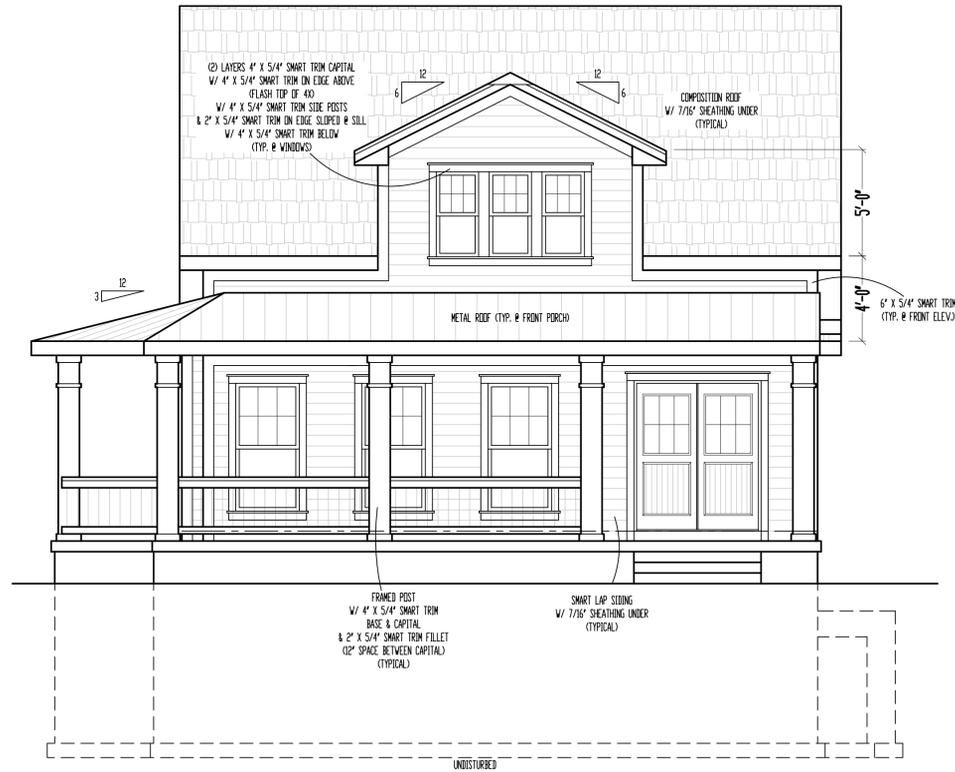
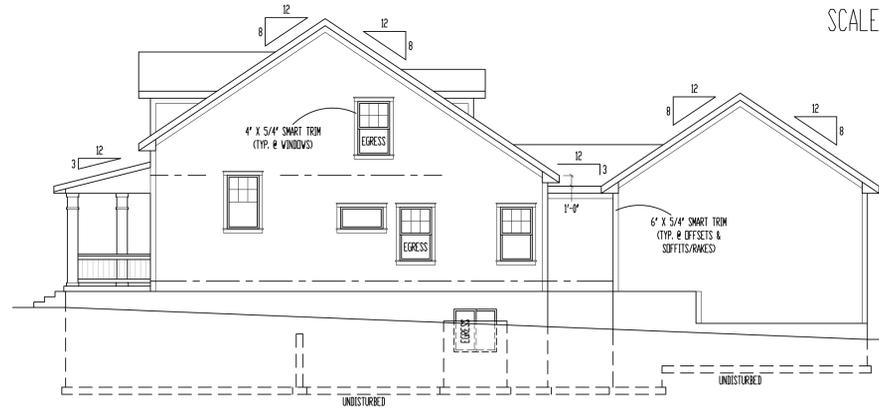


NOTE: GOVERNING CODES &
GENERAL CONTRACTOR'S WRITTEN SPECIFICATIONS
TAKE PRECEDENCE OVER THESE PLANS.

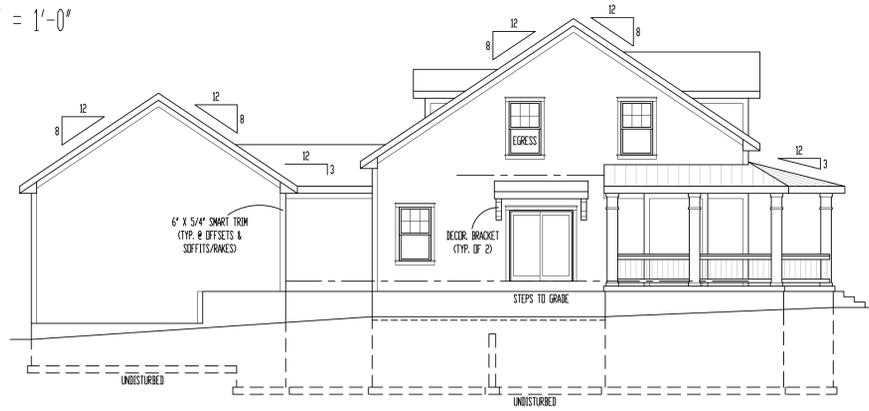


FRONT ELEVATION

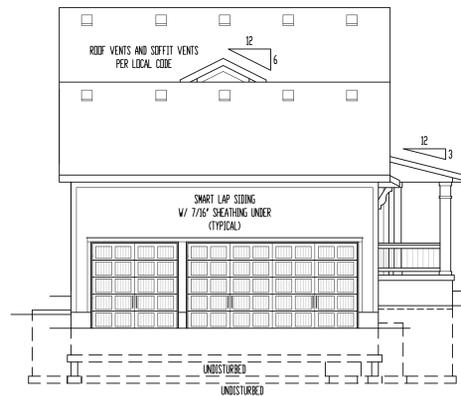
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/8" = 1'-0"



LEFT ELEVATION
SCALE: 1/8" = 1'-0"



REAR ELEVATION
SCALE: 1/8" = 1'-0"

**RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
09/16/2024 4:46:21**

- ELEVATIONS:
- SMART-LAP SIDING ON ALL ELEVATIONS
 - COMPOSITION ROOF SHINGLES
 - LOCATE ROOF AND SOFFIT VENTS PER CODE
 - ADJUST FOUNDATION TO GRADE
- OPTIONAL DECK:
- DECK CONSTRUCTION TO COMPLY WITH MUNICIPALITY'S RESIDENTIAL DECK STANDARDS
 - 2" X 10" @ 16" TYP. @ 16" O.C. FLOOR JOISTS (MAX. SPAN 14'-0")
 - 2" X 6" CEDAR DECKING
 - 6" X 6" CEDAR/PTD. POSTS
 - ALUMINUM SPINDLES
 - 2" X 6" CEDAR TOP RAIL
 - DETERMINE OPTIONAL STAIRS ON SITE

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"For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16).



Site Description:
Lot 129, Pergola Park - 5th Plat
Street Address:
**1101 SW Corinthian Ln.,
Lee's Summit, Missouri**

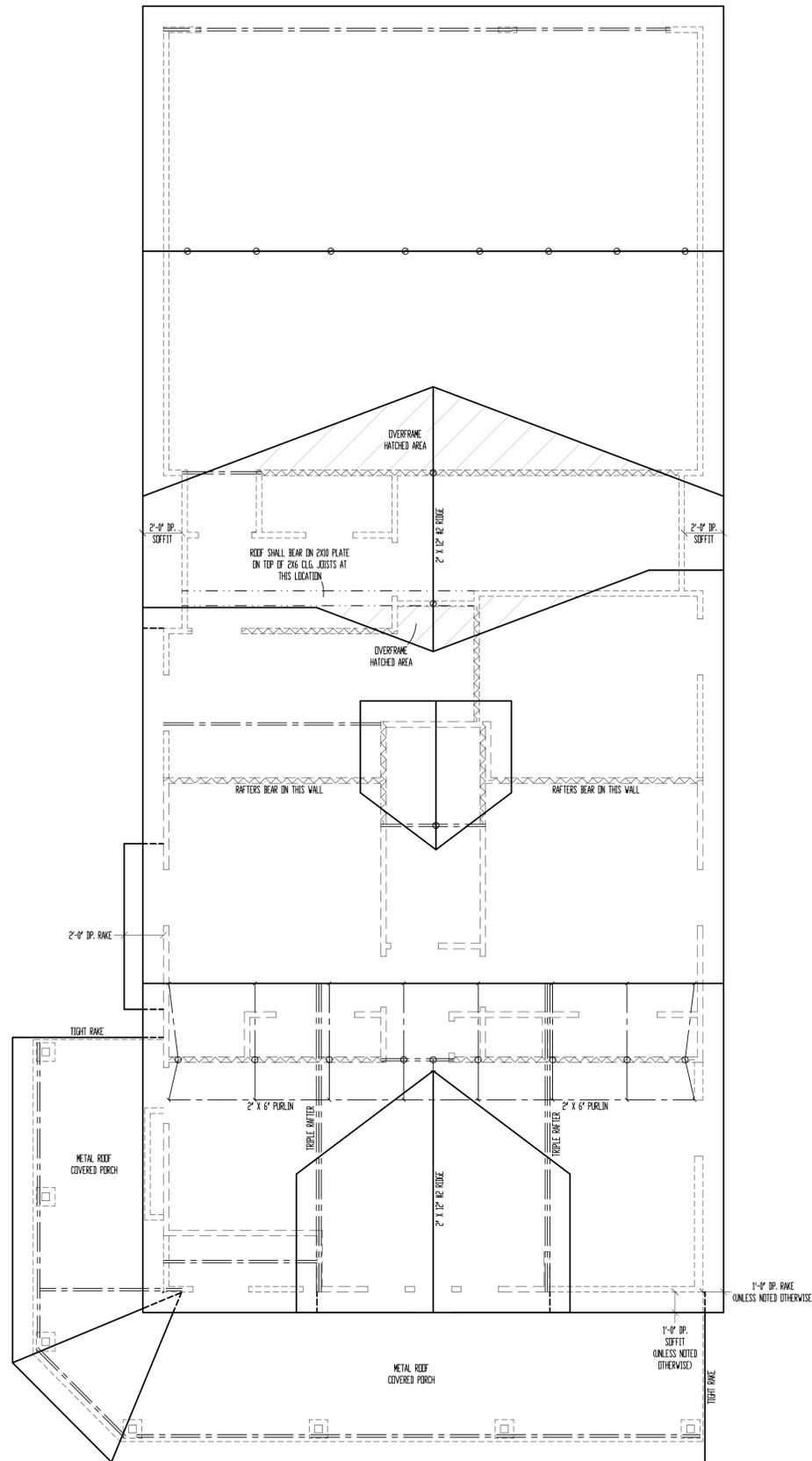
Designed for:
Jeff & Barb HENSON
Belle-Hamilton
The NORTHAMPTON



Date: 8-16-AD 2024
Rev. 1: 9-11-AD 2024
Rev. 2:
Rev. 3:

Sheet Title:
ELEVATIONS

Sheet No.:
A-1 of 5



ROOF
SCALE: 1/4" = 1'-0"

WALL RAFTERS SHALL BE 2" X 6" @ 16" O.C., UNLESS NOTED OTHERWISE.
SEE DETAIL 7/322 FOR ALTERNATE RAFTER BEARING DETAIL WHEN RAFTERS ARE REQUIRED TO BEAR HIGHER THAN THE WALL DOUBLE TOP PLATE.

FLASHING NOTE:
DRIP EDGE, VALLEYS AND FLASHINGS TO BE METAL CLAD.

ROOF NOTES:
ROOF DESIGNED FOR LIGHT ROOF COVERING
(30psf TOTAL LOAD 10psf DL, 20psf LL (SL))

* RAFTERS (HEM-FIR, DOUG-FIR, OR EQUAL)
SEE SPAN CHARTS BELOW

CODE MINIMUM

| RAFTERS | SPACING | MAX HORIZONTAL CLEARSPAN |
|---------|-----------|--------------------------|
| #2-2x6 | #8" O.C. | 11'-7" |
| #2-2x6 | #16" O.C. | 14'-2" |
| #2-2x8 | #8" O.C. | 14'-0" |
| #2-2x8 | #16" O.C. | 17'-0" |
| #2-2x10 | #8" O.C. | 17'-10" |
| #2-2x10 | #16" O.C. | 21'-11" |

NOTE: CODE MINIMUM ALLOWS FOR A RAFTER DEFLECTION OF L/180 TOTAL LOAD

HIGHER PERFORMANCE (RECOMMENDED)

| RAFTERS | SPACING | MAX HORIZONTAL CLEARSPAN |
|---------|-----------|--------------------------|
| #2-2x6 | #8" O.C. | 8'-6" |
| #2-2x6 | #16" O.C. | 9'-9" |
| #2-2x8 | #8" O.C. | 11'-3" |
| #2-2x8 | #16" O.C. | 12'-9" |
| #2-2x10 | #8" O.C. | 14'-9" |
| #2-2x10 | #16" O.C. | 18'-9" |

REFLECTION = L/240 LIVE LOAD, L/240 TOTAL LOAD

- * VAULTS TO BE 2x10 DEPTH
- * RIDGE BRACKETS ARE UNLESS OTHERWISE NOTED:
 - #2-2x8 UP TO 10/12 PITCH
 - #2-2x10 OVER 10/12 PITCH
- * ALL HIPS & VALLEYS ARE UNLESS OTHERWISE NOTED:
 - #2-2x8 UP TO 10/12 PITCH
 - #2-2x10 OVER 10/12 PITCH
- * PURLINS ARE 2x6 MIN.
 - PURLIN STRUTS ARE AT 4'-0" O.C.
 - PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 DEGREE ANGLE WITH THE HORIZONTAL
 - ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED LENGTH OF 8'-0"
 - PURLINS STRUTS SHALL BE CONSTRUCTED IN A "Y" CONFIGURATION AND PER THE FOLLOWING CHART:

| PURLIN STRUT | MAX PURLIN STRUT LENGTH |
|-----------------------|-------------------------|
| (2) 2x4 | 8'-0" |
| (1) 2x4 & (1) 2x6 | 12'-0" |
| (1) 2x6 & (1) 2x8 | 20'-0" |
| (2) 2x6 & (1) 2x8 | 30'-0" |
| (CONSULT ARCH/ENGR.) | 30'-0" |

- * RIDGE BRACKES ARE SAME AS PURLIN BRACKES- SPACING, SIZE, CONFIGURATION & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE)
- * HIP & VALLEY BRACKES ARE SAME AS PURLIN SIZE, CONFIGURATION & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE)

- * VERTICAL BRACE IF DOT IS UNDER HIP OR VALLEY
- * SLASH IS TOP END OF BRACE (/), DOT IS BOTTOM OF BRACE (o)
- * --- DENOTES BEARING WALL
- * --- DENOTES ROOF BRACE
- * --- DENOTES PURLIN
- * --- DENOTES BEARING STRUCTURE

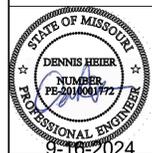
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Site Description:
Lot 129, Pergola Park - 5th Plat
Street Address:
1101 SW Corinthian Ln., Lee's Summit, Missouri

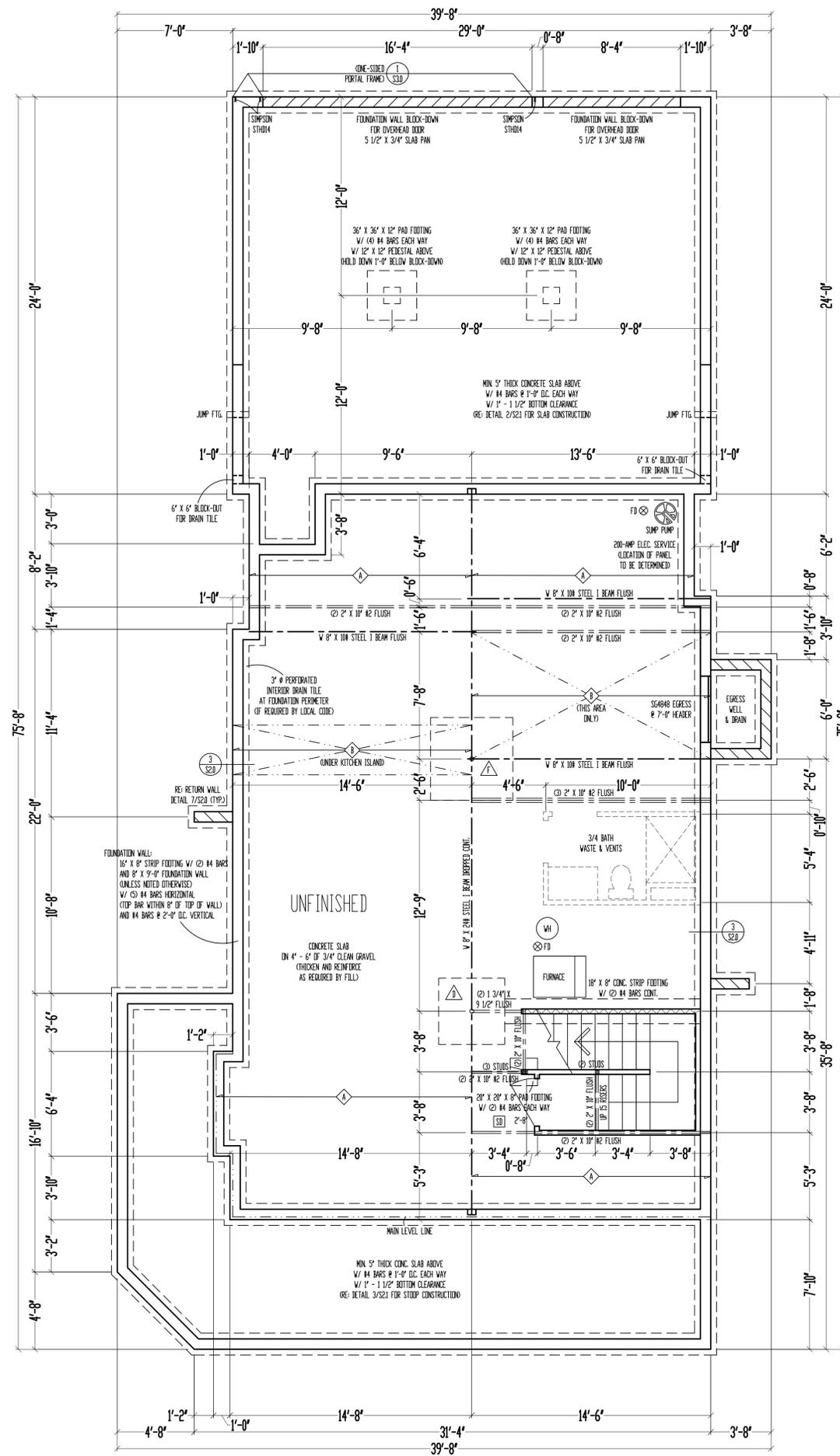
Designed for:
Jeff & Barb HENSON
Barn Plan
The NORTHAMPTON



Date: 8-16-AD 2024
Rev. 1: 9-11-AD 2024
Rev. 2:
Rev. 3:

Sheet Title:
ROOF PLAN

Sheet No.:
A-2 of 5



9'-0" FOUNDATION WALLS
(UNLESS NOTED OTHERWISE)
ON 16" X 8" STRIP FOOTINGS
(STEP WHERE GRADE REQUIRES)

2" X 10" FLOOR SYSTEM ABOVE
FOUNDATION
SCALE: 1/4" = 1'-0"

***** = WALL BRACING PER FRAMING NOTE #1 AND PER CALCULATIONS ON SHEET S11.

- FRAMING NOTES:**
1. BASEMENT LEVEL EXTERIOR WOOD-FRAMED WALLS SHALL BE SHEATHED W/ 7/16" OSB. APA PANELS W/ #4 COMMON WALLS @ 6" O.C. AT EDGES & @ 12" O.C. IN THE FIELD. SMART PANEL, OR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS.
 2. MINIMUM GYPSUM BOARD OVER STUDS SPACED 24" MAX FASTENED W/ NO. 6 - 1 1/4" TYPE W OR S DRYWALL SCREWS @ 7" O.C. EDGES & FIELD. ON 8'-0" SECTIONS ONE SIDE OF WALL. OR MIN. 4'-0" SECTION FOR BOTH SIDES.
 3. LEAD BEARING INTERIOR WALL.
 4. 2" X 10" #2 HEADER AT ALL EXTERIOR AND LEAD BEARING WALLS, UNLESS NOTED OTHERWISE.
 5. LEAD TIES @ 4'-0" O.C. (TYPICAL).
 6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS.
 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LEAD BEARING WALLS WITH JOIST MATERIAL NOT REQUIRED WITH I-JOISTS.
 8. PROVIDE MULTIPLE STUDS FOR SLOID BEARING BELOW ALL BEAMS.
 9. ALL DESIGNATED 2" X 6" WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS.
 10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE.
 11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2" X 4'S @ 16" O.C. UNLESS NOTED OTHERWISE.
 12. 1/2" # ANCHOR BOLTS W/ MIN. 2" EMBEDMENT @ 48" O.C. MAX & WITHIN 6" - 12" OF END OF EACH PLATE LENGTH.
 13. NEW FOUNDATION SHALL BEAR ON ORIGINAL SOIL WITH MINIMUM BEARING CAPACITY OF 1500 PSF. A GEOTECHNICAL ENGINEER IS RECOMMENDED FOR VERIFICATION OF THESE CONDITIONS DURING THE EXCAVATION PHASE. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED IN ANYTHING SHORT OF THE AFORESAID REQUIREMENTS.

| STEEL COLUMN & PAD FOOTING SCHEDULE | | JOIST SCHEDULE | |
|-------------------------------------|--|----------------|--|
| ▲ | 3" X 11 GA. STEEL COLUMN ON 36" X 36" X 10" PAD FOOTING W/ (6) #4 BARS EACH WAY (2530) | ◇ | 2" X 10" #2 FLOOR JOIST @ 16" O.C. |
| ▲ | 3 1/2" X 11 GA. STEEL COLUMN ON 36" X 36" X 10" PAD FOOTING W/ (6) #4 BARS EACH WAY (2530) | ◇ | 2" X 10" #2 FLOOR JOIST @ 16" O.C. - DOUBLED |
| ▲ | 5" S30X 40 STEEL COLUMN ON 48" X 48" X 12" PAD FOOTING W/ (5) #4 BARS EACH WAY (2430) | | |
| ▲ | 3 1/2" S30X 40 STEEL COLUMN ON 48" X 48" X 12" PAD FOOTING W/ (6) #4 BARS EACH WAY (2520) | | |
| ▲ | 3 1/2" S30X 40 STEEL COLUMN ON 54" X 54" X 14" PAD FOOTING W/ (7) #4 BARS EACH WAY (4030) | | |
| ▲ | 3 1/2" S30X 40 STEEL COLUMN ON 60" X 60" X 14" PAD FOOTING W/ (8) #4 BARS EACH WAY (5030) | | |

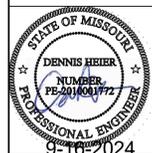
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Site Description:
Lot 129, Pergola Park - 5th Plat
Street Address:
1101 SW Corinthian Ln., Lee's Summit, Missouri

Designed for:
Jeff & Barb HENSON
Basement Plan
The NORTHAMPTON



Date: 8-16-AD 2024
Rev. 1: 9-11-AD 2024
Rev. 2:
Rev. 3:

Sheet Title:
FOUNDATION PLAN

Sheet No.:
A-5 of 5

| FASTENER SCHEDULE FOR STRUCTURAL MEMBERS | | |
|--|---|---|
| DESCRIPTION OF BUILDING ELEMENTS | NUMBER AND TYPE OF FASTENER | SPACING AND LOCATION |
| ROOF¹ | | |
| BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOE NAIL | 4-8d (2 $\frac{1}{2}$ " x 0.113") | TOENAIL |
| CEILING JOISTS TO PLATE, TOE NAIL | 4-8d (2 $\frac{1}{2}$ " x 0.113") | PER JOIST, TOENAIL |
| CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS, FACE NAIL | 4-10d (3" x 0.128") | FACE NAIL |
| CEILING JOIST TO PARALLEL RAFTER (HEEL JOINT) | TBLE R802.5.2 | FACE NAIL |
| COLLAR TIE TO RAFTER, FACE NAIL OR 1 $\frac{1}{2}$ " x 2 $\frac{1}{2}$ " GA. RIDGE STRAP TO RAFTER | 4-10d (3" x 0.128") | FACE NAIL, EACH RAFTER |
| RAFTER OR ROOF TRUSS TO PLATE | 3-16d BOX NAILS (3 $\frac{1}{2}$ " x 0.135") OR 3-10d COMMON NAILS (3" x 0.148") | 2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS |
| ROOF RAFTERS TO RIDGE, VALLEY, OR HIP RAFTERS OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM | 4-16d (3 $\frac{1}{2}$ " x 0.135") - TOENAIL, 3-16d BOX (3 $\frac{1}{2}$ " x 0.135") - END NAIL | TOENAIL, END NAIL |
| WALL | | |
| STUD TO STUD (NOT AT BRACED WALL PANELS) | 10d (3" x 0.128") | 16" O.C. FACE NAIL |
| STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS) | 16d (3 $\frac{1}{2}$ " x 0.135") | 12" O.C. FACE NAIL |
| BUILT-UP HEADER, TWO PIECES WITH 1 $\frac{1}{2}$ " SPACER | 16d (3 $\frac{1}{2}$ " x 0.135") | 12" O.C. EACH EDGE FACE NAIL |
| CONTINUOUS HEADER TO STUD | 4-8d (2 $\frac{1}{2}$ " x 0.131") | TOENAIL |
| TOP PLATE TO TOP PLATE | 10d (3" x 0.128") | 12" O.C. FACE NAIL |
| DOUBLE TOP PLATE SPLICE | 8-16d COMMON (3 $\frac{1}{2}$ " x 0.162") | FACE NAIL ON EACH SIDE OF END JOINT (MIN. 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT) |
| BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (NOT AT BRACED WALL PANELS) | 16d COMMON (3 $\frac{1}{2}$ " x 0.162") | 16" O.C. FACE NAIL |
| BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (AT BRACED WALL PANEL) | 3-16d BOX (3 $\frac{1}{2}$ " x 0.135") | 3 EACH 16" O.C. FACE NAIL |
| TOP OR SOLE PLATE TO STUD, END NAIL | 4-8d BOX (2 $\frac{1}{2}$ " x 0.113") - TOENAIL, 3-16d BOX (3 $\frac{1}{2}$ " x 0.135") - END NAIL | TOENAIL, END NAIL (SEE LEFT) |
| TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS | 3-10d BOX (3" x 0.128") | FACE NAIL |
| 1" BRACE TO EACH STUD AND PLATE | 3-8d BOX (2 $\frac{1}{2}$ " x 0.113") | FACE NAIL |
| 1"x6" SHEATHING TO EACH BEARING | 3-8d BOX (2 $\frac{1}{2}$ " x 0.113") | FACE NAIL |
| 1"x8" SHEATHING TO EACH BEARING | 3-8d BOX (2 $\frac{1}{2}$ " x 0.113") - FACE NAIL, WIDER THAN 1"x8" - 4-8d BOX (2 $\frac{1}{2}$ " x 0.113") | FACE NAIL |
| FLOOR | | |
| JOIST TO SILL, TOP PLATE, OR GIRDER | 4-8d BOX (2 $\frac{1}{2}$ " x 0.113") | TOE NAIL |
| RIM JOIST, BAND JOIST, OR BLOCKING TO SILL OR TOP PLATE (ROOF APPLICATIONS ALSO) | 8d BOX (2 $\frac{1}{2}$ " x 0.113") | 4" O.C. TOE NAIL |
| 1" x 6" SUBFLOOR OR LESS TO EACH JOIST | 3-8d BOX (2 $\frac{1}{2}$ " x 0.113") | FACE NAIL |
| 2" SUBFLOOR TO JOIST OR GIRDER | 3-16d BOX (3 $\frac{1}{2}$ " x 0.135") | BLIND AND FACE NAIL |
| 2" PLANKS (PLAN & BEAM - FLOOR AND ROOF) | 3-16d BOX (3 $\frac{1}{2}$ " x 0.135") | AT EACH BEARING, FACE NAIL |
| BAND OR RIM JOIST TO JOIST | 3-16d COMMON (3 $\frac{1}{2}$ " x 0.162") | END NAIL |
| BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS | 10d BOX (3" x 0.128") | 24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES |
| LEDGER STRIP SUPPORTING JOISTS OR RAFTERS | 4-16d BOX (3 $\frac{1}{2}$ " x 0.135") | AT EACH JOIST OR RAFTER, FACE NAIL |
| BRIDGING OR BLOCKING TO JOIST | 2-10d BOX (3" x 0.128") | EACH END, TOENAIL |

| FASTENER SCHEDULE FOR STRUCTURAL MEMBERS | | | |
|--|--|-----------------------|--------------------------------|
| DESCRIPTION OF BUILDING MATERIALS | DESCRIPTION OF FASTENER | EDGE SPACING (INCHES) | INTERMEDIATE SUPPORTS (INCHES) |
| WOOD STRUCTURAL PANELS, SUBFLOOR, ROOF AND INTERIOR WALL SHEATHING TO FRAMING AND PARTICLEBOARD WALL SHEATHING TO FRAMING | | | |
| 1/2" - 1/2" | 8d COMMON (2" x 0.113") NAIL (SUBFLOOR, WALL) 8d COMMON NAIL (ROOF) | 6 | 12 |
| 1/2" - 1" | 8d COMMON NAIL (2 $\frac{1}{2}$ " x 0.131") | 6 | 12 |
| 1/2" - 1/2" | 10d COMMON (3" x 0.148") NAIL OR 8d (2 $\frac{1}{2}$ " x 0.131") DEFORMED NAIL | 6 | 12 |
| OTHER WALL SHEATHING¹ | | | |
| 1/2" STRUCTURAL CELLULOIC FIBERBOARD SHEATHING | 1 1/2" GALVANIZED ROOFING NAIL, 1/2" HEAD DIAMETER, OR 1 1/2" LONG 16 GA. STAPLE WITH 1/4" OR 1" CROWN | 3 | 6 |
| 5/8" STRUCTURAL CELLULOIC FIBERBOARD SHEATHING | 1 3/4" GALVANIZED ROOFING NAIL, 1/2" HEAD DIAMETER, OR 1 3/4" LONG 16 GA. STAPLE WITH 1/4" OR 1" CROWN | 3 | 6 |
| 1/2" GYPSUM SHEATHING | 1 1/2" GALVANIZED ROOFING NAIL, STAPLE GALVANIZED, 1 1/2" LONG, 1/4" SCREWS, TYPE W OR S | 7 | 7 |
| 5/8" GYPSUM SHEATHING | 1 3/4" GALVANIZED ROOFING NAIL, STAPLE GALVANIZED, 1 3/4" LONG, 1/4" SCREWS, TYPE W OR S | 7 | 7 |
| WOOD STRUCTURAL PANELS, COMBINATION SUBFLOOR UNDERLAYMENT TO FRAMING | | | |
| 1/2" AND LESS | 8d DEFORMED (2" x 0.120") NAIL OR 8d COMMON (2 $\frac{1}{2}$ " x 0.131") NAIL | 6 | 12 |
| 1/2" - 1" | 8d COMMON (2 $\frac{1}{2}$ " x 0.131") NAIL OR 8d DEFORMED (2 $\frac{1}{2}$ " x 0.120") NAIL | 6 | 12 |
| 1/2" - 1 1/2" | 10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (2 $\frac{1}{2}$ " x 0.120") NAIL | 6 | 12 |

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION LISTED IN THIS TABLE

FOUNDATION NOTES

- CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLABS
- THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTIONS RESIDENTIAL FOUNDATION STANDARDS
- PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT
- FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.
- FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLEARANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE FOR FROST PROTECTION
- CONCRETE PADS SUPPORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE
- FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0
- REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE)
- INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND, GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES
- IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB SHALL BE DESIGNED BY A LICENSED ENGINEER
- SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH 2" x 8" ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6" x 2'-0" O.C. (OR AS NOTED ON PLANS) AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6
- FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET S2.0
- THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

- ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS
- ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 - 2x10's, UNLESS NOTED OTHERWISE ON PLANS
- BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS
- INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A MINIMUM OF 1/2"
- ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED OTHERWISE
- WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4'S TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS.
- ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT MATERIAL
- JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.
- JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS
- JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 1 1/2" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT
- ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3
- ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3
- ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER 1/3 OF VERTICAL DISTANCE BETWEEN CEILING AND ROOF
- BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED
- PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A 1/2" GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE RATED SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION)
- ENGINEERED LVL'S SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 1900 ksi, AND Fv = 285 psi
- ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi
- COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. 1/2" x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1:92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.
- WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING.
- ALL ROOF SHEATHING SHALL BE 5/8" OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

- GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0"
- ALL OPERABLE WINDOWS SHALL HAVE FLAT PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

- ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH 1/2" TO 1/2" OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/60 OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/100.

EMERGENCY EGRESS

- PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP.
- PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

MASONRY VENEER

- MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1/2", WITH NOT LESS THAN 3/8" MORTAR OR GROUT COVER TO OUTSIDE FACE.
- VEENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE WIRE 1/2" CORRUGATED.
- EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.
- VEENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

GARAGE NOTES

- DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1 1/2" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE
- VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST LOADING PER DASHA 108 AND ASTM E 330-96 PER IRC 2018

| MULTIPLY WOOD BEAM FASTENING SCHEDULE | | | |
|---------------------------------------|---|--------------------------------|---------------------------------------|
| DIMENSIONAL LUMBER BEAM SIZE/TYPE | FASTENERS | LVL BEAM SIZE/TYPE | FASTENERS |
| (2) 2x | (2) ROWS 10d @ 12" O.C. ONE SIDE | (2) 1 3/4" UP TO 11 1/2" DEPTH | (2) ROWS 16d @ 12" O.C. ONE SIDE |
| (3) 2x | (2) ROWS 10d @ 12" O.C. BOTH SIDES | (2) 1 3/4" 14" DEPTH | (3) ROWS 16d @ 12" O.C. ONE SIDE |
| (4) 2x | (2) ROWS 1/2" x 5" SIMPSON SDS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM, BOTH SIDES | (3) 1 3/4" UP TO 11 1/2" DEPTH | (2) ROWS OF 16d @ 12" O.C. BOTH SIDES |

GARAGE NOTES (CONTINUED)

- THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 1/2" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM 1/2" TYPE X GYP. BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH 1/2" GYP. BOARD.
- GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING AND SHALL BE FASTENED WITH 2x2" x 0.120" NAILS AT 7" O.C. STAGGERED WITH (7) 3/4" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER. MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

DESIGN LOADING (PER TABLE R301.5)

| USE | MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS (PSF) | |
|--|--|----------------------------------|
| | LIVE LOAD | DEAD LOAD |
| UNINHABITABLE ATTICS WITHOUT STORAGE | 10 | 10 |
| UNINHABITABLE ATTICS WITH LIMITED STORAGE | 20 | 10 |
| HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS | 30 | 10 |
| BALCONIES (EXTERIOR) AND DECKS | 40 | 10 ^d |
| FIRE ESCAPES | 40 | 10 |
| GUARDRAILS AND HANDRAILS ^a | 200 ^c | - |
| GUARDRAIL IN-FILL COMPONENTS ^b | 50 ^c | - |
| PASSENGER VEHICLE GARAGES | 50 | DEPENDENT UPON SLAB CONSTRUCTION |
| ROOMS OTHER THAN SLEEPING ROOM | 40 | 10 ^d |
| SLEEPING ROOM | 30 | 10 ^d |
| STAIRS | 40 | 10 ^d |

- A single concentrated load applied in any direction at any point along the top.
- Guard-in-fill components (all those except the handrail), balusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement.
- Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load.
- An additional dead loading of 10 psf shall be applied where thinsert tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed.

INSULATION/EFFICIENCY

- BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1.

| INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT (TABLE N1102.1.1) | |
|---|----------------------------|
| CLIMATE ZONE | 4-A |
| FENESTRATION U-FACTOR | 0.35 |
| SKYLIGHT U-FACTOR | 0.55 |
| GLAZED FENESTRATION SHGC | 0.40 |
| CEILING R-VALUE | 49 |
| WOOD FRAME WALL R-VALUE | 15 |
| MASS WALL R-VALUE | 8 / 13 |
| FLOOR R-VALUE | 19 |
| BASEMENT WALL R-VALUE | 10-CONTINUOUS OR 13-CAVITY |
| SLAB R-VALUE AND DEPTH | 10 AT 2'-0" |
| CRAWL SPACE WALL R-VALUE | 10-CONTINUOUS OR 13-CAVITY |
| DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE | 8 |
| DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE | 6 |
| CATHEDRAL VAULTED CEILING R-VALUE | 38 |

DUCT SEALING

- N1102.2.2 (R302.2) SEALING (MANDATORY): DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2018 IRC.

- EXCEPTIONS:**
- AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT SEALS.
 - WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
 - CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.

- DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:
- POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTHS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST.
 - ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA.
- EXCEPTION:** THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

| MECHANICAL VENTILATION SYSTEM FAN EFFICACY | | | |
|--|-----------------------------|-----------------------------|-----------------------------|
| FAN LOCATION | AIR FLOW RATE MINIMUM (CFM) | MINIMUM EFFICACY (CFM/WATT) | AIR FLOW RATE MAXIMUM (CFM) |
| RANGE HOODS | ANY | 2.8 | ANY |
| IN-LINE FAN | ANY | 2.8 | ANY |
| BATHROOM, UTILITY ROOM | 10 | 1.4 | 90 |
| BATHROOM, UTILITY ROOM | 90 | 2.8 | ANY |

VISTA
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CLIENT: JEFF & BARBARA HENSON
JOB TITLE: PGP129 HENSON LOT 129, PERGOLA PARK - 5TH PLAT
LOCATION: 1101 SW CORINTHIAN LN, LEE'S SUMMIT, MISSOURI

STATE OF MISSOURI
DENNIS HEIER
NUMBER PE-2018001772
PROFESSIONAL ENGINEER
9-16-2024

| NO. | DATE | REVISION | BY |
|-----|------|----------|----|
| | | | |
| | | | |
| | | | |
| | | | |

DRAWING TITLE
STRUCTURAL NOTES

ENGINEER: DMH CHECKED BY: DMH
JOB NO. DRAWN BY: DMH
DATE: 09-16-24
SHEET NUMBER

S10

RELEASE FOR CONSTRUCTION
THIS NOTED ONLY FOR REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
09/16/2024 4:46:21

RESIDENTIAL SEISMIC & WIND ANALYSIS

DETERMINE WEIGHT OF HOUSE:

| LOCATION | DEAD LOAD (psf) | AREA (ft ²) | WEIGHT (lbs.) |
|-------------------------------------|------------------|-------------------------|---------------------|
| ROOF | 10 | 2404 | 24040 |
| CEILING | 10 | 2404 | 24040 |
| SECOND FLOOR | 10 | 668 | 6680 |
| FIRST FLOOR | 10 | 2404 | 24040 |
| SECOND FLOOR EXT. WALL DL | WALL LENGTH (ft) | WALL HEIGHT (ft) | WALL UNIT WT. (psf) |
| | 119 | 9 | 939 |
| FIRST FLOOR EXT. WALL DL | 195.34 | 10 | 19534 |
| SECOND FLOOR INT. PARTITION WALL DL | DEAD LOAD (psf) | AREA (ft ²) | WEIGHT (lbs) |
| | 5 | 668 | 4006 |
| FIRST FLOOR INT. PARTITION WALL DL | 5 | 2404 | 14424 |

| PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED) | | | | | |
|---|--------|---------------------------------|--------------|--------|-------------------------|
| FRONT-TO-BACK | | | SIDE-TO-SIDE | | |
| AREA | LOAD | | AREA | LOAD | |
| SLOPED ROOF | 220 | | SLOPED ROOF | 86 | |
| VERT. ROOF | 472 | CUMULATIVE | VERT. ROOF | 212 | CUMULATIVE |
| 2ND | 290 | 561 | 2ND | 305 | 591 |
| 1ST | 330 | 1064 | 1ST | 744.37 | 1575 |
| | | PRESSURE (PSF) - PER ASCE CH. 6 | | | |
| SLOPED ROOF | ZONE B | 9.7 | ZONE C | 11.3 | 2a (FIG. 28.6-1, ASCE7) |
| WALL/VERT. ROOF | ZONE A | 14.2 | ZONE D | 7.7 | 6 |
| MEAN ROOF HT., ft | | 23 | | | |

a) If there is a breakout wall to be sheathed, determine tributary wind area and enter here. If no breakout, enter 0 for area.
 $q_{net} = 0.0259K_d K_{e1} K_{z1} V^2$ (ASCE7-16 Velocity Pressure) $q_{net, ASD} = 0.6d_{z1,10}$ (Design Velocity Pressure for ASD analysis under ASCE7-16 and IRC/IBC 2018)

2ND FLOOR TRIBUTARY WEIGHT

1ST FLOOR TRIBUTARY WEIGHT

S_s (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP)

F_a (from ASCE7 Table 11.4-1)

S_{DS} (= 2.5 * S_s * F_a)

R (from ASCE7 Table 12.2-1)

| |
|---------|
| 52899.5 |
| 78174 |
| 12.0% |
| 1.6 |
| 0.128 |
| 6.5 |

SEISMIC SHEAR

| LOCATION | From ASCE7 (Eq. 12.8-1): | V (= 1.2 * S _{DS} * W / R) (lbs.) |
|-----------|--------------------------|--|
| 2ND FLOOR | | 1250 |
| 1ST FLOOR | | 1847 |

| Sheathing Location | Min. Sheathing Schedule | Fastening Schedule | Allowable Shear (#/LF) | Code Reference |
|----------------------|---|--|------------------------|--------------------------|
| Exterior (Option #1) | 7/16" APA Rated Plywood/OSB | 1-1/2" 15ga. Staples w/ 1" penetration @ 3" O.C. Edges, 6" O.C. Field For 24" stud spacing, 12" O.C. Field For 16" stud spacing | 155 | per IBC, Table 2306.3(1) |
| Exterior (Option #2) | 7/16" APA Rated Plywood/OSB | 1-1/2" 15ga. Staples w/ 1" penetration @ 4" O.C. Edges, 6" O.C. Field For 24" stud spacing, 12" O.C. Field For 16" stud spacing | 230 | per IBC, Table 2306.3(1) |
| Exterior (Option #3) | 7/16" APA Rated Plywood/OSB | 1-1/2" 15ga. Staples w/ 1" penetration @ 3" O.C. Edges, 6" O.C. Field For 24" stud spacing, 12" O.C. Field For 16" stud spacing | 310 | per IBC, Table 2306.3(1) |
| Exterior (Option #4) | 7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing | 8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing | 240 | AWC SDPWS Table 4.3A |
| Exterior (Option #5) | 7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing | 8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing | 350 | AWC SDPWS Table 4.3A |
| Exterior (Option #6) | 7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge | 8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field | 450 | AWC SDPWS Table 4.3A |
| Interior | 1/2" Gypsum Board | No. 6 - 1/4" Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field | 60 | per IBC, Table 2306.4.4 |
| Interior | 16 Ga. Simpson/USP Type WB Steel X-Brace (or equal) | (3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3) | 325 | |

| | |
|--|---|
| EXTERIOR SHEATHING OPTION FOR SECOND FLOOR | 4 |
| EXTERIOR SHEATHING OPTION FOR FIRST FLOOR | 6 |

EXTERIOR SHEATHING OPTION FOR BASEMENT WALLS

| | | | |
|---------------------------|-------|--------------------------|------|
| WIDTH OF 1ST STORY (FT.) | 30 | WIDTH OF 2ND STORY (FT.) | 29 |
| DEPTH OF 1ST STORY (FT.) | 67.67 | DEPTH OF 2ND STORY (FT.) | 30.5 |
| BACK WALL OF GARAGE (FT.) | 0 | | |
| GAR. WALL: 1=F-B, 2=S-S | 2 | | |

| | SEISMIC | | | | WIND | | | |
|-----------|---------------|-------------------|--------------|-------------------|---------------|-------------------|--------------|-------------------|
| | FRONT-TO-BACK | RESISTANCE (lbs.) | SIDE-TO-SIDE | RESISTANCE (lbs.) | FRONT-TO-BACK | RESISTANCE (lbs.) | SIDE-TO-SIDE | RESISTANCE (lbs.) |
| 2ND FLOOR | 35 | 10500 | 31 | 9300 | 35 | 14700 | 31 | 13020 |
| 1ST FLOOR | 77 | 39270 | 11.5 | 5865 | 77 | 54978 | 11.5 | 8211 |

| | ADDITIONAL RESISTANCE REQUIRED | | Anchor Bolt Spacing (in.) | | 16d Nail Spacing req'd at bottom plate (in.) | |
|-------------------------|--------------------------------|------|---------------------------|---------------|--|---------------|
| | SEISMIC | WIND | diameter (in.) | 2nd Floor F-B | 2nd Floor F-S | 1st Floor S-S |
| 2ND FLOOR FRONT-TO-BACK | 0 | 0 | 0.5 | 61 | 23 | 36 |
| 2ND FLOOR SIDE-TO-SIDE | 0 | 0 | 0.5 | 61 | 23 | 36 |
| 1ST FLOOR FRONT-TO-BACK | 0 | 0 | 0.5 | 61 | 23 | 36 |
| 1ST FLOOR SIDE-TO-SIDE | 0 | 7546 | 0.5 | 61 | 23 | 36 |

| RESISTANCE REQUIRED IN ADDITION TO RESISTANCE PROVIDED BY EXTERIOR WALLS** | | | | | | | |
|--|---|--|--------------------------------|---|--|--|-----|
| | ADDITIONAL RESISTANCE REQUIRED (POUNDS) | PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE | INTERIOR X-BRACES (325#/BRACE) | INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) | INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.) | RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS) | OK? |
| 2ND FLOOR FRONT-TO-BACK | 0 | | | | | 0 | YES |
| 2ND FLOOR SIDE-TO-SIDE | 0 | | | | | 0 | YES |
| 1ST FLOOR FRONT-TO-BACK | 0 | | | | | 0 | YES |
| 1ST FLOOR SIDE-TO-SIDE | 7546 | | | 92 | | 7728 | YES |

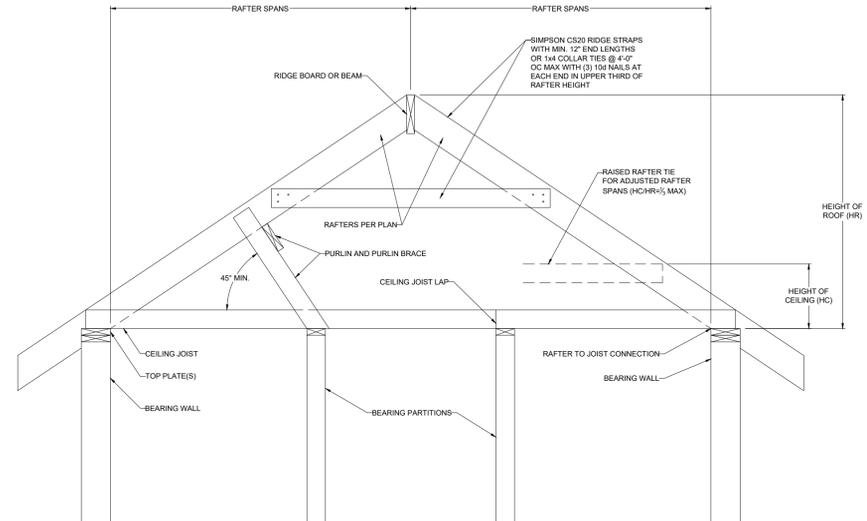
**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE).
 2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER

| WIND UPLIFT ANALYSIS | | | | | | | |
|-------------------------------|--|---|----------------------|----------------------|-------------------|---------------------------------------|--|
| X/12 | DEGREES | PITCH OF 6 OR LESS: EOH -13.3, E -7.2, G -5.2 | | | | | |
| 8 | 33.7 | | | | | | |
| ASCE 7 | | | | | | | |
| LENGTH (FT.) | PRESSURE (PSF) | LINEAL FT. OF OH | UPLIFT PER FT* (LBS) | | | | |
| OVERHANG | -1.08 | 197.34 | -1.08 | | | | |
| TOTAL AREA (FT ²) | ZONE E AREA (FT ²) | ZONE G AREA (FT ²) | PRESSURE ZN. E (PSF) | PRESSURE ZN. G (PSF) | TOTAL FORCE (LBS) | FORCE PER LINEAL FT @ PERIMETER (LBS) | |
| MAIN ROOF** | 2030.1 | 570 | -1.08 | -0.36 | -1141 | -5.8 | |
| *ALONG PERIMETER | TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS) | | | -6.9 | | | |
| **INSIDE EXTERIOR WALLS | RESISTANCE DUE TO DEAD WEIGHT & (3) 16d TOENAILS | | | 251.6 | | | |
| | | | | UPLIFT OK | | | |

NOTE FOR CONSTRUCTION:
 THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX, UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:
 ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2018 IBC SECTION 2306 AND AWC SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC.

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION



1 BRACED RAFTER CONSTRUCTION
 S1.1 SCALE: 1" = 1'-0" (18x20) OR 1/2" = 1'-0" (24x36)

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CLIENT: JEFF & BARBARA HENSON
 JOB TITLE: PGP129 HENSON
 LOT 129, PERGOLA PARK - 5TH PLAT
 LOCATION: 1101 SW CORINTHIAN LN.
 LEE'S SUMMIT, MISSOURI

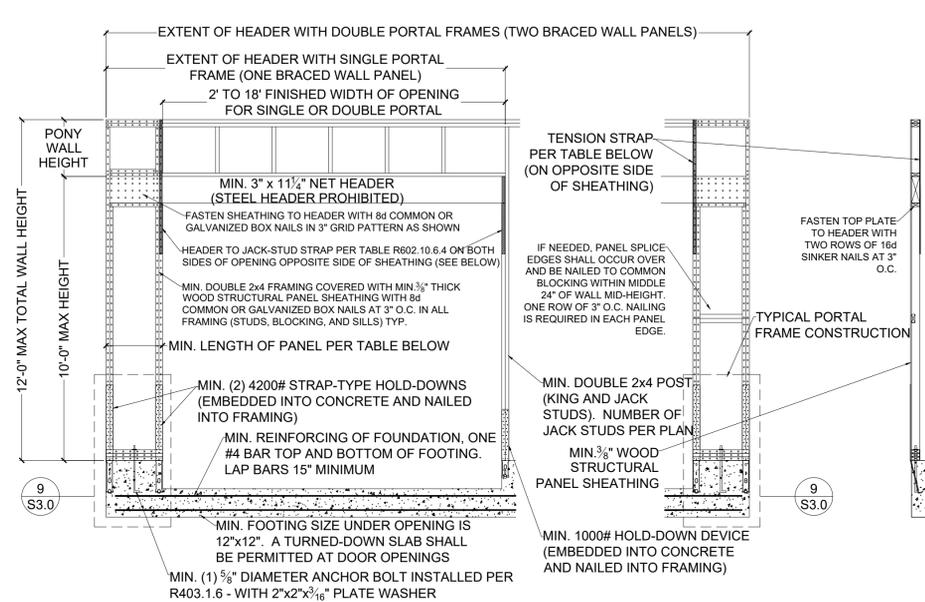
STATE OF MISSOURI
 DENNIS HEIER
 NUMBER: PE-2018001772
 PROFESSIONAL ENGINEER
 9-16-2024

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DRAWING TITLE
STRUCTURAL CALCULATIONS

ENGINEER: DMH CHECKED BY: DMH
 JOB NO. DRAWN BY: DMH
 DATE: 09-16-24
 SHEET NUMBER
S1.1

RELEASE FOR CONSTRUCTION
 AS NOTED ONLY DAYS REVIEW
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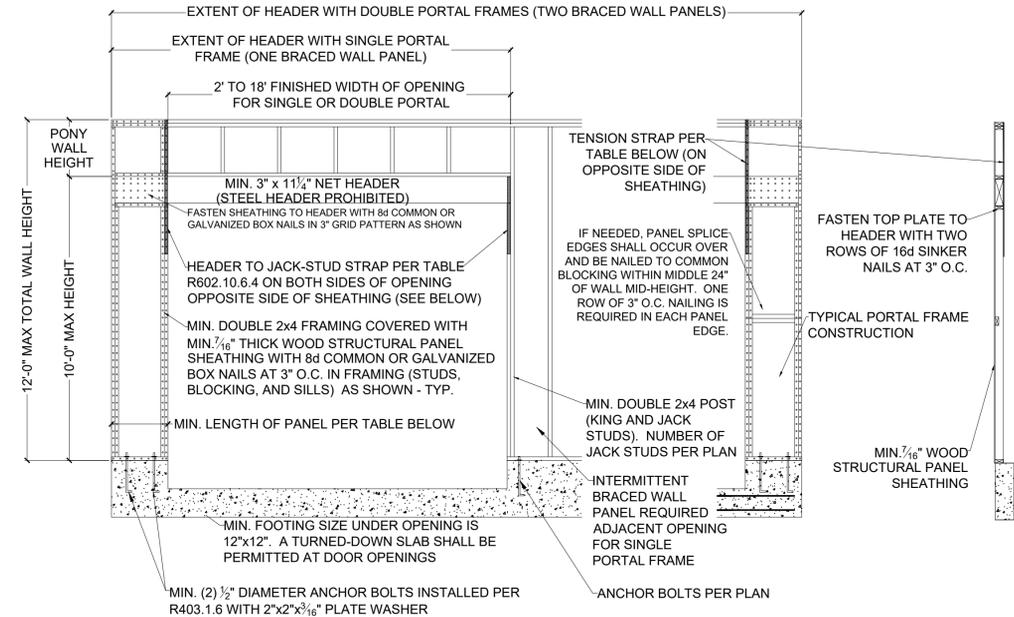


1 METHOD PFH (PORTAL FRAME WITH S3.0 HOLD-DOWNS) - PER FIGURE IRC R602.10.6.2

SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

| | MINIMUM PANEL LENGTH FOR DETAIL 1/S3.0 (INCHES) | | | | |
|-------------------------------|---|--------|---------|---------|---------|
| | WALL HEIGHT | | | | |
| | 8 FEET | 9 FEET | 10 FEET | 11 FEET | 12 FEET |
| SUPPORTING ROOF ONLY | 16 | 16 | 16 | 18 | 20 |
| SUPPORTING ONE STORY AND ROOF | 24 | 24 | 24 | 27 | 29 |

| TENSION STRAP REQUIRED FOR HEADER TO JACK STUD FOR DETAILS 1/S3.0 AND 2/S3.0 (FROM TABLE R602.10.6.4) | | | | | |
|---|--------------------------|------------------------|-----------------------|---|--|
| MAX GARAGE OPENING (FT.) | PONY WALL WALL HT. (FT.) | REQUIRED SIMPSON STRAP | MIN. STRAP END LENGTH | NAILS REQUIRED IN EACH STRAP END LENGTH | |
| 18'-0" | 0'-0" | CS20 | 0'-9" | (7) 8d | |
| 9'-0" | 1'-0" | CS20 | 0'-9" | (7) 8d | |
| 18'-0" | 1'-0" | CS14 | 1'-4" | (15) 8d | |
| 9'-0" | 2'-0" | CS18 | 0'-11" | (9) 8d | |
| 18'-0" | 2'-0" | CMSTC16 | 1'-8" | (25) 16d SINKER | |
| 9'-0" | 4'-0" | CMSTC16 | 1'-8" | (25) 16d SINKER | |
| 16'-0" | 4'-0" | CMST14 | 2'-6" | (33) 10d | |

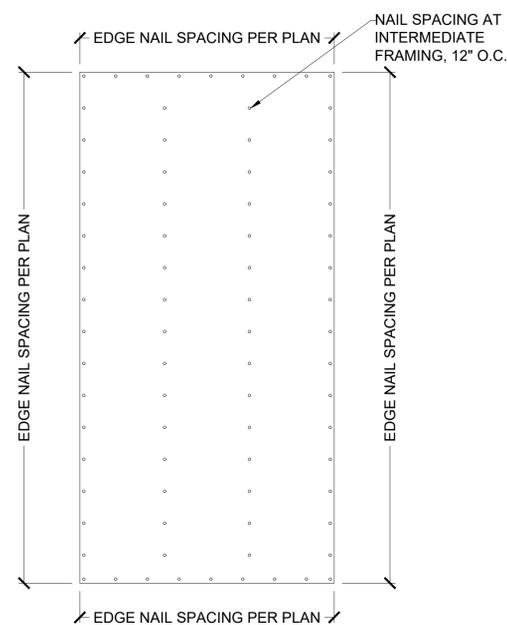


2 METHOD PFG (PORTAL FRAME AT GARAGE S3.0 DOOR) - PER FIGURE IRC R602.10.6.3

SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

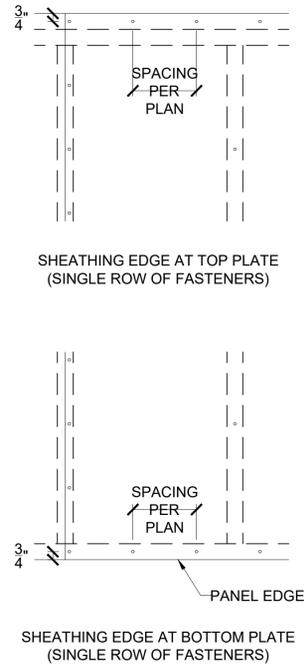
| MINIMUM PANEL LENGTH FOR DETAIL 2/S3.0 (INCHES) | | | | |
|---|--------|---------|-----------------|-----------------|
| WALL HEIGHT | | | | |
| 8 FEET | 9 FEET | 10 FEET | 11 FEET | 12 FEET |
| 24 | 27 | 30 | 33 ^a | 36 ^a |

a. Maximum opening height for PFG is 10 feet in accordance with Figure R602.10.6.3, but wall height may be increased to 12 feet with pony wall



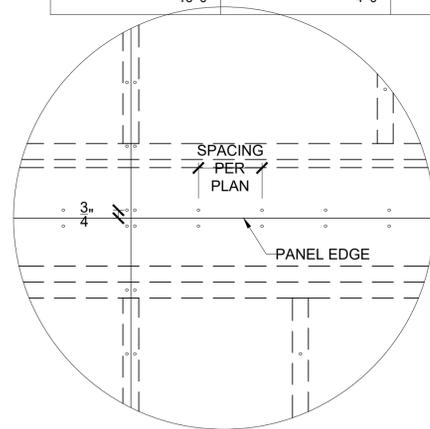
3 EXTERIOR WALL SHEATHING S3.0 PANEL ATTACHMENT

SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)



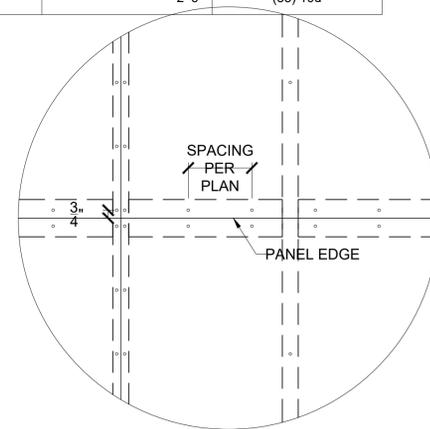
4 SHEATHING EDGE AT TOP AND BOTTOM PLATES S3.0

SCALE: 1" = 1'-0" (18x24) OR 1 1/2" = 1'-0" (24x36)



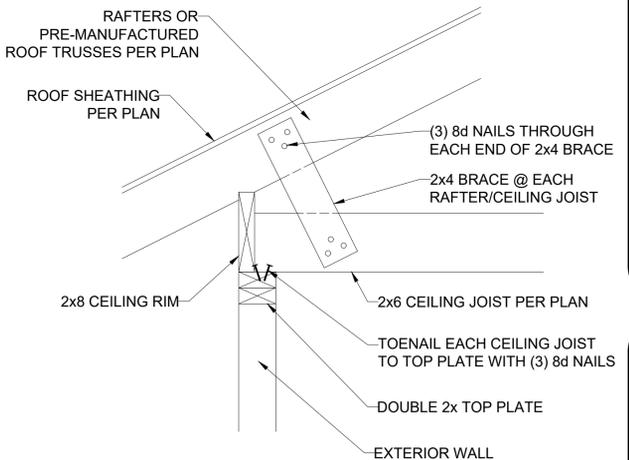
5 SHEATHING EDGE AT HORIZONTAL S3.0 FRAMING MEMBER

SCALE: 1" = 1'-0" (18x24) OR 1 1/2" = 1'-0" (24x36)



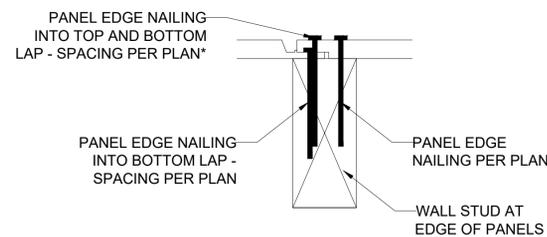
6 SHEATHING EDGE AT PANEL S3.0 SPLICE ACROSS STUDS

SCALE: 1" = 1'-0" (18x24) OR 1 1/2" = 1'-0" (24x36)



7 RAFTER BEARING OPTION DETAIL S3.0

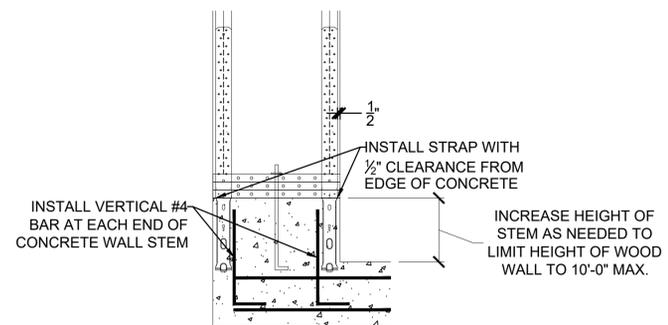
SCALE: 1" = 1'-0" (18x24) OR 1 1/2" = 1'-0" (24x36)



8 FASTENING INSTRUCTIONS FOR S3.0 SHIPLAP PANEL SHEATHING

SCALE: 4" = 1'-0" (18x24) OR 6" = 1'-0" (24x36)

*NOTE: NAILING INTO TOP AND BOTTOM LAP IS IN ADDITION TO NAILING REQUIRED INTO BOTTOM LAP. FOR EXAMPLE, IF PLAN CALLS FOR NAILS @ 6" O.C. AT EDGES, BOTTOM LAP SHALL BE FASTENED AT 6" O.C. AND, IN ADDITION, NAILING SHALL ALSO BE INSTALLED THROUGH TOP AND BOTTOM LAP @ 6" O.C. STAGGERED 3" FROM BOTTOM LAP NAILING



9 GARAGE HOLD-DOWN S3.0 STRAP INSTALLATION

SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

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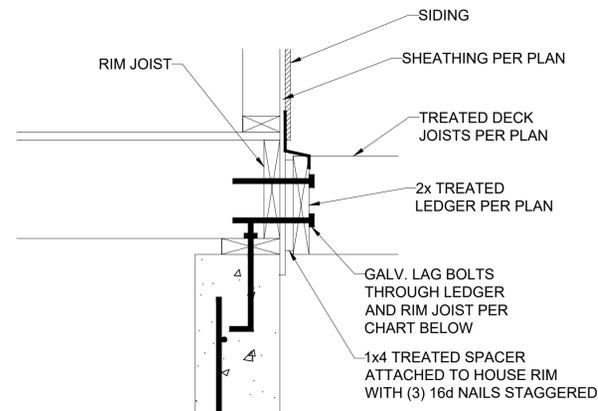
CLIENT: JEFF & BARBARA HENSON
 JOB TITLE: PGPT29 HENSON
 LOT 129, PERGOLA PARK - 5TH PLAT
 LOCATION: 1101 SW CORINTHIAN LN.
 LEE'S SUMMIT, MISSOURI

STATE OF MISSOURI
 DENNIS HEIER
 NUMBER: PE-2018001772
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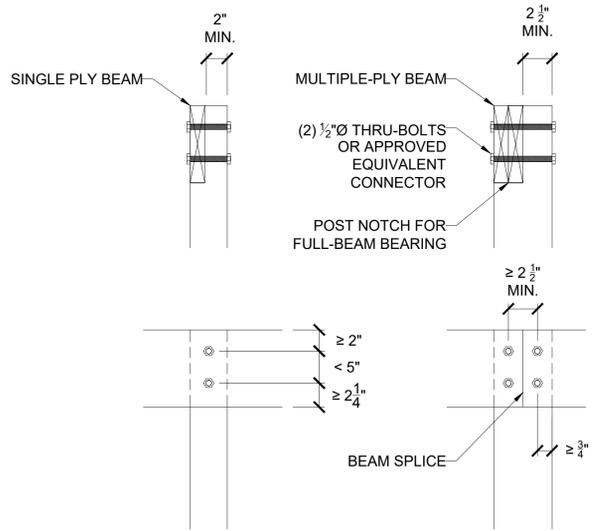
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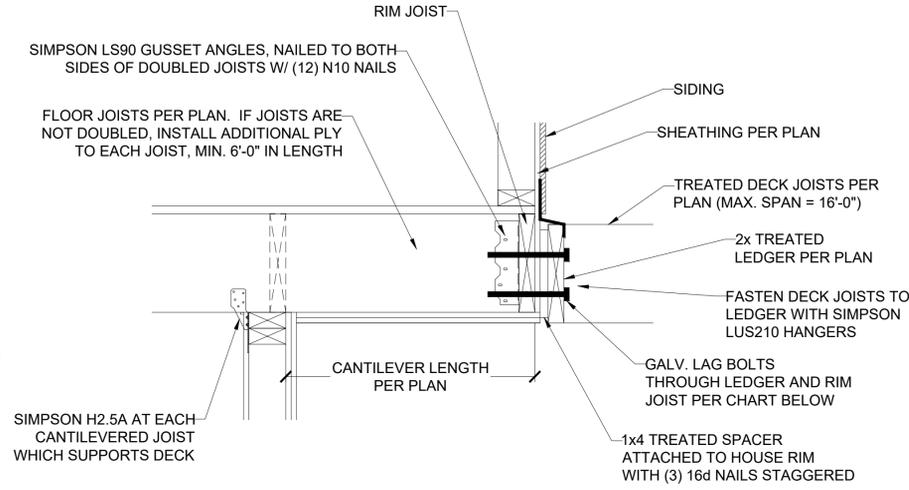
DECK LEDGER ATTACHMENT GUIDE

| DECK JOIST SPAN | 1/2" Ø GALV. LAG OR 3/8" Ø LEDGER-LOK SPACING |
|------------------|---|
| 10'-0" OR LESS | 16" OC |
| 10'-0" - 13'-11" | 12" OC OR @ 16" OC DOUBLED EVERY OTHER |
| 14'-0" - 18'-0" | 8" OC OR @ 16" OC DOUBLED |

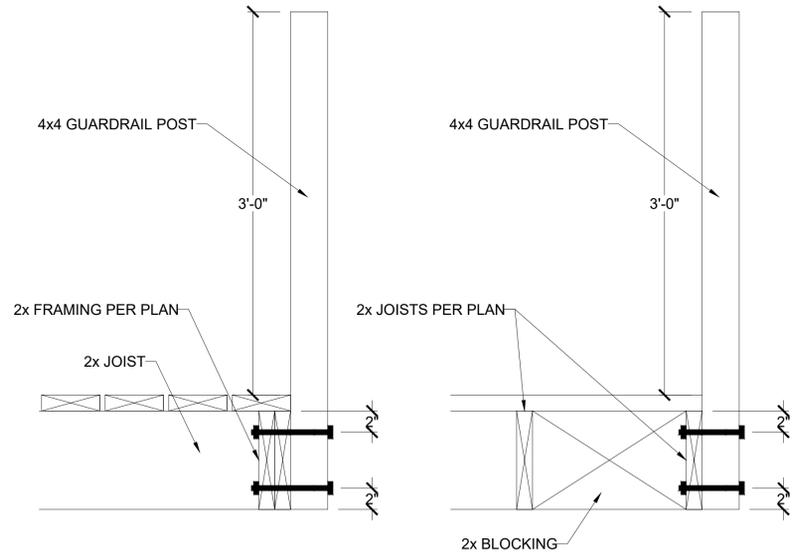
1 LEDGER ATTACHMENT
S3.3 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



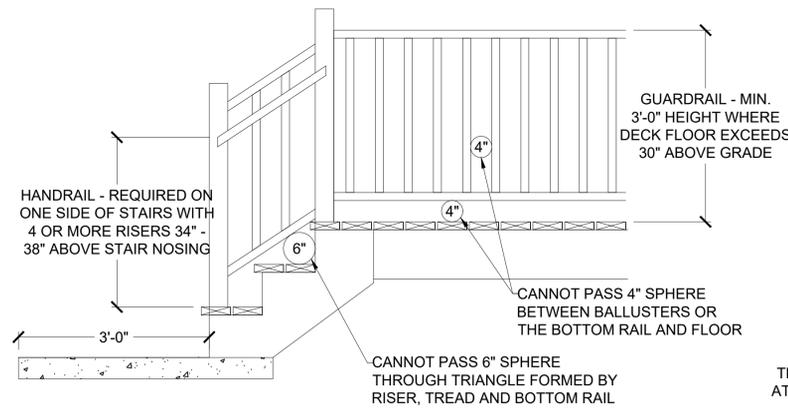
5 LET-IN (COVERED) DECK BEAM CONNECTION
S3.3 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



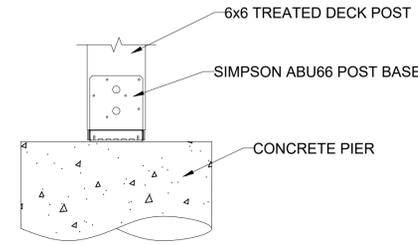
2 CANTILEVER WITH DECK ATTACHMENT
S3.3 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



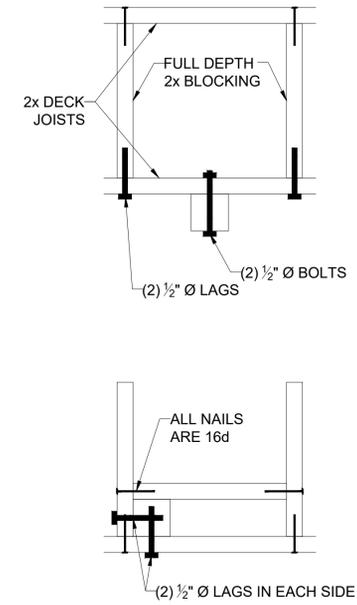
6 GUARDRAIL CONNECTION
S3.3 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



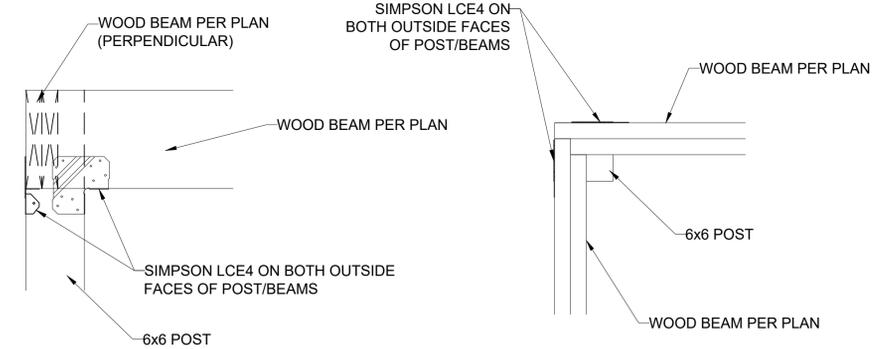
8 GUARDRAIL DETAIL
S3.3 SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)



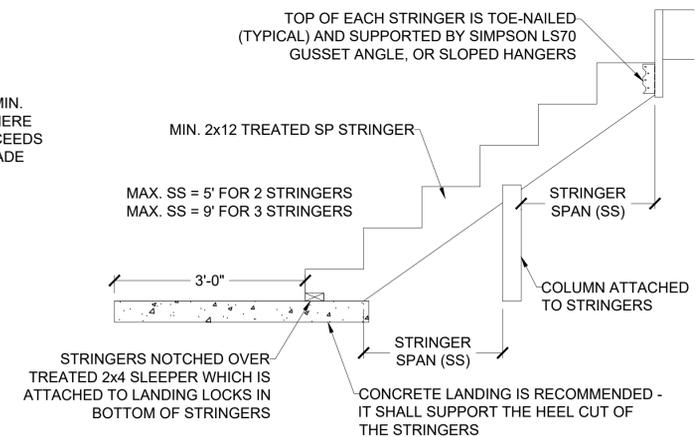
3 DECK POST BASE
S3.3 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



4 REINF. POST CONNECTIONS
S3.3 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



7 ALTERNATE COVERED DECK/PORCH INTERSECTION CORNER BEAM CONNECTION
S3.3 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



9 STAIR STRINGER DETAIL (MAX. 5' STAIR WIDTH)
S3.3 SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

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CLIENT: JEFF & BARBARA HENSON
JOB TITLE: PGP129 HENSON
LOT 129, PERGOLA PARK - 5TH PLAT
LOCATION: 1101 SW CORINTHIAN LN.
LEE'S SUMMIT, MISSOURI

STATE OF MISSOURI
DENNIS HEIER
NUMBER: PE-2018001772
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9-16-2024

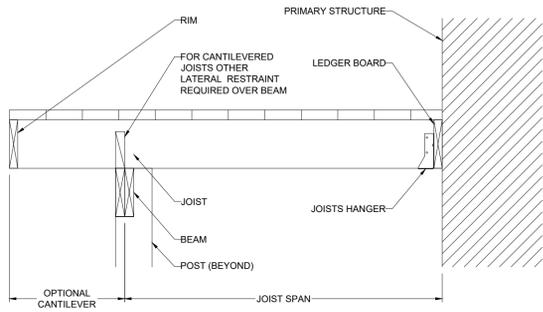
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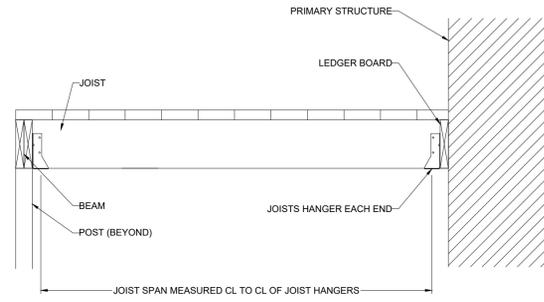
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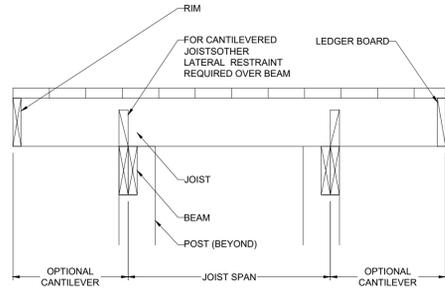
SEE PAGE FOR CONSTRUCTION NOTES ON DAYS REVIEW DEVELOPMENT SERVICES
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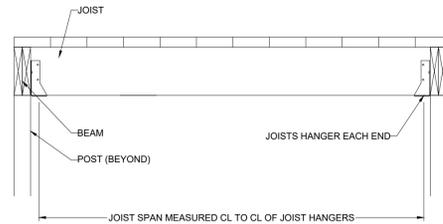
CANTILEVERED JOISTS WITH DROPPED BEAM



JOISTS WITH FLUSH BEAM

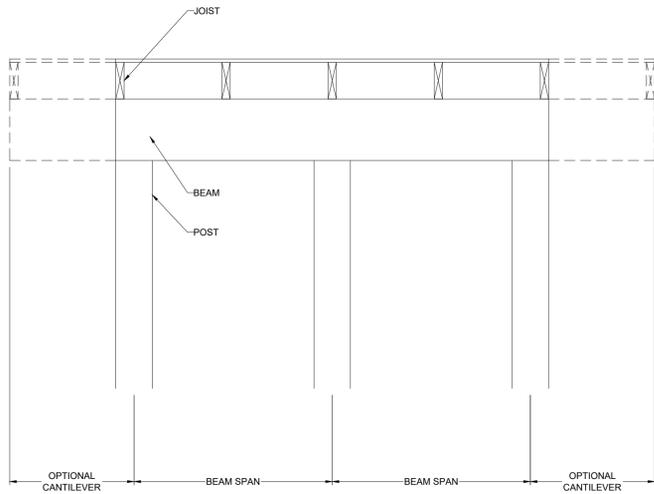


JOISTS ON FREE-STANDING DECK WITH DROPPED BEAM

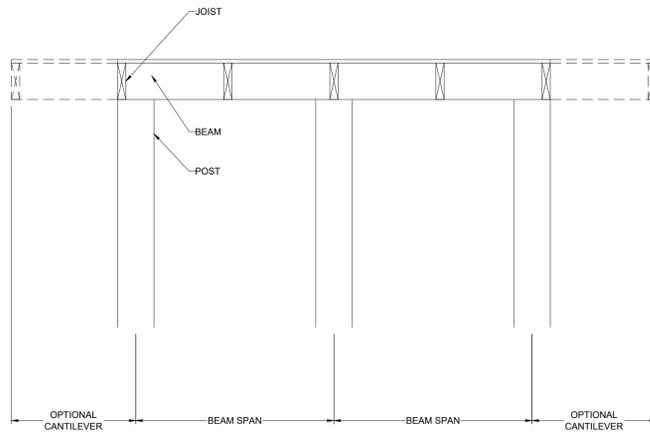


JOISTS WITH FLUSH BEAM

10 TYP. DECK JOIST SPANS
S3.3 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



DROPPED BEAM



FLUSH BEAM

11 TYP. DECK JOIST SPANS
S3.3 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)

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CLIENT: JEFF & BARBARA HENSON
JOB TITLE: PGP129 HENSON LOT 129, PERGOLA PARK - 5TH PLAT
LOCATION: 1101 SW CORINTHIAN LN, LEE'S SUMMIT, MISSOURI

STATE OF MISSOURI
DENNIS HEIER
NUMBER: PE-201801772
PROFESSIONAL ENGINEER
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