

Permit No: PRRES20245465

Plan Name:

Project Address: 3904 SW FLINTROCK DR, LEES SUMMIT, MO 64082

Parcel Number: 277641

Location: NAPA VALLEY 5TH PLAT LOTS 166-196 AND TRACT O --- LOT 195

BILL'S PLUMBING SERVICE LLC 308 E PARK STREET OLATHE, KS 66061 (913) 829-8213

Our firm has been asked to make structural clarifications to the plans of the house to be built at the address listed above. During the permit review process the AHJ has questioned items. Below is a list of our recommendations along with the corresponding city item.

Please clarify suspended slab. Suspended slab removed from plan, see clouded Foundation Plan change.

Finish Line Electric does not have a current Lee's Summit business license. To be completed by contractor.

Suspended stoop details. Suspended stoop removed see clouded Foundation Plan change.

This report has been prepared for the exclusive use of our client for specific application to the project discussed and has been prepared in accordance with generally accepted engineering practices. No warranties, either express or implied, are intended or made.

We appreciate the opportunity to be of service to you on this project. If you have any questions regarding this report, please contact us.



STRUCTURAL REVIEW

HD ENGINEERING & DESIGN
RELEMSEGOR (TONS) TRUETION

DEVELOPMEN-634-2222 SE VICE®NSUMINEUSSOURI

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48058 HD#:

8/6/2024 DATE: CHECKED BY: CLS

Revision ISSUE/REVISION Date

SQUARE FOOTAGE

111 SF

1094 SF

1308 SF

954 SF

60 SF 662 SF

FRONT ELEVATION

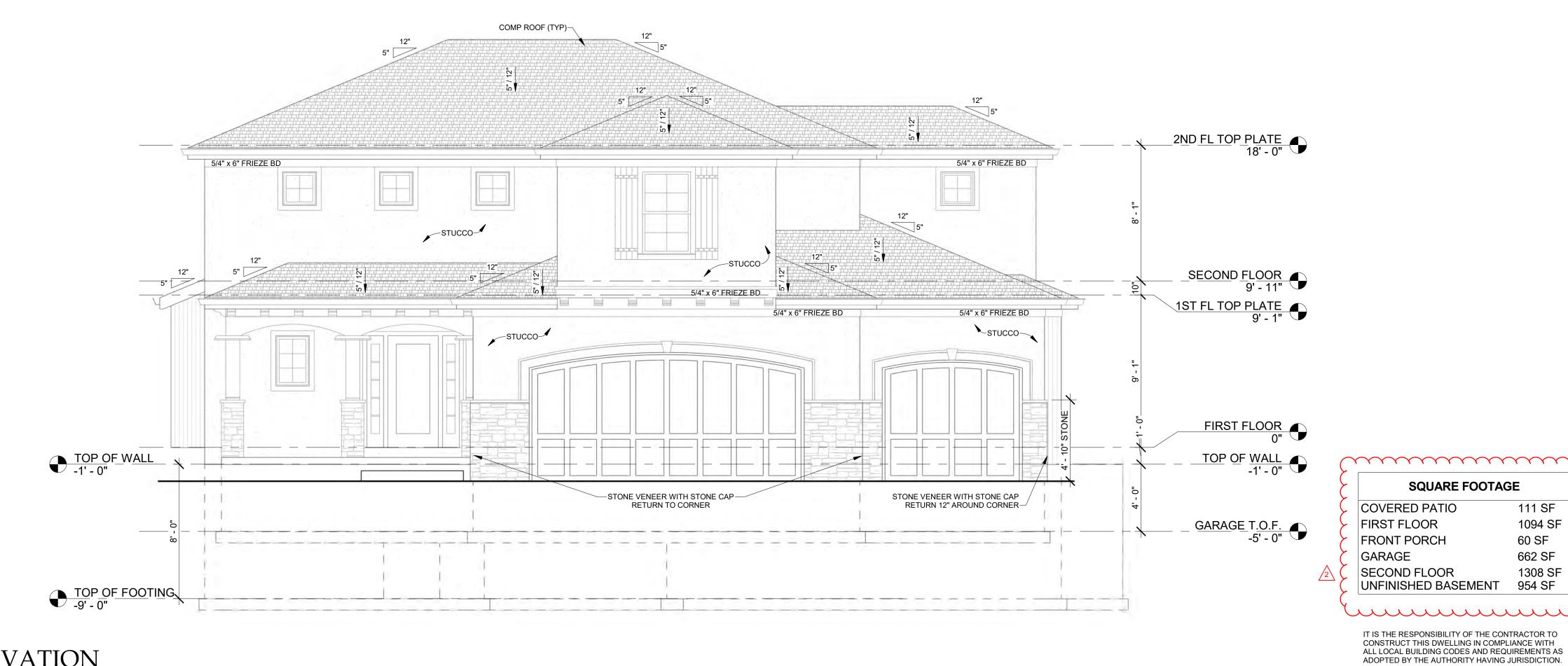
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THE HEATHER

BY FIRST CHOICE CUSTOM HOMES

2 STORY, 5 BEDROOM, 3 BATHROOM, 3-CAR GARAGE IN 3904 S.W. FLINTROCK DR. LEES SUMMIT, MO





RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW **DEVELOPMENT SERVICES** LEE'S SUMMIT, MISSOURI 11/05/2024 4:42:49

BUILDERS PLAN: THIS IS A BUILDERS PLAN, AND BY DEFINITION, REFERS TO A CERTAIN LEVEL

OF DEVELOPMENT OF THE DRAWINGS. AS THE NAME IMPLIES, THESE PLANS REQUIRE THAT

THE CONTRACTOR POSSESSES COMPETENCE IN RESIDENTIAL CONSTRUCTION. THE CONTRACTOR WARRANTS TO HD ENGINEERING & DESIGN, INC AND ITS CONSULTANTS, THAT THEY POSSESS THE PARTICULAR COMPETENCE AND SKILL IN CONSTRUCTION NECESSARY TO BUILD THIS PROJECT WITHOUT FULL ENGINEERING AND ARCHITECTURAL DESIGN SERVICES, AND FOR THAT REASON THE CONTRACTOR OR HOMEOWNER HAS RESTRICTED THE SCOPE OF PROFESSIONAL SERVICES. THE CONSTRUCTION DOCUMENTS PROVIDED BY THE LIMITED

SERVICES SHALL BE TERMED"BUILDERS PLANS" IN RECOGNITION OF THE CONTRACTORS SOPHISTICATION. ALTHOUGH HD ENGINEERING AND DESIGN, INC. AND ITS CONSULTANTS HAVE PERFORMED THEIR SERVICES WITH DUE CARE AND DILIGENCE, WE CANNOT GUARANTEE

PERFECTION. ANY AMBIGUITY OR DISCREPANCY DISCOVERED BY THE USE OF THESE PLANS SHALL BE REPORTED IMMEDIATELY TO MARSHALL HOME DESIGN, LLC. CONSTRUCTION MAY REQUIRE THAT THE CONTRACTOR ADAPT THE "BUILDER PLANS" TO THE FIELD CONDITIONS

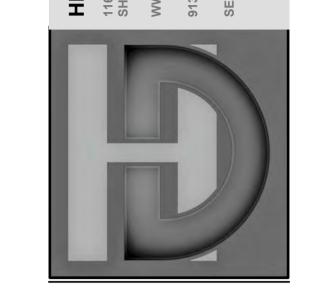
REQUIRE THAT THE CONTRACTOR ADAPT THE BUILDER PLANS TO THE FIELD CONDITIONS ENCOUNTERED AND MAKE LOGICAL ADJUSTMENTS IN FIT, FORM, DIMINSION AND QUALITY. CHANGES MADE FROM THE PLANS WITHOUT THE CONSENT OF MARSHALL HOME DESIGN, LLC. AND ITS CONSULTANTS ARE UNAUTHORIZED. IT IS ALSO UNDERSTOOD THAT THE CONTACTOR WILL BE RESPONSIBLE FOR MEETING ALL APPLICABLE BUILDING CODES. IN THE EVENT

ADDITIONAL DETAIL OR GUIDANCE IS NEEDED BY THE CONTRACTOR OR HOMEOWNER FOR THE CONSTRUCTION OF ANY ASPECT OF THE PROJECT MARSHALL HOME DESIGN, LLC. OR A QUALIFIED ARCHITECT OR ENGINEER SHALL IMMEDIATELY BE RETAINED. FAILURE TO NOTIFY HD

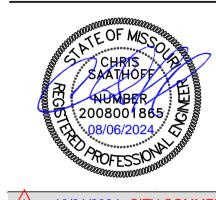
ENGINEERING & DESIGN, INC. OF THESE NEEDS, OR OF CHANGES TO THE PLANS, SHALL RELIEVEMARSHALL HOME DESIGN LLC., AND ITS CONSULTANTS OF ALL RESPONSIBILITIES OF THE CONSEQUENSES. STRUCTURAL DESIGN, SOILS TESTING, HVAC, MEP DESIGNS BY OTHERS

FRONT ELEVATION

1/4" = 1'-0"



THIS DOCUMENT CONTAINS



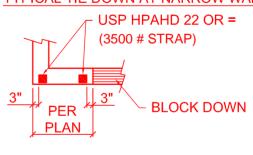
10/31/2024 CITY COMMENTS

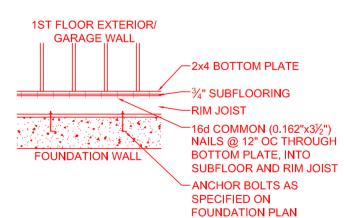
NOTES:

1. COLUMN AND PIER PAD SIZES SHOWN ARE FOR MAX.
COLUMN HEIGHT OF 10'-0" TALL.
2. COLUMN AND PIER PAD SIZES SHOWN ARE BASED ON
AN ASSUMED 1500 PSF. THIS IS THE CAPACITY REQUIRED

GENERAL NOTES:
-WINDOW SHALL HAVE FALL PROTECTION PER IRC 312.2.4
-HOUSE WILL BE PROVIDED WITH A "UFER" GROUND PER IRC SECTION **DETAIL SHEET S-1.0** -ALL HEADERS NOT LABELED SHALL BE MIN (2) #2-2X10 DFL REQUIREMENTS S-4.0 SHALL NOT BE CONSIDERED A COMPLETE SET OF

TYPICAL TIE DOWN AT NARROW WALL





FOUNDATION ANCHORING NOTES

MIN. 1/2" ANCHOR BOLTS SHALL BE INSTALLED @ 36"
O.C. MAX AND WITHIN 6"-12" FROM THE END OF EACH SECTION OF SILL PLATE ALONG ENTIRE PERIMETER OF FOUNDATION

DECK PIER SCHEDULE

54' - 8"

OPT. CONCRETE PATIO ABOVE

BATHROOM

SD

RETURN WALL 11' - 8" SEE DRAWING ON SHEET S-3.0

----BLOCK DOWN FOR O.H. DOOR --

OPENINGS AS REQ'D

9' - 4"

18" CANTILEVER ABOVE -

UNEXCAVATED

18" SQ. CONC. COL. W/ (4) #4

BARS VERT. ON 36"x36"x12" CONC. PAD W/ (6) #4 BARS E/W

SEE SHEET S-3.1 FOR

STRUCTURAL SLAB DETAILS

SEE DETAILS ON SHEET S-1.3 FOR ALL COVERED PATIO FRAMING NOTES & DETAILS

12' - 0"

5" CONC. SLAB W/ #4 BARS @ 12" O.C. E.W.

ON 1.5" CHAIRS TYP.

11' - 8"

<u>FUTURE</u>

BEDROOM

11' - 2"

18" SQ. CONC. COL. W/ (4) #4 BARS VERT. ON 48"x48"x12" CONC. PAD W/ (8) #4 BARS E/W

8"x4'-0" CONCRETE FOUNDATION WA

11' - 0"

CONC FTG W/ (2) #4 BARS CONT

W/ #4 BARS @ 24" OC E/W ON 16"x8"

1' - 4"

3' - 8"

8"x8'-0" CONCRETE FOUNDATION

AND (4) #4 BARS HORIZ ON 16"x8" CONC FTG W/ (2) #4 BARS CONT

15' - 8" 15' - 2"

W8x15 STL. BM.

MECH. / STORAGE

∠8" CANTILEVER ABOVE

(4) 8'-0" #4 BARS E/W ON 3 1/2" CHAIRS (TYP.)

-5" CONC SLAB W/ #4

BARS @ 12" OC E/W ON $1\frac{1}{2}$ " CHAIRS

CONSTRUCT PER PFG PORTAL FRAME DETAIL ON SHEET S-2.0

16' - 4"

23' - 0"

54' - 8"

14' - 4"

FIELD LOCATE

5' - 2"

SUMP PUMPI& PIT

5" CONC. SLAB W/ #4

BARS @ 12" O.C. E.W. ON 1.5" CHAIRS

15' - 0"

1 FOUNDATION PLAN

1/4" = 1'-0"

SEE SHEET S-3.1 FOR STRUCTURAL SLAB DETAILS

APPLIANCES SHALL BE HIGH EFFICIENCY AND DIRECTLY VENTED TO THE OUTSIDE

CANTILEVER F/P 24" ~

MIN. 6X6 TRTD/CDR POST ON 12" CONC PIER WITH USP PAU 66 BASE OR = (1177# MAX)

MIN. 6X6 TRTD/CDR POST ON 16" CONC PIER WITH USP PAU 66 BASE OR = (2050# MAX)

MIN. 6X6 TRTD/CDR POST ON 18" CONC PIER WITH USP PAU 66 BASE OR = (2649# MAX)

MIN. 6X6 TRTD/CDR POST ON 24" CONC PIER $\frac{D}{D}$ WITH USP PAU 66 BASE OR =(4710# MAX)

PIERS TO TERMINATE ON ORIGINAL SOIL OF 1500 PIERS TO TERMINATE AT A POINT 36" MINIMUM BELOW FINISH GRADE. POST ARE NOT TO EXCEED AN UNBRACED LENGTH
OF 12' WITHOUT CONTACTING HD ENGINEERING

COLUMN PAD SCHEDULE

A 3" SCH. 40 STL. COL. ON 30"x30"x12" CONC. PAD W/ (5) #4 BARS E.W. (9.4K MAX.)

B 3" SCH. 40 STL. COL. ON 36"x36"x12" CONC. PAD W/ (6) #4 BARS E.W. (13.5K MAX.)

3 1/2" SCH. 40 STL. COL. ON 42"x42"x14" CONC. PAD W/ (7) #4 BARS E.W. (18.4K MAX.)

D 3 1/2" SCH. 40 STL. COL. ON 48"x48"x16" CONC. PAD W/ (8) #4 BARS E.W. (24K MAX.)

B 3 1/2" SCH. 40 STL. COL. ON 54"x54"x16" CONC. PAD W/ (9) #4 BARS E.W. (30.4K MAX.)

3 1/2" SCH. 40 STL. COL. ON 60"x60"x18" CONC. PAD W/ (10) #4 BARS E.W. (37.5K MAX.)

BY AHJ, UNDERLINED GENERAL NOTES ON S-1.0 FOR MORE DETAILS. 3. ALL STEEL COLUMNS SHALL BE ISOLATED FROM SLABS

WITH APPROVED ISSOLATION DEVICE OR JOINT.

-OVERHEAD GARAGE DOORS MUST MEET DASMA REQUIREMENTS SEE -ALL HEADERS NOT LABELED SHALL BE MIN (2) #2-2X TO DEL
-DBL ALL JST UNDER ISLAND
-SOILS IN THIS AREA COMMONLY HAVE A VERY HIGH SHRINK SWELL
CAPACITY, OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY A
GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF FOUNDATIONS
-PROVIDE CARBON MONOXIDE AND SMOKE DETECTORS PER IRC -ANY PORTION OF THESE PRINTS ISSUED WITHOUT A MIN. OF S-1.0 -CONSTRUCTION DOCUMENTS
-ICE AND WATER SHIELD AS REQUIRED PER IRC

DATE:

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FOUNDATION PLAN

FIRST THE H 3904 S

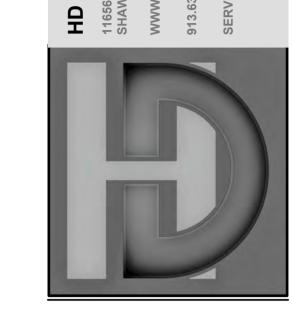
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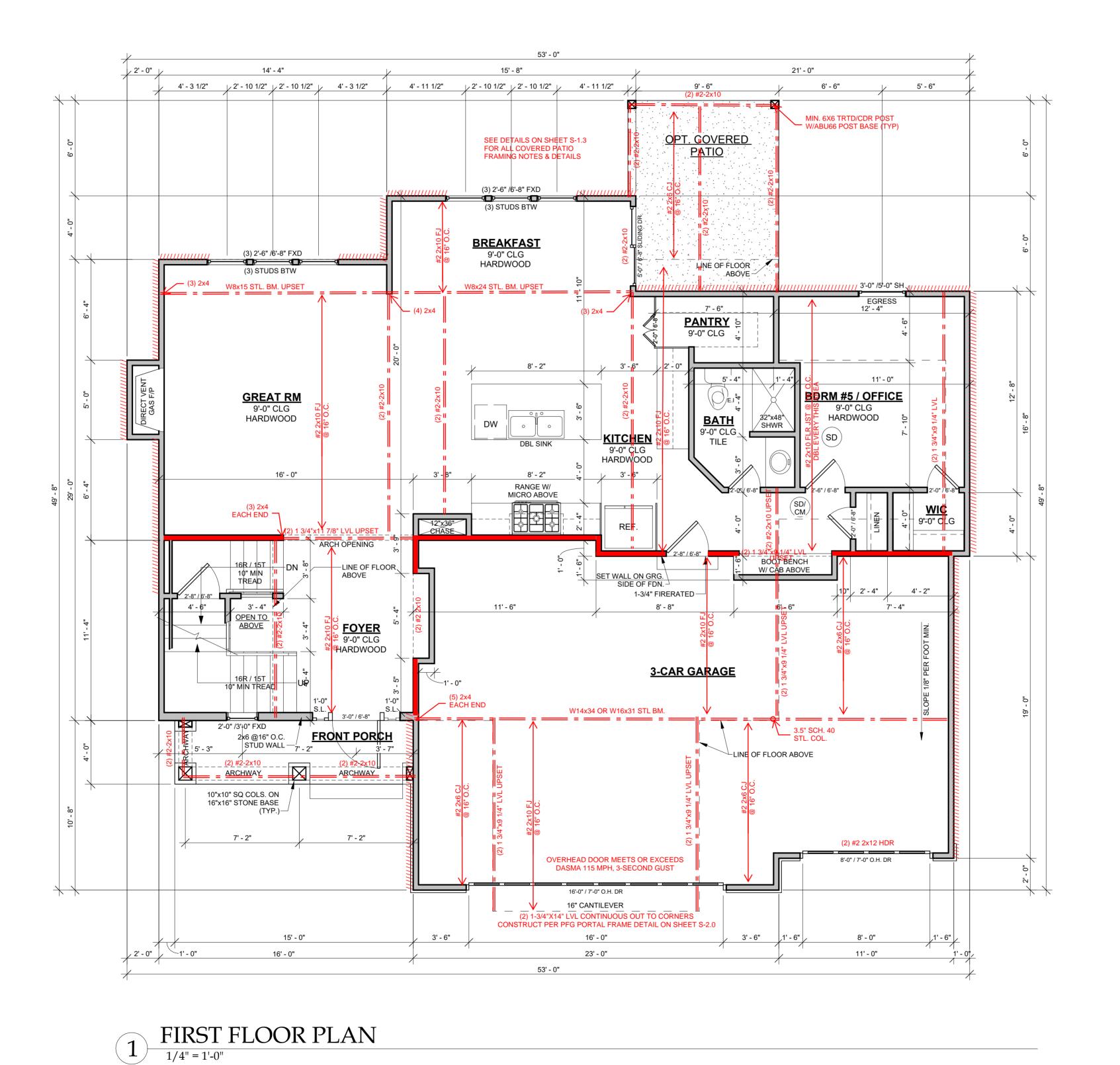
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FIRST FLOOR PLAN



- LOAD BEARING WALL

SD - SMOKE DETECTOR

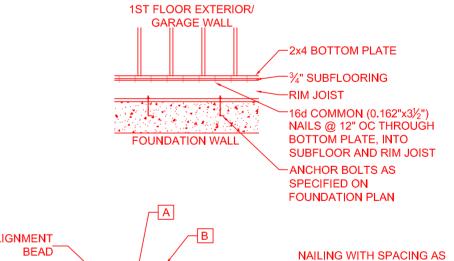
- CARBON MONOXIDE SENSOR

-WINDOW SHALL HAVE FALL PROTECTION PER IRC 312.2.4
-HOUSE WILL BE PROVIDED WITH A "UFER" GROUND PER IRC SECTION 3608.1.5 -OVERHEAD GARAGE DOORS MUST MEET DASMA REQUIREMENTS SEE DETAIL SHEET S-1.0
-ALL HEADERS NOT LABELED SHALL BE MIN (2) #2-2X10 DFL
-DBL ALL JST UNDER ISLAND
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TYPICAL TIE DOWN AT NARROW WALL $_{ au}$ USP HPAHD 22 OR = (4200# STRAP) PER 3" > BLOCK DOWN PLAN

ALL EXTERIOR WALLS SHALL BE SHEATHED PER ANY ONE OF THE FOLLOWING OPTIONS:
-7/16" APA-RATED PLYWOOD/OSB WITH 8d NAILS @ 6" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD .7/16" SHIPLAP PANEL SHEATHING (I.E. LP SMARTSIDE OR EQUIVALENT) WITH 8d NAILS @ 6" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD 3/8" SHIPLAP PANEL SHEATHING (I.E. LP SMARTSIDE OR EQUIVALENT) WITH 6d NAILS @ 4" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD

INTERIOR BRACED WALL LOCATIONS ONLY SHOWN WHEN REQUIRED BY ADDITIONAL BRACING SECTION OF CALCULATIONS ON SHEET S-2.0



SPECIFIED PER PLAN. FOR EXAMPLE, IF REQUIRED SPACING WALL_ STUD

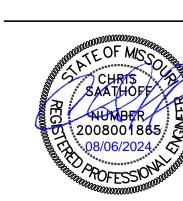
IS 4" O.C., BOTTOM LAP SHALL FIRST BE NAILED AT 4" O.C. (NAIL "A"), THEN FULL DEPTH SECTION OF OVERLAP PANEL SHALL BE NAILED @ 4" O.C. (NAIL "B")

3/8" APA REQUIRED NAILING PATTERN FOR SHIPLAP PANEL SHEATHING

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO

CONSTRUCT THIS DWELLING IN COMPLIANCE WITH ALL LOCAL BUILDING CODES AND REQUIREMENTS AS ADOPTED BY THE AUTHORITY HAVING JURISDICTION.

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GENERAL NOTES:
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- LOAD BEARING WALL

SD - SMOKE DETECTOR

CO - CARBON MONOXIDE SENSOR

BRACED WALLS:
SEE CALCULATIONS ON SHEET S-2.0, PER ASCE7-10
REQUIREMENTS AS ALLOWED BY IRC 2018 R301.2.1

9' - 0"

BEDROOM #2 8'-0" CLG

CARPET

4'-0" / 6'-8" (2) 2'-0"

CLOSET

WIC 8'-0' CLG CARPET

5' - 0"

LINE OF FLOOR

4' - 8"

10' - 6"

/ BELOW

BEDROOM #3

8'-0" CLG

(3) 1 3/4"x9 1/4" LVL UPSET

WIC 8'-0" CLG CARPET

6' - 2"

6' - 6"

VAULT

3' - 6"

9'-0" CLG CARPET

SEE DETIAL 11/S-1.2

EGRESS

^{'//}3'-0" /5¦-0" SH^{//}

13' - 0"

49' - 6"

3' - 4"

VAULT

4' - 9"

4' - 3"

10' - 6"

LINE OF FLOOR

BATHROOM

8' CLG 🗸 😓

/ BELOW

49' - 6"

DESK

15' - 8"

11' - 6"

9' - 9"

LIN. CAB

BATH

ŢILE**√**

MASTER

CLOSET 10'-0" CLG

CARPET

30"x24" ATTIC

ACCESS

HALLWAY 8' CLG

BATH 8' CLG

LINEN

LINE OF FLOOR / BELOW

LAUNDRY

8'-0" CLG 6" TILE/HDWD

3' - 11"

FOLDING TABLE

4' - 3"

21' - 0"

SECOND FLOOR PLAN

1/4" = 1'-0"

14' - 4"

v 3' - 5"

(2) 3'-0" /5'-0" SH

EGRESS EGRESS

MASTER

BEDROOM

9'-0" CLG CARPET

NO ROOF LOAD ON LIGHT TRAY—

16R / 15T 10" MIN TREAD

OPEN TO BELOW

7' - 2"

BOX VAULT

LINE OF FLOOR BELOW—

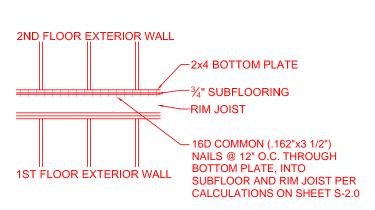
#2 2x6 @ 16" O.C. CONT WALL FRAMING FROM PLATE TO PLATE T

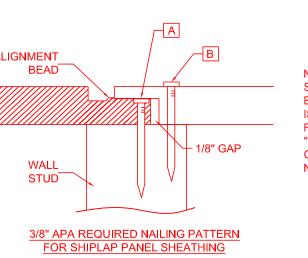
ALL EXTERIOR WALLS SHALL BE SHEATHED PER ANY ONE OF THE FOLLOWING OPTIONS:

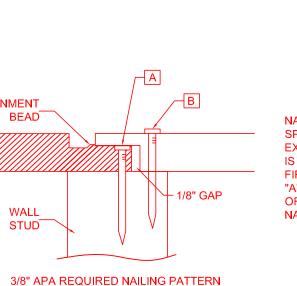
-7/16" APA-RATED PLYWOOD/OSB WITH 8d NAILS @ 6"
O.C. AT EDGES AND @ 12" O.C. IN THE FIELD

-7/16" SHPLAP PANEL SHEATHING (I.E. LP SMARTSIDE OR EQUIVALENT) WITH 8d NAILS @ 6" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD -3/8" SHIPLAP PANEL SHEATHING (I.E. LP SMARTSIDE OR EQUIVALENT) WITH 6d NAILS @ 4" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD

INTERIOR BRACED WALL LOCATIONS ONLY SHOWN WHEN REQUIRED BY ADDITIONAL BRACING SECTION OF CALCULATIONS ON SHEET S-2.0







NAILING WITH SPACING AS SPECIFIED PER PLAN. FOR EXAMPLE, IF REQUIRED SPACING IS 4" O.C., BOTTOM LAP SHALL FIRST BE NAILED AT 4" O.C. (NAIL "A"), THEN FULL DEPTH SECTION OF OVERLAP PANEL SHALL BE NAILED @ 4" O.C. (NAIL "B")

SECOND FLOOR PLAN

48058

DATE: 8/6/2024

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NO. ISSUE/REVISION Revision

Date

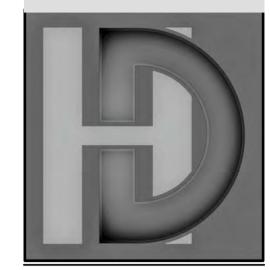
FIRST CHOICE
THE HEATHER III GR
3904 S.W. FLINTROCK

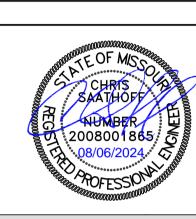
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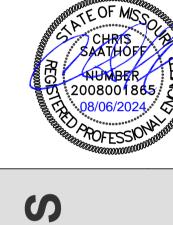
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HD ENGINEERING & 11656 W. 75TH STREET SHAWNEE, KS 66214







FIRST CHOICE CUSTOM HOMES
THE HEATHER III GR LOT 195 NAPA VALLEY
3904 S.W. FLINTROCK DR. LEES SUMMIT, MO

SPACING MAX HORIZONTAL CLEARSPAN

MAX HORIZONTAL CLEARSPAN

DEFLECTION = L/360 LIVE LOAD, L/240 TOTAL LOAD

ROOF DESIGNED FOR LIGHT ROOF COVERING 30PSF TOTAL LOAD [10PSF DL, 20PSF LL (SL)]

RAFTERS (DOUG-FIR, OR EQUAL): SEE SPAN CHARTS BELOW

RAFTERS SPACING
#2-2x6 @24" O.C.
#2-2x6 @16" O.C.
#2-2x8 @24" O.C.
#2-2x8 @16" O.C.
#2-2x8 @16" O.C.
#2-2x10 @24" O.C.
#2-2x10 @16" O.C.

NOTE: CODE MINIMUM L/240 DEFLECTION

NOTES

CRICKET #2 2x5 RAFTER AS REQ'D @ 16" O.C.

─WINDOW PAN

AS REQ'D

@ 16" O.C. 5 / 12 ►

─5 / 12

HIP SUPP SEE DETI.

PURLIN

PURLIN

1' - 0"

─5 / 12

1' - 0"—

ALL RIDGES, HIPS, AND VALLEYS NOT MARKED SHALL BE (1) NOMINAL SIZE LARGER THAN THE INTERSECTING RAFTERS

PURLINS ARE 2x6 MIN. PURLIN STRUTS ARE AT 4'-0" O.C. PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 DEGREE ANGLE WITH THE HORIZONTAL ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED

LENGTH OF 8'-0"
PURLINS STRUTS SHALL BE CONSTRUCTED IN A "T"
CONFIGURATION AND PER THE FOLLOWING CHART

PURLIN STRUT	MAX PURLIN STRUT LENGTH
(2) 2x4	8'-0"
(1) 2x4 & (1) 2x6	12'-0"
(1) 2x6 & (1) 2x8	20'-0"
(2) 2x6 & (1) 2x8	30'-0"
ONSULT ARCH./ENGR.	>30'-0"

SEE DETAILS 1, 5, 6, 7, 11, 12, 13, & 14 ON S-1.2 FOR ROOF FRAMING AND INSULATION OPTIONS

 - PURLIN
- LOAD BEARING WALL
 - LOAD BEARING BEAM/ GIRDER PER PLAN

	HD#:	4805	58
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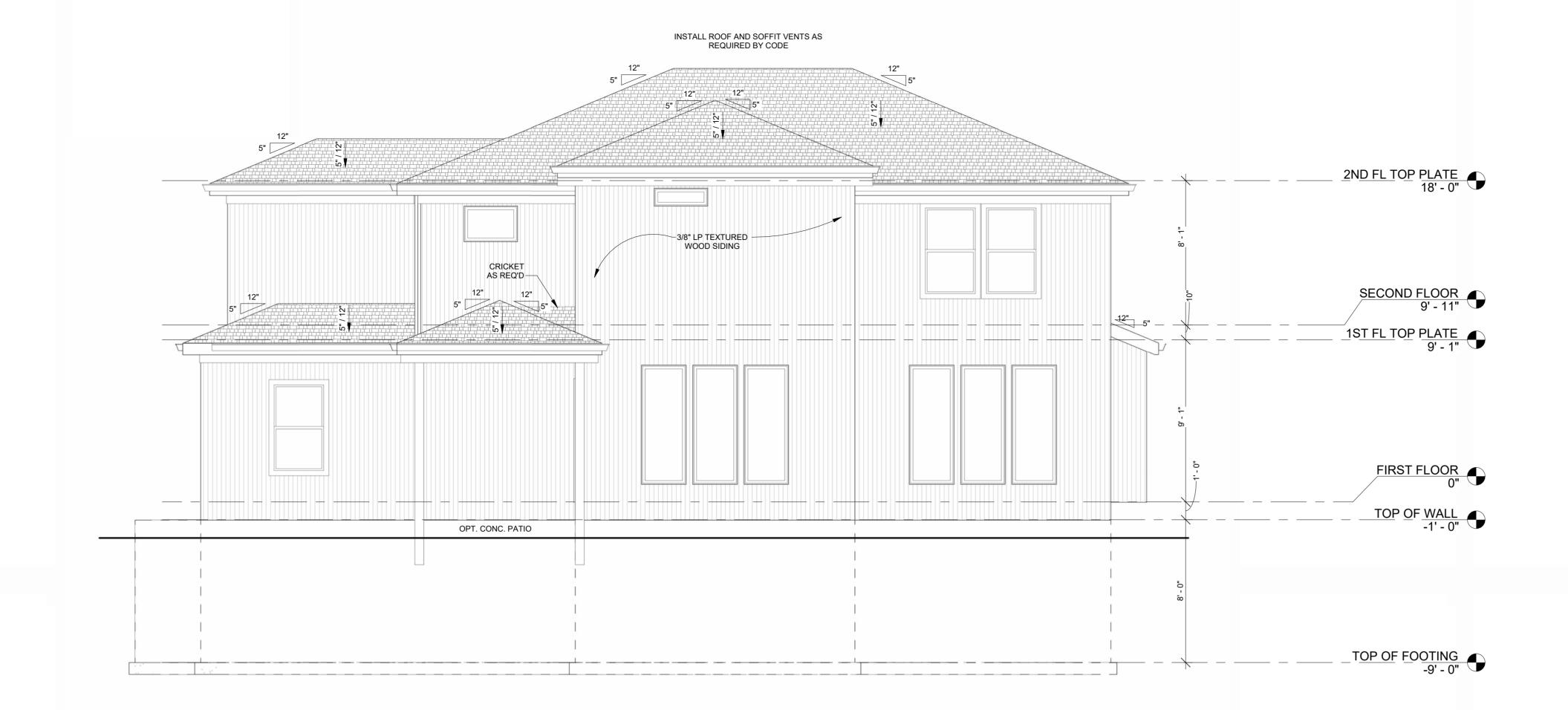
ISSUE/REVISION	Revision Date
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ROOF PLAN

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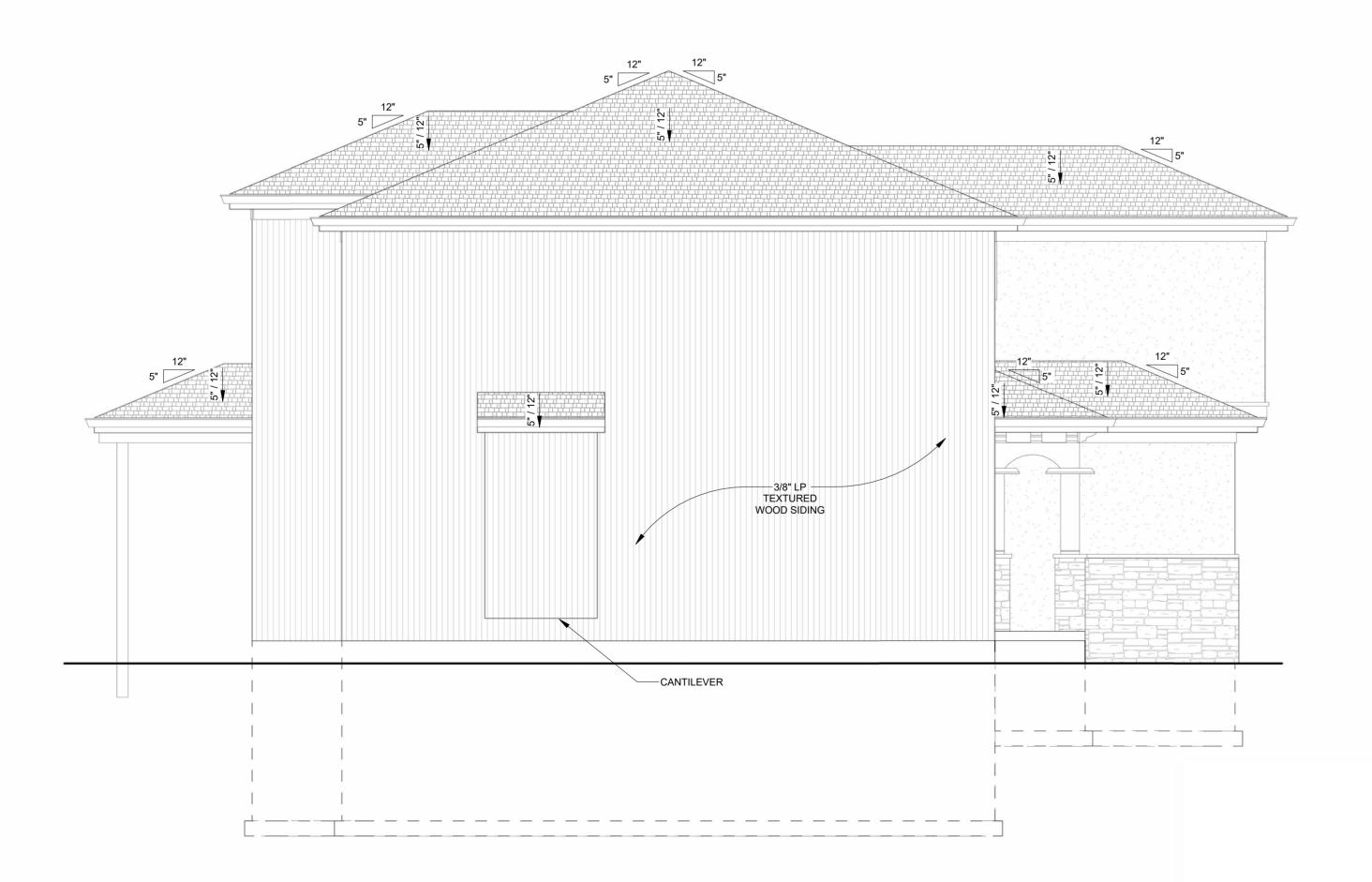
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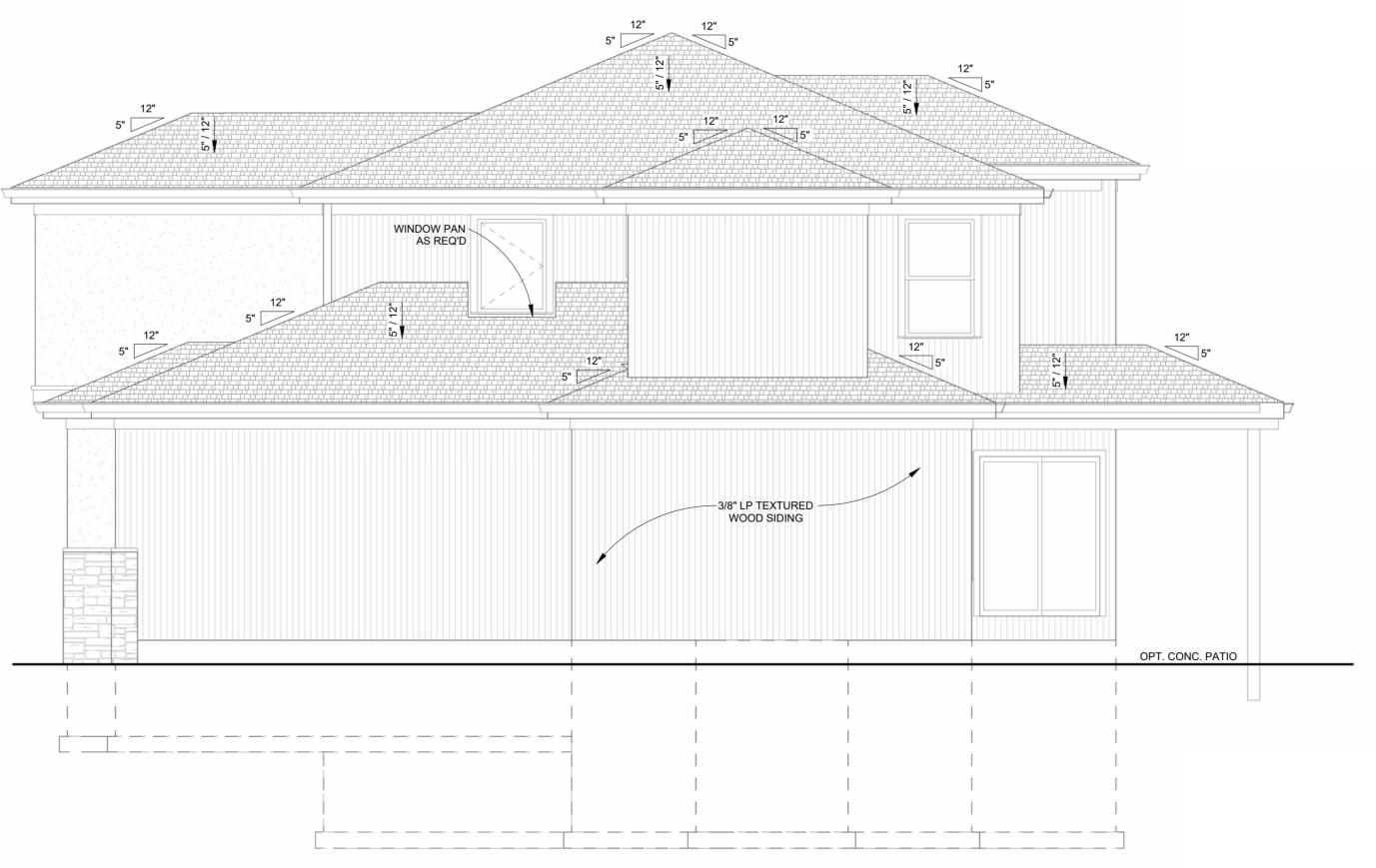
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REAR ELEVATION

1/4" = 1'-0"





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SIDE & REAR ELEVATIONS

8/6/2024

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RIGHT ELEVATION

1/4" = 1'-0"

ALLOWABLE LOADS FOR PNEUMATIC OR MECHANICALLY DRIVEN NAILS AND STAPLES

	NAIL GUN		PENETRATION	Al	LLOWABLE LO	OADS (POUND	S)	
FASTENER DESCRIPTION	NAILS/ WIRE	WIRE GAGE	REQUIRED INTO MAIN MEMBER FOR LATERAL	LATERAL	STRENGTH	WITHDRAWAL STRENGTH		
	DIAMETER		STRENGTH (INCHES)	SP	DF/L	SP	DF/L	
16 GA. STAPLE	.063	16	1	51		36	32	
15 GA. STAPLE	.072	15	1	64		42	37	
14 GA. STAPLE	.080	14	1	75		46	41	
6d COOLER NAIL	000	40	4	46		07	23	
6d SINKER NAIL	.092	13	1	40		27	23	
6d BOX NAIL								
6d CASING NAIL	.099	12-1/2	1-1/8	61	55	31	24	
7d COOLER NAIL								
6d COMMON NAIL								
8d COOLER NAIL								
8d SINKER NAIL	.113	11-1/2	1-1/4	79	72	35	28	
8d BOX NAIL								
8d CASING NAIL								
6d RING SHANK NAIL								
6d SCREW SHANK NAIL	120	11	1-3/8	89	81	41	32	
8d RING SHANK NAIL	.120	11						
8d SCREW SHANK NAIL								
10d COOLER NAIL								
10d SINKER NAIL	.128	10-1/2	1-1/2	89	81	36	31	
12d SHORT								
10d BOX NAILS				101	93	40	31	
12d BOX NAILS	.128	128 10-1/2	1-1/2					
10d CASING NAILS								
8d COMMON NAILS	.131	.131 10-1/4	1-1/2	106	97	41	32	
16d SHORT	.131	10-1/4	1-1/2	100	91	41	32	
12d SINKERS	.135	10	1-1/2	113	103	42	33	
16d BOX NAILS	.100	10	1-1/2	110	103	42	33	
10d RING SHANK NAILS								
10d SCREW SHANK NAILS		10	4.5/0	113	103	46	36	
12d RING SHANK NAILS	.100	.135 10 1-5/8	1 0/0					
12d SCREW SHANK NAILS								
10d COMMON NAILS								
12d COMMON NAILS								
16d SINKER NAILS	.148	9	1-5/8	128	118	46	36	
20d BOX NAILS								
30d BOX NAILS								
16d RING SHANK NAILS	.148	9	1-3/4	128	118	50	40	
16d SCREW SHANK NAILS			1 5/7	120	110	30	-+0	
16d COMMON NAILS	.162	8	1_3/4	154	141	50	40	
40d BOX NAILS	. 102	<u> </u>	1-3/4	1 J 1	141	50	40	
20d RING SHANK NAILS	.177	7	2-1/8	178	163	59	47	
20d SCREW SHANK NAILS		,	2-1/0	170	100	39	71	
20d SINKER NAILS	.177	7	2-1/8	178	163	54	43	
20d COMMON NAILS	.148	9	2-1/8	170	166	59	47	
30d SINKER NAILS	. 140	9	Z-1/O	170	100	29	4/	

MINIMUM SHEATHING REQUIREMENTS

30d SINKER NAILS

BUILDING COMPONENT	MATERIAL
ROOF SHEATHING	7/16" PLYWOOD
ROOF SHEATHING	1 x 4 #3 FURRING
FLOOR SHEATHING	3/4" T&G YELLOW PINE PLYWOOD
WALL COVERING	1/2" GYPSUM SHEATHING
CEILING COVERING	1/2" GYPSUM SHEATHING
EXTERIOR WALL	7/16" APA RATED SHEATHING
SHEATHING	RATED PANEL SIDING, RATED 16" O.C. 7/16" THICK

ALL SHEATHING MATERIALS TO BE APPLIED PERPENDICULAR TO JOISTS AND ENDS STAGGERED REFER TO TABLE R602.3(1) ON S-1.1 FOR FASTENING SCHEDULE

HIP/ VALLEY ALLOWABLE SPAN TABLE

TYPE	MAX. UNSUPPORTED SPAN				
TIPE	2x8	2x10	2x12	1 3/4"x9 1/2" LVL	1 3/4"x11 7/8" LVL
HIP RAFTER	11'-3"	13'-3"	15'-2"	15'-8"	18'-2"
VALLEY RAFTER	8'-11"	10'-6"	12'-0"	13'-2"	15'-3"

FRAME FASTENING SCHEDULE

BUILDING COMPONENT	FASTEN TO	FASTEN WITH	
	RIDGE / VALLEY / HIP	TOENAIL W/ (4) 16D, FACENAIL W/ (3) 16D	
RAFTERS	PLATE	TOENAIL W/ (3) 10D	
NAFIENS	LEDGER STRIPS SUPPORTING JOISTS OR RAFTERS	FACENAIL W/ (3) 16D	
	COLLAR TIE TO RAFTERS	FACENAIL W/ (3) 10D	
	TOP PLATE	TOENAIL W/ (3) 8D @ EACH END	
CEILING JOISTS	WHERE CLG JST RUN PARALLEL TO RAFTERS FAC	ENAIL TO RAFTERS W/ (3) 10D MINIMUM	
CEILING JOISTS	LAPS OVER PARTITIONS	FACENAIL W/ (3) 10D	
	BLOCKING BETWEEN JOISTS/RAFTERS TO TOP PLATE	TOENAIL W/ (3) 8D	
	BUILT-UP BEAMS, 2" LUMBER LAYERS, FACENAIL OPPOSITE SIDES, (2) @ EACH END PLUS	10D @ 32" O.C. STAGGERED, TOP & BOTTOM, OPPOSITE SIDES	
BEAMS	BUILT-UP BEAMS OF ENGINEERED LUMBER, FACE NAIL OPPOSITE SIDES	(2) ROWS @ 12" O.C.	
	BUILT-UP HEADER, TWO PIECES W/ A 1/2" SPACER	16D @ 16" O.C. ALONG EDGES	
	BUILT-UP HEADER, TWO PIECES W/ NO 1/2" SPACER	3" x 0.131" NAILS @ 12" O.C. ALONG EDGI	
	BEARING	TOENAIL W/ (2) 18D @ EACH END	
	RIM JOIST TO SILL OR TOP PLATE	TOENAIL W/ 8D COMMON OR 10D BOX @ 6" O.C.	
	JOIST TO SILL OR GIRDER	TOENAIL W/ (3) 8D	
	JOIST TO RIM JOIST	FACENAIL W/ (3) 16D	
	BRIDGING TO JOIST	TOENAIL W/ (2) 8D	
FLOOR JOISTS	I-JOIST TO BEARING PLATE	TOENAIL W/ (2) 8D - ONE INTO EACH SID AT LEAST 1 1/2" FROM THE END	
	RIM JOIST TO I-JOIST	FACENAIL W/ (2) 10D BOX - ONE INTO EACH FLANGE	
	SOLE PLATE TO LSL RIM BOARD	16D BOX @ 12" O.C.	
	SINGLE JOIST HANGERS*	10D FACENAILS AND TOENAILS	
	DOUBLE JOIST HANGERS*	16D FACENAILS AND TOENAILS	
	TOP AND SOLE PLATE TO STUD	END NAIL W/ (2) 16D	
	STUD TO SOLE AND TOP PLATE	TOENAIL W/ (4) 8D	
	DOUBLE TOP PLATES	FACENAIL W/ 16D @ 16" O.C.	
	DOUBLE TOP PLATE LAP SPLICE	FACENAIL W/ (8) 16D	
	TOP PLATE LAPS AND INTERSECTIONS	FACENAIL W/ (2) 16D	
	DOUBLE STUDS	FACENAIL W/ 16D @ 24" O.C.	
	BUILT-UP CORNER STUDS	FACENAIL W/ 16D - 2 ROWS @ 24" O.C.	
	STEEL "X" BRACING	FACENAIL W/ (2) 16D IN EACH TOP AND BOTTOM PLATE AND (1) 8D PER STUD	
	SOLE PLATE TO JOIST OR BLOCKING	FACENAIL W/ 16D @ 16" O.C.	
WALLS	SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL LINES, PERPENDICULAR TO FRAMING	FACENAIL W/ (3) 16D @ 16" O.C. ALONG BRACED WALL PANEL	
	TOP PLATE TO JOIST OR BLOCKING AT BRACED WALL LINES, PERPENDICULAR TO FRAMING	TOENAIL W/ 8D @ 6" O.C. ALONG BRACED WALL PANEL	
	SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL LINES, PARALLEL TO FRAMING, BLOCKING @ 16" O.C.	FACENAIL W/ (3) 16D @ 16" O.C. ALONG BRACED WALL PANEL AND AT EACH BLO	
	TOP PLATE TO JOIST OR BLOCKING AT BRACED WALL LINES, PARALLEL TO FRAMING, BLOCKING @ 16" O.C.	TOENAIL W/ 8D @ 6" O.C. ALONG BRACE WALL PANEL AND AT EACH BLOCK	
	NON-STRUCT. SIDING OVER STRUCT. SHEATHING	(1) 6D BOX IN EACH STUD	
	FIBER-CEMENT PLANK SIDING	(1) 6D GALVANIZED IN EACH STUD	
	WINDOW INSTALLATION NAILING	1 ³ / ₄ " - 2" ROOFING NAILS @ 12" O.C. MA	

NO JOIST HANGER NAILS ALLOWED FOR TOENAILS.

NO GUN NAILS OR SCREWS ALLOWED IN CONNECTORS. TOENAILS SHALL ALWAYS BE A FULL 3" OR 3.5" NAIL.

COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. 1/2" x 2" BOLTS SHOULD THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

DUCT SEALING METHOD, PER 2018 IRC W1103.3.2

N1103.2.2 (R403.2.2) SEALING (MANDATORY) DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF THIS CODE.

1. AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT 2. WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL

BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT. 3. CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURE LESS THAN 2 INCHES OF WATER COLUMN (500 Pa) PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS. DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:

1. POST CONSTRUCTION TEST: TOTAL LEAKAGE SHALL NOT BE LESS THAN OR EQUAL TO 4 CFM (113.3 L/MIN) PER 100FT² (9.29m²) OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. (25 Pa) ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST.

2. ROUGH-IN TEST: TOTAL AIR LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM (113.3 L/MIN) PER 100FT2 (9.29m²) OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. (25 Pa) ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL AIR LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM (85 L/MIN) PER 100FT2 (9.29m2) OF CONDITIONED FLOOR

EXCEPTION: THE TOTAL LEAKAGE IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN

PLANS SHALL COMPLY WITH THE 2018 INTERNATIONAL RESIDENTIAL CODE, ICC AS ADOPTED BY AHJ, AND ALL AMENDMENTS AS ADOPTED BY THE AHJ. IF ANY CHANGES MAKE ANY APPROPRIATE MODIFICATIONS TO THE PLANS

WHERE DISCREPANCIES EXIST BETWEEN THE STANDARD COMMENTS, NOTES FOR THE DESIGN PROFESSIONAL OR THE CODE, THE MOST RESTRICTIVE SHALL APPLY. THE CONTRACTUAL OBLIGATION OF THESE PLANS IS TO PROVIDE THE OWNER/BUILDER AND THE AHJ WITH A SET OF PLANS THAT MEET AHJ AND CODE REQUIREMENTS FOR A SINGLE SITE CONSTRUCTION PROJECT. UNLESS REQUESTED BY OUR CLIENT, CODE/AHJ MINIMUM DESIGNS WILL BE UTILIZED. ALSO, UNLESS REQUESTED BY THE OWNER, OUR FIRM CAN NOT AND WILL NOT BE AUTHORIZED TO VISIT THE SITE TO EVALUATE THE SITE OR ANY CONSTRUCTION FOR THIS PROJECT. IMPLEMENTATION OF ALTERNATES TO THE DESIGNS INCLUDING BUT NOT LIMITED TO PIER DESIGNS, FOUNDATION ALTERATIONS, OR ANY STRUCTURAL CHANGES NOT PROVIDED BY HD

ENGINEERING OR A PROFESSIONAL REFERRED BY HD ENGINEERING SHALL RELEASE HD ENGINEERING FROM ALL LIABILITY ASSOCIATED WITH THIS DESIGN. OUR FIRM HIGHLY RECOMMENDS THAT ANY SITE WITH GREATER THAN A 15% GRADE, ANY SITE WHERE A PREVIOUS STRUCTURE WAS LOCATED, OR ANY SITE WITH POTENTIAL FILL MATERIAL OR A POTENTIAL SOIL BEARING CAPACITY BELOW 1500 PSF SHOULD BE EVALUATED BY OUR FIRM OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACING FOOTINGS. THE ATTACHED PLANS HAVE BEEN DESIGNED WITH THE UNDERSTANDING THAT <u>OUR FIRM HAS NOT AND CAN NOT VISIT OR INSPECT THE SITE</u> WITHOUT WRITTEN CONSENT/REQUEST OF THE OWNER/BUILDER. DUE TO THIS FACT, OUR FIRM CAN ONLY DESIGN THE ATTACHED PLANS TO CERTAIN CODE REQUIREMENTS WHICH ARE DETAILED THROUGHOUT THE PLAN AND ATTACHED DETAIL SHEETS, IF THE OWNER DESIRES GREATER THAN CODE DESIGNS THAT REQUEST MUST BE MADE CLEARLY AND IN WRITING PRIOR TO ENGINEERING OF THE PLAN.

DUE TO THE WIDE VARIETY OF SOIL CONDITIONS, PLASTICITY INDEXES, AND SOIL BEARING CAPACITIES IN OUR AREA, OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY HD ENGINEERING OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF ANY "STANDARD" FOUNDATIONS.

THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION RESIDENTIAL FOUNDATION STANDARD IN LIEU OF ENGINEERING REPORT REQUIREMENTS BASED ON ACTUAL SITE CONDITIONS.

FOUNDATION WALLS SHALL BE DAMP-PROOFED PER IRC SECTION R406. PROVIDE A MINIMUM 4" PERFORATED DRAIN AROUND USABLE SPACE BELOW GRADE OR OTHER EQUIVALENT MATERIALS PER IRC SECTION 405.1. THE PIPE SHALL BE COVERED WITH NOT LESS THAN 6" OF WASHED GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT TO THE EXTERIOR BELOW THE FLOOR LEVEL OR TERMINATE

IN A MINIMUM 20 GALLON SUMP PIT. FOUNDATION DESIGN SHALL BE BASED ON A MINIMUM SOIL BEARING CAPACITY OF 1500 PSF. FOOTINGS SHALL BE A MINIMUM OF 16" WIDE AND 8" DEEP WITH (2) #4 BARS CONTINUOUS, LOCATED A MINIMUM OF 3" CLEAR FROM THE BOTTOM. FOOTINGS SHALL BE A

COLUMN PADS SHALL BE A MINIMUM OF 24"x24"x8" WITH (3) #4 BARS EACH WAY. FOUNDATION WALLS SHALL BE A MINIMUM OF 8" THICK WITH MINIMUM #4 BARS @ 24" O.C. HORIZONTAL AND VERTICAL WITH THE TOP BAR WITHIN 8" OF THE TOP OF THE WALL UNLESS NOTED OTHERWISE ON PLAN.

REINFORCEMENT SHALL LAP A MINIMUM OF 24". INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB.

INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING, SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE BY A SEPARATION

CONCRETE FLOOR SLABS ON GRADE SHALL BE A MINIMUM OF 4" THICK OVER A MINIMUM 4" BASE OF SAND, GRAVEL, OR CRUSHED STONE. BASEMENT SLABS SHALL HAVE A MINIMUM 6 MIL POLYETHYLENE OR APPROVED VAPOR RETARDER WITH JOINTS LAPPED NOT LESS THAN 6" AND SHALL BE PLACED BETWEEN THE FLOOR SLAB

FLOOR SLABS SUPPORTED BY FILL CONSISTING OF MORE THAN 24" OF GRANULAR FILL OR 8" OF EARTH SHALL BE REINFORCED PER A SEPARATE ENGINEERING DESIGN. BASEMENT FOUNDATION SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WITH MINIMUM 1/2" DIAMETER ANCHOR BOLTS EMBEDDED AT LEAST 7" INTO THE CONCRETE AND SPACED NOT MORE THAN 3' ON CENTER AND WITHIN 12" OF EACH END OF THE PLATE SECTION PER IRC SECTION R403.1.6.

FOUNDATION WINDOW WELLS FOR SECONDARY MEANS OF EGRESS SHALL PROVIDE A MINIMUM 3'x3' HORIZONTAL AREA. THE BASE OF ALL FOOTING EXCAVATIONS SHOULD BE FREE OF ALL WATER AND LOOSE MATERIAL PRIOR TO PLACING CONCRETE. CONCRETE SHOULD BE PLACED AS SOON AS POSSIBLE AFTER EXCAVATING SO THAT EXCESSIVE DRYING OR DISTURBANCE OF BEARING MATERIALS DOES NOT OCCUR. SHOULD THE MATERIALS AT BEARING LEVEL BECOME EXCESSIVELY DRY OR SATURATED, WE RECOMMEND THAT THE AFFECTED MATERIAL BE REMOVED PRIOR TO PLACING CONCRETE.

IT IS RECOMMENDED THAT ALL FOOTING EXCAVATIONS BE EVALUATED AND TESTED BY A GEOTECHNICAL ENGINEER IMMEDIATELY PRIOR TO PLACEMENT OF FOUNDATION CONCRETE. UNSUITABLE AREAS IDENTIFIED AT THIS TIME SHOULD BE CORRECTED. CORRECTIVE PROCEDURES WOULD BE DEPENDENT UPON CONDITIONS ENCOUNTERED AND MAY INCLUDE THE DEEPENING OF FOUNDATION ELEMENTS, OR THE UNDERCUTTING OF UNSUITABLE MATERIALS AND REPLACEMENT WITH ENGINEERED FILL.

STAIRWAYS SHALL PROVIDE A MAXIMUM 7 3/4" RISE AND A MINIMUM 10" RUN. PROVIDE MINIMUM 36" GUARDRAILS ON THE OPEN SIDES OF RAISED FLOORS, PORCHES, AND BALCONIES. PROVIDE MINIMUM 34" GUARDRAILS ON THE OPEN SIDES OF STAIRWAYS LOCATED MORE THAN 30" ABOVE THE FLOOR OR GRADE BELOW. GUARDRAIL ENCLOSURES SHALL HAVE INTERMEDIATE RAILS OR ORNAMENTAL PATTERNS THAT DO NOT ALLOW PASSAGE OF A 4" DIAMETER SPHERE

EACH STAIRWAY OF 3 OR MORE RISERS SHALL PROVIDE A CONTINUOUS HANDRAIL ON AT LEAST ONE SIDE BETWEEN 34" AND 38" ABOVE THE NOSING OF THE TREADS. HANDRAILS SHALL HAVE A CIRCULAR CROSS-SECTION OF 1 1/4" MINIMUM TO 2" MAXIMUM OR ANOTHER APPROVED GRASPABLE SHAPE PER IRC SECTION R311.7.8.5. PROVIDE A MINIMUM 6'-8" OF HEADROOM CLEARANCE IN STAIRWAYS.

ENCLOSED ACCESSIBLE SPACE UNDER STAIRWAYS SHALL HAVE WALLS AND THE UNDERSIDE OF THE STAIR AND LANDING PROTECTED WITH 1/2" GYPSUM BOARD ON THE

WINDERS SHALL PROVIDE A MINIMUM TREAD OF 6" AT ANY POINT WITHIN CLEAR WIDTH OF STAIRS. WINDER TREAD PROPORTION IS TO COMPLY WITH IRC SECTION R311.7.5.2.1.

GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPERABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 24" ARCH OF THE DOOR IN A CLOSED POSITION AND WHOSE BOTTOM EDGE IS WITHIN 60" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 60" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS AND WHIRLPOOLS, GLAZING IN FIXED OR OPERABLE PANELS EXCEEDING 9 S.F. AND WHOSE BOTTOM EDGE IS LESS THAN 18" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 36".

IN DWELLING UNITS WHERE THE OPENING OF AN OPERABLE WINDOW IS LOCATED MORE THAN 72" ABOVE THE FINISHED GRADE OR SURFACE BELOW, THE LOWEST PART OF THE CLEAR OPENING OF THE WINDOW SHALL BE A MINIMUM OF 24" ABOVE THE FINISHED FLOOR OF THE ROOM IN WHICH THE WINDOW IS LOCATED. OPERABLE SECTIONS OF WINDOWS SHALL NOT PERMIT OPENINGS THAT ALLOW PASSAGE OF A 4" DIAMETER SPHERE WHERE SUCH OPENINGS ARE LOCATED WITHIN 24" OF THE FINISHED FLOOR.

ALL LUMBER SIZES ARE FOR DOUGLAS FIR-LARCH UNLESS NOTED OTHERWISE

ALL HEADERS ARE TO BE A MINIMUM OF (2) #2 2x10'S UNLESS NOTED OTHERWISE. BLOCK CANTILEVERS, DOOR JAMBS, AND OVER BEAMS.

ALL HEADERS/BEAMS ARE TO BEAR ON A MINIMUM OF (2) 2x4 POSTS UNLESS NOTED OTHERWISE

INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING, SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE WHERE JOISTS RUN PARALLEL TO FOUNDATION WALLS, SOLID BLOCKING FOR A MINIMUM OF (2) JOIST SPACES SHALL BE PROVIDED AT A MAXIMUM OF 4' ON CENTER TO TRANSFER LATERAL LOADS ON THE WALL TO THE FLOOR DIAPHRAGM. THE BLOCKING SHALL BE SECURELY NAILED TO THE JOISTS AND FLOORING. NAIL JOISTS AND

BLOCKING TO SILL PLATE WITH (4) 10D NAILS. IF DUCTS ARE INSTALLED IN THE FIRST JOIST SPACE(S), NAIL 2x4'S FLAT AT 4' ON CENTER WITHIN THE JOIST SPACE(S) AND THEN PROVIDE SOLID BLOCKING, INSTALLED UPRIGHT, IN THE NEXT TWO JOIST SPACES. SECURE THE 2x4'S TO THE SILL PLATE WITH (4) 10D NAILS. ALL SILLS AND SLEEPERS SUPPORTED ON CONCRETE OR MASONRY AND FURRING ATTACHED TO CONCRETE OR MASONRY SHALL BE OF DECAY RESISTANT MATERIALS.

JOISTS UNDER BEARING PARTITIONS SHALL BE SIZED TO CARRY THE DESIGN LOAD IN ACCORDANCE WITH IRC SECTION R502.4. JOISTS FRAMING FROM OPPOSITE SIDES OVER BEARING SUPPORTS SHALL LAP A MINIMUM OF 3" AND SHALL BE NAILED TOGETHER WITH MINIMUM 10D FACE NAILS. JOISTS FRAMING INTO A WOOD GIRDER OR BEAM SHALL BE SUPPORTED BY APPROVED FRAMING ANCHORS OR ON MINIMUM 2"x2" LEDGER STRIPS.

HEADER AND TRIMMERS SHALL BE OF SUFFICIENT CROSS SECTION TO SUPPORT THE FLOOR FRAMING. TRIMMER JOISTS SHALL BE DOUBLED WHEN THE HEADER IS SUPPORTED MORE THAN 3' FROM THE TRIMMER JOIST BEARING. WHEN THE HEADER SPAN EXCEEDS 4', THE HEADER AND TRIMMER SHALL BE DOUBLED. JOISTS AT SUPPORTS SHALL BE SUPPORTED LATERALLY AT THE ENDS BY FULL-DEPTH SOLID BLOCKING NOT LESS THAN 2" IN NOMINAL THICKNESS OR BY ATTACHMENT

TO A HEADER, BAND, OR RIM JOIST OR TO AN ADJOINING STUD OR OTHERWISE PROVIDED WITH LATERAL SUPPORT TO PREVENT ROTATION. ALL WALL COVERINGS ARE TO COMPLY WITH IRC SECTIONS 702 AND 703.

ALL RAFTER / COLLAR TIES ARE TO COMPLY WITH IRC SECTION 802. ALL RAFTERS ARE TO HAVE 2x4 COLLAR TIES @ 48" O.C. IN THE UPPER 1/3 OF DISTANCE BETWEEN THE CEILING AND ROOF

BLOCKING BETWEEN JOISTS UNDER A PERPENDICULAR LOAD-BEARING WALL IS NOT REQUIRED.

THE BOTTOM OF ALL FLOOR ASSEMBLIES SHALL BE PROVIDED WITH A 1/2" GYPSUM WALLBOARD MEMBRANE (IF REQUIRED BY LOCAL CODE).

I-JOIST AND FLOOR TRUSS SYSTEMS SHALL BE FIRE PROTECTED PER IRC AS ADOPTED BY AHJ. STUDS SHALL BE CONTINUOUS FROM THE FLOOR TO THE ROOF / CEILING DIAPHRAGM PER IRC SECTION 602.3

CONCRETE SHALL BE AIR-ENTRAINED (5%-7%) WITH A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS, 3000 PSI FOR BASEMENT AND FOUNDATION WALLS, AND 3500 PSI FOR PORCHES, CARPORTS AND GARAGE FLOOR SLABS.

EMERGENCY EGRESS AND RESCUE NOTES:

PROVIDE ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 S.F. WITH A MINIMUM OPENABLE HEIGHT OF 24" AND WIDTH OF 21". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 44" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP.

PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA, AND ON EACH FLOOR INCLUDING BASEMENTS. ALARMS SHALL BE INTERCONNECTED IN SUCH A MANNER THAT THE ACTIVATION OF ONE ALARM WILL ACTIVATE ALL OF THE ALARMS IN THE DWELLING.

PROVIDE CARBON MONOXIDE ALARMS AS REQUIRED PER IRC. CARBON MONOXIDE ALARMS SHALL BE INSTALLED OUTSIDE OF EACH SEPARATE SLEEPING AREA. WHERE FUEL-BURNING APPLIANCES ARE LOCATED WITHIN A BEDROOM OR ITS ATTACHED BATHROOM, A CARBON MONOXIDE ALARM SHALL BE INSTALLED IN THE BEDROOM.

THE GARAGE FLOOR SHALL SLOPE TOWARDS THE GARAGE DOORWAYS OR SLOPE TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES DIRECTLY TO THE EXTERIOR ABOVE GRADE.

DOORS BETWEEN THE GARAGE AND DWELLING - MINIMUM 1 3/8" THICK SOLID WOOD, MINIMUM 1 3/8" THICK SOLID OR HONEY-COMB-CORE STEEL DOOR, OR 20-MINUTE FIRE-RATED EQUIPPED WITH A SELF-CLOSING DEVICE PER IRC SECTION R302.5.1.

GARAGE VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER

THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 5/8" GYPSUM BOARD APPLIED TO THE GARAGE SIDE. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE. THE FLOOR/CEILING ASSEMBLY SHALL BE PROTECTED WITH MINIMUM 5/8" TYPE X GYPSUM BOARD ON THE GARAGE CEILING. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE, COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH 5/8"

GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING ATTACHED WITH 1 3/4"x0.120" NAILS AT 7" ON CENTER STAGGERED WITH (7) 3 1/4"x0.120" NAILS THROUGH THE JAMB INTO THE HEADER, MINIMUM 2x8 HEADER FOR ATTACHMENT OF THE COUNTER BALANCE SYSTEM.

ANY ATTACHED GARAGE TO THE MAIN HOUSE SHALL BE PROVIDED WITH A SINGLE HEAT DETECTOR. THE HEAT DETECTOR SHALL BE HARDWIRED AND INTERCONNECTED WITH THE HOUSEHOLD SMOKE ALARM SYSTEM. THE HEAT DETECTOR SHALL BE LISTED FOR THE AMBIENT ENVIRONMENT AND INSTALLED PER

BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.2 OR THE 2018 IECC. (SEE S-6.0 FOR MORE DETAILS)

ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH 1/8" TO 1/4" OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/150th OF THE AREA OF SPACE VENTILATED. WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED, THE

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CHECKED BY: CLS

8/6/2024

NO.	ISSUE/REVISION	Revision Date

GENERAL NOTES

TABLE R602.3(1) FASTENING SCHEDULE

ITEM	DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER ^{a, b, c}	SPACING AND LOCATION					
	ROOF							
2	BLOCKING BETWEEN CEILING JOISTS OR RAFTERS TO TOP PLATE CEILING JOISTS TO PLATE	4-8D BOX (2 ¹ / ₂ " x 0.113"); OR 3-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 3-10D BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	TOE NAIL PER JOIST, TOE NAIL					
3	CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS (SEE SECTION R802.5.2 AND TABLE R802.5.2)	4-10D BOX (3" x 0.128"); OR 3-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 4-3" x 0.131" NAILS	FACE NAIL					
4	CEILING JOIST ATTACHED TO PARALLEL RAFTER (HEEL JOINT) (SEE SECTION R802.5.2 AND TABLE R802.5.2)	TABLE R802.5.2	FACE NAIL					
5	COLLAR TIE TO RAFTER, FACE NAIL OR 1 ¹ / ₄ " x 20 GA. RIDGE STRAP TO RAFTER	4-10D BOX (3" x 0.128"); OR 3-10D COMMON (3" x 0.148"); OR 4-3" x 0.131" NAILS	FACE NAIL EACH RAFTER					
6	RAFTER OR ROOF TRUSS TO PLATE	3-16D BOX NAILS (3 ¹ / ₂ " x 0.135"); OR 3-10D COMMON NAILS (3" x 0.148"); OR 4-10D BOX (3" x 0.128"); OR 4-3" x 0.131" NAILS	2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS ⁱ					
7	ROOF RAFTERS TO RIDGE, VALLEY OR HIP RAFTERS OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM	4-16D (3 ¹ / ₂ " x 0.135"); OR 3-10D COMMON (3" x 0.148"); OR 4-10D BOX (3" x 0.128"); OR 4-3" x 0.131" NAILS 3-16D BOX (3 ¹ / ₂ " x 0.135"); OR 2-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 3-10D BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	TOE NAIL					
		WALL						
	OTHER TO OTHER (MOT PRACED MAN L. DANIELO)	16D COMMON (3 ¹ / ₂ " x 0.162")	24" O.C. FACE NAIL					
8	STUD TO STUD (NOT BRACED WALL PANELS)	10D BOX (3" x 0.128"); OR 3" x 0.131" NAILS	16" O.C. FACE NAIL					
9	STUD TO STUD AND ABUTTING STUDS AT INTERSECTING	16D BOX (3 ¹ / ₂ " x 0.135"); OR 3" x 0.131" NAILS	12" O.C. FACE NAIL					
3	WALL CORNERS (AT BRACED WALL PANELS)	16D COMMON (3 ¹ / ₂ " x 0.162")	16" O.C. FACE NAIL					
10	BUILT-UP HEADER (2" TO 2" HEADER WITH 1/2" SPACER)	16D COMMON (3 ¹ / ₂ " x 0.162")	16" O.C. EACH EDGE FACE NAIL					
	OF MOLIN	16D BOX (3 ¹ / ₂ " x 0.135")	12" O.C. EACH EDGE FACE NAIL					
11	CONTINUOUS HEADER TO STUD	5-8D BOX (2 ¹ / ₂ " x 0.113"); OR 4-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 4-10D BOX (3" x 0.128")	TOE NAIL					
12	TOP PLATE TO TOP PLATE	16D COMMON (3 ¹ / ₂ " x 0.162")	16" O.C. FACE NAIL					
12	TOP PLATE TO TOP PLATE	10D BOX (3" x 0.128"); OR 3" x 0.131" NAILS	12" O.C. FACE NAIL					
13	DOUBLE TOP PLATE SPLICE	8-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 12-16D BOX (3 ¹ / ₂ " x 0.135"); OR 12-10D BOX (3" x 0.128"); OR 12-3" x 0.131" NAILS	FACE NAIL ON EACH SIDE OF END JOINT (MINIMUM 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT)					
1.4	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING	16D COMMON (3 ¹ / ₂ " x 0.162")	16" O.C. FACE NAIL					
14	(NOT AT BRACED WALL PANELS)	16D BOX (3 ¹ / ₂ " x 0.135"); OR 3" x 0.131" NAILS	12" O.C. FACE NAIL					
15	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING (AT BRACED WALL PANEL)	3-16D BOX (3 ¹ / ₂ " x 0.135"); OR 2-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 4-3" x 0.131" NAILS	3 EACH 16" O.C. FACE NAIL 2 EACH 16" O.C. FACE NAIL 4 EACH 16" O.C. FACE NAIL					
16	TOP OR BOTTOM PLATE TO STUD	4-8D BOX (2 ¹ / ₂ " x 0.113"); OR 3-16D BOX (3 ¹ / ₂ " x 0.135"); OR 4-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 4-10D BOX (3" x 0.128"); OR 4-3" x 0.131" NAILS	TOE NAIL					
		3-16D BOX (3 ½" x 0.135"); OR 2-16D COMMON (3 ½" x 0.162"); OR 3-10D BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	END NAIL					
17	TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	3-10D BOX (3" x 0.128"); OR 2-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 3-3" x 0.131" NAILS	FACE NAIL					
18	1" BRACE TO EACH STUD AND PLATE	3-8D BOX (2 1/2" x 0.113"); OR 2-8D COMMON (2 1/2" x 0.131"); OR 2-10D BOX (3" x 0.128"); OR 2 STAPLES 1 3/4"	FACE NAIL					
19	1" x 6" SHEATHING TO EACH BEARING	3-8D BOX (2 ¹ / ₂ " x 0.113"); OR 2-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 2-10D BOX (3" x 0.128"); OR 2 STAPLES, 1" CROWN, 16 GA., 1 ³ / ₄ " LONG	FACE NAIL					
20	1" x 8" AND WIDER SHEATHING TO EACH BEARING	3-8D BOX (2 ¹ / ₂ " x 0.113"); OR 3-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 3-10D BOX (3" x 0.128"); OR 3 STAPLES, 1" CROWN, 16 GA., 1 ³ / ₄ " LONG WIDER THAN 1" x 8"	FACE NAIL					
		4-8D BOX (2 ¹ / ₂ " x 0.113"); OR 3-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 3-10D BOX (3" x 0.128"); OR 4 STAPLES, 1" CROWN, 16 GA., 1 ³ / ₄ " LONG						
		FLOOR 4-8D BOX (2 1/2" x 0.113"); OR						
21	JOIST TO SILL, TOP PLATE OR GIRDER	3-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 3-10D BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	TOE NAIL					
22	RIM JOIST, BAND JOIST OR BLOCKING TO SILL OR TOP PLATE (ROOF APPLICATIONS ALSO)	8D BOX (2 ¹ / ₂ " x 0.113") 8D COMMON (2 ¹ / ₂ " x 0.131"); OR 10D BOX (3" x 0.128"); OR 3" x 0.131" NAILS	4" O.C. TOE NAIL 6" O.C. TOE NAIL					
23	1" x 6" SUBFLOOR OR LESS TO EACH JOIST	3-8D BOX (2 ¹ / ₂ " x 0.113"); OR 2-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 3-10D BOX (3" x 0.128"); OR 2 STAPLES, 1" CROWN, 16 GA., 1 ³ / ₄ " LONG	FACE NAIL					
		FLOOR						
24	2" SUBFLOOR TO JOIST OR GIRDER	3-16D BOX (3 ½" x 0.135"); OR 2-16D COMMON (3 ½" x 0.162")	BLIND AND FACE NAIL					
25	2" PLANKS (PLANK & BEAM-FLOOR AND ROOF)	3-16D BOX (3 ¹ / ₂ " x 0.135"); OR 2-16D COMMON (3 ¹ / ₂ " x 0.162") 3-16D COMMON (3 ¹ / ₂ " x 0.162"); OR	AT EACH BEARING, FACE NAIL					
26	BAND OR RIM JOIST TO JOIST	4-10D BOX (3" x 0.128"); OR 4-3" x 0.131" NAILS; OR 4-3" x 14 GA. STAPLES, ⁷ / ₁₆ " CROWN	END NAIL NAIL EACH LAYER AS FOLLOWS: 32" O.C.					
27	BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS	20D COMMON (4" x 0.192"); OR 10D BOX (3" x 0.128"); OR 3" x 0.131" NAILS AND: 2-20D COMMON (4" x 0.192"); OR	AT TOP AND BOTTOM AND STAGGERED. 24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES					
28	LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	3-10D BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS 4-16D BOX (3 1/2" x 0.135"); OR 3-16D COMMON (3 1/2" x 0.162"); OR 4-10D BOX (3" x 0.128"); OR	FACE NAIL AT ENDS AND AT EACH SPLICE AT EACH JOIST OR RAFTER, FACE NAIL					
29	BRIDGING OR BLOCKING TO JOIST	4-3" x 0.131" NAILS 2-10D BOX (3" x 0.128"); OR 2-8D COMMON	EACH END, TOE NAIL					
	= 25.4 mm. 1 foot = 304.8 mm. 1 mile per hour = 0.447 m/s: 1 ksi = 6.895 MPa.	(2 ¹ / ₂ " x 0.131" OR 2-3" x 0.131") NAILS	EAUTI END, TUE NAIL					

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1 ksi = 6.895 MPa.

- NAILS ARE SMOOTH-COMMON, BOX OR DEFORMED SHANKS EXCEPT WHERE OTHERWISE STATED. NAILS USED FOR FRAMING AND SHEATHING CONNECTIONS SHALL HAVE MINIMUM AVERAGE BENDING YIELD STRENGTHS AS SHOWN: 80 KSI FOR
- SHANK DIAMETER OF 0.192 INCH (20D COMMON NAIL), 90 KSI FOR SHANK DIAMETERS LARGER THAN 0.142 INCH BUT NOT LARGER THAN 0.177 INCH, AND 100 KSI FOR SHANK DIAMETERS OF 0.142 INCH OR LESS. STAPLES ARE 16 GAGE WIRE AND HAVE A MINIMUM 7/16-INCH ON DIAMETER CROWN WIDTH. NAILS SHALL BE SPACED AT NOT MORE THAN 6 INCHES ON CENTER AT ALL SUPPORTS WHERE SPANS ARE 48 INCHES OR GREATER. FOUR-FOOT BY 8-FOOT OR 4-FOOT BY 9-FOOT PANELS SHALL BE APPLIED VERTICALLY.
- SPACING OF FASTENERS NOT INCLUDED IN THIS TABLE SHALL BE BASED ON TABLE R602.3(2).
- FOR WOOD STRUCTURAL PANEL ROOF SHEATHING ATTACHED TO GABLE END ROOF FRAMING AND TO INTERMEDIATE SUPPORTS WITHIN 48 INCHES OF ROOF EDGES AND RIDGES, NAILS SHALL BE SPACED AT 6 INCHES ON CENTER WHERE THE
- ULTIMATE DESIGN WIND SPEED IS LESS THAN 130 MPH AND SHALL BE SPACED 4 INCHES ON CENTER WHERE THE ULTIMATE DESIGN WIND SPEED IS 130 MPH OR GREATER BUT LESS THAN 140 MPH.

 GYPSUM SHEATHING SHALL CONFORM TO ASTM C1396 AND SHALL BE INSTALLED IN ACCORDANCE WITH GA 253. FIBERBOARD SHEATHING SHALL CONFORM TO ASTM C208. SPACING OF FASTENERS ON FLOOR SHEATHING PANEL EDGES APPLIES TO PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING AND AT FLOOR PERIMETERS ONLY. SPACING OF FASTENERS ON ROOF SHEATHING PANEL EDGES APPLIES TO PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING. BLOCKING OF ROOF OR FLOOR SHEATHING PANEL EDGES PERPENDICULAR TO THE FRAMING MEMBERS NEED NOT BE PROVIDED EXCEPT AS
- REQUIRED BY OTHER PROVISIONS OF THIS CODE. FLOOR PERIMETER SHALL BE SUPPORTED BY FRAMING MEMBERS OR SOLID BLOCKING.

 WHERE A RAFTER IS FASTENED TO AN ADJACENT PARALLEL CEILING JOIST IN ACCORDANCE WITH THIS SCHEDULE, PROVIDE TWO TOE NAILS ON ONE SIDE OF THE RAFTER AND TOE NAILS FROM THE CEILING JOIST TO TOP PLATE IN ACCORDANCE WITH THIS SCHEDULE. THE TOE NAIL ON THE OPPOSITE SIDE OF THE RAFTER SHALL NOT BE REQUIRED. RSRS-01 IS A ROOF SHEATHING RING SHANK NAIL MEETING THE SPECIFICATIONS IN ASTM F1667.

CONTINUED TABLE R602.3(1) FASTENING SCHEDULE

			SPACING OF FASTENERS			
ITEM	DESCRIPTION OF BUILDING ELEMENTS NUMBER AND TYPE OF FASTENER ^{a, b, c}		EDGES (INCHES)h	INTERMEDIATE SUPPORTS ^{c, e} (INCHES)		
	WOOD STRUCTURAL PANELS, SUBFLOOR, ROOF AND INTERIOR [SEE TABLE R602.3(3) FOR WOOD STRUCT	WALL SHEATHING TO FRAMING AND PARTICLEBOARD W. FURAL PANEL <i>EXTERIOR</i> WALL SHEATHING TO WALL FRA	ALL SHEATHING TO			
30	³ / ₈ " - ¹ / ₂ "	6D COMMON (2" x 0.113") NAIL (SUBFLOOR, WALL) ⁱ 8D COMMON (2 ¹ / ₂ " x 0.131") NAIL (ROOF); OR RSRS-01 (2 ³ / ₈ " x 0.113") NAIL (ROOF)	6	12 ^f		
31	¹⁹ / ₃₂ " - 1"	8D COMMON NAIL (2 ¹ / ₂ " x 0.131"); OR RSRS-01 (2 ³ / ₈ " x 0.113") NAIL (ROOF) ^j	6	12 ^f		
32	1 ¹ /8" - 1 ¹ /4"	10D COMMON (3" x 0.148") NAIL; OR 8D (2 ½" x 0.131") DEFORMED NAIL	6	12		
	OTHER WALL SHEATHING ⁹					
33	1/2" STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	1 1 / ₂ " GALVANIZED ROOFING NAIL, 7 / ₁₆ " HEAD DIAMETER, OR 1 1 / ₄ " LONG 16 GA. STAPLE WITH 7 / ₁₆ " OR 1" CROWN	3	6		
34	²⁵ / ₃₂ " STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	1 3 / ₄ " GALVANIZED ROOFING NAIL, 7 / ₁₆ " HEAD DIAMETER, OR 1 1 / ₂ " LONG 16 GA. STAPLE WITH 7 / ₁₆ " OR 1" CROWN	3	6		
35	¹ / ₂ " GYPSUM SHEATHING ^d	1 1/2" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1 1/2" LONG; 1 1/4" SCREWS, TYPE W OR S	7	7		
36	5/8" GYPSUM SHEATHING ^d	1 ³ / ₄ " GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1 ⁵ / ₈ " LONG; 1 ⁵ / ₈ " SCREWS, TYPE W OR S	7	7		
	WOOD STRUCTURAL PANELS, COMBINATION SUBFLOOR UNDERLAYMENT TO FRAMING					
37	³ / ₄ " AND LESS	6D DEFORMED (2" x 0.120") NAIL; OR 8D COMMON (2 1/2" x 0.131") NAIL	6	12		
38	⁷ / ₈ " - 1"	8D COMMON (2 ¹ / ₂ " x 0.131") NAIL; OR 8D DEFORMED (2 ¹ / ₂ " x 0.120") NAIL	6	12		
39	1 1/8" - 1 1/4"	10D COMMON (3" x 0.148") NAIL; OR 8D DEFORMED (2 1/2" x 0.120") NAIL	6	12		

TABLE R602.3(2) ALTERNATE ATTACHMENTS TO TABLE R602.3(1)

NOMINAL MATERIAL		SPACING° OF FASTENERS		
THICKNESS (INCHES)	DESCRIPTION ^{a, b} OF FASTENER AND LENGTH (INCHES)	EDGES (INCHES) INTERMEDIATE SUPPORTS (INCHES		
WOOD STRUCT	JRAL PANELS SUBFLOOR, ROOF® AND WALL SHEATHING TO FRAMING AND PART	ICLEBOARD WALL SH	EATHING TO FRAMING ^f	
	STAPLE 15 GA. 1 ³ / ₄	4	8	
UP TO ¹ / ₂	0.097 - 0.099 NAIL 2 ¹ / ₄	3	6	
	STAPLE 16 GA. 1 ³ / ₄	3	6	
	0.113 NAIL 2	3	6	
¹⁹ / ₃₂ AND ⁵ / ₈	STAPLE 15 AND 16 GA. 2	4	8	
	0.097 - 0.099 NAIL 2 ¹ / ₄	4	8	
	STAPLE 14 GA. 2	4	8	
227 AND 27	STAPLE 15 GA. 1 3/4	3	6	
²³ / ₃₂ AND ³ / ₄	0.097 - 0.099 NAIL 2 ¹ / ₄	4	8	
	STAPLE 16 GA. 2	4	8	
	STAPLE 14 GA. 2 1/4	4	8	
	0.113 NAIL 2 ¹ / ₄	3	6	
1	STAPLE 15 GA. 2 ¹ / ₄	4	8	
	0.097 - 0.099 NAIL 2 ¹ / ₂	4	8	
NOMINAL MATERIAL		SPA	ACING° OF FASTENERS	
THICKNESS (INCHES)	DESCRIPTION ^{a, b} OF FASTENER AND LENGTH (INCHES)	EDGES (INCHES)	BODY OF PANEL ^d (INCHES)	
	FLOOR UNDERLAYMENT; PLYWOOD-HARDBOARD-PARTICLEBOARD	FIBER-CEMENT ^h		
	FIBER-CEMENT			
	3D, CORROSION-RESISTANT, RING SHANK NAILS (FINISHED FLOORING OTHER THAN TILE)	3	6	
	STAPLE 18 GA., ⁷ / ₈ LONG, ³ / ₄ CROWN (FINISHED FLOORING OTHER THAN TILE)	3	6	
1/4	1 1/4 LONG x .121 SHANK x .375 HEAD DIAMETER CORROSION-RESISTANT (GALVANIZED OR STAINLESS STEEL) ROOFING NAILS (FOR TILE FINISH)	8	8	
	1 1/4 LONG, NO. 8 x .375 HEAD DIAMETER, RIBBED WAFER-HEAD SCREWS (FOR TILE FINISH)	8	8	
	PLYWOOD			
	1 ¹ / ₄ RING OR SCREW SHANK NAIL-MINIMUM 12 ¹ / ₂ GA. (0.099") SHANK DIAMETER	3	6	
¹ / ₄ AND ⁵ / ₁₆	STAPLE 18 GA., ⁷ / ₈ , ³ / ₁₆ CROWN WIDTH	2	5	
¹¹ / ₃₂ , ³ / ₈ , ¹⁵ / ₃₂ AND ¹ / ₂	1 ¹ / ₄ RING OR SCREW SHANK NAIL-MINIMUM 12 ¹ / ₂ GA. (0.099") SHANK DIAMETER	6	8e	
	1 1/2 RING OR SCREW SHANK NAIL-MINIMUM	6	8	
¹⁹ / ₃₂ , ⁵ / ₈ , ²³ / ₃₂ AND ³ / ₄	12 ¹ / ₂ GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 ¹ / ₂	6	8	
	HARDBOARD ^f			
	1 ¹ / ₂ LONG RING-GROOVED UNDERLAYMENT NAIL	6	6	
0.200	4D CEMENT-COATED SINKER NAIL	6	6	
	STAPLE 18 GA., ⁷ / ₈ LONG (PLASTIC COATED)	3	6	
	PARTICLEBOARD	1		
	4D RING-GROOVED UNDERLAYMENT NAIL	3	6	
1/4	STAPLE 18 GA., ⁷ / ₈ LONG, ³ / ₁₆ CROWN	3	6	
	6D RING-GROOVED UNDERLAYMENT NAIL	6	10	
³ / ₈	STAPLE 16 GA., 1 ¹ / ₈ LONG, ³ / ₈ CROWN	3	6	
	6D RING-GROOVED UNDERLAYMENT NAIL			
¹ / _{2,} ⁵ / ₈				
¹ / ₂ , ⁵ / ₈	6D RING-GROOVED UNDERLAYMENT NAIL STAPLE 16 GA., 1 ⁵ / ₈ LONG, ³ / ₈ CROWN	6	10 6	

- NAIL IS A GENERAL DESCRIPTION AND SHALL BE PERMITTED TO BE T-HEAD, MODIFIED ROUND HEAD OR ROUND HEAD. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16-INCH ON DIAMETER EXCEPT AS NOTED.
- NAILS OR STAPLES SHALL BE SPACED AT NOT MORE THAN 6 INCHES ON CENTER AT ALL SUPPORTS WHERE SPANS ARE 48 INCHES OR GREATER. NAILS OR STAPLES SHALL BE SPACED AT NOT MORE THAN 12 INCHES ON CENTER AT INTERMEDIATE
- FASTENERS SHALL BE PLACED IN A GRID PATTERN THROUGHOUT THE BODY OF THE PANEL.
 FOR 5-PLY PANELS, INTERMEDIATE NAILS SHALL BE SPACED NOT MORE THAN 12 INCHES ON CENTER EACH WAY.
- HARDBOARD UNDERLAYMENT SHALL CONFORM TO CPA/ANSI A135.4

 SPECIFIED ALTERNATE ATTACHMENTS FOR ROOF SHEATHING SHALL BE PERMITTED WHERE THE ULTIMATE DESIGN WIND SPEED IS LESS THAN 130 MPH. FASTENERS ATTACHING WOOD STRUCTURAL PANEL ROOF SHEATHING TO GABLE END WALL
- FRAMING SHALL BE INSTALLED USING THE SPACING LISTED FOR PANEL EDGES.
 FIBER-CEMENT UNDERLAYMENT SHALL CONFORM TO ASTM C1288 OR ISO 8336, CATEGORY C.

DESIGN LOADS (PSF)

THE DWELLING SHALL COMPLY WITH THE FOLLOWING LOAD CONDITIONS

AREA	MIN. DEAD LOAD	MIN. LIVE LOAD
EXTERIOR BALCONIES	10	60
DECKS, STAIRS	10	40
CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE 3:12 OR LESS	10	10
CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE OVER 3:12	10	10
CEILING JOISTS / ATTICS WITH STORAGE - DOOR PULL DOWN LADDER ACCESS	10	20
ROOMS: NON-SLEEPING	10	40
ROOMS: SLEEPING	10	30
ROOF: LIGHT ROOF COVERING	10	20
ROOF: HEAVY ROOF COVERING / CONCRETE / TILE / SLATE	20	20
GUARDRAILS, HANDRAILS	200# LL 1	NORMAL

HEAVY ROOF COVERING MATERIAL (TILE, CONCRETE, SLATE, ETC.) SHALL NOT BE USED UNLESS 20 PSF DEAD LOAD AND HEAVY ROOF IS NOTED ON THE ROOF PLAN. IF HEAVY ROOFING IS TO BE USED AND IS NOT NOTED ON THE ROOF PLAN, NOTIFY ENGINEER PRIOR TO ANY CONSTRUCTION, INCLUDING FOUNDATION AND SITE WORK. IF THE PLAN HAS BEEN DESIGNED FOR HEAVY

COLUMN SCHEDULE

BASED ON FOOTING SIZE (ASSUME 1500 PSF SOIL)

PAD SIZE	ZE REINFORCEMENT COL. MIN.		COL. TYPE	MAX. LOAD
24"x24"x12"	(4) #4 BARS E/W	3"	SCH40	6K
30"x30"x12"	(5) #4 BARS E/W	3"	SCH40	9.4K
36"x36"x12"	(6) #4 BARS E/W	3"	SCH40	13.5K
42"x42"x14"	(7) #4 BARS E/W	3 1/2"	SCH40	18.4K
48"x48"x16"	(8) #4 BARS E/W	3 1/2"	SCH40	24.0K
54"x54"x16"	(9) #4 BARS E/W	3 1/2"	SCH40	30.4K
60"x60"x18"	(10) #4 BARS E/W	3 1/2"	SCH40	37.5K

COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A SHOULD THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE

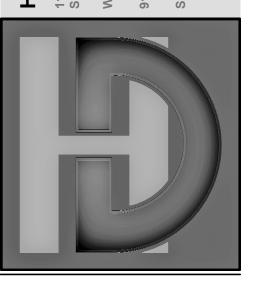
ENGINEERED LUMBER

MIN. DESIGN REQUIREMENTS

	F _b (psi)	E (psi)	F _v (psi)
LVL	2600	1.8x10	285
GLULAM	2400	1.8x10	190
PARALAM	2600	2.0x10	290

<u>BUILDER'S PLANS:</u> THE TERM "BUILDER'S PLANS" REFERS TO A CERTAIN LEVEL OF DEVELOPMENT OF THE DRAWINGS. AS THE NAME IMPLIES, THESE PLANS REQUIRE THAT THE CONTRACTOR POSSESSES COMPETENCE IN RESIDENTIAL CONSTRUCTION AND A THOROUGH UNDERSTANDING OF THE INTERNATIONAL RESIDENTIAL CODE (IRC). THE CONTRACTOR WARRANTS TO HD ENGINEERING & DESIGN THAT THEY POSSESSES THE PARTICULAR COMPETENCE AND SKILL IN CONSTRUCTION NECESSARY TO BUILD THIS PROJECT WITHOUT FULL ENGINEERING AND DESIGN SERVICES, AND FOR THAT REASON THE CONTRACTOR OR HOME OWNER HAS RESTRICTED THE SCOPE OF PROFESSIONAL SERVICES. THE CONSTRUCTION DOCUMENTS PROVIDED BY THE LIMITED SERVICES SHALL BE TERMED "BUILDER'S PLANS" IN RECOGNITION OF THE CONTRACTOR'S SOPHISTICATION. ALTHOUGH HD ENGINEERING & DESIGN HAVE PERFORMED THEIR SERVICES WITH DUE CARE AND DILIGENCE, WE CANNOT GUARANTEE PERFECTION. ANY AMBIGUITY OR DISCREPANCY DISCOVERED BY THE USE OF THESE PLANS SHALL BE REPORTED IMMEDIATELY TO HD ENGINEERING. CONSTRUCTION MAY REQUIRE THAT THE CONTRACTOR ADAPT THE "BUILDER'S PLANS" TO THE FIELD CONDITIONS ENCOUNTERED AND MAKE LOGICAL ADJUSTMENTS IN FIT, FORM, DIMENSION AND QUANTITY. CHANGES MADE FROM THE PLANS WITHOUT THE CONSENT OF HD ENGINEERING & DESIGN ARE UNAUTHORIZED. IT IS ALSO UNDERSTOOD THAT THE CONTRACTOR WILL BE RESPONSIBLE FOR MEETING ALL APPLICABLE BUILDING CODES INCLUDING BUT NOT LIMITED TO MECHANICAL, ELECTRICAL, AND PLUMBING CODE REQUIREMENTS (WHICH IS EXCLUDED FROM THESE PLANS). IN THE EVENT ADDITIONAL DETAIL OR GUIDANCE IS NEEDED BY THE CONTRACTOR OR HOMEOWNER FOR CONSTRUCTION OF ANY ASPECT OF THE PROJECT, HD ENGINEERING & DESIGN OR A QUALIFIED ENGINEER SHALL IMMEDIATELY BE RETAINED. FAILURE TO NOTIFY US OF THESE NEEDS OR OF CHANGES TO THE PLANS SHALL RELIEVE HD ENGINEERING & DESIGN OF ALL RESPONSIBILITIES OF THE CONSEQUENCES.

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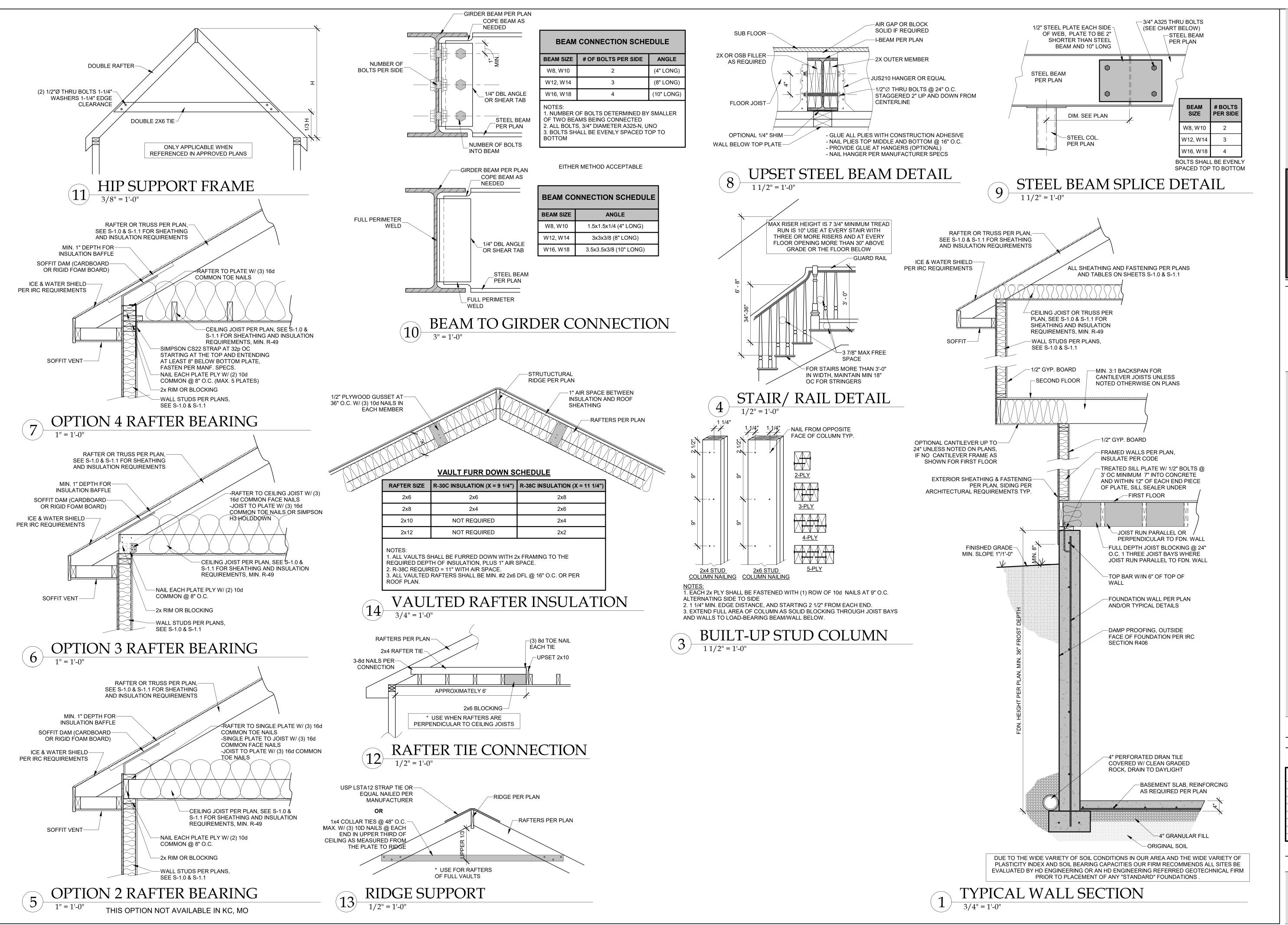


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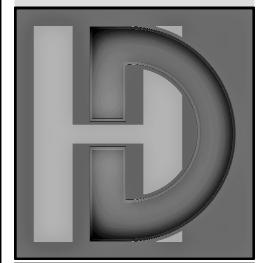
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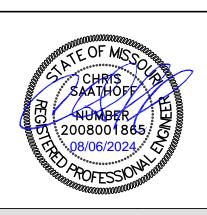
GENERAL NOTES



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WHEE, KS 66214
W.HDENGINEERS.COM
631.2222





ST CHOICE CUSTOM HON HE HEATHER III GR LOT 195 NAPA VALLE 104 S.W. FLINTROCK DR. LEES SUMMIT, M

D#: 48058

DATE: 8/6/2024

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FRAMING SECTIONS

S-1.2

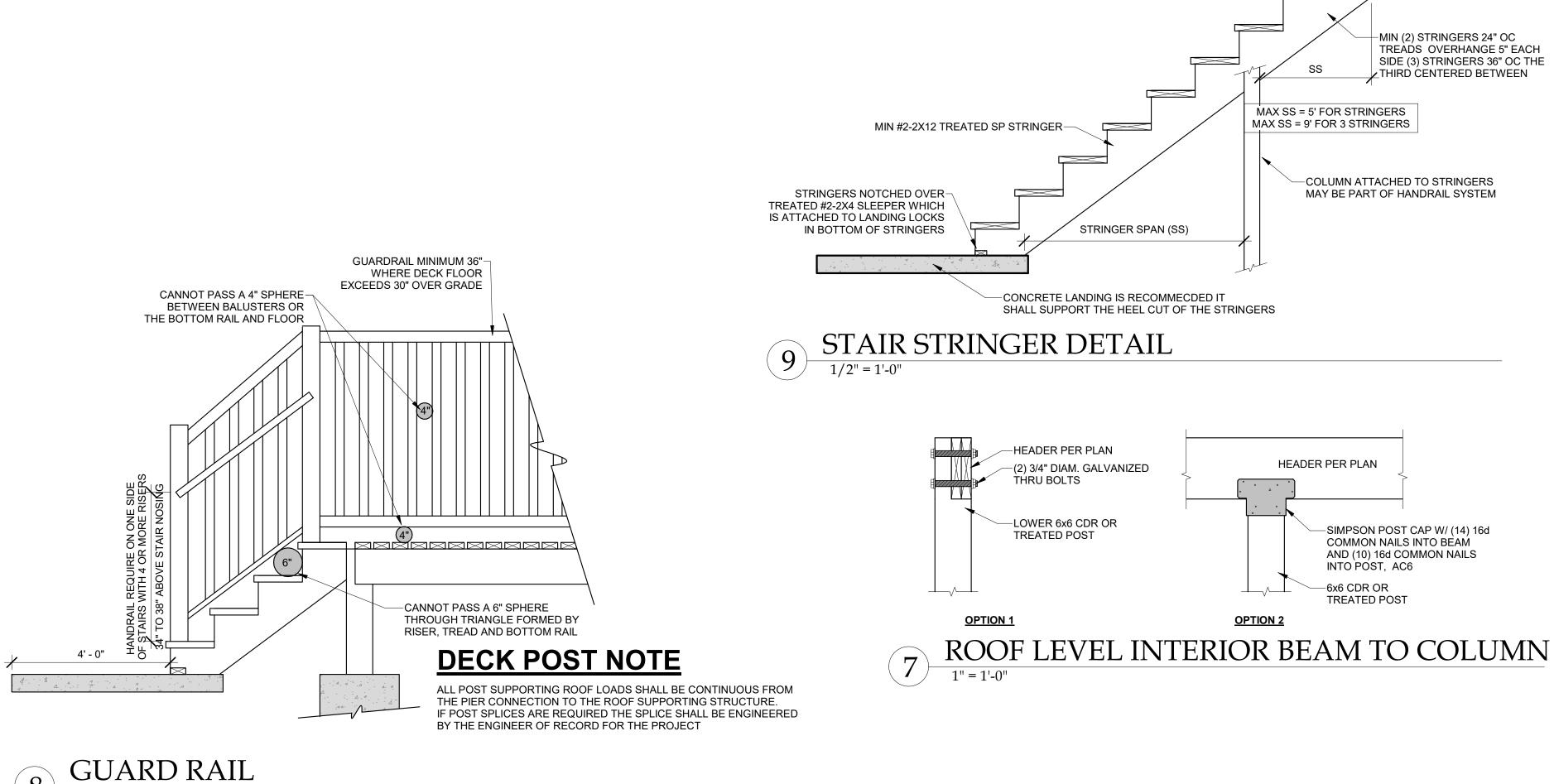


TABLE IRC2018 R507.9.1.3(1) DECK LEDGER CONNECTION TO BAND JOIST a.b (DECK LIVE LOAD = 40 PSF, DECK HEAD LOAD = 10 PSF, SNOW LOAD < 40 PSF)

TOP OF EACH STRINGER IS-TOE-NAILED (TYP) AND

ON ONE SIDE, OR SLOPED

SUPPORTED BY SIMPSON LS70

GUSSET ANGLE OR EQUIVALENT

JOIST SPAN	6' AND LESS	6'-1" TO 8'	8'-1" TO 10'	10'-1" TO 12'	12'-1" TO 14'	14'-1" TO 16'	16'-1" TO 18'
CONNECTION DETAILS			ON-CENTE	R SPACING OF F	ASTENERS d, e		
1/2" LAG SCREW WITH 15/32" MAX. SHEATHING ^{c,d}	30	23	18	15	13	11	10
1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING ^d	36	36	34	29	24	21	19
1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING & 1/2" STACKED WASHERS ^e	36	36	29	24	21	18	16

For SI: 1 inch = 25.4mm, 1 foot = 304.8mm, 1 pound per square foot = 0.0479 kPa

a. Ledges shall be flashed in accordance with Section R703.4 to prevent water from contacting the house band joist.

b. Snow load shall not be assumed to act concurrently with live load. c. The tip of the lag screw shall fully extend beyond the inside face of the band joist.

d. Sheathing shall be wood structural panel or solid sawn lumber

e. Sheathing shall be permitted to be wood structural panel, gypsum board, fiberboard lumber or foam sheathing. Up to 1/2" thinckness of stacked washers shall be permitted to substitute for you to 1/2" of allowable sheathing thickness where combined with wood structural panel or lumbers sheathing.

TABLE IRC2018 R507.9.1.3(2) PLACEMENT OF LAG SCEWS AND BOLT IN **DECK LEDGERS AND BAND JOISTS**

MINIMUM END AND EDGE DISTANCES AND SPACING BETWEEN ROWS									
	TOP EDGE	BOTTOM EDGE	ENDS	ROW SPACING					
LEDGER ^a	2 inches ^d	3/4 inches	2 inches ^b	1 5/8 inches ^b					
BAND JOIST °	3/4 inches	2 inches	2 inches	1 5/8 inches ^b					

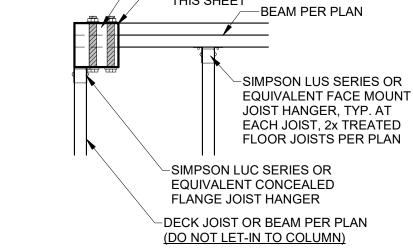
For SI: 1 inch = 25.4mm. a. Lag screws of bolts shal lbe staggered from the top to the bottom along the horizontal run of the deck ledger in accordance with Figure R507.9.1.3(1)

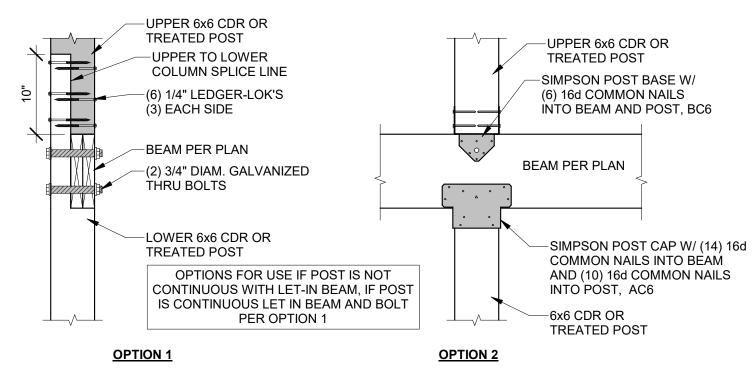
c. For engineered rim joists, the manufacturer's recommendations shall govern.

d. The minimum distances from bottom row of lag screws or bolts to the top of the ledger shall be in accordance with Figure R507.9.1.3(1)

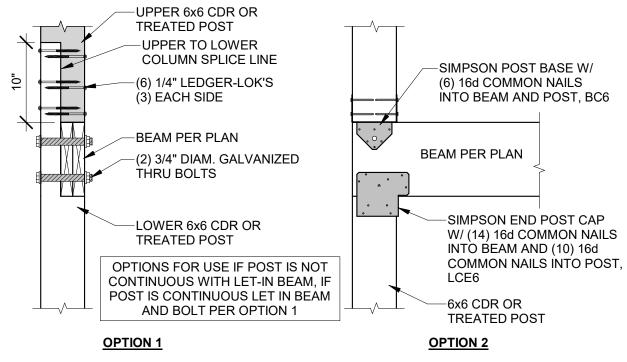
1X4 TREATED SPACER NAILED TO--FLOOR JOISTS PER PLAN THE HOUSE RIM W/(3) - 16d NAILS 2x TREATED LEDGER-TREATED DECK JOIST -FOUNATION OR FRAMED WALLS PER PLAN, INSULATE PER PLAN 1/2" CORROSION RESISTANT LAG-SCREWS OR BOLT MUST PENETRATE HOUSE RIM SEE R507.2 THIS SHEET

DECK LEDGER ATTACHMENT

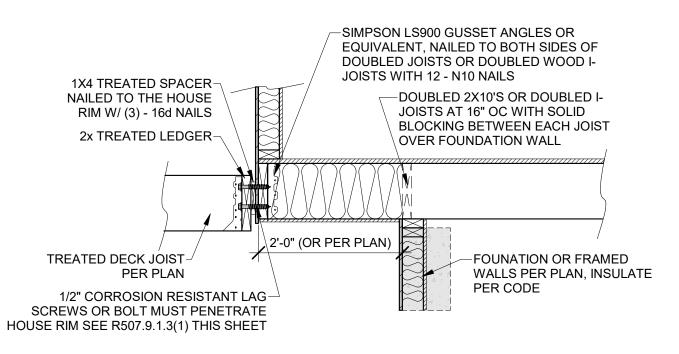




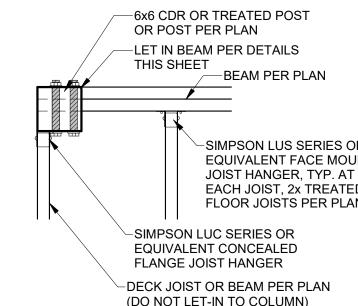
DECK LEVEL INTERIOR BEAM TO COLUMN



DECK LEVEL EXTERIOR BEAM TO COLUMN



DECK LEDGER TO CANTILEVER



DECK CORNER COLUMN

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DECK DETAILS

			NON-BEAR	ING WALLS			
STUD SIZE (INCHES)	LATERALLY UNSUPPORTED STUD HEIGHT ^a (FEET)	MAXIMUM SPACING WHERE SUPPORTING A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY, ONLY (INCHES)	MAXIMUM SPACING WHERE SUPPORTING ONE FLOOR, PLUS A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY (INCHES)	MAXIMUM SPACING WHERE SUPPORTING TWO FLOORS, PLUS A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY (INCHES)	MAXIMUM SPACING WHERE SUPPORTING ONE FLOOR HEIGHT ^a (INCHES)	LATERALLY UNSUPPORTED STUD HEIGHT ^a (FEET)	MAXIMUM SPACING (INCHES)
2 x 3 ^b						10	16
2 x 4	10	24°	16°		24	14	24
3 x 4	10	24	24	16	24	14	24
2 x 5	10	24	24		24	16	24
2 x 6	10	24	24	16	24	20	24

a. LISTED HEIGHTS ARE DISTANCES BETWEEN POINTS OF LATERAL SUPPORT PLACED PERPENDICULAR TO THE PLANE OF THE WALL. BEARING WALLS SHALL BE SHEATHED ON NOT LESS THAN ONE SIDE OR BRIDGING SHALL BE INSTALLED NOT GREATER THAN 4 FEET APART MEASURED VERTICALLY FROM EITHER END OF THE STUD. INCREASES IN UNSUPPORTED HEIGHT ARE PERMITTED WHERE IN COMPLIANCE WITH EXCEPTION 2 OF SECTION R602.3.1 OR DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICE.

b. SHALL NOT BE USED IN EXTERIOR WALLS.
c. A HABITABLE ATTIC ASSEMBLY SUPPORTED BY 2 x 4 STUDS IS LIMITED TO A ROOF SPAN OF 32 FEET. WHERE THE ROOF SPAN EXCEEDS 32 FEET, THE WALL STUDS SHALL BE INCREASED TO 2 x 6 OR THE STUDS SHALL BE DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICE.

	RESIDENTIAL SEISMIC & WIND ANALYSIS									
							INPUT			
ETERMINE WEIGH	T OF HOUSE:						CALCULATED VALUE			
OCATION					DEAD LOAD (psf)	AREA (ft²)	WEIGHT (lbs.)			
00F					10	3388	33880			
EILING					10	3283	32830			
ECOND FLOOR					10	1308	13080			
RST FLOOR					10	1094	10940			
				WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WEIGHT (lbs)			
ECOND FLOOR EX	T. WALL DL			189	9	9	15309			
RST FLOOR EXT. \	WALL DL			155.66	10	10	15566			
					DEAD LOAD (psf)	AREA (ft2)	WEIGHT (lbs)			
ECOND FLOOR INT	. PARTITION WALL DL				6	1308	7848			
	. PARTITION WALL DL ARTITION WALL DL				6 6	1308 1094	7848 6564			
	ARTITION WALL DL				6	1094				
	ARTITION WALL DL	· · · · · · · · · · · · · · · · · · ·) DESIGN PER 115 MPH	3-SECOND GUST, EXPOSUR	6 RE C AND MEAN ROOF HEIGHT <=	1094 = 30 FT ASSUMED)				
	ARTITION WALL DL PRO FRONT-	-TO-BACK	DESIGN PER 115 MPH	3-SECOND GUST, EXPOSUR	6 RE C AND MEAN ROOF HEIGHT <- SIDE-TO	1094 - 30 FT ASSUMED) - SIDE				
RST FLOOR INT. P	ARTITION WALL DL PRO FRONT- AREA	-TO-BACK LOAD	DESIGN PER 115 MPH		6 RE C AND MEAN ROOF HEIGHT <= SIDE-TO AREA	1094 = 30 FT ASSUMED) SIDE LOAD				
RST FLOOR INT. P	PRO FRONT- AREA 284	TO-BACK LOAD 1233		SLOPED ROOF	6 RE C AND MEAN ROOF HEIGHT <- SIDE-TO AREA 281	1094 30 FT ASSUMED) SIDE LOAD 1238	6564			
RST FLOOR INT. P	PRO. FRONT- AREA 284 0	TO-BACK LOAD 1233 0	CUMULATIVE	SLOPED ROOF VERT. ROOF	6 RE C AND MEAN ROOF HEIGHT <- SIDE-TO AREA 281 0	1094 30 FT ASSUMED) SIDE LOAD 1238 0	6564 CUMULATIVE			
SLOPED ROOF VERT. ROOF 2ND	PRO. FRONT AREA 284 0 495	TO-BACK LOAD 1233 0 6906	CUMULATIVE 8139	SLOPED ROOF VERT. ROOF 2ND	6 RE C AND MEAN ROOF HEIGHT <- SIDE-TO AREA 281 0 450	1094 30 FT ASSUMED) SIDE LOAD 1238 0 6382	6564 CUMULATIVE 7620			
SLOPED ROOF VERT. ROOF 2ND 1ST	PRO. FRONT- AREA 284 0	TO-BACK LOAD 1233 0	CUMULATIVE	SLOPED ROOF VERT. ROOF 2ND 1ST	6 RE C AND MEAN ROOF HEIGHT <- SIDE-TO AREA 281 0	1094 30 FT ASSUMED) SIDE LOAD 1238 0	6564 CUMULATIVE			
SLOPED ROOF VERT. ROOF 2ND	PRO. FRONT AREA 284 0 495	TO-BACK LOAD 1233 0 6906	CUMULATIVE 8139 16184 19664	SLOPED ROOF VERT. ROOF 2ND 1ST BSMT ^a	6 RE C AND MEAN ROOF HEIGHT <- SIDE-TO AREA 281 0 450	1094 30 FT ASSUMED) SIDE LOAD 1238 0 6382	6564 CUMULATIVE 7620			
SLOPED ROOF VERT. ROOF 2ND 1ST	PRO. FRONT- AREA 284 0 495 583 200	TO-BACK LOAD 1233 0 6906 8045 3480	CUMULATIVE 8139 16184 19664	SLOPED ROOF VERT. ROOF 2ND 1ST	6 RE C AND MEAN ROOF HEIGHT <- SIDE-TO AREA 281 0 450 546.26 0	1094 30 FT ASSUMED) SIDE LOAD 1238 0 6382 7617	6564 CUMULATIVE 7620 15237 0			
SLOPED ROOF VERT. ROOF 2ND 1ST	PRO. FRONT AREA 284 0 495 583 200 SLOPED ROOF	TO-BACK LOAD 1233 0 6906 8045 3480 ZONE B	CUMULATIVE 8139 16184 19664 PRESSURE (PSF	SLOPED ROOF VERT. ROOF 2ND 1ST BSMT ^a) - PER ASCE CH. 26 5.9	6 RE C AND MEAN ROOF HEIGHT <	1094 30 FT ASSUMED) SIDE LOAD 1238 0 6382 7617 0	CUMULATIVE 7620 15237 0			
SLOPED ROOF VERT. ROOF 2ND 1ST	PRO. FRONT- AREA 284 0 495 583 200	TO-BACK LOAD 1233 0 6906 8045 3480	CUMULATIVE 8139 16184 19664 PRESSURE (PSF	SLOPED ROOF VERT. ROOF 2ND 1ST BSMT ^a) - PER ASCE CH. 26	6 RE C AND MEAN ROOF HEIGHT <- SIDE-TO AREA 281 0 450 546.26 0	1094 30 FT ASSUMED) SIDE LOAD 1238 0 6382 7617 0	CUMULATIVE 7620 15237			

 q_{z10}=0.00256K_zK_{zt}K_dV² (ASCE7-16 Velocity Pressure)
 q_{z10_ASD}=0.6q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-16 and IRC/IBC 2018)

 2ND FLOOR TRIBUTARY WEIGHT
 74364.5

 1ST FLOOR TRIBUTARY WEIGHT
 110730

 BASEMENT TRIBUTARY WEIGHT
 110730

 S_S (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP)
 10.0%

F_a (from ASCE7 Table 11.4-1)

1ST FLOOR FRONT-TO-BACK

WITH CONSTRUCTION

(= 2/3 * S _S * F _a)				0.073	
om ASCE7 Table 12.2-1)				6.5	
		SEISMIC SHEAR			
ATION			n ASCE7 (Eq. 12.8-1):	V (= 1.25 * S _{DS}	* W / R) (lbs.)
FLOOR				104	
FLOOR				156	
EMENT				156	52
Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowat	ole Shear (#/LF)	Code Reference
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing		155	per IBC, Table 2306.3(1)
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing		230	per IBC, Table 2306.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing		310	per IBC, Table 2306.3(1)
Exterior (Option #4)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing		220	AF&PA SDPWS Table 4.3A
Exterior (<i>Option #5</i>)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing		320	AF&PA SDPWS Table 4.3A
Exterior (<i>Option #6</i>)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing	6d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field		200	AF&PA SDPWS Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 11/4" Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field		60	per IBC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)		325	

EXTERIOR SHEAT	RIOR SHEATHING OPTION FOR SECOND FLOOR		4		WIDTH OF 1ST STORY (FT.)	53	WIDTH OF 2ND STORY (FT.)	49.5
EXTERIOR SHEAT	THING OPTION FOR FIRS	T FLOOR	4		DEPTH OF 1ST STORY (FT.)	49.66	DEPTH OF 2ND STORY (FT.)	45
EXTERIOR SHEAT	THING OPTION FOR BASE	EMENT WALLS	4	1	BACK WALL OF GARAGE (FT.)	34		
				<u>-</u>	GAR. WALL: 1=F-B, 2=S-S	2		
			FVTFD	NOD CTRUCTURAL WALL	LENGTHS (ft.) & RESISTANCES			
		SE	ISMIC	TIOR STRUCTURAL WALL	LENGTHS (IL.) & RESISTANCES	WIND	ı	
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs
2ND FLOOR	76	21280	87	24360	76	29792	87	34104
1ST FLOOR	81	22680	83	23240	81	31752	83	32536
BASEMENT	106	29680	99.32	27810	106	41552	99.32	38933
		ADDITIONAL RESIS	TANCE REQUIRED		Anchor Bolt Spacing	(in.)	16d Nail Spacing req'd at	bottom plate (in.)
	SEISMIC WIND		1	diameter (in.)	0.5	2nd Floor F-B		
2ND FLOOR FROI	NT-TO-BACK	0	0		Shear value (per NDS)	944	2nd Floor S-S	
2ND FLOOR SIDE	-TO-SIDE	0	0		Spacing F-B (inches)	111.2	1st Floor F-B	

1ST FLOOR SIDE-TO-SIDE	0	0					
BASEMENT FRONT-TO-BACK	0	0					
BASEMENT SIDE-TO-SIDE	0	0					
			•				
		RESISTANCE REQUI	RED IN ADDITION TO RES	SISTANCE PROVIDED BY EXTERIOR W	/ALLS**		
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)	OK?
2ND FLOOR FRONT-TO-BACK	0					0	YES
2ND FLOOR SIDE-TO-SIDE	0					0	YES
1ST FLOOR FRONT-TO-BACK	0					0	YES
1ST FLOOR SIDE-TO-SIDE	0					0	YES
BASEMENT FRONT-TO-BACK	0					0	YES
BASEMENT SIDE-TO-SIDE	0					0	YES
**NOTES: 1) SEE ATTACHED CALCULATIO	NIC EOD DODTAL EDAME	OD DEDEODATED SH	EAD WALL DESIGNANCE	CADACITIES (IE ADDI ICADI E)			

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE),
2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING
PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER

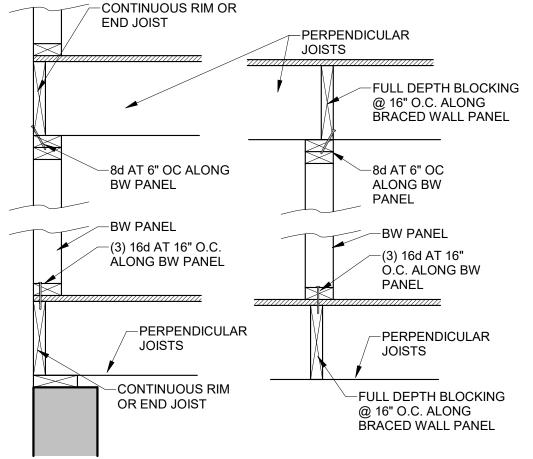
				WIND UPLIFT	ANALYSIS		
	X/12	DEGREES					
ROOF PITCH (MAX)	5	22.6	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2			
		ASCE 7					
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)			
OVERHANG	2	16.56	209.32	33.12			
	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LB
MAIN ROOF**	2631.98	1482.569504	1149.410496	15.12	10.5	34485	168.0
*ALONG PERIMETER		TOTAL UPLIFT PER LINEAL	FOOT ALONG EXTERIOR (PC	DUNDS)	201.1	UPLIFT OK	
**INSIDE EXTERIOR V	VALLS	RESISTANCE DUE TO DEAD	WEIGHT & (3) 10d TOENAILS	3	258.6		

NOTE FOR CONSTRUCTION:

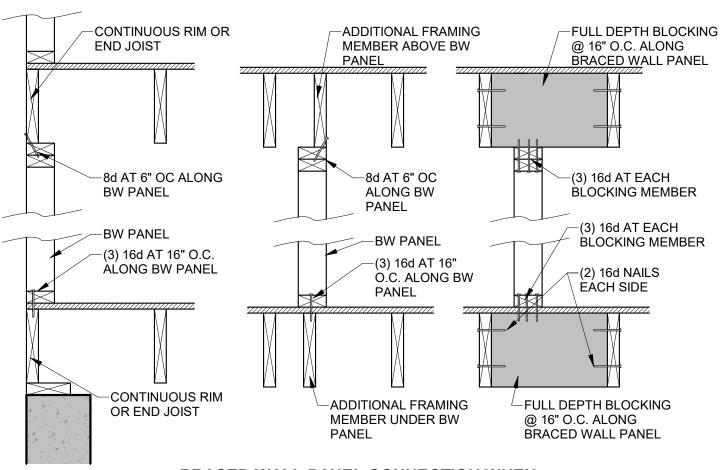
THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:
ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2018 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 220 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING

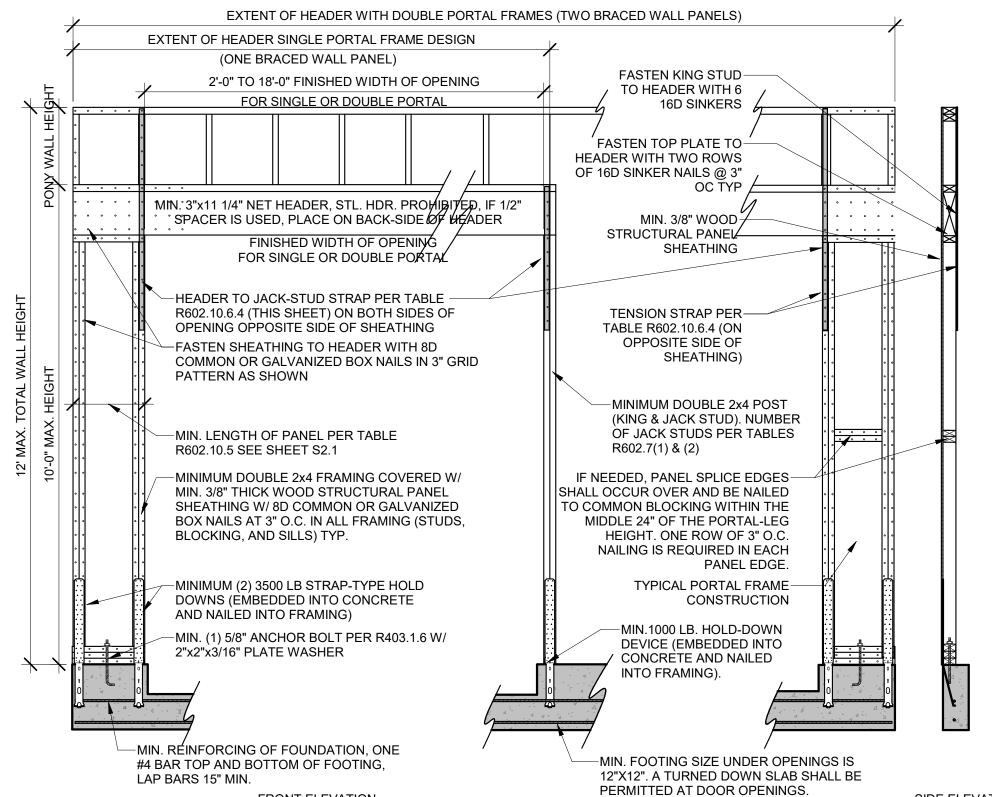


BRACED WALL PANEL CONNECTION WHEN PERPENDICULAR TO FLOOR/CEILING JOISTS



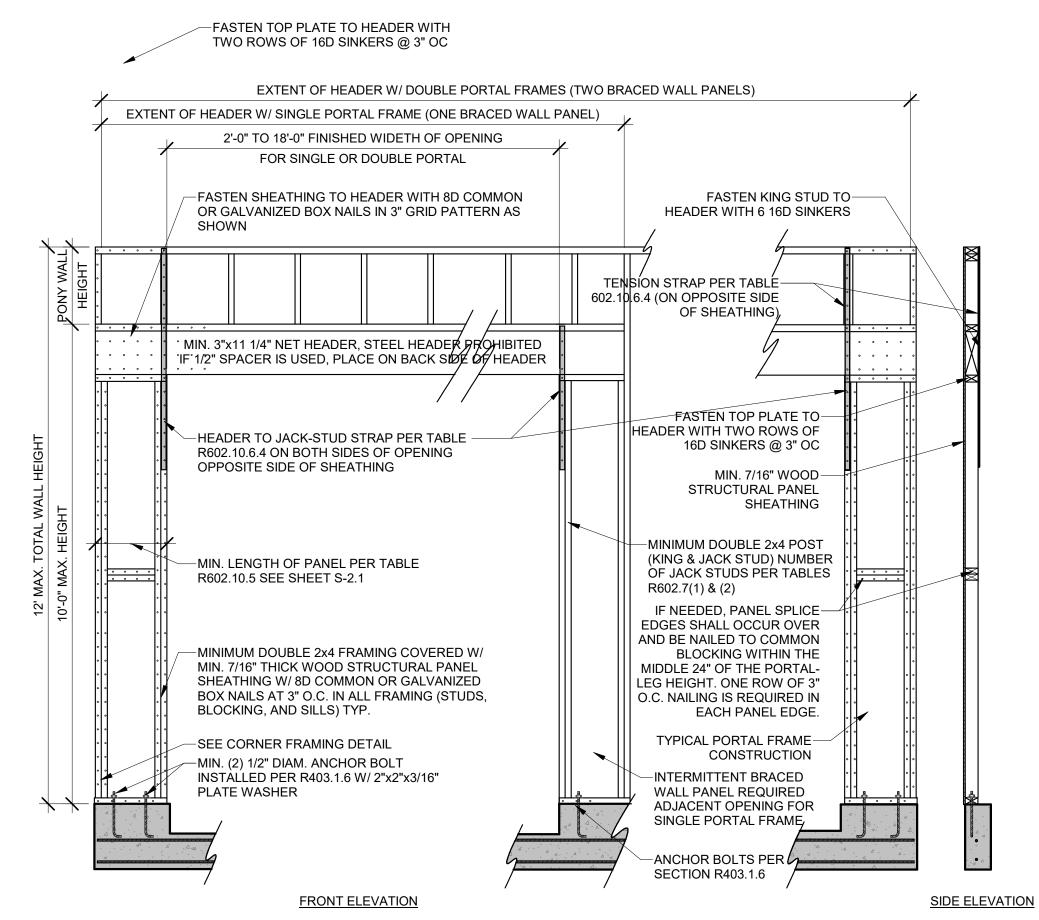
BRACED WALL PANEL CONNECTION WHEN PARALLEL TO FLOOR/CEILING JOISTS

BRACED WALL PANEL CONNECTIONS



PFH PORTAL FRAME W/ HOLD DOWNS (R602.10.6.2)

1/2" = 1'-0"

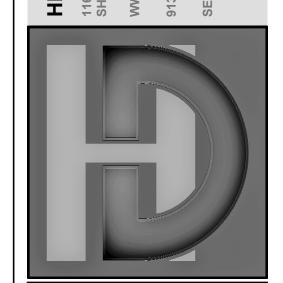


PFG PORTAL FRAME W/OUT HOLD DOWNS (R602.10.6.3)

1/2" = 1'-0"

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INTROCK DR. LEES SUMMIT, MO

D#: 48058

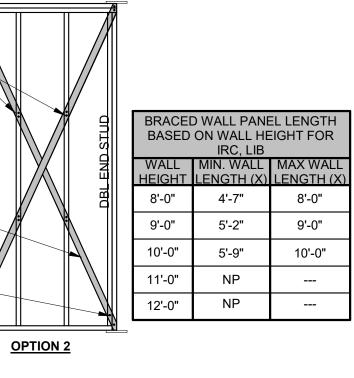
DATE: 8/6/2024

NO. ISSUE/REVISION Revision Date

BRACED WALL NOTES & DETAILS

S-2.0

TENSION STRAP CAPACITY REQUIRED FOR RESISTING WIND PRESSURES PERPENDICULAR TO METHOD PFH, PFG AND CS-PF BRACED WALL PANELS IRC2018 TABLE R602.10.6.4



6 LIB BRACING 3/8" = 1'-0"

(2)|BD NAILS @ EACH-

INTÉRMEDIAITE STUDS

(2) 16 NAIL\$ @ EACH

PLATE FACE NAILED

FOR IRC CODE PRESCRIPTIVE METHOD

TABLE R602.10.5 MINIMUM LENGTH OF BRACED WALL PANELS

(2) 8D NAILS @ EACH-NTERMEDIATE STUDS

16 GA. STL. STRAP-

SIMPSON / USP TYPE

WB (OR EQUIVALENT)

(2) 16D NAILS @ EACH-

PLATE FACE NAILED

	METHOD			LENGTH	CONTRIBUTING LENGTH		
	(SEE TABLE R602.10.4)		W	ALL HEIGI			(INCHES)
			9 FEET	10 FEET	11 FEET	12 FEET	
DWB,WSP,SFB,PBS,PCP,HPS,BV-WSP			48	48	53	58	ACTUAL ^b
	GB	48	48	48	53	58	DOUBLE SIDED = ACTUAL SINGLE SIDED=.5xACTUAL
	LIB	55	62	69	NP	NP	ACTUAL ^b
A D) A /	SDC A, B, AND C ULTIMATE DESIGN WIND SPEED<140	28	32	34	38	42	40
ABW	SDC D ₀ ,D ₁ ,D ₂ ULTIMATE DESIGN WIND SPEED<140	32	32	34	NP	NP	48
PFH	SUPPORTING ROOF ONLY	16	16	16	NOTE C	NOTE C	48
PFH	SPTNG. ONE STORY & ROOF	24	24	24	NOTE C	NOTE C	48
	PFG	24	27	30	NOTE D	NOTE D	1.5 x ACTUAL ^b
	CS-G	24	27	30	33	36	ACTUAL ^b
	CS-PF	16	18	20	NOTE E	NOTE E	ACTUAL ^b
	ADJACENT CLEAR OPENING HEIGHT (INCHES)						
	≤64	24	27	30	33	36	
	68	26	27	30	33	36	
	72	27	27	30	33	36	
	76	30	29	30	33	36	
	80	32	30	30	33	36	
	84	35	32	32	33	36	
	88	38	35	33	33	36	
	92	43	37	35	35	36	
CS-WSP,	96	48	41	38	36	36	ACTUAL ^b
CS-SFB	100	-	44	40	38	38	AUTUAL
	104	-	49	43	40	39	
	108	-	54	46	43	41	
	112	-	-	50	45	43	
	116	-	-	55	48	45	
	120	-	-	60	52	48	
	124	-	-	-	56	51	
	128	-	-	-	61	54	
	132	_	-	-	66	58	
	136	-	-	-	-	62	
	140	-	-	-	-	66	
	144	-	-	-	-	72	

a. LINEAR INTERPOLATION SHALL BE PERMITTED
b. USE THE ACTUAL LENGTH WHEN IT IS GREATER THAN OR EQUAL TO THE MINIMUM LENGTH
c. MAX HEADER HEIGHT FOR PEH IS 10' IN ACCORDANCE WITH R602 10.6.2. WALL HEIGHT MAY BE INCREASED TO 12' WITH PON

c. MAX. HEADER HEIGHT FOR PFH IS 10' IN ACCORDANCE WITH R602.10.6.2, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.
d. MAX. OPENING HEIGHT FOR PFG IS 10' IN ACCORDANCE WITH R602.10.6.3, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.
e. MAX. OPENING HEIGHT FOR CS-PF IS 10' IN ACCORDANCE WITH R602.10.6.4, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.

BRACED WALL PRESCRIPTIVE METHOD:

CONTINOUS EXTERIOR SHEATHING (CS-WSP) PER WSP METHOD (BELOW) UNLESS OTHERWISE NOTED ON THE PLAN

EXTERIOR BRACED WALL METHOD: (SEE ON THIS SHEET) WOOD STRUCKTEAL DANIEL SHEATHING WITH A THICKNESS NOT

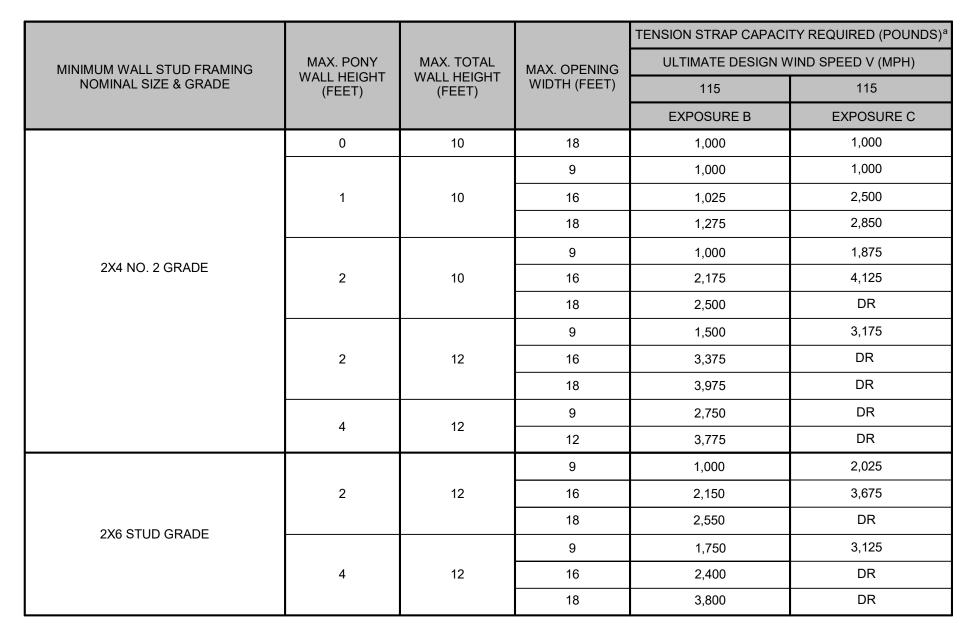
WOOD STRUCUTRAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 3/8" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" O.C. STUD SPACING WITH 6d NAILS COMMON NAILS @ 6" O.C. EDGES AND 12" O.C. FIELD OR SHEATHING THICKNESS NOT LESS THANK 7/16" WITH MINIMUM SPAN RATING OF 24/16 FOR 24" O.C. SPACING WITH 8d COMMON NAILS @ 6" O.C. EDGES AND 12" O.C. IN FIELD (NOTE: FRAMING MEMBERS 16" O.C. MAX, UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS)

INTERIOR BRACED WALLS (SEE ON THIS SHEET)

1/2" MINIMUM GYPSUM BOARD OVER STUDS SPACED @ 24" MAXIMUM FASTENED W/ #6- 1 1/4" TYPE "W" OR "S" DRYWALL SCREWS @ 7" O.C. EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES)

OR LIB METHOD:

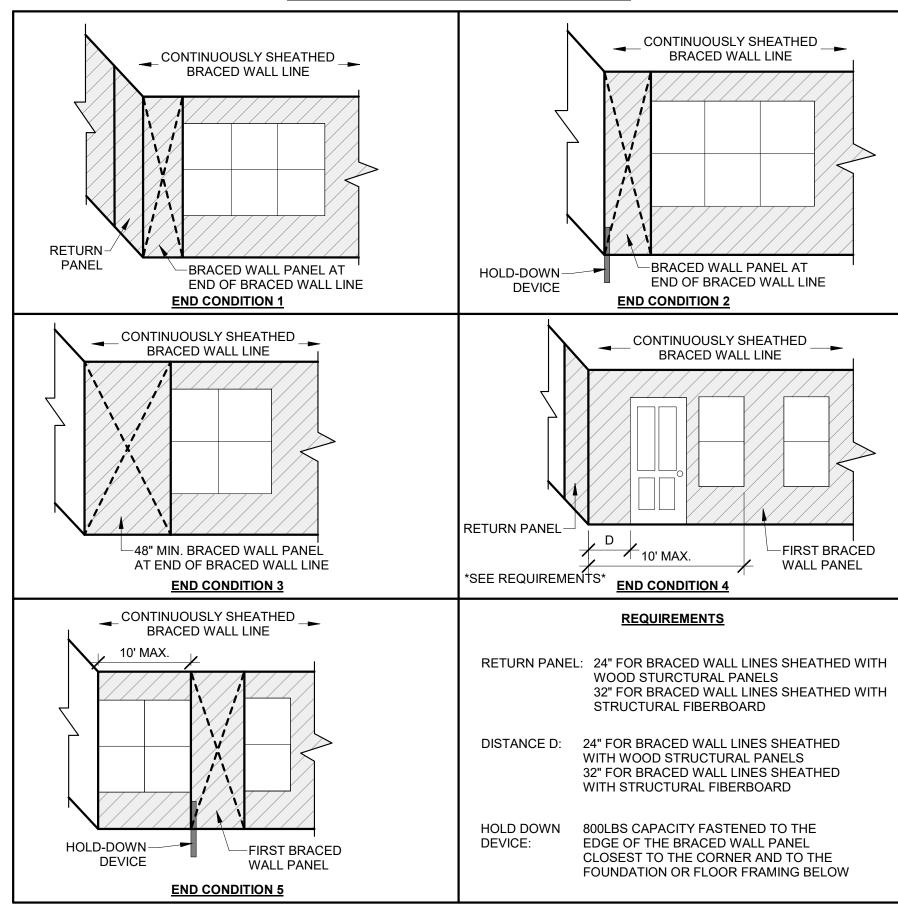
1X4 WOOD FASTENED W/ (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUIVALENT) STL. X-BRACE(S) @ 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUDS FASTENED PER MANUF. SPECS.

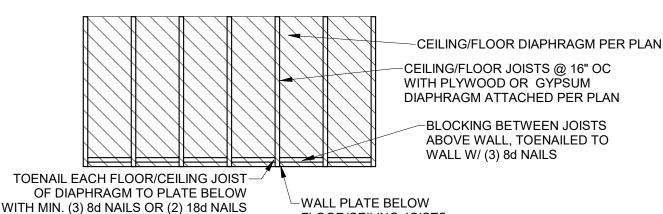


a. DR = DESIGN REQUIREDb. STRAP SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

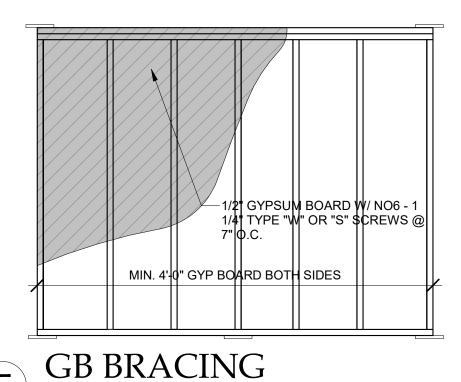
END WALL CONDITIONS

FOR CONTINUOUSLY SHEATHED BRACED WALL LINES



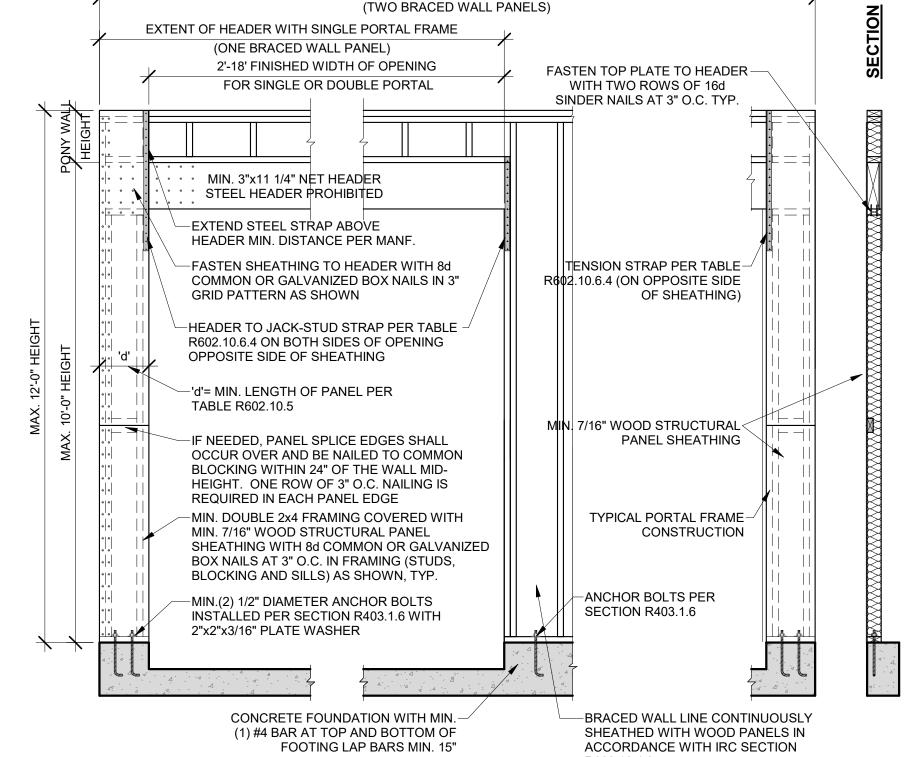


DIAPHRAGM CONNECTION TO INTERIOR WALL

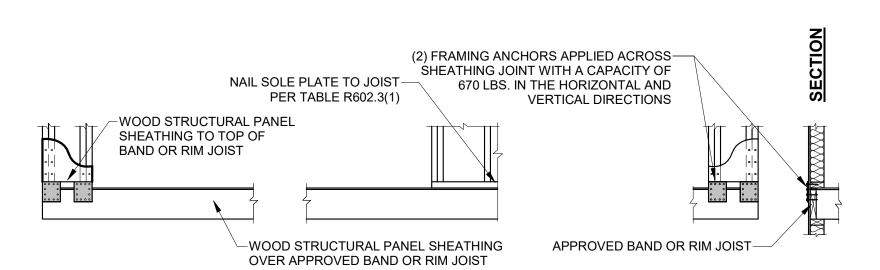


FRONT ELEVATION

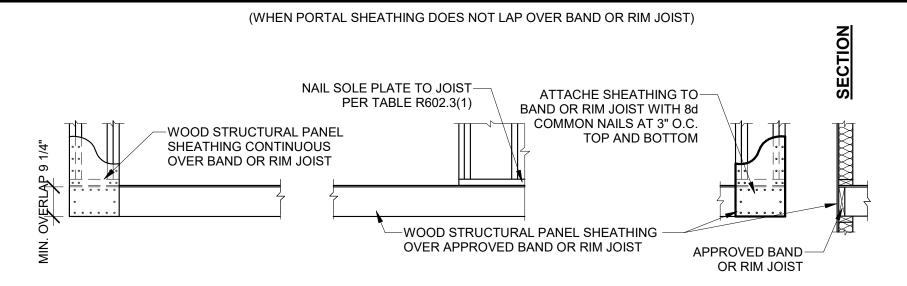
EXTENT OF HEADER WITH DOUBLE PORTAL FRAMES



OVER CONCRETE OR MASONRY BLOCK FOUNDATION

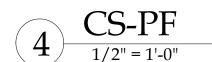


OVER RAISED WOOD FLOOR - FRAMING ANCHOR OPTION



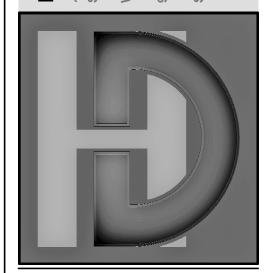
OVER RAISED WOOD FLOOR - OVERLAP OPTION

(WHEN PORTAL SHEATHING LAPS OVER BAND OR RIM JOIST)



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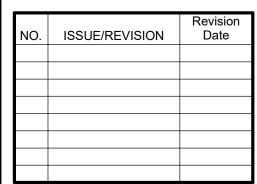


ST CHOICE CUSTOM HON HE HEATHER III GR LOT 195 NAPA VALLE 304 S.W. FLINTROCK DR. LEES SUMMIT, M

HD#: 48058

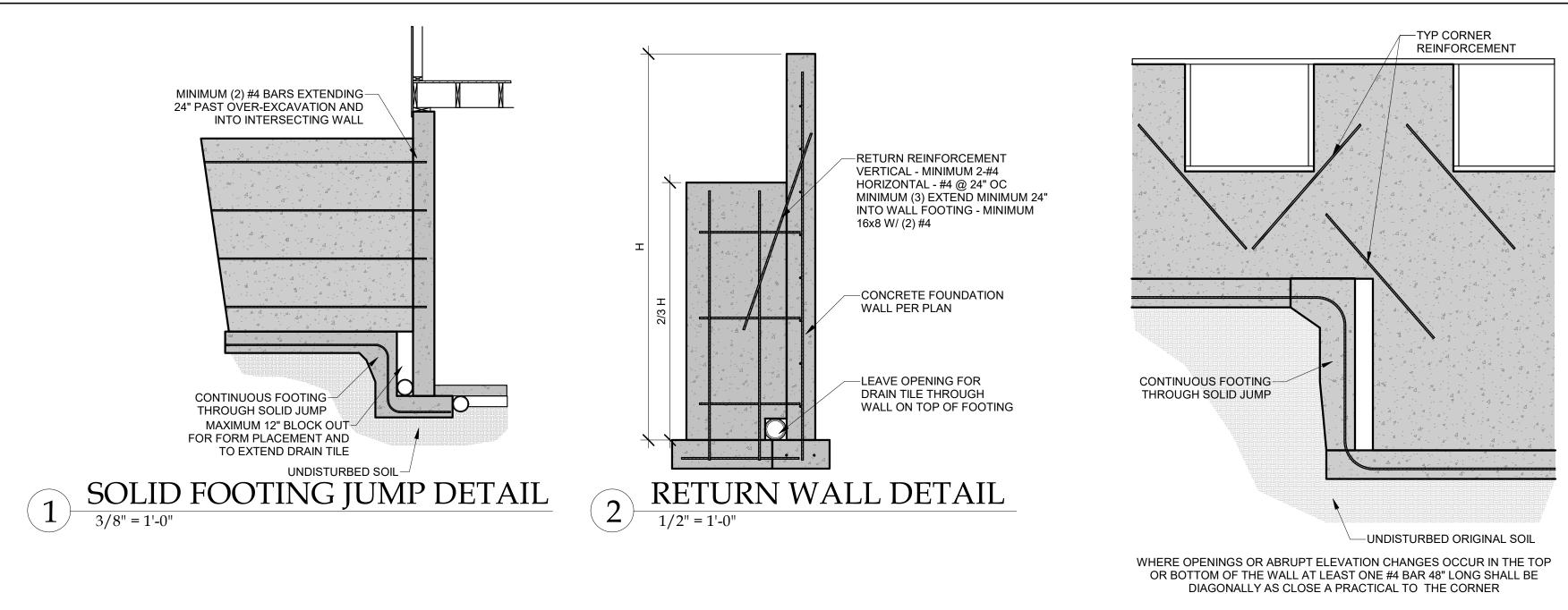
DATE: 8/6/2024

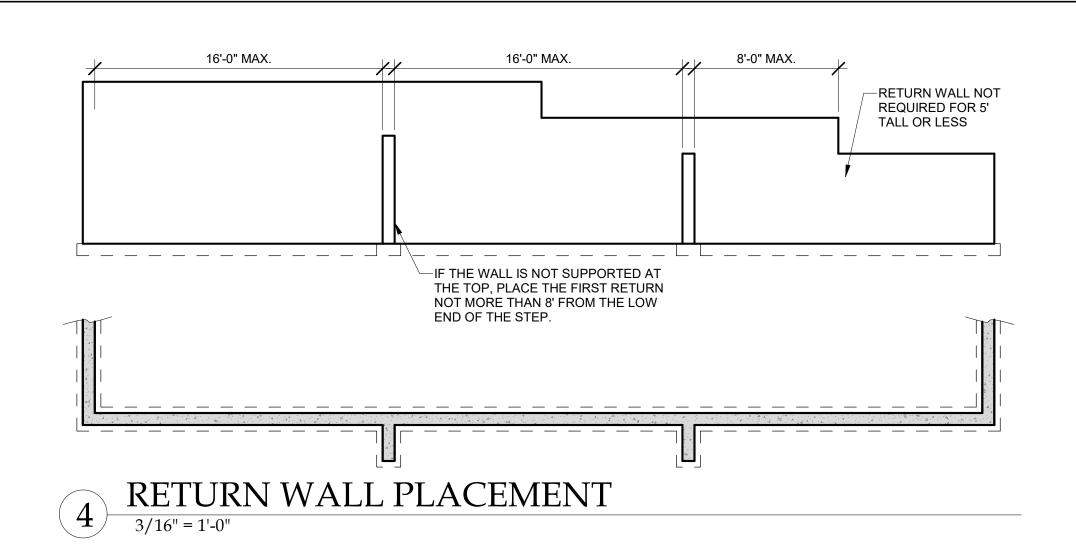
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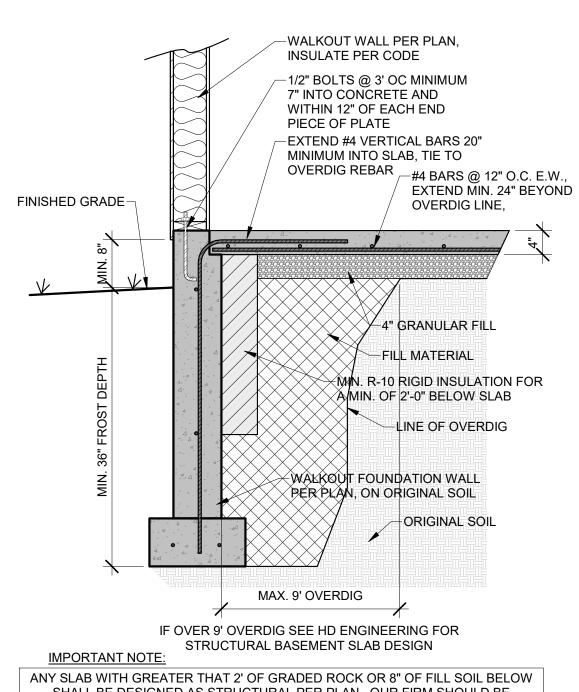
BRACED WALLS NOTES & DETAILS

S-2.1



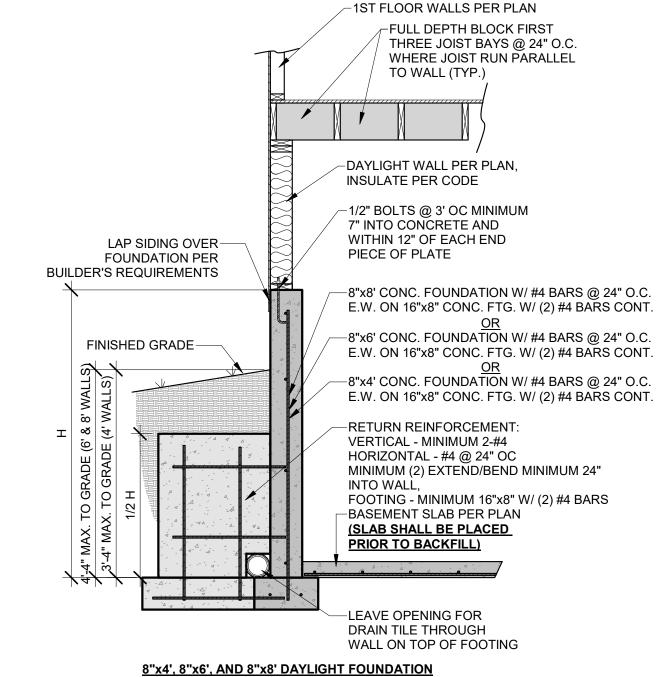


REINFORCEMENT AT CORNERS AND STEPS



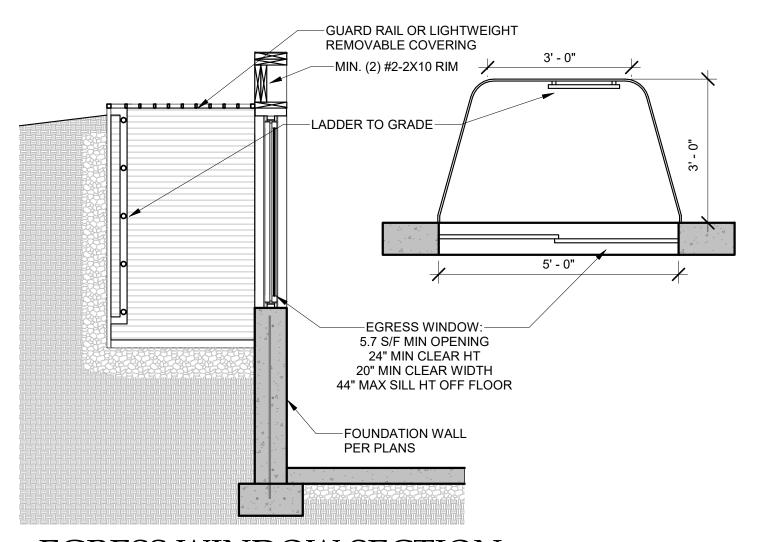
SHALL BE DESIGNED AS STRUCTURAL PER PLAN. OUR FIRM SHOULD BE CONTACTED IMMEDIATELY FOR DESIGN RECOMMENDATIONS. DESIGN MUST BE COMPLETED PRIOR TO PLACEMENT OF PIERS OR FOOTINGS.

WALKOUT DETAIL
3/4" = 1'-0"



IF SLAB IS NOT PLACED PRIOR TO BACKFILL CONTRACTOR IS RESPONSIBLE FOR BRACING THE FOUNDATION AS REQUIRED

UNRESTRAINED FOUNDATION WALL



EGRESS WINDOW SECTION

1/2" = 1'-0"

9' 16 1 24 1 20	1
1 24	1
20	1
, 20	1
1 24	1
-	20

* MINIMUM REQUIREMENT FOR VERTICAL REBAR IN PLAIN CONCRETE WALLS IS #4 @ 36" ON CENTER (ACI 332). * VERTICAL BARS SHALL BE CONTINUED UP TO WITHIN 8" OF THE TOP OF THE WALL.

* REBAR SHALL BE POSITIONED AT THE TENSION FACE OF THE WALL (2" FROM THE INSIDE

* REINFORCEMENT SHALL LAP A MINIMUM OF 24 INCHES AT ENDS, SPLICES, AND AROUND CORNERS.

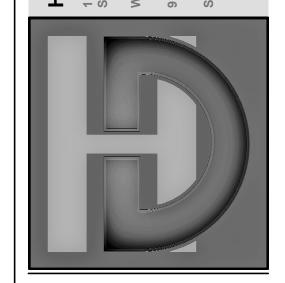
** #4 BARS @ 24" ON CENTER.

** #4 BAR WITHIN 12 OF TOP AND BOTTOM OF WALL. ** MINIMUM GRADE 40 (40ksi) STEEL (PER ACI 332).

** HORIZONTAL REINFORCEMENT SHALL BE INSTALLED ON THE COMPRESSION SIDE (SOIL

SIDE) OF THE VERTICAL REINFORCEMENT

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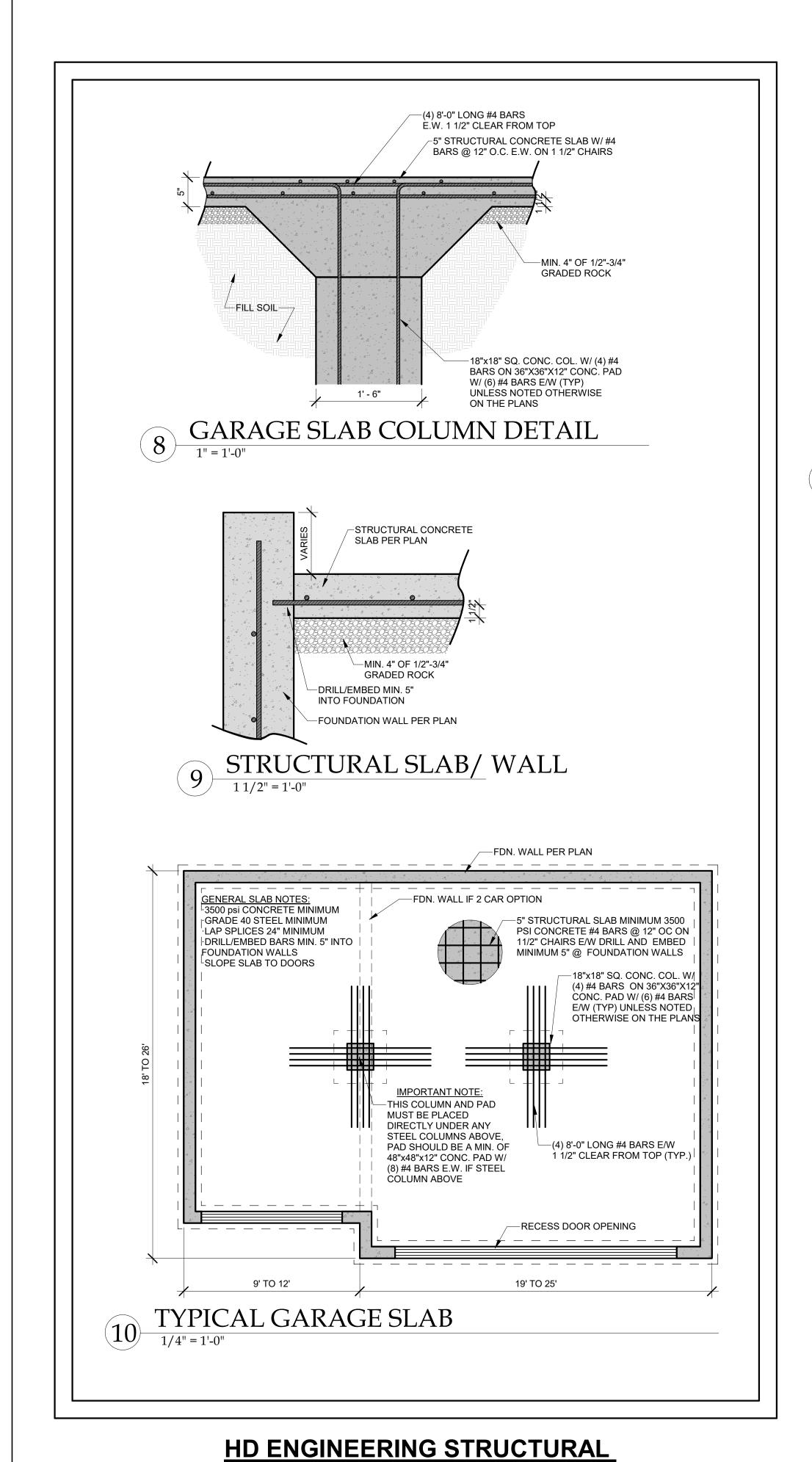


48058 8/6/2024 DATE:

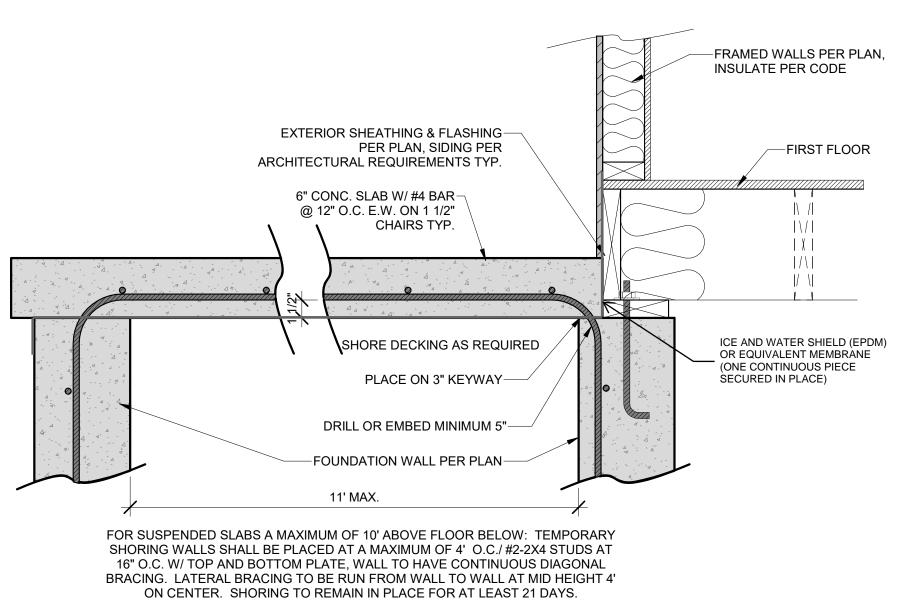
CHECKED BY: CLS

	,	
NO.	ISSUE/REVISION	Revision Date

CONCRETE DETAILS



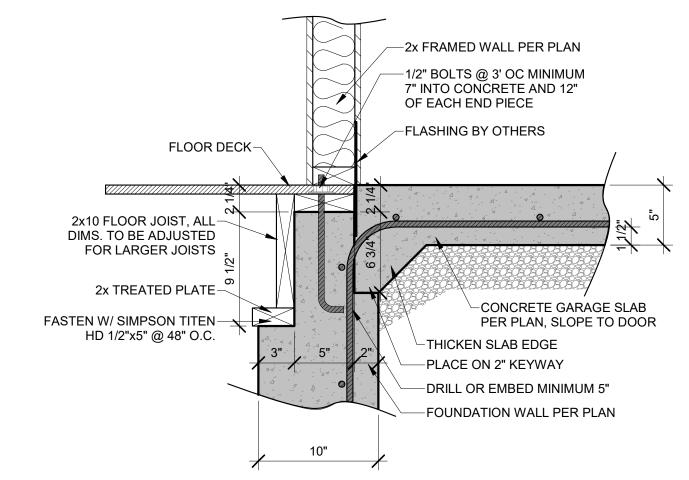
GARAGE SLAB DETAILS



6 SUSPENDED PORCH STOOP SLAB

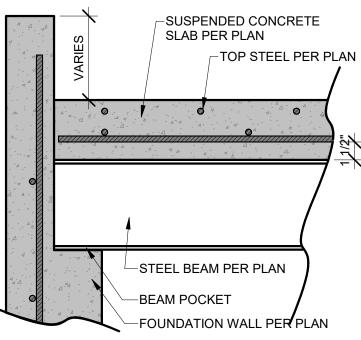
1 1/2" = 1'-0"

SEE JOCOBO FLASHING DETAIL FOR ADDITIONAL INFORMATION



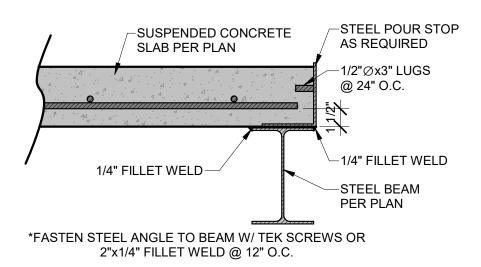
ZERO ENTRY GARAGE DETAIL

1 1/2" = 1'-0"



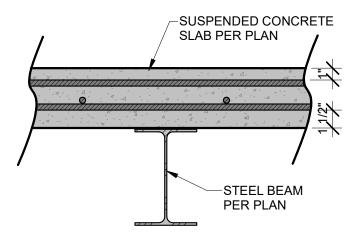
SUSPENDED SLAB BEAM/WALL CONNECTION

1 1/2" = 1'-0"



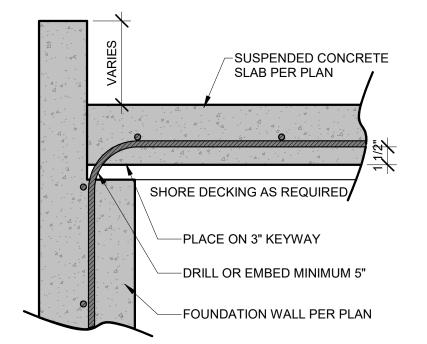
SUSPENDED SLAB POUR STOP

1 1/2" = 1'-0"



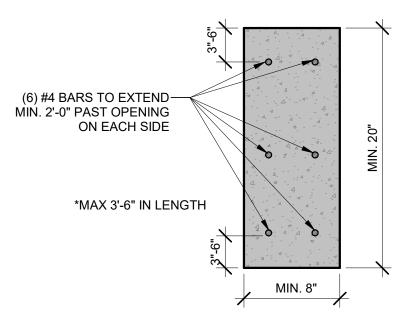
SUSPENDED SLAB/STEELBEAM CROSS SECTION

1 1/2" = 1'-0"



SUSPENDED SLAB/WALL CONNECTION

11/2" = 1' 0"



CONCRETE HEADER DETAIL

IN 1/2" = 1'-0"

FOR SUSPENDED SLABS A MAXIMUM OF 10' ABOVE FLOOR BELOW: TEMPORARY SHORING WALLS SHALL BE PLACED AT A MAXIMUM OF 4'

O.C. / #2-2X4 STUDS AT 16" O.C. W/ TOP AND BOTTOM PLATE, WALL TO HAVE CONTINUOUS DIAGONAL BRACING. LATERAL BRACING TO BE
RUN FROM WALL TO WALL AT MID HEIGHT 4' ON CENTER. SHORING TO REMAIN IN PLACE FOR AT LEAST 21 DAYS.

-ANY CAST IN PLACE SLABS FORMED MORE THAN 10' ABOVE THE FLOOR BELOW SHALL HAVE A SITE SPECIFIC SHORING DESIGN DONE. OUR
FIRM SHOULD BE CONSULTED FOR THIS DESIGN ONCE FOUNDATION WALLS ARE IN PLACE TO EVALUATE ALL FIELD CONDITIONS. IT SHOULD

D ENGINEERING & DESIGN, INC

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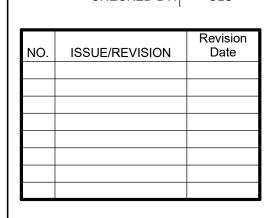
E CUSTOM HCGR LOT 195 NAPA VAIOCK DR. LEES SUMMITHER DETAILS & NOTES

THE HEATHER III GR LOT 13904 S.W. FLINTROCK DR.

HD#: 48058

DATE: 8/6/2024

CHECKED BY: CLS



SUSPENDED SLAB DETAILS

S-3.1

MINIMUM INSULATION & FENSTRATION VALUES BY COMPONENT, PER IRC2018 N1102.1.2

CLIMATE ZONE	FENSTRATION U-FACTOR	SKYLIGHT U-FACTOR	GLAZED SHGC FENSTRATION	IINSULATED METAL	INSULATED WOOD DOOR U-VALUE	CEILING R-VALUE	WOOD FRAMED WALL R-VALUE	FLOOR R-VALUE	BASEMENT WALL R-VALUE	SLAB R-VALUE & DEPTH		DUCTWORK OVER OUTSIDE R-VALUE	,
4 EXCEPT MARINE	0.32	0.55	0.40	0.60	0.50	49	20 OR 13 CAV. +5	19	10 CONTINUOUS OR 13 CAVITY	R-10, 2 FT.	10 CONTINUOUS OR 13 CAVITY	8	6

NOTES: 1) BUILDING THERMAL ENVELOPE IS REQUIRED TO BE SEALED WITH AN AIR BARRIER AS PER N1102.4.1 OF THE 2018 IRC
2) RECESSED LIGHTING SHALL BE SEALED TO PREVENT LEAKAGE BETWEEN THE CONDITIONED SPACE AND UNCONDITIONED SPACE
3) ALL DUCTS, AIR HANDLERS, FILTER BOXES, AND BUILDING CAVITIES USED AS DUCTS SHALL BE SEALED AS PER N1103.2 OF THE 2018 IRC

VALUES BELOW ARE PER 2018 IECC, ACTUAL VALUES MAY VARY BASED ON ALTERNATE ENERGY COMPLIANCE PATH CHOSEN (IN JURISDITIONS WHERE ALTERNATIVE PATHS ARE AVAILABLE)

CATHEDRAL / VAULTED CEILING FRAMING AND INSULATION

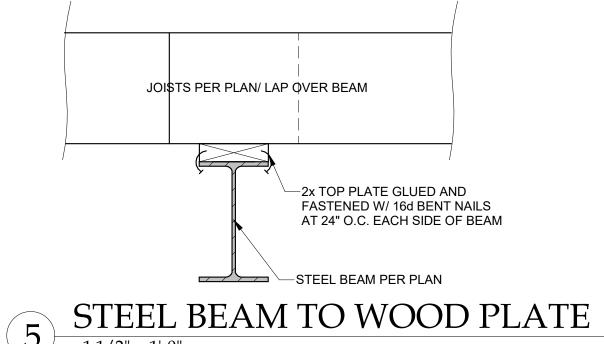
MINIMUM R-38 INSULATION REQUIRED, SEE DETAIL 14/S-1.2

WHERE THE CEILING IS APPLIED DIRECTLY TO THE BOTTOM OF THE RAFTERS, A MINIMUM 1" AIR SPACE SHALL BE PROVIDED BETWEEN THE TOP OF THE INSULATION AND THE SHEATHING FOR VENTILATION (R806.3) NOTE: RAFTER SIZES SPECIFIED ON PLANS ARE THE MINIMUM REQUIRED FOR STRUCTURAL PURPOSES ONLY.

IF FULL RAFTER DEPTH IS NOT ADEQUATE FOR MINIMUM INSULATION VALUE, RAFTER SIZES WILL NEED TO BE INCREASED, OR ADEQUATE FURRING SHALL BE USED TO OBTAIN THE MINIMUM JOIST DEPTH FOR THE REQUIRED INSULATION. IN ADDITION, IF THE RAFTER SIZE IS INCREASED IT SHALL BE VERIFIED THAT THE RIDGE BE A MINIMUM OF ONE NOMINAL SIZE LARGER THAN THE RAFTERS BEING RECEIVED. (SEE CHART BELOW)

MAXIMUM INSULATION VALUE	2x6	2x8	2x10	2x12
1" AIR SPACE (FIBERGLASS)	R-13, 3 1/2"	R-19, 6 1/4"	CONDENSED R-38, 8 1/4"	R-38, 10 1/4"

-JOIST SECTION TO BE REMOVED FOR SHOWER PAN -APA SHEATHING FILLER -JOIST SECTION TO BE REMOVED -1.75"x9.25" LVL (11-7/8" I-JOIST FLOOR) FOR SHOWER PAN 1.75"x7.25" LVL (9-1/2" I-JOIST FLOOR) -1.75"x9.25" LVL (2X12 FLOOR) * SISTER TO RUN FULL 1.75"x7.25" LVL (2X10 FLOOR) LENGTH OF FLOOR JOIST TO BE ALTERED SISTER TO RUN FULL LENGTH OF FLOOR JOIST TO BE ALTERED -12D 16" O.C FROM THIS SIDE -12D 16" O.C FROM THIS SIDE -JOIST PER PLAN ZERO ENTRY SHOWER DETAIL



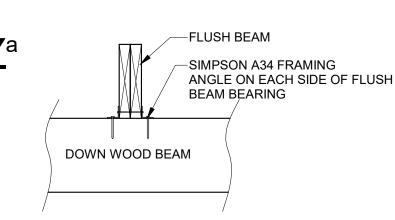
STEEL BEAM TO WOOD PLATE 1 1/2" = 1'-0"

TABLE N1103.6.1 (R403.6.1) WHOLE-HOUSE MECHANICAL VENTILATION SYSTEM FAN EFFICACY

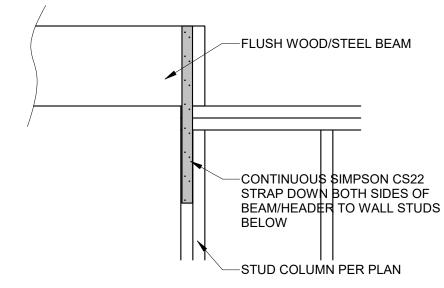
FAN LOCATION	AIR FLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY (CFM/WATT)	AIR FLOW RATE MAXIMUM (CFM)
HRV OR ERV	ANY	1.2 CFM/WATT	ANY
RANGE HOODS	ANY	2.8 CFM/WATT	ANY
IN-LINE FAN	ANY	2.8 CFM/WATT	ANY
BATHROOM, UTILITY ROOM	10	1.4 CFM/WATT	< 90
BATHROOM, UTILITY ROOM	90	2.8 CFM/WATT	ANY

For SI: 1 cubic foot per minute = 28.3 L/min.

WHEN TESTED IN ACCORDANCE WITH HVI STANDARD 916

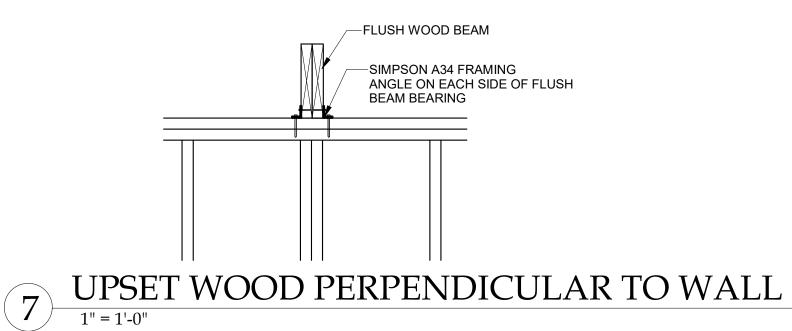


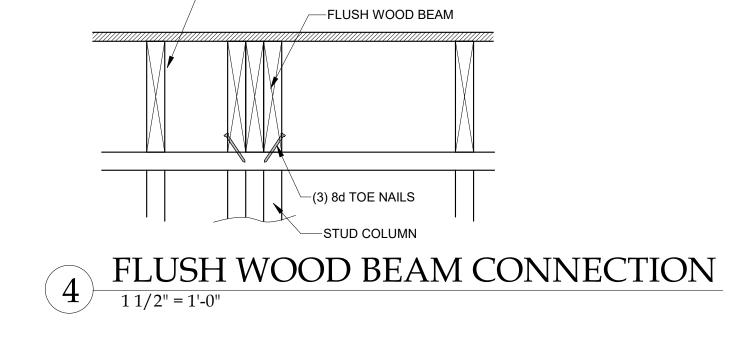
WOOD TO WOOD STACKED CONNECTION

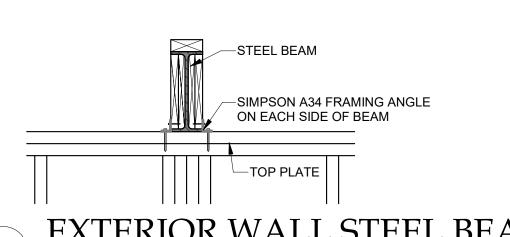


8 UPSET WOOD/STEEL PARALLEL TO WALL

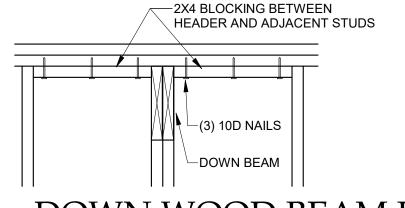
1" = 1'-0"



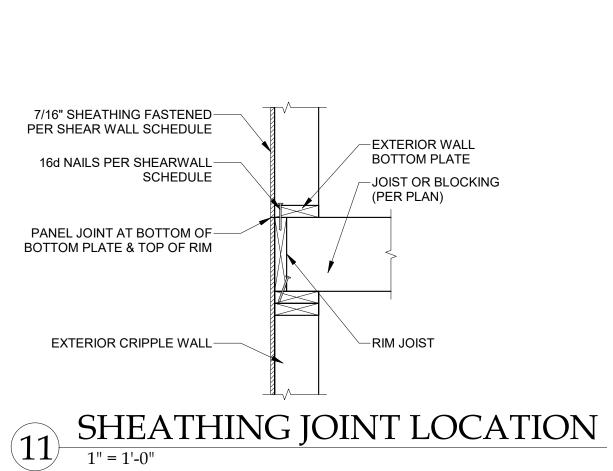


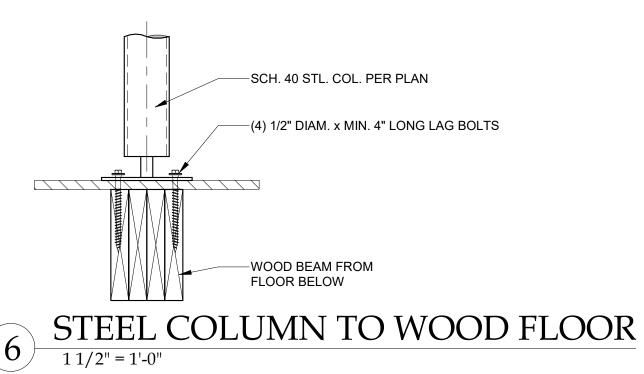


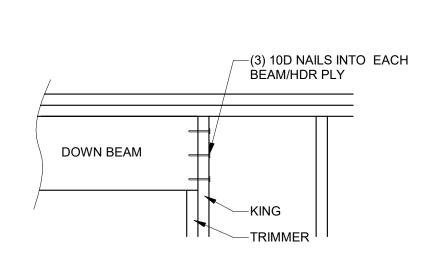
EXTERIOR WALL STEEL BEAM BEARING



2 DOWN WOOD BEAM PERPENDICULAR







DOWN WOOD BEAM PARALLEL

1" = 1'-0"

GENERAL DETAILS

CHRIS SAATHOFF 2008001865 COMMINGHING SAATHOFF SAATHOFF CONTROLL S

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HD#: 48058

DATE: 8/6/2024

NO. ISSUE/REVISION Revision Date

S-4.0