDING F 2.10 STAIR FRAMING DETAILS 2.11 | ELEVATOR FRAMING DETAILS 2.20 BALCONY FRAMING PLANS

Sheet

Number

S0.10

GENERAL NOTES

0.04 STEEL SCHEDULES

CMU DETAILS

1.01 FOUNDATION PLAN

1.06 ROOF FRAMING PLAN

CONCRETE SCHEDULE

1.02 SECOND FLOOR FRAMING PLAN

1.04 FOURTH FLOOR FRAMING PLAN

1.11A BUILDING A FOUNDATION PLAN

1.15A BUILDING A ROOF FRAMING PLAN

1.16A BUILDING A SHEARWALL PLAN

1.21B BUILDING B FOUNDATION PLAN

1.12A BUILDING A SECOND FLOOR FRAMING PLAN

1.14A BUILDING A FOURTH FLOOR FRAMING PLAN

1.22B | BUILDING B SECOND FLOOR FRAMING PLAN

1.24B BUILDING B FOURTH FLOOR FRAMING PLAN

1.23B | BUILDING B THIRD FLOOR FRAMING PLAN

1.13A BUILDING A THIRD FLOOR FRAMING PLAN

1.03 THIRD FLOOR FRAMING PLAN

1.05 | FIFTH FLOOR FRAMING PLAN

WOOD SCHEDULES & TYPICAL DETAILS

0.03 WOOD SCHEDULES & TYPICAL DETAILS

WOOD SHRINKAGE & MOVEMENT

STRUCTURAL SHEET LIST

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Sheet Name

Current

Revision

Revision

Date

9.28.22

7.20.22

7.20.22

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WELDED WIRE FABRIC

3.07 FOUNDATION DETAILS 3.08 FOUNDATION DETAILS 3.11 CONCRETE FRAMING DETAILS 3.12 CONCRETE FRAMING DETAILS 3.15 | SHEAR RAIL DETAILS 3.30 WOOD FLOOR FRAMING DETAILS 3.31 WOOD FLOOR FRAMING DETAILS 3.41 WOOD ROOF FRAMING DETAILS

WOOD ROOF FRAMING DETAILS

PRECAST GARAGE FRAMING DETAILS

PRECAST GARAGE FRAMING DETAILS

S3.45 WOOD FIREWALL DETAILS

2.21 BALCONY FRAMING PLANS

S2.22 BALCONY FRAMING PLANS

3.04 | FOUNDATION DETAILS

3.05 | FOUNDATION DETAILS

3.06 FOUNDATION DETAILS

BALCONY FRAMING DETAILS

TYPICAL FOUNDATION DETAILS

GARAGE FOUNDATION DETAILS

33.03 APARTMENT FOUNDATION DETAILS

PILE & PODIUM FOUNDATION DETAILS

G3.60 CFMF DETAILS SPAN DIRECTION OF DECK - DECK TYPE PER SCHEULE ON S0.01 HSS 6x6x1/4 COLUMN SIZE

SHEARWALL HOLDDOWN TYPE PER SCHEDULE ON S0.03

— BASE PLATE MARK - SEE SCHEDULE ON SHEET S0.04 BEAM OR HEADER PER SCHEDULE ON S0.02 (A#-#u) UPSET BEAM OR HEADER PER SCHEDULE ON S0.02 BEARING WALL TYPE PER SCHEDULE ON S0.02

NUMBER OF WALL STUDS IN STUD PACK NUMBER OF JACK STUDS/KING STUDS IN STUD PACK PLAN NOTE PER SCHEDULE ON S0.02

SHEARWALL TYPE PER SCHEDULE ON S0.03

SHEAR RAIL TYPE PER SCHEDULE ON S3.15

CONCRETE SHEARWALL TYPE PER SCHED ON S0.10 AMOUNT OF UPWARD POSITIVE CAMBER PILE CAP SIZE PER SCHEDULE ON S3.02

PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08 10.18.24 Issued For: GARAGE PERMIT **REVISIONS** 6.08.22 Permit Response

7.11.22 ADDENDUM 1

7.20.22 ADDENDUM 2

4 9.28.22 ASI 1

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REGISTRATION



PROJECT TEAM ARCHITECT FINKLE+WILLIAMS ARCHITECTURE CIVIL **GBA ENGINEERS** LANDSCAPE LAND 3 BOB D. CAMPBELL STRUCTURAL LATIMER SOMMERS **PLUMBING** LATIMER SOMMERS **MECHANICAL**

FIRE PROTECTION LATIMER SOMMERS

ELECTRICAL

CONTRACTOR

LATIMER SOMMERS

CONSTRUCTORS

က စ

GENERAL NOTES

SHEET TITLE

TYPICAL CMU WALL REINFORCING AT OPENINGS

LEGEND:

- 1 FULL HEIGHT VERTICAL BARS AS JAMB REINFORCING IN FIRST 2 CELLS ADJACENT TO OPENING. REINFORCE EACH CELL WITH SIZE & QUANTITY OF BAR TO MATCH WALL REINFORCING (1 BAR TYPICAL IN 8" WALLS AND 2 BARS TYPICAL IN 12" WALLS).
- 2 LINTEL REINFORCING PER SECTION C. EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).
- 2-#5 CONTINUOUS HORIZONTAL BARS AS SILL REINFORCING IN 8" COURSE BELOW OPENING (U.N.O.). EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).
- 4 FULL HEIGHT VERTICAL BARS PER MASONRY VERTICAL REINFORCING SCHEDULE LOCATED IN END CELL AT

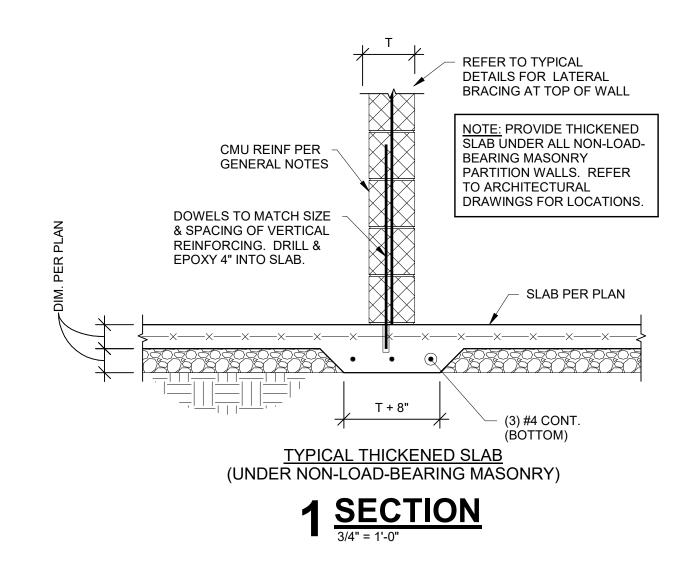
EACH SIDE OF VERTICAL WALL CONTROL JOINTS.

- GENERAL CRITERIA: (SECTION A CONTINUED): 1. VERTICAL REINFORCING BARS SHALL BE DOWELED TO FOUNDATION WITH A DOWEL OF MATCHING SIZE
- 2. CONTRACTOR SHALL COORDINATE AND VERIFY OPENINGS IN MASONRY WALLS. OPENINGS SHALL BE
- DETAILED ON REINFORCING STEEL SHOP DRAWING ELEVATIONS. VERTICAL CONTROL JOINTS IN MASONRY WALLS SHALL BE 3/8" WIDE, FULL HEIGHT OF WALL. JOINTS SHALL BE SPACED AT A MAXIMUM OF 24'-0" ON CENTER AND NOT LESS THAN 2'-0" FROM THE EDGE OF ANY OPENING. ALL HORIZONTAL JOINT REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS. ALL BOND BEAM HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL JOINTS. CONTRACTOR SHALL COORDINATE AND VERIFY ALL CONTROL JOINT LOCATIONS.

WALL THICKNESS LOCATION VERTICAL REINF. (IN GROUTED CELLS) SPACING							
8"	GARAGE ATOP PRECAST	1- #5	48"oc				
8"	EXT WALLS BELOW PODIUM	1- #5	32"oc				
8"	ELEVATOR	1- #5	32"oc				

- A.) IN THE FIRST 2 CELLS ADJACENT TO EACH OPENING B.) IN THE END CELLS ON EACH SIDE OF VERTICAL CONTROL JOINTS C.) IN THE END CELLS OF EACH LENGTH OF WALL D.) AT EACH CORNER OF WALLS
- ALL MASONRY VOIDS AND BOND BEAMS TO BE GROUTED SHALL BE FREE OF DEBRIS AND MORTAR DROPPINGS PRIOR TO GROUTING. ANY MASONRY w/ DROPPINGS OR DEBRIS OBSERVED IN VOIDS SHALL BE REJECTED.

A CMU WALL ELEVATION



ADJUSTMENTS TO DIMENSIONS TO PROVIDE

ACCEPTABLE, BUT ALL ADJUSTMENTS SHALL

CONSTRUCTION AND SHALL NOT EXCEED 1/4".

8" CMU WALI

GROUT VOID (TYP.).

FOR 12" CMU WALL

MAXIMUM OF 32" o.c.

GALVANIZED.

→ CMU WALL CENTERLINE

WIRE (MIN.) & HOT DIP

LAPPING BAR LOCATION

SINGLE BAR REINFORCING

FOR NEXT LIFT (TYP.)

FOR 8" CMU WALL

RECONSOLIDATE GROUT w/

DOUBLE BAR REINFORCING

MECHANICAL VIBRATOR (TYP.)

REBAR POSITIONERS SHALL BE

PROVIDED TO SUPPORT BOTH ENDS OF EACH BAR AND AT A

POSITIONER SHALL BE 9 GA.

CONSOLIDATE &

DEFINED CLEAR GROUT COVER ARE

BE APPROVED BY ENGINEER PRIOR TO

NOTE: REINFORCING

SHALL BE PLACED IN

POSITIONEERS PRIOR

TO GROUTING.

MORTAR CMU

ADJACENT TO

VOIDS (TYP.)

1 BAR DIAMETER

NOTE: VERTICAL

REINFORCING

SHALL BE +/-1/4"

FROM LOCATIONS

ALL MORTAR PROJECTIONS

INTO GROUTED VOIDS

SHALL BE LESS THAN 1/2"

BEYOND INSIDE FACE OF

PLACEMENT

NOTED.

MASONRY.

XXXXXX

NOTE: ALL MASONRY VOIDS AND BOND BEAMS TO

TYPICAL REBAR POSITIONING DETAIL

B SECTION 1 1/2" = 1'-0"

BE GROUTED SHALL BE FREE OF DEBRIS AND

VOIDS SHALL BE REJECTED.

MORTAR DROPPINGS PRIOR TO GROUTING. ANY MASONRY w/ DROPPINGS OR DEBRIS OBSERVED IN

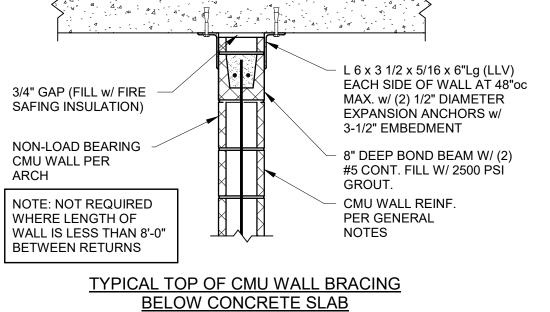
CLEAR GROUT

COVER

GROUTED

WEBS





TYPICAL MASONRY REINFORCING NOTE:

ALL INTERIOR & EXTERIOR MASONRY WALLS SHOWN ON ARCHITECTURAL

BOND BEAMS (2 - #5 BOTTOM) AT BOTTOM COURSE, TOP COURSE, JOIST

BEARING ELEVATION AND AT 8'-0" MAXIMUM O.C. AND VERTICALLY AS

RE: DETAILS "A" THROUGH "E" ON THIS SHEET.

REBAR POSITIONER

OR SUPPORT @ 48"

(2) #4 CONT. TOP

48"oc

(2) #4 CONT.

WALL

THICKNESS

PER PLAN ²

OPENINGS UP TO 4'-0"

REINF. CHAIR

SUPPORT @

o.c. MAX.

AND STRUCTURAL DRAWINGS ARE TO BE REINFORCED HORIZONTALLY WITH

INDICATED ON DRAWINGS. THESE WALLS ARE TO BE ANCHORED TOP AND

BOTTOM TO THE FOUNDATION, FLOOR, OR ROOF PER TYPICAL DETAILS. THE

VERTICAL REINFORCING IS CONTINUOUS (IN 6'-6" MAXIMUM LENGTHS, LAPPED 2'-6" MINIMUM). FILL BLOCK CELLS AND BOND BEAMS WITH 2,500psi GROUT.

REBAR POSITIONER

2-#6 CONT.

(TOP)

OR SUPPORT @ 48"

o.c. MAX.

CONT. TOP

GROUT COURSES

SIMULTANEOUSLY

#3 @8"o.c. EACH

48"oc

(2) #5 CONT.

BOTTOM

WALL

THICKNESS

PER PLAN

OPENINGS 4'-0" TO 7'-4"

TYPICAL LINTELS AT ALL CMU WALLS (U.N.O.)

- CONCRETE SLAB

SHOWN)

(REINFORCING NOT

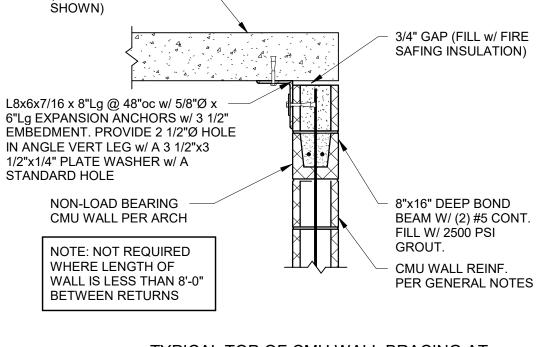
C SECTION

FACE w/ 90 DEGREE

HOOK @ EACH END

REINF. CHAIR

SUPPORT @



(2) #6 CONT.

ВОТТОМ

CONCRETE SLAB

(REINFORCING NOT

WALL

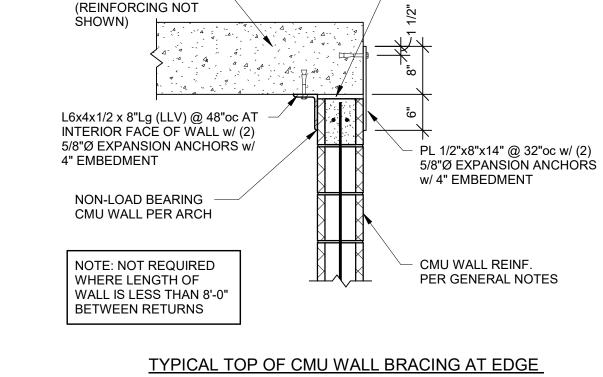
PER PLAN

OPENINGS 7'-4" TO 12'-0"

THICKNESS

TYPICAL TOP OF CMU WALL BRACING AT EDGE ON CONCRETE SLAB @ INTERIOR WALL

2B <u>SECTION</u>



- 3/4" GAP (FILL w/ FIRE SAFING INSULATION)

- #2 TIES @8"oc THROUGH

COLUMN HEIGHT PLUS 2'-0" ABOVE AND BELOW

OPENING. TIES SHALL BE

SINGLE LAYER OF TIE

HORIZONTAL MORTAR

JOINT. CUT WEBS OF

RECEIVE TIES WHERE

CONFLICTS OCCUR.

(2) TYPICAL VERTICAL

HEIGHT OF WALL)

CONCRETE SLAB

BARS PER VOID (FULI

BLOCK AS REQUIRED TO

FABRICATED TO MAINTAIN A

REINFORCING WITHIN THE

"KNOCKOUT " (K.O.) or TROUGH BOND

BEAM BLOCK (TYPICAL UNIT EXCEPT @

DOOR OPENINGS; SOLID BOTTOM BOND

STOP (RE: SPECS.) UNDER K.O. BOND

BE REINFORCED AND GROUTED.

SLOTS

BEAM SHALL BE USED). PROVIDE GROUT

BEAMS OVER CELLS WHICH ARE NOT TO

TOP BOND BEAM

OR K.O. BLOCK

SHOWN FOR

CLARITY)

(REINFORCING NOT

- SAW CUT OR

SPECIAL BLOCK

TYPICAL BOND BEAM DETAIL AT CORNER OF CMU WALL

D DETAIL 3/4" = 1'-0"

COLUMN DIMENSIONAL RANGE

16" MIN. TO 40" MAX.

<u>TYPICAL MASONRY COLUMN</u>

E SECTION1 1/2" = 1'-0"

PROVIDE CORNER

CONTINUOUS BOND

BEAM REINFORCING

ALL VOIDS IN

BE GROUTED

COLUMN SHALL

BARS TO MATCH

OF CONCRETE SLAB @ EXTERIOR WALL

2C <u>SECTION</u>

NOT USED

NOT USED

3 **SECTION**

3A SECTION



PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08 10.18.24 Issued For: GARAGE PERMIT REVISIONS

7.11.22 ADDENDUM 1

REGISTRATION

BEVERLIN NUMBER PE-2017012583 10.18.24

PROJECT TEAM FINKLE+WILLIAMS ARCHITECT ARCHITECTURE CIVIL **GBA ENGINEERS** LANDSCAPE LAND 3 STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS PLUMBING LATIMER SOMMERS **MECHANICAL**

FIRE PROTECTION LATIMER SOMMERS BRINKMANN CONSTRUCTORS CONTRACTOR

ELECTRICAL

LATIMER SOMMERS

AMPI

CMU DETAILS

					E	3E /	4M	S	CF	<u> </u>		D	ULES			
MARK	SIZE	E d	SHAPE OR SECTION	QUANTITY	SIZE	TOTAL LENGTH	LONGIT		NAL PLAC			ГЕЕ Вот.	EL REMARKS	NO.	STIRR Shape	UPS SPACING
						LLINO III		•	•	•			— € OF SUPPORT— 1/4 PT. OF SPAN— FACE OF SUPPORT			
D 4				4	#8	32-0	•	•			•		— 1/4 PT. OF ADJ. SPAN			18 @ 4"oc RIGHT END
B1	36" 3	36"		6 6 4	#8 #8 #8	12-0 27-0 34-0	•	Ц				•		#4		RIGHT END RMDR @ 12"oc
B2	36" 3	36"		6	#8 #8	12-0 27-0	•					•		#4		10 @ 4"oc, 12" @ 8"oc LEFT END RMDR @ 12"oc
В3	48" 3	36"		6 8	#8	32-0 36-0	•			-		•	PROJECT 4'-0" PAST GRID F.2 6" CLR FROM BOTT	(2) #4		@ 10"oc
B4	48"	36"		12 6 12	#8 #8 #8	46-0 40-0 24-0	•					•	PROJECT 4'-0" PAST 12" WALL	(2) #4		@ 10"oc
	36" 3	00"		8 4 4	#8 #8 #8	14-0 32-0 36-0	•			-	١	•	PROJECT 4'-0" PAST GRID F.2 6" CLR FROM BOTT			0.40
B5	30 .	36"		10	#8 #8	46-0 40-0	•	L				•	PROJECT 4'-0" PAST 12" WALL	#4		@ 10"oc
B6	36" 3	36"		10 6 4	#8 #8 #8	24-0 14-0 42-0	•					•		#4		@ 10"oc
B7	36"	32"		2 3	#8 #8	16-0 22-0	•					•		#4		@ 12"oc
B8	36" 3	32"		5 4 2	#8 #8 #8	34-0 42-0 24-0	•					•		#4		@ 12"oc
	0011	00"		3	#8	36-0 22-0	•				1	•				
B9	36" 3	32"		4	#8	40-0	•					•		#4		@ 12"oc
B10	36" 3	32"		4 4 8	#8 #8 #8	16-0 22-0 34-0	•					•		#4		12 @ 6"oc RIGHT END RMDR @ 12"oc
B11	36" 3	32"		4 4	#8 #8	42-0 16-0	•					_		#4		@ 12"oc
		-		2 4 2	#8 #8 #8	22-0 34-0 18-0	•					•		,,		(e) 12 00
B12	36" 3	32"		6	#8	12-0 16-0		L				•		#4		@ 8"oc
B13	36" 3	32"		6 8 2	#8 #8 #8	28-0 26-0 24-0	•				1	•		#4		@ 3"oc BTWN GRID 16.2 & GA
B14	48" 3	32"		6 12 12	#8 #8 #8	27-0 32-0 26-0	•	F			J T	•	ALT HOOK DIRECTION 6" CLR FROM BOTT	(2) #4		RMDR @ 8"oc @ 12"oc
				18	#8 #8	32-0 32-0	•	r=				•	ALT HOOK DIRECTION ALT HOOK DIRECTION	(2) #4		W 12 00
B15	42" 3	32"		8 14 8	#8 #8 #8	26-0 32-0 40-0		<u> </u>				•	6" CLR FROM BOTT ALT HOOK DIRECTION	#4		@ 8"oc
B16 B16R	36" 4	40"		4	#8 #5	16-0 25-0	•						1/2 EA FACE @ 10"oc FROM BOTT	#4		@4"oc AT CANTILEVER RMDR @12"oc
B17				6 8 2	#8 #8 #8	30-0 36-0 24-0	•					•	1/2 AT EA COLUMN	#4		@ 12"oc
B17R	36" 4	40"		4	#5 #8	42-0 42-0		-				•	1/2 EA FACE @ 10"oc FROM BOTT			
D40	36" 4	40"		4 4 4	#8 #8 #8	48-0 36-0 18-0	•					•				@ 10"00
B18	30 2	40		2 4	#5 #8 #8	18-6 20-0 24-0						•	1/2 EA FACE @ 10"oc FROM BOTT	#4		@ 12"oc
B19	36" 4	40"	Г	4 4	#8 #5	24-0 17-0	•	-			7		1/2 EA FACE @ 10"oc FROM BOTT	#4		@ 12"oc
D 19				4 4	#8 #8 #8	17-0 24-0 40-0	•	-				•		,,,		@ 12 00
B20	24"	40"		4 2	#5 #8	32-9 32-0		-				•	1/2 EA FACE @ 10"oc FROM BOTT	#4		@ 12"oc
D04				6 10 2	#8 #8 #8	34-0 36-0 20-0	•					•	ALT HOOK DIRECTION			
B21	36" 4	40"		4	#5 #8 #8	21-0 28-0 38-0		L				•	1/2 EA FACE @ 10"oc FROM BOTT 1/2 AT EA COLUMN	#4		@ 12"oc
B22	36" 4	40"		8 4 6	#5 #8	40-0 40-0	•	-				•	1/2 EA FACE @ 10"oc FROM BOTT	#4		(20) @ 6"oc EA END RMDR @ 12"oc
				6 10 4	#8 #8 #5	46-0 28-0 12-0	•				7	•	1/2 EA FACE @ 10"oc FROM BOTT			
B23	36" 4	40"		2 4	#8 #8	12-0 16-0						•	<u></u>	#4		@ 12"oc
B24	36" 4	40"		2 4 2	#8 #5 #8	28-0 19-0 20-0	•					•	1/2 EA FACE @ 10"oc FROM BOTT	#4		@ 12"oc
				4 4 4	#8 #8 #5	24-0 36-0 19-6	•	L				•	1/2 EA FACE @ 10"oc FROM BOTT			
B25	36" 4	40"		2 4	#8 #8	20-0 24-0						•	-	#4		@ 12"oc
B26	36" 4	40"		8 4 4	#8 #5 #8	38-0 40-0 40-0	•					•	1/2 AT EA COLUMN 1/2 EA FACE @ 10"oc FROM BOTT	#4		@ 12"oc
				4 6	#8 #8	46-0 28-0	•	-			1	•	4/0 54 54 05 0 100			
B27	36" 4	40"		2 4	#5 #8 #8	12-0 12-0 16-0						•	1/2 EA FACE @ 10"oc FROM BOTT	#4		@ 12"oc
B28	36" 4	40"	7	2 4	#8 #5 #8	30-0 24-0 18-0	•						1/2 EA FACE @ 10"oc FROM BOTT	#4		(18) @ 4"oc RIGHT END
				4 4 4	#8	28-0 18-0	•	L			1	•				RMDR @ 12"oc
B29	36" 4	40"		2 4	#5 #8 #8	12-0 16-0						•	1/2 EA FACE @ 10"oc FROM BOTT	#4		(20) @ 4"oc LEFT END RMDR @12"oc
B30	48" 4	40"		4	#8 #8	36-0 36-0	•	- 			1		4/0 FA FAOE O (0"			12"oc AT CANTILEVER (10) @ 6"oc EA.
				10 16	#5 #8 #8	11-0 26-0 36-0						•	1/2 EA FACE @ 10"oc FROM BOTT 6" CLR FROM BOTT ALT HOOK DIRECTION	(2) #4		END BTWN COLS RMDR @ 12"oc
B31	48" 4	40"		8 4	#8 #5	36-0 35-0	•				1		ALT HOOK DIRECTION 1/2 EA FACE @ 10"oc FROM BOTT	(2) #4		(10) @ 6"oc EA END RMDR @12"oc
				10 16	#8 #8	32-0 36-0		L				•	6" CLR FROM BOTT ALT HOOK DIRECTION			

	BEAM SCHEDULES													
	SIZ	ZE	SHAPE		LONGITUDINAL STEEL			(STIRRUPS					
MARK	b	d	OR SECTION	QUANTITY	SIZE	TOTAL LENGTH	MK	TOP	PLACED	ВОТ.	REMARKS	NO.	SHAPE	SPACING
								• •			— Q OF SUPPORT — 1/4 PT. OF SPAN — FACE OF SUPPORT — 1/4 PT. OF ADJ. SPAN			
B32	24"	36"		4 2 4	#7 #7 #7	26-0 20-0 22-0		•		1 •	ALT HOOK DIRECTION	#4		@ 12"oc
B33	24"	36"		4 4 4	#7 #7 #7	22-0 20-0 22-0		•			ALT HOOK DIRECTION ALT HOOK DIRECTION	#4		@ 12"oc
B34	24"	36"		4 4 4	#7 #8 #8	29-0 26-0 27-0		•		•	ALT HOOK DIRECTION ALT HOOK DIRECTION	#4		@ 12"oc

			CONCRETE S			
TYPE	THICKNESS	VERTICAL REINFORCEMENT	HORIZONTAL REINFORCEMENT	COLUMN @ END OF WALL	LATERAL LOAD ALONG WALL FOR PILE DESIGN (kif)	UPLIFT AT EACH END OF WALL FOR PILE DESIGN (kips)
CSW1	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW2	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW3	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW4	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW5	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW6	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW7	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW8	12"	#5 @ 12"oc EA FACE	#6 @ 12"oc EA FACE	12"x24"		
CSW9	8"	#5 @ 12"oc EA FACE	#6 @ 12"oc EA FACE	12"x24"		
CSW10	12"	#5 @ 12"oc EA FACE	#6 @ 12"oc EA FACE	12"x24"		
CSW11	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW12	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW13	12"	#5 @ 12"oc EA FACE	#6 @ 12"oc EA FACE	12"x24"		
CSW14	12"	#5 @ 12"oc EA FACE	#6 @ 12"oc EA FACE	12"x24"		
CSW15	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW16	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW17	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW18	12"	#5 @ 12"oc EA FACE	#6 @ 12"oc EA FACE	12"x24"		
CSW19	12"	#5 @ 12"oc EA FACE	#6 @ 12"oc EA FACE	12"x24"		
CSW20	8"	#5 @ 12"oc CTR IN WALL	#5 @ 12"oc CTR IN WALL	8"x32"		
CSW21	12"	#5 @ 12"oc EA FACE	#6 @ 12"oc EA FACE	12"x24"		
CSW22	12"	#5 @ 12"oc EA FACE	#6 @ 12"oc EA FACE	12"x24"		
CSW23	12"	#5 @ 12"oc EA FACE	#6 @ 12"oc EA FACE	12"x24"		

1) PROVIDE (2)#7 CONT TOP AND BOTTOM OF EACH WALL WITHIN 4" OF THE TOP AND BOTTOM. SPLICE BOTTOM BARS 5'-0" AND TOP BARS 6'-0" WITH 90 DEG HOOKS AT DISTONT ENDS 2) VERTICAL FOUNDATION DOWELS TO MATCH VERTICAL WALL REINFORCEMENT SIZE AND SPACING WITH 48 BAR Ø LAP INTO WALL AND 90 DEG HOOK INTO BOTTOM OF FOOTING BELOW. 3) TERMINATE TOP OF VERTICAL BARS WITH 90 DEG HOOK INTO TOP OF SLAB WITH 3" OF CLEAR COVER.

COLUMN SIZE

16X32

24X24

24Ø

√ (3) #3 TIES

└─ (3) #3 TIES

24"x24" COLUMN

14"x34" COLUMN

CONCRETE COLUMN SCHEDULE

REINFORCEMENT

(8) #7 VERTICAL

(2) #3 TIES @ 8"oc

(8) #7 VERTICAL

(2) #3 TIES @ 12"oc

(10) #8 VERTICAL

─ VERT REINF

─ VERT REINF

√ (2) #3 TIES 16"x32" COLUMN

√ (2) #3 TIES

8"x32" & 12"x24" COLUMN

24"Ø COLUMN

(3) #3 TIES @ 14"oc

3/4" CHAMFER

- 3/4" CHAMFER

(TYP)

1 1/2" CLR CVR (TYP)

CVR (TYP)

VERT REINF

7 CONCRETE COLUMN DETAILS

COLUMN SIZE

8X32

12X24

14X34

	 PROVIDE (4) SETS OF TIES AT 3"oc TOP & BOTTOM OF EACH COLUM ALL COLUMNS TO CENTER ON GRIDLINE AND PIER/FOUNDATION U PROVIDE VERTICAL FOUNDATION DOWELS MATCH SIZE AND QUAN
EINFORCEMENT	VERTICAL REINFORCEMENT WITH 48 BAR Ø LAP INTO COLUMN ANI
0) #8 VERTICAL #3 TIES @ 16"oc	DEG HOOK INTO BOTTOM OF FOOTING. 4) PROVIDE VERTICAL SLAB DOWELS AT TOP OF COLUMN WITH 48 BA LAP INTO COLUMN AND 90 DEG HOOK INTO TOP OF SLAB ABOVE.

(12) #8 VERTICAL

(3) #3 TIES @ 16"oc

(10) #8 VERTICAL

#3 TIES @ 12"oc

CVR (TYP)

VERT REINF

3/4" CHAMFER

(TYP)

CVR (TYP)

VERT REINF

CONCRETE COLUMN NOTES:

	TYPICAL BOTTOM	2" L	<u> </u>	+		\perp	: !
	TYPICAL TOP	1	Ŧ	_	7	$\stackrel{\square}{\rightarrow}$	+
	-			PER NOTE 2'-6" MIN			
	AT CANTILEVER						
	2" EDGE OF CANT		_	30% OF SPAN OR CANT DIST.			
	OI CANI	'	ı	WHICHEVER IS GREATER			
OTTOM OF EACH COLUMN ND PIER/FOUNDATION U.N.O.							
MATCH SIZE AND OLIANITY OF							

102

61

#10

SLAB NOTES

- 1. SEE GENERAL NOTES (STRUCTURAL) ON SHEET S001. 2. PODIUM SLAB IS 15" THICK REINFORCED WITH A CONTINUOUS (60" LAP AT COLUMN CENTERLINE OF COLUMN STRIPS AND 24" LAP AT COLÚMN CENTERLINE OF MID-STRIPS) BOTTOM MAT OF #6 @ 12" EACH WAY. SEE PLAN FOR BOTTOM MAT EXTENDING EAST/WEST THAT SHALL BE SUPPORTED ON 1" SLAB BOLSTERS AT 4'-0"o.c.
- 3. TOP REINFORCING BARS PLACING SEQUENCE: 14A 5 19'-9"
- TOTAL LENGTH OF BAR IN FEET AND INCHES SIZE OF BAR AND LOCATION IN SLAB AS NOTED BELOW TOTAL NUMBER OF EXTRA BARS IN STRIP DEFINED ON PLAN
- "A" #6 EXTRA BOTTOM BARS WITH 1" CLEAR COVER BOTTOM. (PLACE WITH 1" CLEAR COVER BOTTOM MAT BARS.)
- "B" #6 EXTRA BOTTOM BARS WITH 1 3/4" CLEAR COVER BOTTOM. (PLACE WITH 1 3/4" CLEAR COVER BOTTOM MAT BARS.) PLACE ON TOP OF PERPENDICULAR (1" CLEAR COVER) BOTTOM MAT AND "A" BARS.
- OCCUR AND 1" CLEAR COVER WHERE ONE LAYER OF BARS OCCUR ON IHC @4'-0" o.c. AND #5 SUPPORT BARS @4'-0"o.c. "D" #7 TOP BARS WITH 1" CLEAR COVER TOP. PLACE ON TOP OF "C"

"C" #7 TOP BARS WITH 1 7/8" CLEAR COVER WHERE TWO LAYERS OF BARS

- BARS WHERE THEY OCCUR OR OTHERWISE PLACE ON IHC AT 4'-0"o.c. AND #5 SUPPORT BARS AT 4'-0" o.c. "E" #6 TOP BARS WITH 1" CLEAR COVER WHERE ONE LAYER OF BARS
- OCCUR ON IHC AT 4'-0" o.c. AND #5 SUPPORT BARS AT 4'-0"o.c. "F" #5 TOP BARS WITH 1 7/8" CLEAR COVER WHERE TWO LAYERS OF BARS OCCUR AND 1" CLEAR COVER WHERE ONE LAYER OF BARS OCCUR ON
- IHC AT 4'-0" o.c. AND #5 SUPPORT BARS AT 4'-0"o.c. "G" #5 TOP BARS WITH 1" CLEAR COVER TOP, PLACE ON TOP OF "F" BARS
- WHERE THEY OCCUR, OTHERWISE PLACE ON IHC AT 4'-0" o.c. AND #5 SUPPORT BARS AT 4'-0"o.c. 4. REINFORCING SHALL BE SPLAYED AROUND OPENINGS LESS THAN 18" WIDE.
- REINFORCING SHALL BE CUT AT OPENINGS GREATER THAN 18" WIDE WITH EQUAL CONTINUOUS BARS ADDED ONE-HALF EACH SIDE OF OPENING. PROVIDE REINFORCING PER GENERAL NOTE 7F AT ALL OPENINGS LARGER
- 5. STRIP LINES ARE LOCATED AT 1/4 POINTS BETWEEN COLUMN CENTERLINES UNLESS NOTED ON PLAN OTHERWISE. 6. SEE DETAIL 2/S3.10 FOR PLACING PATTERN FOR TOP REINFORCING BARS
- OVER INTERIOR COLUMN AS NOTED. 7. TOP BARS SHOWN STAGGERED ON PLAN SHALL BE STAGGERED WHEN PLACED; THE END OF EVERY OTHER BAR TO BE PLACED AT RELATIVE STRIP LINE, UNLESS NOTED ON PLAN.
- 8. BOTTOM BARS ARE SHOWN THUS -----TOP BARS ARE SHOWN THUS — TOP BARS SHOWN ON PLAN THUS ______ SHALL HAVE A STANDARD ACI 90 DEG. HOOK.
- 9. UNLESS SHOWN ON "S" SERIES DRAWINGS, NO HOLES LARGER THAN TEN INCH DIAMETER SHALL BE PLACED THROUGH SLAB. NOT MORE THAN ONE, SIX TO EIGHT INCH DIAMETER HOLES, OR TWO FOUR INCH DIAMETER HOLES, OR THREE TWO INCH DIAMETER OR SMALLER HOLES SHALL BE PLACE WITHIN 20" OF THE FACE OF THE COLUMNS. 10. CAMBER ALL SPANS BETWEEN 16'-0" AND 24'-0" (CENTERLINE TO CENTERLINE OF SUPPORTS) FOR L/600 MINIMUM AT MIDSPAN (WITH L = SPAN IN INCHES)

(I.E., 3/8 AT MIDSPAN FOR 18'-0" SPAN. CAMBER ALL SPANS LONGER THAN 24'-0" FOR L/480 (I.E., 3/4" AT MIDSPAN FOR 30'-0" SPAN.)

11. AT TERMINATION OF COLUMN STRIP AT COLUMN, WALL, BEAM, PROVIDE 90° STANDARD ACI HOOK EACH END AT (4) BOTTOM BARS NEAREST TO COLUMN CENTERLINE PER 6/S3.10

BEAM SCHEDULE PLACING NOTES

- 1. See General Notes (Structural) on sheet S0.01. 2. Orientation of beams in schedule are as seen from the bottom or right of
- the plan sheet. 3. Center group of top bars indicated thus — in "placed" schedule on centerline of support. Stagger bars 5% of longer span. Bar length =
- 55% of longer span. 4. Top bars scheduled thus + | || extend 2" from face of exterior
- soffet to 5% of span past 1/4 point of span. 5. All lapped top bars shall have a minimum of lap of 2'-6" or 48 bar diameters. Perimeter beams shall have two bars lapped a minimum of
- 66 bar diamters. 6. Bottom bars indicated thus | in "placed schedule to have
- scheduled bars extend 12" past centerline of support each end and a minimum 2 bars with a 66 bar diameter lap. 7. Bottom bars scheduled thus $\| + + \|$ extend to within 1/8 point of
- 8. Start stirrups 2" from face of support each end unless noted.
- 9. All bars shown thus ______ to have standard ACI hook. Extend to within 2" of exterior face.
- 10. No holes, sleeves, or conduit larger than 1" diameter round shall be put through beams without written authorization from the engineer. All conduit shall be PVC (non metalic).
- 11. Splice length at bars of different sizes shall be based on the larger of the two bar diameters.

BEAM PLACEMENT SCHEDULE

NOTE: ALL SIMILAR CONDITIONS TO BE PLACED PER THE PLACING SCHEDULE.

	1	_		1	1		4	_	© SPAN
								•	1/4 POINT SPA
_			_				1	ı	1/4 POINT ADJ
BM SCHEDULE NOTE #						—— 55% LONGER SPAN	Ш		<u> </u>
#3 TOP BEAM				5% SPAN		0070 20110211 017111			
#4 TOP BEAM	2"	*		5% OF SPAN					
#5 TOP BEAM		\dagger		PER NOTE -			+		
#6 BOTTOM BEAM	PER NOTE	**		2'-6" MIN —			-	PER	
#7 BOTTOM BEAM		+	1/8 SPAN			1/8 SPAN	\downarrow	۷	
TYPICAL BOTTOM	2"	*				<u></u> 1/8 SPAN		 	
TYPICAL TOP				DED MOTE					
				PER NOTE 2'-6" MIN					
AT CANTILEVER									
2" EDGE			30% OF SPAN OR C	ANT DIST.			İ	i	
OF CANT	11	1	WHICHEVER IS GR	EATER			ı		I

TION DOWLES MATCH SIZE AND QUANTITION	
WITH 48 BAR Ø LAP INTO COLUMN AND 90	
F FOOTING.	
WELS AT TOP OF COLUMN WITH 48 BAR Ø	

			F	REBAR D	EVELO	PMENT L	ENGTH	AND LAF	P SPLICE	SCHE	DULE			
CONCRETE STRENGTH = 5000 psi									00 psi					
CASE	LENG	OPMENT TH OR S A LAP	CLASS	S B LAP	CASE	CASE DEVELOPMENT LENGTH OR CLASS A LAP					LENG ²	OPMENT TH OR S A LAP	CLASS	B LAP
BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	24	24	24	24	#3	24	24	24	24	#3	24	24	26	24
#4	24	24	29	24	#4	25	24	33	25	#4	27	24	35	27
#5	28	24	36	28	#5	31	24	41	31	#5	33	26	43	33
#6	34	26	43	34	#6	37	29	49	37	#6	40	31	52	40
#7	49	38	63	49	#7	54	42	71	54	#7	58	45	75	58

#10

#11

1. UNLESS SPECIFICALLY INDICATED OTHERWISE, USE THE MINIMUM LENGTH FOR A CLASS B LAP SPLICE OR THE MINIMUM DEVELOPMENT LENGTH INDICATED IN THE TABLES ABOVE MULTIPLIED BY THE

92

70

APPLICABLE FACTOR(S) LISTED BELOW. 2. WHERE THE CLEAR SPACING BETWEEN BARS LAP SPLICED OR EMBEDDED AT ANY SECTION IS LESS THAN 2 BAR DIAMETERS, OR WHERE THE BAR COVER IS LESS THAN OR EQUAL TO THE BAR DIAMETER. INCREASE THE INDICATED BAR SPLICE OR DEVELOPMENT LENGTH BY 50%.

#10

3. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS. 4. MECHANICAL COUPLERS MAY BE SUBSTITUTED FOR TENSION LAP SPLICED BARS PROVIDED THAT THEY MEET THE REQUIREMENTS OF ACI 318-11, 12.14. 5. AT LOCATIONS WHERE REINFORCING WITHIN A STRUCTURAL ELEMENT WILL BE SPLICED, ALTERNATING SPLICES SHALL BE STAGGERED A MINIMUM OF THE CLASS B SPLICE LENGTH UNLESS INDICATED OTHERWISE.

PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY

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Description 7.11.22 ADDENDUM 1

____ ____ ____ ____

REGISTRATION



PROJECT TEAM FINKLE+WILLIAMS ARCHITECT ARCHITECTURE CIVIL **GBA ENGINEERS** LAND 3 LANDSCAPE STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS PLUMBING LATIMER SOMMERS MECHANICAL LATIMER SOMMERS ELECTRICAL

FIRE PROTECTION LATIMER SOMMERS CONTRACTOR CONSTRUCTORS

OTHER

BARS

66

75

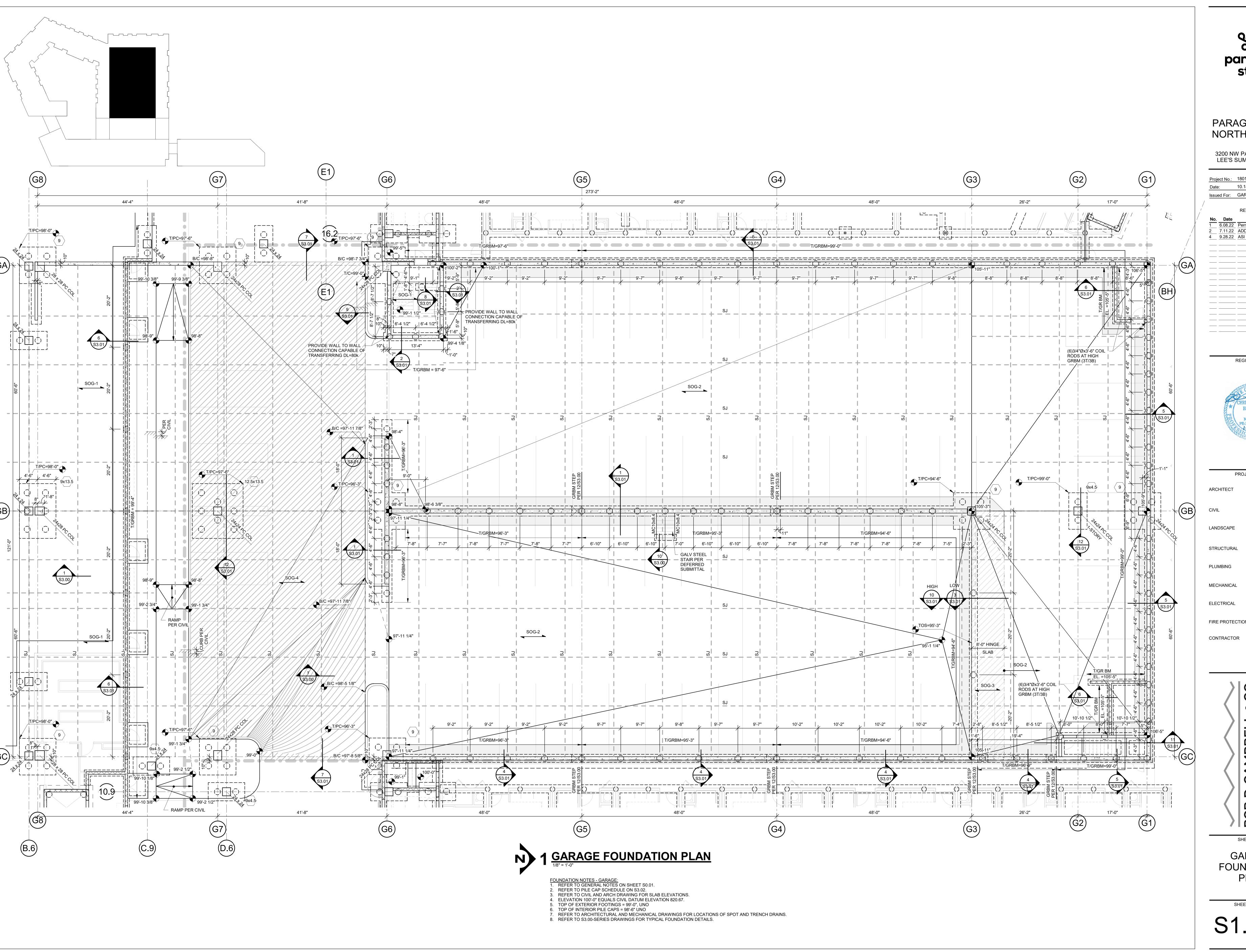
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109

121

SHEET TITLE

CONCRETE SCHEDULE





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LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

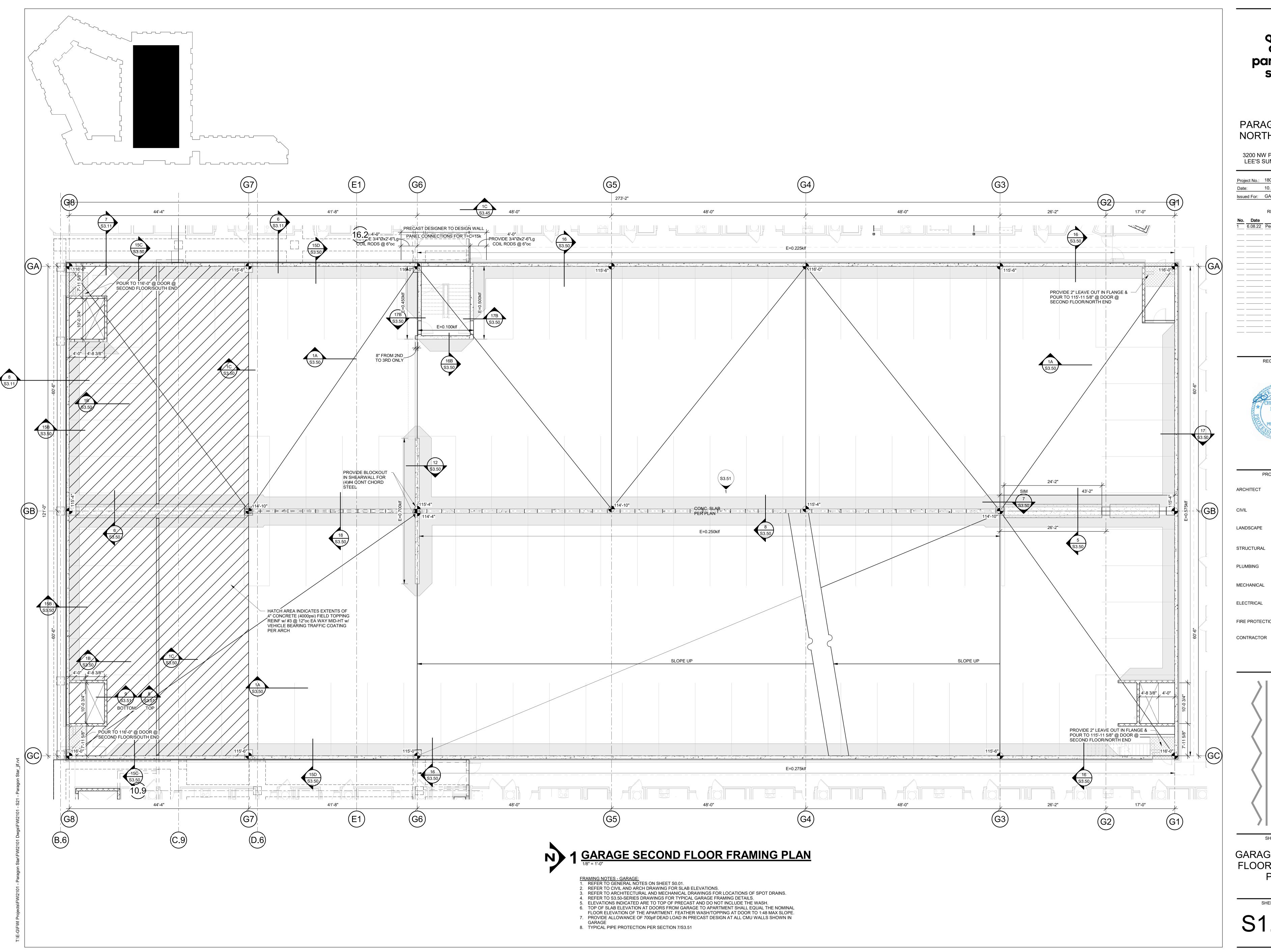
FIRE PROTECTION LATIMER SOMMERS CONTRACTOR

BOB Structu 4338 B Kansas SHEET TITLE

GARAGE FOUNDATION PLAN

SHEET NUMBER

S1.71G





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ARCHITECTURE

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MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

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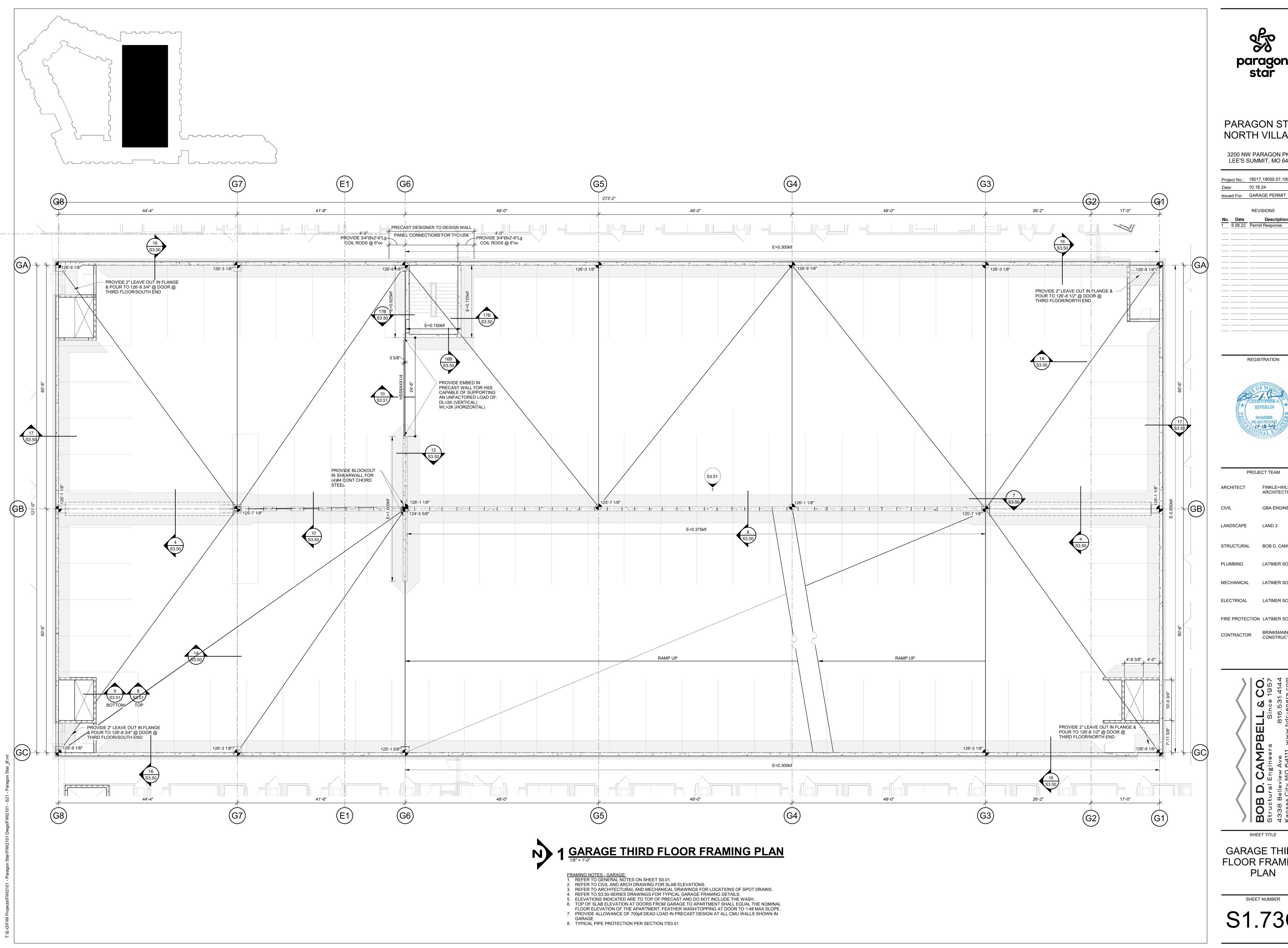
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SHEET TITLE

BOB Structu 4338 B Kansas

GARAGE SECOND FLOOR FRAMING PLAN

SHEET NUMBER
S1.72G





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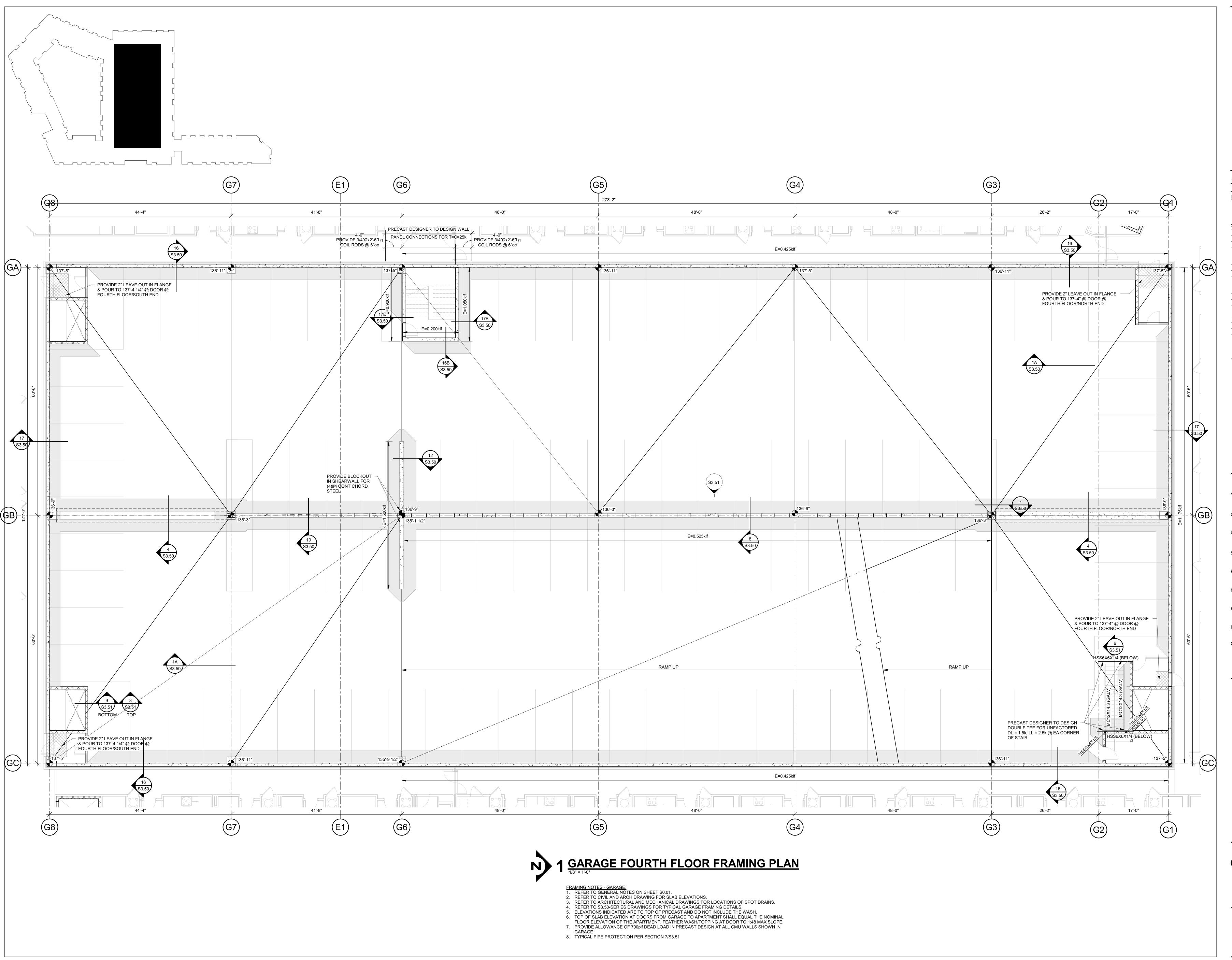
MECHANICAL LATIMER SOMMERS LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS CONTRACTOR BRINKMANN CONSTRUCTORS

SHEET TITLE

GARAGE THIRD FLOOR FRAMING PLAN

SHEET NUMBER S1.73G





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LANDSCAPE LAND 3 STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS MECHANICAL LATIMER SOMMERS

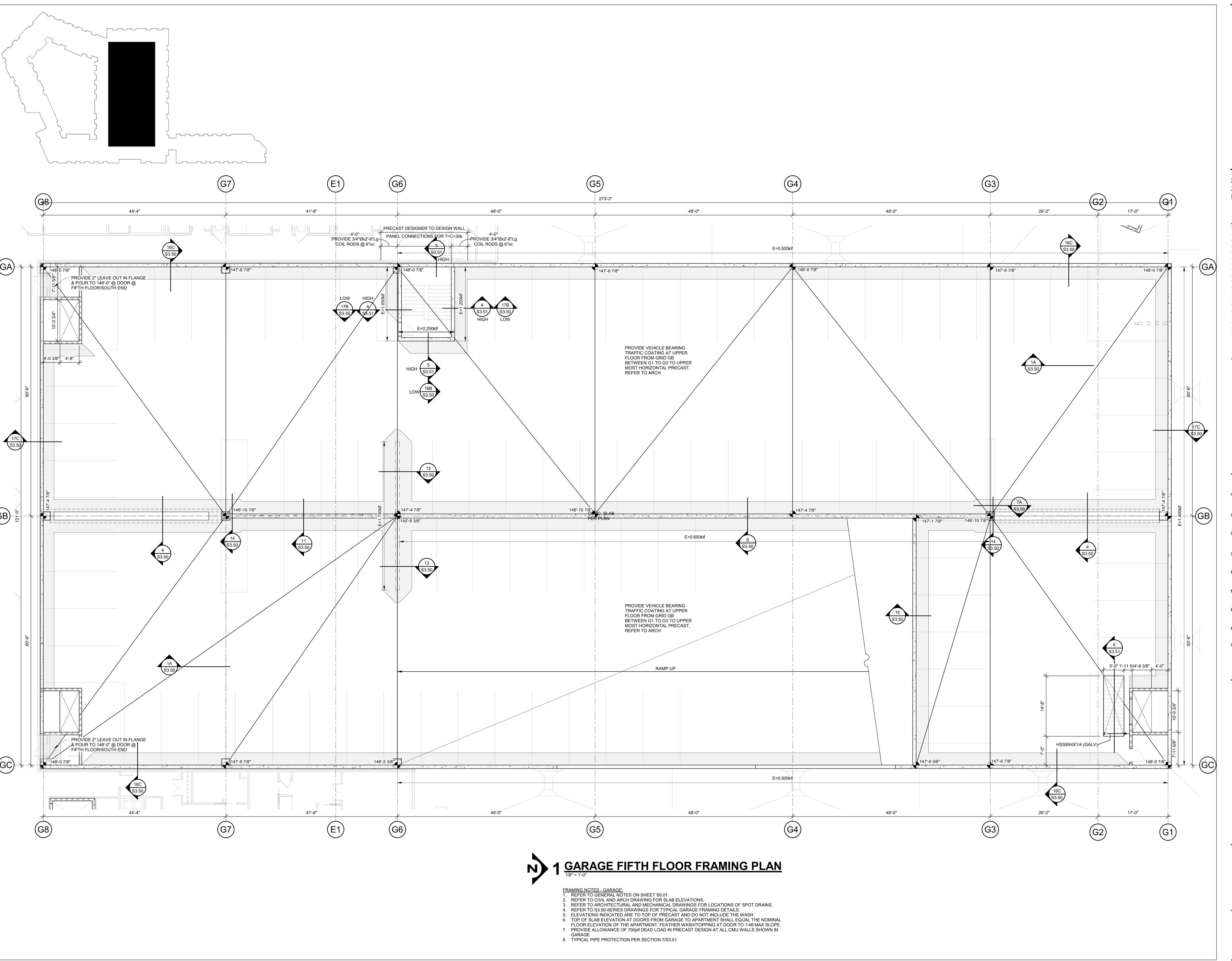
ELECTRICAL LATIMER SOMMERS FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR BRINKMANN CONSTRUCTORS

SHEET TITLE

GARAGE FOURTH FLOOR FRAMING PLAN

SHEET NUMBER S1.74G





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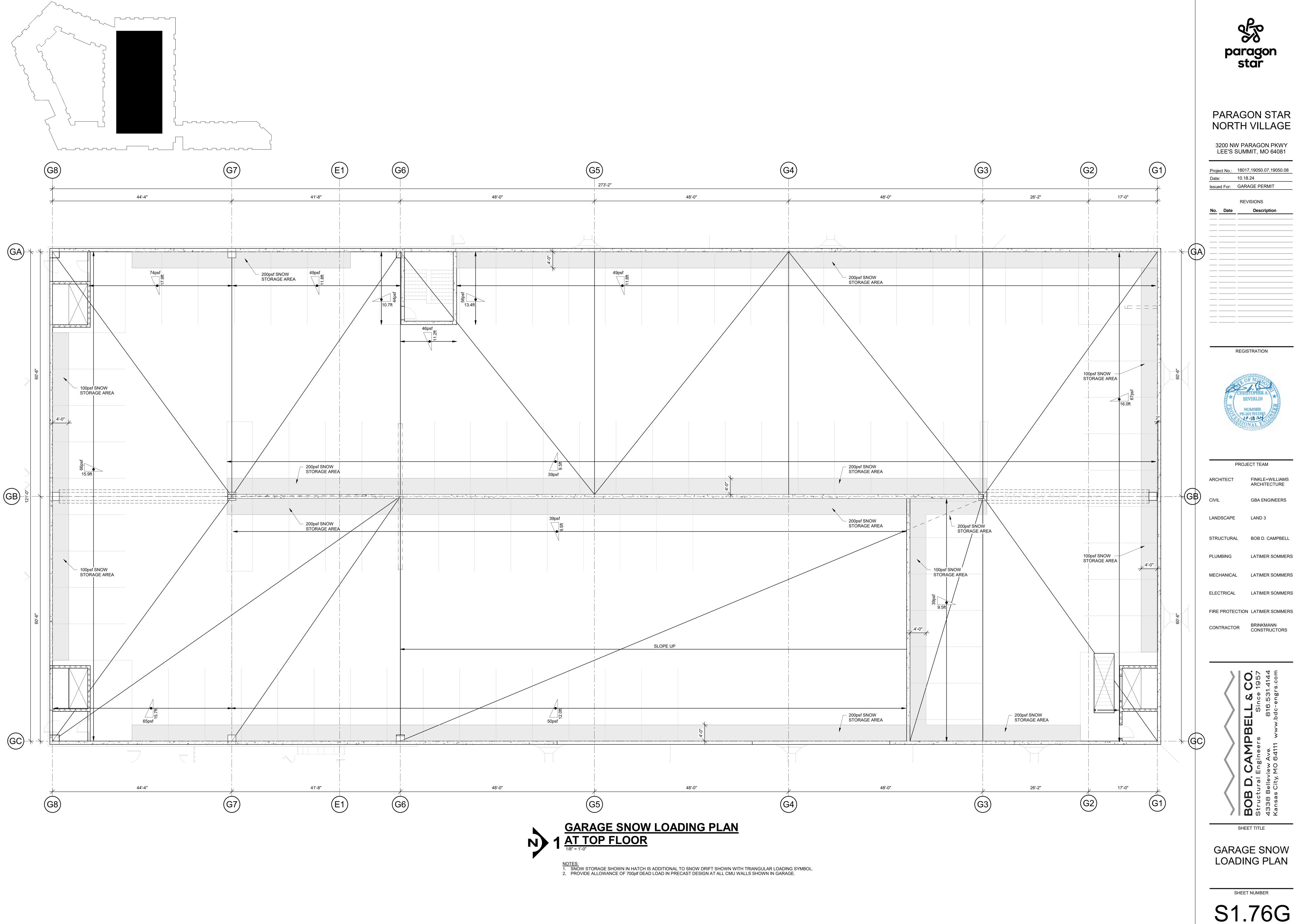
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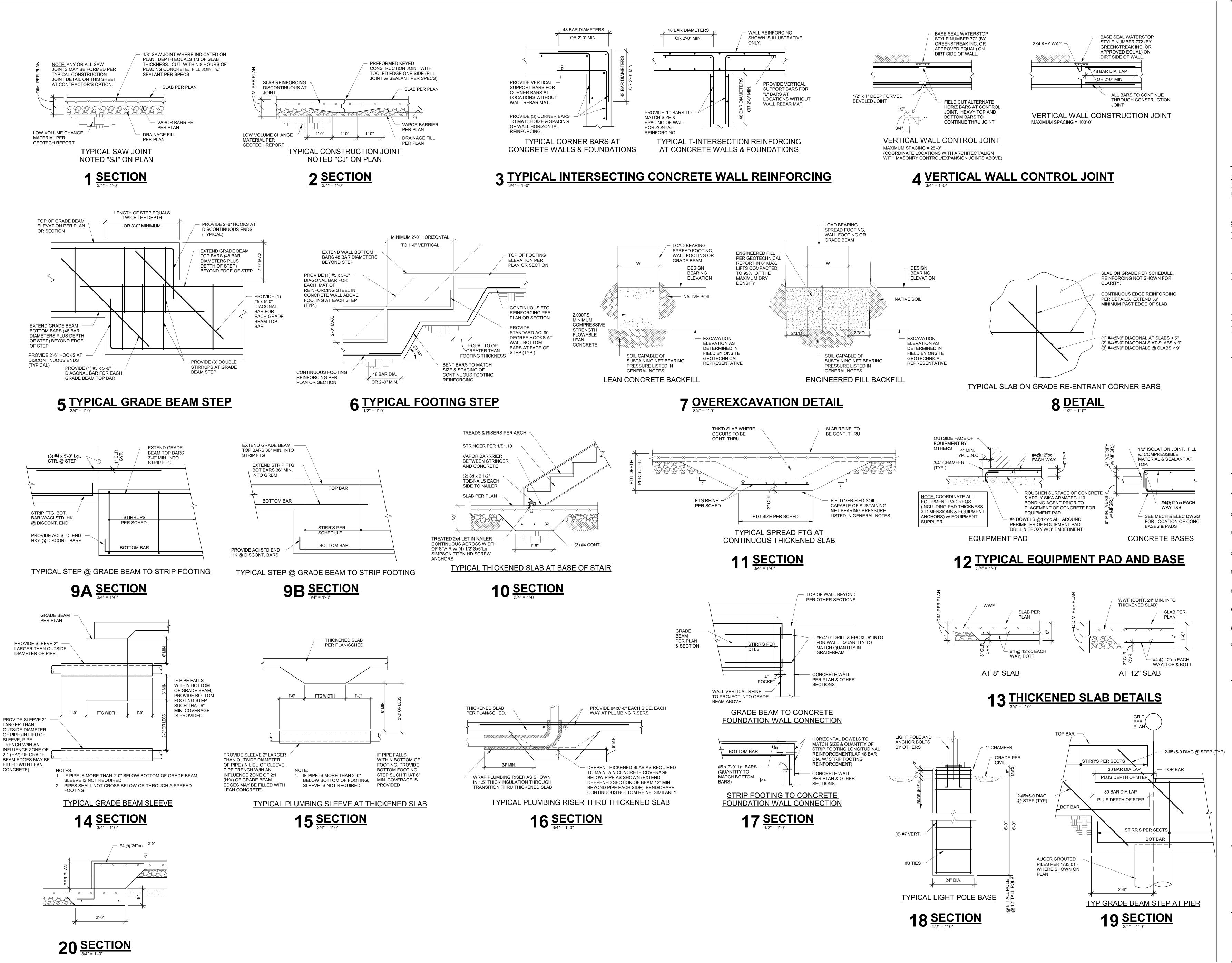
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SHEET TITLE

GARAGE FIFTH FLOOR FRAMING PLAN

S1.75G





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GBA ENGINEERS LAND 3 LANDSCAPE BOB D. CAMPBELL STRUCTURAL

LATIMER SOMMERS

LATIMER SOMMERS MECHANICAL LATIMER SOMMERS ELECTRICAL

FIRE PROTECTION LATIMER SOMMERS

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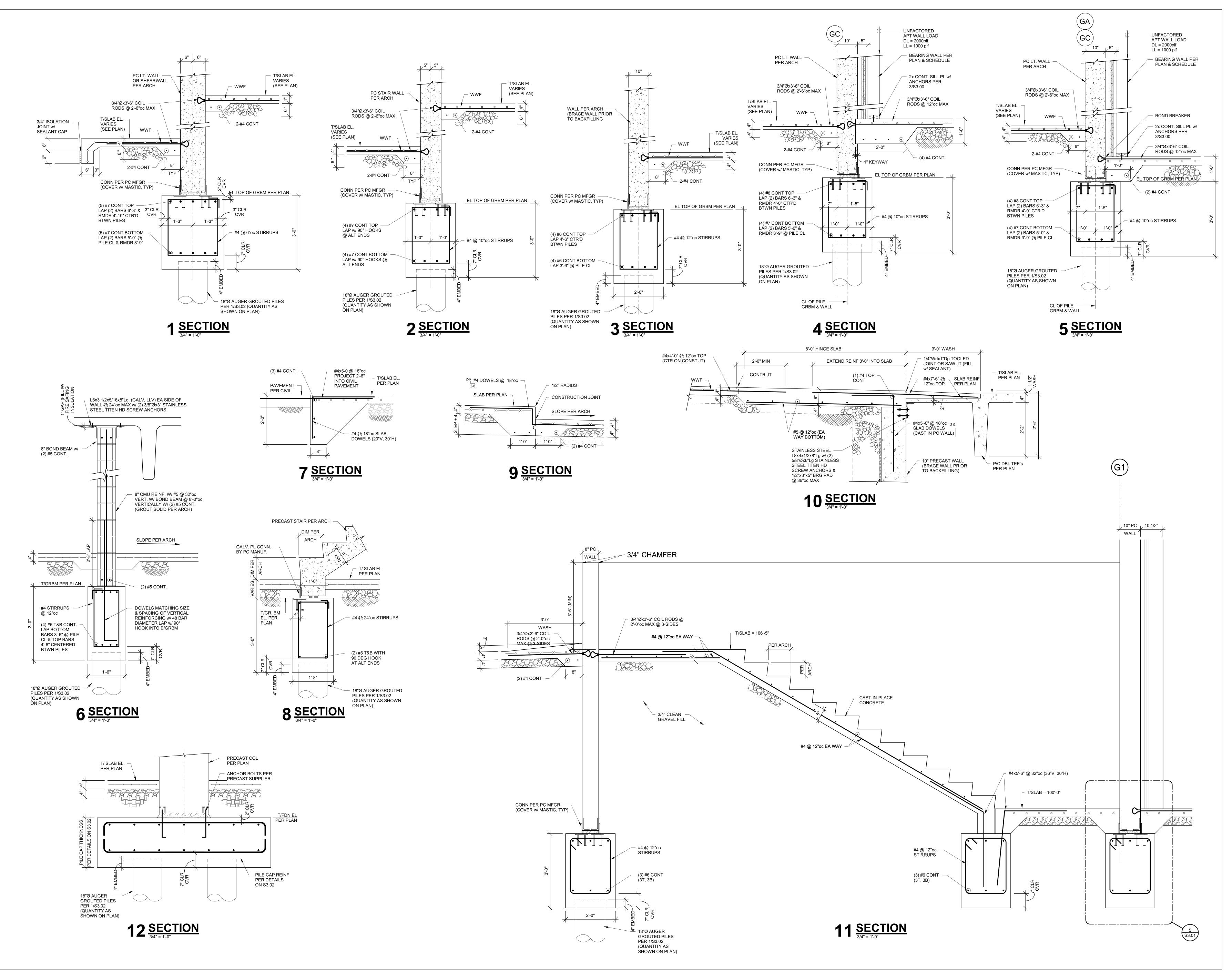
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BOB Structu 4338 B Kansas

SHEET TITLE **TYPICAL**

FOUNDATION **DETAILS**

S3.00





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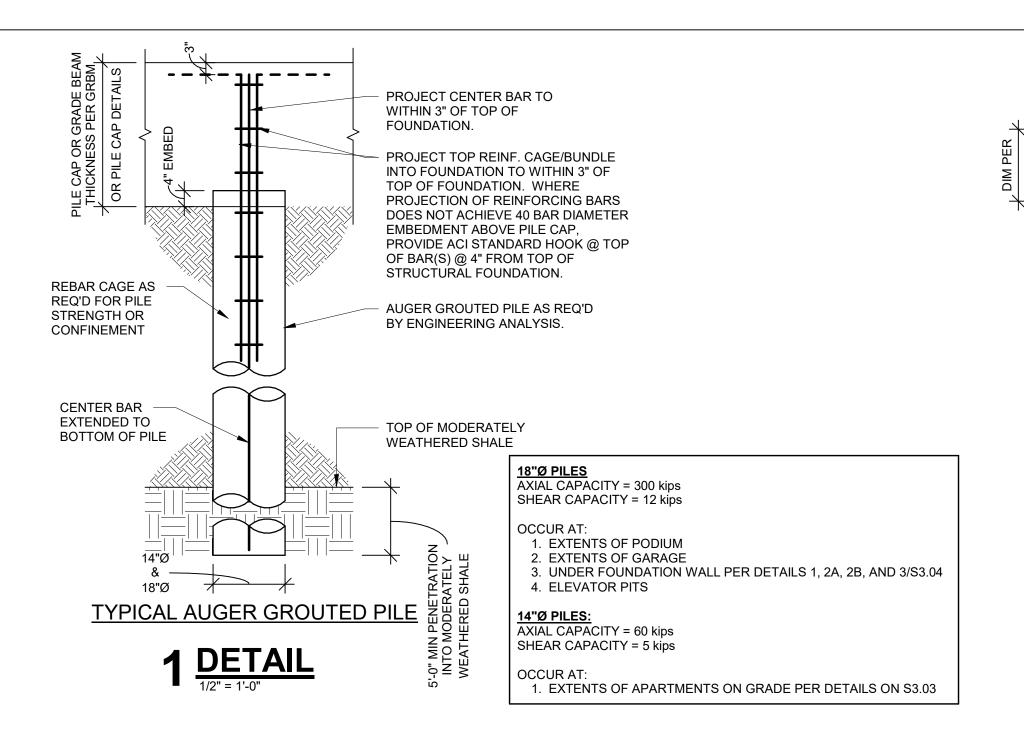
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GARAGE FOUNDATION DETAILS

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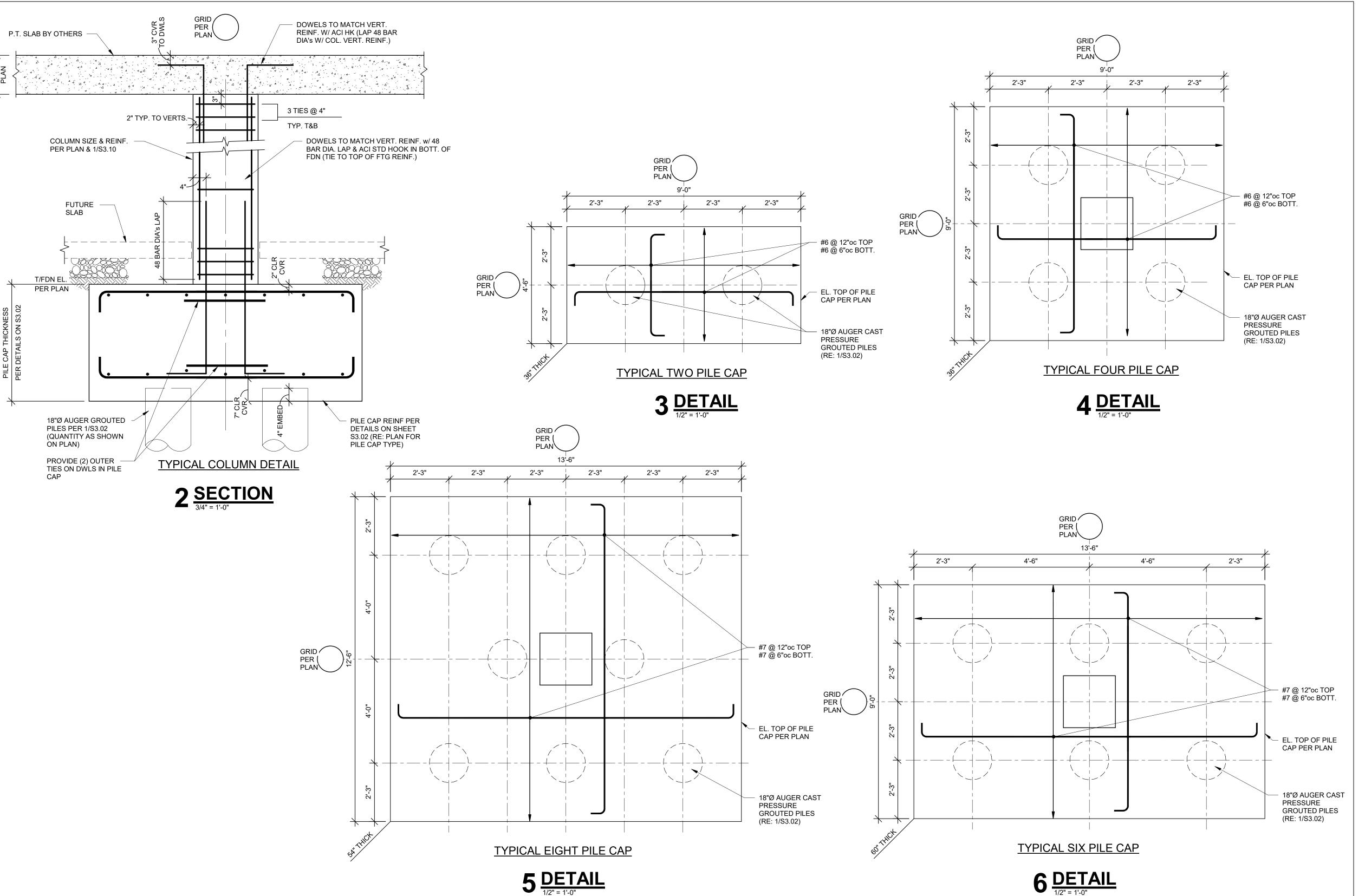
S1.01



	PILE	CAP SO	CHE	DULE		
PILE CA 300 @ 18"Ø	APACTY (KIPS): PILE / 600 @ 14"Ø	CONCRETE 3500	(PSI): REBAR (KSI): 60			
TYPE	FOOTING S THICKNES	SIZE (FT.) SS (IN.)	QTY/SIZE OF BARS EACH WAY			
3.5	3'-6" x 3'-6" x 30"	w/ 14"Ø PILE	#5 @ 6"oc BOTTOM / #5 @ 12"oc TOP			
9x4.5	9'-0" x 4'-6'	" x 36"	#6 @ 6"oc BOTTOM / #6 @ 12"oc TOP			
9	9'-0" x 9'-0'	" x 36"	#7 @ 6"od	BOTTOM / #7 @ 12"oc TOP		
9x13.5	9'-0" x 13'-6	6" x 60"	#7 @ 6"oc BOTTOM / #7 @ 12"oc TOP			
12.5x13.5	12'-6" x 13'-	6" x 54"	#7 @ 6"oc BOTTOM / #7 @ 12"oc TOP			

1.) EXTERIOR PILE CAPS SHALL BE POURED MONOLITHIC WITH GRADE BEAMS AND EXTERIOR.
2.) PROVIDE #4 @ 12"o.c. EACH WAY IN TOP OF FTG. AT ALL MOMENT FRAMES AND AT

- 3.) CENTER PILE CAPS ON COLUMNS AND/OR WALL CENTER LINES PER PLAN, UNLESS OTHERWISE NOTED.
- 4.) PROVIDE ACI STANDARD HOOK AT EACH END OF BARS.





PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081 Project No.: 18017,19050.07,19050.08

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 No.
 Date
 Description

 2
 7.11.22
 ADDENDUM 1

 3
 7.20.22
 ADDENDUM 2

 4
 9.28.22
 ASI 1

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PROJECT TEAM

ARCHITECT FINKLE+WILLIAMS ARCHITECTURE

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STRUCTURAL BOB D. CAMPBELL

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CONTRACTOR BRINKMANN CONSTRUCTORS

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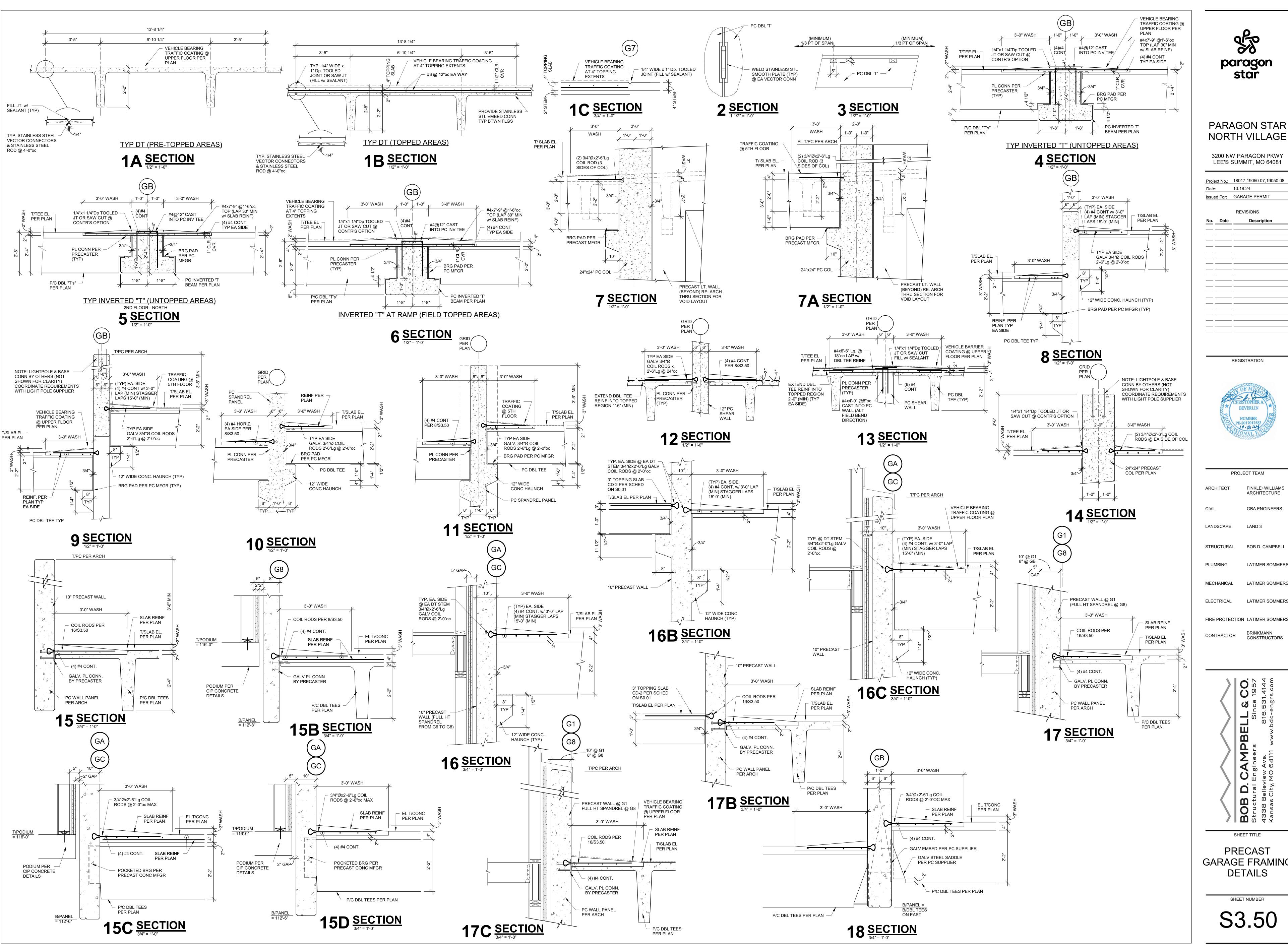
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ELECTRICAL

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SHEET TITLE

PILE & PODIUM FOUNDATION DETAILS





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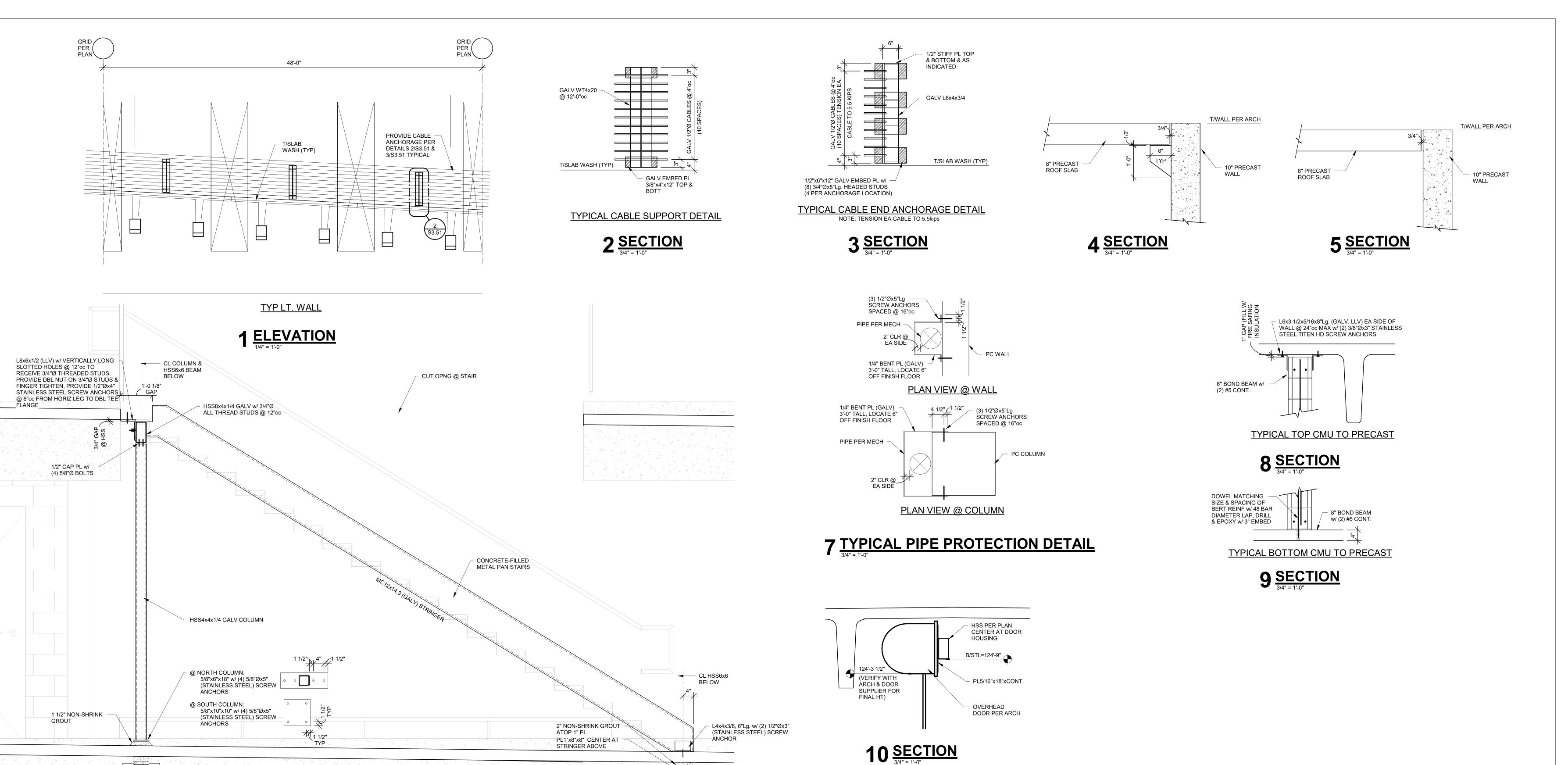
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BOB Structu 4338 Be Kansas (

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SHEET TITLE **PRECAST** GARAGE FRAMING **DETAILS**

> SHEET NUMBER S3.50



HSS6x6x1/4 (GALV) w/ 1/2"x14"x8" -

END PLATES, ATTACH TO DBL TEE

STEM w/ (4) 5/8"Øx5" (STAINLESS STEEL) SCREW ANCHORS

HSS6x6x1/4 (GALV) w/ 1/2"x14"x8" END PLATES

 $6\frac{\text{SECTION}}{3/4" = 1'-0"}$

ATTACH TO DBL TEE STEM w/ (4) 5/8"Øx5"

(STAINLESS STEEL) SCREW ANCHORS

paragon star

PARAGON STAR NORTH VILLAGE

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9.28.22 ASI 1

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SHEET TITLE

PRECAST GARAGE FRAMING DETAILS

0	SMOKE DETECTOR
Øs	SMOKE DETECTOR WITH SOUNDER BASE
	SMOKE DETECTOR WITH ISOLATOR BASE
① 130	HEAT DETECTOR
कि	DUCT DETECTOR
	ADDRESSABLE MANUAL PULL STATION
_ 오	DOOR HOLDER
₽ \ \$	FLOW DETECTOR/SWITCH
¸Q¸	TAMPER DETECTOR
	TEST STATION
[R]	MR101/C SHUTDOWN RELAY, SPDT W/RED
	A/V (WALL MOUNTED) 24 VDC
×	STROBE
<u>P</u>	BELL ANNUNCIATOR
	HORN/SPEAKER
[FCP]	FIRE ALARM CONTROL PANEL
	FIREMAN'S PHONE
[ARA]	AREA RESCUE CALL STATION
[ARA] _M	AREA RESCUE MASTER STATION
[ZAM] _S	SIGNAL ZAM
[ZAM] _C	CONTROL ZAM
[ZAM] _{DET}	DETECTOR ZAM
[IAM]	MONITOR MODULE
[IAM] _R	RELAY IAM
PC_	GRAPHIC COMMAND CENTER
[FAA]	REMOTE FIRE ALARM AUDIO
[FSA]	REMOTE ANNUNCIATOR WITH AUDIO
[ANN]	ANNUNCIATOR
-[FS]-	FIRE SMOKE DAMPER
[NAC]	NAC POWER EXTENDER

SYMBOL	DESCRIPTION	REMARK
▼ D201/A	TELECOMMUNICATIONS OUTLET WITH ROOM AND TYPE IDENTIFIER	1
▼ W	TELECOMMUNICATIONS OUTLET WALL PHONE PLATE	2
V _{AV}	AUDIO/VISUAL OUTLET	3
Tv	TELEVISION OUTLET	1
	EMT CONDUIT BY E/C (1 1/4" UNLESS NOTED OTHERWISE)	4
	EMT SLEEVE BY E/C (2" UNLESS NOTED OTHERWISE)	4
AFF	ABOVE FINISHED FLOOR	
T/C	TELECOMMUNICATIONS CONTRACTOR	
E/C	ELECTRICAL CONTRACTOR	
G/C	GENERAL CONTRACTOR	1
AC	DEVICE LOCATED ABOVE COUNTER	
TMGB	TELECOMMUNICATIONS MAIN GROUNDING BUSBAR	
TGB	TELECOMMUNICATIONS GROUNDING BUSBAR	
	TELECOMMUNICATIONS CABLING	5
FACP	FIRE ALARM CONTROL PANEL	
SM	SINGLEMODE FIBER	
ММ	MULTIMODE FIBER	
WAP	WIRELESS ACCESS POINT	6
TELECON	MMUNICATIONS CABLING IDENTIFIER	

1 - 4x4 STEEL CITY BACKBOX, MODEL NUMBER 72171-1-1/4 W/ SINGLE GANG PLASTER RING AND 1 1/4" CONDUIT TO ABOVE ACCESSIBLE CEILING AS INDICATED ON DRAWINGS BY E/C. 2 - 2x4 BACKBOX WITH 3/4" CONDUIT TO ABOVE ACCESSIBLE CEILING. 3 - 4x4 STEEL CITY BACKBOX, MODEL NUMBER 72171-1-1/4 WITH DOUBLE GANG PLASTER RING BY E/C. CONDUITS AS INDICATED ON PLANS. 4 - E/C TO PROVIDE CONDUIT BUSHING ON CONDUIT PRIOR TO T/C INSTALLING CABLING. 5 - CABLING SHALL BE SUPPORTED WITH J-HOOKS AT 48" O.C. WHERE NOT IN CONDUIT. 6 - 2x4 SURFACE MOUNT BACKBOX LOCATED ABOVE ACCESSIBLE CEILING.

	MECHANICAL S		ZEO EEGEND
P-1 D	WATER CLOSET & TYPE (TYP. FOR ALL PLUMBING FIXTURES)	— CHS—	CHILLED HOT SUPPLY
+ +	WASTE LINE ABOVE EARTH (W.)	— CHR—	CHILLED HOT RETURN
	WASTE LINE IN EARTH (W.)		UNION
— 11 co	CLEAN OUT	——————————————————————————————————————	FLEXIBLE PIPE CONNECTION
FFCO O	FLUSH FLOOR CLEAN OUT	7	MANUAL DAMPER
FGCO O	FLUSH GRADE CLEAN OUT	BD	BACKDRAFT DAMPER
2" (1) FD	FLOOR DRAIN AND TYPE	AD AD	AUTOMATIC DAMPER
—RD —	ROOF DRAIN	FD	FIRE DAMPER
—ORD—	OVERFLOW ROOF DRAIN	FS)	FIRE/SMOKE DAMPER
2" (1) RD	ROOF DRAIN AND TYPE	\$D	SMOKE DAMPER
	VENT LINE (V.)	6x6 A 80	GRILLE, REGISTER OR DIFFUSER, SIZE, TYPE & CFM
	DOMESTIC COLD WATER SUPPLY (DCW)		VOLUME EXTRACTOR AND TURNING VANES
	DOMESTIC HOT WATER SUPPLY (DHW)		RETURN, EXHAUST OR FRESH AIR DUCT SECTION UP & DOWN
	DOMESTIC HOT WATER RETURN (DHWR)		SUPPLY AIR DUCT SECTION UP AND DOWN
→ HB/36"	HOSE BIBB AND MOUNTING HEIGHT		FLEXIBLE DUCT CONNECTION
—⊟ WH	WALL HYDRANT		ROUND OR RECTANGULAR DUCT
— F —	FIRE LINE/STANDPIPE		FLEXIBLE DUCT
— D —	DRAIN LINE	P	THERMOSTAT
— G —	NATURAL GAS LINE	— R —	REFRIGERANT LIQUID/SUCTION
-121-13	RISE & DROP IN PIPE WITH CUT-OFF VALVE	AD	ACCESS DOOR
	REDUCER	AFF	ABOVE FINISHED FLOOR
<u> </u>	CHECK VALVE	EA	EXHAUST AIR
—⋈—	STOP VALVE	OA	OUTSIDE AIR
—⊠—	BALANCING VALVE/AUTOFLOW VALVE	RA	RETURN AIR
⋈	PLUG VALVE	SA	SUPPLY AIR
—₩ —	2-WAY CONTROL VALVE OR SOLENOID VALVE	VBS	VENT BELOW SLAB
────	3-WAY CONTROL VALVE OR SOLENOID VALVE	VTR	VENT THRU ROOF
<u></u> &	PRESSURE REDUCING VALVE	•	CONNECT NEW TO EXISTING
	STRAINER		LOCKABLE GUARD
—cws—	CHILLED WATER SUPPLY	VFD	VARIABLE FREQUENCY DRIVE
—cwr—	CHILLED WATER RETURN		
—HWS—	HOT WATER SUPPLY		
—HWR—	HOT WATER RETURN		

	CONDUIT CONCEALED IN CEILING OR WALL. 2 HOT + GROUND.	P	THERMOSTAT
/ _ \	CONDUIT CONCEALED IN FLOOR SLAB		LOCKABLE GUARD
/\	EXPOSED CONDUIT		JUNCTION BOX
	HOMERUN - ARROW INDICATES CKT., LINES INDICATE WIRES	\$	SWITCH - SINGLE POLE
— ⊨	GROUNDING ROD	\$	SWITCH - 3-WAY
ф	SINGLE RECEPTACLE	\$ 4	SWITCH - 4-WAY
Þ	DUPLEX RECEPTACLE (20 AMP UNLESS NOTED)	A	LIGHT FIXTURE AND TYPE
þ u	DUPLEX RECEPTACLE WITH USB OUTLETS	\searrow	EMERGENCY LIGHT FIXTURE WITH BATTERY PACK
þ sw	SWITCHED DUPLEX RECEPTACLE		FIXTURE ON LIFE SAFETY BRANCH OF EMERGENCY SYSTEM
#	FOURPLEX RECEPTACLE	어머	LIGHT FIXTURE (WALL MOUNTED)
ф	208 OR 240 VOLT RECEPTACLE (20 AMP UNLESS NOTED)	⊗⊗	EXIT LIGHT (CEILING OR WALL MOUNTED)
Ø	GROUND FAULT INTERRUPTER (GFI) DUPLEX RECEPTACLE		FLUSH PANELBOARD (LIGHT & RECEPTACLES)
•	TELE/DATA OUTLET *		SURFACE PANELBOARD (LIGHT & RECEPTACLES)
8	PUSHBUTTON		DISTRIBUTION PANEL OR SWITCHBOARD
[VFD]	VARIABLE FREQUENCY DRIVE	AC	DEVICE LOCATED ABOVE COUNTER
[ORT]	OVERRIDE TIMER	AFF	ABOVE FINISHED FLOOR
[PC]	PHOTOCELL	D	DIMMER
Ó	MOTOR	E	INDICATES EXISTING DEVICE
\$	FUSIBLE SWITCH (BUSSMAN SSU)	EDF	ELECTRIC DRINKING FOUNTAIN
D	DISCONNECT SWITCH (D.S.)	NL	NIGHTLIGHT FIXTURE, WIRED HOT
4⊠	COMBINATION MOTOR STARTER (CMS)	WP	WEATHERPROOF
[R]	RELAY	AFCI	ARC FAULT CIRCUIT INTERRUPTER
⊚	FLOOR BOX	•	CONNECT NEW TO EXISTING

PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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REVISIONS



PROJECT TEAM

GBA ENGINEERS

LATIMER SOMMERS

ARCHITECT FINKLE+WILLIAMS ARCHITECTURE

CIVIL

LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL

MECHANICAL LATIMER SOMMERS

PLUMBING

ELECTRICAL LATIMER SOMMERS

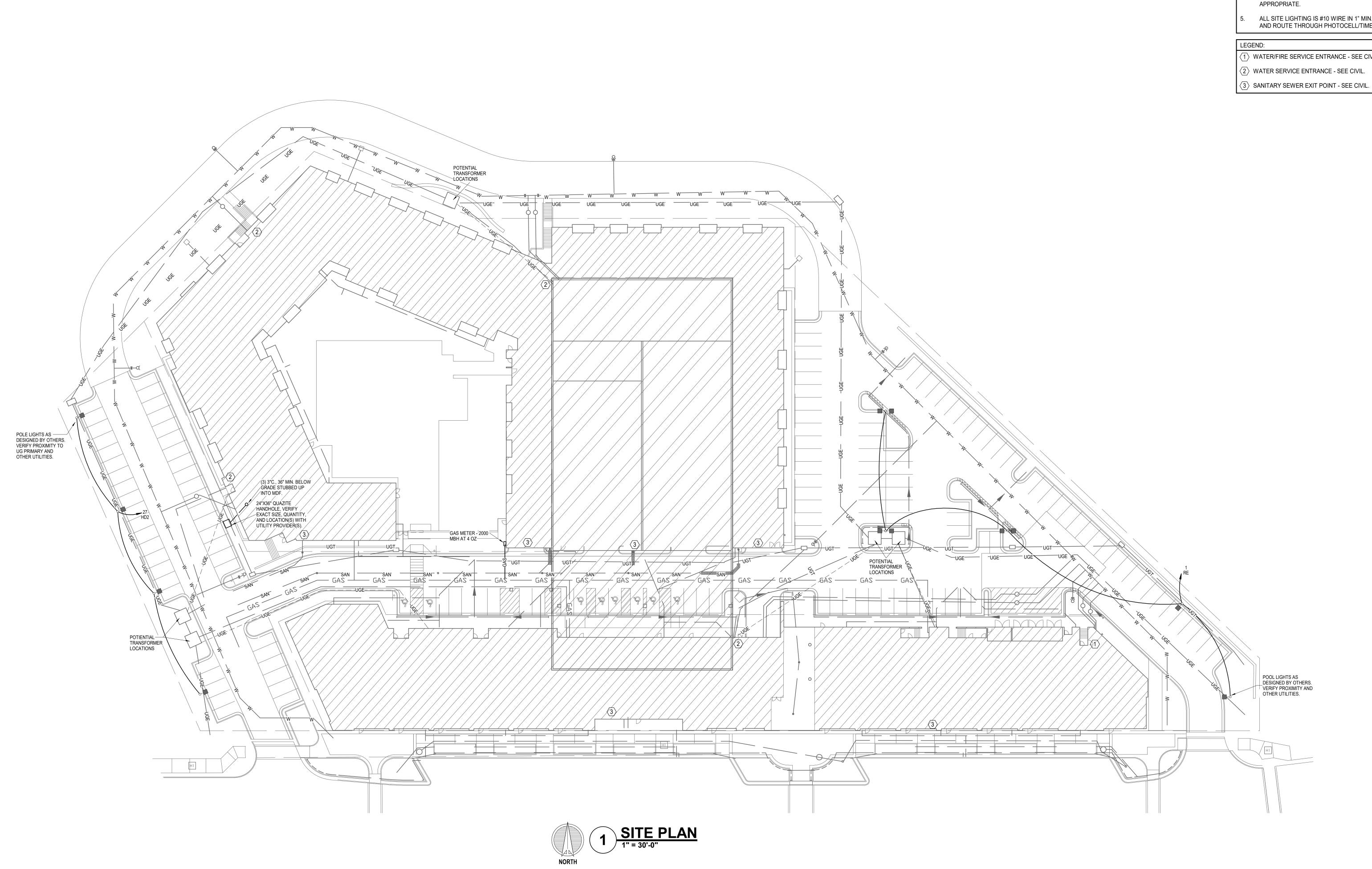
FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR BRINKMANN CONSTRUCTORS



SHEET TITLE

COVER SHEET



COORDINATE WITH CIVIL PLANS FOR EXACT LOCATIONS OF UTILITIES, SITE FEATURES AND GRADE.

- COORDINATE WITH UTILITY PROVIDERS, INCLUDE ALL REQUIREMENTS AND FEES WITHIN THE BID WORK. IF NO FEE IS AVAILABLE, PROVIDE AN ALLOWANCE AND LIST ON BID SUBMISSION.
- TRANSFORMER LOCATIONS ARE AS SUGGESTED AND PREFERRED. POWER COMPANY WILL MAKE FINAL DETERMINATION.
- FOR ANY GAS SERVICES, PROVIDE STEEL RISER, SHUT-OFF AND APPROPRIATE PRV AT APPLIANCES IF
- ALL SITE LIGHTING IS #10 WIRE IN 1" MIN. PVC CONDUIT AND ROUTE THROUGH PHOTOCELL/TIMER.

1 WATER/FIRE SERVICE ENTRANCE - SEE CIVIL.

- $\langle 2 \rangle$ WATER SERVICE ENTRANCE SEE CIVIL.

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ARCHITECT

LANDSCAPE

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LATIMER SOMMERS MECHANICAL

FIRE PROTECTION LATIMER SOMMERS

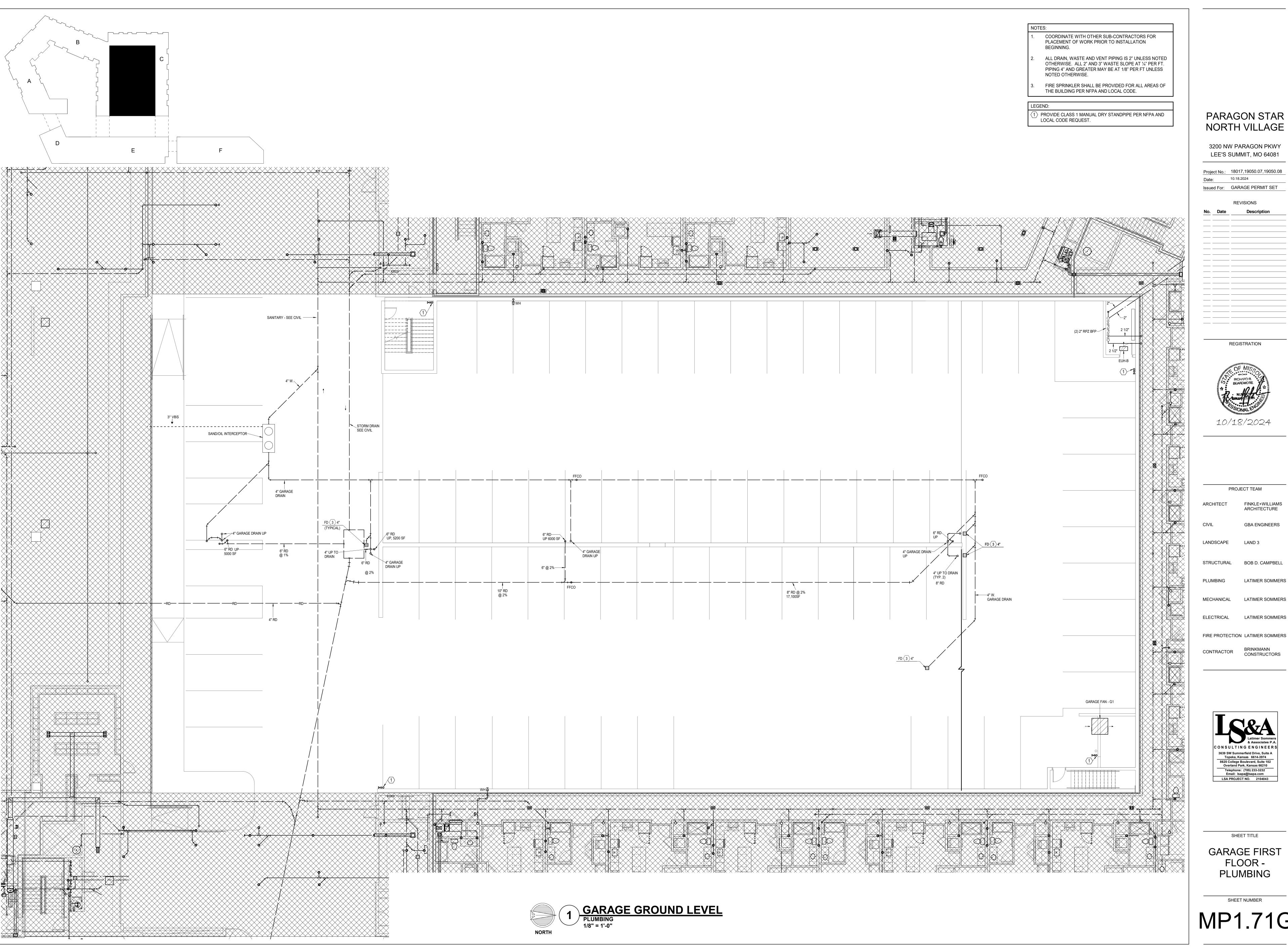
ELECTRICAL LATIMER SOMMERS

BRINKMANN CONSTRUCTORS CONTRACTOR



LSA PROJECT NO. 2104043

SITE PLAN



> 3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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FINKLE+WILLIAMS ARCHITECTURE

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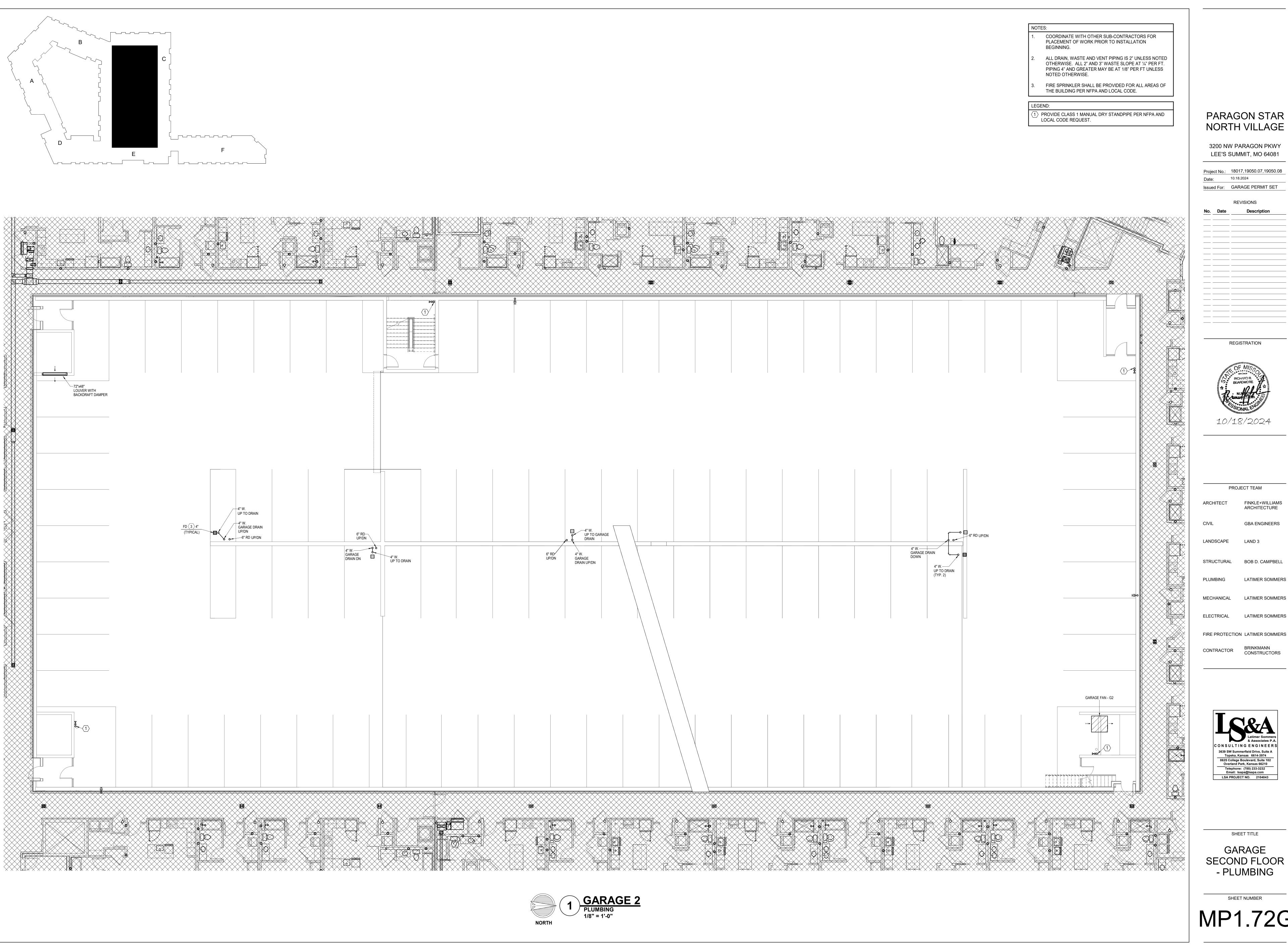
LATIMER SOMMERS

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SHEET TITLE

GARAGE FIRST FLOOR -PLUMBING

MP1.71G



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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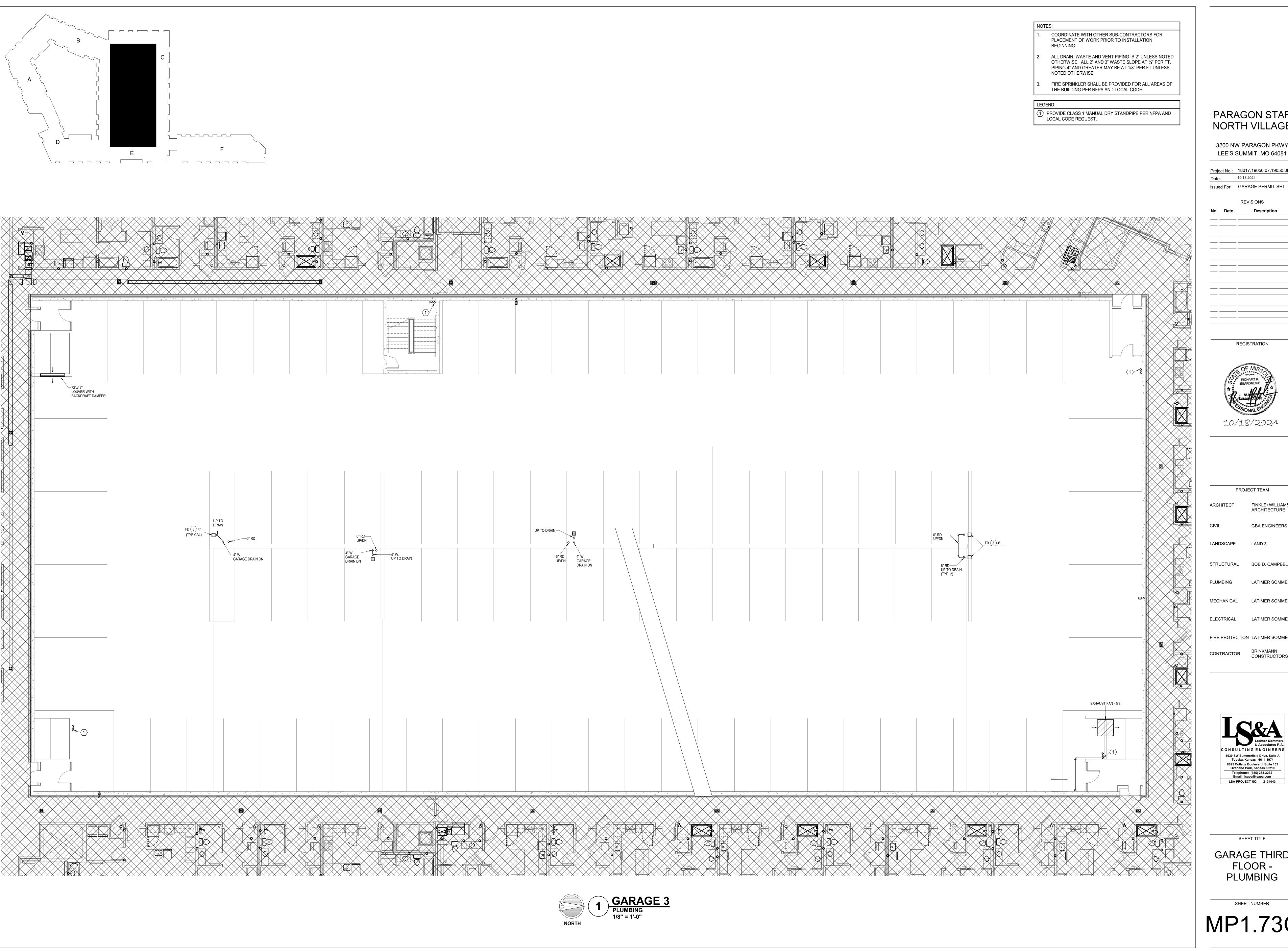
BRINKMANN CONSTRUCTORS CONTRACTOR



SHEET TITLE

GARAGE SECOND FLOOR - PLUMBING

MP1.72G



> 3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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LATIMER SOMMERS

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FIRE PROTECTION LATIMER SOMMERS

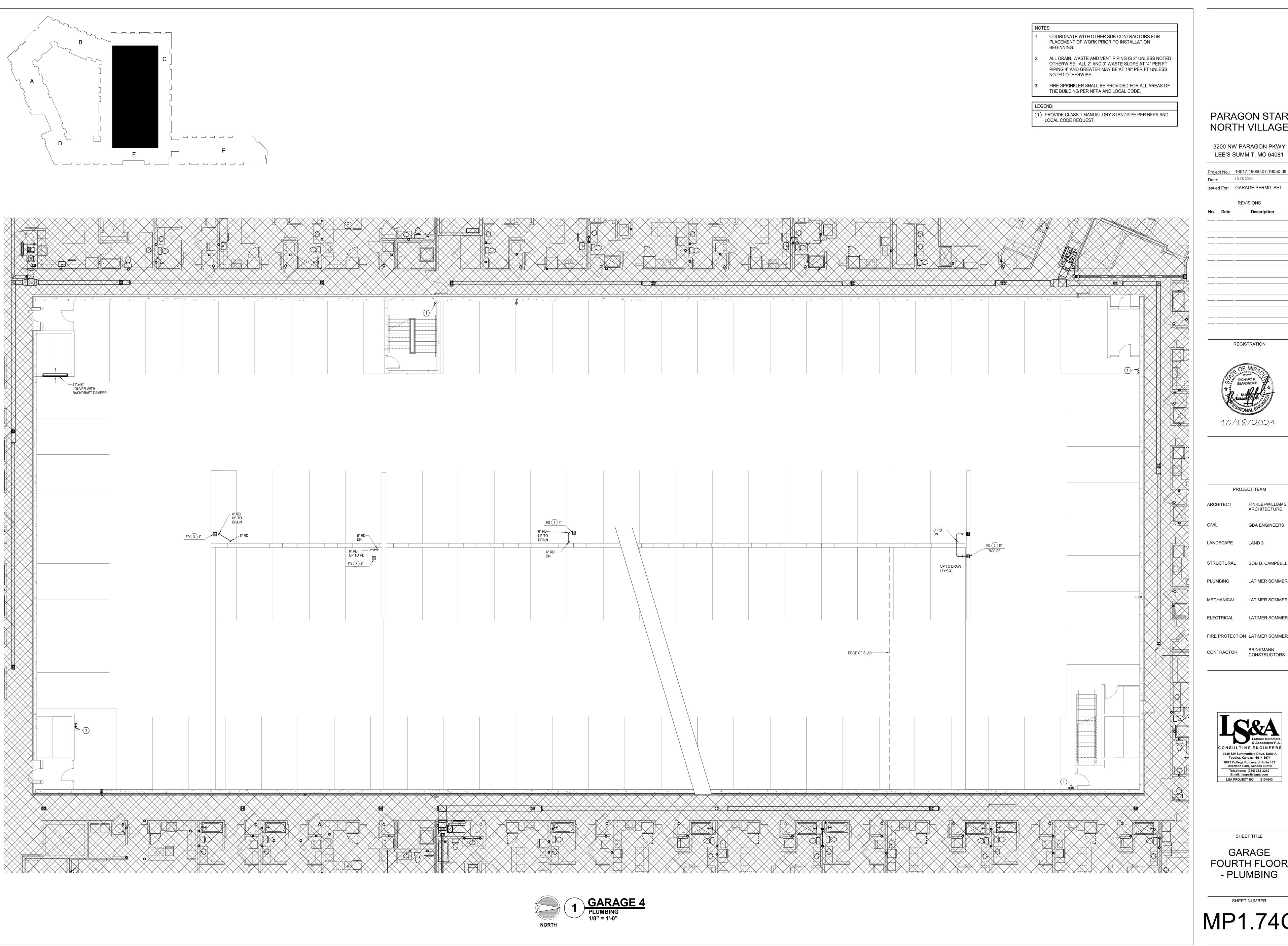
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SHEET TITLE

GARAGE THIRD FLOOR -PLUMBING

MP1.73G



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GBA ENGINEERS

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8625 College Boulevard, Suite 102
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Telephone: (785) 233-3232
Email: Isapa@Isapa.com
LSA PROJECT NO. 2104043

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GARAGE FOURTH FLOOR - PLUMBING



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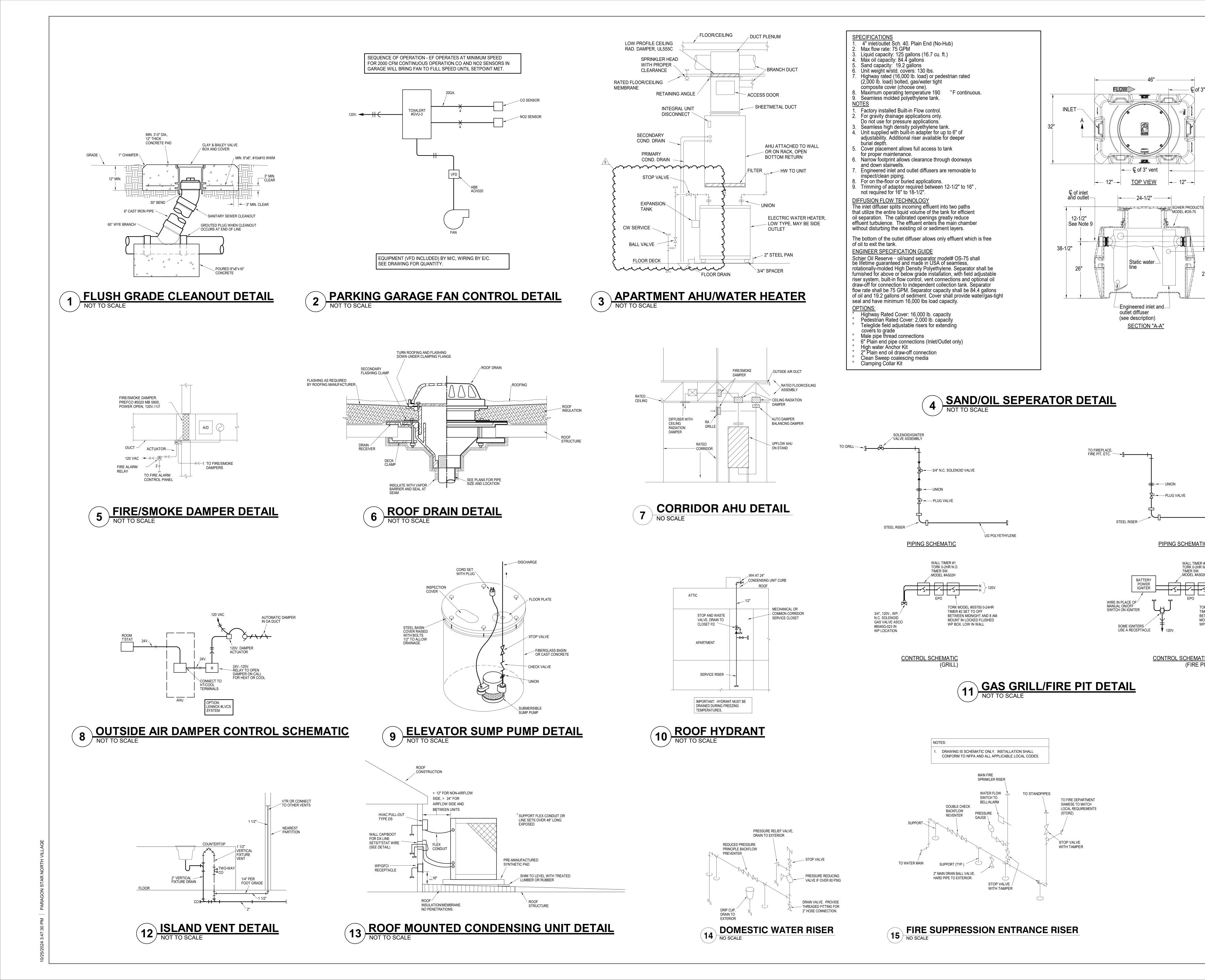
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BRINKMANN CONSTRUCTORS

3639 SW Summerfield Drive, Suite A Topeka, Kansas 6614-3974 8625 College Boulevard, Suite 102 Overland Park, Kansas 66210

GARAGE FIFTH FLOOR -PLUMBING

MP1.75G



-OUTLET

– **ଢ଼ି** of 2" optional

- $oldsymbol{\mathbb{Q}}$ of optional 2'

– ♀ of 3" vents

oil draw-off

23-3/4"

Invert

UG POLYETHYLENE

TORK MODEL #SS700 0-24HR

BETWEEN MIDNIGHT AND 8 AM.

MOUNT IN LOCKED FLUSHED

TIMER #2 SET TO OFF

WP BOX, LOW IN WALL

∔ → UNION

X→—PLUG VALVE

PIPING SCHEMATIC

CONTROL SCHEMATIC (FIRE PIT)

WALL TIMER #1 TORK 0-2HR N.O. TIMER SW.

MODEL #A502H

oil draw-off

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GBA ENGINEERS

LAND 3 LANDSCAPE

STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

LATIMER SOMMERS

ELECTRICAL

CONTRACTOR

FIRE PROTECTION LATIMER SOMMERS BRINKMANN CONSTRUCTORS

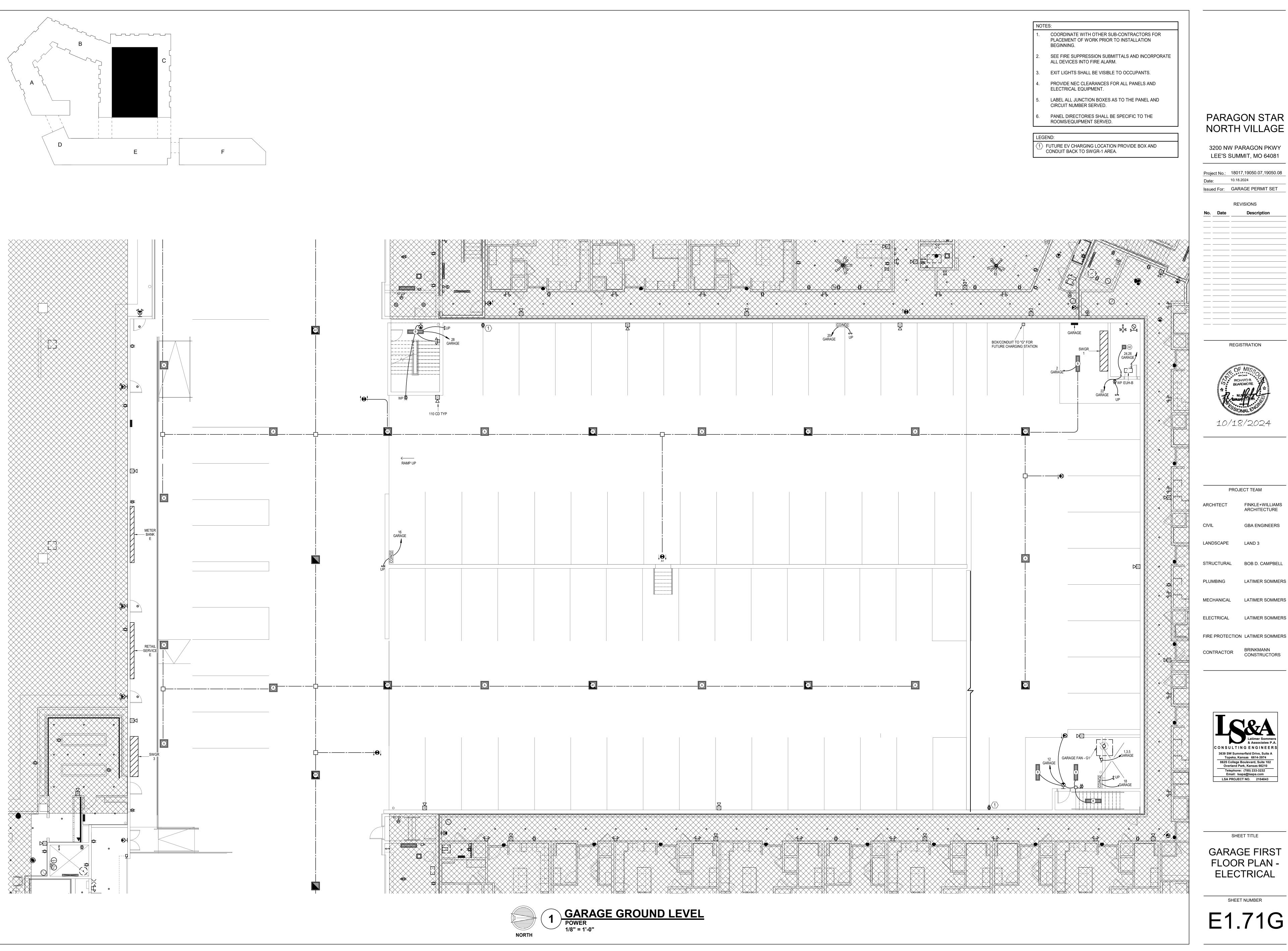


SHEET TITLE

MECHANICAL DETAILS/SCHEDULES

SHEET NUMBER

MP2.01



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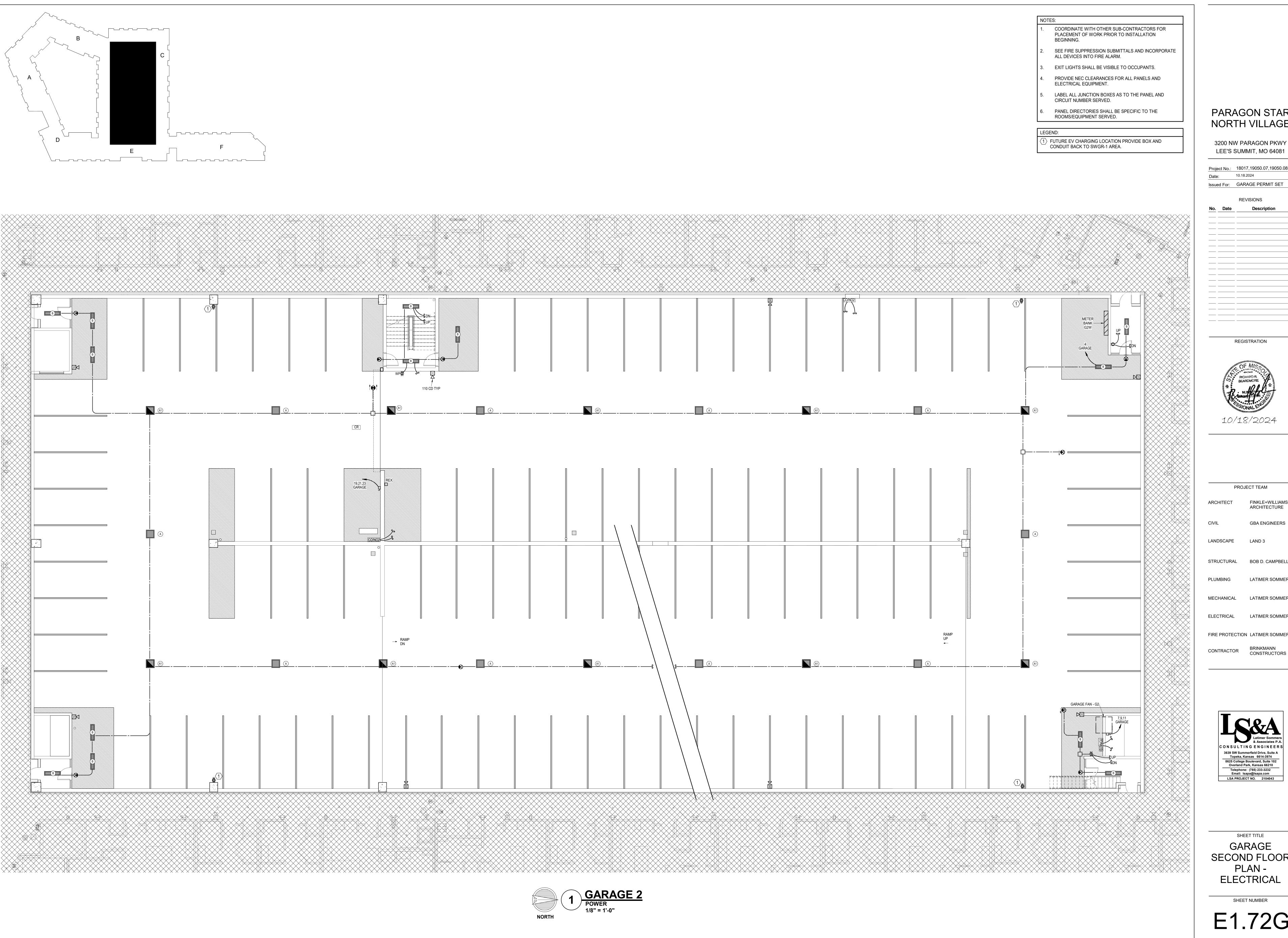
LATIMER SOMMERS

SHEET TITLE

GARAGE FIRST FLOOR PLAN -ELECTRICAL

SHEET NUMBER

E1.71G



3200 NW PARAGON PKWY

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PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE

GBA ENGINEERS

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MECHANICAL LATIMER SOMMERS

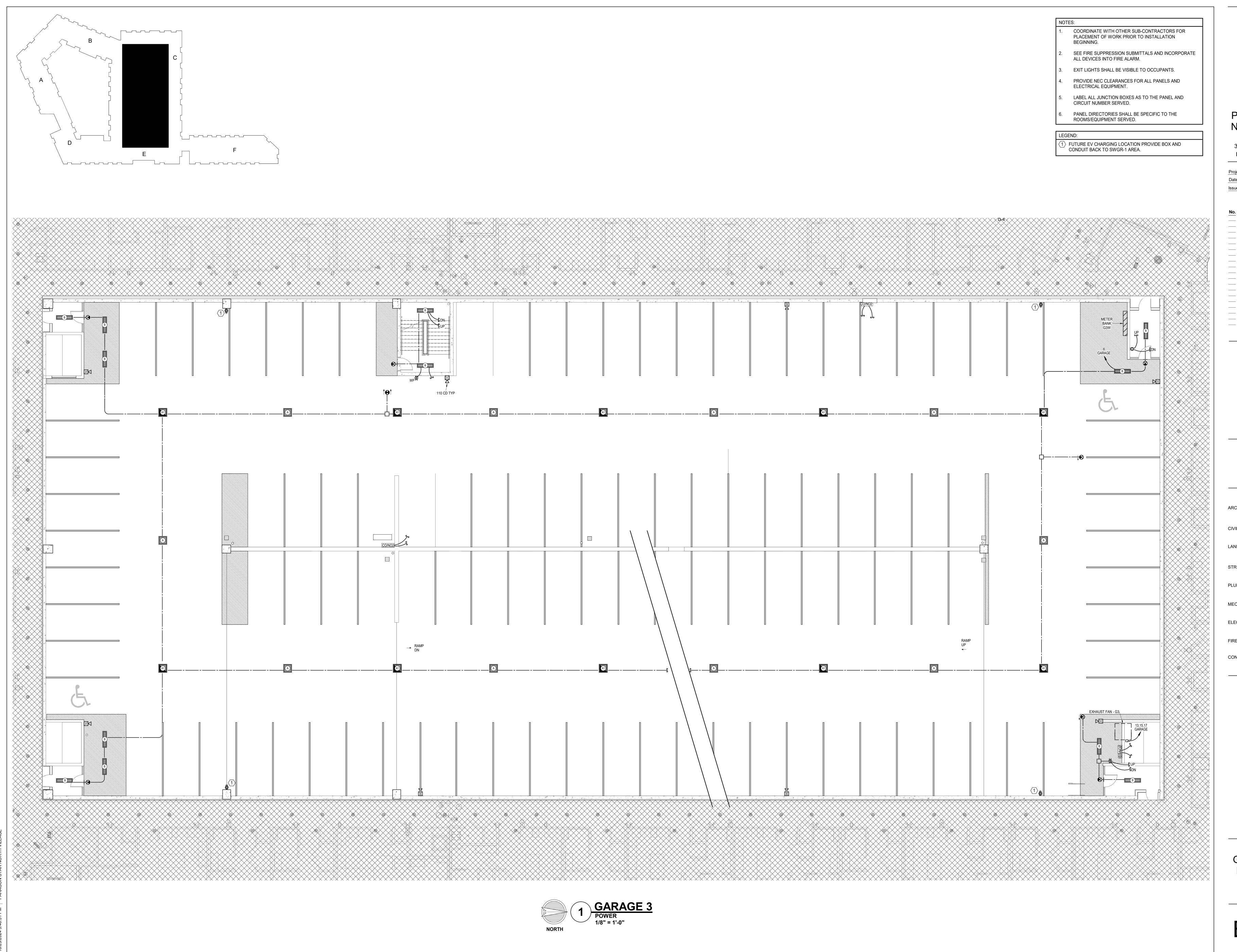
FIRE PROTECTION LATIMER SOMMERS

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CTURAL BOB D. CAMPBELI

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FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR BRINKMANN CONSTRUCTORS

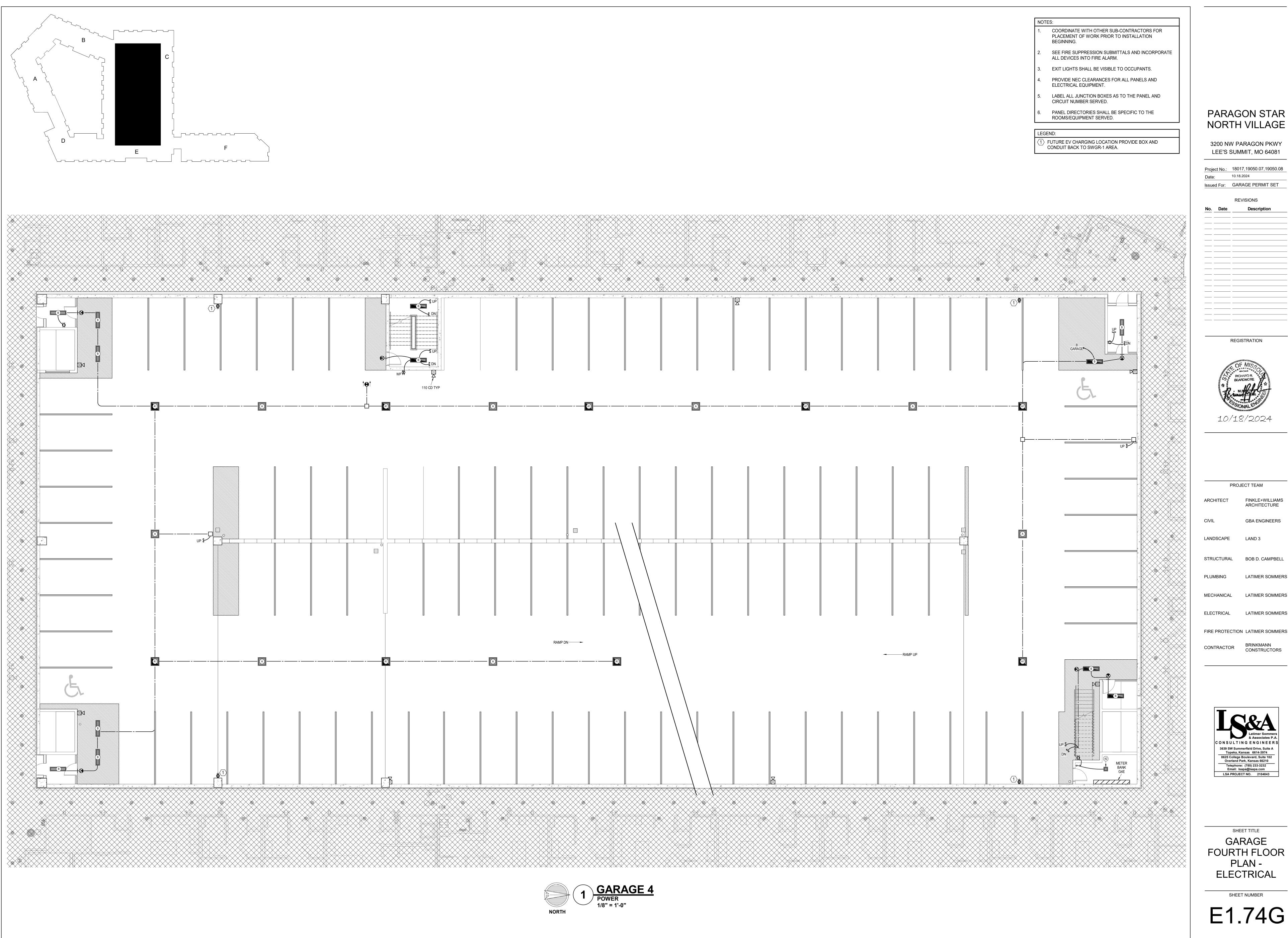


SHEET TITLE

GARAGE THIRD FLOOR PLAN -

SHEET NUMBER

E1.73G



3200 NW PARAGON PKWY

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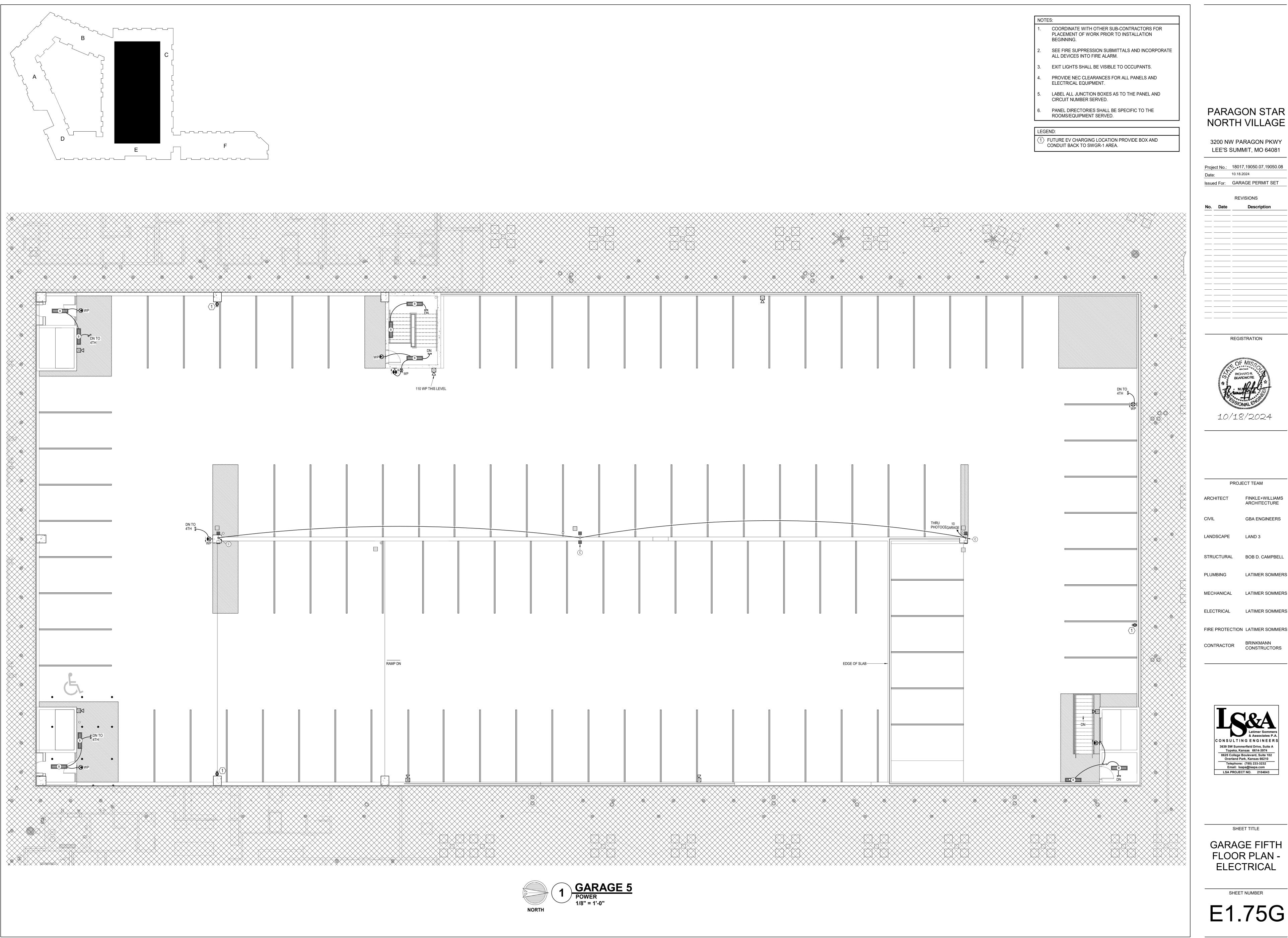
LATIMER SOMMERS

LATIMER SOMMERS

BRINKMANN CONSTRUCTORS CONTRACTOR

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SHEET TITLE GARAGE FOURTH FLOOR PLAN -



> 3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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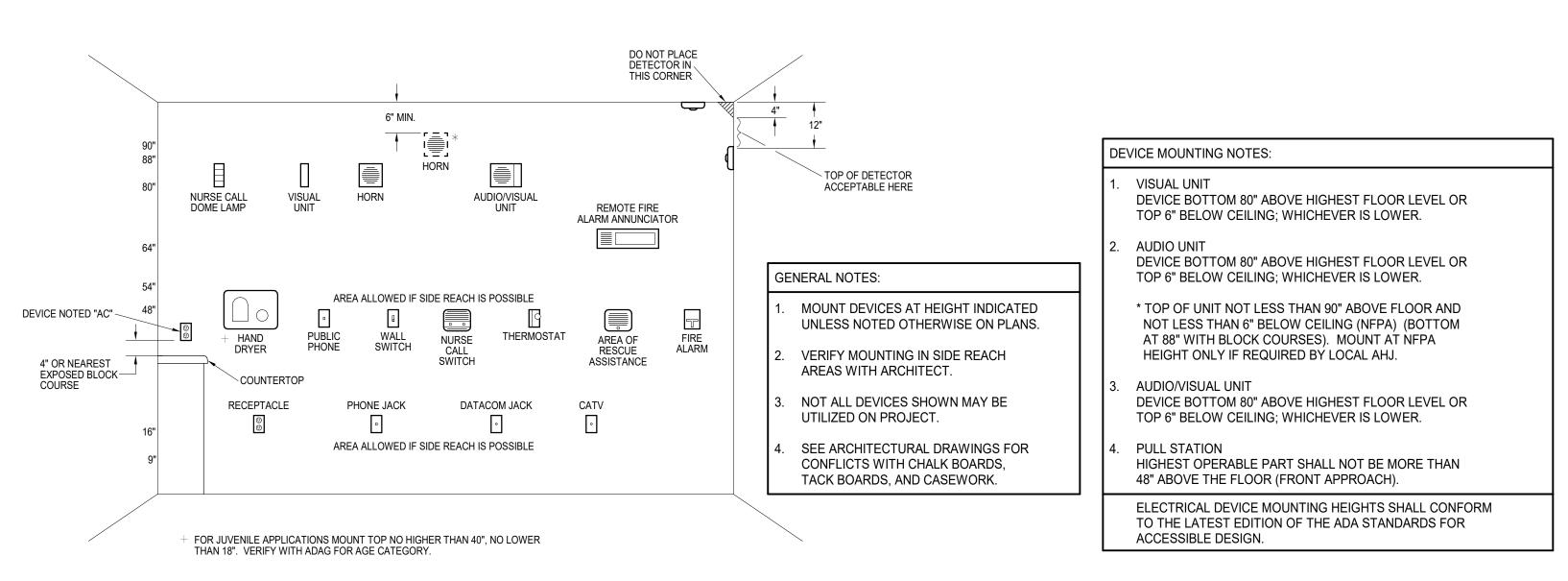
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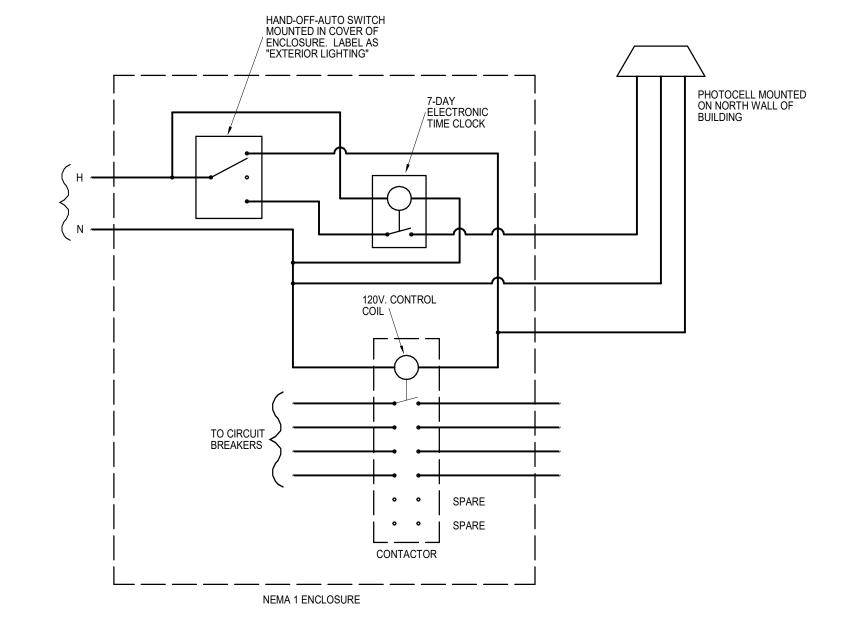
BRINKMANN CONSTRUCTORS



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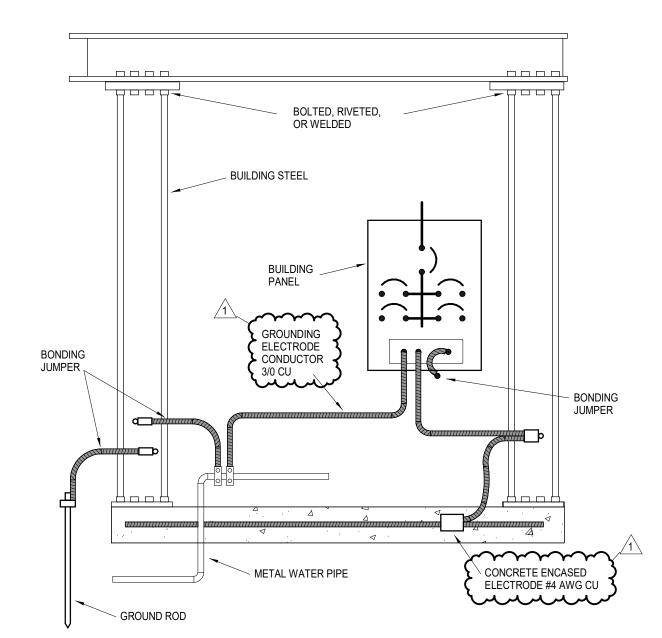
FLOOR PLAN -ELECTRICAL



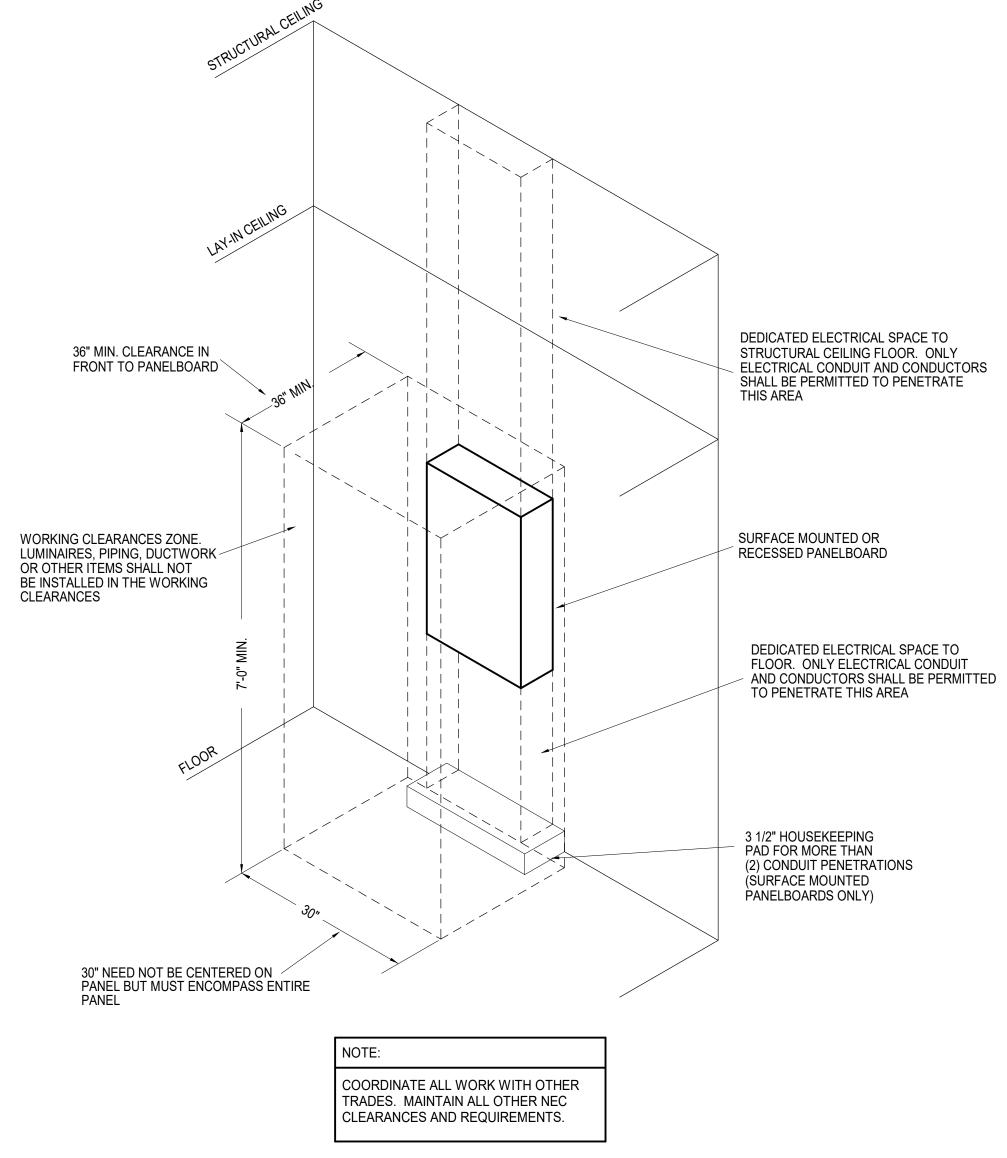


1 ELECTRICAL DEVICE MOUNTING HEIGHTS NOT TO SCALE

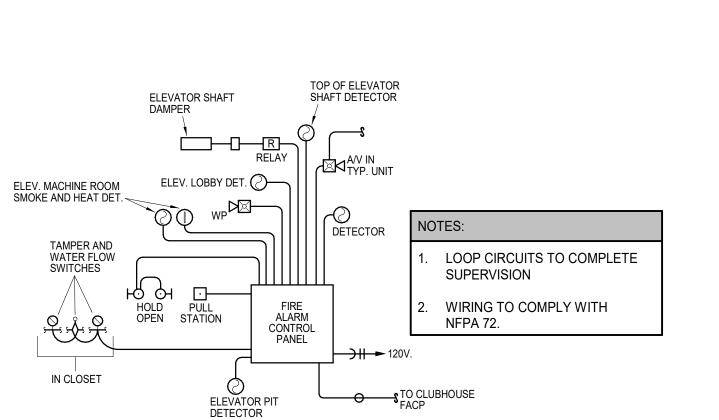
2 EXTERIOR LIGHTING CONTROL PANEL SCHEMATIC NOT TO SCALE



3 GROUNDING ELECTRODE SYSTEM DETAIL
NOT TO SCALE



4 TYPICAL PANELBOARD INSTALLATION DETAIL
NOT TO SCALE



5 FIRE ALARM RISER DIAGRAM
NOT TO SCALE

PARAGON STAR NORTH VILLAGE

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PROJECT TEAM

ARCHITECT FINKLE+WILLIAMS
ARCHITECTURE

BOB D. CAMPBELL

CIVIL GBA ENGINEERS

LANDSCAPE LAND 3

STRUCTURAL

PLUMBING LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

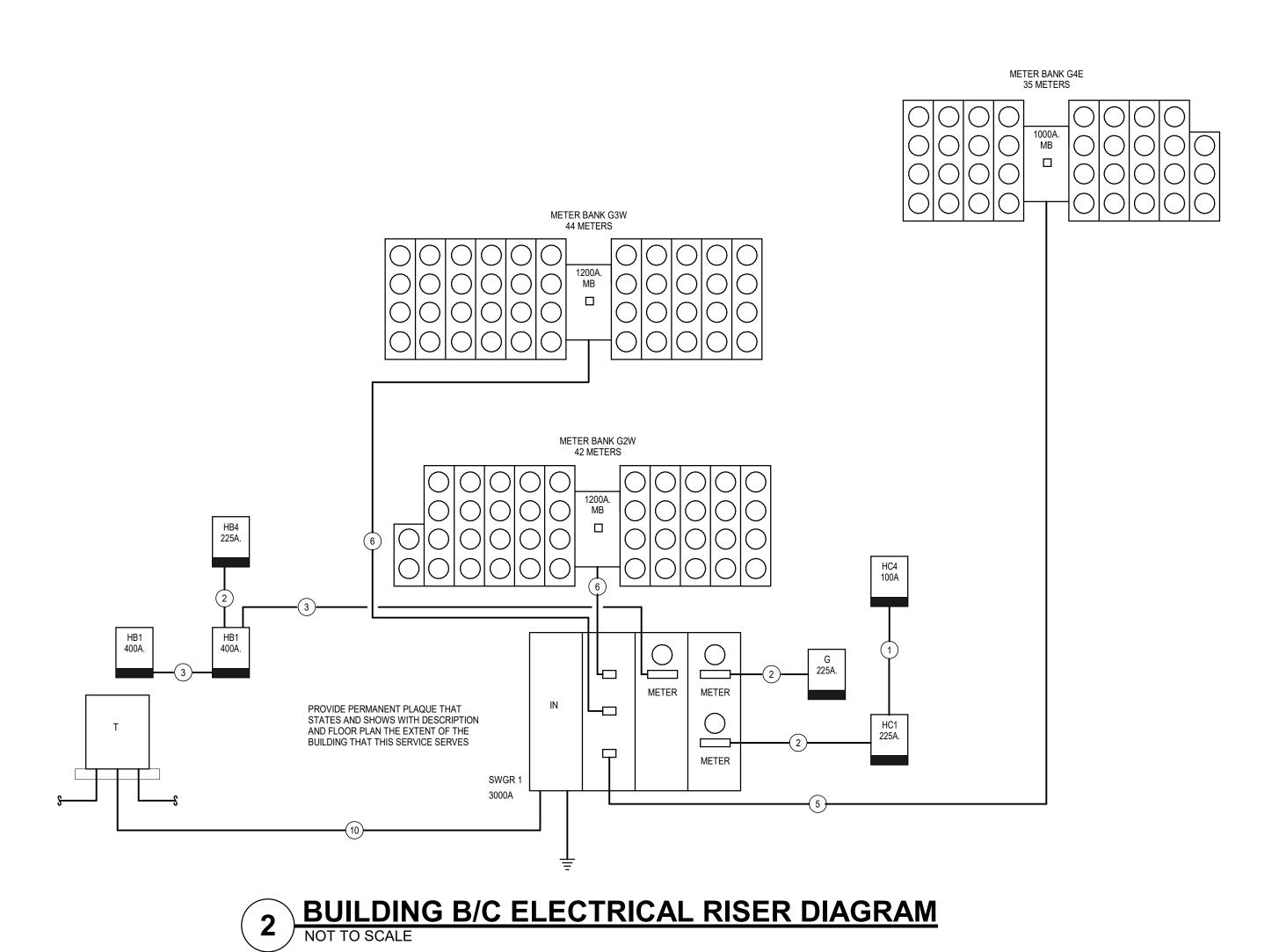
FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR BRINKMANN CONSTRUCTORS



SHEET TITLE

ELECTRICAL DETAILS



			SWITCH	GEAR LOAD AN	ALYSIS GARAGE			
Panel Name	Total Units	Total Connected	NEC Building Diversity	Diversified KVA	Amps @ 208V/3PH	House Load	Retail Load	Total Demand (Amps)
SWGR 1	121	3,802,320 VA	0.23	875 VA	2429	560	0	2989

			Multi-Family	Building Load Analysis	GARAGE			
Meter Bank	Total Units	Total Connected Load	Total KVA	NEC Building Diversity	Diversified KVA	Amps @ 208V/3PH	House Load	Total Demand - Am
Unit Type	Unit Quantity	Sum of Units						
METER BANK G2W	42	1,316,085 VA	1316	28%	369	1024	0	1024
A-1 ADA	2	57780 VA	1310	2070	309	1024	U	1024
B-1	6	175140 VA						
C-1	17	536180 VA						
C-1S	1	31540 VA						
C-3	9	282645 VA						
C-5	2	63530 VA						
D-3	2	65360 VA						
D-4	1	32830 VA						
D-10	2	71080 VA						
D-10		71000 VA						
METER BANK G3W	44	1,401,565 VA	1402	27%	378	1051	0	1051
A-1 ADA	2	57780 VA						
B-1	6	175140 VA						
C-1	16	504640 VA						
C-1S	2	63080 VA						
C-3	9	282645 VA						
C-5	2	63530 VA						
D-3	1	32680 VA						
D-4	2	65660 VA						
D-10	1	35540 VA						
PH-3	1	40290 VA						
PH-4	1	39690 VA						
PH-5	1	40890 VA						
METER RANK OAF	25	4.004.070.1/4	1005	200/	205	004	0	004
METER BANK G4E	35	1,084,670 VA	1085	30%	325	904	0	904
A-1	12	346680 VA						
C-1S	16	504640 VA						
C-12 D-5	3	127180 VA 106170 VA		-				

		F	EEDER SCHEDU	ILE		
MARK	OCP	SETS	NO. COND.	SIZE ALUM.	SIZE GRD. CU	CONDUIT
1	100 A	1	4	#1/0	#4	1 1/2"
2	225 A	1	4	300 KCMIL	#2	3"
3	400 A	2	4	#4/0	#1	2"
4	600 A	2	4	500 KCMIL	#2/0	3"
5	1000 A	3	4	600 KCMIL	#4/0	4"
6	1200 A	4	4	500 KCMIL	250 KCMIL	3"
7	1600 A	5	4	600 KCMIL	350 KCMIL	4"
9	800 A	3	4	400 KCMIL	#3/0	3"
10	2000 A	6	4	600 KCMIL	400 KCMIL	4"

Garage Luminaire Schedule										
MARK	DESCRIPTION	MFGR	MODEL	MOUNTING	FINISH	LAMPS	NOTES			
Α	Garage Light	Royal Pacific	44310B-60-MS	surface	standard	60W 4000K	with 50% motion dimming			
A1	Garage Light	Royal Pacific	44310B-60-MS	surface	standard	60W 4000K	with 50% motion dimming/battery			
A2	Surface Globe	Lithonia	LDN4CYL-40/LO4/AR/LSS/fcm	surface	white	1000 Lumen 4000K 10W				
В	Strip	Lithonia	MNSL-L46 1LL MVOLt 40k	surface	white	20W 4000K LED	with battery			
С	Double head Pole	McGraw Edison	(2) GLEON-SA2C-740-U-5WQ	jb/surface	white	15,200 Lumen 4000K 115W LED	16 ft pole			
X1	Exit	Lithonia	LQM SW3R	surface	white/red	LED				
X2	Exit - waterproof	Lithonia	WLTE-W-1R-EL	surface	white/red	LED				

	Panel: G	ARA	GE												
Location: GARAGE Supply From: Mounting: SURFACE Enclosure:					Volts: 120/208 Wye Phases: 3 Wires: 4						A.I.C. Rating: 10000 Mains Type: MLO Mains Rating: 225 A MCB Rating:				
Notes	:														
СКТ	Circuit Description	Trip	Wire Size		A		В	(2	Wire Size	Trip	Circu	it Description	СК	
1	•			1321	1820					12	20 A	Lighting		2	
3	GARAGE EF	15 A	12			1321	1600	1001	4000	12	20 A	Lighting		4	
5 7				1321	1752	,		1321	1680	12 12	20 A 20 A	Lighting		8	
9	GARAGE EF	15 A	12	1321	1/52	1321	600			12	20 A	Lighting Lighting		10	
11	OAIVAGE EI	15 A	12			1021	000	1321	833	12	20 A	Lighting		12	
13				1321				1021			2071			14	
15	GARAGE EF	15 A	12			1321	800			12	20 A	CO SENSO	PR	16	
17								1321	800	12	20 A	CO SENSO	PR	18	
19		20 A	12	0	800					12	20 A	CO SENSO	R	20	
21	Power Space 1305					0	296			12	20 A	Receptacle		22	
23								0	2704	8	35 A	EUH-B		24	
25					2704	ŀ								26	
27							1995			12	20 A	Lighting ST	AIR 106	28	
29														30	
31														32	
35														36	
37														38	
39														40	
41														42	
		Tota	al Load:	1092	<u></u>	918	4 VA	9912	2 VA						
			I Amps:	92	2 A	77	7 A	84	- A						
Leger	nd:					·									
Load Classification			Connected Load		ıd C	Demand Factor		Estimated			Pane		Totals		
HVAC			17297 VA			65.00%		11243 VA							
Lighting			8652 VA			125.00%		10815 VA		Total Conn. Load					
Other		2400 VA			100.00%		2400 VA		Total Est. Demand						
Power Receptacle		500 VA 1440 VA			100.00%		500 VA 1440 VA		Total Conn. Total Est. Demand						
kecep	ласіе		144	U VA		100.009	70	144	U VA		ı otal E	sı. Demand:	12 A		
					_										

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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ARCHITECTURE

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LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL
PLUMBING LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR BRINKMANN
CONSTRUCTORS



SHEET TITLE

ELECTRICAL DETAILS/SCHEDULES

SHEET NUMBER

E3.02G