

RE: P241025-01 - Roof - HM Lot 180 Site Information: Project Customer: Clayton Properties Project Nam Lot/Block: 180 Subdivis Model:	MiTek, Inc. 16023 Swingley Ridge Rd. e: Basswood - Farmhouse 3Caff4.434.1200 sion: Highland Meadows
Address: 1102 SW Fiord Dr	
City: Lee's Summit State: M	10
General Truss Engineering Criteria & Design Load	ds (Individual Truss Design
Drawings Show Special Loading Conditions):	
Design Code: IRC2018/TPI2014 Wind Code: ASCE 7-16	Design Program: MiTek 20/20 8.6 Design Method: MWFRS (Envelope)/C-C hybrid Wind ASCE 7-16
Wind Speed: 115 mph	Floor Load: N/A psf
Roof Load: 45.0 psf	•
Mean Roof Height (feet): 35	Exposure Category: C
No. Seal# Truss Name Date 1 168681679 A1 10/7/24 2 168681680 A2 10/7/24	

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Premier Building Supply (Springhill, KS)20300 W 207th Street.

Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of Missouri is December 31, 2025.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



													RELEAS	E FOR CONSTRUCTION	
Job		Truss		Truss Ty	/pe		Qty		Ply	Roof - H	IM Lot 18)		ED FOR PLAN REVIEW	
P241025-01		A1		Commo	on Supported	Gable	1		1		erence (o	otional		LOPMENT SERVICES 168681679 SUMMIT, MISSOURI	
Premier Building	Supply (Spring	hill, KS), S	pring Hills, KS - 66083,			Run: 8.63 S Sep 26 ID:NyJzGF_AVBEW				26 2024 Mi	ek Industrie	es, Inc. ¹		21/2024	
	-0-1	1-0		11	-0-4						22-0	0		22-11-8	
	0-1	1-0			-0-4						11-0			0-11-0	
							4x4 =	-							
							7								
0-10-0	- 1	2	3 26	4	12 27 5				8		28	10	29 11	12 13	
			24	23	22	21 20	2000 19	****	**************************************	<u>*******</u> 17	******	<u>*****</u> 16	**************************************		
						3x4 =									
		0-1 _ī 12				2	1-10-1	2						22 ₁ 0-8	
		0-1-12					21-9-0							0-1-12	
Scale = 1:42.3															
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDI		(psf) 25.0 25.0 10.0 0.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018	3/TPI2014	BC	0.15 0.06	,	L) CT)		- n/a - n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190 ET = 20%	
Snow (Pf) 25.0 Lumber DOL 1.15 BC 0.06 Vert(CT) n/a - n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.07 Horz(CT) 0.00 14 n/a n/a									ain formula. Building bearing surface. y others) of truss to ing 66 lb uplift at joint ift at joint 21, 52 lb 23, 81 lb uplift at joint ift at joint 17, 42 lb oint 15. ce with the 2018 tions R502.11.1 and d ANSI/TPI 1.						
TOP CHORD	TOP CHORD 2-25=-185/131, 1-2=0/40, 2-3=-70/57, 3-4=-51/94, 4-5=-60/128, 5-6=-72/164, 6-7=-85/200, 7-8=-85/194, 8-9=-73/150, 9-10=-60/114, 10-11=-51/80, 11-12=-62/45, 12-13=0/40, 12-14=-185/125 SEVIER All plates are 1.5x4 MT20 unless otherwise indicated. 6 Gable requires continuous bottom chord bearing. 9 Truss to be fully sheathed from one face or securely braced acquired teared movement (i.e. diagonal web)									LER 018807					

October 7,2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

									RELEAS	E FOR CONSTRUCTION	N
Job	Truss		Truss Type		Qty	Ply	Roof - HM	Lot 180		ED FOR PLAN REVIEW LOPMENT SERVICES 168681680	
P241025-01	A2		Common		4	1	Job Refere	ence (optional	1 5 5 10	SUMMIT, MISSOURI	
Premier Building Supply (Springhill, KS), S	Spring Hills, KS - 66083,		Run: 8.63 S Se ID:r9tMTb2oGU					hu Oct 03 15:50:51 WrCDoi7J4zJC?	21/2024	1
				15.1910115:000			or on iquitogra		WICD0173423011		
	-0-11-0	5-11-6		11-0-4		16-1-		-	22-0-8	22-11-8	
	0-11-0	5-11-6	Į	5-0-14	ļ	5-0-1	4	I	5-11-6	b-11-d	
					4x4 =						
			1 <u>2</u> 4 [4						
			4x4					4x4 🕿			
			3		//			5			
4-6-1		13	- Et		/			T	14		
4	_	A		\sim						8x8 =	
e⊤	1 2									6 7	
0-10-0	12						¥⁄				
	Ř			11 10)		9			Ĭ	
	8x8 =			3x4 = 3x	×4 =		3x4 =				
	0 4 40									~ ~ ~	
	0-1-12	7-1(7-9			<u>14-1-11</u> 6-2-13				1-10-12 7-9-1	22-0-8	
Scale = 1:42.3	0-1-12				0210				7.5.1	0-1-12	
Plate Offsets (X, Y): [6:0-4-8,0-2-12], [12:0-4-8,0-2-12]									_
Loading	(psf)	Spacing	2-0-0	CSI		EFL	in (loc)	l/defl L/d		GRIP	
TCLL (roof) Snow (Pf)	25.0 25.0	Plate Grip DOL Lumber DOL	1.15 1.15	TC BC		. ,	11 11-12 23 11-12	>999 240 >999 180		197/144	
TCDL BCLL	10.0 0.0	Rep Stress Incr Code	YES IRC2018/TPI2	WB 014 Matrix-S	0.58 Ho	orz(CT) 0.	06 8	n/a n/a			
BCDL	10.0	Couc							Weight: 94 lb	FT = 20%	
LUMBER TOP CHORD 2x4 SI				.: ASCE 7-16; Pr=25.0 ps DOL=1.15); Pf=25.0 psf							
BOT CHORD 2x4 SI	P No.2		DOL	=1.15); Is=1.0; Rough Ca							
WEBS 2x3 SI BRACING	PF No.2 ^Exce	pt* 12-2,8-6:2x4 SP №	4) Unba	alanced snow loads have	been conside	ered for this					
		athing directly applied cept end verticals.	5) This	truss has been designed							
	ceiling directly	applied or 8-10-3 oc		of 12.0 psf or 2.00 times nangs non-concurrent wit							
WEBS 1 Row	at midpt	3-12, 5-8		truss has been designed d live load nonconcurrent							
	8=0-5-8, 1 oriz 12=-54 (L	C 21)	capa	earings are assumed to b city of 565 psi.	e SP No.2 cr	ushing					
		C 13), 12=-220 (LC 1 _C 24), 12=1116 (LC :	²⁾ 8) Bear	ing at joint(s) 12, 8 consid ANSI/TPI 1 angle to gra							
	/laximum Com	pression/Maximum	desig	gner should verify capacit ide mechanical connection	y of bearing s	surface.					
TOP CHORD 1-2=0/	/40, 2-3=-582/2	210, 3-4=-1747/469, 582/211, 6-7=0/40,	bear	ing plate capable of withs	tanding 220 I						
2-12=	-450/274, 6-8=	-450/274	10) This	12 and 220 lb uplift at join truss is designed in acco	rdance with t						
8-9=-4	100/1833	-11=-243/1270,	R802	national Residential Code 2.10.2 and referenced sta							
3-11=-	-413/206, 3-12	13/206, 4-11=-87/565 2=-1466/367,	, LOAD C	ASE(S) Standard					STORE OF	A DE	
5-8=-1 NOTES	466/367							- 3	TEOT	MISSOL	
 Unbalanced roof li this design. 	ve loads have	been considered for						E	SCOT	TM. Yr	
 Wind: ASCE 7-16; 		(3-second gust) DL=6.0psf; h=35ft;							SEV		
Ke=1.00; Cat. II; E	xp C; Enclose	d; MWFRS (envelope)						att.	Conver	>
Interior (1) 4-1-0 to	o 11-0-4, Exter	E) -0-11-0 to 4-1-0, ior(2R) 11-0-4 to 16-2	-6,					K	NUM PE-2001	018807	•
right exposed ; en	d vertical left a	ne; cantilever left and nd right exposed;C-C						Ŷ	A The	1SB	
for members and f Lumber DOL=1.60		RS for reactions show L=1.60	n;						SIONA	L EN	
	- •								alle	er 7.2024	

October 7,2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

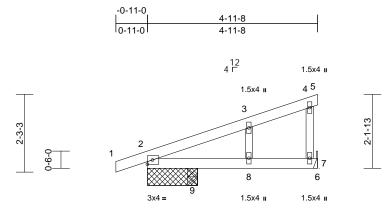
												RELEA	SE FOR CONSTRUCTION
Job	Truss		Truss T	уре		Qty		Ply	Roof - HM	Lot 180			TED FOR PLAN REVIEW ELOPMENT SERVICES 168681681
P241025-01	A3		Comm	on		1		1	Job Refer	ence (op	tional		SUMMIT, MISSOURI
Premier Building Supply (S	pringhill, KS), S	Spring Hills, KS - 66083,			Run: 8.63 S S ID:r9tMTb?oG							hu Oct 03 155 WrCDoi7J4zJ9?	
	ļ	5-11-6		1	11-0-4			16-1-2		-		22-0-8	22-11-8
	I	5-11-6		I	5-0-14	I		5-0-14		I		5-11-6	0-11-0
						4x4 =							
			1 <u>2</u> 4 [3							
										44			
			4	x4 =		$// \searrow$			_	4x4 ≈ 4			
-		12	13 2	1	/	/ \	$\langle \rangle$			4		14	
4-6-1	8x8 =			9				\					8x8 =
	1	A						$\langle \rangle$					5
	FT												6
⊥ 5⊥ 11				¥				<u></u>	Ľ				
				1				8					
				3	3x4 = 3x4	=		3	5x4 =				
0	1 10												22.0.8
0	-1-12 -1-12	7-10-13 7-9-1				<u>14-1-11</u> 6-2-13						0-12 9-1	22-0-8 0-1-12
Scale = 1:41.1	-1-12	-										-	0-1-12
Plate Offsets (X, Y): [1]	:0-4-8,0-2-12	2], [5:0-4-8,0-2-12]	-		1								
Loading TCLL (roof)	(psf) 25.0	Spacing Plate Grip DOL	2-0-0 1.15		CSI TC	0.58	DEFL Vert(LL) -0	in (loc) .11 7-8	l/defl >999	L/d 240	PLATES MT20	GRIP 197/144
Snow (Pf)	25.0	Lumber DOL	1.15		BC	0.64	Vert(C	́Г) -0.	.23 7-8	>999	180	WIT20	131/144
TCDL BCLL	10.0 0.0	Rep Stress Incr Code	YES IRC201	8/TPI2014	WB Matrix-S	0.62	Horz(C	(I) U.	.06 7	n/a	n/a		
BCDL	10.0											Weight: 93 lt	ET = 20%
LUMBER TOP CHORD 2x4 SP	No.2		3)		E 7-16; Pr=25.0 ∣ 1.15); Pf=25.0 p								
BOT CHORD 2x4 SP	No.2	ept* 11-1,7-5:2x4 SP I	No 2	DOL=1.15); Cs=1.00; Ct	Is=1.0; Rough C =1.10	Cat C; Fully	Exp.; Ce	e=0.9;					
BRACING		. ,	4)		snow loads hav	e been con	sidered	for this					
3-5-2 o		eathing directly applied accept end verticals.	d or 5)	This truss ha	as been designe psf or 2.00 time								
BOT CHORD Rigid co bracing	• •	/ applied or 8-9-6 oc		overhangs r	ion-concurrent w	vith other liv	e loads.		I				
WEBS 1 Row a REACTIONS (size)	at midpt 7=0-5-8.	2-11, 4-7 11-0-5-8	6)	chord live lo	as been designe ad nonconcurre	nt with any	other live	e loads.					
Max Hor	iz 11=-60 (L	_C 17)	7)	All bearings capacity of s	are assumed to 565 psi.	be SP No.2	2 crushir	ng					
		_C 13), 11=-171 (LC 1 LC 24), 11=1040 (LC			pint(s) 11, 7 cons TPI 1 angle to g								
FORCES (lb) - Ma Tensior		npression/Maximum	9)		ould verify capac chanical connect								
	30/168, 2-3= 749/485 4-5	-1761/502, =-582/210, 5-6=0/40,	- /	bearing plat	e capable of with 221 lb uplift at jo	nstanding 1	,						
1-11=-3	324/175, 5-7		10)) This truss is	designed in acc Residential Co	ordance wi							
7-8=-41	5/1836			R802.10.2 a	nd referenced s								
2-10=-4	29/213, 2-1	14/206, 3-10=-94/578 1=-1589/420,	^{3,} L0	DAD CASE(S)	Standard							Contraction of the	A COM
4-7=-14 NOTES	109/382										4	TEOI	MISSOL
 Unbalanced roof live this design. 	e loads have	been considered for									A	× /	M. T.
 Wind: ASCE 7-16; V Vasd=91mph; TCD 											Ba	SE SE	VIER
Ke=1.00; Cat. II; Ex	p C; Enclose	ed; MWFRS (envelope	e)							6	×	ott	Serly
Interior (1) 5-1-12 to	o 11-0-4, Ext		1-4								83		MBER 01018807
and right exposed ;	end vertical		left								Q	The second	158
exposed;C-C for me reactions shown; Lu		forces & MWFRS for 1.60 plate grip										S'SION	AL EN
DOL=1.60												all a	
												Ocit	ber 7,2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent toules bible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

Job	Truss	Truss Type	Qty	Ply	Roof - HM Lot 180	
P241025-01	B1	Monopitch Supported Gable	1	1	Job Reference (optional	

Run: 8.63 E Aug 30 2023 Print: 8.630 E Aug 30 2023 MiTek Industries, Inc. Mon Oct 01 1 21 83 ID:OaSecMxhRSfQIAVWAXas1vylfDC-M4Moi1kZoPfV9dC_O3VNKmdAUB2DRNgVsqqym2Vxoo





Scale = 1:33.6

Loading	(psf)	Spacing	1-11-4		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15		TC	0.21	Vert(LL)	0.02	7-8	>999	240	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15		BC	0.35	Vert(CT)	-0.03	7-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES		WB	0.07	Horz(CT)	0.00	7	n/a	n/a		
BCLL	0.0	Code	IRC2018	/TPI2014	Matrix-P								
BCDL	10.0											Weight: 19 lb	FT = 20%
	Max Horiz 2=84 (LC Max Uplift 2=-40 (LC 9=-53 (LC	cept end verticals. applied or 10-0-0 or 7= Mechanical, 9=0- 12) 2 12), 7=-46 (LC 16) 2 16)	6) 7) ed or 8) 9) c 3-8 10) ,	load of 12.0 overhangs n Gable studs This truss ha chord live loa Refer to gird Provide mec bearing plate 7 and 53 lb u N/A This truss is International	is been designed psf or 2.00 times on-concurrent wit spaced at 2-0-0 c is been designed ad nonconcurrent er(s) for truss to t hanical connectio c capable of withs uplift at joint 9. designed in accoo Residential Code nd referenced sta	flat roof lo h other liv oc. for a 10.0 with any russ conr on (by oth tanding 4 rdance w e sections	bad of 25.0 p ve loads. D psf bottom other live loa nections. ers) of truss i 6 lb uplift at j ith the 2018 i R502.11.1 a	ads. to joint					
	Max Grav 2=171 (L0 9=293 (L0),										
FORCES	(lb) - Max. Comp./M (lb) or less except w	ax. Ten All forces	250										
WEBS	3-8=-222/268	nen snown.											
NOTES													
Vasd=91rr Ke=1.00; (exterior zo Exterior(21 right expos members	CE 7-16; Vult=115mph nph; TCDL=6.0psf; BC Cat. II; Exp C; Enclose one and C-C Corner(3I N) 4-1-0 to 4-11-8 zon sed ; end vertical left e and forces & MWFRS OL=1.60 plate grip DC	DL=6.0psf; h=35ft; d; MWFRS (envelop E) -0-11-0 to 4-1-0, e; cantilever left and xposed;C-C for for reactions shown									la l	STATE OF I	1 CAN

Lumber DOL=1.60 plate grip DOL=1.60 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.



October 7,2024

SEVIER

PE-2001018807

OFFESSIONAL

BE

E

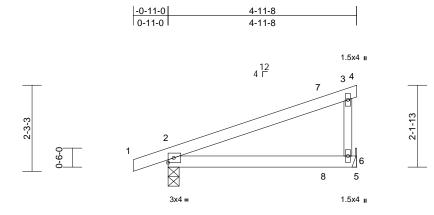
RELEASE FOR CONSTRUCTION AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES 168681682 LEE'S SUMMIT, MISSOURI

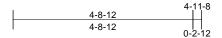
1/202

 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to dury with with where outputs into design is based only door parameters shown, and is for an individual building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH1 Quality Criteria**, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)

						RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type	Qty	Ply	Roof - HM Lot 180	AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES 168681683
P241025-01	B2	Monopitch	3	1	Job Reference (optional	

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Thu Oct 05 15 31 2 1/2 2 1





Scale = 1:30.3

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 25.0 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC201	8/TPI2014	CSI TC BC WB Matrix-P	0.62 0.48 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.10 0.08 0.00	(loc) 2-6 2-6 6	l/defl >549 >651 n/a	L/d 240 180 n/a		GRIP 197/144 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD BOT CHORD BOT CHORD BOT CHORD NOTES 1) Wind: ASC Vasd=91m Ke=1.00; (exterior zo Interior (1)	2x4 SP No.2 2x4 SP No.2 2x3 SPF No.2 Structural wood she 5-0-0 oc purlins, ex Rigid ceiling directly bracing.	cept end verticals. applied or 10-0-0 or 6= Mechanical 12) C 12), 6=-106 (LC 1 C 23), 6=299 (LC 23 pression/Maximum 64, 3-4=-7/0, (3-second gust) DL=6.0psf; h=35ft; d; MWFRS (envelop E) -0-11-0 to 4-1-0, cantilever left and ri	c 9; (2) 11) L De)	 chord live lo Bearings are capacity of 5 Refer to gird Provide mec bearing plate joint 6. One H2.5T \$ recommend, UPLIFT at jt does not cor This truss is International 	ler(s) for truss to the capable of withs Simpson Strong-T ed to connect trus (s) 2. This connect nsider lateral force designed in accord Residential Code nd referenced states the states the connect trus (s) 2. This connect trus (s) 2. This connect trus (s) 2. This connect trus (s) 2. This connect (s) 2. T	t with any Joint 2 SI truss conr on (by oth standing 1 Tie conner ss to bear ction is for es. ordance w e sections	other live loa No.2 crushi nections. ers) of truss i 06 lb uplift ar ctors ing walls due uplift only ar ith the 2018 R502.11.1 a	ing to to nd					
 reactions s DOL=1.60 2) TCLL: ASC Plate DOL DOL=1.15 Cs=1.00; 0 	CE 7-16; Pr=25.0 psf (=1.15); Pf=25.0 psf (L ;; Is=1.0; Rough Cat C	1.60 plate grip roof LL: Lum DOL=1 um DOL=1.15 Plate C; Fully Exp.; Ce=0.9	1.15 9;							4	8	STATE OF SCOT	Sante

- Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.



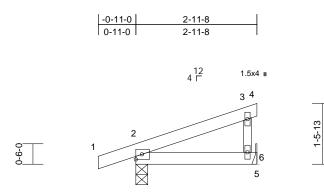
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200 / MiTek-US.com

						RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type	Qty	Ply	Roof - HM Lot 180	AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES 168681684
P241025-01	B3	Monopitch	7	1	Job Reference (optional)	

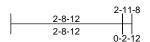
1-7-3

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Thu Oct 05 15 St. 21/2024 ID:OfYMtcmLQLDnSdia9popoFylfC8-RfC?PsB70Hq3NSgPqnL8w3ulTXbGK VrCDoi7J4zJ54



3x4 =





Scale = 1:28.2

Load TCLL Snow TCDI BCLL BCDI	_ (roof) v (Pf) _	(psf) 25.0 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018	3/TPI2014	CSI TC BC WB Matrix-P	0.16 0.07 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 -0.01 0.00	(loc) 2-6 2-6 6	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 11 lb	GRIP 197/144 FT = 20%
LUM TOP BOT BOT BOT REAT TOP BOT REAT TOP BOT 1) V V K c c c c c c c c c c c c c c c c c	BER CHORD CHORD S CING CHORD CHORD CTIONS CES CHORD CHORD CHORD CHORD ES Vind: ASC (Asd=91m (ce=1.00; (c) vixterior zo ond right e exposed; C eactions s 00L=1.60 CCL: ASC 00L=1.15 Ss=1.00; (Jinbalance lesign. This truss	2x4 SP No.2 2x4 SP No.2 2x3 SPF No.2 Structural wood she 3-0-0 oc purlins, ex. Rigid ceiling directly bracing. (size) 2=0-3-8, 6 Max Horiz 2=57 (LC Max Uplift 2=-78 (LC Max Grav 2=281 (LC (lb) - Maximum Com Tension 1-2=0/13, 2-3=-83/4 2-6=-23/25, 5-6=0/0 CE 7-16; Vult=115mph ph; TCDL=6.0psf; BC Cat. II; Exp C; Enclose ne and C-C Exterior(2 :xposed ; end vertical I C-C for members and f shown; Lumber DOL= ⁻¹ CE 7-16; Pr=25.0 psf (L =1.15); Pf=25.0 psf (L =1.10 ed snow loads have be has been designed for	cept end verticals. applied or 10-0-0 or 6= Mechanical 13) 212), 6=-30 (LC 16) 223), 6=161 (LC 23) apression/Maximum 7, 3-4=-7/0, 3-6=-13 (3-second gust) EDL=6.0psf; h=35ft; d; MWFRS (envelop 2E) zone; cantilever I left and right orces & MWFRS for 1.60 plate grip roof LL: Lum DOL=1 um DOL=1.15 Plate C; Fully Exp.; Ce=0.9 een considered for th r greater of min roof	9) 10 10 10 LC 1/144 De) eft 1.15 0; his live	capacity of 5 Refer to gird Provide mec bearing plate 6. One H2.5T S recommende UPLIFT at jt(does not com) This truss is International	er(s) for truss to tru- hanical connection capable of withste Simpson Strong-Tie d to connect truss s) 2. This connect sider lateral forces designed in accord Residential Code and referenced stan	uss conr (by oth anding 3 connecto to bear ion is for s. dance w sections	nections. ers) of truss 80 lb uplift at ctors ing walls due r uplift only a ith the 2018 5 R502.11.1 a	to joint e to nd				STATE OF J	MISSOLUTION T.M. BER
с 5) Т	overhangs This truss	0 psf or 2.00 times fla non-concurrent with o has been designed for load nonconcurrent wi	other live loads. r a 10.0 psf bottom									Ø	FR SSIONA	L ENGINE

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



									RELEASE		1
Job	Truss	S	Truss Type		Qty	Ply	Roof - HM Lo	ot 180		ED FOR PLAN REVIEW	
P241025-02	I C1		Common Structura	l Gable	1	1	Job Reference	ce (optional	LEE'S	OPMENT SERVICES 168681685 SUMMIT, MISSOURI	
Premier Building	Supply (Springhill, KS),	, Spring Hills, KS - 66083,		Run: 8.63 S Sep 26 ID:gfrsVxpQoNOr6c		•	26 2024 MiTek Ind	dustries, Inc.	hu Oct 03 15:50:51	21/2024	1
					KUUQOW		Bronqorogi qili	-000301170501	WICD01/34236 H		
				4-11-4 4-11-4		9-8-8		-7-8			
			' '	4-11-4		4-9-4	0-1	11-0			
				12 8	x4 =						
				1.5x4 u 4	1						
Scale = 1:39.4			2 1 3x6 II			1.5x4 II 5 9 1.5x4 II <u>9-8-8</u> 4-9-4		13 7 3	0-10-3		
	X, Y): [1:0-3-0,0-0-1	1]									_
Loading TCLL (roof) Snow (Pf)	(psf) 25.0 25.0	Spacing Plate Grip DOL Lumber DOL	1-11-4 1.15 1.15	BC	0.16 V 0.06 V	ert(CT) 0	.00 1-11 > .00 1-11 >	/defl L/d >999 240 >999 180	PLATES MT20	GRIP 244/190	
TCDL BCLL	10.0 0.0	Rep Stress Incr Code	YES IRC2018/TPI2014	WB Matrix-S	0.08 H	orz(CT) 0	.00 8	n/a n/a			
BCDL	10.0								Weight: 48 lb	FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS OTHERS SLIDER BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x4 SPF No.3 *Exc 2x4 SPF No.3 Left 2x4 SP No.2 - Structural wood sh 6-0-0 oc purlins, e Rigid ceiling direct bracing. (size) 1=9-8-8 10=9-8-8 10=9-8-8 Max Horiz 1=116 (I Max Uplift 1=-50 (L 9=-103 11=-117 Max Grav 1=187 (I 9=338 (I	neathing directly applied except end verticals. ly applied or 6-0-0 oc , 8=9-8-8, 9=9-8-8, 8, 11=9-8-8 LC 15) LC 17), 8=-63 (LC 17), (LC 17), 10=-9 (LC 15), 7 (LC 16) LC 23), 8=284 (LC 24), LC 24), 10=144 (LC 29)	Vasd=91mp Ke=1.00; Cc exterior zom Corner(3R) 10-7-8 zone vertical left a forces & MV DOL=1.60 p 3) Truss desig only. For st see Standar or consult q 4) TCLL: ASCI Plate DOL= DOL=1.15); Cs=1.00; Ct 5) Unbalanced design.	E 7-16; Vult=115mph h; TCDL=6.0psf; BCI at. II; Exp C; Enclosed e and C-C Corner(3E 4-11-4 to 9-11-4, Exte ; cantilever left and right exposed;C-C VFRS for reactions sh vlate grip DOL=1.60 ned for wind loads in uds exposed to wind d Industry Gable End ualified building desig Z 7-16; Pr=25.0 psf (L 1.15); Pf=25.0 psf (L 1.15); Rough Cat C =1.10 snow loads have been as been designed for	DL=6.0ps ; MWFR) 0-0-0 to erior(2N) ght expose C for men- iown; Lur the planet (normal t I Details a iner as per- oof LL: L im DOL= ; Fully Ex-	f; h=35ft; S (envelope) o 4-11-4, 9-11-4 to sed ; end nbers and mber e of the truss o the face), as applicable, er ANSI/TPI 1. um DOL=1.15 1.15 Plate p.; Ce=0.9; lered for this	i				
FORCES	11=382 (lb) - Maximum Co Tension	(LC 23) mpression/Maximum	overhangs r	psf or 2.00 times flat non-concurrent with of spaced at 2-0-0 oc.			ı				
TOP CHORD		=-136/221, 4-5=-137/21		as been designed for	a 10.0 ps	sf bottom			Same	ADD	

 TOP CHORD
 1-3=-134/109, 3-4=-136/221, 4-5=-137/219, 5-6=-123/98, 6-7=0/69, 6-8=-262/208

 BOT CHORD
 1-11=-27/56, 10-11=-27/56, 9-10=-27/56, 8-9=-27/56

 WEBS
 4-10=-150/56, 3-11=-318/265, 5-9=-286/231

NOTES

1) Unbalanced roof live loads have been considered for this design.

 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

10) N/A

 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent college with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200 / MiTek-US.com

			•						
Job	Truss		Truss Type		Qty	Ply	Roof - HM Lot	180	AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES 168681686
P241025-01	C2		Common		1	1	Job Reference	(optional	
Premier Building Supply (Springhill, KS), S	pring Hills, KS - 66083					26 2024 MiTek Indu 70Hq3NSgPqnL8w3		
				4-11-4 4-11-4		<u>9-8-8</u> 4-9-4		-11-0	
					4x4 = 2				
	4-1-11	0-8-13	6x6 =	812	6 3x8=		3 10x10	8	0-10-3
Scale = 1:37	4.0.0.0.5.4			<u>4-11-4</u> 4-11-4		<u>9-8-8</u> 4-9-4			
Plate Offsets (X, Y): [1:0-3-0,0-5-4],	[5:Edge,0-8-2]							
Loading	(psf)	Spacing	2-0-0	CSI		EFL	in (loc) l/c		PLATES GRIP

		1 3											
Loading TCLL (roof) Snow (Pf) TCDL BCLL	(psf) 25.0 25.0 10.0 0.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES	8/TPI2014	CSI TC BC WB Matrix-S	0.60 0.22 0.11	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.03 -0.02 0.00	(loc) 6-7 6-7 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL	10.0	Code	11/0201	0/1112014	Matrix-5							Weight: 45 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Unbalance this desig 2) Wind: AS	 2x4 SP No.2 2x4 SP No.2 2x3 SPF No.2 *Exce Structural wood she 5-9-13 oc purlins, e Rigid ceiling directly bracing. (size) 5=0-3-0, 7 Max Horiz 7=-114 (L Max Uplift 5=-84 (LC Max Grav 5=638 (LC (lb) - Maximum Com Tension 1-2=-550/461, 2-3=- 1-7=-507/346, 3-5=- 6-7=-219/317, 5-6=- 2-6=-317/187, 1-6=- ced roof live loads have 	athing directly applie xcept end verticals. applied or 10-0-0 or 7=0-5-8 C 12) C 17), 7=-58 (LC 16) C 24), 7=50 (LC 23) pression/Maximum 540/469, 3-4=0/71, 597/422 195/322 74/197, 3-6=-106/24 been considered fo (3-second gust)	ed or 7) c 8) 9)) L(load of 12.0 overhangs n This truss ha chord live lo All bearings capacity of § One H2.5T s recommend UPLIFT at jt and does nc This truss is International	Simpson Strong- ed to connect tru (s) 7 and 5. This of consider lateral designed in acco I Residential Cod nd referenced sta	s flat roof lo vith other liv d for a 10.0 th with any be SP No. Tie connect sss to bear connectio al forces. ordance w de sections	bad of 25.0 p ve loads.) psf bottom other live loa 2 crushing ctors ing walls due n is for uplift ith the 2018 s R502.11.1 a	eds. eds. e to only				THE OF I	
Ke=1.00; exterior z Exterior(2 10-7-8 zo vertical rig for memb	Cat. II; Exp C; Enclose tone and C-C Exterior(2 2R) 4-11-4 to 9-11-4, In one; cantilever left and r ght exposed; porch left vers and forces & MWF DOL=1.60 plate grip DC	d; MWFRS (envelop E) 0-1-12 to 4-11-4, terior (1) 9-11-4 to ight exposed ; end and right exposed;CRS for reactions sho	с-С							ė		SCOT SEV	I M. HER Server
3) TCLL • 49	SCE 7-16: Pr-25 0 nsf (roof LL · Lum DOL -	1 15								17 2		ANG

Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

PE-200101800.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



]		FOR CONSTRUCTION
Job	Truss	Truss Type		Qty	Ply	Roof - HM	Lot 180		OPMENT SERVICES 168681687
P241025-01	C3	Common		1	1	Job Refere	ence (optional		SUMMIT, MISSOURI
Premier Building Supply ((Springhill, KS), Spring Hills, KS - 66083	3,	Run: 8.63 S Sep 2 ID:bBOxdUdBwZC	.6 2024 Prin ≀GDBIEEzD	t: 8.630 S Sep : _0KylfMf-RfC?F	26 2024 MiTek I	Industries, Inc.	hu Oct 03 15:5):51 KWrCDoi7J425C?f	21/2024
			4-5-12	<u> </u>	9-3-0		10-2-0		
		4	4-5-12	_	4-9-4	i	0-11-0		
	4-1-11 4-0-5		2 8 4-1-0 6			0-4-13		0-10-3	
Scale = 1:37			4-5-12 4-5-12		<u>9-3-0</u> 4-9-4		1		
Loading TCLL (roof) Snow (Pf) TCDL BCLL	(psf)Spacing25.0Plate Grip DOL25.0Lumber DOL10.0Rep Stress Incr0.0Code	2-0-0 1.15 1.15 YES IRC2018/TPI2014	CSI TC BC WB Matrix-R	0.47 Ve 0.31 Ve	ert(CT) -0	in (loc) 0.04 5-6 0.04 5-6 0.00 5	l/defl L/d >999 240 >999 180 n/a n/a		GRIP 244/190 FT = 20%

- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WFBS 2x4 SP No.2 *Except* 6-2:2x3 SPF No.2 BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. **REACTIONS** (size) 5=0-3-0, 7= Mechanical Max Horiz 7=-113 (LC 12) Max Uplift 5=-82 (LC 17), 7=-53 (LC 16) Max Grav 5=597 (LC 24), 7=514 (LC 23) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=-475/433, 2-3=-488/427, 3-4=0/71, TOP CHORD 1-7=-454/330, 3-5=-549/400 BOT CHORD 6-7=-215/270, 5-6=-215/270 WEBS 2-6=-293/171 NOTES
- Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 4-5-12, Exterior(2R) 4-5-12 to 9-5-12, Interior (1) 9-5-12 to 10-2-0 zone; cantilever left and right exposed ; end vertical right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearings are assumed to be: , Joint 5 SP No.2 crushing
- capacity of 565 psi.8) Refer to girder(s) for truss to truss connections
- 8) Refer to girder(s) for truss to truss connections.
 9) Provide mechanical connection (by others) of tru
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 7.
- One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard







							RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type		Qty	Ply	Roof - HM Lot 180	AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES 168681688
P241025-01	D1	Common S	Common Structural Gable		1	Job Reference (optiona	
Premier Building Supply	(Springhill, KS), Spring Hills, KS	S - 66083,				p 26 2024 MiTek Industries, Inc. PsB70Hq3NSgPqnL8w3uITXb0	
		-0-11-0	6-10-0			13-8-0	14-7-0
		0-11-0	6-10-0	I		6-10-0	b-11-d
				4x4 =			
				6			
	6-10-1 6-8-8 1-0-3	2 3 1 20			8	7 8 23 24	9 10 10 12 11

18

17

16

13-8-0

15

19

+

Scale = 1:43.3

Plate Offsets (X, Y): [10:0-0-0,Edge]

Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15		тс	0.16	Vert(LL)	n/a	-	n/a	999	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15		BC	0.08	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES		WB	0.21	Horz(CT)	0.00	12	n/a	n/a	1	
BCLL	0.0	Code	IRC2018/1		Matrix-R	0.21	11012(01)	0.00	12	n/a	n/a	1	
BCDL	10.0	Code	11/02/010/1	1712014	Maultx-IX							Weight: 71 lb	FT = 20%
BCDL	10.0											weight. 71 lb	FT = 2076
LUMBER			NOT	ES					13) Prov	ide me	chanic	al connection (by	y others) of truss to
TOP CHORD	2x4 SP No.2		1) (Unbalanced	roof live loads ha	ve been o	considered for		bear	ing plat	e capa	able of withstand	ing 168 lb uplift at
BOT CHORD	2x4 SP No.2		· t	this design.					joint	20, 126	b lb up	lift at joint 12, 99	lb uplift at joint 17,
WEBS	2x4 SP No.2		2)	Wind: ASCE	7-16; Vult=115m	ph (3-sec	ond gust)		101	lb uplift	at join	nt 18, 194 lb uplifi	t at joint 19, 98 lb
OTHERS	2x3 SPF No.2		í v	Vasd=91mpl	n; TCDL=6.0psf;	BCDL=6.0	Opsf; h=35ft;		uplif	t at join	t 15, 10	01 lb uplift at join	t 14 and 181 lb uplift
BRACING	2.00 01 1 11012		1	Ke=1.00; Ca	t. II; Exp C; Enclo	sed; MW	FRS (envelop	e)	at jo	int 13.			
TOP CHORD	Structurel wood abo	athing directly applied			and C-C Exterio			- /	14) This	truss is	desia	ned in accordan	ce with the 2018
TOP CHORD					-1-0 to 6-10-0, Ex	· · ·	,		Íntei	nationa	I Resi	dential Code sec	tions R502.11.1 and
	6-0-0 oc purlins, ex				rior (1) 11-10-0 to			ər	R80	2.10.2 a	and ref	erenced standar	d ANSI/TPI 1.
BOT CHORD	Rigid ceiling directly	applied or 6-0-0 oc			exposed ; end ve				LOAD C				
	bracing.				for members an				LUAD C		J	nuaru	
REACTIONS	()	0, 13=13-8-0, 14=13-8	^{3-0,}		own; Lumber DO								
		0, 16=13-8-0, 17=13-8	^{3-0,} I	DOL=1.60		L=1.00 pi	ato grip						
		0, 19=13-8-0, 20=13-8	8-0		ed for wind loads	in the nl	one of the true						
	Max Horiz 20=-213 ((LC 14)			ids exposed to w								
	Max Uplift 12=-126 ((LC 13), 13=-181 (LC ⁻			d Industry Gable								
	14=-101 ((LC 17), 15=-98 (LC 17			alified building de								
	17=-99 (L	.C 16), 18=-101 (LC 16			7-16; Pr=25.0 p								
	19=-194 ((LC 16), 20=-168 (LC ·			.15); Pf=25.0 psf								
	Max Grav 12=203 (I	LC 30), 13=195 (LC 15											
	14=263 (l	LC 24), 15=306 (LC 24			ls=1.0; Rough Ca	at C; Fully	Exp.; Ce=0.9	,					
	16=214 (l	LC 29), 17=306 (LC 23		Cs=1.00; Ct=									
		_C 23), 19=224 (LC 14	1) (D		snow loads have	been cor	isidered for th	IS					
	20=237 (I		<i>"</i> (design.								an	acon
FORCES	(lb) - Maximum Com	,			is been designed							THE OF	MIG
I ONOLO	Tension	procordination			psf or 2.00 times			on at				BAR	-sold
TOP CHORD	1-2=0/82, 2-3=-166/	151 3-4-105/103			on-concurrent wit						A		N.S.
	,	38/229, 6-7=-138/223			e 1.5x4 MT20 unl			l.			A	SCOT	TM. PN
		4/83, 9-10=-133/114,	· 0) ·		es continuous bo						B	SEV	
	,	184/116, 10-12=-184/8			ully sheathed from						Rea		
	,	,	1		nst lateral movem		iagonal web).				810	the	
BOT CHORD	19-20=-100/120, 18	,			spaced at 2-0-0 of						2	alt	JONN &
	17-18=-100/120, 16				is been designed					-	VIE	NUM	BER A
	15-16=-100/120, 14				ad nonconcurrent			ds.			47		
	13-14=-100/120, 12		12) /	All bearings	are assumed to b	e SP No.	2 crushing				N	O PE-2001	01000/ 201
WEBS	6-16=-210/68, 5-17=		. (capacity of 5	65 psi.						V	N'AN	154
	4-18=-221/145, 3-19	,			-							N.S.S.	ENU'S
	7-15=-266/134, 8-14	i=-221/145,										CSSIONA	LEY
	9-13=-132/133											Car	The

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling or individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

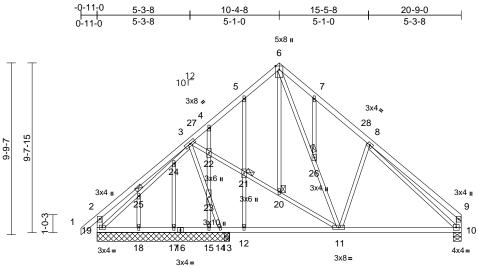
14

13



16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200 / MiTek-US.com

						RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type	Qty	Ply	Roof - HM Lot 180	AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES 168681689
P241025-01	D2	Common Structural Gable	1	1	Job Reference (optional)	LEE'S SUMMIT, MISSOURI
Premier Building Supply	(Springhill, KS), Spring Hills, k	KS - 66083, Run: 8.63 S Sep ID:DWJnveVQ7o	o 26 2024 Print: oN6jItbUjayQQy	8.630 S Sep IfPO-RfC?Ps	26 2024 MiTek Industries, Inc. B70Hq3NSgPqnL8w3uITXbGK	hu Oct 00119:/21/2024
		0.44.0				



	6-11-13 6-11-13	7-4-12 0-4-15	<u>13-9-3</u> 6-4-7	<u>20-9-0</u> 6-11-13	

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 25.0 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	1-11-4 1.15 1.15 YES IRC2018	3/TPI2014	CSI TC BC WB Matrix-S	0.46 0.39 0.68	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.05 -0.10 0.01	(loc) 10-11 10-11 10	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 134 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD JOINTS	No.2 2x3 SPF No.2 Structural wood sh 6-0-0 oc purlins, e Rigid ceiling direct bracing. 1 Brace at Jt(s): 20 21, 25, 26		d or NC 1) 2)	2 8 1 2 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0	3-23=-471/167, 14- -1-22=0/389, 20-21 3-26=-242/357, 11- 3-11=-336/279, 19- -4-25=-200/138, 3- 3-10=-559/2, 6-20= 2-21=-161/98, 4-2 2-23=-180/23, 15- 7-24=-56/31, 18-2 roof live loads have 7-16; Vult=115mpt	I=0/388 26=-21: 25=-19: 24=-23: -4/16, 5 2=-145/ 23=-18: 5=-1/5, e been o h (3-sec	, 11-20=0/385 2/327, 3/136, 3/161, -21=-191/108 17, 4/23, 7-26=-73/32 considered for cond gust)	ə, 3,	cho 11) All t cap 12) N/A 13) This Inte	rd live lo bearings acity of s truss is rnationa 02.10.2 a	ad nor are as 565 ps design l Resign and refe	sumed to be SP i. ned in accordanc lential Code sect erenced standard	any other live loads. No.2 crushing we with the 2018 ions R502.11.1 and
	15=7-6- 19=7-6- Max Horiz 19=278 Max Uplift 10=-97 (14=-283 19=-101 Max Grav 10=729 14=172	(LC 13) LC 17), 13=-176 (LC 1 (LC 23), 17=-8 (LC 12 (LC 17) (LC 24), 13=804 (LC 2 (LC 24), 15=278 (LC 2 .C 27), 18=89 (LC 7),	16), 2), 23),	Ke=1.00; Cat exterior zone Interior (1) 4- 15-6-1, Interii and right exp exposed;C-C reactions sho DOL=1.60 Truss design	a); TCDL=6.0psf; BC . II; Exp C; Enclose and C-C Exterior(2 1-0 to 10-4-8, Exter or (1) 15-6-1 to 20- osed; end vertical for members and wn; Lumber DOL= ed for wind loads in ds exposed to wind	ed; MW 2E) -0-1 prior(2R -7-4 zor left and forces 8 =1.60 pla n the pla	FRS (envelop 1-0 to 4-1-0,) 10-4-8 to le; cantilever I right MWFRS for ate grip ane of the trus	left					
FORCES		mpression/Maximum	0	or consult qu	I Industry Gable Er alified building des	igner as	s per ANSI/TF	ข 1.				TATE OF M	AISS
TOP CHORD	4-5=-471/185, 5-6=	3/239, 3-4=-463/172, 423/223, 6-7=-578/28 326/166, 2-19=-364/2	36,	Plate DOL=1	7-16; Pr=25.0 psf .15); Pf=25.0 psf (L s=1.0; Rough Cat (:1.10	Lum DC	L=1.15 Plate					STAT SCOTT	M . $\sum V$
BOT CHORD	9-10-310/161 18-19=-134/264, 1 15-17=-134/264, 1 13-14=-150/156, 1 11-12=-150/156, 1	4-15=-134/264, 2-13=-150/156,	6) 7) 8)	design. This truss ha load of 12.0 p overhangs no All plates are Truss to be fu braced again	snow loads have b s been designed for osf or 2.00 times fit on-concurrent with 1.5x4 MT20 unles illy sheathed from st lateral movemer spaced at 2-0-0 oc.	or greate at roof le other liv s other one fac one fac	er of min roof bad of 25.0 ps ve loads. wise indicated e or securely	live sf on 1.			A INCOM	PE-2001	ENGINE

9) Gable studs spaced at 2-0-0 oc.

Scale = 1:65.6

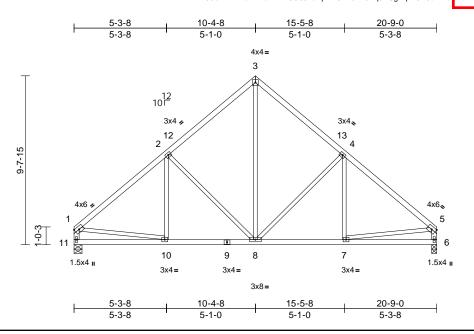


						REL
Job	Truss	Truss Type	Qty	Ply	Roof - HM Lot 180	AS D
P241025-01	D3	Common	7	1	Job Reference (optional	L

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. hu Oct 05 14 5: 5 ID:XO3uKwH4?aKwEaMRkzSC66zd1ip-RfC?PsB70Hq3NSgPqnL8w3uITXb6KWrCDor704z6c?



1/202



Scale = 1:65.9

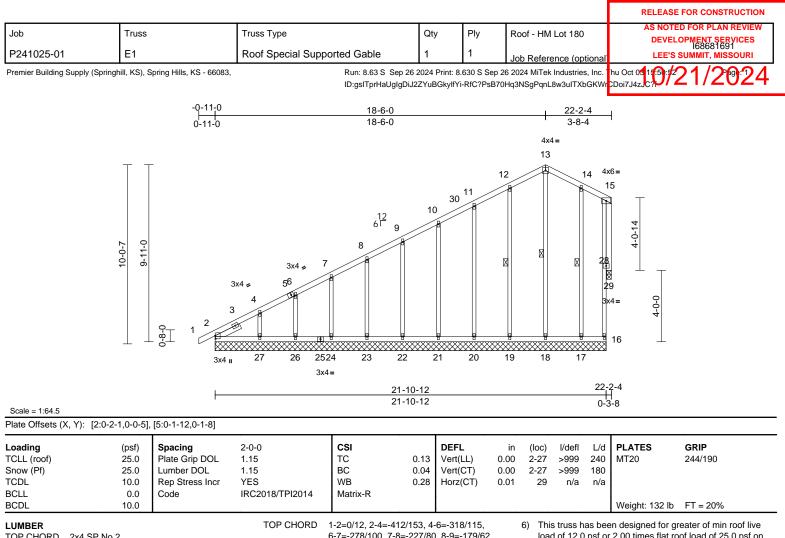
Plate Offsets (X, Y): [4:0-0-0,0-0-0]

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL LUMBER	(psf) 25.0 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018		CSI TC BC WB Matrix-S 7-16; Pr=25.0 ps				(loc) 7-8 7-8 6	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 108 lb	GRIP 197/144 FT = 20%
TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD	 2x4 SP No.2 2x3 SPF No.2 *Exce Structural wood she 4-9-0 oc purlins, ex 	athing directly appli cept end verticals.	4) ed or 5)	DOL=1.15); Cs=1.00; Ct Unbalanced design. This truss ha chord live lo	I.15); Pf=25.0 psf Is=1.0; Rough Car =1.10 snow loads have as been designed ad nonconcurrent are assumed to be	t C; Fully been cor for a 10. with any	Exp.; Ce=0. nsidered for t 0 psf bottom other live loa	.9; this					
REACTIONS	0	LC 12) C 17), 11=-113 (LC	,	recommende UPLIFT at jt	i65 psi. Simpson Strong-Ti ed to connect truss (s) 11 and 6. This as not consider late	s to bear connecti	ing walls due on is for uplit						
FORCES TOP CHORD	(lb) - Maximum Com Tension	pression/Maximum -882/253, 3-4=-882	8) 2/253,	This truss is International R802.10.2 a	designed in accor Residential Code nd referenced star	dance w	ith the 2018 R502.11.1	and					
BOT CHORD WEBS	 10-11=-265/344, 8-1 7-8=-81/810, 6-7=-6 1-10=-23/695, 5-7=- 	0=-155/810, 6/145 26/695, 2-8=-406/23	34,	OAD CASE(S)	Standard								
	3-8=-175/547, 4-8=- 4-7=-7/153 ced roof live loads have	,	,									STATE OF M	AISSO
Vasd=91 Ke=1.00; exterior z	n. iCE 7-16; Vult=115mph mph; TCDL=6.0psf; BC Cat. II; Exp C; Enclose icone and C-C Exterior(2) 5-3-8 to 10-4-8. Exter	DL=6.0psf; h=35ft; d; MWFRS (envelo E) 0-1-12 to 5-3-8,	pe)								ER STATE	ST SCOTT SEVI	ER

Interior (1) 5-3-8 to 10-4-8, Exterior(2R) 10-4-8 to 15-5-8, Interior (1) 15-5-8 to 20-7-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 SCOTT M. SEVIER NUMBER PE-2001018807 STONAL ENGINE October 7,2024

> 16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200 / MiTek-US.com





BCDL	10.0			Weight: 132 lb FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS SLIDER BRACING TOP CHORD BOT CHORD WEBS REACTIONS	$\begin{array}{c} 2\text{x4 SP No.2} \\ 2\text{x3 SPF No.2} \\ \text{Left 2x4 SP No.2 - 1-6-7} \\ \end{array}$	 this design Wind: ASC Vasd=91n Ke=1.00; exterior zc Exterior(2 21-9-8 zor vertical left MWFRS f grip DDL= Truss des only. For see Stand or consult TCLL: AS Plate DOL 	CE 7-16; Vult=115mph (3-second gust) nph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) one and C-C Corner(3E) -0-11-0 to 4-1-0, N) 4-1-0 to 18-6-0, Corner(3E) 18-6-0 to ne; cantilever left and right exposed ; end t exposed;C-C for members and forces & or reactions shown; Lumber DOL=1.60 plate :1.60 igned for wind loads in the plane of the truss studs exposed to wind (normal to the face), ard Industry Gable End Details as applicable, qualified building designer as per ANSI/TPI 1. CE 7-16; Pr=25.0 psf (Lorm DOL=1.15 Plate i); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9;	 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads. 7) All plates are 1.5x4 MT20 unless otherwise indicated. 8) Gable studs spaced at 2-0-0 oc. 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 11) Bearing at joint(s) 29 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 12) N/A 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP1 1. LOAD CASE(S) Standard

Unbalanced snow loads have been considered for this



16023 Swingley Ridge Rd. Chesterfield MO 63017 314.434.1200 / MiTek-US.com

 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to dury with with where outputs into design is based only door parameters shown, and is for an individual building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH1 Quality Criteria**, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)

design.

5)

Cs=1.00; Ct=1.10

FORCES

Tension

(lb) - Maximum Compression/Maximum

									RELEASE FOR CONSTRUCTION
Job	Т	russ	Truss Type		Qty	Ply	Roof - HM Lot 180		AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES 168681692
P241025-01	E	2	Roof Special		8	1	Job Reference (op	tional	I68681692 LEE'S SUMMIT, MISSOURI
Premier Building	Supply (Springhill,	KS), Spring Hills, KS - 66083,					6 2024 MiTek Industries Hq3NSgPqnL8w3uITXI		
		-0-11-0	0.0.44		,				
		-0-11-0 	<u>6-3-11</u> 6-3-11	<u>12-4-13</u> 6-1-3		<u>18-6-0</u> 6-1-3	22-2		1
							4x6 II		
	10-0-7		1.5 3x4 = 4 3x4 = 15 3 3	6^{12} $3x4 = 5x4 = 10^{-12}$	3x4 = 16 6	10 5x5=	7		9
			7-5-4		3-14		21-10-12	22-2	2-4
Scale = 1:64.2		⊢—	7-5-4		D-10		6-6-14	0-3	
Loading		sf) Spacing	2-0-0	CSI	DEF		in (loc) l/defl	L/d	PLATES GRIP
TCLL (roof) Snow (Pf)	25	5.0 Plate Grip DOL 5.0 Lumber DOL	1.15 1.15	BC 0	.73 Vert(.61 Vert(CT) -0.	07 10-12 >999 17 10-12 >999	240 180	MT20 244/190
TCDL BCLL BCDL		0.0 Rep Stress Incr 0.0 Code	YES IRC2018/TPI2014	WB 0 Matrix-S	.96 Horz	(CT) 0.1	23 14 n/a	n/a	
BOT CHORD WEBS REACTIONS FORCES TOP CHORD BOT CHORD WEBS NOTES	2x4 SP No.2 2x3 SPF No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 3x4 SP No.2 3x4 SP No.2 4:3-1 oc purlins Rigid ceiling dii bracing. 1 Row at midpt (size) 2=0 Max Horiz 2=38 Max Uplift 2=-1 Max Grav 2=10 (Ib) - Maximum Tension 1-2=0/12, 2-4= 6-7=-826/183, 8-13=-219/955 2-12=-468/137: 9-10=-93/340 4-12=-333/241 6-10=-752/326 7-9=-947/261, 1	d sheathing directly applied s, except end verticals. rectly applied or 8-6-13 oc t 7-9 -5-8, 14=0-3-2 82 (LC 16) 60 (LC 16), 14=-229 (LC 1 091 (LC 23), 14=978 (LC 2 o Compression/Maximum -1651/207, 4-6=-1505/273 7-8=-91/56, 9-13=-219/955 3, 10-12=-288/922, , 6-12=-169/602, , 7-10=-211/878, 8-14=-980/263	 Plate DOL=1 DOL=1.15); Cs=1.00; Cti 4) Unbalanced design. 5) This truss ha load of 12.0 overhangs n 6) This truss ha chord live loa 7) All bearings capacity of 5 8) Bearing at joint 6) designer sho 3) 9) One H2.5T S recommended UPLIFT at jtto only and doe 5) This truss is International 	snow loads have beer as been designed for g psf or 2.00 times flat r on-concurrent with oth as been designed for a ad nonconcurrent with are assumed to be SP bint(s) 14 considers pa TPI 1 angle to grain for build verify capacity of I Simpson Strong-Tie co ed to connect truss to I (s) 2 and 14. This com as not consider lateral designed in accordand Residential Code sec nd referenced standar	n DOL=1.1 Fully Exp.; n considered reater of m bof load of load of load of 10.0 psf b any other No.2 crus rallel to gra mula. Buil bearing sur nnectors bearing sur nnectors bearing wa forces. ce with the tions R502	5 Plate Ce=0.9; ed for this hin roof live 25.0 psf on ds. ottom live loads. hing hin value lding face. Ils due to or uplift 2018 .11.1 and			THE OF MISSOL
this design 2) Wind: ASC Vasd=91m Ke=1.00; C exterior zon Interior (1) zone; canti exposed;C	E 7-16; Vult=11 ph; TCDL=6.0ps Cat. II; Exp C; En ne and C-C Exte 4-1-0 to 18-6-0, lever left and rig -C for members	have been considered for 5mph (3-second gust) sf; BCDL=6.0psf; h=35ft; iclosed; MWFRS (envelope rior(2E) -0-11-0 to 4-1-0, Exterior(2E) 18-6-0 to 21-5 ht exposed ; end vertical le and forces & MWFRS for DOL=1.60 plate grip)-8				~	Plan	SEVIER NUMBER PE-2001018807 October 7,2024
•									

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

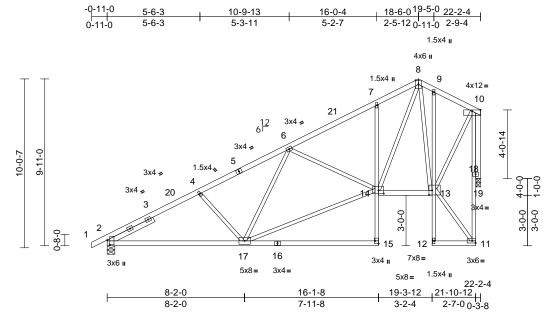
16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200 / MiTek-US.com

						RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type	Qty	Plv	Roof - HM Lot 180	AS NOTED FOR PLAN REVIEW
			,			DEVELOPMENT SERVICES I68681693
P241025-01	E3	Roof Special	2	1	Job Reference (optional	

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Thu Oct 05 1163 32 ID:HXyS?ajf9Rc8qbAhrG?Jw7ylfVZ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKV rCDoi7J4JJC-

.....





Scale = 1:68.5 Plate Offsets (X, Y): [2:0-3-5,0-1-13], [13:0-2-4,0-3-4], [14:0-3-12,0-2-4], [15:Edge,0-2-8]

Loading TCLL (roof) Snow (Pf) TCDL BCLL	(psf) 25.0 25.0 10.0 0.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES	8/TPI2014	CSI TC BC WB Matrix-S	0.43 0.72 0.91	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.11 -0.25 0.26	(loc) 15-17 15-17 19	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code	INC201	0/1112014	Wattix-S							Weight: 139 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS SLIDER BRACING	2x4 SP No.2 2x4 SP No.2 *Excep No.2 2x3 SPF No.2 2x4 SP No.2 Left 2x4 SP No.2 5		2)	Vasd=91mpl Ke=1.00; Ca exterior zone Interior (1) 4 zone; cantile and right exp MWFRS for	7-16; Vult=115m ;; TCDL=6.0psf; t. II; Exp C; Encle and C-C Exteric 1-0 to 18-6-0, Es ver left and right tosed;C-C for me reactions shown;	BCDL=6. osed; MW or(2E) -0- kterior(2E exposed embers ar	Dpsf; h=35ft; FRS (envelop 1-0 to 4-1-0, 18-6-0 to 21 ; end vertical d forces &	-9-8 left					
TOP CHORD BOT CHORD	Structural wood she 4-6-7 oc purlins, ex Rigid ceiling directly bracing.	cept end verticals.	d or 3)	Plate DOL=1 DOL=1.15);	7-16; Pr=25.0 ps .15); Pf=25.0 ps s=1.0; Rough Ca	f (Lum DC	L=1.15 Plate	•					
	(size) 2=0-5-8, 2 Max Horiz 2=306 (LC Max Uplift 2=-167 (L Max Grav 2=1091 (L	C 13) C 16), 19=-222 (LC ⁻		design. This truss ha	₌1.10 snow loads have s been designed osf or 2.00 times	for great	er of min roof	live					
FORCES	(lb) - Maximum Com Tension	pression/Maximum	C)	overhangs n	on-concurrent wit	th other li	/e loads.	0.01					
TOP CHORD	1-2=0/12, 2-4=-1667 6-7=-1112/272, 7-8= 8-9=-479/221, 9-10= 11-18=-100/116, 10-	-1091/369, -450/155,	6) 5, 7) 8)	chord live loa All bearings capacity of 5	s been designed ad nonconcurrent are assumed to b 65 psi. int(s) 19 conside	t with any be SP No.	other live loa 2 crushing						
BOT CHORD	2-17=-529/1392, 15- 7-14=-398/192, 13-1 12-13=-22/48, 9-13=	17=0/25, 14-15=0/13 4=-247/481,	31, 5,	using ANSI/ designer sho	FPI 1 angle to gra ould verify capacit	ain formul ty of bear	a. Building ng surface.	e				TE OF M	AISSO
WEBS NOTES 1) Unbalance	4-17=-278/211, 6-17 14-17=-512/1319, 6- 11-13=-148/191, 10- 8-14=-386/1213, 8-1 10-19=-980/271	'=-92/168, 14=-413/196, 13=-335/865, 3=-545/174,	10	recommende UPLIFT at jt(only and doe) This truss is International	ed to connect trus s) 2 and 19. This is not consider la designed in acco Residential Code nd referenced sta	ss to bear connecti teral force ordance w e sections	ing walls due on is for uplift es. ith the 2018 5 R502.11.1 a	t				SCOT SEVI	E Serves

Unbalanced roof live loads have been considered for 1) this design.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not

besign value to dury with with where outputs into design is based only door parameters shown, and is for an individual building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH1 Quality Criteria**, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



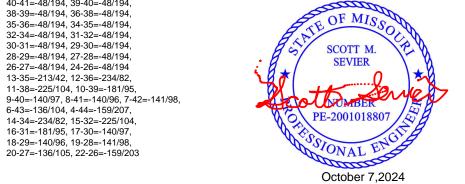
								RELEAS	E FOR CON	STRUCTION
Job	Truss	Truss Type		Qty	Ply	Roof - HM Lo	t 180		ED FOR PL	
P241025-01	E4	Common		7	1	Job Referenc	e (optional)		LOPMENT S 168681 S SUMMIT, N	
Premier Building Supply (Sprin	ghill, KS), Spring Hills, KS - 66083,		•			26 2024 MiTek Ind sB70Hq3NSgPqnLi	lustries, Inc.	hu Oct 03 11:5):5 WrCDoi7J4z39?	21/2	2024
-0-11- 0-11-0	0 6-3-11) 6-3-11	12-4-13 6-1-3	<u>18-6-0</u> 6-1-3	5x5=	24-7-3 6-1-3		<u>30-8-5</u> 6-1-3		67-0-0 6-3-11	37-11-0
10-0-7 0-8-0 1 5 7	1.5x4, 3x4 = 4 3x4 = 19 3	6 ¹² 3x6 = 6 5	4 = 20	7		3x4s 21 8		1.5x4 ¢	3x4 2 11 3x4	* 12 13
Le contra de la co	⊠ ix5 ∥	18 3x4=	17 3x6=	16 4x8=	15 3x6=		14 3x4=			⊠ 5x5 ∎
ŀ	<u>9-4-4</u> 9-4-4		18-6-0 9-1-12			7-12 I-12		<u>37-0-0</u> 9-4-4		—
Scale = 1:67.9										
Plate Offsets (X, Y): [2:0-2	2-13,0-0-12], [12:0-2-13,0-0-12]								
oading CLL (roof) inow (Pf) CDL ICLL ICDL	(psf)Spacing25.0Plate Grip DOL25.0Lumber DOL10.0Rep Stress Incr0.0Code10.0	2-0-0 1.15 1.15 YES IRC2018/TPI2014	CSI TC BC WB Matrix-S	0.99 Ve	rt(LL) - rt(CT) -	0.19 2-18 > 0.42 2-18 >	/defl L/d 999 240 999 180 n/a n/a	PLATES MT20 Weight: 171 lb	GRIP 244/190 FT = 20 ^o	%
No.2 3-5 IRACING OP CHORD Structural racing. VEBS I Row at r EACTIONS I Row at r I Row at r I Row at r I Row at r I Row at r Max Horiz Max Uplift Max Grav ORCES (b) - Maxi Tension OP CHORD 1-2=0/12, 6-7=-1924 8-10=-265 12:13=0/1 SOT CHORD 2-18=-488 14-16=-210 8-14=-58/4 6-16=-864 IOTES	.2 lo.2 P No.2 3-5-15, Right 2x4 SF -15 wood sheathing directly applied ing directly applied or 2-2-0 oc nidpt 8-16, 6-16 2=0-5-8, 12=0-5-8 2=183 (LC 16) 2=-276 (LC 16), 12=-276 (LC 2=1729 (LC 1), 12=1729 (LC num Compression/Maximum 2-4=-2919/471, 4-6=-2659/458 /432, 7-8=-1924/432, 9/458, 10-12=-2918/471,	Vasd=91m Ke=1.00; C exterior zo0 Interior (1) 23-6-0, Inta and right e exposed; C reactions s DOL=1.60 3) TCLL: ASC Plate DOL= DOL=1.10; Cs=1.00; C S=1.00; C (17) 4) Unbalance design. 5) This truss I load of 12.1 overhangs 6) All plates a 7) This truss I chord live I 8) All bearing capacity of 9) One H2.5T recommen I/230 UPLIFT at only and da (10) This truss i Internation	d snow loads have b has been designed fo o psf or 2.00 times fl non-concurrent with re 3x4 MT20 unless has been designed fo oad nonconcurrent w s are assumed to be 565 psi. Simpson Strong-Tie ded to connect truss it(s) 2 and 12. This o bes not consider late s designed in accorr al Residential Code and referenced stan	CDL=6.0psf; ed; MWFRS 2E) -0-11-0 2E) -0-11-0 2F) -0-11-0 2CD -2 rior(2R) 18- -11-0 zone; I left and righ forces & MV =1.60 plate g (roof LL: Lui Lum DOL=1 C; Fully Exp even conside or greater of at roof load c other live lo other vise in or a 10.0 psf vish any othe SP No.2 cru e connectors is ral forces. Jance with th sections R50	h=35ft; (envelope) to 4-1-0, 6-0 to cantilever let two set of the vFRS for rip m DOL=1.1 .15 Plate .; Ce=0.9; red for this min roof liv of 25.0 psf c ads. dicated. bottom r live loads. ishing valls due to for uplift e 2018 02.11.1 and	e on		S/ SCOT	TER 1018807	

October 7,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



								RELEASE	FOR CONSTRUCTION
Job	Truss	Truss Type		Qty	Ply	Roof - HM I	_ot 180		D FOR PLAN REVIEW DPMENT SERVICES 168681695
P241025-01	E5	Common Support	ted Gable	1	1	Job Refere	nce (optional		I68681695 SUMMIT, MISSOURI
Premier Building Supply (Spring	hill, KS), Spring Hills, KS - (66083,		ep 26 2024 Print: g5xShQIN2gNrT				hu Oct 03 15:59:52 VrCDoi7J4zJS4f	21/2024
-0-11-0)	18-6-0					37-0-0		37-11-0
0-11-0		18-6-0		5x5=			18-6-0		0-11-0
⊢ Scale = 1:67.9	3x6 = 3x4 = 56 3 4 6 = 44 43		9 38 3736 3x6=	13	14 15 16 18 18 18 18 18 18 18 18 18 18 18 18 18			19 3x6 221 221 28 27	22 ^{3x4} 23 24 25 26 3x6 II
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCLL BCDL	(psf) Spacing 25.0 Plate Grip DO 25.0 Lumber DOL 10.0 Rep Stress In 0.0 Code	. 1.15	CSI TC BC WB Matrix-S	0.05 Vei	t(LL) t(CT)	in (loc) n/a - n/a - 0.01 24	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 195 lb	GRIP 244/190 FT = 20%
1-6-7 BRACING TOP CHORD Structural v 6-0-0 oc pu BOT CHORD Rigid ceilin, bracing. WEBS 1 Row at m REACTIONS (size) 2 Max Horiz 2 Max Uplift 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	2 p.2 No.2 1-6-7, Right 2x- vood sheathing directly irlins. g directly applied or 10-6	applied or 0-0 oc 14-34 =37-0-0, 19=37-0-0, 12=37-0-0, 16=37-0, 16=37-0, 16=37-0, 16=37-0, 16=37-0, 16=37-0, 16=37-0, 16=37-0, 16=37-0, 16=37-0, 16=37-0, 16=	28=18 30=18 32=26 35=20 38=26 40=18 42=18	1 (LC 37), 27= 2 (LC 37), 27= 2 (LC 37), 29= 0 (LC 37), 31= 5 (LC 24), 34= 8 (LC 29), 36= 5 (LC 23), 39= 0 (LC 36), 41= 2 (LC 36), 43= 1 (LC 36) :compression/M 43/84, 4-6=-16 8=-102/131, 8- 0-11=-91/238, 12-13=-128/34 14-15=-111/25 6-17=-73/184, 8-19=-58/77, 7 -24=-165/50, 2 -44=-48/194, 1-42=-48/194, 1-32=-48/19	172 (LC 1), 180 (LC 24) 274 (LC 24) 274 (LC 23) 221 (LC 23) 180 (LC 23) 172 (LC 1), aximum 2/90, 9=-82/159, -2, 5, 9-20=-73/3	this (), 2) Wind), Vaso), Exte), Exte), 23-6 left a expo reac DOL 3) Trus only see or cc 4) TCL Plate 1, DOL Cs=	design. :: ASCE 7-16 d=91mph; TCI i.00; Cat. II; E rior zone and rior(2N) 4-1-0 -0, Exterior(2I and right expo sed;C-C for r tions shown; I =1.60 s designed fo .: For studs e) Standard Indt onsult qualified L: ASCE 7-16 e DOL=1.15); Is=1.0 1.00; Ct=1.10 alanced snow gn.	; Vult=115mph (3 DL=6.0psf; BCDL ixp C; Enclosed; C-C Corner(3E) - to 18-6-0, Corne N) 23-6-0 to 37-1 sed ; end vertical nembers and forc Lumber DOL=1.6 r wind loads in thh typosed to wind (n ustry Gable End E d building designe ; Pr=25.0 psf (roc Pf=25.0 psf (Lum 0; Rough Cat C; F loads have been	=6.0psf; h=35ft; WWFRS (envelope) 0-11-0 to 4-1-0, r(3R) 18-6-0 to 1-0 zone; cantilever left and right es & MWFRS for 0 plate grip e plane of the truss ormal to the face), tetails as applicable, er as per ANSI/TPI 1. f LL: Lum DOL=1.15 DOL=1.15 Plate ully Exp.; Ce=0.9; considered for this



ek

16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200 / MiTek-US.com

Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPTI Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

WEBS

NOTES

						RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type	Qty	Ply	Roof - HM Lot 180	AS NOTED FOR PLAN REVIEW
D044005.04	Fr		,			DEVELOPMENT SERVICES 168681695
P241025-01	E5	Common Supported Gable	1	I	Job Reference (optional	
Premier Building Supply (Springh	nill, KS), Spring Hills, KS - 66083,	Run: 8.63 S Sep 26 2	2024 Print: 8	.630 S Sep 2	6 2024 MiTek Industries, Inc.	hu Oct 0315:51:52

- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- All plates are 1.5x4 MT20 unless otherwise indicated. 7) Gable requires continuous bottom chord bearing.

8) 9) Gable studs spaced at 2-0-0 oc.

10) This truss has been designed for a 10.0 psf bottom

- chord live load nonconcurrent with any other live loads. 11) All bearings are assumed to be SP No.2 crushing
- capacity of 565 psi.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 2, 55 lb uplift at joint 36, 65 lb uplift at joint 38, 60 lb uplift at joint 39, 61 lb uplift at joint 40, 60 lb uplift at joint 41, 64 lb uplift at joint 42, 48 lb uplift at joint 43, 118 lb uplift at joint 44, 50 lb uplift at joint 34, 67 lb uplift at joint 32, 60 lb uplift at joint 31, 61 lb uplift at joint 30, 61 lb uplift at joint 29, 63 lb uplift at joint 28, 51 lb uplift at joint 27 and 105 lb uplift at joint 26.
- 13) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

ID:WUXtsnQTcg5xShQIN2gNrTylfdh-RfC?PsB70Hq3NSgPqnL8w3uITXbGK VrCDoi7J4zJS41



 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not beign value of use only wan win exec connectors, this design is based only upon parameters shown, and is for an individual building domponent, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality** Criteria, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



										REL	EASE FOR CONSTRUCTION
Job	Truss		Truss Ty	vpe		Qty	Ply	Roof - HM	/ Lot 180		NOTED FOR PLAN REVIEW
P241025-01	R1		Flat Gir	der		1	2	Job Refer	ence (optional)		EVELOPMENT SERVICES 168681696 EE'S SUMMIT, MISSOURI
Premier Building Supply (pringhill, KS), S	Spring Hills, KS - 66083,	•		Run: 8.63 S Sep 26 D:p1ILYtHBqmZt0H			p 26 2024 MiTel	k Industries, Inc.	'hu Oct 03 15:: VrCDoi7J4zJO	
		5-2-14	1	10-4-8		15	5-6-2		20-9-0		1
		5-2-14		5-1-10		5-	1-10	I	5-2-14		1
4-0-0	7x8 = 1 12 12 MT18HS :		3x6 II 2 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	15 16 15 16 10	4x8= 173	18	7x8	= 3x6 II 5 19 0 0 0 8 5x10=	20 ×	7x8	7
		5 0 44	5x10=		3x4 II	4.5		5x10=			
		<u>5-2-14</u> 5-2-14		<u>10-4-8</u> 5-1-10			5-6-2 1-10		<u>20-9-0</u> 5-2-14		
Scale = 1:44.9 Plate Offsets (X, Y): [7	:Edge,0-2-8],	[8:0-1-12,0-2-4], [11:	0-4-4,0-2-	4]							
BOT CHORD 2x6 SF WEBS 2x3 SF 1650F BRACING TOP CHORD 2-0-0 (end ve BOT CHORD Rigid (bracin) REACTIONS (size) Max HO Max Up Max Gra FORCES (lb) - M Tensic TOP CHORD 1-12=- 2-3=-6 5-6=-6 BOT CHORD 11-12=- 2-3=-6 5-6=-7 BOT CHORD 11-12=- 3-11=- 5-8=-2 WEBS 1-11=- 5-8=-2 NOTES 1) 2-ply truss to be cc (0.131"x3") nails at Top chords connec oc, 2x8 - 2 rows st Bottom chords con staggered at 0-9-0	1.5E c purlins (5-1 rticals. elling directly J. 7=0-3-8, 7 iz 12=-105 (ift 7=-1249 (v 7=5753 (I aximum Com 5096/1320, 1- 442/1646, 3-5 472/1610, 6-7 86/87, 9-11= 126/8458, 7-8 1922/7732, 2- 2434/617, 3-9 951/820, 6-8= nnected toge i follows: ted as follows ted as follows ted as follows ted as follows	LC 14) LC 13), 12=-1134 (LC LC 13), 12=5188 (LC 1 ppression/Maximum -2=-6442/1646, 5=-6472/1610, '=-5657/1429 2126/8458, B=-9/37 -11=-2907/822, 0=0/199, 3-8=-2397/62 1932/7766 ther with 10d s: 2x3 - 1 row at 0-9-0 0-0 oc.	2) 3) pt 4) 5) 6) 7) 8) 23, 10) 11) 12) 3 -	All loads are conserved if noted as capacity of 805 pt LGT2 Singers and 7. This consider lateral for the orientation bottom chord.	sidered equally a sitrix-S sidered equally a s front (F) or bac . Ply to ply conno- bute only loads r indicated. S; Vult=115mph .DL=6.0psf; BCD Exp C; Enclosed I C-C Corner (3) d; end vertical le ces & MWFRS f 00 plate grip DOL 6; Pr=25.0 psf (r ; Pf=25.0 psf (r ; Pf=25.0 psf (u 0; Rough Cat C;) v loads have bee e drainage to pre 20 plates unless en designed for onconcurrent witi sustand to be S si. trong-Tie conner searing walls durection ceton is for uplif prces. gned in accordan idential Code se ferenced standa epresentation do of the purlin aloo	0.36 Ve 0.77 Ho 0.77 H	rt(LL) rt(CT) rz(CT) all plies, in the LOA ve been -7) or (B), gust) h=35ft; i (envelope (envelope d;C-C for is shown; m DOL=1. .15 Plate .; Ce=0.9; ered for this r ponding. ered for this r ponding. ered for this r ponding. ered for this r live loads 2.0E crushi mmended TT at jt(s) 1 does not he 2018 D2.11.1 and Pl 1. pict the siz and/or	D pro lb (up dov at dov) up ann Th res LOAD 15 1) D Ir U 5 C 5 5 C	Inger(s) or othe by ded sufficien down and 198 la at 4-0-0, 958 lb wn and 198 lb 1 0-0-0, 958 lb wn and 198 lb 1 d 962 lb down a e design/select sponsibility of o CASE(S) Sta bead + Snow (b horerease=1.15 inform Loads (i Vert: 1-6=-70, concentrated Lc Vert: 4=-908, 18=-	t to support of b up at 2-0-1 b down and up at 8-0-0, down and 15 up at 14-0-0 1958 lb dowr and 196 lb up ion of such of thers. Indard alanced): Lu lb/ft) 7-12=-20 bads (lb) 13=-908, 19=-90	GRIP 197/144 197/144 22 lb FT = 20% device(s) shall be concentrated load(s) 958 0, 958 lb down and 198 lb 198 lb up at 6-0, 958 lb 958 lb down and 198 lb up 38 lb up at 12-0-0, 958 lb , 958 lb down and 198 lb n and 191 lb up at 18-0-0, p at 20-0-0 on top chord. connection device(s) is the mber Increase=1.15, Plate =-908, 15=-908, 16=-908, 8, 20=-908, 21=-920 OF MISSOURCE COTT M. SEVIER UNDER 001018807

October 7,2024

16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200 / MiTek-US.com

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

		T .		₊	10.0						-		ASE FOR CONSTRUCTION OTED FOR PLAN REVIEW	
Job		Truss		Truss T	yhe		Qty	Ply	R	loof - HM L	Lot 180	DEV	VELOPMENT SERVICES 168681697	
P241025-01		V1	Dania - 1990 - 100	Valley		De Cita	1	1			nce (optional	LEE	E'S SUMMIT, MISSOURI	
-remier Building	Supply (Spring)	nill, KS), S	Spring Hills, KS - 66083	3		Run: 8.63 S Sep ID:znOG5Q?SPP						hu Oct 03 15:50 VrCDoi7J4zJ9?1		4
							-				l			
			F			13	3-8-9				———————————————————————————————————————			
		_	T								7			
										6				
								5	/					
							1	4	A					
		6	,				4	/	Ĩ			Ō		
		6-10-9				3						6-10-9		
		Ŷ			2	ł						-		
				12	2									
			6	 /										
			4	1		e					8			
		_	-0							*****	× × ×			
				3x4 ≠	13	12	11		10	9				
0			F			13	8-8-9							
Scale = 1:42.8							<u> </u>				'	1		
oading CLL (roof)		(psf) 25.0	Spacing Plate Grip DOL	1-11-4 1.15		CSI TC		EFL ert(LL)	in n/a	. ,	l/defl L/d n/a 999	PLATES MT20	GRIP 244/190	
now (Pf)		25.0 10.0	Lumber DOL Rep Stress Incr	1.15 YES		BC WB	0.07 V	ert(TL) oriz(TL)	n/a 0.00	-	n/a 999 n/a n/a			
CLL		0.0	Code		3/TPI2014	Matrix-S	···/		0.00	U	.⊮a il/d	14/	. FT 000'	
		10.0	L		10.0	7.40-14.1		al - ¹				Weight: 61 lb	b FT = 20%	
OP CHORD	2x4 SP No.2			1)	Vasd=91mph	7-16; Vult=115mp ; TCDL=6.0psf; B0	CDL=6.0ps	f; h=35ft						
SOT CHORD	2x4 SP No.2 2x3 SPF No	2			exterior zone	. II; Exp C; Enclos and C-C Exterior((2E) 0-7-9	o 5-9-1,	• /					
THERS	2x3 SPF No				Interior (1) 5-	9-1 to 13-7-13 zon I ;C-C for members	ne; cantilev	er left an						
RACING OP CHORD			eathing directly appli	ed or		shown; Lumber D			-					
OT CHORD	Rigid ceiling		cept end verticals. applied or 10-0-0 o	c 2)	Truss design	ed for wind loads i ds exposed to win								
EACTIONS	bracing. (size) 1:	=13-8-9	, 8=13-8-9, 9=13-8-9	Э,	see Standard	Industry Gable Er	nd Details	as applic	able,					
	1		9, 11=13-8-9, 12=13		TCLL: ASCE	alified building des 7-16; Pr=25.0 psf	(roof LL: L	um DOL:	=1.15					
	Max Horiz 1:	=276 (LC	C 16)		DOL=1.15); I	.15); Pf=25.0 psf (s=1.0; Rough Cat								
	1	0=-59 (L	C 16), 9=-62 (LC 16) C 16), 11=-62 (LC 1 C 16), 12= 05 (LC 1	l6), <u>4</u>)	Cs=1.00; Ct= Unbalanced s	1.10 snow loads have b	een consid	lered for	this					
	Max Grav 1	=134 (Ľ	LC 16), 13=-95 (LC 1 C 27), 8=98 (LC 22)	(6) , 5)	design.	1.5x4 MT20 unles								
			C 22), 10=254 (LC 2 LC 22), 12=136 (LC	(2), 6	Gable require	es continuous botto spaced at 2-0-0 oc	om chord b							
ORCES	1:	3=280 (L		// /) 8)	This truss has	s been designed fo	or a 10.0 p							
	Tension			9)	All bearings a	d nonconcurrent ware assumed to be			aus.					
OP CHORD	4-5=-149/60		249/97, 3-4=-204/83 1/48, 6-7=-54/28,			nanical connection						A OF	E MISSO	
OT CHORD			1, 11-12=0/1, 10-11=		bearing plate	capable of withsta at joint 9, 59 lb up	anding 24 I	b uplift at	t joint		E	1251	No.	
/EBS	9-10=0/1, 8- 6-9=-228/11	6, 5-10=				Ib uplift at joint 12					R		OTT M.	
			2=-110/83, 2-13=-20	9/165 11) This truss is o	designed in accord					an an		> l 1+1	
OTES					R802.10.2 an	Residential Code s id referenced stan			and			ott.	Level.	ノ
				LC	AD CASE(S)	Standard					N.		MBER 01018807	
											V	The second	158	
												SION	VAL ENCE	
												111	mark the second	

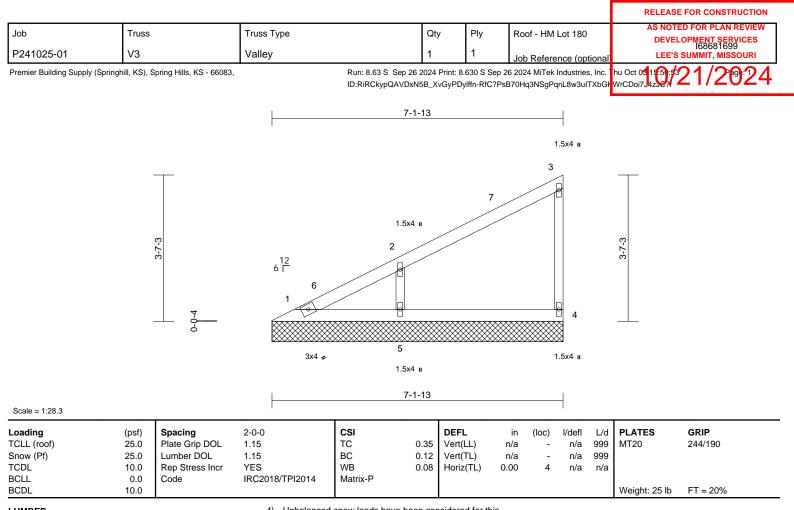
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



	_			_	_	_	_		RELEASE	FOR CONSTRUCTION
Job	Truss	,	Truss Type		Qty	Ply	Roof - HM L	∟ot 180		D FOR PLAN REVIEW
P241025-01	V2		Valley		1	1	Job Refere	nce (optional)		OPMENT SERVICES 168681698 SUMMIT, MISSOURI
Premier Building Supply (Sp	pringhill, KS),	Spring Hills, KS - 66083,		Run: 8.63 S Sep 26	3 2024 Print:	8.630 S Sep	p 26 2024 MiTek Ir	Industries, Inc.	hu Oct 05 15:5):53 VrCDoi7J4zJ091	21/2024
				ID:CEVEQNvκπyυρι	КЈОХ / DP ЦКУ	/ylttt-KIU (PSi	B70Hq3NSg⊬qn∟	-883011XDGK	/rCDoi/J#zJC+1*	
				10-5-	-13			_		
								1.5x4 u		
								4		
		\top						P	\top	
					1.5	5x4 II				
					3	//	/			
		5-3-3	1.	.5x4 u 9	10				5-3-3	
		Q		8					ف	
			12 2 6 T							
		_	1	ĺ						
						 ******		5		
			3x4 = 7		6	<u> ////////////////////////////////////</u>	<u></u>	1.5x4 II		
			1.!	.5x4 II	1.5	5x4 u				
Scale = 1:37.7			 	10-5-	-13			_		
Loading	(nef)	Spacing	2-0-0	CSI	DEF		in (loc)	l/defl L/d	PLATES	GRIP
TCLL (roof)	(psf) 25.0 25.0	Plate Grip DOL	1.15	TC	0.38 Ver	rt(LL)	n/a -	n/a 999	MT20	244/190
Snow (Pf) TCDL	25.0 10.0	Lumber DOL Rep Stress Incr	1.15 YES	WB		ert(TL) priz(TL) (n/a - 0.00 5	n/a 999 n/a n/a		
BCLL BCDL	0.0 10.0	Code	IRC2018/TPI2014	Matrix-S					Weight: 39 lb	FT = 20%
		<u></u>		E 7-16; Pr=25.0 psf (ro			15			
TOP CHORD 2x4 SP BOT CHORD 2x4 SP	No.2			:1.15); Pf=25.0 psf (Lu ; Is=1.0; Rough Cat C; t=1 10						
WEBS 2x3 SPF OTHERS 2x3 SPF				I snow loads have bee	en conside	red for this	,			
		eathing directly applied	d or 5) Gable require	res continuous bottom s spaced at 4-0-0 oc.	n chord be:	aring.				
BOT CHORD Rigid ce	eiling directly	xcept end verticals. ly applied or 10-0-0 oc	This truss hat	as been designed for a bee			•			
bracing. REACTIONS (size)	1=10-5-13	13, 5=10-5-13, 6=10-5-	All bearings :	are assumed to be SI						
	7=10-5-13 iz 1=214 (L0	_C 16)	 Provide mechanical de la periodicidade de la periodic	chanical connection (b te capable of withstand			nt			
	7=-101 (L		^{),} 5, 137 lb upli	lift at joint 6 and 101 lt designed in accordar	lb uplift at jo	joint 7.	t.			
	6=569 (L0	C 16), 5=206 (LC 22), LC 22), 7=302 (LC 22)	Ínternational	al Residential Code second	ections R50	02.11.1 and	i.			
Tension	n	mpression/Maximum	LOAD CASE(S)							
4-5=-17	74/99	=-214/95, 3-4=-120/54,								
WEBS 3-6=-48	′5, 6-7=-2/5, 5 80/289, 2-7=-									
NOTES 1) Wind: ASCE 7-16; V									OF N	MISSO
Vasd=91mph; TCDL Ke=1.00; Cat. II; Exp	L=6.0psf; BC (p C; Enclose	CDL=6.0psf; h=35ft; ed; MWFRS (envelope	e)					E	122	N S S
exterior zone and C- Interior (1) 5-7-9 to 1	C-C Exterior(2 10-5-1 zone;	(2E) 0-7-9 to 5-7-9, e; cantilever left and rigl	ght					A	SCOTT SEVIE	
exposed ;C-C for me reactions shown; Lu	embers and	forces & MWFRS for						87	1 mg	0
DOL=1.60 Truss designed for v			s					K)	golfom:	Server
only. For studs expo see Standard Indust	oosed to wind stry Gable En	d (normal to the face), nd Details as applicable	, Ie,					Ø.	PE-20010	18807
		signer as per ANSI/TPI						7	CSSIONAL	I ENGIA
									1000	er 7,2024
									UCtope	.sr /.ZUZ4

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

Antitlek Big Ridge Rd. Chesterfield, MO 63017 314.434.1200 / MITek-US.com



LUMBER		
TOP CHORD	2x4 SP N	0.2
BOT CHORD	2x4 SP N	0.2
WEBS	2x3 SPF I	No.2
OTHERS	2x3 SPF I	No.2
BRACING		
TOP CHORD		l wood sheathing directly applied or burlins, except end verticals.
BOT CHORD	Rigid ceil bracing.	ing directly applied or 10-0-0 oc
REACTIONS	(size)	1=7-1-13, 4=7-1-13, 5=7-1-13
REACTIONS	()	1=7-1-13, 4=7-1-13, 5=7-1-13 1=141 (LC 16)
REACTIONS	Max Horiz	, ,
REACTIONS	Max Horiz Max Uplift	1=141 (LC 16)
REACTIONS	Max Horiz Max Uplift Max Grav	1=141 (LC 16) 4=-48 (LC 16), 5=-128 (LC 16) 1=72 (LC 27), 4=206 (LC 22),
	Max Horiz Max Uplift Max Grav	1=141 (LC 16) 4=-48 (LC 16), 5=-128 (LC 16) 1=72 (LC 27), 4=206 (LC 22), 5=541 (LC 22)
	Max Horiz Max Uplift Max Grav (Ib) - Max Tension	1=141 (LC 16) 4=-48 (LC 16), 5=-128 (LC 16) 1=72 (LC 27), 4=206 (LC 22), 5=541 (LC 22)
FORCES	Max Horiz Max Uplift Max Grav (lb) - Max Tension 1-2=-227/	1=141 (LC 16) 4=-48 (LC 16), 5=-128 (LC 16) 1=72 (LC 27), 4=206 (LC 22), 5=541 (LC 22) imum Compression/Maximum 105, 2-3=-115/59, 3-4=-175/119

NOTES

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-7-9 to 5-7-9, Interior (1) 5-7-9 to 7-1-1 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss 2) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- 4) Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing. 5)
- 6) Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom 7)
- chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SP No.2 crushing 8)
- capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 4 and 128 lb uplift at joint 5.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not beigh valid for use only with with with sets outputs into design is based only door parameters shown, and is for an individual dualing component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria**, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



									RELEAS	E FOR CONSTRUCTION	
Job	Truss		Truss Type		Qty	Ply	Roof - HM	Lot 180		TED FOR PLAN REVIEW	1
P241025-01	V4		Valley		1	1	Ioh Refer	ence (optiona	1 5 5 10	LOPMENT SERVICES 168681700 S SUMMIT, MISSOURI	
Premier Building Supr	ply (Springhill, KS), S	Spring Hills, KS - 66083,	<u> </u>				26 2024 MiTek	Industries, Inc	c. Thu Oct 03 15:51:53 bG (WrCDoi754250?f	21/2024	-
			-	3.	3-9-13					_	
							1.5x4 u				
		0-0-4-11-3	6	12 1 0 3x4 =			2 3 3. 1.5x4 II		1-11-3		
Scale = 1:20.7				3.	3-9-13						
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 25.0 25.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018/TPI2014	BC 0	0.11 Vert(rt(LL) r rt(TL) r	in (loc) n/a - n/a - n/a -	l/defl L/d n/a 999 n/a 999 n/a n/d	99 MT20 99	GRIP 244/190 FT = 20%	-
LUMBER TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x3 BRACING TOP CHORD Str 3- BOT CHORD Rig br REACTIONS (size Max Max Max FORCES (lb) Te TOP CHORD 1-2	x4 SP No.2 x4 SP No.2 x3 SPF No.2 structural wood shea -10-5 oc purlins, ex ligid ceiling directly racing. ze) 1=3-9-13, ax Horiz 1=68 (LC ax Grav 1=68 (LC ax Grav 1=191 (LC b) - Maximum Com ension -2=-74/57, 2-3=-160	C 16), 3=-48 (LC 16) .C 22), 3=191 (LC 22) npression/Maximum	d or d or d or d or d or d or d or d or	chanical connection (by te capable of withstand uplift at joint 3. s designed in accordand al Residential Code sec and referenced standar	h any other P No.2 crus by others) of ding 14 lb up nce with the ctions R502	r live loads. Ishing of truss to uplift at joint e 2018 02.11.1 and					_
BOT CHORD 1-3 NOTES 1) Wind: ASCE 7- Vasd=91mph; Ke=1.00; Cat. I exterior zone a and right expos MWFRS for ree grip DOL=1.60	-3=0/0 7-16; Vult=115mph ; TCDL=6.0psf; BCI II; Exp C; Enclosed and C-C Exterior(2l osed; C-C for memb eactions shown; Lui 0	h (3-second gust) CDL=6.0psf; h=35ft; ed; MWFRS (envelope 2E) zone; cantilever lef	e						5 TE OF	MISSOF	

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- or consult qualified building designer as per ANSI/TPI 1.
 TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 4-0-0 oc.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulgase with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

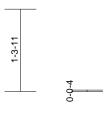


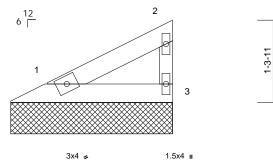


							RELEASE FOR CONSTRUCTION
Job	Truss	Truss Type		Qty	Ply	Roof - HM Lot 180	AS NOTED FOR PLAN REVIEW
P241025-01	V5	Valley		1	1	Job Reference (optional	DEVELOPMENT SERVICES 168681701 LEE'S SUMMIT, MISSOURI
Premier Building Supply (Spring	hill, KS), Spring Hills, KS - 66083,		Run: 8.63 S Sep 26 ID:zFLPyob7PzyVDK	2024 Print: 8 pXU7xkgQyl	.630 S Sep 2 fg3-RfC?PsE	6 2024 MiTek Industries, Inc. 370Hq3NSgPqnL8w3uITXbGK	hu Oct 05111:5):#321/219:24









2-6-14

Scale = 1:18.3					1					
Loading (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	Spacing2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI	CSI TC BC WB 2014 Matrix-P	0.09 0.04 0.00	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a n/a	(loc) - -	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 8 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x3 SPF No.2 BRACING TOP CHORD Structural wood shear 2-7-6 oc purlins, exc BOT CHORD Rigid ceiling directly bracing. REACTIONS (size) 1=2-6-14, Max Horiz 1=41 (LC Max Uplift 1=-8 (LC ' Max Grav 1=108 (LC FORCES (lb) - Maximum Com Tension TOP CHORD 1-2=-44/32, 2-3=-89/ BOT CHORD 1-2=-44/32, 2-3=-89/ BOT CHORD 1-3=0/0 NOTES 1) Wind: ASCE 7-16; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCI Ke=1.00; Cat. II; Exp C; Enclose exterior zone and C-C Exterior(2 and right exposed ;C-C for memi MWFRS for reactions shown; Lu grip DOL=1.60 2) Truss designed for wind loads in only. For studs exposed to wind see Standard Industry Gable Enc or consult qualified building desig 3) TCLL: ASCE 7-16; Pf=25.0 psf (L DOL=1.15); Is=1.0; Rough Cat C Cs=1.00; Ct=1.10 4) Unbalanced snow loads have be design. 5) Gable requires continuous bottor 6) Gable studs spaced at 4-0-0 oc.	 chc athing directly applied or cept end verticals. applied or 10-0-0 oc 3=2-6-14 16), 3=-29 (LC 16) 22), 3=108 (LC 22) pression/Maximum 778 (3-second gust) DL=6.0psf; h=35ft; d; MWFRS (envelope) E) zone; cantilever left bers and forces & mber DOL=1.60 plate the plane of the truss (normal to the face), d d Details as applicable, gner as per ANSI/TPI 1. roof LL: Lum DOL=1.15 um DOL=1.15 Plate c; Fully Exp.; Ce=0.9; en considered for this 	s truss has been designed rd live load nonconcurrent bearings are assumed to b acity of 565 psi. vide mechanical connectio ring plate capable of withs 129 lb uplift at joint 3. s truss is designed in accou- rnational Residential Code 12.10.2 and referenced sta CASE(S) Standard	with any e SP No.: n (by othe tanding 8 rdance wi s sections	other live loads 2 crushing ers) of truss to Ib uplift at join th the 2018 R502.11.1 an	ıt 1				STATE OF STATE OF SEV SEV PE-2001	LER Jerrich 018807

- 5)
- 6) Gable studs spaced at 4-0-0 oc.



October 7,2024

Com

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

										RELEASE FOR CONSTRUCTION
Job	Truss		Truss Ty	уре	Qty	Ply	\neg	Roof - HM L	Lot 180	AS NOTED FOR PLAN REVIEW
P241025-01					1	1	ļ		nce (optional	DEVELOPMENT SERVICES 168681702 LEE'S SUMMIT, MISSOURI
Premier Building Supply		Spring Hills, KS - 66083,	<u> </u>	Run: 8.63 S Sep 26	2024 Pri	int: 8.630 S 5	Sep 2f	26 2024 MiTek Ir	Industries, Inc.	c. Thu Oct 05 15 51:53 1 / Prain 1
				ID:gvQmUPVk3p3UtF	FmBZ9J5	5uxylfgA-RfC	J?PsB	370Hq3NSgPqr	nL8w3ulTXbG	GWrCDoi7J4z924 Z 17ZUZ4
				5-1	0-14				1	
								1.5	5x4 u	
								2		
	_	\top							1	$\overline{}$
						,	/		1	
		_				//	/		1	
	T	2-11-11		12 6	/				1	2-11-11
	Ċ	4		6 F	/				1	5
				1					I	
		4					·	[0]	3	
	_	0 			***		***		4	
					<u>XXXXX</u>	<u></u>	<u> </u>	***************************************		
				3x4 ≠				1.0	5x4 u	
				5-1	0-14]	1	
Scale = 1:24.8				 				I		- <u>.</u>
Loading	(psf)	Spacing	2-0-0	CSI		DEFL		. ,	l/defl L/d	
TCLL (roof) Snow (Pf)	25.0 25.0	Plate Grip DOL Lumber DOL	1.15 1.15	BC 0	0.34 V	Vert(LL) Vert(TL)	n/	n/a - n/a -	n/a 999 n/a 999	99
TCDL BCLL	10.0 0.0	Rep Stress Incr Code	YES			Horiz(TĹ)		n/a -	n/a n/a	
BCDL	10.0									Weight: 20 lb FT = 20%
				This truss has been designed for a chord live load nonconcurrent with			-de			
BOT CHORD 2x4 S	SP No.2 SP No.2			All bearings are assumed to be SF			ü5.			
BRACING	SPF No.2		9)							
TOP CHORD Struc	uctural wood shea ept end verticals.	eathing directly applied s.		bearing plate capable of withstand 1 and 80 lb uplift at joint 3.	0		oint			
	id ceiling directly	y applied or 10-0-0 oc	·) This truss is designed in accordant International Residential Code sec	ctions R8	R502.11.1 ar	and			
REACTIONS (size)) 1=5-10-14	4, 3=5-10-14		R802.10.2 and referenced standar DAD CASE(S) Standard	d ANSI/	./TPI 1.				
Max U		C 16), 3=-80 (LC 16)								
	•	C 22), 3=341 (LC 22) mpression/Maximum								
Tens										
BOT CHORD 1-3=0										
NOTES 1) Wind: ASCE 7-16 Vood 01mph; TC										
Ke=1.00; Cat. II;	; Exp C; Enclose	CDL=6.0psf; h=35ft; ed; MWFRS (envelope								
and right exposed	ed ;C-C for memb	2E) zone; cantilever lean nbers and forces &								ann
MWFRS for react grip DOL=1.60	tions shown; Lu	umber DOL=1.60 plate	,							SE OF MISSO
 Truss designed for 		n the plane of the truss d (normal to the face),							1	BAR CAN
see Standard Ind	dustry Gable End	nd Details as applicable	le,						AA	SCOTT M. SEVIER
3) TCLL: ASCE 7-16	16; Pr=25.0 psf (i	(roof LL: Lum DOL=1. Lum DOL=1.15 Plate							(A)	*
DOL=1.15); Is=1.	1.0; Rough Cat C	C; Fully Exp.; Ce=0.9;							X	to attom of the
		een considered for this	3						N.	PE-2001018807
design.5) Gable requires contractions									1	A Pose work
Gable studs space	ced at 4-0-0 oc.									SIONAL ENO.
										October 7,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



											RELEASE	FOR CONSTRUCTION	
Job	Truss		Truss Type		Qty	Ply	Ro	oof - HM	Lot 180			D FOR PLAN REVIEW	
P241025-01	V7		Valley		1	1	Jo	b Refere	nce (op	tional		OPMENT SERVICES 168681703 SUMMIT, MISSOURI	
Premier Building	Supply (Springhill, KS), S	Spring Hills, KS - 66083,					Sep 26 20	24 MiTek	Industries	s, Inc. T	hu Oct 03 15:51:53 /rCDoi7J4zJO	21/2024	-
				9.	-2-14				1.5x4	II			
		4-0	6 ¹² 6 1 3x4 =	7	1.5x4 2 8 5 1.5x4				3	4 1	4-7-11		
Scale = 1:32.5				9.	-2-14								
Loading TCLL (roof) Snow (Pf) TCDL BCLL	(psf) 25.0 25.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI TC BC WB	0.46 V 0.18 V	PEFL /ert(LL) /ert(TL) loriz(TL)	in n/a n/a 0.00	(loc) - - 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190	
BCDL	0.0 10.0	Code	IRC2018/TPI2014	Matrix-S							Weight: 33 lb	FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x3 SPF No.2 2x3 SPF No.2 Structural wood she 6-0-0 oc purlins, ex Rigid ceiling directly bracing.	applied or 10-0-0 oc , 4=9-2-14, 5=9-2-14	design. 5) Gable requir 6) Gable studs 7) This truss ha chord live loz d or 8) All bearings capacity of 5 9) Provide mec bearing plate 4 and 162 lb 10) This truss is	snow loads have b es continuous botto spaced at 4-0-0 oc is been designed fo ad nonconcurrent v are assumed to be 65 psi. hanical connection e capable of withsta uplift at joint 5. designed in accord Residential Code	om chord b c. or a 10.0 p vith any oth SP No.2 c (by others anding 42 b dance with	bearing. sf bottom her live loa crushing s) of truss t b uplift at j the 2018	ds. o oint						

- Max Uplift 4=-42 (LC 16), 5=-162 (LC 16) Max Grav 1=171 (LC 22), 4=190 (LC 22), 5=645 (LC 22) FORCES (Ib) - Maximum Compression/Maximum Tension
- TOP CHORD
 1-2=-240/112, 2-3=-125/49, 3-4=-163/98

 BOT CHORD
 1-5=-2/5, 4-5=-2/5

 WEBS
 2-5=-529/341

NOTES

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-7-9 to 5-7-9, Interior (1) 5-7-9 to 9-2-2 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 CADE CASE(S) Standard

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Claulity Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com) * I6023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200 / MITEk-US.com

										E FOR CONSTRUCTION	
Job	Truss		Truss Type	Qty	Ply	Roof - HM Lot 180		AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES			
P241025-01	V8		Valley		1	1	Job Refere	nce (optional	DEVELOPMENT SERVICES 168681704 LEE'S SUMMIT, MISSOURI		
Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,			3	Run: 8.63 S Sep 26 202			26 2024 MiTek	Industries, Inc.	hu Oct 05 15 5 5 1 / 7 9 1		
				ID:UFCl20yWf_Ot_KqXI	RC?XaSyli	fgt-RfC?PsB	370Hq3NSgPqr	1L8w3ulTXbGK	VrCDoi7J4zJO?i		
			L	12-6-14							
			I					4 5-4			
								1.5x4 ∎ 4			
		\top						-	\top		
						1.5x4 ॥					
						3					
				1.5x4 n 9	10	ł					
		6-3-11		8					6-3-11		
		99		2					6		
			10								
			¹² 6Г								
		4	1	A		G		A			
		è					******				
			3x4 🛩	7	******	6	*****	1.5x4 I			
				1.5x4 u		1.5x4 ॥					
			L	12-6-14							
Scale = 1:42.1									1		
L oading TCLL (roof)	(psf) 25.0		2-0-0 1.15	CSI TC 0.3	6 Vert(in (loc) n/a -	l/defl L/d n/a 999		GRIP 244/190	
Snow (Pf)	25.0	Lumber DOL	1.15	BC 0.1	· ·	,	n/a -	n/a 999		244/190	
TCDL BCLL	10.0 0.0		YES IRC2018/TPI2014	WB 0.1 Matrix-S	9 Horiz	2(TL) 0	.00 5	n/a n/a			
BCDL	10.0								Weight: 48 lb	FT = 20%	
	0.40DN 0			E 7-16; Pr=25.0 psf (roof			i				
TOP CHORD BOT CHORD	2x4 SP No.2 2x4 SP No.2		DOL=1.15);	1.15); Pf=25.0 psf (Lum I Is=1.0; Rough Cat C; Fu							
WEBS OTHERS	2x3 SPF No.2 2x3 SPF No.2		Cs=1.00; Ct 4) Unbalanced	=1.10 I snow loads have been c	onsidere	ed for this					
BRACING			design. 5) Gable requir	res continuous bottom ch	ord bear	ina					
TOP CHORD		sheathing directly applie except end verticals.	6) Gable studs	spaced at 4-0-0 oc.		-					
BOT CHORD	Rigid ceiling direct bracing.	ctly applied or 10-0-0 or	chord live lo	as been designed for a 1 ad nonconcurrent with an	ny other I	live loads.					
REACTIONS	(size) 1=12-6	6-14, 5=12-6-14, 6=12-6	6-14, 8) All bearings capacity of 5	are assumed to be SP N 565 psi.	lo.2 crusł	hing					
	7=12-6 Max Horiz 1=259	(LC 16)	bearing plat	chanical connection (by c e capable of withstanding							
		(LC 16), 6=-129 (LC 16 § (LC 16)	5, 129 lb upl	lift at joint 6 and 136 lb up designed in accordance	olift at joi	nt 7.					
		(LC 27), 5=210 (LC 22) (LC 22), 7=405 (LC 22)), International	I Residential Code sectio	ns R502.	.11.1 and					
FORCES	(lb) - Maximum C	compression/Maximum	ΓΟυΖ. Ι U.Ζ a	and referenced standard Standard	ANSI/TPI	11.					
TOP CHORD		3=-193/90, 3-4=-118/56	б,								
BOT CHORD	4-5=-176/94 1-7=-2/4, 6-7=-2/4	4, 5-6=-2/4									
WEBS	3-6=-465/247, 2-7	7=-308/243								an.	
NOTES	E 7-16: \/ult=115m	nph (3-second gust)							OF I	MISSO	
 Wind: ASC 	~ <i>r</i> -ro, vuit_rrom								F. NO	-0, W	
Vasd=91m	ph; TCDL=6.0psf; I	BCDL=6.0psf; h=35ft; osed: MWFRS (envelop	be)					6		No. N	
Vasd=91m Ke=1.00; C exterior zor	ph; TCDL=6.0psf; Cat. II; Exp C; Enclo ne and C-C Exterio	osed; MWFRS (envelop or(2E) 0-7-9 to 5-7-9,						E.	SCOT	тм.	
Vasd=91m Ke=1.00; C exterior zon Interior (1) exposed ;C	ph; TCDL=6.0psf; I Cat. II; Exp C; Enclo ne and C-C Exterio 5-7-9 to 12-6-2 zor C-C for members ar	osed; MWFRS (envelop or(2E) 0-7-9 to 5-7-9, ne; cantilever left and ri nd forces & MWFRS for	ight						S SCOT	тм.	
Vasd=91m Ke=1.00; C exterior zon Interior (1) exposed ;C reactions s DOL=1.60	ph; TCDL=6.0psf; I Cat. II; Exp C; Enclo ne and C-C Exterio 5-7-9 to 12-6-2 zor C-C for members ar hown; Lumber DO	osed; MWFRS (envelop or(2E) 0-7-9 to 5-7-9, ne; cantilever left and ri nd forces & MWFRS for L=1.60 plate grip	ight r							тм.	
Vasd=91m Ke=1.00; C exterior zon Interior (1) exposed ;C reactions s DOL=1.60 2) Truss desig	ph; TCDL=6.0psf; I Cat. II; Exp C; Enclo ne and C-C Exterio 5-7-9 to 12-6-2 zor C-C for members ar hown; Lumber DOI gned for wind loads	osed; MWFRS (envelop or(2E) 0-7-9 to 5-7-9, ne; cantilever left and ri nd forces & MWFRS for L=1.60 plate grip s in the plane of the trus	ight r ss						SEV.	T M. IER	
Vasd=91m Ke=1.00; C exterior zoo Interior (1) reactions s DOL=1.60 2) Truss desig only. For s see Standa	ph; TCDL=6.0psf; I cat. II; Exp C; Enclo ne and C-C Exterio 5-7-9 to 12-6-2 zor C-C for members ar hown; Lumber DOI gned for wind loads studs exposed to w ard Industry Gable	osed; MWFRS (envelop or(2E) 0-7-9 to 5-7-9, ne; cantilever left and ri nd forces & MWFRS for L=1.60 plate grip s in the plane of the trus ind (normal to the face) End Details as applicat	ight r ss), ble,						SEV. NUM PE-2001	T M. HER 018807	
Vasd=91m Ke=1.00; C exterior zoo Interior (1) exposed ;C reactions s DOL=1.60 ?) Truss desig only. For s see Standa	ph; TCDL=6.0psf; I cat. II; Exp C; Enclo ne and C-C Exterio 5-7-9 to 12-6-2 zor C-C for members ar hown; Lumber DOI gned for wind loads studs exposed to w ard Industry Gable	osed; MWFRS (envelop or(2E) 0-7-9 to 5-7-9, ne; cantilever left and ri nd forces & MWFRS for L=1.60 plate grip s in the plane of the trus ind (normal to the face)	ight r ss), ble,						SEV.	T M. HER 018807	





