

GOVERNMENTAL AGENCIES

BUILDING DEPARTMENT

AGENCY: CITY OF LEE'S SUMMIT DEVELOPMENT SERVICES DEPARTMENT
ADDRESS: 220 SE GREEN, LEE'S SUMMIT, MO 64063
CONTACT: JOE FROGGE
PHONE #: 816.969.1200
EMAIL: DEVTECH@CITYOFLS.NET

FIRE MARSHALL

AGENCY: CITY OF LEE'S SUMMIT FIRE DEPARTMENT
ADDRESS: 207 SE DOUGLAS, LEE'S SUMMIT, MO 64063
CONTACT: CHIEF JIM EDEN
PHONE #: 816.969.1300
EMAIL: JIM.EDEN@CITYOFLS.NET

BUILDING DATA

BUILDING CODE: 2018 INTERNATIONAL BUILDING CODE W/ CITY OF LEE'S SUMMIT AMENDMENTS
PLUMBING CODE: 2018 INTERNATIONAL PLUMBING CODE
MECHANICAL CODE: 2018 INTERNATIONAL MECHANICAL CODE
FUEL GAS CODE: 2018 INTERNATIONAL FUEL GAS CODE
ELECTRIC CODE: 2017 NATIONAL ELECTRICAL CODE
ACCESSIBILITY CODE: 2009 ICC/ ANSI A117.1
ENERGY CODE: 2018 INTERNATIONAL ENERGY CONSERVATION CODE
FIRE CODE: 2018 INTERNATIONAL FIRE CODE

EXIST. BUILDING OCCUPANCY A2 (NO CHANGES)
EXIST. BLDG. CONSTRUCTION TYPE VB (SPRINKLERED, NO CHANGES)
EXIST. BUILDING AREA: 4,161 GSF (NO CHANGES)
EXISTING BLDG OCCUPANCY LOAD: 135 OCCUPANTS (CHANGES)

OCCUPANT LOAD CALCULATION:

	NEW
DINING AREA:	1,539 SF / 15 SF/PERSON = 103
ORDER AREA:	113 SF / 5 SF/PERSON = 23
KITCHEN / DT / SERVING / MULTI-PURPOSE	1,936 SF / 200 SF/PERSON = 10
BUSINESS AREA (OFFICE)	69 SF / 150 SF/PERSON = 1

NEW TOTAL OCCUPANCY LOAD 137 OCCUPANTS

PROPOSED NEW F2F CANOPY AREA: 1,464 SF

PROPOSED NEW OMD CANOPY AREA: 1,890 SF

SCOPE OF WORK

THESE DOCUMENTS REPRESENT AN ADDITION OF A NEW DUAL LANE OUTSIDE MEAL DELIVERY CANOPY ADDITION & DUAL LANE FACE TO FACE CANOPY AND PLAY AREA CONVERSION TO DINING.

ITEMS OF IMPORTANCE

1. REFER TO CIVIL PLANS FOR EXTENT OF SITE WORK.
2. EXISTING SITE ITEMS TO REMAIN UNLESS NOTED OTHERWISE.
3. F2F & OMD CANOPIES BY LANE
4. NO CHANGES TO BUILDING FOOTPRINT & OCCUPANCY TYPE.
5. EXISTING CANOPY AT DRIVE THRU SIDE TO BE REMOVED.

PROJECT GENERAL NOTES

1. ELECTRICAL WORK WILL BE PERFORMED UNDER THIS CONTRACT. ALL TO REMAIN UNLESS NOTED OTHERWISE.
2. ALL WORK SHALL BE IN COMPLIANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL BUILDING CODES, REGULATIONS, ORDINANCES, STANDARDS INCLUDING ADA, OTHER HANDICAP ACCESSIBILITY CODES AND INSURANCE RATING BOARDS. NO WORK SHALL COMMENCE UNTIL ALL JURISDICTIONAL PERMITS AND APPROVALS ARE OBTAINED.
3. GENERAL CONTRACTOR SHALL COORDINATE WITH THE OWNER'S VENDORS REGARDING SCHEDULING AND SEQUENCING OF THE WORK.
4. THE CONSTRUCTION NOTES AND DRAWINGS ARE SUPPLIED TO ILLUSTRATE THE DESIGN AND GENERAL TYPE OF CONSTRUCTION DESIRED AND ARE INTENDED TO IMPLY THE FINEST QUALITY OF CONSTRUCTION, MATERIALS AND WORKMANSHIP THROUGHOUT AND SHALL CONFORM TO THE APPROPRIATE NATIONAL TRADE PUBLICATION.
5. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO FULLY EXAMINE THE SITE SPACE PRIOR TO THE START OF CONSTRUCTION. THE G.C. SHALL VERIFY ALL DIMENSIONS, (VERTICAL, HORIZONTAL AND OTHERWISE), AS WELL AS TO VERIFY THE CONDITIONS AND NATURE OF THE PROPOSED CONSTRUCTION, MATERIALS, AVAILABLE UTILITIES AND STRUCTURAL ELEMENTS. THE G.C. SHALL NOTIFY THE OWNER'S REPRESENTATIVE (OWNER'S REP), IN WRITING OF ANY AND ALL DISCREPANCIES BETWEEN THE EXISTING CONDITIONS AND THE CONSTRUCTION DOCUMENTS.
6. IT SHALL BE THE JOINT RESPONSIBILITY OF THE G.C. AND ALL SUBCONTRACTORS AND SUPPLIERS OF MATERIALS TO SECURE ALL NECESSARY ADAPTATIONS AS MAY BE REQUIRED FOR THEIR RESPECTIVE WORK, PRIOR TO ORDERING, FABRICATION OR INSTALLATION OF ANY MATERIALS, EQUIPMENT OR COMPONENTS WHICH ARE TO BE INTEGRATED INTO THE WORK. NO CLAIMS FOR ADDITIONAL COMPENSATION SHALL BE MADE OR SHALL BE VALID UNLESS WRITTEN NOTIFICATION IS RECEIVED BY THE OWNER'S REP AND THE ADDITIONAL COMPENSATION IS APPROVED IN ADVANCE OF PROCEEDING WITH THE WORK.
7. REFERENCE ALL DRAWINGS FOR A COMPLETE DESCRIPTION OF THE WORK.
8. COMMENCEMENT OF WORK IN ANY AREA BY THE CONTRACTOR SHALL BE CONSTRUED THAT THE CONTRACTOR HAS CHECKED THE EXISTING CONDITIONS AND FOUND THEM TO BE SATISFACTORY TO ACCEPT THIS PORTION OF THE WORK.
9. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING UTILITIES WHETHER SHOWN IN THE DRAWINGS OR NOT & TO PROTECT THEM FROM DAMAGE DURING THE WORK. THE CONTRACTOR SHALL BEAR ALL EXPENSES OF REPAIR OR REPLACEMENT OF UTILITIES OR OTHER PROPERTY DAMAGED BY OPERATIONS IN CONJUNCTION WITH THE PERFORMANCE OF THE WORK.
10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COMPLETE SECURITY OF THE SITE WHILE JOB IS IN PROGRESS & UNTIL JOB IS COMPLETED.
11. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS & SHALL MAINTAIN THE STRUCTURAL INTEGRITY OF ALL WORK.
12. RESTAURANT REFUSE/DUMPSTER SHALL NOT BE USED FOR CONSTRUCTION DEBRIS.
13. CAP AND SEAL OFF ANY PLUMBING/ELECTRICAL PENETRATIONS AS NECESSARY. DO NOT ABANDON ANY UTILITIES OR MATERIALS WITHIN THE SPACE - REMOVE BACK TO THE SOURCE.
14. COVER RETURN AIR DUCTS AS NECESSARY BEFORE AND DURING CONSTRUCTION.

ARCHITECT:

INTERPLAN LLC,
220 E CENTRAL PKWY, SUITE 4000
ALTAMONTE SPRINGS, FL 32701
AOR: LAUREL MARTIN, R.A., NCARB
CONTACT: JESSICA CHERKASSKY
PHONE (407) 645-5008
FAX (407) 629-9124
EMAIL: JCHERKASSKY@INTERPLANLLC.COM

ELECTRICAL ENGINEER:

INTERPLAN LLC
220 E CENTRAL PKWY,
SUITE 4000
ALTAMONTE SPRINGS,
FL 32701
PHONE: (407) 645-5008
ENGINEER OF RECORD:
STACY HENSON
CONTACT: MARYANA IBRAHIM

PLUMBING ENGINEER:

INTERPLAN LLC
220 E CENTRAL PKWY,
SUITE 4000
ALTAMONTE SPRINGS,
FL 32701
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MECHANICAL ENGINEER:

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SUITE 4000
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FL 32701
PHONE: (407) 645-5008
ENGINEER OF RECORD:
STACY HENSON
CONTACT: MARYANA IBRAHIM

CIVIL ENGINEER:

GBC DESIGN, INC
565 WHITE POND DRIVE
AKRON, OH 44320
CONTACT: JACK MEANEY, P.E.
PHONE: (330) 836-0228

CANOPY SUPPLIER

LANE SUPPLY, INC.
120 FAIRVIEW
ARLINGTON, TX 76010
CONTACT: LARRY TOLBERT
PHONE: (817) 261-9116



5200 BUFFINGTON ROAD
ATLANTA, GEORGIA 30349-2998
PHONE: (404) 765-8000
FAX: (404) 684-8550

S08N-104-R
CUSTOM PROJECT SOLUTIONS
DUAL LANE OUTSIDE MEAL DELIVERY
CANOPY & DUAL LANE FACE TO
FACE CANOPY ADDITION.
PLAY AREA CONVERSION TO DINING

SUMMIT FAIR FSR #02859
690 NW BLUE PARKWAY,
LEE'S SUMMIT, MO 64086
AUGUST 2023

REVISION SCHEDULE				
REVISION NUMBER	REVISION DATE	ISSUE DESCRIPTION	CHANGE DESCRIPTION	AFFECTED SHEETS
1	02/19/24	PLAY AREA REMOVAL	-	G-000, ASP-1.1, D-201, D-221, A-005, A-201, A-211, A-221, A-601, A-620, M-1.1, M-2.1, E-2.1, F-201, F-211, F-701
2	03/11/24	COORDINATION REVISIONS	-	F-201
3	04/03/24	DESIGN COMMENTS	-	A-221, A-601
4	04/17/24	RFI #5	-	G-000, FP-1.1
5	07/15/24	COORDINATION REVISIONS	ADDED OFFSET FOOTING DETAILS	G-000, OMD-4 - OMD-10

ELECTRICAL ENGINEER:

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ALTAMONTE SPRINGS,
FL 32701
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DRAWING INDEX

ARCHITECTURAL

G-000 COVER SHEET
ASP-1.0 ARCHITECTURAL SITE PLAN- F2F CANOPY
ASP-1.1 ARCHITECTURAL SITE PLAN- OMD CANOPY
D-201 DEMOLITION FLOOR PLAN
D-221 DEMOLITION REFLECTED CEILING PLAN
A-005 FINISH SCHEDULE
A-201 PROPOSED FLOOR PLAN
A-211 FINISH FLOOR PLAN
A-221 PROPOSED REFLECTED CEILING PLAN
A-601 INTERIOR ELEVATIONS
A-620 INTERIOR DETAILS

MECHANICAL

M-1.1 MECHANICAL FLOOR PLAN
M-2.1 MECHANICAL SPECIFICATIONS & SCHEDULES

PLUMBING

P-1.1 OMD GAS PLUMBING PLAN
P-1.2 F2F GAS PLUMBING PLAN
P-2.1 PLUMBING DETAILS

ELECTRICAL

E-1.1 CANOPY POWER & LIGHTING PLAN
E-1.2 CANOPY ELECTRICAL DETAILS
E-2.1 PHOTOMETRIC PLAN
E-2.2 ELECTRICAL LIGHTING PLAN

FURNITURE

F-201 FURNITURE FLOOR PLAN
F-211 FURNITURE CORE DRILL PLAN
F-701 DECOR ELEVATIONS

F2F CANOPY

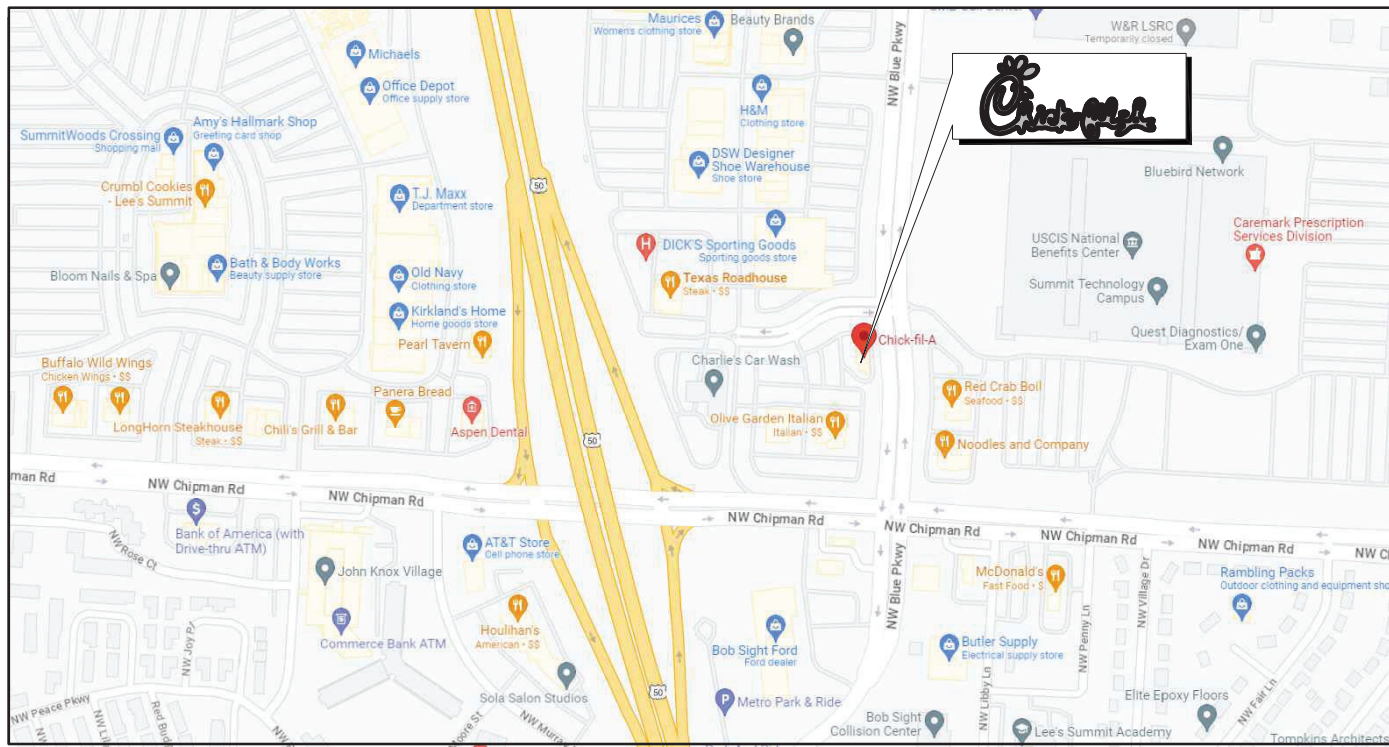
F2FC-1 CANOPY FOOTING LOCATIONS
F2FC-2 CANOPY FOOTINGS
F2FC-3 CANOPY FRAMING PLAN
F2FC-4 CANOPY SECTIONS
F2FC-5 CANOPY SECTIONS
F2FC-6 CANOPY SECTIONS
F2FC-7 CANOPY ELEVATION PLAN
F2FC-8 CANOPY LIGHT LAYOUT

OMD CANOPY

OMD-1 CANOPY FOOTING LOCATIONS
OMD-2 CANOPY FOOTINGS
OMD-3 CANOPY FOOTINGS
OMD-4 CANOPY FOOTINGS
OMD-5 CANOPY FRAMING PLAN
OMD-6 CANOPY SECTIONS
OMD-7 CANOPY SECTIONS
OMD-8 CANOPY SECTIONS
OMD-9 CANOPY ELEVATION PLAN
OMD-10 CANOPY LIGHT LAYOUT

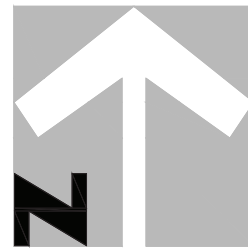
FIRE PROTECTION

FP-1.1 FIRE SPRINKLER PLAN

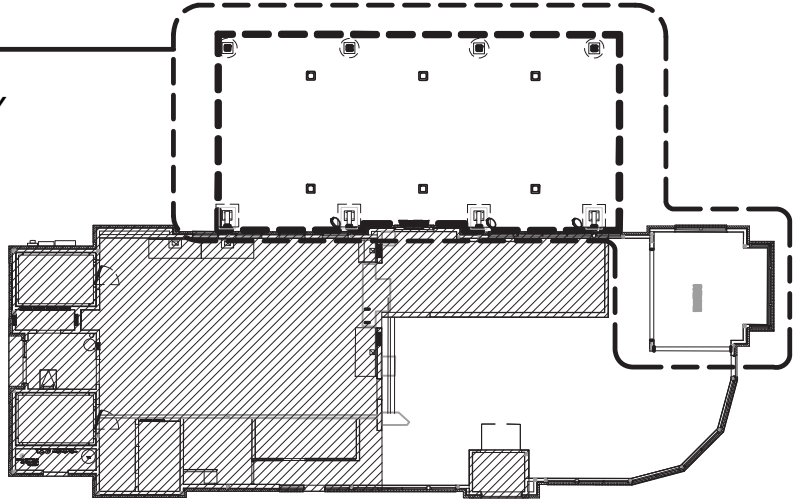


VICINITY MAP

NOT TO SCALE



SCOPE OF WORK.
DL-OMD CANOPY
AND DL-F2F CANOPY
ADDITION. PLAY
AREA CONVERSION
TO DINING



REFER TO SITE PLAN FOR
EXACT F2F CANOPY
LOCATION

KEY PLAN



Chick-fil-A
Chick-fil-A

5200 Buffington Road
Atlanta, Georgia
30349-2998

INTERPLAN

INTERPLAN LLC
ARCH COA #2015008774
ENG COA #2003026904

ARCHITECTURE
ENGINEERING
PERMITTING

220 E. CENTRAL PKWY, STE 4000
ALTAMONTE SPRINGS, FL 32701
407.645.5008

SEAL:



LAUREL ROSE MARTIN - ARCHITECT
LIC. # A-2019008772

CHICK-FIL-A

SUMMIT FAIR

690 NW BLUE PKWY
LEE'S SUMMIT, MO 64086

FSR#02859

BUILDING TYPE / SIZE: S08N-104-R, V8
RELEASE:
PRINTED FOR
PERMIT

REVISION SCHEDULE		
NO.	DATE	DESCRIPTION
1	02/19/24	PLAY AREA REMOVAL
4	04/17/24	RFI#5
5	07/15/24	COORDINATION REVS

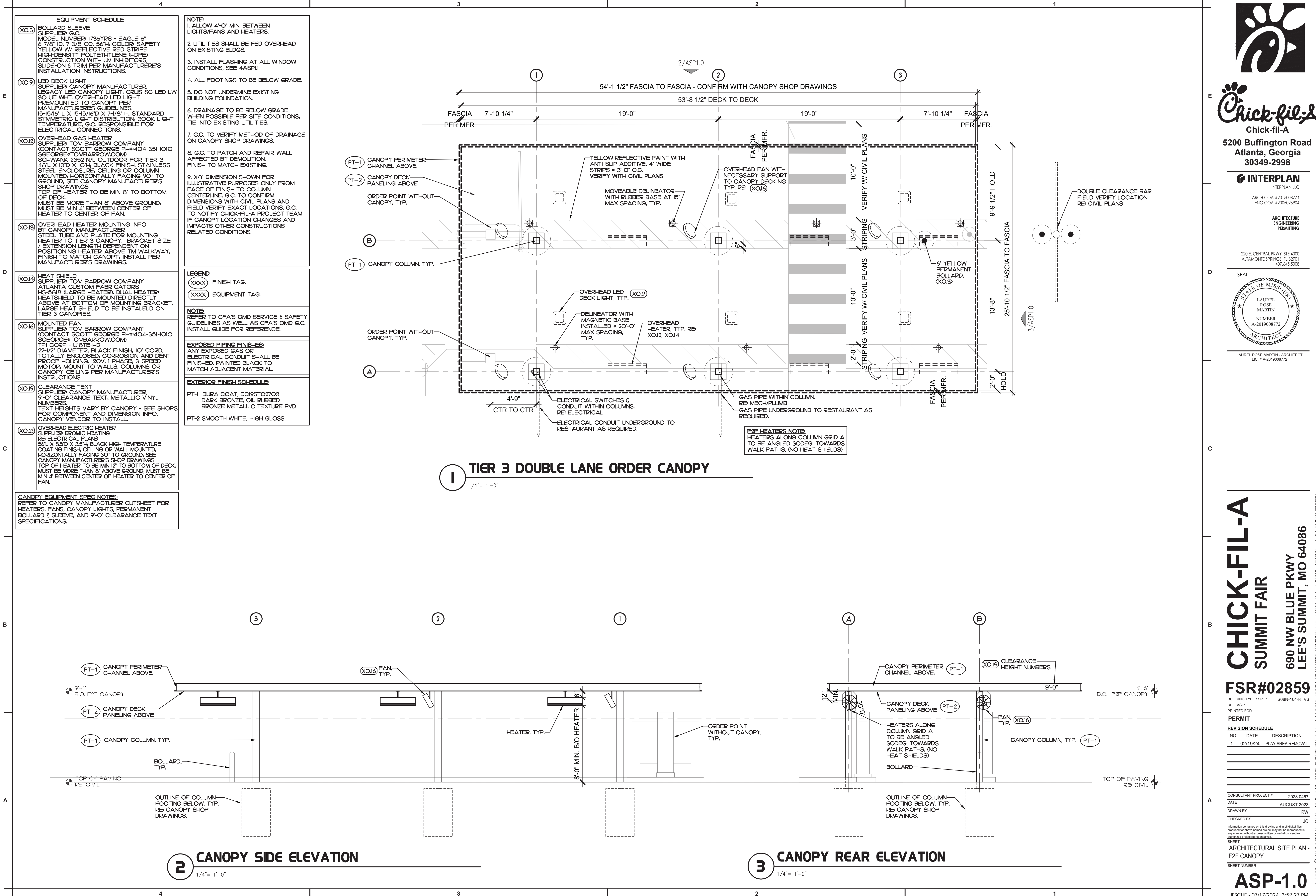
CONSULTANT PROJECT # 2023.0467
DATE AUGUST 2023
DRAWN BY SN
CHECKED BY JC

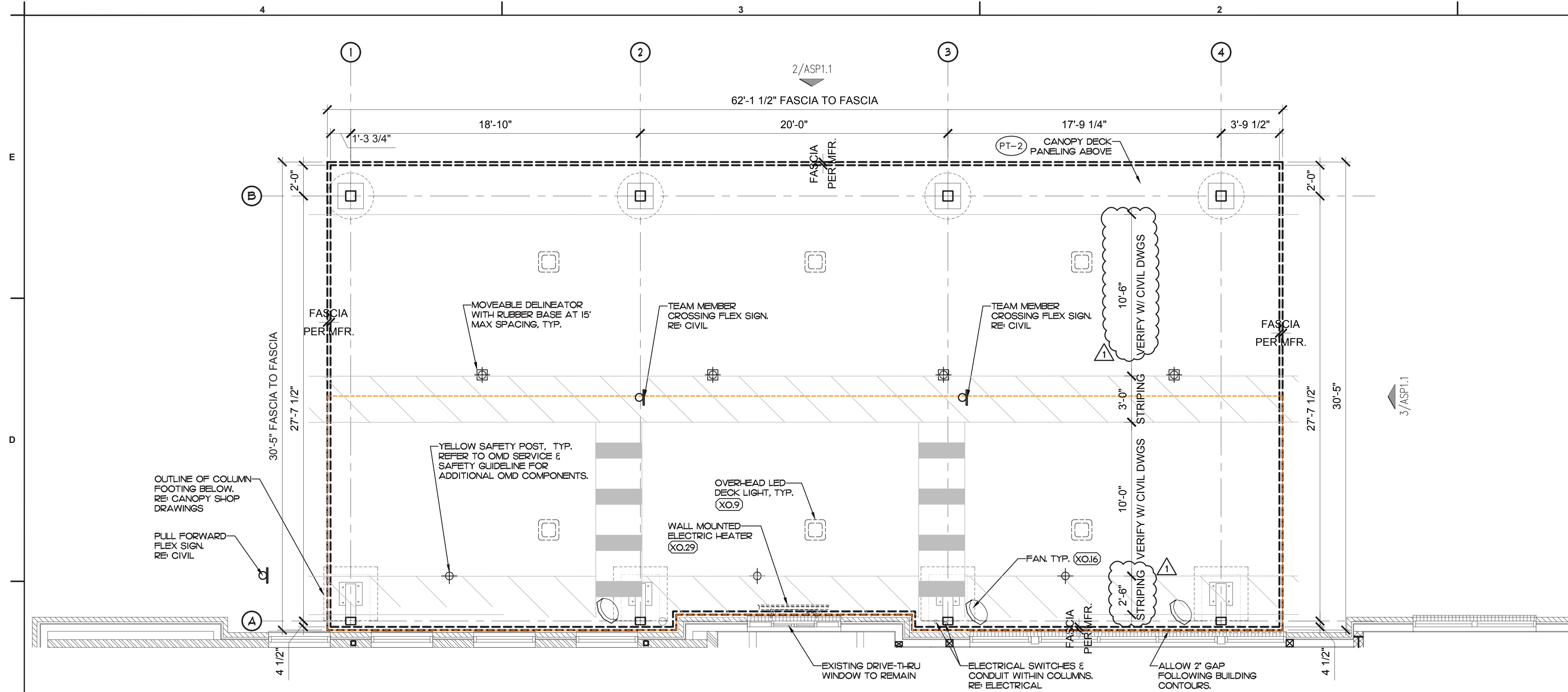
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COVER SHEET

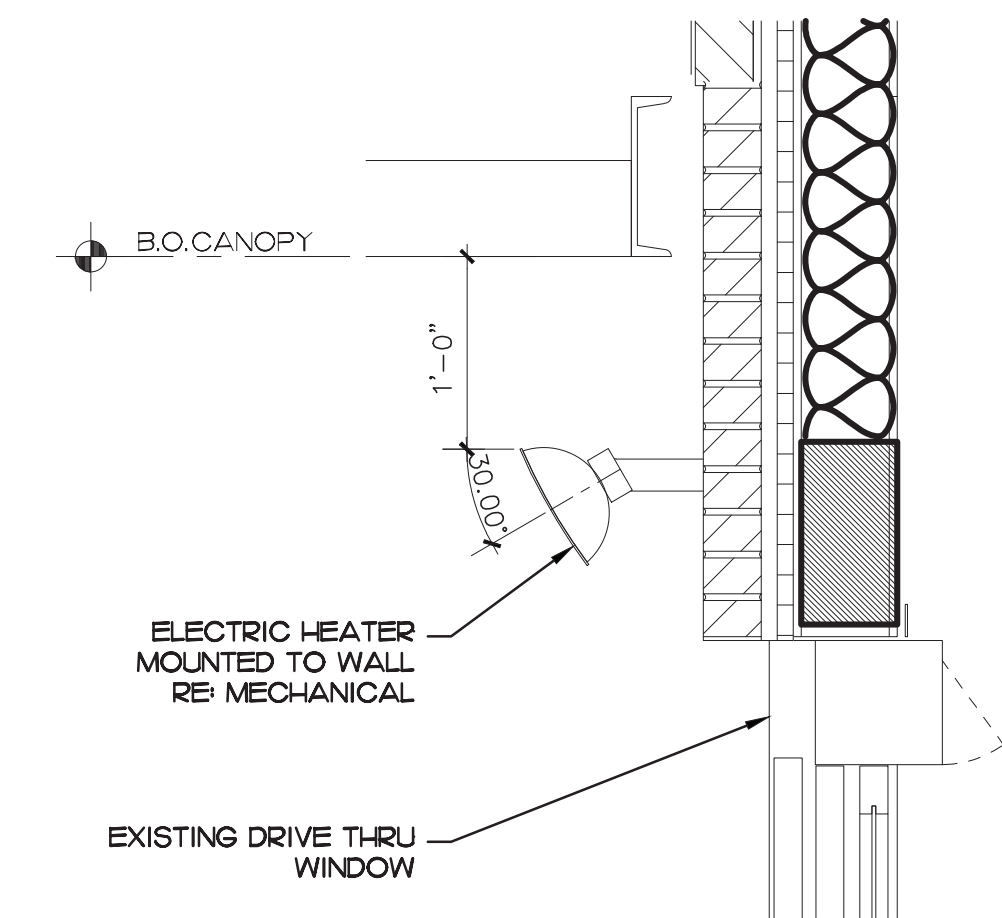
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G-000

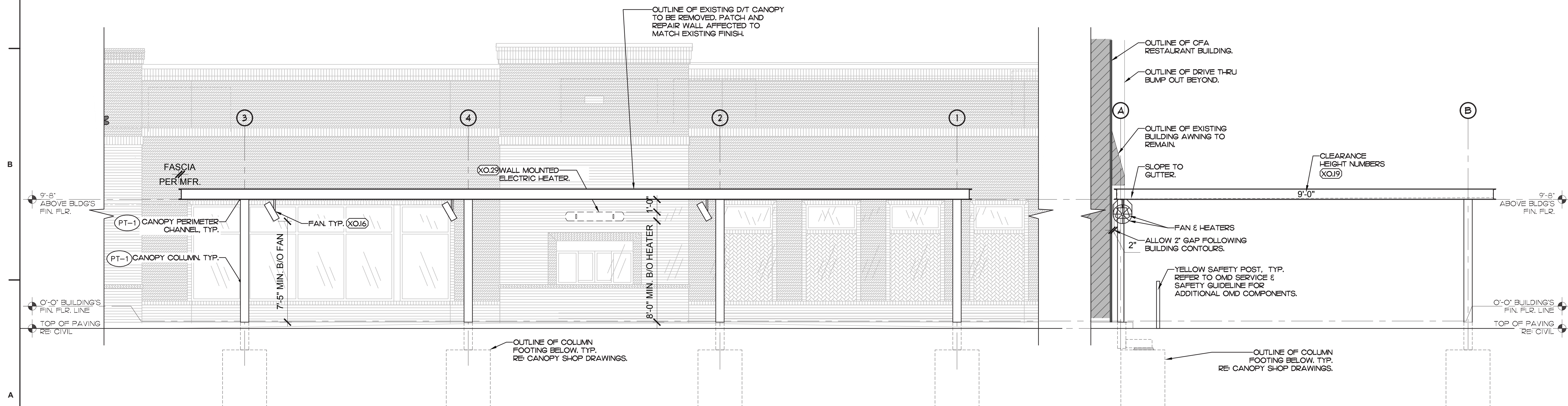




4 FLASHING DETAIL
NTS



5 HEATER DETAIL
NTS



Chick-fil-A
Chick-fil-A

5200 Buffington Road
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ENG COA #2003026904

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LEE'S SUMMIT, MO 64086

FSR#02859

BUILDING TYPE / SIZE: S08N-104-R, V6
RELEASE:
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REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	02/19/24	PLAY AREA REMOVAL

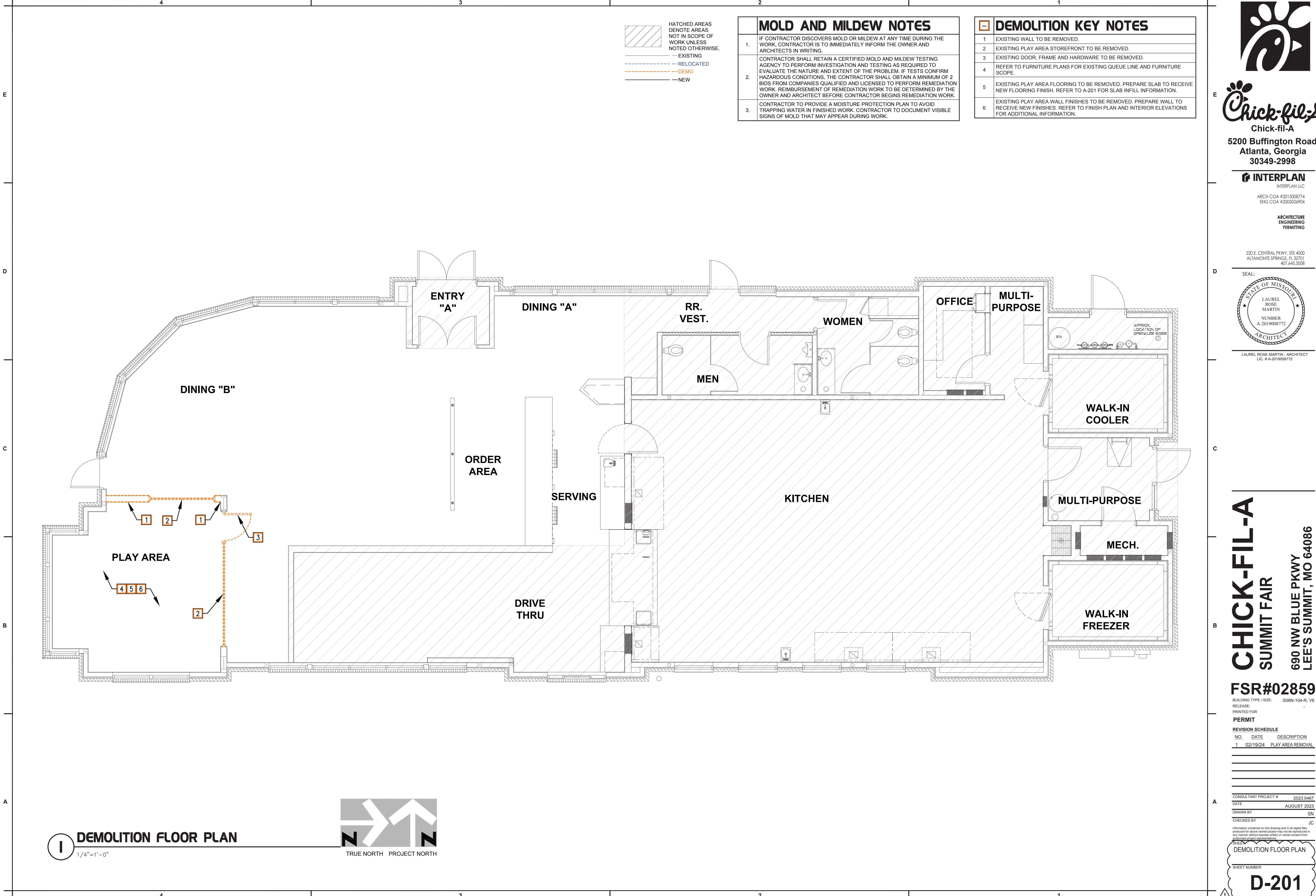
CONSULTANT PROJECT # 2023.0467
DATE AUGUST 2023
DRAWN BY RW
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ARCHITECTURAL SITE PLAN -
OMD CANOPY

SHEET NUMBER

ASP-1.1

JESCHE - 07/17/2024 3:51:49 PM



Chick-fil-A
Chick-fil-A
5200 Buffington Road
Atlanta, Georgia
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SEAL:



LAUREL ROSE MARTIN - ARCHITECT
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690 NW BLUE PKWY
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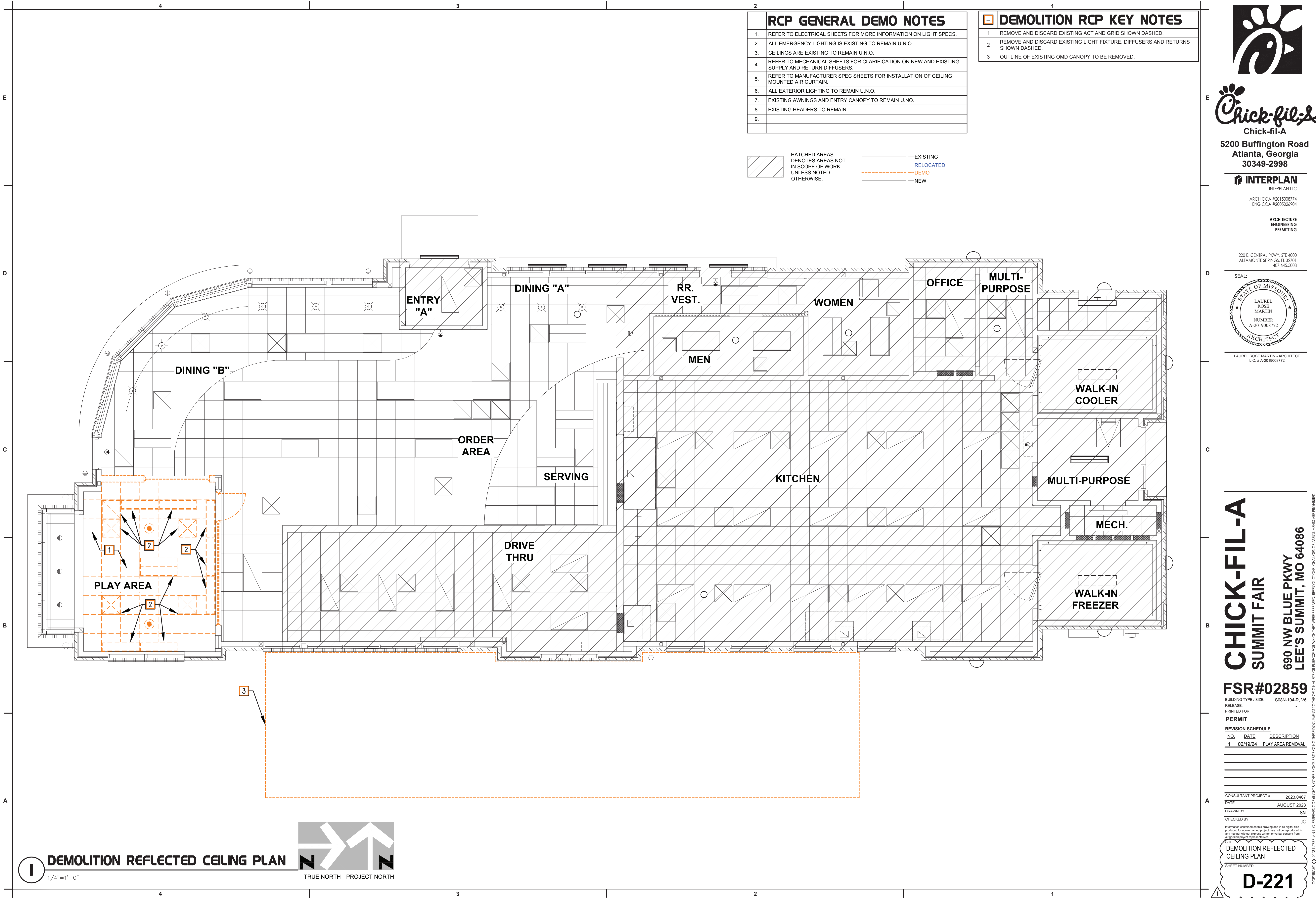
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SHEET
DEMOLITION FLOOR PLAN


SHEET NUMBER
D-201

JESCHE - 07/17/2024 3:52:34 PM




RCP GENERAL DEMO NOTES	
1.	REFER TO ELECTRICAL SHEETS FOR MORE INFORMATION ON LIGHT SPECS.
2.	ALL EMERGENCY LIGHTING IS EXISTING TO REMAIN U.N.O.
3.	CEILINGS ARE EXISTING TO REMAIN U.N.O.
4.	REFER TO MECHANICAL SHEETS FOR CLARIFICATION ON NEW AND EXISTING SUPPLY AND RETURN DIFFUSERS.
5.	REFER TO MANUFACTURER SPEC SHEETS FOR INSTALLATION OF CEILING MOUNTED AIR CURTAIN.
6.	ALL EXTERIOR LIGHTING TO REMAIN U.N.O.
7.	EXISTING AWNINGS AND ENTRY CANOPY TO REMAIN U.N.O.
8.	EXISTING HEADERS TO REMAIN.
9.	

DEMOLITION RCP KEY NOTES	
1	REMOVE AND DISCARD EXISTING ACT AND GRID SHOWN DASHED.
2	REMOVE AND DISCARD EXISTING LIGHT FIXTURE, DIFFUSERS AND RETURNS SHOWN DASHED.
3	OUTLINE OF EXISTING OMD CANOPY TO BE REMOVED.



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SEAL:



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SUMMIT FAIR

690 NW BLUE PKWY
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NO.	DATE	DESCRIPTION
1	02/19/24	PLAY AREA REMOVAL

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DEMOLITION REFLECTED
CEILING PLAN

SHEET NUMBER
D-221

JESCHE - 07/17/2024 3:52:50 PM

4

3

2

1

I

INTERIOR FINISH SCHEDULE

CEILING FINISHES

ACT-2

ACOUSTICAL CEILING TILE
CERTAINTED CEILINGS, PERFORMA SYMPHONY M, #1222BF-IOF-1, 24" X 24" X 3/4" WHITE GRID, ELITE NARROW REVEAL, (CORNER BEVEL) 9/16" GRID, WHITE
[VERIFY EXISTING ACT SPECIFICATION, SEE NOTE BELOW]

PT-1A

PRIMARY INTERIOR CEILING PAINT
SHERWIN WILLIAMS, PRO MAR 200 ZERO VOC PAINT
SW-7011 / NATURAL CHOICE, FINISH: FLAT (SEMI-GLOSS ON HARDIE PANEL)

PT-3A

PRIMARY INTERIOR CEILING PAINT
SHERWIN WILLIAMS, PRO MAR 200 ZERO VOC PAINT
SW7600/ BOLERO, FINISH: FLAT

WALL FINISHES

PT-1B

PRIMARY INTERIOR WALL PAINT
SHERWIN WILLIAMS, PRO MAR 200 ZERO VOC PAINT
SW-7011 / NATURAL CHOICE, FINISH: EGG SHELL

WD-3

WAINSCOTING
PER NATIONAL ACCOUNT 1/4" PLYWOOD
(RED OAK, PLAIN CUT LAUAN CORE WITH METAL TRIM)
CUSTOM, FINISH: RED OAK STAINED MINWAX DRIFTWOOD.
RE: DETAILS AND SHOP DRAWINGS FROM MANUFACTURER.

WALL BASES

T-4

PORCELAIN TILE COVE BASE
CREATIVE MATERIALS CORPORATION
LAVA LIGHT GRAY, 6"X12" COVE BASE
GROUT: G1, JOINT WIDTH: 3/16"

FLOOR FINISHES

G-1

FLOOR TILE GROUT (T-3 AND T-4, USE WITH PORCELAIN TILE)
MAPEI, ULTRA COLOR GROUT
47 / CHARCOAL

T-3

PORCELAIN TILE
CREATIVE MATERIALS CORPORATION
LAVA LIGHT GRAY, 12"X12"
GROUT: G1, JOINT WIDTH: 3/16"

MISCELLANEOUS FINISHES

RS-1

ROLLER SHADE
PHIFER SHEERWEAVE 4000 U61 ECO/GREYSTONE, 5% OPENNESS - NOTCHLESS 4" FASCIA, DARK BRONZE TO MATCH STOREFRONT COLOR, ROLLEASE SKYLINE CLUTCH, (HEAT SEALED INTERNAL POCK HEM BAR), 1110LB STAINLESS ROLLEASE CHAIN, CHAIN LOCATED AT JAMB ONLY, NOT IN CENTER WINDOW RE: INTERIOR ELEVATIONS

INTERIOR FINISH NOTES

1. PROVIDE 5" HIGH BAND OF 1/2" CEMENTITIOUS BOARD AT BASE OF ALL WALLS IN DINING ROOMS AND VESTIBULES. 12" HIGH AT BASE OF KITCHEN WALLS. FOR ALL WALLS WITH TILE - PROVIDE FULL COVERAGE OF CEMENT BOARD SUBSTRATE FROM FLOOR TO THE HEIGHT OF WALL TILE. CEMENTITIOUS BOARD SHALL BE "DUROCK" BY U.S. GYPSUM. RE: INTERIOR ELEVATIONS.

2. ROLLER SHADE WIDTH DIMENSIONS TO BE 2" LESS THAN THE FINISHED DIMENSION OF EACH WINDOW TO ACCOUNT FOR WAINSCOT TOP CAP RETURNING TO STOREFRONT - VALENCE WILL MAINTAIN FULL LENGTH & ROLLER SHADE WILL BE 2" LESS THAN TOTAL MEASUREMENT.

3. AT T-2/CT-45 OUTSIDE CORNERS IN PUBLIC AREAS. USE BLANKE (RE: TR-1) TRIM STRIP. SEE NATIONAL ACCOUNTS FOR ORDERING INFORMATION.

4. FOR WAINSCOTING WHERE GRAPHIC MESSAGE EXTENDS ABOVE TOP HORIZONTAL TRIM CAP, INSTALLER TO FIELD CUT VERTICAL BATTENS AND TOP CAP AT GRAPHICS AS REQUIRED. COORDINATE WITH CHARTER HOUSE.

5. FOR CEILING FINISHES, IF PATCHING AND REPAIRING ACT, VERIFY EXISTING SPECIFICATION PRIOR TO ORDERING.

6. FOR EXISTING RESTAURANTS THAT NEED 6 INCH TILE FOR REPAIR, THE INSTALLER SHOULD SPECIFY NEEDING A COVE BASE (DAL TILE LAVA LIGHT GREY SPEC) WITH A 6 INCH HEIGHT AND ORDER DIRECTLY FROM DALTILE (CHICK-FIL-A@DALTILE.COM).

EXTERIOR FINISH NOTES

1. FINISHES LISTED IN THIS SCHEDULE DO NOT REPRESENT ORIGINAL PROTOTYPE FINISHES. CONFIRM ACTUAL FINISHES TO MATCH ON SITE.

2. REFER TO EXTERIOR ELEVATIONS FOR AWNING TYPES AND CORRESPONDING FINISH.

3. G.C. TO OBTAIN PAINT, COLOR, BRICK, MATERIAL SAMPLES AND TAKE PHOTOS OF SAMPLES NEXT TO EXISTING BUILDING IN FIELD, SEND PICTURES OF SAMPLES TO ARCHITECT, CFA CONSTRUCTION MANAGER, AND CHICK-FIL-A DESIGN TEAM FOR APPROVAL BEFORE START OF CONSTRUCTION AS TO NOT DELAY THE PROJECT.

PAINT NOTES:
VERIFY MANUFACTURER, COLOR, AND MODEL NUMBERS WITH EXTERIOR FINISH SCHEDULE

1. BLACK

1.1. SITE SIGNAGE (MAIN ID, SECONDARY ID, DIRECTIONAL, VERIFY OTHER POSSIBLE SIGNAGE)

1.2. NEW CANOPIES AT MAIN ENTRANCE AND DRIVE-THRU

1.3. AWNING FRAMES (VERIFY FABRIC WITH EXTERIOR FINISH SCHEDULE)\

1.4. DRIVE-THRU ORDER POINTS/MENU BOARDS

1.5. DRIVE-THRU CLEARANCE BARS

2. DARK BRONZE

2.1. SITE METALS, NOT MENTIONED ABOVE, INCLUDING BUT NOT LIMITED TO: BOLLARDS, DUMPSTER GATE POSTS, LIGHT POLES, HANDRAILS

2.2. BUILDING METALS, NOT MENTIONED ABOVE, INCLUDING BUT NOT LIMITED TO: LIGHT FIXTURES, DOWNSPOUTS, SCUPPERS

2.3. NEW BUILDING PARAPET COPINGS (IF APPLICABLE)

2.4. EXISTING BUILDING PARAPET COPINGS (REPAINT TO MATCH NEW) - IF APPLICABLE

4

3

2

1

1

FINISH SCHEDULE

A-005

CHICK-FIL-A

SUMMIT FAIR

690 NW BLUE PKWY

LEE'S SUMMIT, MO 64086

FSR#02859

BUILDING TYPE / SIZE: S08N-104-R, V6

RELEASE: PRINTED FOR PERMIT

REVISION SCHEDULE

NO.

DATE

DESCRIPTION

1

02/19/24

PLAY AREA REMOVAL

CONSULTANT PROJECT #

2023.0467

DATE

AUGUST 2023

DRAWN BY

SN

CHECKED BY

JC

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SHEET

FINISH SCHEDULE

SHEET NUMBER

A-005

CHICK-FIL-A

220 E. CENTRAL PKWY, STE 4000

ALTAMONTE SPRINGS, FL 32701

407.645.5008

ARCHITECTURE

ENGINEERING

PERMITTING

220 E. CENTRAL PKWY, STE 4000

ALTAMONTE SPRINGS, FL 32701

407.645.5008

SEAL:

STATE OF MISSOURI

LAUREL ROSE MARTIN

NUMBER A-2019008772

ARCHITECT

LAUREL ROSE MARTIN - ARCHITECT

LIC. # A-2019008772

1

JESCHE - 07/17/2024 3:52:11 PM

LEGEND

DOOR TAG.
RE: DOOR SCHEDULE

WINDOW TAG.
RE: DOOR SCHEDULE

EXISTING

RELOCATED

DEMO

NEW

HATCHED AREAS
DENOTE AREAS
NOT IN SCOPE OF
WORK UNLESS
NOTED OTHERWISE.

WOOD STUD WALL

METAL STUD WALL

NW - NEW WALL TO MATCH
EXISTING

BATT INSULATION

NEW EXTERIOR FINISH
RE: EXTERIOR ELEVATIONS

2x4 WOOD STUDS

2x6 WOOD STUDS

2x8 WOOD STUDS

2x INFILL WOOD STUDS

3 5/8" METAL STUDS

6" METAL STUDS

8" METAL STUDS

NOTE 1: WALL BLOCKING SHALL BE THE GENERAL CONTRACTOR'S RESPONSIBILITY. BLOCKING SHALL INCLUDE, BUT IS NOT LIMITED TO: AREAS INDICATED ON INTERIOR ELEVATIONS FOR GRAB BARS, SHELVING BRACKETS, MONITORS, FIXTURES, ETC.; AS WELL AS BLOCKING FOR WINDOWS, CANOPIES ROOF FRAMING, ROOF TOP UNITS, ETC.

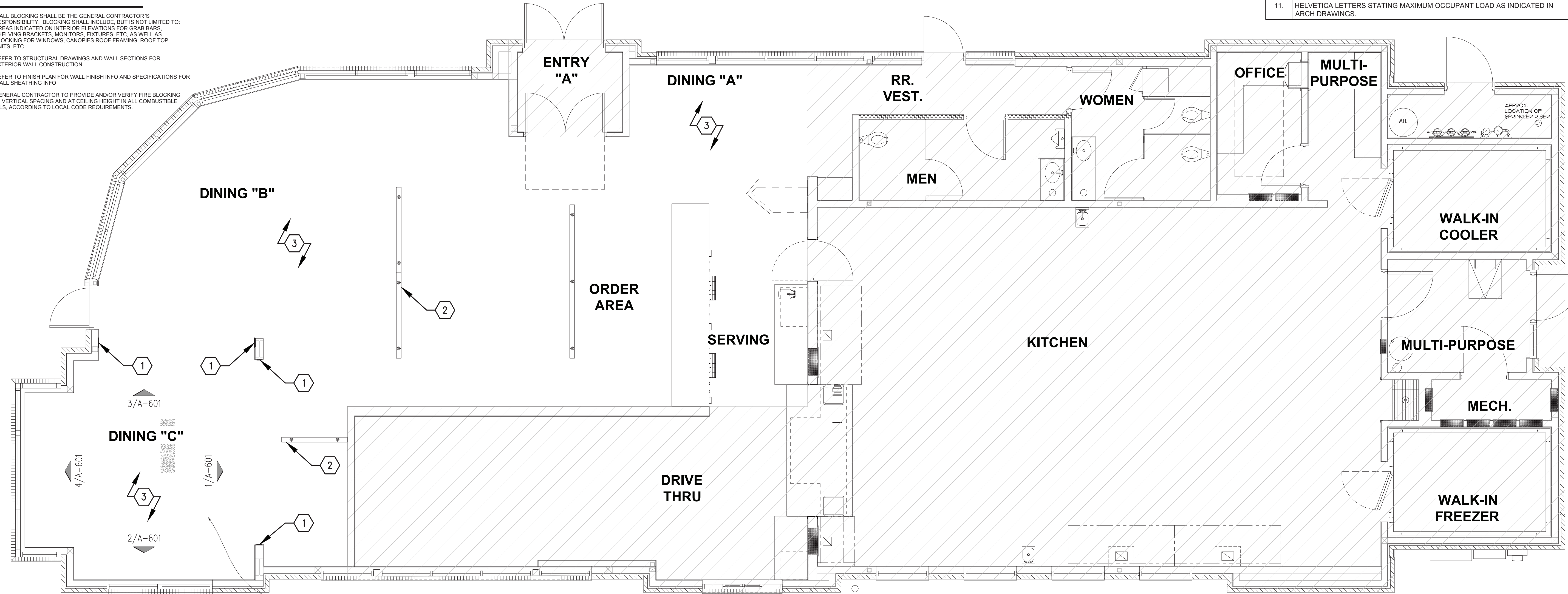
NOTE 2: REFER TO STRUCTURAL DRAWINGS AND WALL SECTIONS FOR EXTERIOR WALL CONSTRUCTION.

NOTE 3: REFER TO FINISH PLAN FOR WALL FINISH INFO AND SPECIFICATIONS FOR WALL SHEATHING INFO.

NOTE 4: GENERAL CONTRACTOR TO PROVIDE AND/OR VERIFY FIRE BLOCKING AT 10' MAX VERTICAL SPACING AND AT CEILING HEIGHT IN ALL COMBUSTIBLE STUD WALLS, ACCORDING TO LOCAL CODE REQUIREMENTS.

	PROPOSED PLAN KEY NOTES
1	PATCH AND REPAIR FINISHES WHERE DEMOLITION OCCURED. RE: FINISH FLOOR PLAN
2	NEW LOW HEIGHT WALL. RE: FURNITURE
3	REFER TO FURNITURE PLANS FOR SEATING AND QUEUING LAYOUT.

	FLOOR PLAN GENERAL NOTES
1.	ALL DIMENSIONS SHOWN ARE FRAMING DIMENSIONS (FACE OF STUD/JAMB) UNLESS OTHERWISE NOTED.
2.	FASTENERS, ANCHORS, CLIPS, STRAPS, ETC WHICH ARE IN CONTACT WITH PRESERVATIVE AND/OR FIRE TREATED WOOD SHALL BE OF G-185 HOT DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, OR AN APPROVED EQUAL.
3.	REFER TO CIVIL AND LANDSCAPE FOR LOCATIONS OF WALKS, BOLLARDS, LANDSCAPING AREAS, FLAG POLE, AND OTHER SITE ITEMS.
4.	REFER TO INTERIOR ELEVATIONS FOR LOCATIONS AND TYPES OF CORNER GUARDS.
5.	CONTRACTOR TO COORDINATE LOCATION OF POLE MOUNTED EXTERIOR CAMERA WITH STRONG SYSTEMS AND INSTALL UNDERGROUND CONDUIT AS REQUIRED. RE: ELECTRICAL.
6.	REFER TO ACCESSIBILITY PLAN AND FURNITURE DRAWINGS FOR SEATING LAYOUT & SPECIFICATIONS. CONTACT: OWNER.
7.	REFER TO ACCESSIBILITY PLAN AND OWNER DRAWINGS FOR CONDIMENT COUNTERS AND TRASH RECEPTACLES.
8.	REFER TO IT WALLBOARD USER GUIDE FOR WALLBOARD INSTALLATION, IF APPLICABLE.
9.	REFER TO MILLWORK PLAN FOR RAISED CONCRETE CURB LOCATIONS.
10.	REFER TO FINISH PLAN FOR NEW FINISHES.
11.	OCCUPANT LOAD SIGNS BY GC. PROVIDE 2"x8" BLACK SIGN WITH 1/2" WHITE HELVETICA LETTERS STATING MAXIMUM OCCUPANT LOAD AS INDICATED IN ARCH DRAWINGS.



FOUNDATION

FORMER PLAY AREA EXISTING RECESSED SLAB TO RECEIVE 1 1/2" CONCRETE TOPPING PER THE FOLLOWING SPECIFICATIONS.

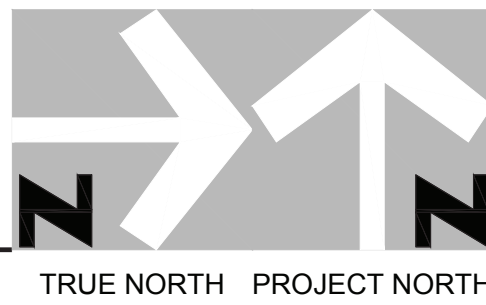
1. PRIOR TO PLACING SLAB, EXISTING CONCRETE SLAB SURFACE SHALL HAVE ANY EXISTING SEALERS REMOVED AND CLEANED OF DIRT AND DEBRIS.
2. SURFACE OF EXISTING CONCRETE SLAB SHALL BE ROUGHENED AND ALL LATENT MATERIAL AND DEBRIS REMOVED.
3. SURFACE OF EXISTING CONCRETE SLAB SHALL BE DAMPENED JUST PRIOR TO PLACING TOPPING SLAB.
4. CONCRETE TOPPING SHALL BE 3/8" MAX. SIZE AGGREGATE WITH 28 DAY MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI. MAXIMUM SLUMP SHALL BE 3 INCHES PRIOR TO ANY WATER REDUCING AGENTS ADDED.
5. CONCRETE TOPPING SHALL BE REINFORCED W 1-1/2 LBS. OF POLYPROPYLENE FIBERMESH PER CUBIC YARD OF CONCRETE.
6. SAWCUT CONTROL JOINTS SHALL BE PLACED WHERE EXISTING CONTROL JOINTS ARE LOCATED IN THE EXISTING CONCRETE SLAB.

FOUNDATION NOTES

1. TOP OF SLAB = 0'-0". 0'-0" IS FOR REFERENCE ONLY. SEE CIVIL FOR NGVD ELEVATION.
2. SEE MECH FOR ALL SLEEVES, PIPES, INSERTS AND EMBEDDED ITEMS.

PROPOSED FLOOR PLAN

1/4"=1'-0"



Chick-fil-A
Chick-fil-A

5200 Buffington Road
Atlanta, Georgia
30349-2998

INTERPLAN

INTERPLAN LLC

ARCH COA #2015008774
ENG COA #2003026904

ARCHITECTURE
ENGINEERING
PERMITTING

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ALTAMONTE SPRINGS, FL 32701
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SEAL:



LAUREL ROSE MARTIN - ARCHITECT
LIC. # A-2019008772

CHICK-FIL-A
SUMMIT FAIR

690 NW BLUE PKWY
LEE'S SUMMIT, MO 64086

FSR#02859

BUILDING TYPE / SIZE: S08N-104-R, V6
RELEASE:
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NO.	DATE	DESCRIPTION
1	02/19/24	PLAY AREA REMOVAL

CONSULTANT PROJECT #	2023.0467
DATE	AUGUST 2023
DRAWN BY	SN
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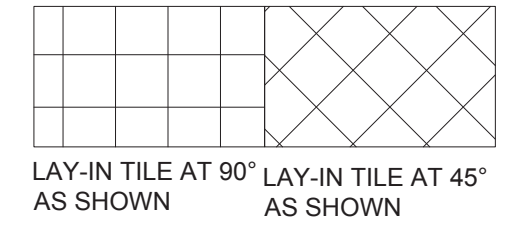
PROPOSED FLOOR PLAN

SHEET NUMBER
A-201

JESCHE - 07/17/2024 3:52:41 PM

LEGEND

FLOOR TILE.
REFER TO FINISH PLAN AND SCHEDULE



HATCHED AREAS
DENOTES AREAS NOT
IN SCOPE OF WORK
UNLESS NOTED
OTHERWISE.

FINISH PLAN KEY NOTES

- EXISTING FLOORING TO REMAIN. PATCH AND REPAIR FINISHES AFFECTED BY DEMOLITION.
- NEW FLOORING AS SCHEDULED.
- PLAY AREA - NEW FLOORING AS SCHEDULED. REFER TO A-201 FOR SLAB-INFILL INFORMATION.

FINISH PLAN GENERAL NOTES

- PATCH AND REPAIR EXISTING FLOORING DISTURBED BY DEMOLITION. MATCH EXISTING TILE AND FINISH AND ADJACENT SURFACE. SET NEW TILE WITH ULTRA FLEX RAPID THIN SET GROUT. COLOR TO MATCH EXISTING.
- REFER TO SHEET A-005 FOR SCHEDULE OF FINISHES.
- TILE BASE TO WRAP CONCRETE CURBS AT SERVING COUNTER.
- DINING AREAS PART OF SCOPE OF WORK TO RECEIVE LEVEL 4 PAINT FINISHES.
- REFER TO PLUMBING PLANS FOR FLOOR DRAIN / SINK LOCATIONS.
- ALL DRAIN OVERS TO BE FLUSH WITH THE TOP OF TILE.
- ALL EXISTING FLOOR DRAINS THAT ARE ABANDONED NEED TO BE LEVELED AROUND ANY DRAINS OR CLEANOUT LOCATIONS.
- PROVIDE CG-3 AROUND PERIMETER OF BOH WINDOWS / DOOR
- ALL EXTERIOR DOORS TO RECEIVE THRESHOLD SET IN BED OF MASTIC.
- NO FINGER TILES ALLOWED AT CORNERS OF FLOOR TILE BASE.
- REFER TO FURNITURE PLANS FOR CORE DRILLING LOCATIONS IN DINING AREAS
- TILE CONTRACTOR SHALL PULL FLOOR AND WALL TILE FROM MULTIPLE BOXES TO ENSURE TILE COLOR VARIATION IN PUBLIC AREAS.
- PROVIDE FRACTURE MEMBRANE/SLIP SHEET AT ALL CONTROL/EXPANSION JOINTS TO ALLOW TILE TO BRIDGE.
- FLOAT & PREPARE FLOOR SLAB FOR NEW FLOOR TILE.
- CFA VENDOR TO CLEAN FLOOR AND EQUIPMENT.
- FOR WAINSCOTTING WHERE GRAPHIC MESSAGE EXTENDS ABOVE TOP HORIZONTAL TRIM CAP. INSTALLER TO FIELD CUT VERTICAL BATTENS AND TOP CAP AT GRAPHICS AS REQUIRED.



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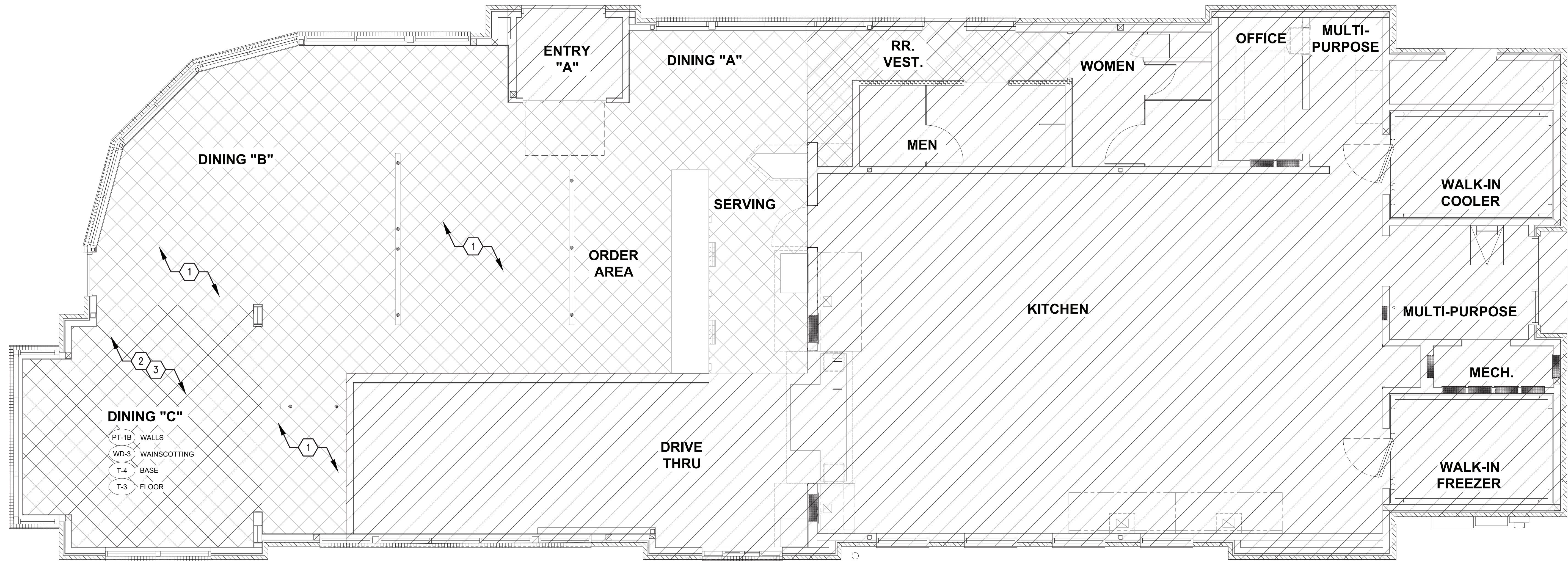
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LAUREL ROSE MARTIN - ARCHITECT
LIC. # A-2019008772



CHICK-FIL-A
SUMMIT FAIR
690 NW BLUE PKWY
LEE'S SUMMIT, MO 64086

FSR#02859

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CONSULTANT PROJECT # 2023.0467
DATE AUGUST 2023
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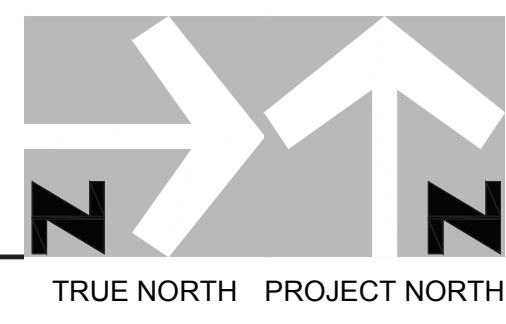
SHEET
FINISH FLOOR PLAN

SHEET NUMBER

A-211

JESCHE - 07/17/2024 3:53:08 PM

FINISH FLOOR PLAN
1/4"=1'-0"



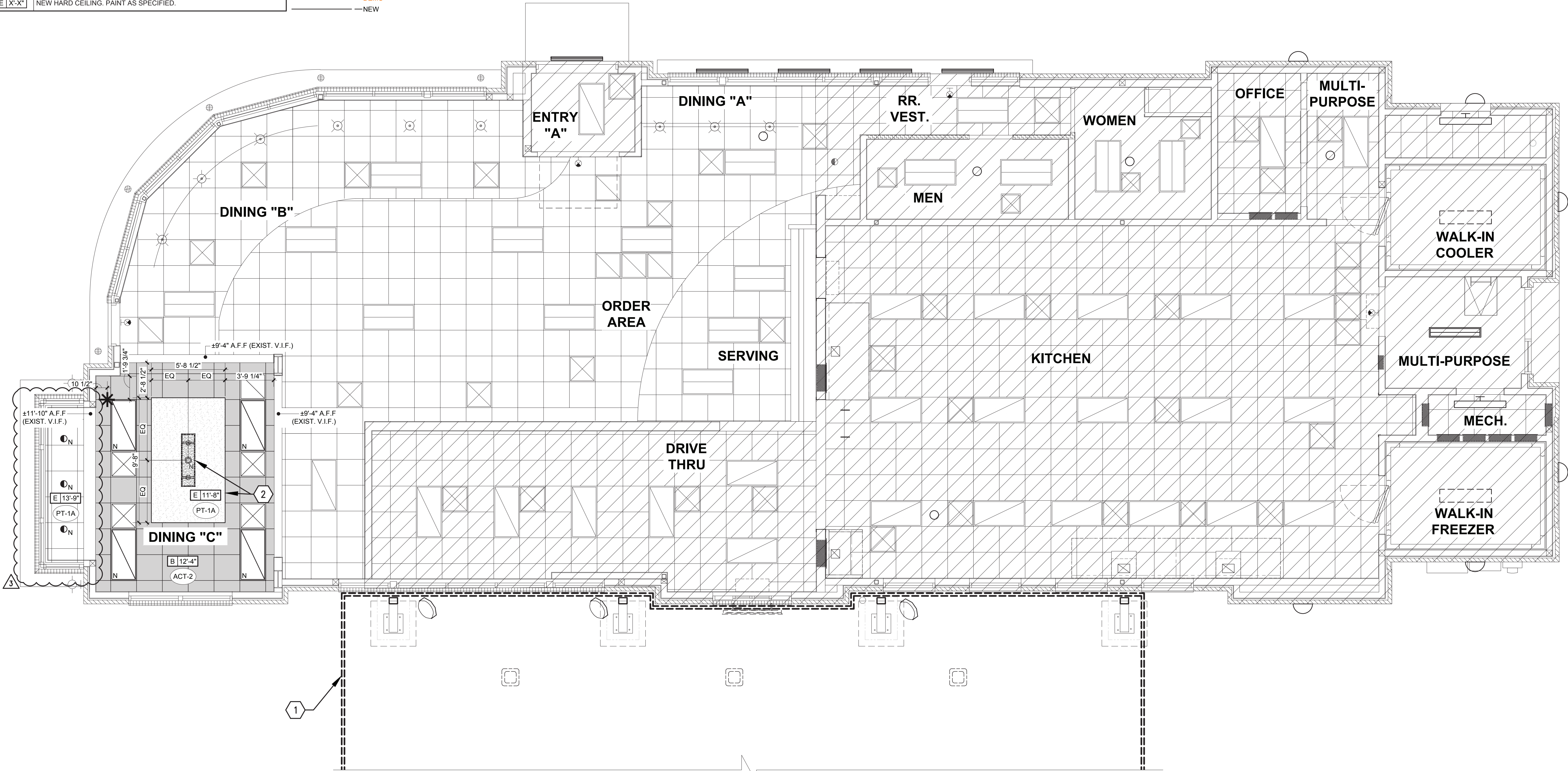
RCP GENERAL NOTES	
1.	REFER TO ELECTRICAL SHEETS FOR MORE INFORMATION ON LIGHT SPECS.
2.	ALL EMERGENCY LIGHTING IS EXISTING TO REMAIN U.N.O.
3.	CEILINGS ARE EXISTING TO REMAIN U.N.O.
4.	REFER TO MECHANICAL SHEETS FOR CLARIFICATION ON NEW AND EXISTING SUPPLY AND RETURN DIFFUSERS.
5.	REFER TO MANUFACTURER SPEC SHEETS FOR INSTALLATION OF CEILING MOUNTED AIR CURTAIN.
6.	ALL EXTERIOR LIGHTING TO REMAIN U.N.O.
7.	EXISTING AWNINGS AND ENTRY CANOPY TO REMAIN U.N.O.
8.	GC TO COORDINATE WITH OWNER REPRESENTATIVE FOR SECURITY CAMERAS SCOPE.

CEILING TYPE LEGEND	
SYMBOL	DESCRIPTION
<div>xxx-x</div>	FINISH TAG. RE: FINISH SCHEDULE
<div>A EXIST</div>	EXISTING ACOUSTICAL CEILING TILE AND SUSPENSION GRID TO REMAIN. PATCH AND REPAIR AS NEEDED. MATCH HEIGHT OF EXISTING CEILING.
<div>B X'-X"</div>	NEW ACOUSTICAL CEILING TILE AND SUSPENSION GRID SYSTEM. MATCH HEIGHT OF EXISTING CEILING
<div>C EXIST</div>	NEW ACOUSTICAL CEILING TILES - EXISTING GRID TO REMAIN. MATCH EXISTING TILE TYPE.
<div>D EXIST</div>	EXISTING TO REMAIN CLEAN, PATCH & REPAIR AS NEEDED
<div>E X'-X"</div>	NEW HARD CEILING. PAINT AS SPECIFIED.

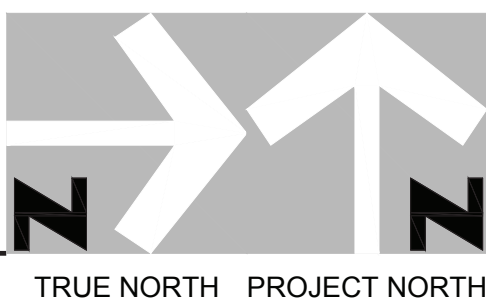
CEILING LEGEND		
<div></div>	NEW ACT AND GRID PER SCHEDULE	<div></div> EXISTING ACT AND GRID TO REMAIN
<div></div>	6" RECESSED LED FIXTURE	<div></div> DIFFUSERS/ RETURNS. RE: MECH
<div></div>	2x2 RECESSED FLUORESCENT LIGHT FIXTURE	<div></div> 2x4 RECESSED FLUORESCENT LIGHT FIXTURE
<div></div>	CEILING MOUNTED AIR CURTAIN. RE: MECH	<div></div> BACK OF HOUSE FLY LIGHT. RE: KITCHEN DRAWINGS
<div></div>	HATCHED AREAS DENOTES AREAS NOT IN SCOPE OF WORK UNLESS NOTED OTHERWISE.	<div>*</div> CEILING STARTING POINT
LIGHTING NOTES: 1. REFER TO ELECTRICAL SHEETS FOR MORE INFORMATION ON LIGHT SPECS. 2. ALL EMERGENCY LIGHTING IS EXISTING TO REMAIN U.N.O.		

— EXISTING
- - - RELOCATED
- - - DEMO
— NEW

PROPOSED RCP KEY NOTES	
1	REFER TO ARCHITECTURAL SITE PLAN & CANOPY SHOP DRAWINGS FOR CANOPY SCOPE.
2	NEW SOFFIT AND COKE LIGHT FIXTURE CENTERED ABOVE GATHERING TABLE. RE: FURNITURE



PROPOSED REFLECTED CEILING PLAN
1/4"=1'-0"



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LIC. # A-2019008772

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SUMMIT FAIR

690 NW BLUE PKWY
LEE'S SUMMIT, MO 64086

FSR#02859

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REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	02/19/24	PLAY AREA REMOVAL
3	04/03/24	DESIGN COMMENTS

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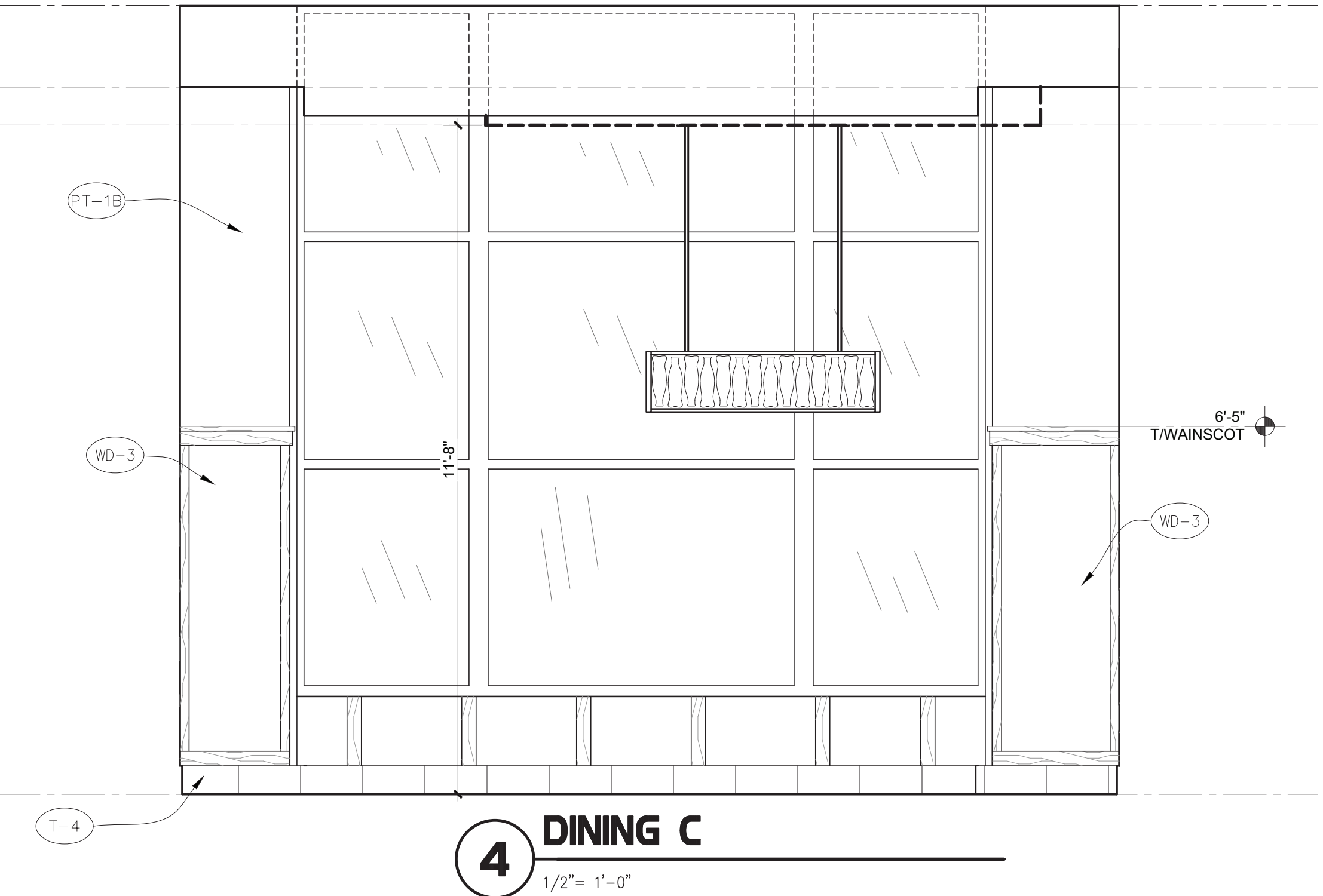
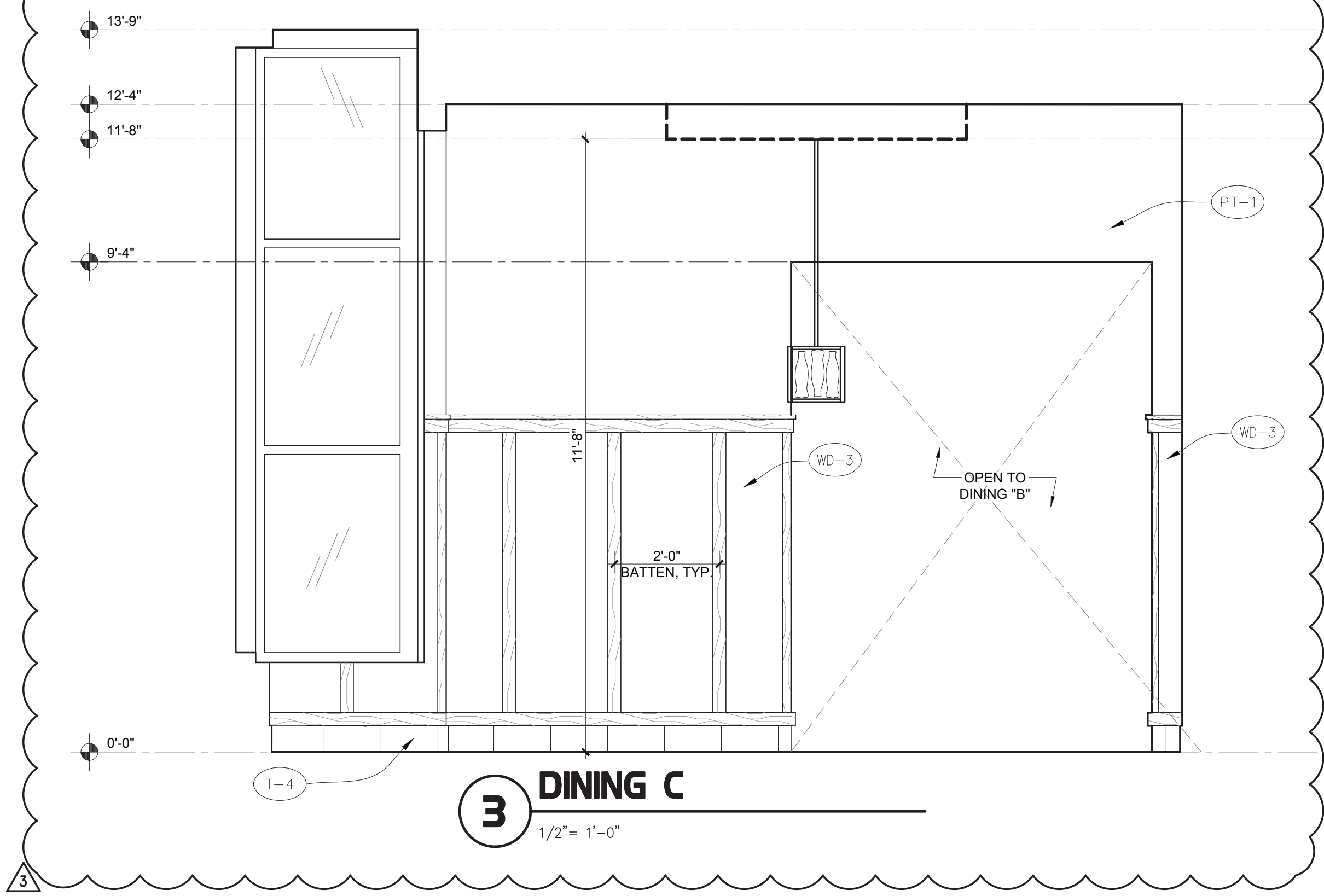
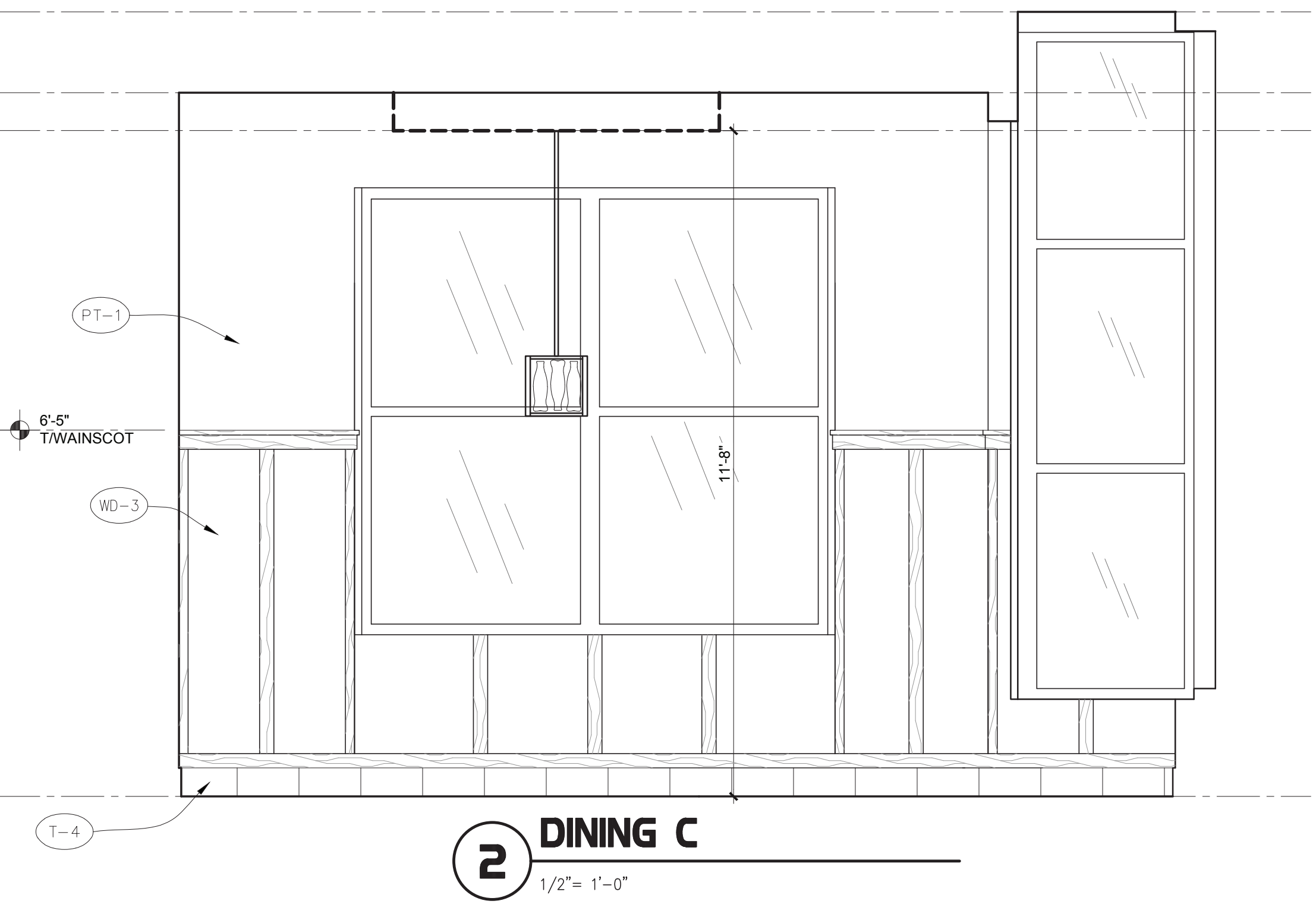
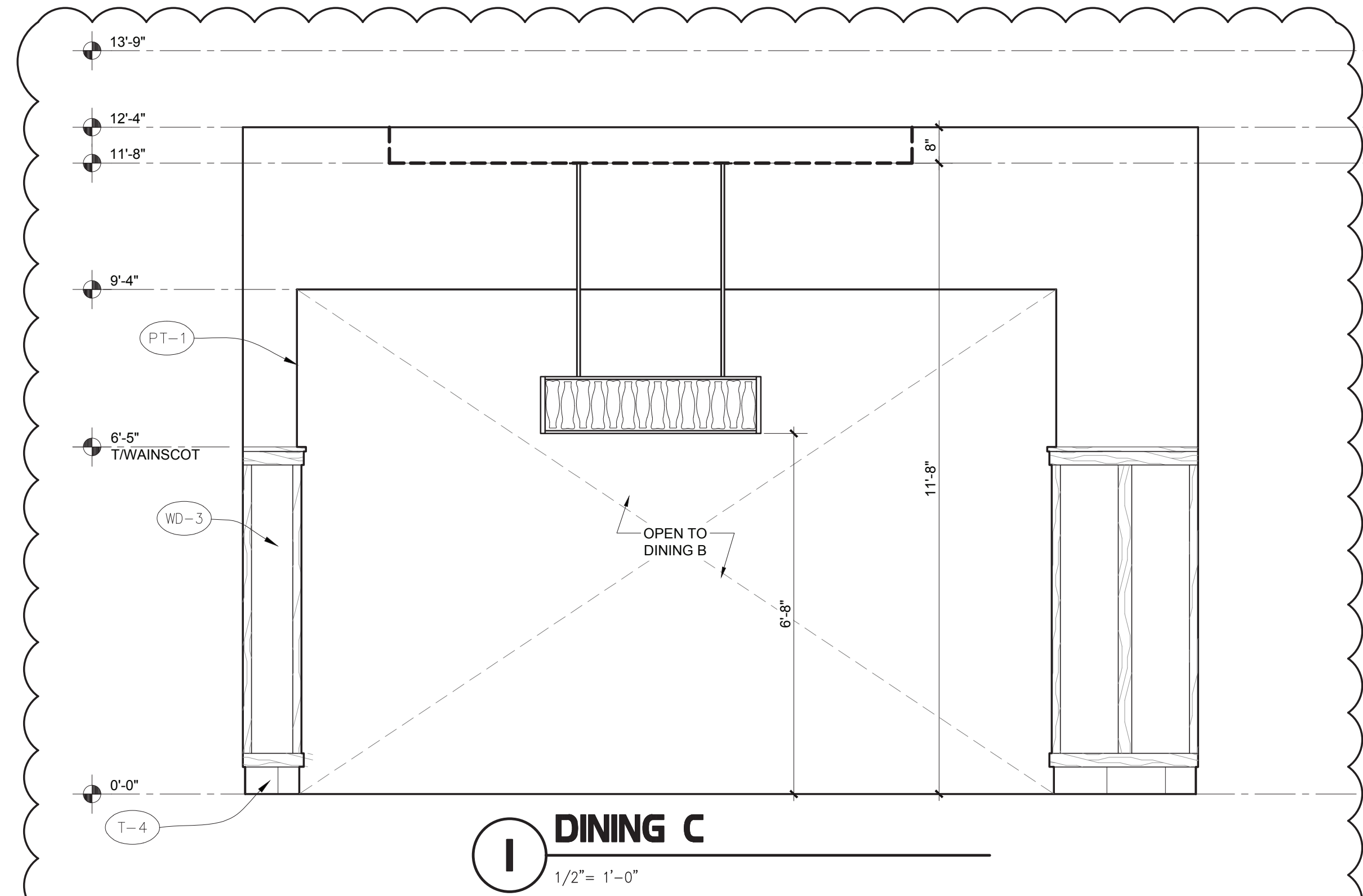
PROPOSED REFLECTED
CEILING PLAN

SHEET NUMBER

A-221

JESCHE - 07/17/2024 3:52:02 PM

INTERIOR ELEVATION GENERAL NOTES	
1.	ALL DIMENSIONS SHOWN ARE FRAMING DIMENSIONS (FACE OF STUD/JAMB) UNLESS OTHERWISE NOTED.
2.	GC TO PROVIDE BLOCKING FOR ALL WALL SHELVEING AND EQUIPMENT. REFER TO K SHEET FOR LOCATIONS. REFER TO ARCH DETAIL SHEETS FOR LOCATIONS.
3.	PROVIDE 5" HIGH CEMENTITIOUS BOARD SUBSTRATE AT BASE OF ALL WALLS IN DINING ROOM AND VESTIBULE.
4.	PROVIDE 12" HIGH CEMENTITIOUS BOARD SUBSTRATE AT BASE OF ALL WALLS IN KITCHEN.
5.	PROVIDE CEMENTITIOUS BOARD SUBSTRATE FOR THE FULL HEIGHT OF TILE AT ALL WALLS WITH WALL TILE.
6.	AT WAINSCOT LOCATIONS, PROVIDE 5" HIGH CEMENTITIOUS BOARD BEHIND TILE BASE, THEN PLYWOOD SUBSTRATE TO TOP OF WAINSCOT, THEN GYPSUM BOARD TO CEILING.
7.	BRAND ICON DIRECTION: WHEN ENTERING THE STORE FROM THE MAIN ENTRY, THE BEAK SHOULD POINT TO THE RIGHT.
8.	REFER TO FRN SHEETS FOR SIGNAGE LOCATIONS.
9.	REFER TO ARCH DETAILS FOR CORNER GUARD DETAIL.
10.	REFER TO KITCHEN PLANS FOR EQUIPMENT AND COUNTER TOP LAYOUTS.





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Atlanta, Georgia
30349-2998

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LAUREL ROSE MARTIN - ARCHITECT
LIC. # A-2019008772

CHICK-FIL-A
SUMMIT FAIR
690 NW BLUE PKWY
LEE'S SUMMIT, MO 64086

FSR#02859
BUILDING TYPE / SIZE: S08N-104-R, V8
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3	04/03/24	DESIGN COMMENTS

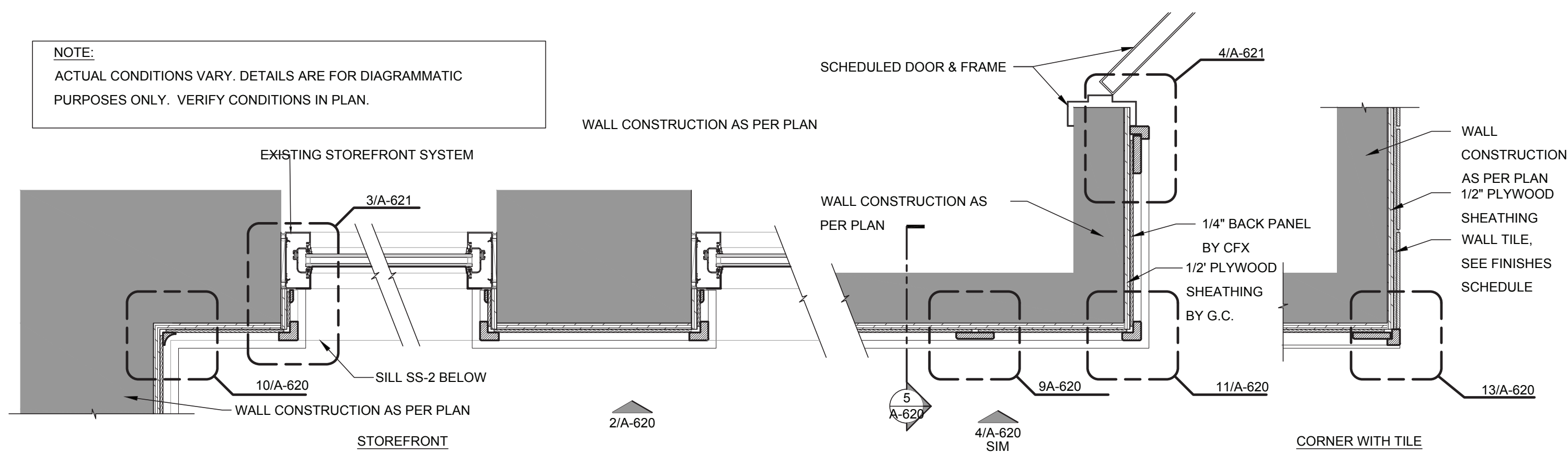
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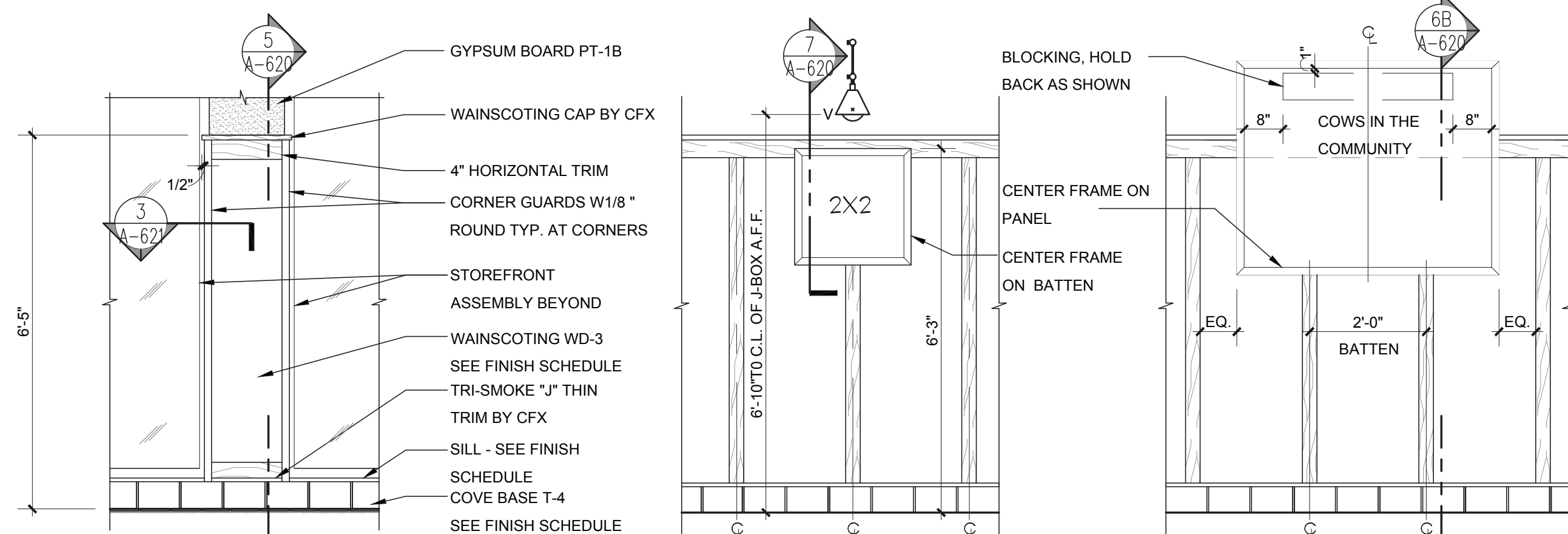
INTERIOR ELEVATIONS
SHEET NUMBER
A-601

JESCHE - 07/17/2024 3:53:00 PM

NOTE:
ACTUAL CONDITIONS VARY. DETAILS ARE FOR DIAGRAMMATIC
PURPOSES ONLY. VERIFY CONDITIONS IN PLAN.

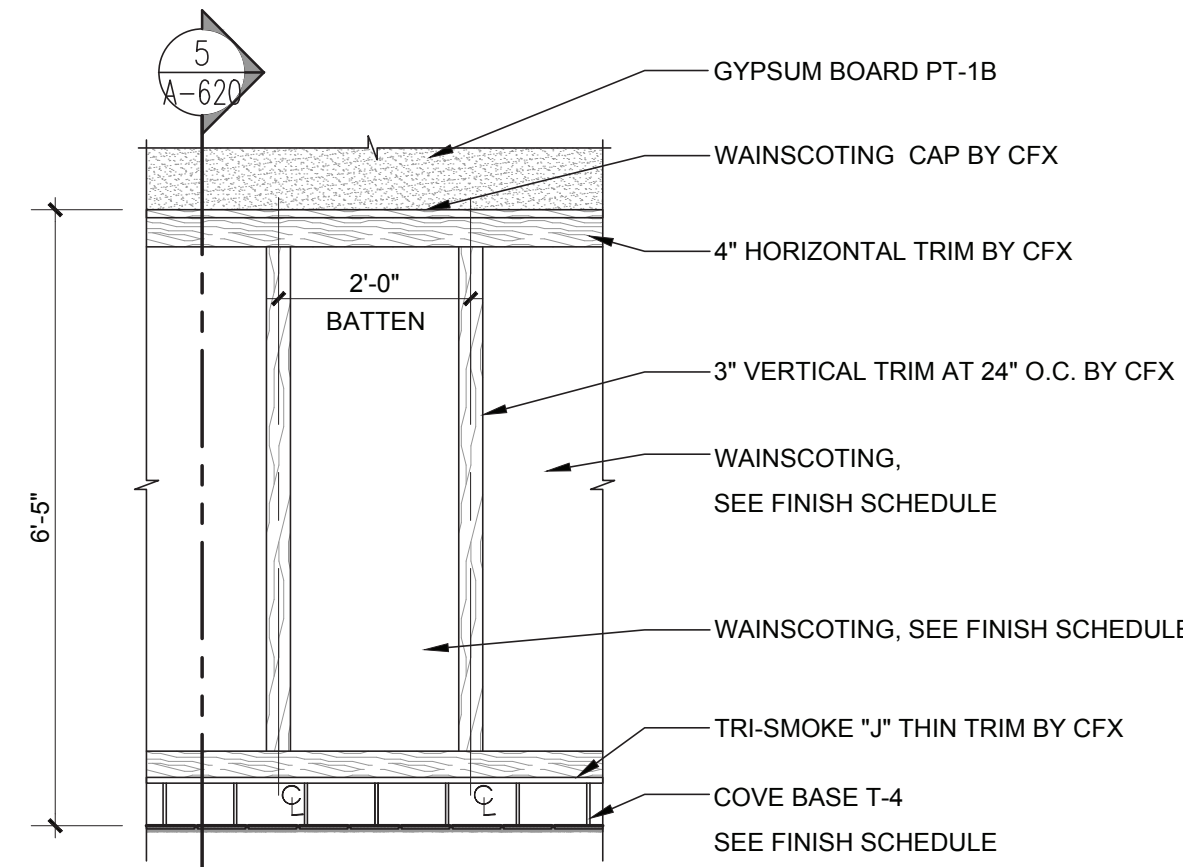


1 WAINSCOTTING PLAN DIAGRAM
SCALE: 1 1/2" = 1'-0"

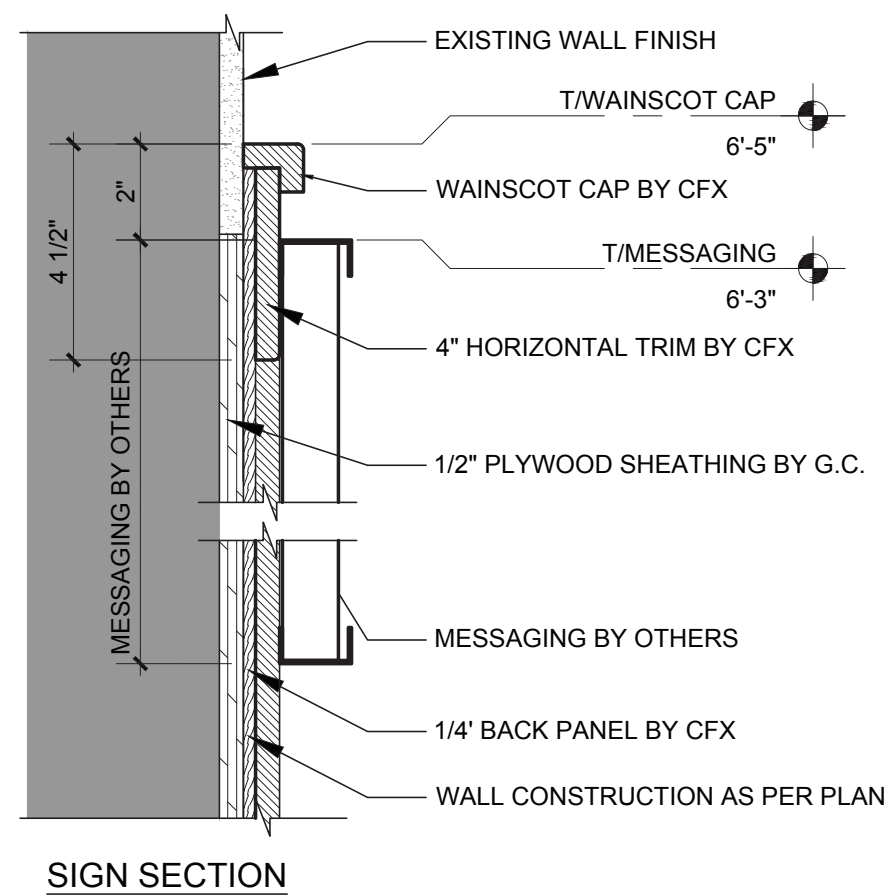


2 WAINSCOTTING ELEVATION AT WINDOW
SCALE: 1/2" = 1'-0"

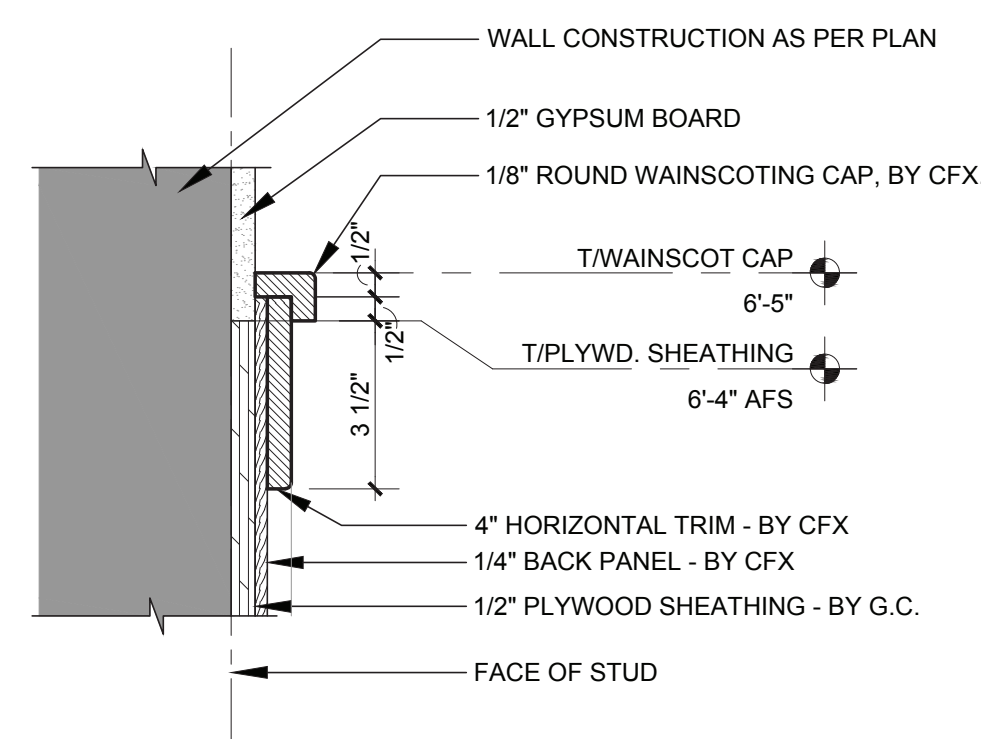
3 WAINSCOTTING ELEVATION AT SIGNAGE
SCALE: 1/2" = 1'-0"



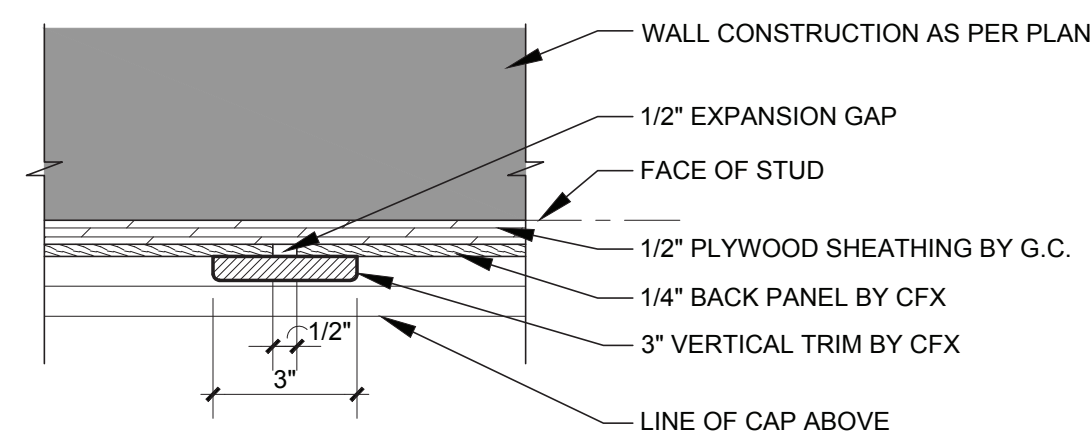
4 TYPICAL WAINSCOTTING ELEVATION
SCALE: 1/2" = 1'-0"



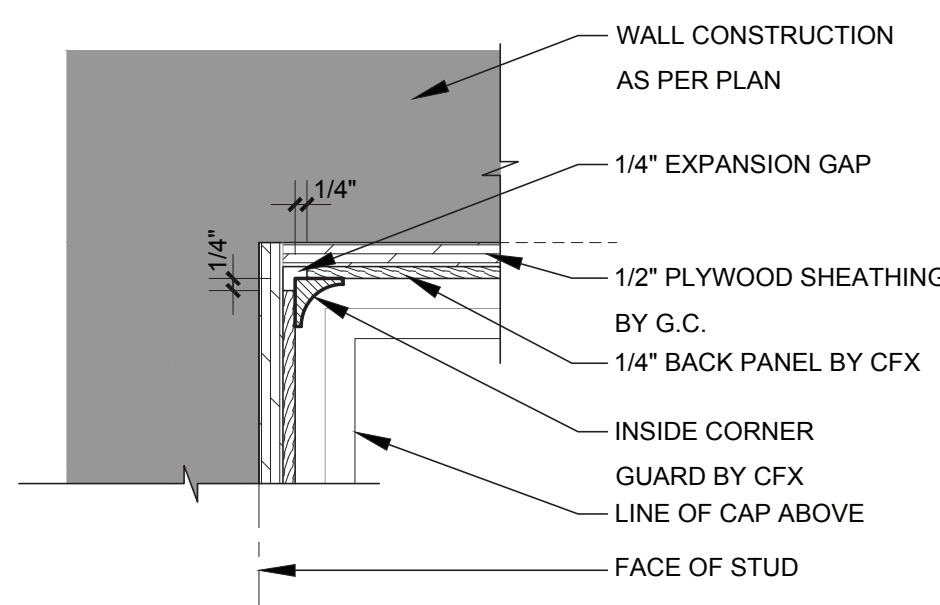
7 MESSAGING AT WAINSCOTTING
SCALE: 3" = 1'-0"



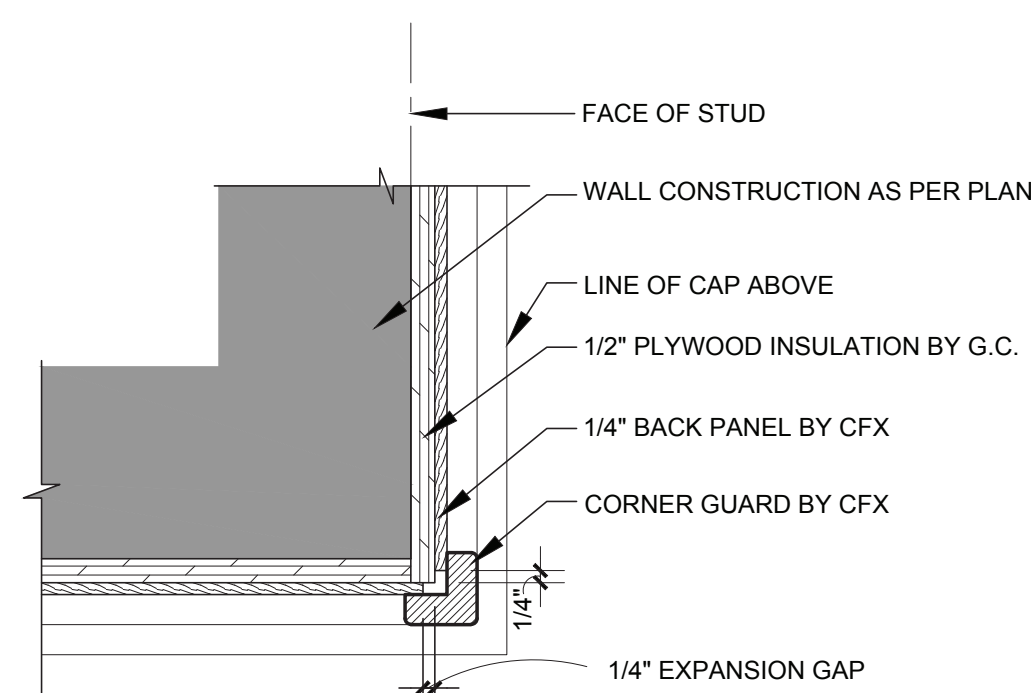
8 WAINSCOTTING CAP DETAIL
SCALE: 3" = 1'-0"



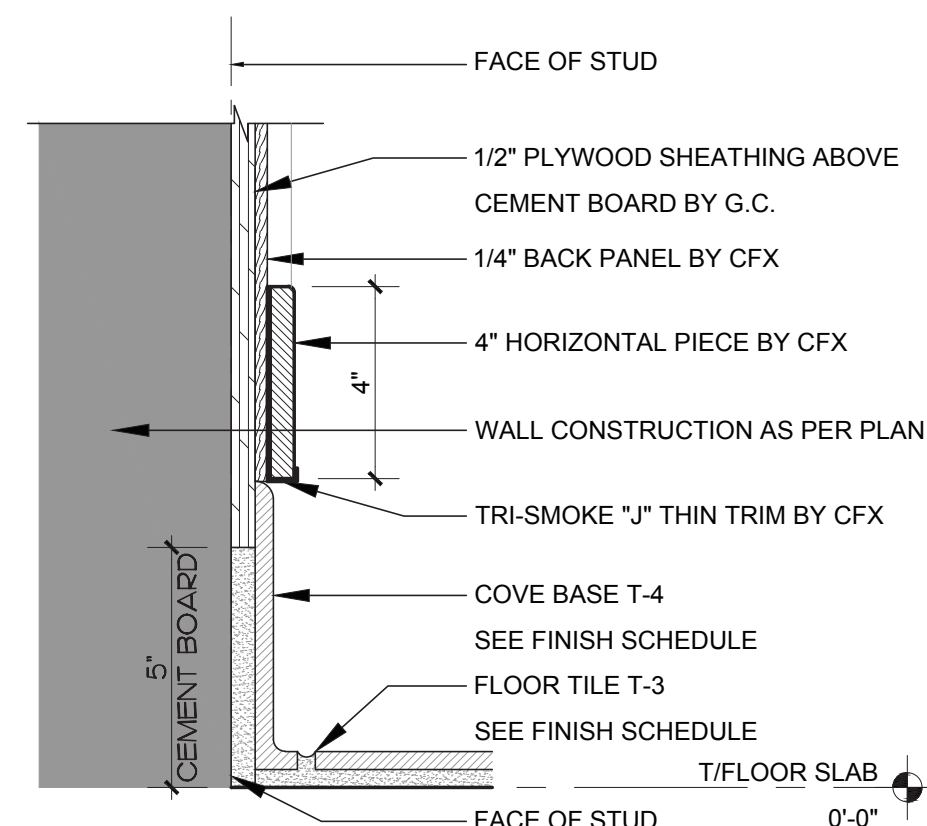
9 WAINSCOTTING TRIM DETAIL
SCALE: 3" = 1'-0"



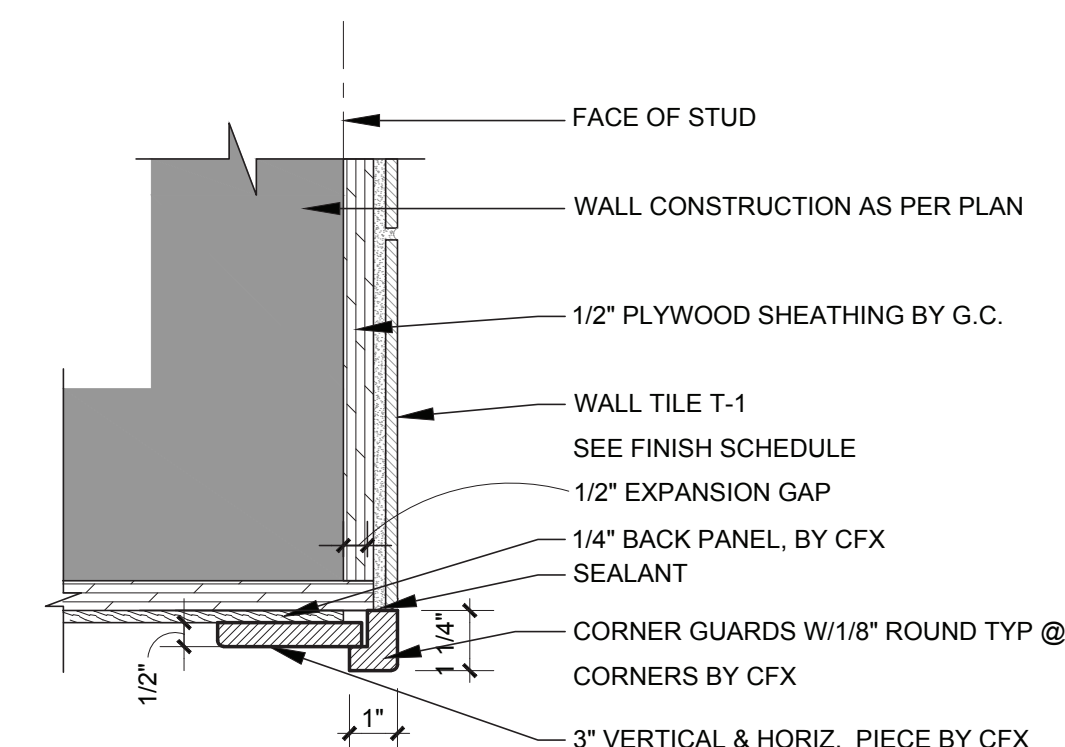
10 WAINSCOTTING CORNER DETAIL
SCALE: 3" = 1'-0"



11 WAINSCOTTING CORNER DETAIL
SCALE: 3" = 1'-0"



12 WAINSCOTTING BASE DETAIL
SCALE: 3" = 1'-0"



13 WAINSCOTTING TO TILE CORNER
SCALE: 3" = 1'-0"

GENERAL NOTE:
WAINSCOTTING MATERIAL BY CFX/ G.C. TO ORDER FROM CFX & INSTALL:
1. 1/4" PLYWOOD (WHITE OAK, PLAIN CUT W/ LUAN CORE)
2. SOLID STOCK TO BE WHITE OAK (STAINED GRIFFIN LIGHT W/ GLAZE)

5 WALL SECTION AT WAINSCOTTING
SCALE: 1" = 1'-0"

6A PLAN VIEW

6B COWS IN THE COMMUNITY
SIGN SECTION

6 MESSAGING AT WAINSCOTTING
SCALE: 3" = 1'-0"



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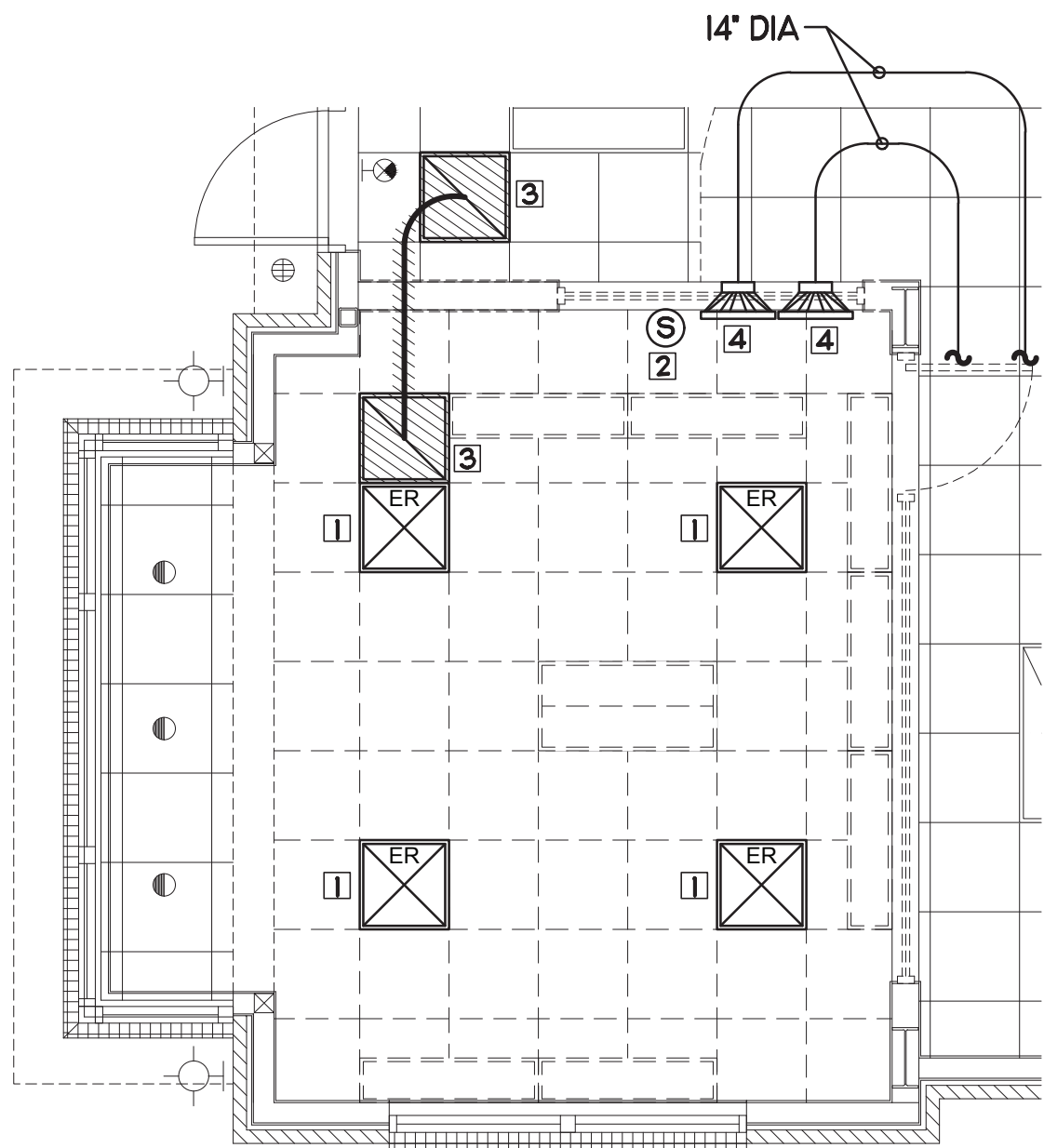
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SHEET

INTERIOR DETAILS
SHEET NUMBER

A-620

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1 MECH DEMOLITION PLAN
SCALE: 1/4"=1'-0"

DEMOLITION KEY NOTES

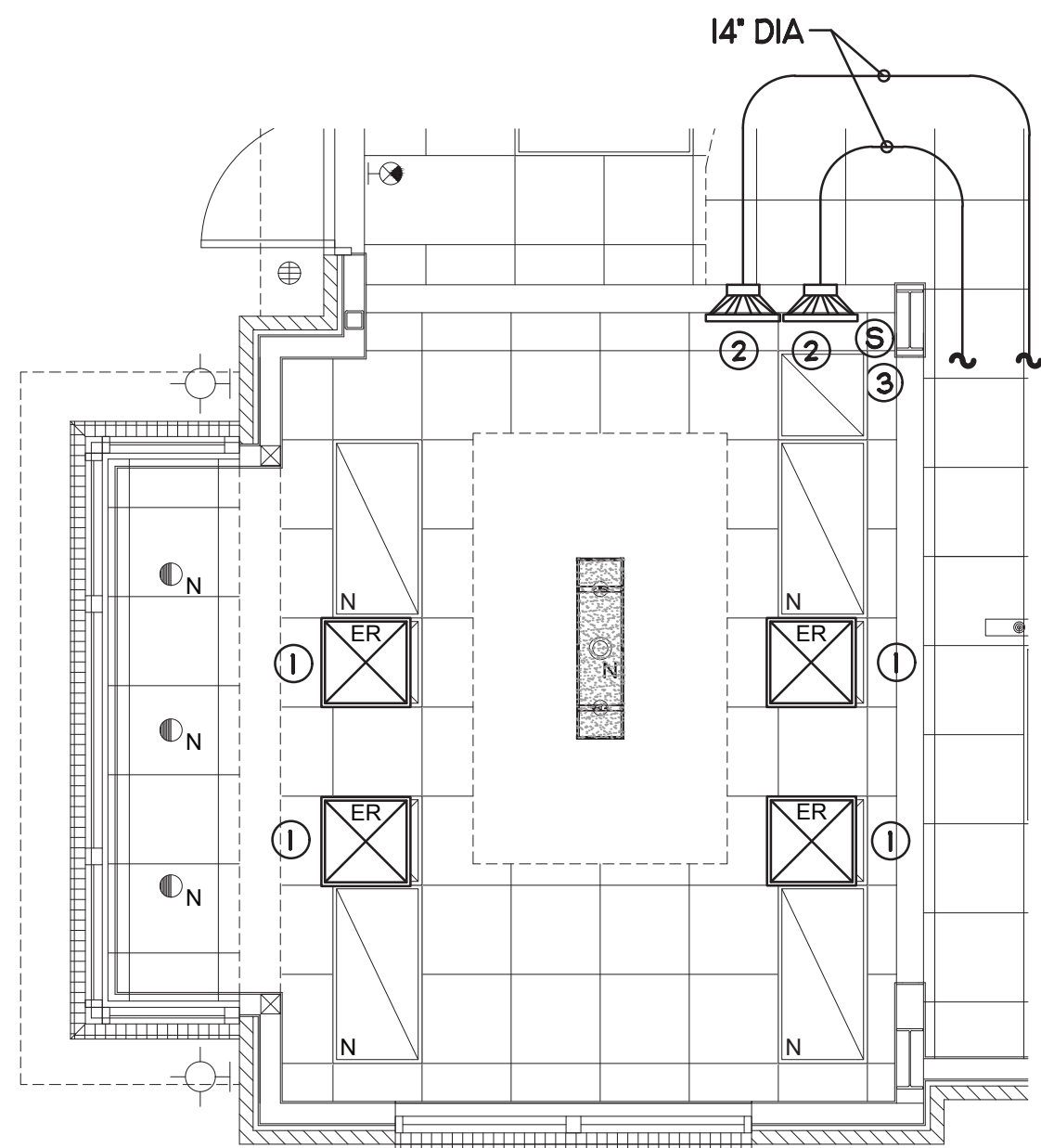
- 1 EXISTING AIR DEVICE TO BE RELOCATED.
- 2 EXISTING REMOTE TEMPERATURE SENSOR TO BE RELOCATED.
- 3 DEMOLISH EXISTING TRANSFER AIR GRILLS.
- 4 EXISTING AIR DEVICE TO REMAIN.

DEMOLITION LEGEND

- EXISTING EQUIPMENT, DUCT, AIR DEVICES ETC. TO REMAIN INTACT.
- EXISTING EQUIPMENT, DUCT, AIR DEVICES ETC. TO BE DEMOLISHED.

HVAC LEGEND

A-12-400	TYPE - NECK SIZE - CFM	EF#1	EX-HAUST FAN #1 (TYP.)
	SPIN-IN FITTING WITH MANUAL BALANCING DAMPER, WITHOUT SCOOP	AC#1	AIR CONDITIONING UNIT #1
	SPIN-IN		HARD DIFFUSER
	FLEXIBLE DIFFUSER		RETURN/EXHAUST (TYP.)
	EXISTING EQUIPMENT, DUCT, & AIR DEVICE		SUPPLY DIFFUSER, SQ FACE
	NEW EQUIPMENT, DUCT, & AIR DEVICE		PLAN NOTE REFERENCE
	THERMOSTAT		MANUAL VOLUME DAMPER



2 MECH FLOOR PLAN
SCALE: 1/4"=1'-0"

MECH. KEY NOTES

- 1 EXISTING AIR DEVICE TO BE RELOCATED.
- 2 EXISTING AIR DEVICE TO BE REMAIN.
- 3 RELOCATE AC#3 THERMOSTAT WALL MOUNTED AT 5'-0" AFF, ROUTE WIRING BACK TO SUNCOAST CONTROL PANEL.



Chick-fil-A
Chick-fil-A
5200 Buffington Road
Atlanta, Georgia
30349-2998



ARCH COA #2015008774
ENG COA #2003026904

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ENGINEERING
PERMITTING

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STACY HENSON - PROFESSIONAL ENGINEER
LIC. # PE-2016036828

CHICK-FIL-A
SUMMIT FAIR

690 NW BLUE PKWY
LEE'S SUMMIT, MO 64086

FSR#02859

BUILDING TYPE / SIZE: S08N-104-R, V6
RELEASE:
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REVISION SCHEDULE
NO. DATE DESCRIPTION
01 02/19/24 PLAY AREA REMOVAL

CONSULTANT PROJECT # 2023.0467
DATE AUGUST 2023
DRAWN BY AR
CHECKED BY DAK

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MECHANICAL FLOOR PLAN

SHEET NUMBER

M1.1

E

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C

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C

B

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SHEET

MECH. SPECIFICATIONS & SCHEDULES

SHEET NUMBER

M2.1



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AIR DEVICE SCHEDULE						
MARK	DESCRIPTION	LOCATION	NECK SIZE	FACE SIZE	FRAME TYPE	REMARKS
A	PRICE MODEL APDC ALUMINUM SUPPLY AIR DIFFUSER WITH INDIVIDUALLY ADJUSTABLE CURVED AIR PATTERN CONTROLLERS.	DINING AREA KITCHEN TEAM MEMBER	SEE PLAN	24X24	LAY-IN	I2
NOTES	<ul style="list-style-type: none">MECHANICAL CONTRACTOR SHALL PURCHASE THE AIR DEVICES DIRECTLY FROM TOM BARROW COMPANY. CONTACT MR. SCOTT GEORGE AT 404-351-HOIO, FOR PRICING AND AVAILABILITY. AIR DEVICES NOT PURCHASED THRU TOM BARROW COMPANY WILL NOT BE ACCEPTED.					
REMARKS	<ul style="list-style-type: none">1. STANDARD OFF WHITE FINISH.2. FACTORY INSULATED R-6 BACKPAN.					

I. SECTION C15000 - MECHANICAL SPECIFICATIONS

PART I - GENERAL

1.01 SCOPE

- A. IT IS THE RESPONSIBILITY OF CONTRACTOR TO READ ALL SPECIFICATIONS AND CONSULT ALL DRAWINGS WHICH MAY AFFECT THE INSTALLATION AND COORDINATION OF HIS WORK WITH OTHER TRADES. CONTRACTOR SHALL COORDINATE AND MAKE MINOR ADJUSTMENTS IN LOCATION OF EQUIPMENT AND MATERIALS AS NECESSARY TO SECURE COORDINATION.
- B. COMPLETED INSTALLATION SHALL CONFORM TO ALL APPLICABLE FEDERAL, STATE AND LOCAL CODES AND ORDINANCES, INCLUDING BUT NOT LIMITED TO THE LATEST APPROVED EDITIONS OF NFPA-96, NFPA-90A, NFPA-54, SMACNA, ASHRAE 90.1 AND ASHRAE 62.
- C. SYSTEM LAYOUT IS SCHEMATIC AND EXACT LOCATIONS SHALL BE DETERMINED BY STRUCTURAL CONDITIONS, COORDINATION WITH OTHER TRADES, COORDINATION WITH FINISHES AND OTHER CONDITIONS. STRUCTURAL SUPPORTS SHALL NOT BE CUT OR ALTERED TO ASSURE FIT OF HVAC SYSTEM. TEN FOOT CLEARANCE SHALL BE MAINTAINED BETWEEN OUTSIDE AIR INTAKES AND EXHAUST FANS AND PLUMBING VENT TERMINALS.
- D. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DEFECTS, REPAIRS AND REPLACEMENTS IN MATERIALS AND WORKMANSHIP FOR A PERIOD OF ONE (1) YEAR AFTER FINAL PAYMENT IS APPROVED. CONTRACTOR SHALL HONOR FACTORY WARRANTIES ON ALL EQUIPMENT PROVIDED AS PART OF THIS SYSTEM.
- E. UPON COMPLETION OF PROJECT, ALL SYSTEM EQUIPMENT AND MATERIALS SHALL BE IN NEW, CLEAN CONDITION WITH ALL DAMAGE RESTORED TO CONDITION ACCEPTABLE TO THE OWNER'S REPRESENTATIVE. ALL EQUIPMENT, COMPONENTS AND DUCTWORK SHALL BE INSPECTED AND THOROUGHLY CLEANED, READY FOR USE. AT COMPLETION OF JOB, ALL MISCELLANEOUS TOOLS, SCAFFOLDING, SURPLUS MATERIALS, RUBBISH AND DEBRIS SHALL BE REMOVED BY CONTRACTOR.
- F. CONTRACTOR SHALL PROVIDE TWO SETS OF 2" MERV 8 OR HIGHER THROW AWAY TYPE FILTERS. A CLEAN SET SHALL BE PROVIDED PRIOR TO TEST AND BALANCE AND AGAIN PRIOR TO OPENING.

2.01 DUCTWORK (C15735)

- A. ACCEPTABLE MANUFACTURERS OF INSULATION ARE MANVILLE, OWENS CORNING OR KNAUF.
- B. ALL DUCTWORK SHALL BE SHEET METAL, UNLESS NOTED OTHERWISE (U.N.O.).
- C. DUCT DIMENSIONS SHOWN ARE INSIDE CLEAR DIMENSIONS, U.N.O.
- D. CONSTRUCTION OF DUCTWORK SHALL MEET SMACNA 1" W.G. PRESSURE CLASS STANDARD AND RECOMMENDATIONS. SMACNA SHALL BE FOLLOWED WITH RESPECT TO GAGE THICKNESS, JOINTS, REINFORCING, CONSTRUCTION, INSTALLATION AND SUPPORT FOR PRESSURE CLASS STATED. ALL TRANSVERSE JOINTS IN RECTANGULAR AND ROUND DUCT INCLUDING DUCT CONNECTION TO AIR DEVICE COLLAR SHALL BE SEALED PER SMACNA SEAL CLASS C WITH UL DUCT MASTIC SEALANT APPROVED FOR INTENDED USE. DUCT TAPE IS NOT AN ACCEPTABLE SUBSTITUTE FOR MASTIC UNLESS EQUAL TO HARDCAST FOIL-GRIP 1402 BUTYL RUBBER ADHESIVE TAPE.
- E. DUCT SHALL BE SUPPORTED AT BASE OF DUCT DROPS. CURB DUCT RAILS ARE NOT INTENDED TO AND SHALL NOT SUPPORT THE WEIGHT OF THE DUCT.
- F. ALL DUCT INSULATION SHALL MEET MINIMUM R-VALUE REQUIRED BY ASHRAE 90.1 LATEST EDITION. ALL DUCT WRAP SHALL BE MINIMUM 2" THICK, 3/4 PCF AND 5.6 R-VALUE INSTALLED WITH EITHER A VAPOR BARRIER WITH MAXIMUM PERMEANCE 0.05 OR A MINIMUM 2 MIL ALUMINUM REINFORCED FOIL/CRAFT FACING.
- G. ALL DUCT DROPS FROM THE ROOFTOP UNITS SHALL BE EXTERNALLY INSULATED.
- H. SUPPLY AND RETURN AIR DUCTWORK SERVING ALL AREAS SHALL BE EXTERNALLY INSULATED.
- I. ALL AIR CONVEYANCE COMPONENTS SUCH AS, BUT NOT LIMITED TO DUCT, DUCT PLenums, GRILLES/DIFFUSERS, BACK PANS, AND BOOTS SHALL BE INSULATED. INSULATION TYPE IS COVERED ELSEWHERE IN THIS SPECIFICATION.
- J. RESTROOM RECTANGULAR EXHAUST AIR DUCTWORK SHALL BE LINED WITH 1" THICK, 1/2 PCF INSULATION.

- K. TRUNK DUCTS SHALL BE ISOLATED FROM UNIT VIBRATION WITH THE USE OF NFPA AND UL APPROVED FLEXIBLE CONNECTORS INSTALLED AT THE TOP OF BOTH SUPPLY AND RETURN DROPS.
- L. INSULATED FLEXIBLE DUCT MAY BE UTILIZED FOR RUNOUTS TO GRILLES AND DIFFUSERS ONLY IN THE HORIZONTAL POSITION AND IN MAXIMUM LENGTHS OF 4'-0", NO EXCEPTIONS. SEE TAKE-OFF DETAIL ON DRAWING M3.1.
- M. CONSTRUCTION OF FLEXIBLE DUCTWORK SHALL INCLUDE SPIRAL METAL HELIX BONDED TO A POLYESTER CORE, FIBERGLASS INSULATION WITH POLYETHYLENE OR MYLAR VAPOR BARRIER. ALL COMPONENTS SHALL HAVE APPROPRIATE UL APPROVAL AND SHALL BE EQUIVALENT TO THERMAFLEX MKE.
- N. FLEXIBLE DUCT SHALL BE INSTALLED PER THE "ADC FLEXIBLE DUCT PERFORMANCE AND INSTALLATION STANDARDS, 4TH ED" USING FOIL TAPE AND DRAWBAND ON THE INNER CORE AND TAPE OR DRAWBAND ON THE OUTER JACKET.
- O. DUCT TAPE SHALL BE EQUAL TO FASSON 181-B FX, 2-1/2" WIDE.
- P. SINGLE THICKNESS TURNING VANES SHALL BE INSTALLED AT 90 DEGREE TURNS IN SUPPLY DUCTWORK WHERE ANY ONE DIMENSION IS GREATER THAN 12".
- Q. RADIUSSED ELBOWS MAY BE SUBSTITUTED FOR 90 DEGREE ELBOWS AT THE DISCRETION OF THE CONTRACTOR. CENTERLINE RADIUS EQUAL TO, R=W PER FIGURE NO. 2-2 IN SMACNA HVAC DUCT CONSTRUCTION STANDARDS.
- R. EXTERNAL INSULATION ON BOTTOM OF DUCTS 24" OR WIDER SHALL BE SUPPORTED WITH STICK PINS ON 18" CENTERS. STICK PIN WASHERS SHALL BE COVERED WITH DUCT TAPE OR MASTIC.

PART III - EXECUTION

3.01 SCOPE

- A. FURNISH AND INSTALL SYSTEM IN ACCORDANCE WITH REFERENCED STANDARDS, APPLICABLE CODES, MANUFACTURER'S RECOMMENDATIONS AND AS INDICATED ON DRAWINGS.
- B. OWNER SHALL TEST AND BALANCE MECHANICAL SYSTEM IN ACCORDANCE WITH NC1 OR AABC STANDARDS TO ASSURE CONFORMANCE WITH DESIGN. GC WILL MAKE MECHANICAL CONTRACTOR AVAILABLE DURING TEST AND BALANCE TO ASSIST TESTING AGENCY AND TO MAKE CORRECTIONS IMMEDIATELY NECESSARY. CONTRACTOR SHALL CORRECT ITEMS ON WRITTEN TEST AND BALANCE REPORT.
- C. CONTRACTOR SHALL INSTRUCT THE OWNER'S REPRESENTATIVE IN ALL MATTERS PERTAINING TO THE PROPER MAINTENANCE OF EQUIPMENT FURNISHED UNDER THIS CONTRACT THROUGH DEMONSTRATION AND EXPLANATION OF OPERATING & MAINTENANCE MANUALS.
- D. CONTRACTOR SHALL PROVIDE A "SAMPLE MAINTENANCE PROPOSAL" TO THE OWNER'S REPRESENTATIVE IN ALL MATTERS PERTAINING TO THE PROPER MAINTENANCE OF EQUIPMENT FURNISHED UNDER THIS CONTRACT.
- E. CONTRACTOR SHALL COMPLETE A/C EQUIPMENT STARTUP DOCUMENTATION PROVIDED BY OWNER.

FIELD VERIFY ALL CONDITIONS

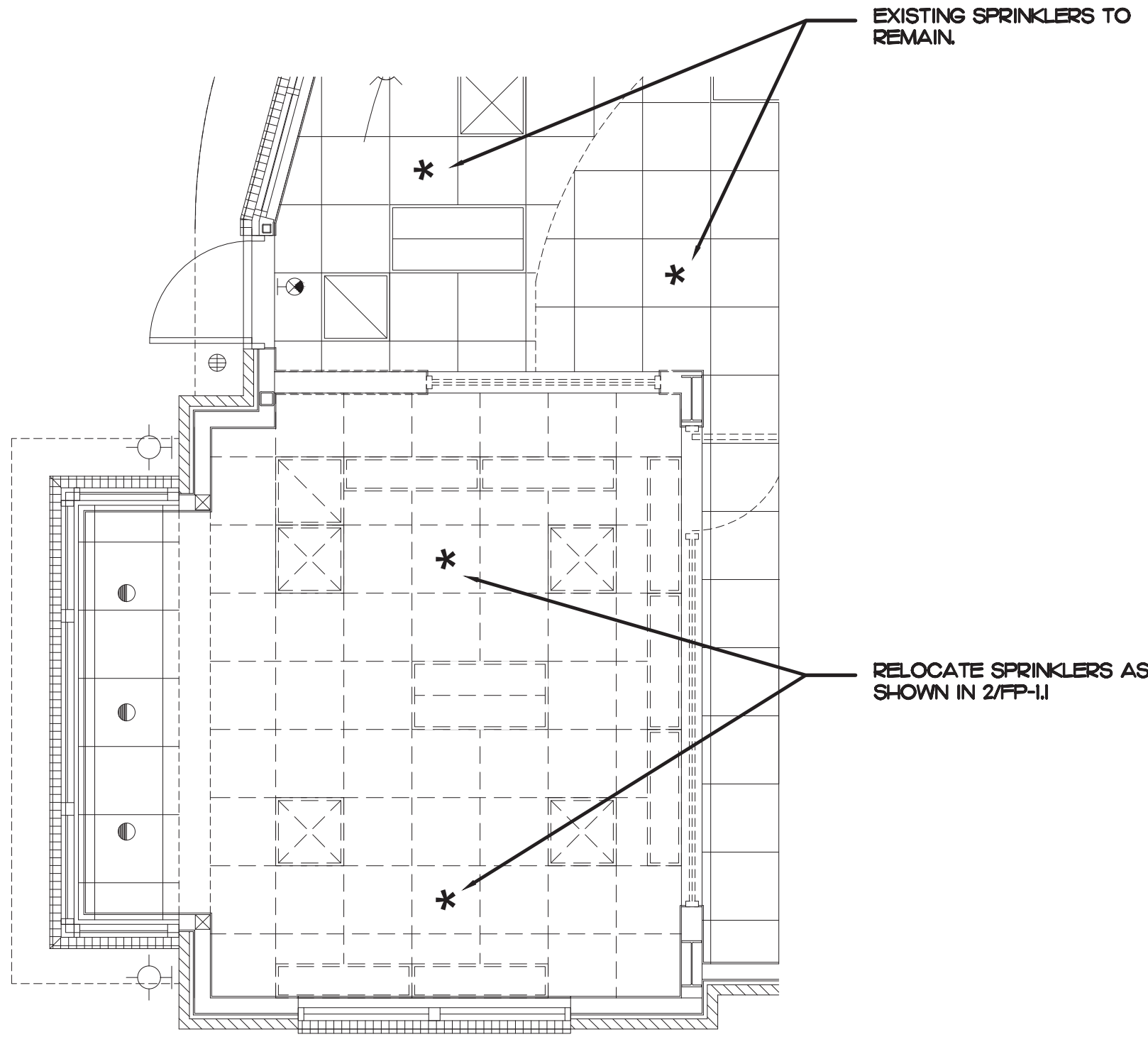
NOTE: AS NOTED IN THE SPECIFICATIONS, ALL WIRING LAYOUTS, LAYOUTS ARE SCHEMATIC. EXACT LOCATIONS SHALL BE DETERMINED BY THE CONSTRUCTION AND STRUCTURE OF THE BUILDING AND SHALL BE VERIFIED AND COORDINATED IN THE FIELD. EACH TRADE CONTRACTOR SHALL VERIFY WITH THE GENERAL CONTRACTOR THAT HE HAS THOROUGHLY REVIEWED AND COORDINATED ALL LOCATIONS AND ROUTINGS WITH ALL OTHER TRADES PRIOR TO FABRICATION OF CONDUITS, DUCTS, OR PIPING, AND START OF INSTALLATION OF SAME (INCLUDING SPRINKLER PIPING WHEN PRESENT ON JOB). ANY INSTALLATION OR CONSTRUCTION CONFLICTS WHICH OCCUR IN THE FIELD SHALL BE RESOLVED BY THE TRADE CONTRACTOR TO THE SATISFACTION OF THE OWNER AND ARCHITECT AND AT NO EXPENSE TO THE OWNER, ARCHITECT AND/OR GENERAL CONTRACTOR.

THE CONTRACTOR SHALL CONTACT THE ARCHITECT, ENGINEER OR OWNER PRIOR TO BIDDING FOR INTERPRETATIONS AND CLARIFICATIONS OF THE DESIGN AND INCLUDE IN HIS BID ALL COSTS TO MEET THE DESIGN INTENT. CLARIFICATIONS MADE BY THE ARCHITECT, ENGINEER OR OWNER AFTER BIDDING WILL BE FINAL AND SHALL BE IMPLEMENTED AT CONTRACTOR'S COST.

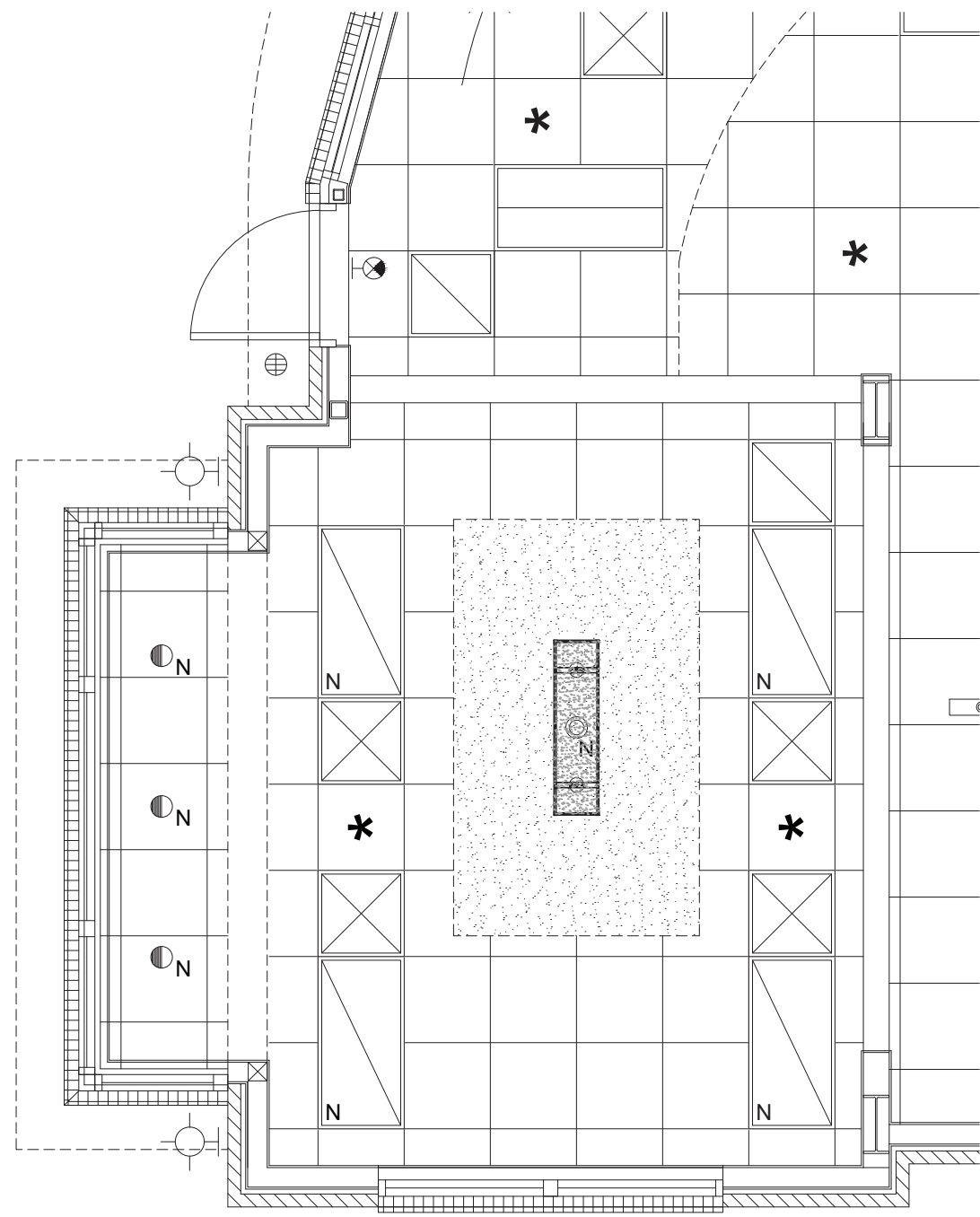
BIDDING CONTRACTORS SHALL HAVE A WORKING KNOWLEDGE OF LOCAL CODES AND ORDINANCES AND SHALL INCLUDE IN THEIR BIDS THE COSTS FOR ALL WORK INSTALLED IN STRICT ACCORDANCE WITH GOVERNING CODES, THE PLANS AND SPECIFICATIONS. NOT WITHSTANDING, THE CONTRACTOR SHALL ALERT ARCHITECT, ENGINEER OR OWNER OF ANY APPARENT DISCREPANCIES BETWEEN GOVERNING CODES AND DESIGN INTENT.

NATIONAL ACCOUNTS

- | | |
|----|--|
| 1. | PRICE AIR DEVICES - THE MECHANICAL CONTRACTOR IS REQUIRED TO PURCHASE THE AIR DEVICES DIRECTLY FROM TOM BARROW COMPANY. CONTACT MR. SCOTT GEORGE AT 404-351-HOIO FOR PRICING AND AVAILABILITY. AIR DEVICES NOT PURCHASED THRU TOM BARROW COMPANY WILL NOT BE ACCEPTED. |
| 2. | AIR DOORS - THE MECHANICAL CONTRACTOR IS REQUIRED TO PURCHASE THE AIR DOORS DIRECTLY FROM TOM BARROW COMPANY. CONTACT MR. SCOTT GEORGE AT 404-351-HOIO FOR PRICING AND AVAILABILITY. AIR DOORS NOT PURCHASED THRU TOM BARROW COMPANY WILL NOT BE ACCEPTED. |



1 DEMOLITION PLAN
1/4"= 1'-0"



2 FIRE PROTECTION PLAN
1/4"= 1'-0"

LEGEND	
*	PENDANT SPRINKLER
●	DRY SPRINKLER

SPRINKLER NOTES

THE WET SPRINKLER WET SPRINKLER SYSTEM SHALL BE INSTALLED IN ACCORDANCE WITH NFPA #13 (2013 EDITION) AND NFPA #14 (2013 EDITION) AND IFC (2012 EDITION) HOSE STREAM TO MEET OR EXCEED THE REQUIREMENTS OF NFPA #13 AND LOCAL AUTHORITY.

THESE DRAWINGS ARE PERFORMANCE SPECIFICATION ONLY AND ARE ONLY INTENDED TO CONVEY THE GENERAL SCOPE OF WORK. IT IS THE RESPONSIBILITY OF THE FIRE SPRINKLER CONTRACTOR TO SUBMIT SIGNED AND SEALED DRAWINGS AND HYDRAULIC CALCULATIONS TO THE AUTHORITY HAVING JURISDICTION AS DEFINED IN THE INTERNATIONAL BUILDING CODE AND NFPA #13, 144 WORKING PLANS.

THE CONTRACTOR SHALL PROVIDE ALL MATERIAL LABOR, TRUCKING, HOISTING, ENGINEERING, SCAFFOLDING, POWER HOOK UPS, PROTECTION, SHOP DRAWINGS, LAYOUT, EQUIPMENT, SUPERVISION, AND INSURANCE ETC. NECESSARY FOR THE FURNISHING AND INSTALLATION OF ALL SPECIFIED AND RELATED WORK IN ACCORDANCE WITH THE CONTRACT DRAWINGS AND SPECIFICATIONS.

THE CONTRACTOR SHALL GIVE ALL NECESSARY NOTICES, APPLY FOR ALL PERMITS AND PAY ALL GOVERNMENTAL TAXES, FEES AND SEAL ALL NECESSARY PLANS ALL DOCUMENTS AND OBTAIN ALL NECESSARY APPROVALS FORM ALL DEPARTMENTS HAVING JURISDICTION. OBTAIN ALL REQUIRED CERTIFICATIONS OF INSPECTION FOR HIGHER WORK AND DELIVER SAME TO THE OWNER BEFORE REQUEST FOR ACCEPTANCE AND FINAL PAYMENT FOR THE WORK.

THE CONTRACTOR SHALL COORDINATE WORK WITH ALL OTHER TRADES. CONTRACTOR TO COORDINATE FIRE PROTECTION EQUIPMENT LOCATIONS TO AVOID LIGHTS AND AIR DIFFUSERS.

PIPE MATERIAL AND HANGERS AND INSTALLATION METHODS SHALL BE IN ACCORDANCE WITH LATEST EDITION OF NFPA #13.

IT IS THE FIRE SPRINKLER CONTRACTOR'S RESPONSIBILITY TO PROVIDE ADEQUATE SPRINKLER COVERAGE IN AREAS OUTSIDE SCOPE OF WORK THAT BECOME UNPROTECTED BY EXISTING SPRINKLERS DUE TO NEW CONSTRUCTION.

PROVIDE UPRIGHT SPRINKLER HEADS IN THE NON-CONDITIONED SPACE ABOVE CEILING.

GIGIS CRITERIA

32.00.4(2)(A)
THE POINT OF SERVICE IS INDICATED AT THE BACKFLOW PREVENTER.
AT THIS POINT, THE SYSTEM IS DEDICATED SOLELY FOR FIRE PROTECTION PURPOSES. NO DOMESTIC WATER SHALL BE TAKEN FROM THE SYSTEM BEYOND THIS POINT FOR OTHER PURPOSES.

32.00.4(2)(B)
THE FOLLOWING ARE APPLICABLE STANDARDS:
NFPA 13, 2013, EDITION, INSTALLATION OF SPRINKLER SYSTEMS.
NFPA 24, 2010, EDITION, STANDARD FOR THE INSTALLATION OF PRIVATE OF FIRE SERVICE MAINS AND FIRE APPLURTENANCES.
NFPA 25, 2011 EDITION, INSPECTION, TESTING AND MAINTENANCE OF WATER BASED FIRE PROTECTION SYSTEMS.

32.00.4(2)(C)
CLASSIFICATIONS OF HAZARD OCCUPANCIES FOR THE AREAS OF DESIGN SHALL BE AS FOLLOWS:
(CORRIDORS, RESTROOMS, OFFICES AND OTHER SIMILAR OCCUPANCIES) LIGHT HAZARD OCCUPANCY PER NFPA# 13, 2013 EDITION.
(RESTAURANT PREPARATION AREAS) ORDINARY HAZARD GROUP 1 PER NFPA# 13, 2013 EDITION.
(SALES FLOOR, MECHANICAL ELECTRICAL AND STORAGE ROOMS) ORDINARY HAZARD GROUP 2 PER NFPA #13, 2013 EDITION.

32.00.4(2)(D)
THE SPRINKLER SYSTEM SHALL BE WET PIPE FOR LIGHT HAZARD OCCUPANCY AND SHALL BE HYDRAULICALLY CALCULATED PER NFPA #13, 2013 EDITION AS A DENSITY OF 0.10 GPM/SQFT. OVER THE MOST REMOTE 1500 SQFT. SPRINKLER SHALL HAVE A TEMPERATURE RATING OF 155 F° AND THE SPACING SHALL BE 225 SQFT. INCLUDE A FIRE HOSE ALLOWANCE OF 100 GPM.

THE SPRINKLER SYSTEM SHALL BE WET PIPE FOR ORDINARY HAZARD GROUP 1 OCCUPANCY AND SHALL BE HYDRAULICALLY CALCULATED PER NFPA #13, 2013 EDITION AS A DENSITY OF 0.15 GPM/SQFT. OVER THE MOST REMOTE 1500 SQFT. SPRINKLER SHALL HAVE A TEMPERATURE RATING OF 155 F° AND THE SPACING SHALL BE 150 SQFT. INCLUDE A FIRE HOSE ALLOWANCE OF 100 GPM.

THE SPRINKLER SYSTEM SHALL BE WET PIPE FOR ORDINARY HAZARD GROUP 2 OCCUPANCY AND SHALL BE HYDRAULICALLY CALCULATED PER NFPA #13, 2013 EDITION AS A DENSITY OF 0.2 GPM/SQFT. OVER THE MOST REMOTE 1500 SQFT. SPRINKLER SHALL HAVE A TEMPERATURE RATING OF 155 F° AND THE SPACING SHALL BE 150 SQFT. INCLUDE A FIRE HOSE ALLOWANCE OF 250 GPM.

32.00.4(2)(E)
WATER SUPPLY: THIS BUILDING WILL BE SUPPLIED BY A NEW 6" UNDERGROUND FIRE MAIN WHICH CONNECTS TO TO AN EXISTING 6" WATER MAIN LOCATED IN THE BACK OF THE BUILDING.

32.00.4(2)(F)
SEE ATTACHED MOST RECENT FLOW TEST DATA, PROVIDED BY SPRINKLER CONTRACTOR.

32.00.4(2)(G)
VALVE AND ALARM REQUIREMENTS: ALL CONTROL VALVES ON THE SPRINKLER RISER AND BACKFLOW PREVENTER SHALL HAVE TAMPER SWITCHES.
FLOW SWITCHES SHALL BE SET NOT TO ALARM WITH MINOR CITY WATER PRESSURE FLUCTUATIONS. HOWEVER THE FLOW OF ONE SPRINKLER HEAD SHALL PRODUCE AN ALARM CONDITION BY TRIPPING THE FLOW SWITCH.
ALL FLOW AND TAMPER SWITCHES SHALL ALSO SOUND THE ELECTRIC ALARM BELL ON THE OUTSIDE WALL. FIRE SPRINKLER CONTRACTOR SHALL VERIFY AND TEST PER NFPA#3.

32.00.4(2)(H)
THE LOCAL WATER PURVEYOR IS REQUESTED TO ADVISE THE ENGINEER OF RECORD IF CONDITIONS EXIST IN THEIR WATER SUPPLY THAT COULD LEAD TO MIC. SO THAT THE ENGINEER CAN DESIGN CORRECTIVE MEASURES.

32.00.4(2)(I)
BACKFLOW PREVENTER AND METERING SPECIFICATIONS SHALL MEET OR EXCEED REQUIREMENTS OF LOCAL JURISDICTION.

32.00.4(2)(J)
YARD AND INTERIOR FIRE PROTECTION COMPONENTS:
PRODUCT DATA SHEETS SHALL BE SUBMITTED BY THE SPRINKLER CONTRACTOR ALONG WITH THEIR SHOP DRAWINGS.
ALL FIRE PROTECTION DEVICES AND COMPONENTS SHALL BE UL LISTED AND FM APPROVED.



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04	04/17/24	RF#5

CONSULTANT PROJECT # 2023.0467

DATE AUGUST 2023

DRAWN BY AAE

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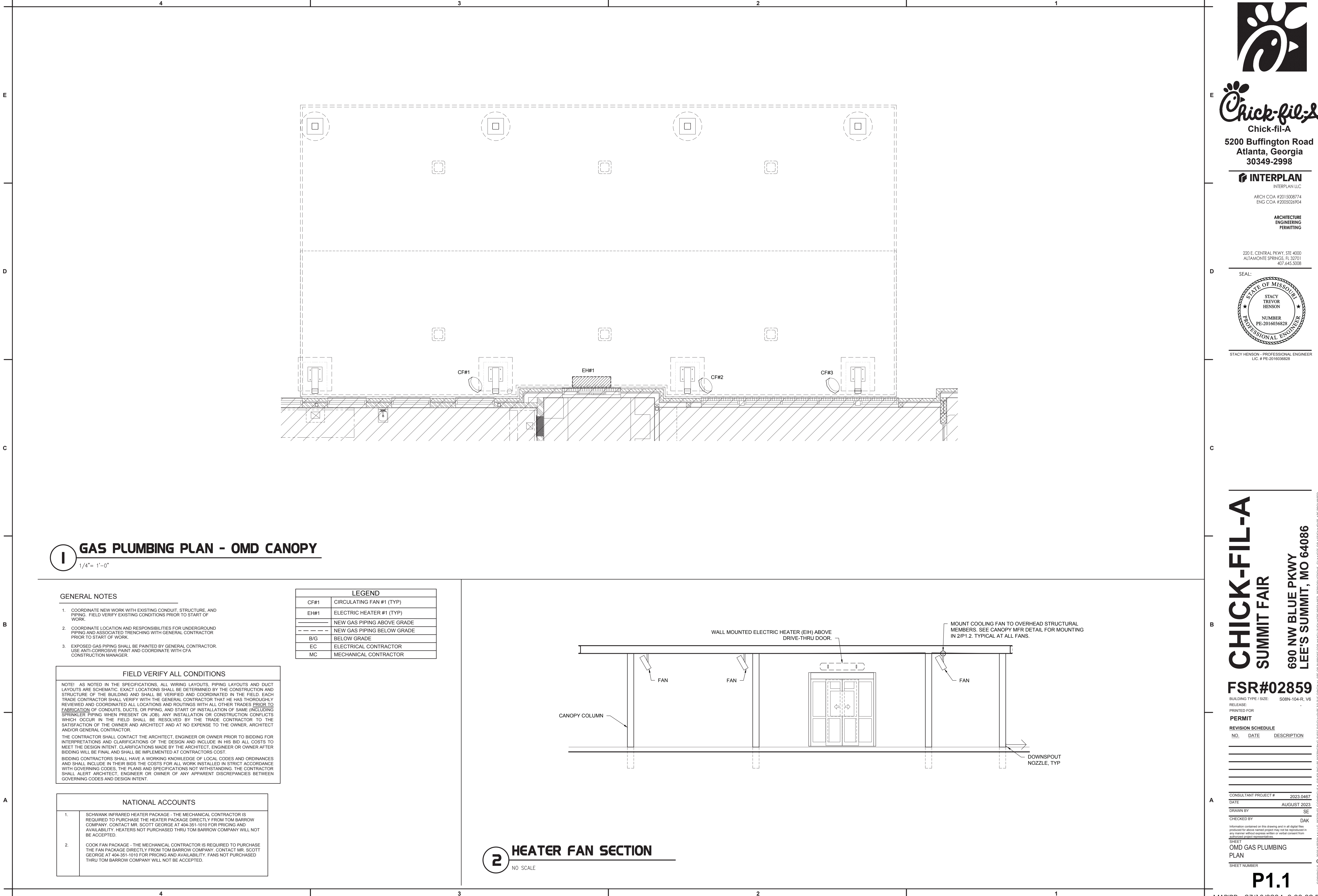
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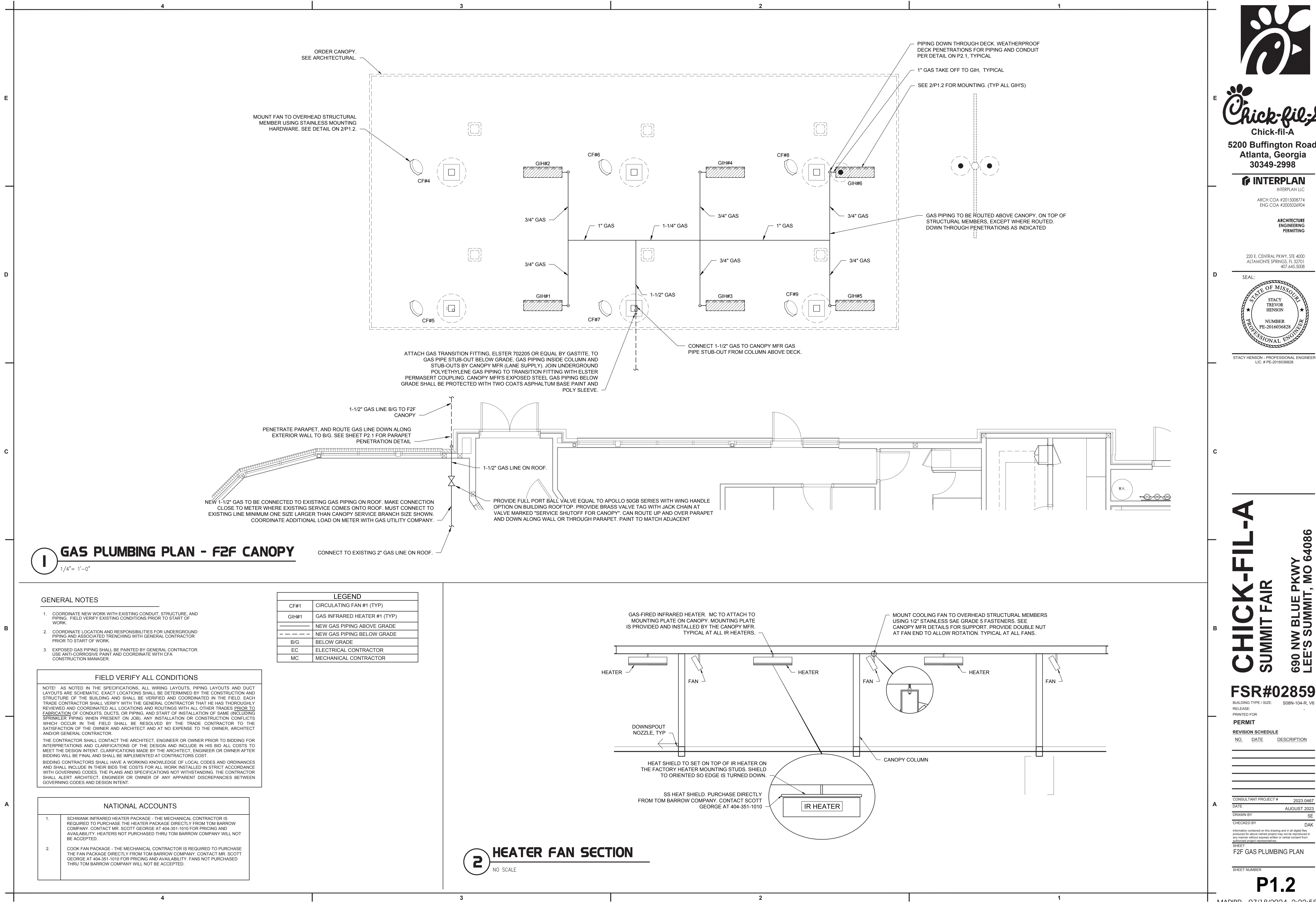
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OMD GAS PLUMBING
PLAN
SHEET NUMBER

P1.1

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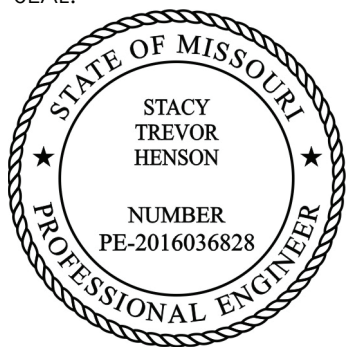
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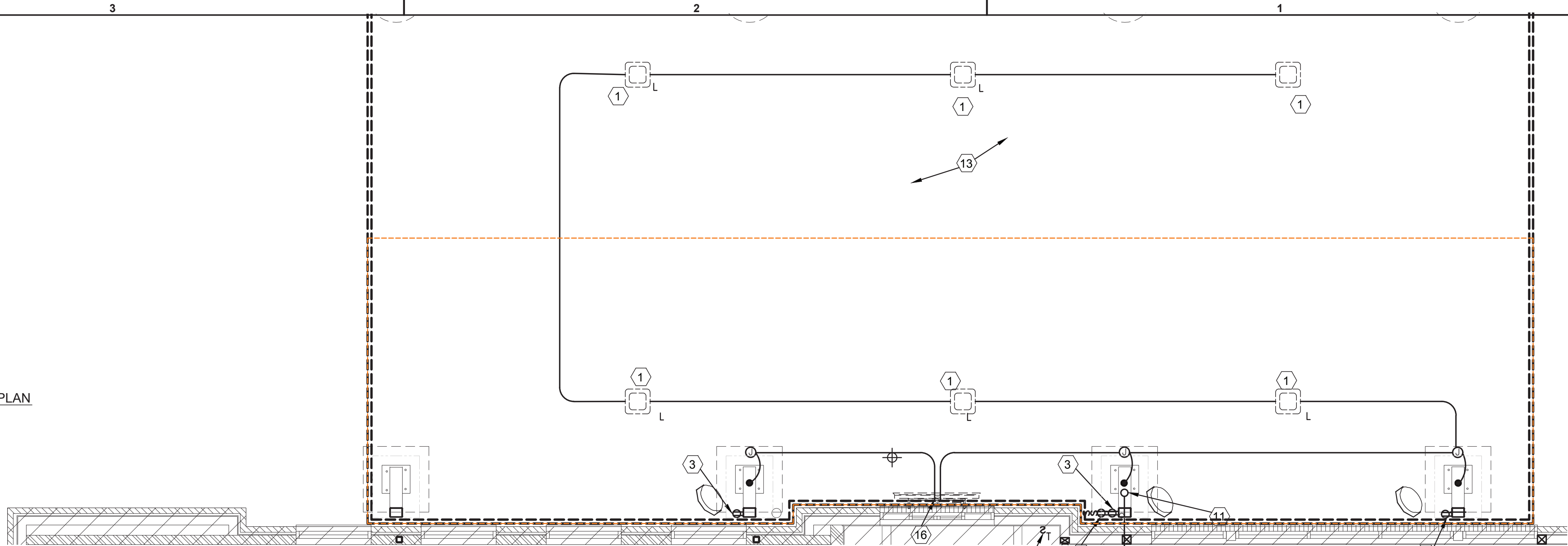
SHEET
F2F GAS PLUMBING PLAN

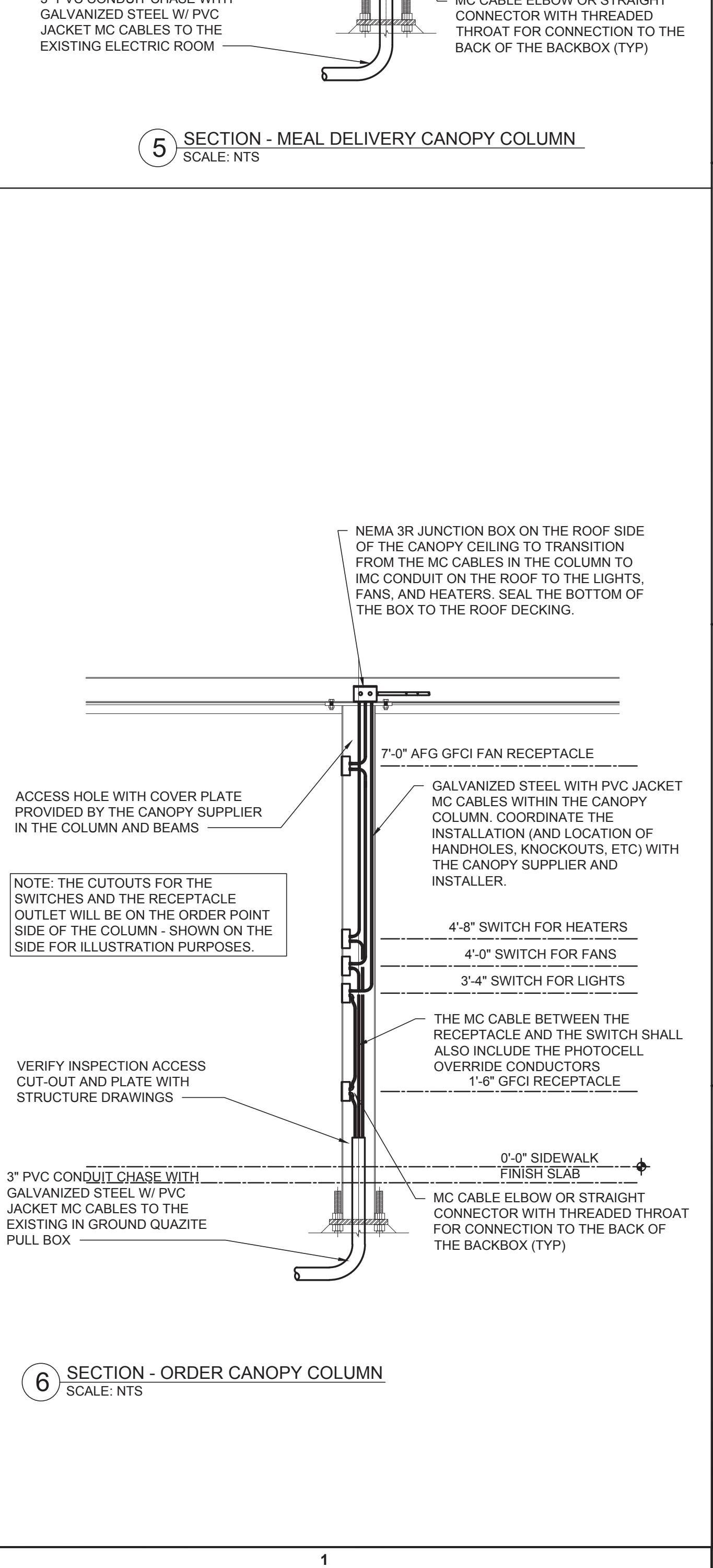
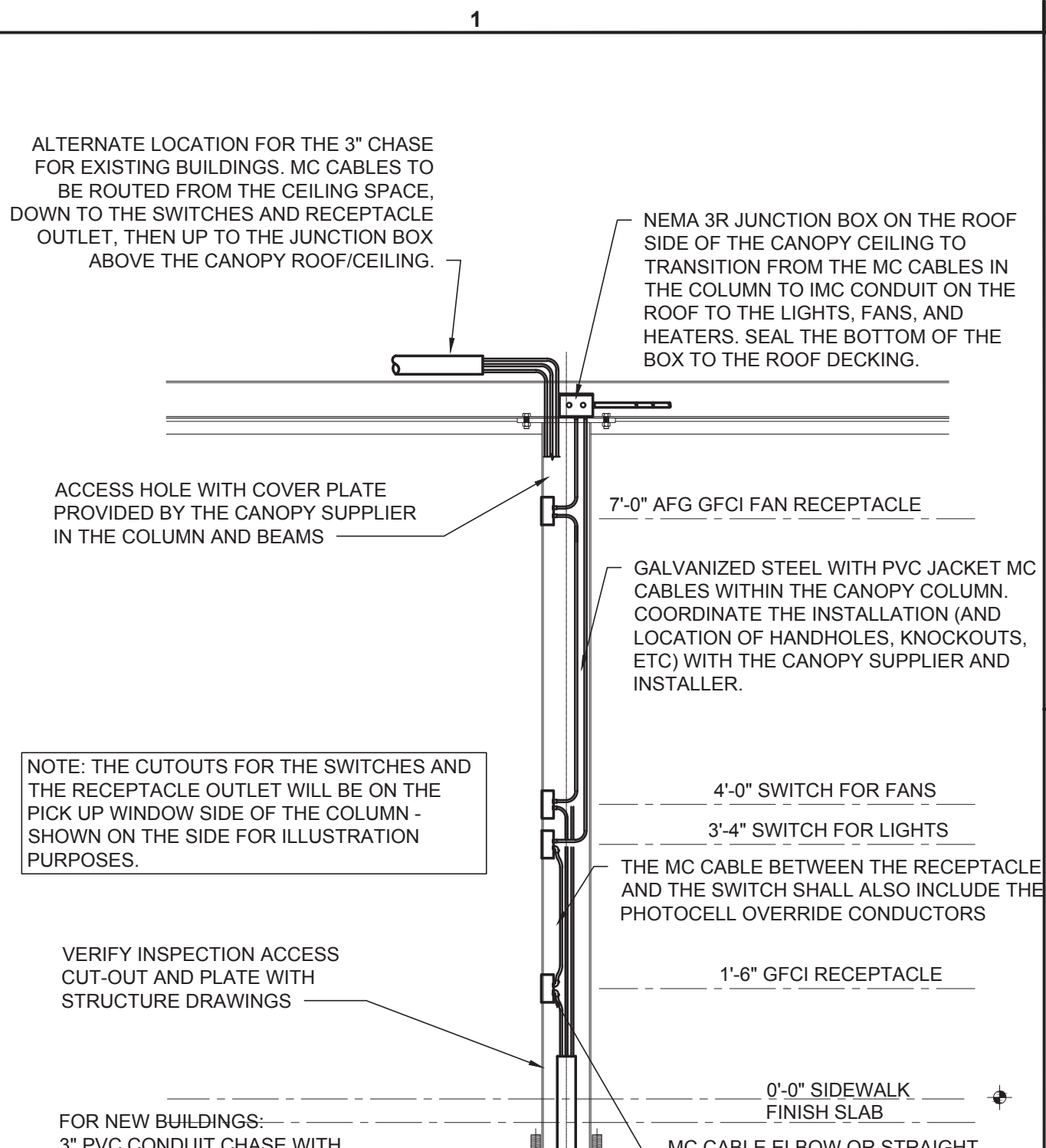
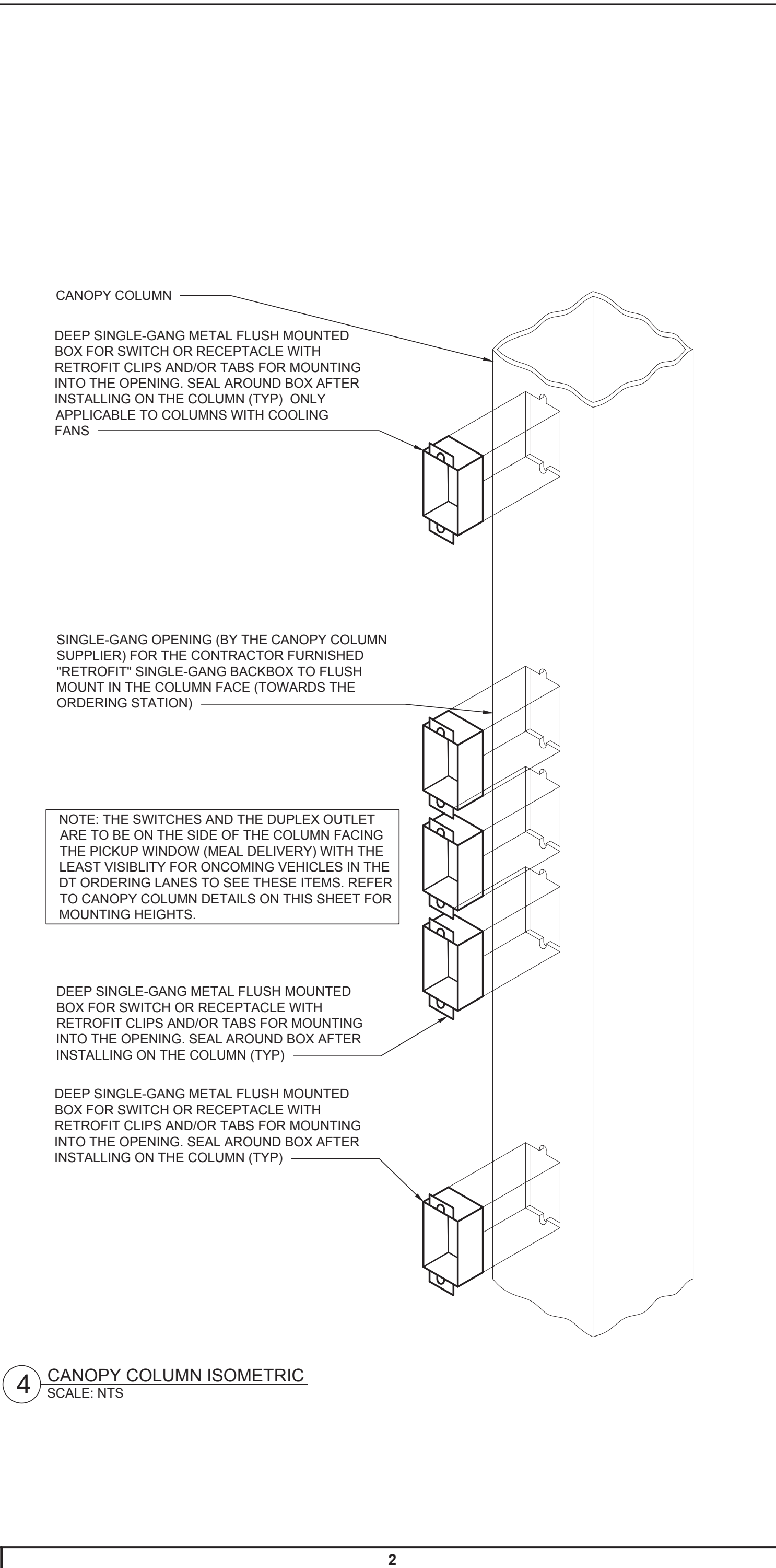
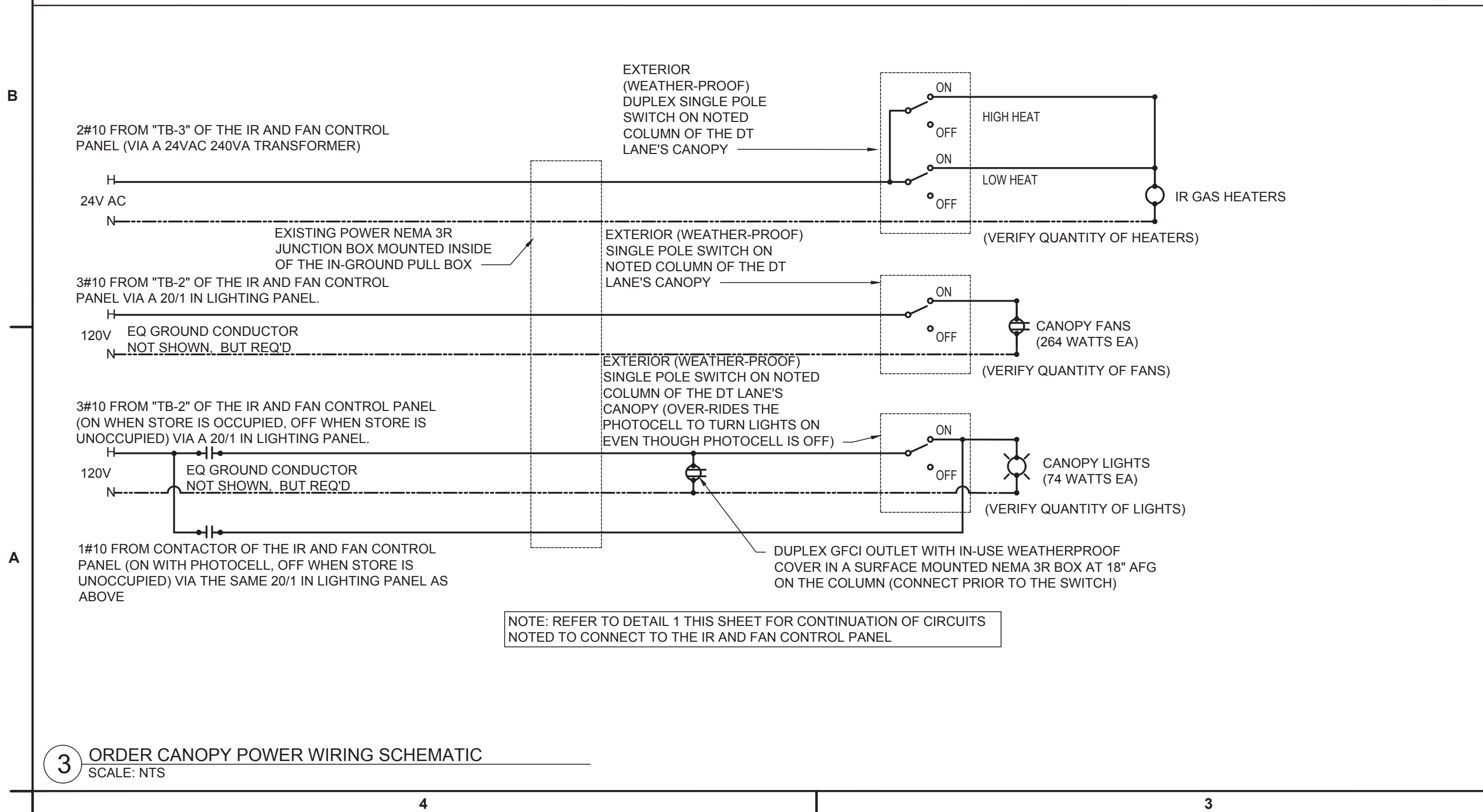
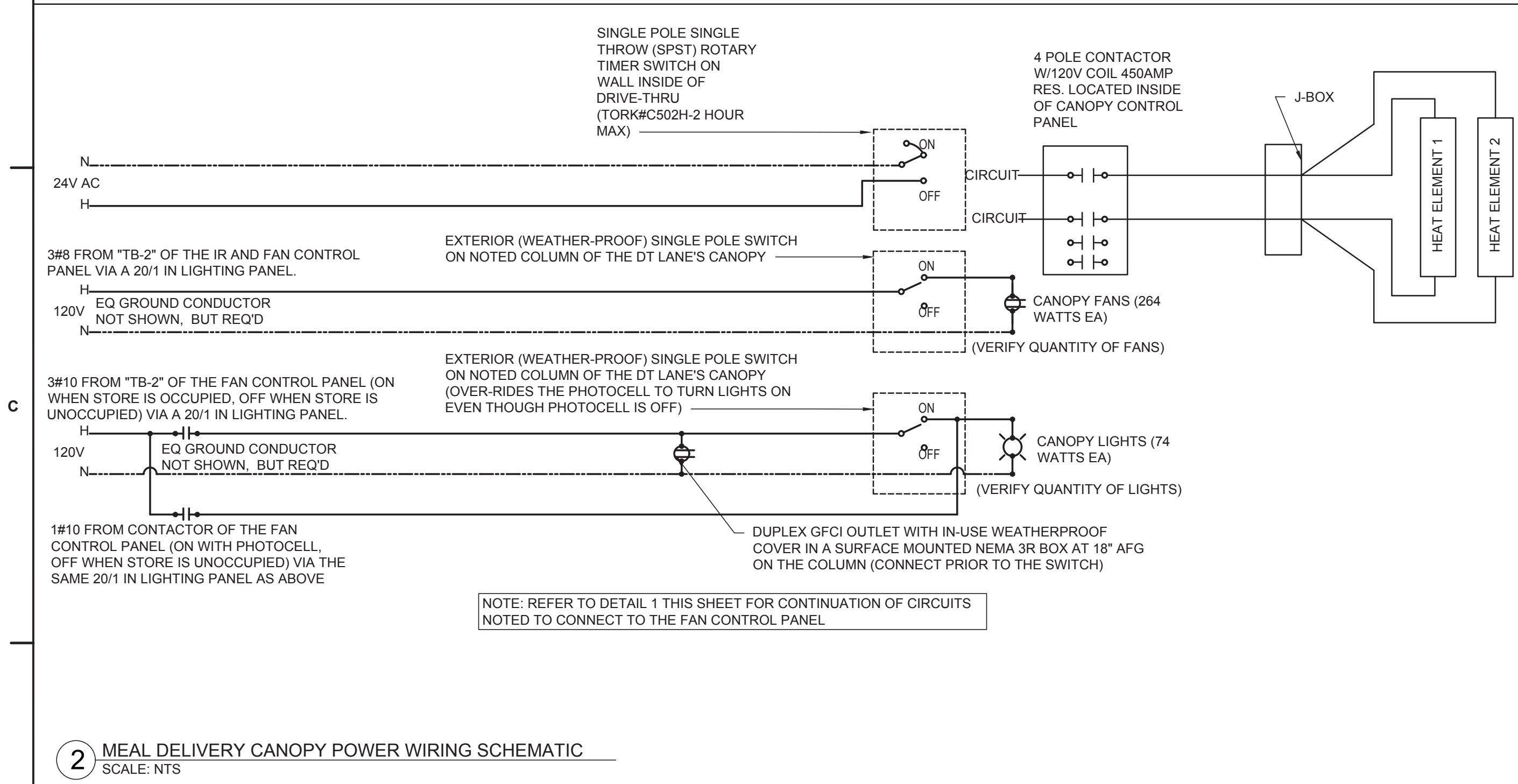
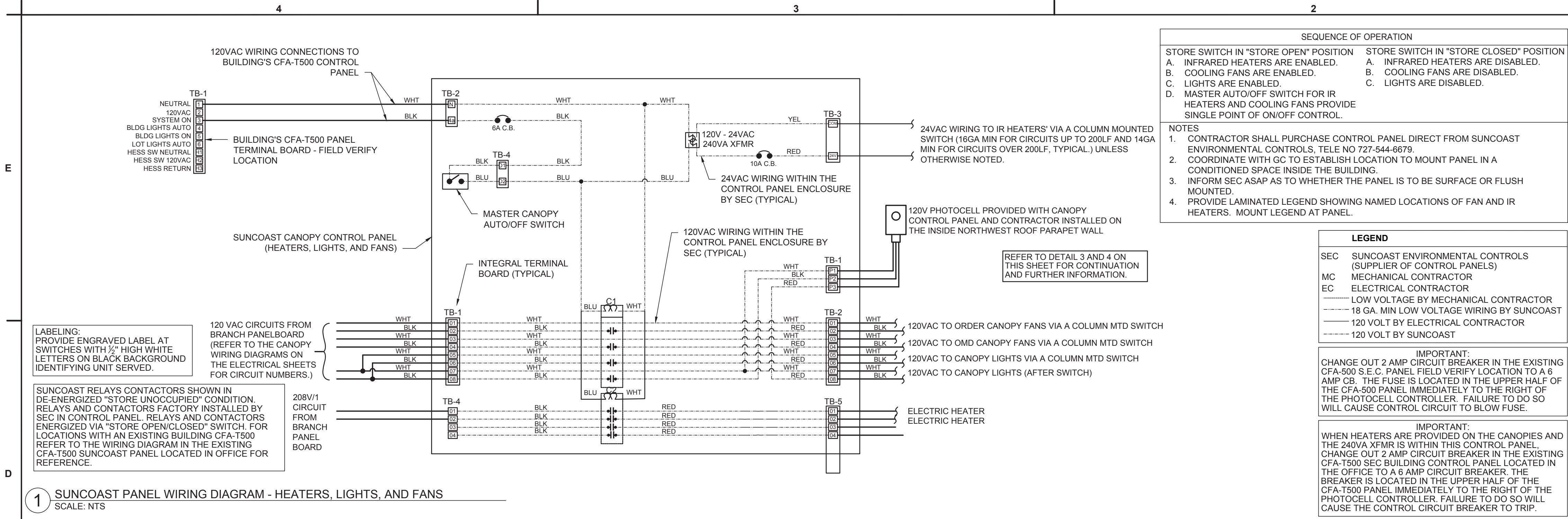
SHEET NUMBER

P1.2

ELECTRIC INFRARED HEATER SCHEDULE - CHICK-FIL-A						
MARK	MANUFACTURER	CATALOG NUMBER	VOLTAGE/PH	WATTS	AMPS	REMARKS
H	BROMIC	BH042003	208V/1PH	6000	25	
E						

3 MEAL DELIVERY CANOPY POWER PLAN
SCALE: 1/4" = 1'-0"





Chick-fil-A

Chick-fil-A

5200 Buffington Road
Atlanta, Georgia
30349-2998

INTERPLAN
INTERPLAN LLC
ARCH COA #2015008774
ENG COA #2003026904

ARCHITECTURE
ENGINEERING
PERMITTING

220 E. CENTRAL PKWY, STE 4000
ALTAMONTE SPRINGS, FL 32701
407.645.5008

SEAL:

STATE OF MISSOURI
STACY TREVOR HENSON
NUMBER
PE-2016036828
PROFESSIONAL ENGINEER

STACY HENSON - PROFESSIONAL ENGINEER
LIC. # PE-2016036828

CHICK-FIL-A

SUMMIT FAIR

690 NW BLUE PKWY
LEE'S SUMMIT, MO 64086

FSR#02859

BUILDING TYPE / SIZE: S08N-104-R, V8
RELEASE:
PRINTED FOR
PERMIT

REVISION SCHEDULE

NO.	DATE	DESCRIPTION

CONSULTANT PROJECT # 2023.0467
DATE AUGUST 2023
DRAWN BY RZ
CHECKED BY MI

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SHEET

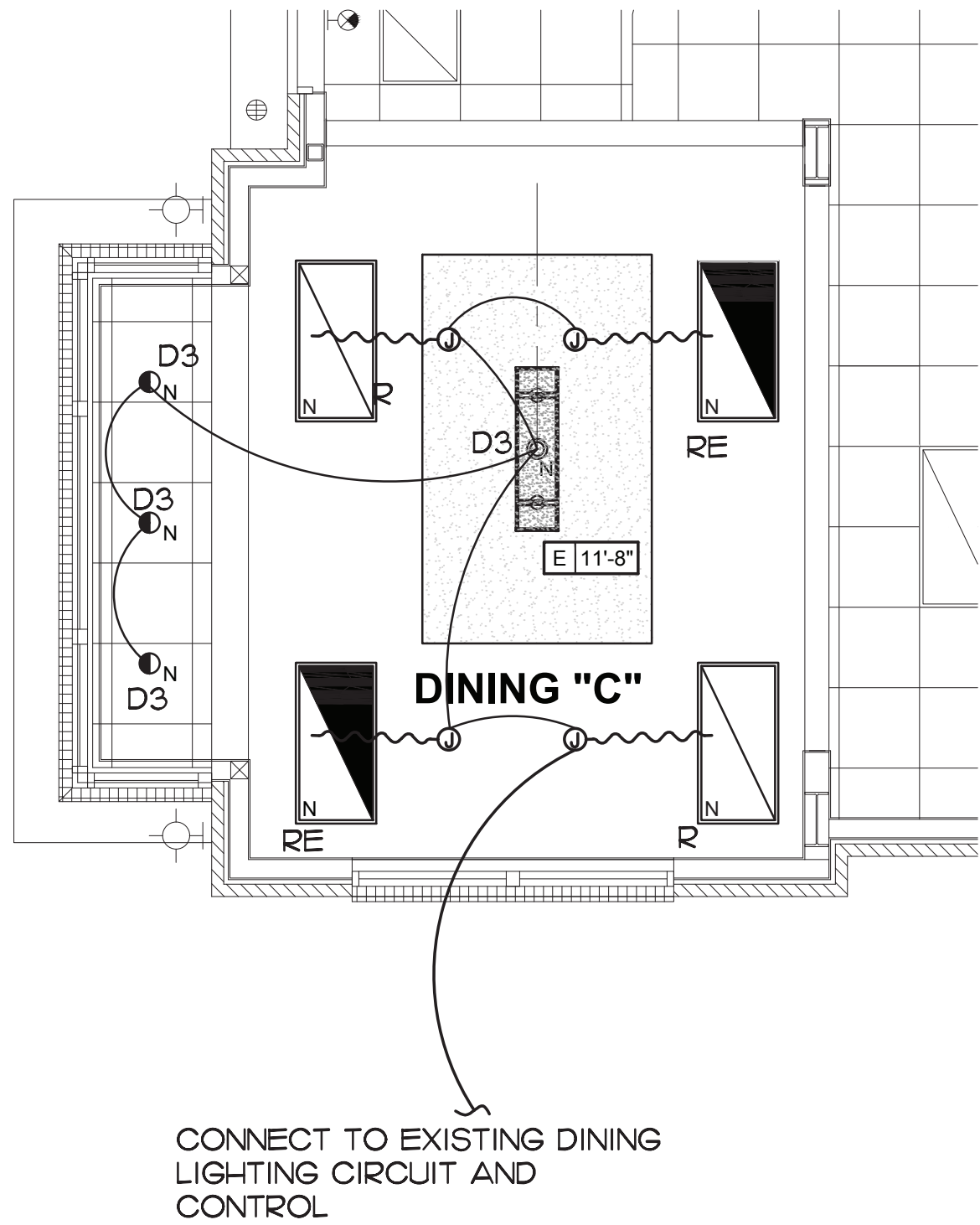
CANOPY ELEC DETAILS

SHEET NUMBER

E1.2

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LIGHTING FIXTURE (LUMINAIRE) SCHEDULE - CHICK-FIL-A							
MARK	MANUFACTURER	CATALOG NUMBER	NO. LAMPS/TYPE	SYL LAMP NO.	WATTS	VOLTS	REMARKS
A	LITHONIA	2GTL-472L-A12125V-EZ1-LP840	INTEGRAL WITH FIXTURE	-	52.3	120	2'X4' STATIC LED TROFFER RATED 7200 LUMENS, 4000K COLOR TEMP
AE	LITHONIA	2GTL-472L-A12125V-EZ1-LP840-EL14L	INTEGRAL WITH FIXTURE	-	52.3	120	SAME AS 'A' WITH EMERGENCY BATTERY PACK. SEE PLAN NOTES ABOUT LAMP SWITCHING
R	LITHONIA	2RTL4-48L-EZ1-LP830	LED	-	48	120	2'X4' VOLUMETRIC RECESSED LIGHTING
RE	LITHONIA	2RTL4-48L-EZ1-EL14L-LP830	LED	-	48	120	SAME AS R WITH EMERGENCY BATTERY PACK
D3	LITHONIA	LDN6-30/20-LO6AR-LSS-MVOLT	FURNISHED	-	22.6	120	
NOTES: 1. LUMINAIRES UTILIZING DOUBLE-ENDED LAMPS AND CONTAIN BALLASTS THAT CAN BE SERVICED IN PLACE SHALL HAVE A DISCONNECTING MEANS EITHER INTERNAL OF EXTERNAL TO EACH LUMINAIRE PER NEC 410.130(G). 2. THE LIGHTING FIXTURE PACKAGE IS AVAILABLE THROUGH A NATIONAL ACCOUNT PROGRAM. REFER TO SHEET E4.2, SECTION 16500 FOR VENDOR INFORMATION. 3. THE FLUORESCENT BALLAST CATALOG NUMBER INDICATES OSRAM AND ALL LAMP DESIGNATIONS ARE FOR OSRAM/SYLVANIA PER A NATIONAL ACCOUNT AGREEMENT. 4. THE ASTERIK (*) BESIDE THE FIXTURE MARK IN THE ABOVE SCHEDULE INDICATES THE FIXTURE IS A NON-PROTOTYPICAL LIGHT FIXTURE PER THE CFA NATIONAL HERITAGE PROTOTYPE.							

SECTION C16124
SUPPORTING DEVICES AND HANGERS

PART 1 - PRODUCTS

- 1.O1 ACCEPTABLE MANUFACTURERS
A. SUPPORTING DEVICES AND HANGERS SHALL BE MANUFACTURED BY RACO FASTENERS, OR APPROVED EQUIVALENT.

PART 2 - EXECUTION

- 2.O1 INSTALLATION
A. SECURE CONDUITS TO WITHIN 3' OF EACH OUTLET BOX, JUNCTION BOX, CABINET, FITTING, ETC., AND AT INTERVALS NOT TO EXCEED TEN FEET (10') AND IN ACCORDANCE WITH THE NATIONAL ELECTRIC CODE. IN SEISMIC ZONES, SUPPORT CONDUITS 1' AND UNDER AT 6' INTERVALS.

B. INSTALL CLAMPS SECURED TO STRUCTURE FOR FEEDER AND OTHER CONDUITS. ROUTED AGAINST THE STRUCTURE. USE DROP RODS AND HANGERS OR RACKS TO SUPPORT CONDUITS RUN APART FROM THE STRUCTURE.

C. PROVIDE AND INSTALL SUITABLE ANGLE IRON, CHANNEL IRON OR STEEL METAL FRAMING WITH ACCESSORIES TO SUPPORT OR BRACE ELECTRICAL EQUIPMENT INCLUDING SAFETY SWITCHES, FIXTURES, PANELBOARDS, ETC.

D. USE OF CHAINS, PERFORATED IRON, BALING WIRE, OR TIE WIRE FOR SUPPORTING CONDUIT RUNS IS NOT PERMITTED.

E. FOR SUPPORT OF LOW VOLTAGE WIRING NOT REQUIRED TO BE IN CONDUIT, BUNDLE CABLES TOGETHER IN A NEAT MANNER USING APPROVED NYLON TIE WRAPS. BUNDLED CABLES SHALL BE SUPPORTED WITH "J" HOOKS ON TELEPHONE TYPE BRIDLE RINGS, A MINIMUM OF 6 FEET ON CENTERS. CLEARLY IDENTIFY ALL DIFFERING TYPES OF CABLES BEING RUN AND TAG WITH TAPE TAGS REGARDING TELEPHONE, POS SYSTEM, MUSIC/COMMUNICATION, SECURITY, ETC. FOR VARIOUS SYSTEM UTILIZING SAID CABLE. IDENTIFICATION TAPE SHALL BE PROVIDED AT MINIMUM INTERVALS OF 25 FEET ON CENTER AND WITHIN EACH BUILDING SPACE.

F. PROVIDE A SYSTEM OF SUPPORTING DEVICES AND HANGERS TO INSURE SECURE SUPPORT OR BRACING FOR CONDUIT, ELECTRICAL EQUIPMENT, INCLUDING SAFETY SWITCHES, FIXTURES, PANELBOARDS, OUTLET BOXES, JUNCTION BOXES, CABINETS, ETC.

LIGHTING FIXTURE (LUMINAIRE) SCHEDULE - CHICK-FIL-A

MARK	MANUFACTURER	CATALOG NUMBER	NO. LAMPS/TYPE	SYL LAMP NO.	WATTS	VOLTS	MOUNTING	REMARKS
A	LITHONIA	2GTL-472L-A12125V-EZ1-LP840	INTEGRAL WITH FIXTURE	-	52.3	120	RECESSED	2'X4' STATIC LED TROFFER RATED 7200 LUMENS, 4000K COLOR TEMP
AE	LITHONIA	2GTL-472L-A12125V-EZ1-LP840-EL14L	INTEGRAL WITH FIXTURE	-	52.3	120	RECESSED	SAME AS 'A' WITH EMERGENCY BATTERY PACK. SEE PLAN NOTES ABOUT LAMP SWITCHING
R	LITHONIA	2RTL4-48L-EZ1-LP830	LED	-	48	120	RECESSED	2'X4' VOLUMETRIC RECESSED LIGHTING
RE	LITHONIA	2RTL4-48L-EZ1-EL14L-LP830	LED	-	48	120	RECESSED	SAME AS R WITH EMERGENCY BATTERY PACK
D3	LITHONIA	LDN6-30/20-LO6AR-LSS-MVOLT	FURNISHED	-	22.6	120	RECESSED	

SECTION C16500
LIGHTING FIXTURES (LUMINAIRES)

PART 1 - GENERAL

1.O1 ACCEPTABLE MANUFACTURERS AND VENDORS

- A. LIGHTING FIXTURES INDICATED ON LIGHTING FIXTURE SCHEDULE ARE TO BE PURCHASED FROM THE **NATIONAL ACCOUNT VENDOR** FOR THE REGION OF THE PROJECT (VERIFY REGION DESIGNATION WITH OWNER'S REPRESENTATIVE):

1. ACCU-SERV LIGHTING - NORTH REGION AND SOUTHEAST REGION. CONTACT AT ACCU-SERV: BOB HARPRING AT 877-707-7378, FAX - 502-961-0357, EMAIL - BHARPRING@ACCU-SERV.COM

2. VILLA LIGHTING - CENTRAL REGION, SOUTHWEST REGION, AND WEST REGION. CONTACT AT VILLA LIGHTING: DAVE CHRISTIANELL AT 800-325-0963, FAX- 314-531-8720, EMAIL - DAVEC@VILLALIGHTING.COM

B. BALLASTS TO BE ELECTRONIC BALLAST PROVIDED WITH LIGHTING FIXTURE BY THE MANUFACTURER.

C. LAMPS TO BE OSRAM-SYLVANIA AND WILL TYPICALLY BE PROVIDED WITH THE LUMINAIRE BY THE LIGHTING MANUFACTURER.

1.O2 FIXTURE REQUIREMENTS
A. PROVIDE REGULATING, HPF BALLASTS IN ALL HID LIGHTING FIXTURES. HID LAMP TYPES SHALL BE AS INDICATED ON THE DRAWINGS.

B. RECESSED FLUORESCENT LIGHTING FIXTURE BALLASTS SHALL BE PROVIDED WITH INTEGRAL THERMAL PROTECTION.

C. PROVIDE ENERGY-SAVING INSTANT OR RAPID START LAMPS FOR ALL FLUORESCENT FIXTURES.

D. ALL LAMPS AND BALLASTS SHALL MEET OR EXCEED THE REQUIREMENTS OF THE NATIONAL ENERGY POLICY ACT OF 1992 AND ANY OTHER APPLICABLE CODES OR CRITERIA.

E. ALL COMPONENTS OF RECESSED FIXTURES SHALL BE ACCESSIBLE WITHOUT DISTURBING FIXTURE IN OR ON CEILING.

F. ENERGY SAVING BALLASTS AND ENERGY SAVING LAMPS PROVIDED SHALL BE COMPATABLE FOR OPERATION TOGETHER.

G. EXTERIOR FIXTURES AND POLES SHALL BE SUITABLE FOR EXTERIOR USE, SHALL BE UL LISTED, AND SHALL BE A STANDARD DESIGN FOR EXTERIOR APPLICATION.

H. EXTERIOR POLES FOR FIXTURES WITH LUMINAIRES INSTALLED SHALL BE DESIGNED FOR MAXIMUM CONSTANT VELOCITY WIND LOAD WITH LUMINAIRES INSTALLED, APPLICABLE TO THE GEOGRAPHIC AREA.

1.O3 CONTROLS

- A. LIGHTING CONTACTORS SHALL BE SQUARE-D, GENERAL ELECTRIC, CUTLER-HAMMER OR SIEMENS OF TYPES AND QUANTITY SHOWN ON DRAWINGS, EXCEPT THOSE FURNISHED WITH THE SWITCHGEAR AS PART OF THE **NATIONAL ACCOUNT PROGRAM** BY SUNCOAST ENVIRONMENTAL CONTROLS (SEC).

1.O4 EMERGENCY LIGHTING UNITS

- A. BATTERIES SHALL SUPPLY EMERGENCY POWER FOR LIGHTING WITH MINIMUM OPERATING TIME OF 1-1/2 HOURS.

B. EMERGENCY LIGHTING SHALL BE AUTOMATICALLY OPERATIONAL UPON NORMAL UTILITY POWER FAILURE.

PART 3 - EXECUTION

3.O1 INSTALLATION

- A. LIGHTING FIXTURES SHALL BE STRUCTURALLY SUPPORTED. FLUORESCENT FIXTURES MOUNTED IN SUSPENDED CEILINGS SHALL BE SUPPORTED BY AND ATTACHED TO CEILING SYSTEM AS REQUIRED BY NEC ARTICLE 410. IN ADDITION, FLUORESCENT TROFFERS SHALL BE SUPPORTED AT TWO OPPOSITE CORNERS TO BUILDING STRUCTURE.

B. RECESSED FIXTURES IN DROPPED CEILING AREAS SHALL BE CONNECTED TO POWER SOURCE USING FLEXIBLE CONDUIT. FLEXIBLE CONDUIT SHALL CONTAIN A SEPARATE INSULATED GREEN NO. 12 COPPER GROUND WIRE. FLEXIBLE CONDUIT SHALL BE CONNECTED TO JUNCTION BOX AND FIXTURE. GREEN GROUND WIRE SHALL PROVIDE GROUND CONTINUITY BETWEEN CONDUIT SYSTEM AND FIXTURE. GROUNDING CONDUCTORS SHALL BE PERMANENTLY AND MECHANICALLY CONNECTED BETWEEN FIXTURE AND CONDUIT SYSTEM SO AS TO BE ELECTRICALLY CONTINUOUS.

C. FIXTURES SURFACE MOUNTED ON EXPOSED TEE BAR CEILINGS SHALL USE GRIP CLAMPS ON TEE BARS TO SUPPORT FIXTURES.

D. WIRE SHALL BE CONTINUOUS FROM SPLICE IN OUTLET BOX OF BUILDING WIRING SYSTEM TO LAMP SOCKET OR BALLAST TERMINALS.

E. MAINTAIN THE INTEGRITY OF ENCLOSURES ON ENCLOSED AND GASKETED FIXTURES. MINIMIZE THE NUMBER OF ENCLOSURE PENETRATIONS AND MAKE SUCH PENETRATIONS WATER AND DUST TIGHT WITH APPROPRIATE GASKETS AND FITTINGS.

F. CONCRETE BASES SHALL BE PROVIDED FOR ALL EXTERIOR GROUND MOUNTED OR POLE MOUNTED FIXTURES.

G. INSTALL ACCESSORIES FURNISHED WITH EACH FIXTURE.

H. WIRING FROM POLE BASES TO POLE MOUNTED LUMINAIRE SHALL BE NO. 12 WITH FUSE PROTECTION PROVIDED BY A 30 AMP, 600 VOLT WATERPROOF FUSEHOLDER WITH BUSSMAN 'LIMITRON' FUSE OF AMPERE RATING 3 TIMES THE LOAD CURRENT.



Chick-fil-A
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5200 Buffington Road
Atlanta, Georgia
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ENG COA #2003026904

ARCHITECTURE
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220 E. CENTRAL PKWY, STE 4000
ALTAMONTE SPRINGS, FL 32701
407.645.5008

SEAL:



STACY HENSON - PROFESSIONAL ENGINEER
LIC. # PE-2016036828

CHICK-FIL-A
SUMMIT FAIR

690 NW BLUE PKWY
LEE'S SUMMIT, MO 64086

FSR#02859

BUILDING TYPE / SIZE: S08N-104-R, V8
RELEASE:
PRINTED FOR

PERMIT

REVISION SCHEDULE

NO. DATE DESCRIPTION

1 02/19/24 PLAY AREA REMOVAL

CONSULTANT PROJECT # 2023.0467

DATE AUGUST 2023

DRAWN BY MI

CHECKED BY SN

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SHEET

ELECTRICAL LIGHTING PLAN

SHEET NUMBER

E2.1

FLOOR PLAN KEY

ELEVATION TAG

DECOR TAG

WALL KEY

DIVIDER WALL by CHI

LOW WALL by G.C.

FULL HT. WALL by G.C.

DEMO WALLS KNEEWALL / MISC. by G.C.

EXISTING WALL TO REMAIN / MULTI-USE

BUILDING SHELL USED FOR FURNITURE PLANS PROVIDED BY SITE ADAPT ARCHITECT.

VERIFY DINING ROOM DIMENSIONS FOR FURNITURE & MESSAGING.

SEATING LAYOUT PROVIDED BY ARCHITECT. ARCHITECT IS RESPONSIBLE FOR SEATING PLAN MEETING CODE REQUIREMENTS (LOCAL OR OTHER).

NOTE: CONDIMENTS AND TRASH ARE NOTED FOR SPACE PLANNING PURPOSES ONLY. EXACT MODELS AND DETAILS ON THOSE ITEMS TO BE DETERMINED BY THE MILLWORK PROVIDER, CLAYTON FIXTURES (CFX).

Project Contacts

PLEASE CONTACT ONE OF THE FOLLOWING CHI TEAM MEMBERS TO ADDRESS CONCERNS YOU MAY HAVE:

CHARTER HOUSE HOLDINGS, LLC
Phone: 616.399.6000

Regional Sales Manager: Julia Woodward
Phone: 404.784.2960 | Email: cfo@charter-house.com

Project Management Team:
Allison Marsh Phone: 616.796.1159
Jami Fraser Phone: 616.796.1178

Customer Service Inquiries:
Phone: 616.796.1020

CHI Project Log

DATE	#	DESCRIPTION	INITIALS
02.09.24		LAYOUT - PLAY CONV.	RK
02.19.24	2	BASE UPDATE	-
03.05.24	3	PERMIT SET	RK
03.15.24	4	OP REDLINES	BS
03.21.24	5	BID SET	MM
04.04.24	6	CONSTRUCTION SET	EB
04.29.24	7	UPDATE EXISTING FURNITURE	JS

PROJECT:

STORE #:

02859

Chick-fil-A

BLDG. SHELL:

S08N-SM-R PLAY CONV.

LOCATION:

LEES SUMMIT, MO.

ADDRESS:

690 NW BLUE PARKWAY

NICKNAME:

LEES SUMMIT FSU

DESIGN:

RK

FDD SET:

FDD CHECK:

AM

PROJECT MGR:

ALLISON MARSH

SALES MGR:

JULIA WOODWARD

SET ISSUED DATE:

29 APRIL 2024

DRAWING TITLE:

FLOOR PLAN

STORE NUMBER:

02859

SCALE:

1/4" = 1'-0"

ACCESSIBILITY KEY

THESE PLANS, AS DRAWN, COMPLY WITH ALL APPLICABLE LAWS, REGULATIONS OR RULES OF ANY LOCAL, COUNTY, STATE, FEDERAL AND OTHER GOVERNING AUTHORITY AND ANY BUREAU OR DEPARTMENT THERE OF, WHICH MAY BE APPLICABLE INCLUDING, BUT NOT LIMITED TO THE AMERICANS WITH DISABILITIES ACT AND THE REGULATIONS THERE UNDER.

MANEUVERING CLEARANCES AT DOORS (N.I.S.)

ADA COMPLIANT CLEARANCES AT TABLES (N.I.S.)

IMPORTANT NOTE: NOT ALL DOOR CLEARANCE, ACCESSIBLE AIS, SHOWN IN THIS DIAGRAM.

General Contractor Notes and Information

*CHARTER HOUSE HOLDINGS, L.L.C. / CHARTER-HOUSE INNOVATIONS / CHI / CHARTER-HOUSE

GENERAL CONSTRUCTION

1. ALL WORK, CONSTRUCTION, AND MATERIALS SHALL COMPLY WITH ALL PROVISIONS OF THE UNIFORM BUILDING CODE AND WITH OTHER RULES, REGULATIONS, AND ORDINANCES GOVERNING THE PLACE OF CONSTRUCTION.

2. IT IS THE RESPONSIBILITY OF ALL PARTIES SUPPLYING LABOR OR MATERIAL, OR BOTH, TO BRING TO THE ATTENTION OF THE PROJECT MANAGER AND THE OWNER ANY DISCREPANCIES OR CONFLICT BETWEEN THE CODE REQUIREMENTS AND THESE PLANS.

3. THE APPLICABLE CODES SHALL INCLUDE, BUT SHALL NOT BE LIMITED TO THE UNIFORM BUILDING CODE, BOCA NATIONAL BUILDING CODE, UNIFORM PLUMBING CODE, UNIFORM ELECTRICAL CODE, UNIFORM MECHANICAL CODE, AND THE UNIFORM FIRE CODE.

4. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL WORK AND MATERIALS IN ACCORDANCE WITH ALL LOCAL AND / OR CITY BUILDING CODES.

5. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL AMERICANS WITH DISABILITIES ACT GUIDELINES, TO INCLUDE BUT NOT LIMITED TO TABLE TOP/BASE INSTALLATIONS, AISLE WAY CLEARANCES, AND QUEUE LINE AREA CLEARANCES.

6. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL REQUIRED BUILDING PERMITS.

7. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COPYING AND DISTRIBUTING CHARTER-HOUSE PLANS AND DOCUMENTS TO ALL SUBCONTRACTORS INVOLVED.

DELIVERY OF PRODUCT

1. DAMAGED PRODUCT UPON RECEIPT MUST BE DOCUMENTED ON THE BILL OF LADING. MUST CONTACT PROJECT MANAGER IMMEDIATELY (616) 399.6000 - FAILURE TO DOCUMENT DAMAGE AND CONTACT CHI WILL ABSOLVE CHI FROM RESPONSIBILITY.

2. INSPECT THE EXTERIOR OF ALL PACKAGING AND VERIFY BILL OF LADING. *CHECK FOR TEARS, CRUSHING, HOLES, ETC.....* COMPARE SHRINK-WRAP OR TAPE AND WATCH FOR MULTIPLE KINDS OF WRAP, OR DIFFERENT TAPE ON BOXES *LISTEN FOR RATTLING OR LOOSE PARTS DURING UNLOADING * WATCH FOR POSSIBLE WEIGHT DISCREPANCIES DURING UNLOADING.

3. IF YOU NOTICE ANY OF THE ABOVE, ASK FOR A JOINT INSPECTION AND SPECIFY NOTATIONS ON THE CARRIER'S RECEIPT. IF THE DELIVERING PARTY REFUSES, INSIST ON A NOTATION OF WHAT YOU HAVE NOTICED AND CALL THE CARRIER IMMEDIATELY UPON COMPLETING YOUR INSPECTION.

4. ALWAYS KEEP GOOD NOTES OF DATES, TIMES, AND REASONS SPOKEN TO.

5. CONCEALED DAMAGE: *IF THE CARRIER'S RECEIPT HAS BEEN SIGNED OFF, AND DAMAGE IS NOTICED INSIDE OF PACKAGING, CALL THE CARRIER AND REQUEST INSPECTION IMMEDIATELY. ALSO NOTIFY YOUR CHI PROJECT MANAGER OR SHIPPING DEPT WITHIN 48 HOURS.

6. ANY OTHER CONCERNS OR QUESTIONS PLEASE CALL YOUR PROJECT MANAGER.

7. GENERAL CONTRACTOR AND/OR OWNER ARE RESPONSIBLE FOR PROTECTION OF PRODUCT WHILE IN THE PROCESS OF BEING INSTALLED. ANY PRODUCT DELIVERED IN GOOD CONDITION, BUT DAMAGED ON SITE IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND/OR OWNER.

NEW AND REMODEL PROJECTS

1. THESE PLANS ARE THE PROPERTY OF CHARTER HOUSE HOLDINGS, L.L.C. THESE PLANS MAY NOT BE REPRODUCED OR IMPLEMENTED IN ANY WAY, IN PART OR IN WHOLE, WITHOUT THE EXPRESS CONSENT OF CHI. UNAUTHORIZED USE WILL RESULT IN PAYMENT DUE FOR SERVICES RENDERED. CHI REQUESTS VERIFICATION OF FIELD DIMENSIONS FROM THE GENERAL CONTRACTOR FOR THE FABRICATION OF COMMERCIAL FURNITURE, DECOR, CORE DRILL, AND SPACE PLANS. WHEN FIELD DIMENSIONS CAN NOT BE ATTAINED, LATE SHIPMENTS MAY RESULT.

2. IF SPECIFIED ITEMS ARE NOT AVAILABLE, CONTACT YOUR PROJECT MANAGER AT CHI.

3. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE JOB SITE AND NOTIFY CHI OF ALL DISCREPANCIES PRIOR TO COMMENCING WORK.

4. GENERAL CONTRACTOR TO PROVIDE A MINIMUM OF 40 LBS OF FLOOR TILE GROUT TO FINISH THE CORE DRILLED HOLES.

5. GENERAL CONTRACTOR WILL BE RESPONSIBLE FOR DISPOSAL OF ALL PACKAGING MATERIAL FROM SEATING AND DECOR PRODUCTS.

6. G.C. TO POSITION THERMOSTAT / VOLUME CONTROL / AND OTHER LIKE ITEMS TO NOT INTERFERE WITH THE CHI DECOR.

REMODEL PROJECTS ONLY

1. IN CASES OF REMODELING, THE EXISTING SEATING, STEEL, DECOR, AND WALL FINISHES ARE TO BE REMOVED BY THE OWNER AND / OR GENERAL CONTRACTOR. IT IS NOT THE RESPONSIBILITY OF CHI.

2. FLOOR / BASE TILE MUST BE IN GOOD REPAIR BEFORE ANY CORE DRILLING BEGINS.

3. ALL DIMENSIONS ARE FROM EXISTING FINISHES. PLEASE MAKE APPROPRIATE ALLOWANCES FOR ANY NEW FINISHES.

4. PLEASE CONTACT CHI CONCERNING ANY CONFLICTS BETWEEN CHI DECOR AND ELECTRICAL OR HVAC PLACEMENT.

FLOORING NOTES

1. A MINIMUM 5" CONCRETE SLAB IS REQUIRED FOR ALL CORE DRILLING.

2. ALL FLOOR AND COVE BASE TILE TO BE PROVIDED AND INSTALLED BY THE GENERAL CONTRACTOR.

3. NEW FLOOR AND BASE TILE TO BE INSTALLED AND GROUTED 48 HOURS PRIOR TO CHI PRODUCT INSTALLATION. IF DAMAGE OCCURS DUE TO INSUFFICIENT CURING TIME, REPAIRS WILL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

4. CHI IS NOT LIABLE FOR CORED STEEL THAT BECOMES UNSTABLE AS A RESULT OF DRILLING THROUGH A CONCRETE SLAB LESS THAN 5" THICK.

5. CHI IS NOT RESPONSIBLE FOR DAMAGE OF ANY MECHANICAL LOCATED WITHIN 5' OF THE FINISHED FLOOR.

6. COVE BASE / BULLNOSE TILE ON ALL CHI DIVIDER WALLS TO BE SUPPLIED AND INSTALLED BY THE GENERAL CONTRACTOR.

ELECTRICAL, HVAC, AND CEILING NOTES

1. G.C. TO SUPPLY AND CONNECT ELECTRICAL PIGTAIL AT ALL CODED LOCATIONS FOR CHI PRODUCT.

2. ALL ELECTRICAL FIXTURES, WIRING, AND INSTALLATIONS TO BE UL APPROVED.

3. LIGHTING FIXTURES AND/OR HVAC MAY NEED TO BE RELOCATED BY G.C. TO ACCOMMODATE DECOR AND ELECTRICAL OR HVAC PLACEMENT.

4. PLEASE CONTACT YOUR CHI PROJECT MANAGER CONCERNING ANY CONFLICTS BETWEEN PLACEMENT OF CHI DECOR ELEMENTS.

5. GENERAL CONTRACTOR TO CONTACT CONSTRUCTION ENGINEER TO VERIFY FINAL PLACEMENT OF HVAC.

IMPORTANT NOTES:

1. WRITTEN DIMENSIONS HAVE PRECEDENCE OVER SCALE IN ALL CASES.

2. THE GENERAL CONTRACTOR MUST VERIFY ALL BUILDING DIMENSIONS, EQUIPMENT, FURNITURE PLACEMENT, UTILITY LOCATIONS, AND CONDITIONS AT THE JOB SITE.

3. THE GENERAL CONTRACTOR MUST NOTIFY CHI OF ALL DISCREPANCIES AND/OR REQUIRED ADJUSTMENTS PRIOR TO STARTING WORK.

4. NOT SCALE OFF PLANS. CALL YOUR CHI PROJECT MANAGER TO VERIFY ANY QUESTIONABLE DIMENSIONS (616) 399.6000.

5. ANY DEVIATIONS FROM THESE DRAWINGS FOR ANY REASON SHALL NOT BE DONE WITHOUT CONSENT OF CHARTER-HOUSE IN ADVANCE, IN WRITING. CHARTER-HOUSE IS NOT RESPONSIBLE FOR DEVIATIONS, CHANGES, ADDITIONS, OR DELETIONS NOT REVIEWED AND APPROVED IN ADVANCE BY CHARTER-HOUSE.

6. CHARTER-HOUSE HAS PREPARED THESE DOCUMENTS AS ACCURATELY AS POSSIBLE WITH THE ARCHITECTURAL INFORMATION PROVIDED. DUE TO LACK OF ACCURATE AND COMPLETE DIMENSIONED ARCHITECTURAL PLANS, OWNER UNDERSTANDS THAT ADJUSTMENTS TO THE FINAL LOCATIONS OF EQUIPMENT AND/OR FURNITURE PLACEMENT MAY BE REQUIRED AT THE OWNER'S EXPENSE DUE TO DISCREPANCIES BASED ON ACTUAL JOB SITE DIMENSIONS AND CONDITIONS.

7. REFER TO ARCHITECTURAL OR ENGINEER DRAWINGS FOR COMPLETE AND ACCURATE DIMENSIONS OF BUILDING STRUCTURE.

8. THESE ARE CONTRACT DOCUMENTS AND THE INFORMATION THEY CONTAIN ARE THE PROPERTY OF CHARTER-HOUSE.

9. ALL CONTRACT DOCUMENTS MAY NOT BE COPIED, REPRODUCED, USED OR IMPLEMENTED IN ANY WAY, IN PART OR IN WHOLE, WITHOUT THE EXPRESS WRITTEN CONSENT OF CHARTER-HOUSE.

10. ANY UNAUTHORIZED USE OF THESE PLANS WILL RESULT IN PAYMENT DUE FOR SERVICES RENDERED BY CHARTER-HOUSE.

11. REFER TO ARCHITECTURAL OR ENGINEER DRAWINGS FOR COMPLETE AND ACCURATE DIMENSIONS OF BUILDING STRUCTURE.

SITE CONDITIONS FOR CHI DECOR INSTALLATION

1. IT IS IMPORTANT FOR THE FOLLOWING WORK TO BE COMPLETE PRIOR TO THE ARRIVAL OF CHI PRODUCT AND / OR INSTALL CREWS. THESE CONDITIONS ARE ESTABLISHED TO INSURE THAT PRODUCT IS INSTALLED MOST EFFICIENTLY (WITHOUT DAMAGE TO PRODUCT) AND UNDER SAFE CONDITIONS.

2. ALL INTERIOR WALL FINISHES TO BE COMPLETE, INCLUDING (BUT NOT LIMITED TO): WALLCOVERING, WAINSCOTING, PAINT AND CHAIR RAIL.

3. FLOOR AND BASE TILE TO BE TILED AND GROUTED FOR A MINIMUM OF 48 HOURS PRIOR TO ARRIVAL.

4. GENERAL CONTRACTOR TO PROVIDE A MINIMUM OF 40 POUNDS FLOOR TILE GROUT TO FINISH THE CORE DRILLED HOLES.

5. ALL HVAC WORK TO BE COMPLETE.

6. CEILING TRACK AND TILE TO BE INSTALLED.

7. ALL WINDOWS TO BE INSTALLED IN BUILDING.

8. ELECTRICAL SERVICE TO 120V / 30 AMP CIRCUITS FOR CORE DRILLING.

9. POTABLE WATER SUPPLY.

10. OPERATING LIGHTS (ADEQUATE FOR EVENING WORK).

11. CLEAN AND OPEN WORK AREA IN DINING ROOM.

12. WASTE DUMPSTER ON SITE.

13. FINISHED PARKING LOT OR SUBSURFACE WITH 1" OF FINISHED GRADE.

14. UNENCUMBERED AND CONTINUOUS ACCESS TO THE BUILDING.

15. GENERAL CONTRACTOR RESPONSIBLE FOR TEMPORARY POWER IF ELECTRICAL SERVICE IS UNAVAILABLE.

SEATING & TABLE SCHEDULE

TABLE TOP STYLES	SINGLES	2 TOPS	3 TOPS	4 TOPS	5 TOPS	6 TOPS	7 TOPS	8 TOPS	10 TOPS	TOTALS
TABLE TOP QUANTITY	13	12	-	15	-	1	-	-	-	35
SEAT QUANTITY	13	12	-	60	-	8	-	-	-	93
ACCESSIBLE TABLE RATIO:	5	SEATING TO TABLE RATIO: 2.6								

STEEL KEY

NOTE: THIS KEY REPRESENTS GENERAL STEEL SYMBOLS. REFERENCE PROJECT SPECIFICATIONS FOR PRODUCT DESCRIPTIONS DETAILED IN PLAN.

CORE-DRILLED STEEL SYMBOLS	GENERIC	12" X 12" PLATE	12" X 12" PLATE (OFFSET)	1" X 3" STEEL	1" X 3" STEEL (OFFSET)	FOOT RINGS	GENERIC	12" X 12" PLATE (ROUND)	1" X 3" STEEL	1" X 3" STEEL (OFFSET)	FOOT RINGS	FREESTANDING STEEL SYMBOLS	X-BASE	BI-POD
TAPERED							TAPERED							

SEATING & DECOR KEY

#	DESCRIPTION
1	LIBRARY CHAIR, STANDARD HT (STYLE 1)
2	LIBRARY CHAIR, TALL HEIGHT (STYLE 2)
3	FARM STOOL, 22" HEIGHT
4	MINI ARENA BOOTH *MATCH EXISTING*
5	TABLE *MATCH EXISTING*
6	COKE FIXTURE
7	DIVIDER WALL *MATCH EXISTING*
8	DIVIDER WALL *MATCH EXISTING*
9	WOOD DINING COUNTER *ACCESSIBLE*

INTERIOR MESSAGING

REFER TO FURNITURE PLAN & INTERIOR ELEVATIONS FOR LOCATIONS

QTY:	PART #:	ITEM #:	DESCRIPTION:	SIZE: H X W
		300246	DIMO - TABLE MARKERS *NUMBERS VARY BY BUILDING STYLE*	

Chick-fil-A

5200 Buffington Road

Atlanta, Georgia 30349

CHI

CHARTER HOUSE HOLDINGS, LLC

200 N. Franklin Street

Zeland, MI 49464

Phone: 616.399.6000 Fax: 616.796.1199

www.gotochi.com

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CHICK-FIL-A

LEES SUMMIT

690 NW BLUE PARKWAY

SUMMIT, MO. 64086

FSR#02859

BUILDING TYPE / SIZE: S08N - SML-R

RELEASE: PLAY CONV.

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
1	02/19/24	PLAY AREA REMOVAL
2	03/11/24	COORDINATION REVISIONS

CONSULTANT PROJECT # ####

PRINTED FOR CONSTRUCTION

DATE 02/09/2024

DRAWN BY RK

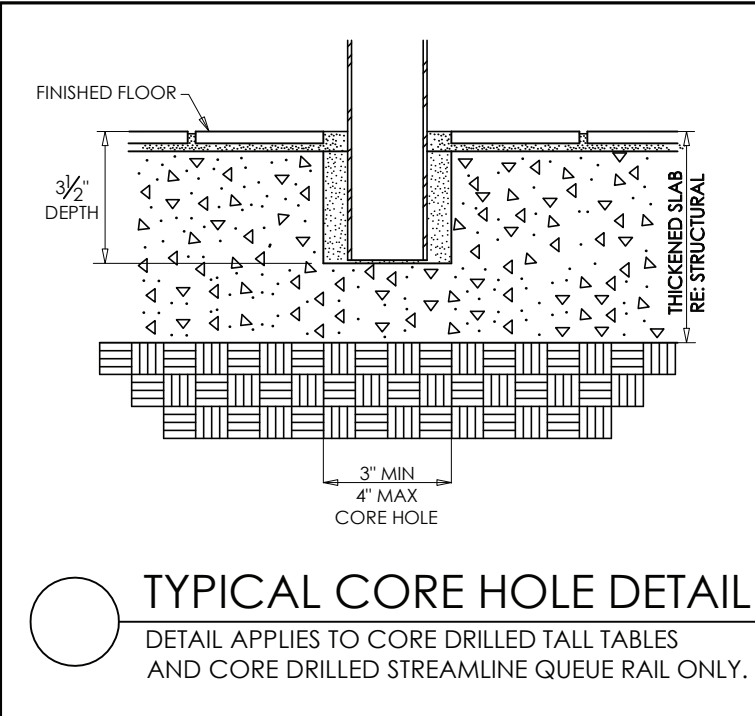
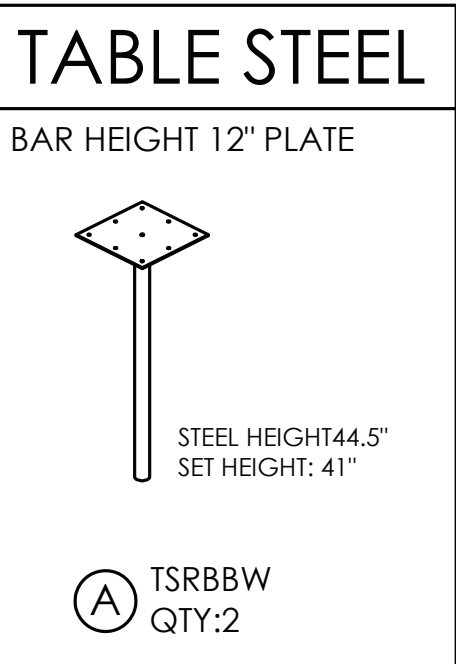
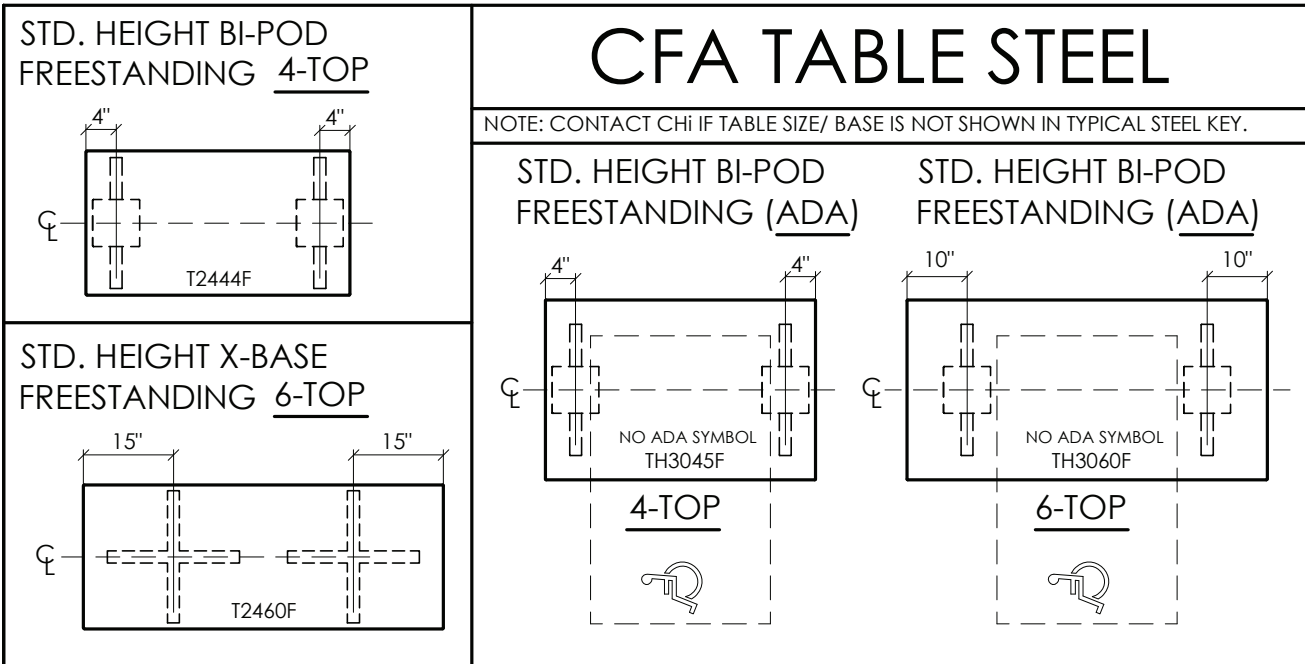
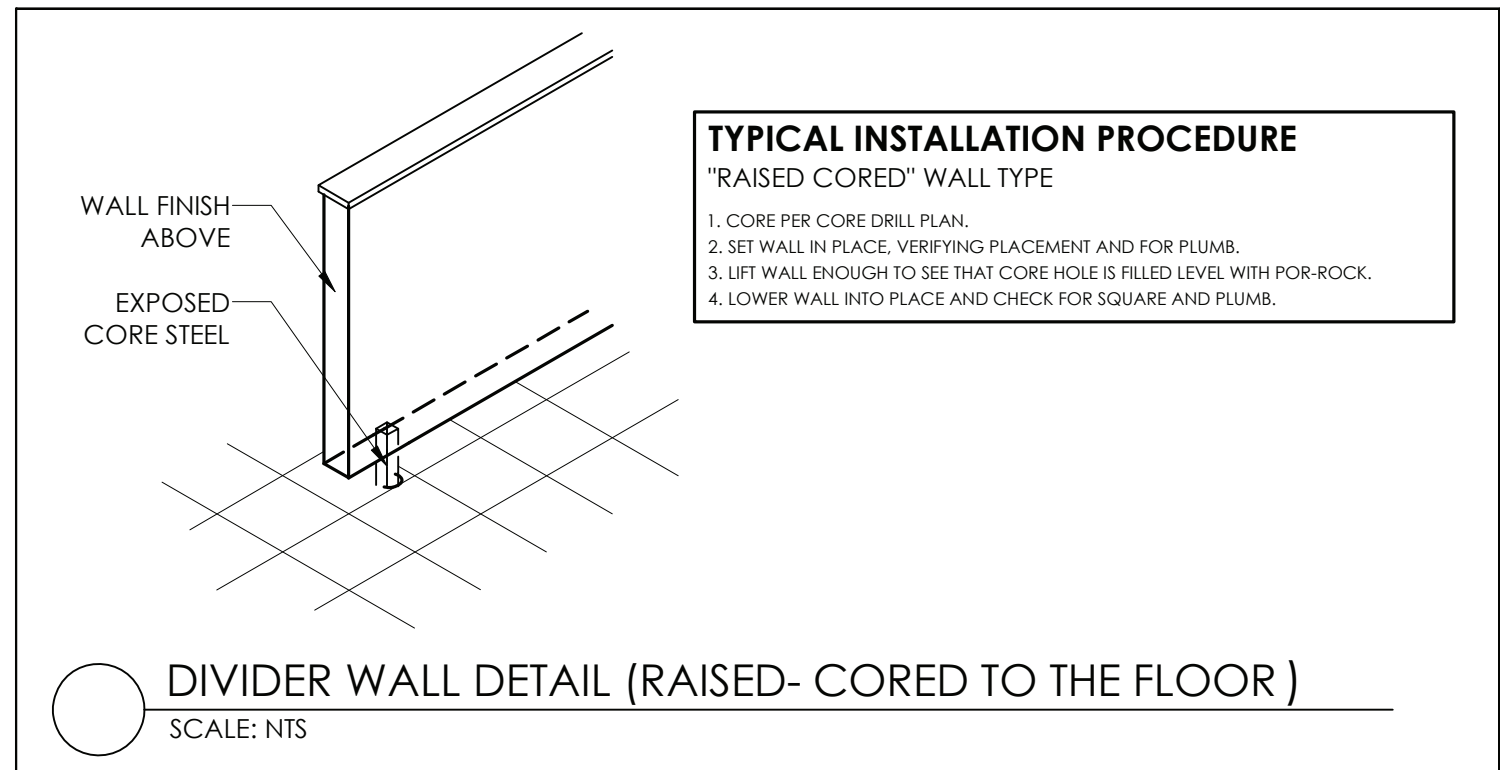
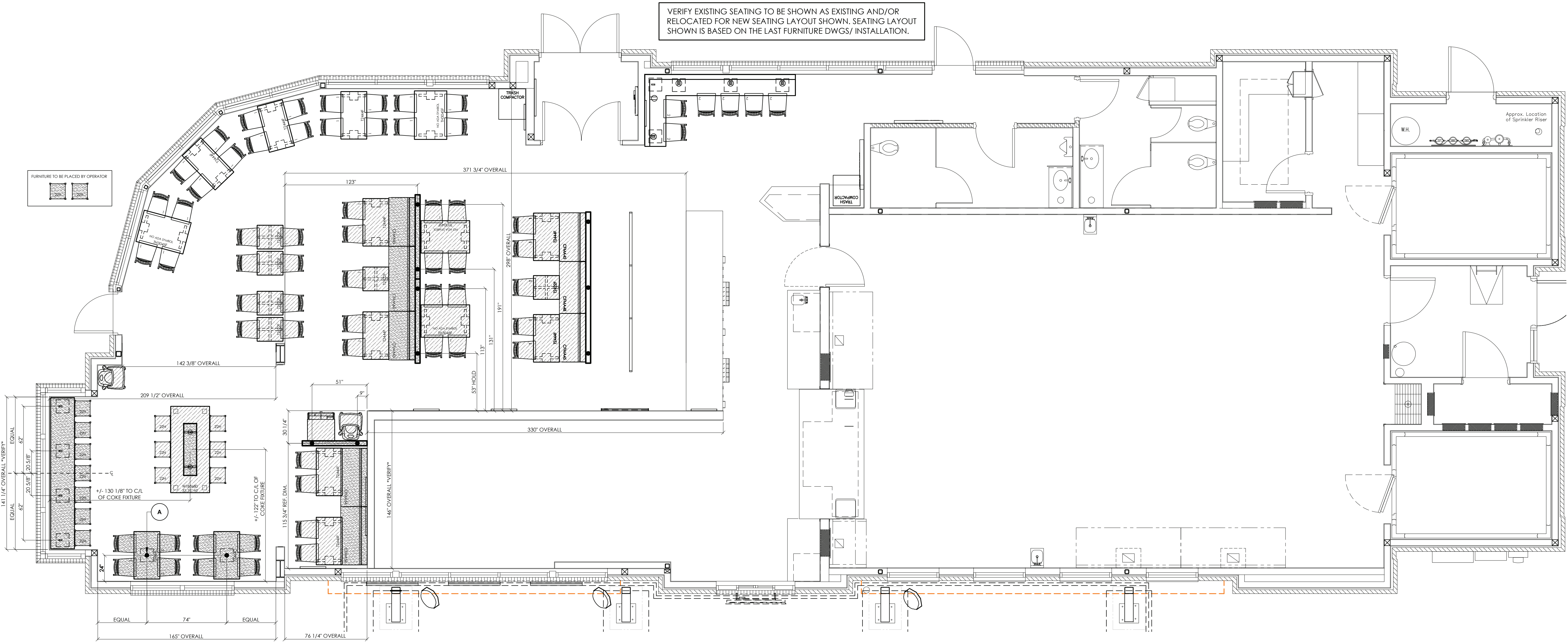
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SHEET FURNITURE FLOOR PLAN

SHEET NUMBER

F-201

STEEL KEY										PRODUCT KEY		
CORE-DRILLED STEEL SYMBOLS	GENERIC	12" X 12" PLATE	12" X 12" PLATE (OFFSET)	1" X 3" STEEL	1" X 3" STEEL (OFFSET)	FOOT RINGS	GENERIC	12" X 12" PLATE (ROUND)	1" X 3" STEEL	1" X 3" STEEL (OFFSET)	FOOT RINGS	FREESTANDING STEEL SYMBOLS
	TAPERED						TAPERED					X-BASE
NOTE: THIS KEY REPRESENTS GENERAL STEEL SYMBOLS. REFERENCE PROJECT SPECIFICATIONS FOR PRODUCT DESCRIPTIONS DETAILED IN PLAN.										NOTE: REFER TO FURNITURE FLOOR PLAN FOR LAYOUT.		
										NEW	RELOCATED	EXISTING
												NO HATCH USED



Chick-fil-A

Chick-fil-A
5200 Buffington Road
Atlanta, Georgia 30349

Chi

CHARTER HOUSE HOLDINGS, LLC
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CHICK-FIL-A

LEES SUMMIT

690 NW BLUE PARKWAY
SUMMIT, MO. 64086

FSR#02859

BUILDING TYPE / SIZE: S08N - SML-R
RELEASE: PLAY CONV.

REVISION SCHEDULE		
NO.	DATE	DESCRIPTION
1	02/19/24	PLAY AREA REMOVAL
2	03/11/24	COORDINATION REVISIONS

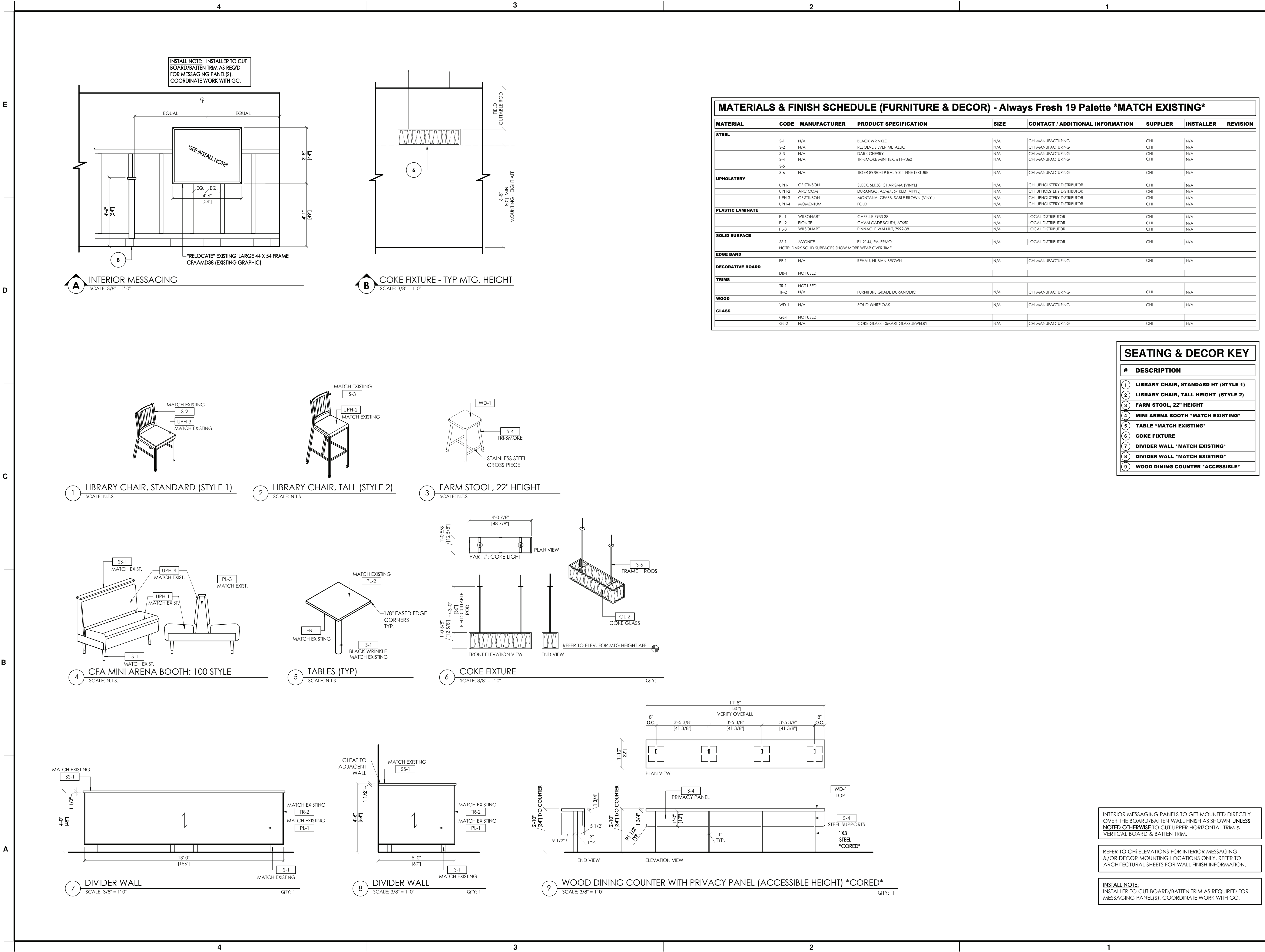
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SHEET CORE DRILL PLAN

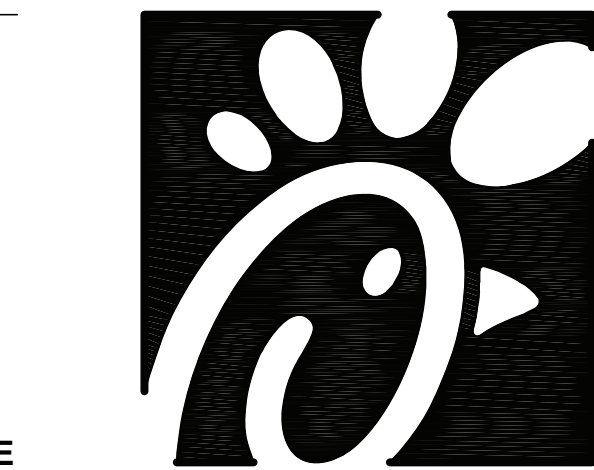
SHEET NUMBER

F-211



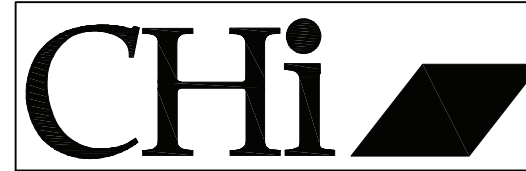
MATERIALS & FINISH SCHEDULE (FURNITURE & DECOR) - Always Fresh 19 Palette *MATCH EXISTING*								
MATERIAL	CODE	MANUFACTURER	PRODUCT SPECIFICATION	SIZE	CONTACT / ADDITIONAL INFORMATION	SUPPLIER	INSTALLER	REVISION
STEEL	S-1	N/A	BLACK WRINKLE	N/A	CHI MANUFACTURING	CHI	N/A	
	S-2	N/A	RESOLVE SILVER METALLIC	N/A	CHI MANUFACTURING	CHI	N/A	
	S-3	N/A	DARK CHERRY	N/A	CHI MANUFACTURING	CHI	N/A	
	S-4	N/A	TRI-SMOKE MINI TEX. #T1-7060	N/A	CHI MANUFACTURING	CHI	N/A	
	S-5							
	S-6	N/A	TIGER 89/80419 RAL 9011-FINE TEXTURE	N/A	CHI MANUFACTURING	CHI	N/A	
UPHOLSTERY	UPH-1	CF STINSON	SLEEK, SLK38, CHARDMA (VINYL)	N/A	CHI UPHOLSTERY DISTRIBUTOR	CHI	N/A	
	UPH-2	ARC COM	DURANGO, AC-67567 RED (VINYL)	N/A	CHI UPHOLSTERY DISTRIBUTOR	CHI	N/A	
	UPH-3	CF STINSON	MONTANA, CFAS8, SABLE BROWN (VINYL)	N/A	CHI UPHOLSTERY DISTRIBUTOR	CHI	N/A	
	UPH-4	MOMENTUM	FOLD	N/A	CHI UPHOLSTERY DISTRIBUTOR	CHI	N/A	
PLASTIC LAMINATE	PL-1	WILSONART	CAFELLE 7953-38	N/A	LOCAL DISTRIBUTOR	CHI	N/A	
	PL-2	PIONITE	CAVALCADE SOUTH, A1650	N/A	LOCAL DISTRIBUTOR	CHI	N/A	
	PL-3	WILSONART	PINNACLE WALNUT, 7992-38	N/A	LOCAL DISTRIBUTOR	CHI	N/A	
SOLID SURFACE	SS-1	AVONITE	F1-9144, PALERMO	N/A	LOCAL DISTRIBUTOR	CHI	N/A	
			NOTE: DARK SOLID SURFACES SHOW MORE WEAR OVER TIME					
EDGE BAND	EB-1	N/A	REHAU, NUBIAN BROWN	N/A	CHI MANUFACTURING	CHI	N/A	
DECORATIVE BOARD	DB-1	NOT USED						
TRIMS	TR-1	NOT USED						
	TR-2	N/A	FURNITURE GRADE DURANODIC	N/A	CHI MANUFACTURING	CHI	N/A	
WOOD	WD-1	N/A	SOLID WHITE OAK	N/A	CHI MANUFACTURING	CHI	N/A	
GLASS	GL-1	NOT USED						
	GL-2	N/A	COKE GLASS - SMART GLASS JEWELRY	N/A	CHI MANUFACTURING	CHI	N/A	

SEATING & DECOR KEY	
#	DESCRIPTION
1	LIBRARY CHAIR, STANDARD HT (STYLE 1)
2	LIBRARY CHAIR, TALL HEIGHT (STYLE 2)
3	FARM STOOL, 22" HEIGHT
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9	WOOD DINING COUNTER *ACCESSIBLE*



Chick-fil-A

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LEES SUMMIT
690 NW BLUE PARKWAY
SUMMIT, MO. 64086

FSR#02859

BUILDING TYPE / SIZE: S08N - SML-R
RELEASE: PLAY CONV.

REVISION SCHEDULE		
NO.	DATE	DESCRIPTION
1	02/19/24	PLAY AREA REMOVAL
2	03/11/24	COORDINATION REVISIONS

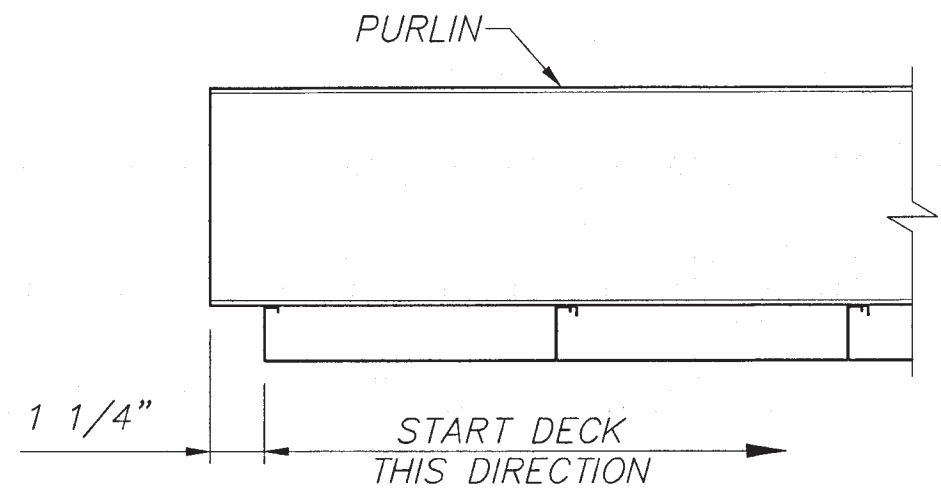
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SHEET
DECOR ELEVATIONS
PRODUCT DETAILS

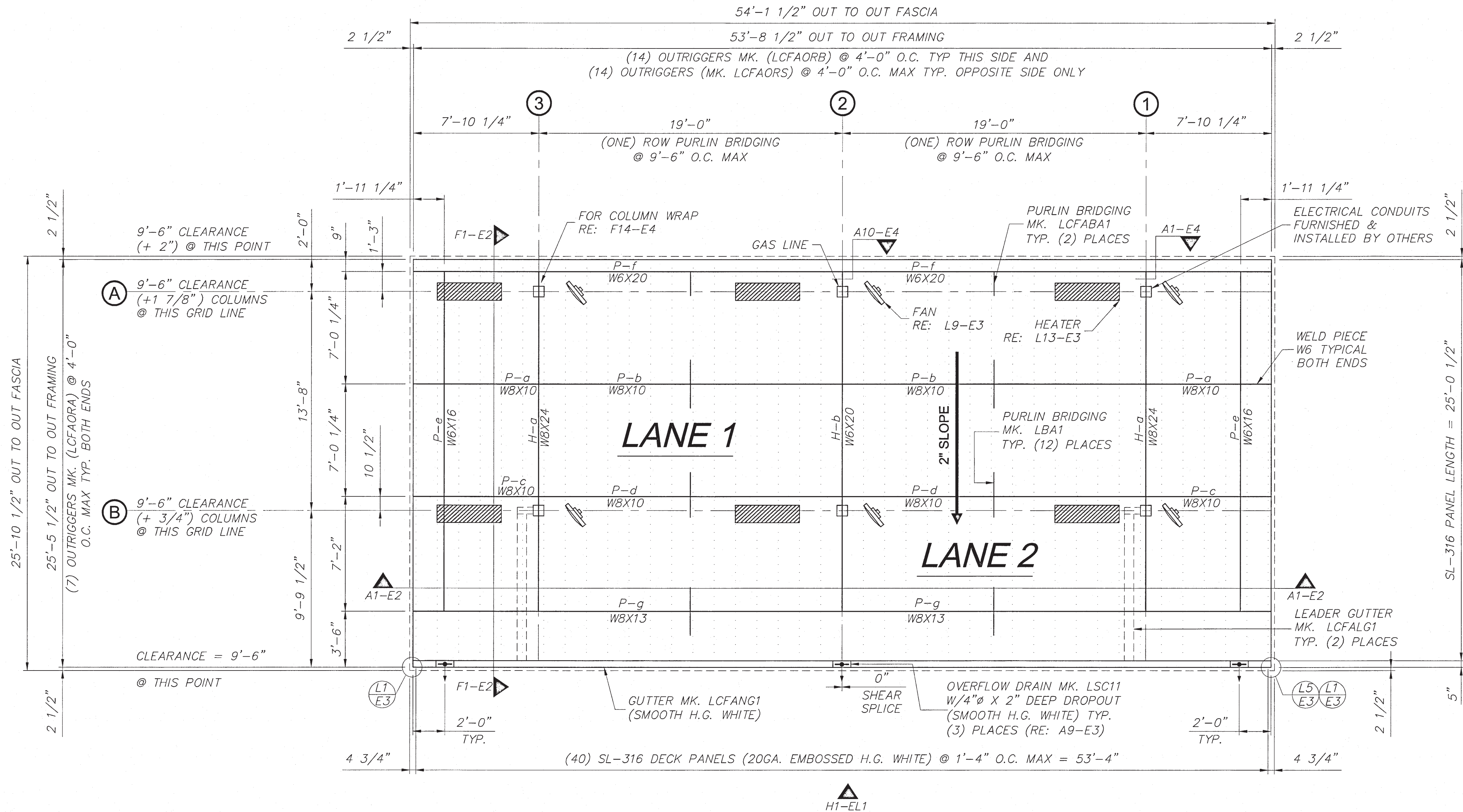
SHEET NUMBER

F-701



LANE 1 ENTRANCE

LANE 2 ENTRANCE



LANE 1 EXIT

LANE 2 EXIT

FI CANOPY FRAMING LAYOUT

1/4" = 1'-0"

STRUCTURAL STEEL SHALL MEET THE AISC 2017 SPECIFICATION 15TH EDITION AND THE AISC CODE OF STANDARD PRACTICE, CURRENT VERSION.
COLUMNS TO BE ASTM A500, GRADE B
BOLTS TO BE ASTM A325 OR ASTM F1852 (A325-TC)
INSTALLATION OF BOLTS SHALL BE BROUGHT TO A SNUG TIGHT CONDITION-DEFINED AS THE CONDITION THAT EXISTS WHEN ALL OF THE PLIES IN A CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT BY THE BOLTS IN THE JOINT AND ALL OF THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

WIDE FLANGE BEAMS TO BE ASTM A992
ANGLES, & PLATES TO BE ASTM A36
REINFORCING STEEL TO BE ASTM 615, GRADE 60
DECK PANELS TO BE ASTM 653, GRADE C MINIMUM
WELD FILLER METALS SHALL MEET THE MINIMUM CHARPY V-NOTCH REQUIREMENT OF 20 FT-LB AT 0°F WELDING SHALL MEET THE REQUIREMENTS OF THE AWS FOR BUILDING CONSTRUCTION USING E70XX ELECTRODES
ALL STRUCTURAL STEEL TO BE PAINTED WITH ONE SHOP COAT PRIMER
CANOPY FABRICATOR SHALL BE AISC CERTIFIED
LANE SUPPLY INC. IS AN AISC CERTIFIED FABRICATOR (AISC # C-00022431)

1. REFERENCE SEALANT SCHEDULE FOR ALL APPLICATIONS
2. SEAL ALL JOINTS WITH A SMOOTH, CLEAN APPLICATION
3. APPLY CAULK CLEAR AROUND THE COLUMNS ON THE TOP SIDE OF THE DECK AFTER BOTTOM SIDE HAS BEEN CAULKED.
4. DECK PANELS AND TRIM WILL BE WIPED CLEAN AFTER INSTALLATION
5. ALL TRASH AND EXTRA MATERIALS WILL BE HAULED OFF JOBSITE
6. CHECK WITH GENERAL CONTRACTOR FOR DRAIN ORIENTATION
7. FURNISH & INSTALL LANE DESIGNED AND ENGINEERED "HUNG" DECK
8. FURNISH & INSTALL SUPPORT FRAMING FOR (6) FANS & (6) HEATERS (FANS & HEATERS FURNISHED & INSTALLED BY OTHERS.)
9. FURNISH & INSTALL (6) LSI CRUS-CS-LED-LW-30-UE-WHT CANOPY DECK LIGHTS.
10. FURNISH & INSTALL SHEET METAL FASCIA "CHICK-FIL-A BRONZE".

SEALANT SCHEDULE		
SEALANT	COLOR	APPLICATION
SOUDEASEAL FC	WHITE	DECK TO COLUMN @ BOTTOM
SOUDEASEAL FC	WHITE	GUTTER JOINTS
SOUDEASEAL FC	WHITE	DECK TO COLUMN @ TOP
SOUDEASEAL FC	WHITE	SEAL @ OVERFLOW DROPOUTS
SOUDEASEAL FC	WHITE	SEAL BOLTS @ HEATER SUPPORTS
SOUDEASEAL FC	WHITE	DAM UP DECK @ DECK CLOSURE
SOUDEASEAL FC	WHITE	SEAL FASCIA @ DECK CLOSURE

DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) +
WEIGHT OF STRUCTURAL COMPONENTS
LIVE LOAD = 20 p.s.f.
SNOW LOAD = 20 p.s.f.
WIND LOAD V_{ULT} = 116 m.p.h. EXP. C
WIND V_{ASD} = 90 m.p.h. EXP. C
BLDG CODE = MISSOURI BUILDING CODE 2018
ADOPTING 2018 INTERNATIONAL BUILDING CODE
EQUIVALENT LATERAL FORCE PROCEDURE
LATERAL FORCE RESISTING SYSTEM = CANTILEVERED
COLUMN SYSTEM-ORDINARY STEEL MOMENT FRAME
P_f = 20 p.s.f. C_e = 1.2 C_t = 1.2 I_s = 1.0
W = DRIFT LOADS NOT CONSIDERED
P_d = DRIFT LOADS NOT CONSIDERED
SITE CLASS = D
S_s (0.2) = 0.099
S₁ (1.0) = 0.068
SDS = 0.11
SD1 = 0.11
F_a = 1.60
F_v = 2.40
R = 1.25
IMPORTANCE FACTOR = 1.0
RISK CATEGORY = II
SEISMIC DESIGN CATEGORY = D
CS = 0.084
CONSTRUCTION TYPE = IIB
OCCUPANCY CATEGORY = A2
TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 0.84 KIPS

AI GENERAL NOTES

N.T.S.

A6 ERECTOR'S NOTES

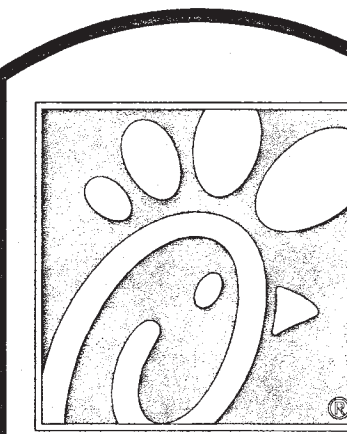
N.T.S.

AII NOT USED

N.T.S.

A14 DESIGN LOADS

N.T.S.



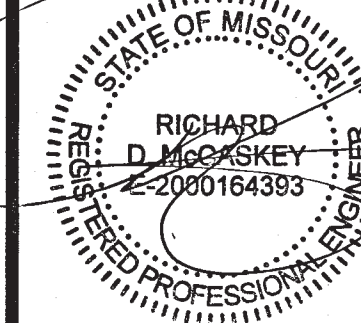
Chick-fil-A

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:

Mark Date By

Seal



SEP 07 2023

C.O.A. 2001015838

LANE
SUPPLY, INC.
120 FAIRVIEW
ARLINGTON, TX. 76010
(817) 261-9116

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STORE
Chick-fil-A #02859
690 NW BLUE
PARKWAY.
LEE'S SUMMIT, MO
64086

SHEET TITLE

CANOPY FRAMING
PLAN

25'-10 1/2" X 54'-1 1/2"

Job No.: LSC: 759966

Store : 02859

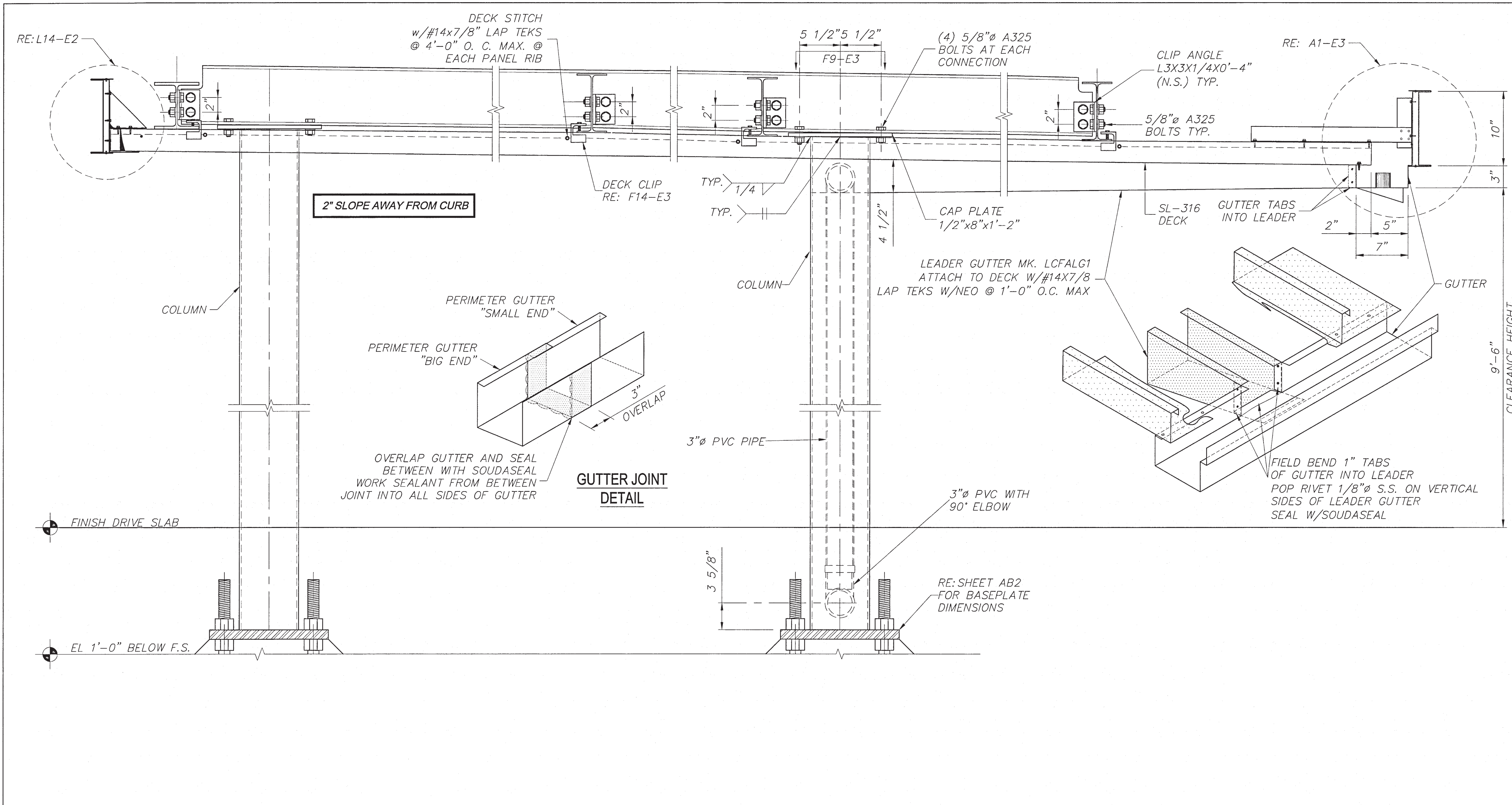
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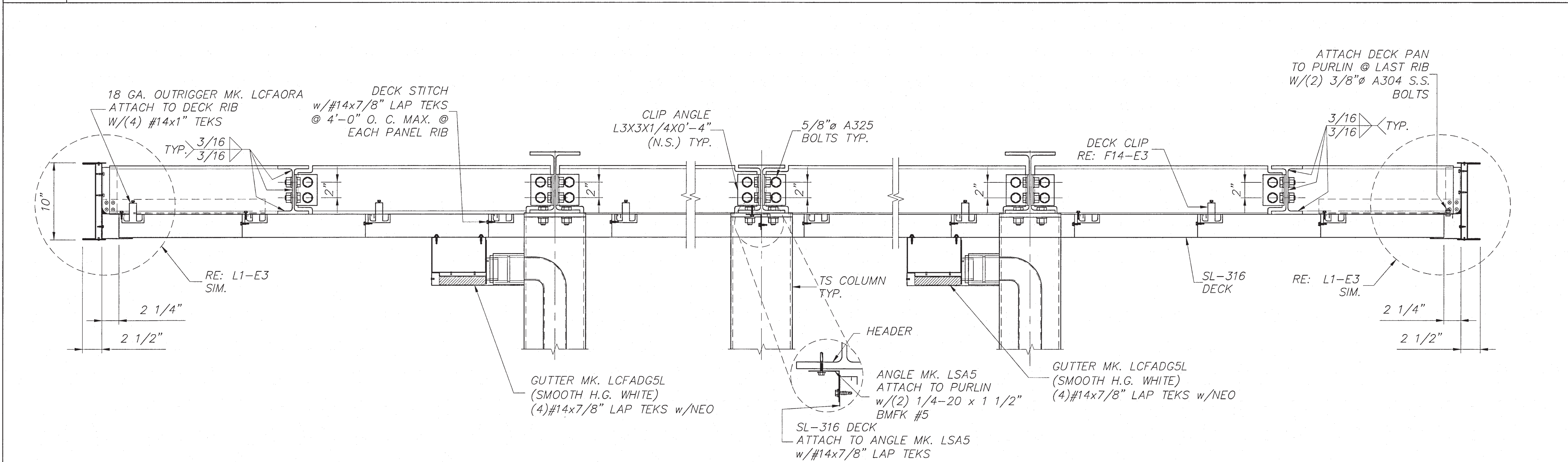
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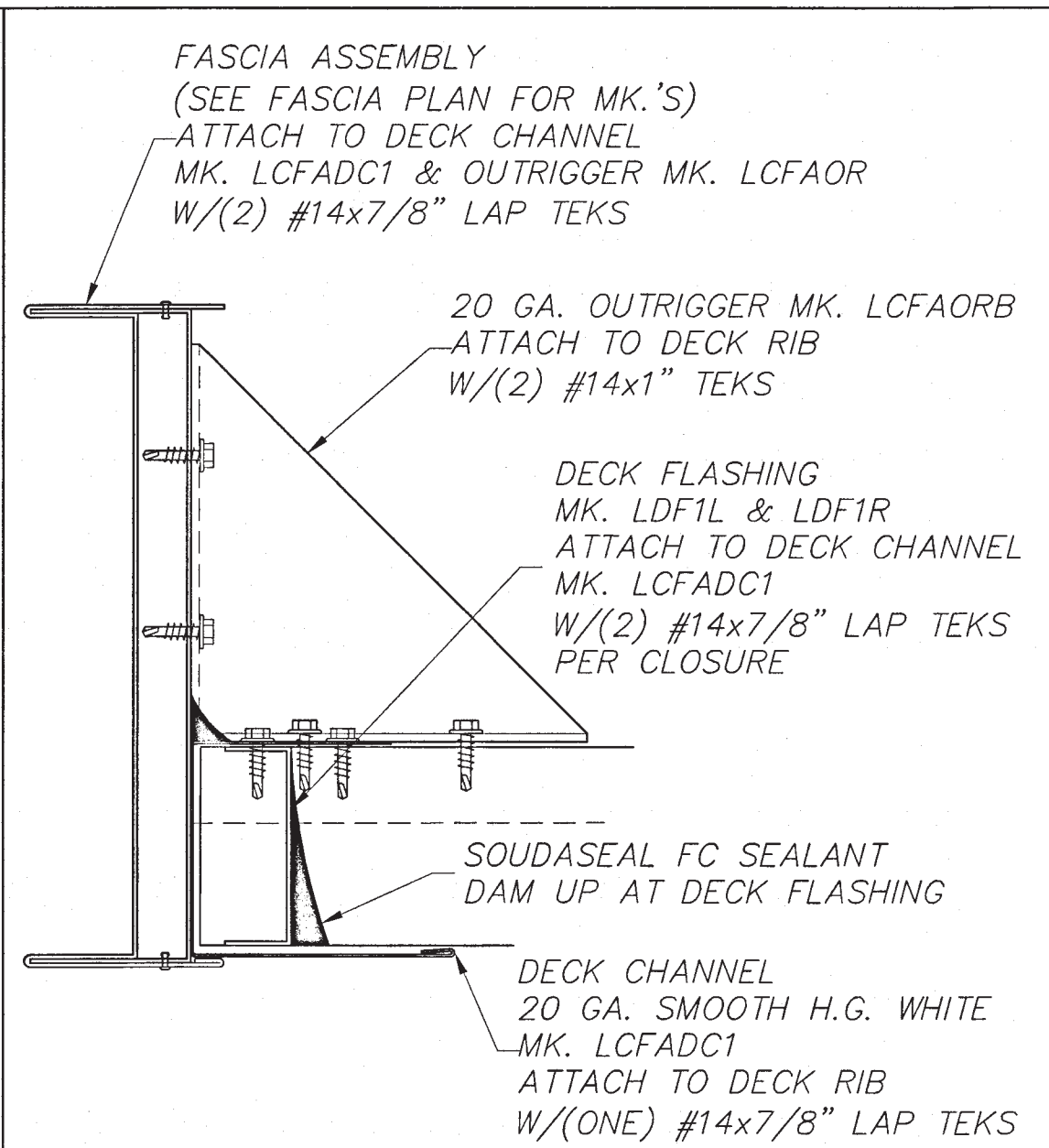
F2FC-3
EI OF 4



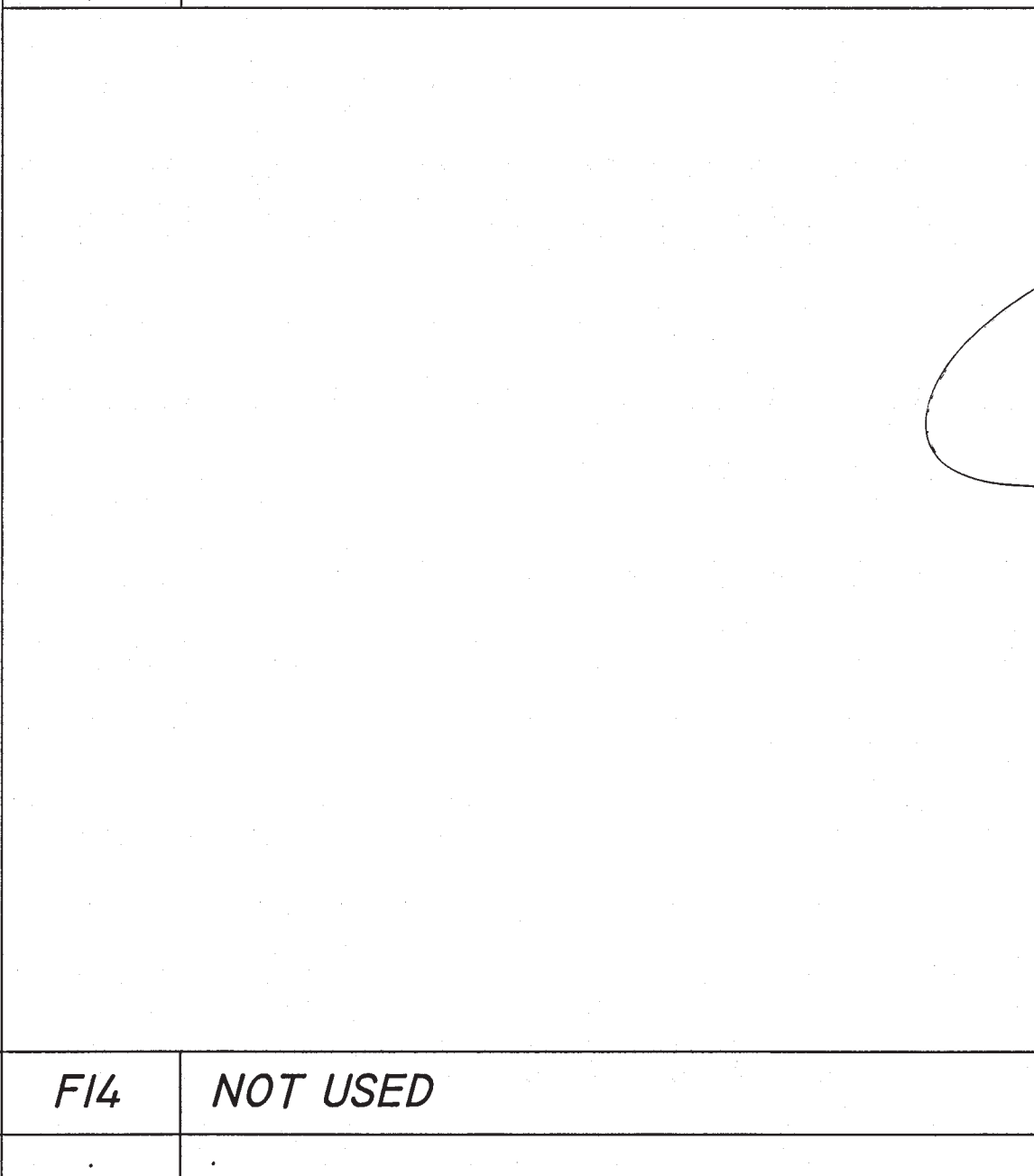
FI	FASCIA SECTION
1 1/2" = 1'-0"	FI-EI



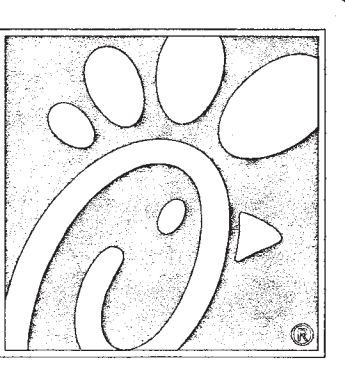
AI	FASCIA SECTION
1 1/2" = 1'-0"	FI-EI



LI4	SECTION AT FASCIA
N.T.S.	FI-E2



FI4	NOT USED
N.T.S.	



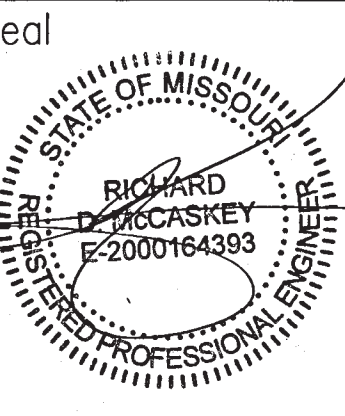
Chick-fil-A

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Mark	Date	By

Seal



SEP 07 2023

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STORE
Chick-fil-A #02859
690 NW BLUE
PARKWAY.
LEE'S SUMMIT, MO
64086

SHEET TITLE

CANOPY SECTIONS

25'-10 1/2" X 54'-1 1/2"

Job No.: LSC: 75966

Store : 02859

Date : 8.30.23

Drawn By : RED

Checked By: RM

Sheet

F2FC-4

E2 OF 4

<p>PURLIN</p> <p>PANEL CLIP</p> <p>ATTACH DECK PAN TO PURLIN @ LAST RIB W/(2) 3/8"Ø A304 S.S. BOLTS</p> <p>SL-316 DECK</p> <p>GUTTER END CAP MK. LCFANEC</p> <p>GUTTER</p>		<p>.032 CHICK-FIL-A BRONZE FASCIA BAND</p> <p>.032 CHICK-FIL-A BRONZE JOINT SLEEVE</p> <p>.032 CHICK-FIL-A BRONZE JOINT SLEEVE</p> <p>APPLY AFT 701 2" SEALER TAPE @ EACH FASCIA JOINT ON THE BACK OF PANEL</p> <p>.032 CHICK-FIL-A BRONZE JOINT SLEEVE</p> <p>.032 CHICK-FIL-A BRONZE FASCIA BAND</p> <p>ERECTOR NOTE: USE PANEL JOINT SLEEVES TO ALIGN FASCIA</p>		<p>INSTALLER NOTE: VERIFY LOCATION WITH G.C. BEFORE INSTALLING</p> <p>14 GA. HAT SECTION MK. LSH2, ATTACH TO DECK W/(2) #14x1"TEKS TO EACH DECK RIB</p> <p>14 GA. HAT SECTION MK. LCFAHSF, ATTACH TO HAT SECTION W/(4) #14x1" TEKS</p> <p>14 GA. HAT SECTION MK. LSH2</p> <p>14 GA. HAT SECTION MK. LCFAHSF</p> <p>SL-316 DECK</p> <p>FANS (FURNISHED & INSTALLED BY OTHERS INCLUDING SEALANT AT ALL THREAD BOLTS TO BRACKET)</p>		<p>3/8"Ø BOLT</p> <p>7/16"Ø HOLE</p> <p>1/4" PL (TOP & BOTTOM)</p> <p>2" TUBE</p> <p>SEAL W/ Soudaseal BETWEEN HAT SECTION AND DECK, AND BETWEEN DECK TO TUBE</p> <p>14 GA. HAT SECTION MK. LSH2, ATTACH TO DECK W/(2) #14x1"TEKS TO EACH DECK RIB</p> <p>14 GA. HAT SECTION MK. LCFAHSH, ATTACH TO HAT SECTION W/(4) #14x1" TEKS</p> <p>SL-316 DECK</p> <p>HEATER (BY OTHERS)</p> <p>INSTALLER NOTE: VERIFY LOCATION WITH G.C. BEFORE INSTALLING</p>	
LI	DETAIL AT SIDES OF CANOPY	L5	DETAIL AT FASCIA CORNER AND SPLICE	L9	SECTION AT FAN SUPPORT	LI3	SECTION AT HEATER SUPPORT
N.T.S.	AI-E2, FI-EI	N.T.S.	FI-EI	N.T.S.	FI-EI	N.T.S.	FI-EI
<p>L1 1/2X 1 1/2X 1/8" MK. LCFABA1 & LBA1 FASTEN TO PURLIN W/(2) 1/4-20 x 1 1/2" BMFK #5 TEKS</p> <p>PURLIN</p> <p>PANEL CLIP</p> <p>ATTACH TO DECK W/(2) #14x1" TEKS PER CONNECTION</p> <p>SL-316 DECK</p>				<p>TUBE COLUMN RE: FI-E1 FOR SIZE AND MARK</p> <p>TOOL SEALANT TO ENSURE PROPER BOND TO COLUMN AND DECK</p> <p>BEND DECK PANEL UP MIN 2" FROM COLUMN CUTOUT AND ATTACH TO SIDES OF COLUMN WITH 1/4-20 x 1 1/2" BMFK #5 TEKS (2) PER LIP</p> <p>CLEAN SURFACE ON COLUMNS WITH ALCOHOL PRIOR TO APPLYING SEALANT.</p> <p>DECK RIB</p>		<p>PURLIN</p> <p>PANEL CLIP</p> <p>SL-316 DECK</p> <p>SL-316 PANEL</p> <p>INSTALLER NOTE: CLAMP DECK RIBS NEAR FACE OF PAN PRIOR TO INSTALLING DECK CLIP SCREW</p> <p>SECURE THE PANEL CLIP BY THE USE OF A #14x7/8" LAP TEKS (BEHIND THE CLIP AND THROUGH THE LEG OF THE PANEL) TO PREVENT THE CLIP FROM RELEASING FROM THE PURLIN.</p>	
FI	SECTION AT PURLIN BRIDGING	F5	NOT USED	F9	DETAIL AT DECK SUPPORT	FI4	DETAIL AT DECK CLIP
N.T.S.	FI-EI			1 1/2" = 1'-0"	FI-E2	N.T.S.	AI-E2, FI-E2
<p>GUTTER MK. LCFANG1 ATTACH TO BACK OF FASCIA W/(2)#14x7/8 LAP TEKS W/NEO AT EVERY OUTRIGGER. W/CONTINUOUS Soudaseal BEAD</p> <p>18 GA. OUTRIGGER MK. LCFAORS ATTACH TO DECK RIB W/(3) #14x1" TEKS</p> <p>SL-316 DECK</p> <p>FASCIA ASSEMBLY (SEE FASCIA PLAN FOR MK.'S) ATTACH TO DECK CHANNEL MK. LCFADC1 & OUTRIGGER MK. LCFAORS W/(2) #14x7/8" LAP TEKS</p> <p>GUTTER MK. LCFANG1 SMOOTH H.G WHITE ATTACH TO DECK W/(ONE) #14x7/8" LAP TEKS W/NEO @ 8" O.C. AND TO EACH OUTRIGGER W/POP RIVET 1/8"Ø S.S.</p> <p>2"</p>		<p>OVERFLOW DIVERTER MK. LCFADVT ATTACH TO DECK W/(3) #14x1" TEKS</p> <p>DECK CHANNEL MK. LCFADC1 ATTACH TO DECK W/#14x1" TEKS</p> <p>GUTTER END CAP MK. LCFACEL/R SMOOTH H.G WHITE ATTACH TO GUTTER W/(2) POP RIVETS 1/8"Ø S.S. SEAL W/Soudaseal</p> <p>LIGHT DIVERTER</p> <p>4"Ø DROP-OUT 2" TALL TURN UPSIDE DOWN W/(8) POP RIVETS 1/8"Ø S.S. TO GUTTER</p> <p>DRAIN COVER MK. LSC11 ATTACH TO GUTTER W/(8) #8x3/4" MOD TRUSS S.S. PLAIN</p> <p>GUTTER</p>		<p>DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) + WEIGHT OF STRUCTURAL COMPONENTS LIVE LOAD = 20 p.s.f. SNOW LOAD = 20 p.s.f. WIND LOAD V_{ULT} = 116 m.p.h. EXP. C WIND V_{ASD} = 90 m.p.h. EXP. C BLDG CODE = MISSOURI BUILDING CODE 2018 ADOPTING 2018 INTERNATIONAL BUILDING CODE EQUIVALENT LATERAL FORCE PROCEDURE LATERAL FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEM-ORDINARY STEEL MOMENT FRAME P_f = 20 p.s.f. C_e = 1.2 C_t = 1.2 I_s = 1.0 W = DRIFT LOADS NOT CONSIDERED P_d = DRIFT LOADS NOT CONSIDERED SITE CLASS = D S_s (0.2) = 0.099 S₁ (1.0) = 0.068 SDS = 0.11 SD₁ = 0.11 F_a = 1.60 F_v = 2.40 R = 1.25 IMPORTANCE FACTOR = 1.0 RISK CATEGORY = II SEISMIC DESIGN CATEGORY = D CS = 0.084 CONSTRUCTION TYPE = IIB OCCUPANCY CATEGORY = A2 TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 0.84 KIPS</p>			
AI	DETAIL AT END OF CANOPY	A9	DETAIL OF OVERFLOW DRAIN	AI4	DESIGN LOADS		
N.T.S.	FI-EI, FI-E2	N.T.S.	FI-EI	N.T.S.			

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:
 Mark Date By

Seal

SEP 07 2023

C.O.A. 2001015838

120 FAIRVIEW
ARLINGTON, TX. 76010
(817) 261-9116

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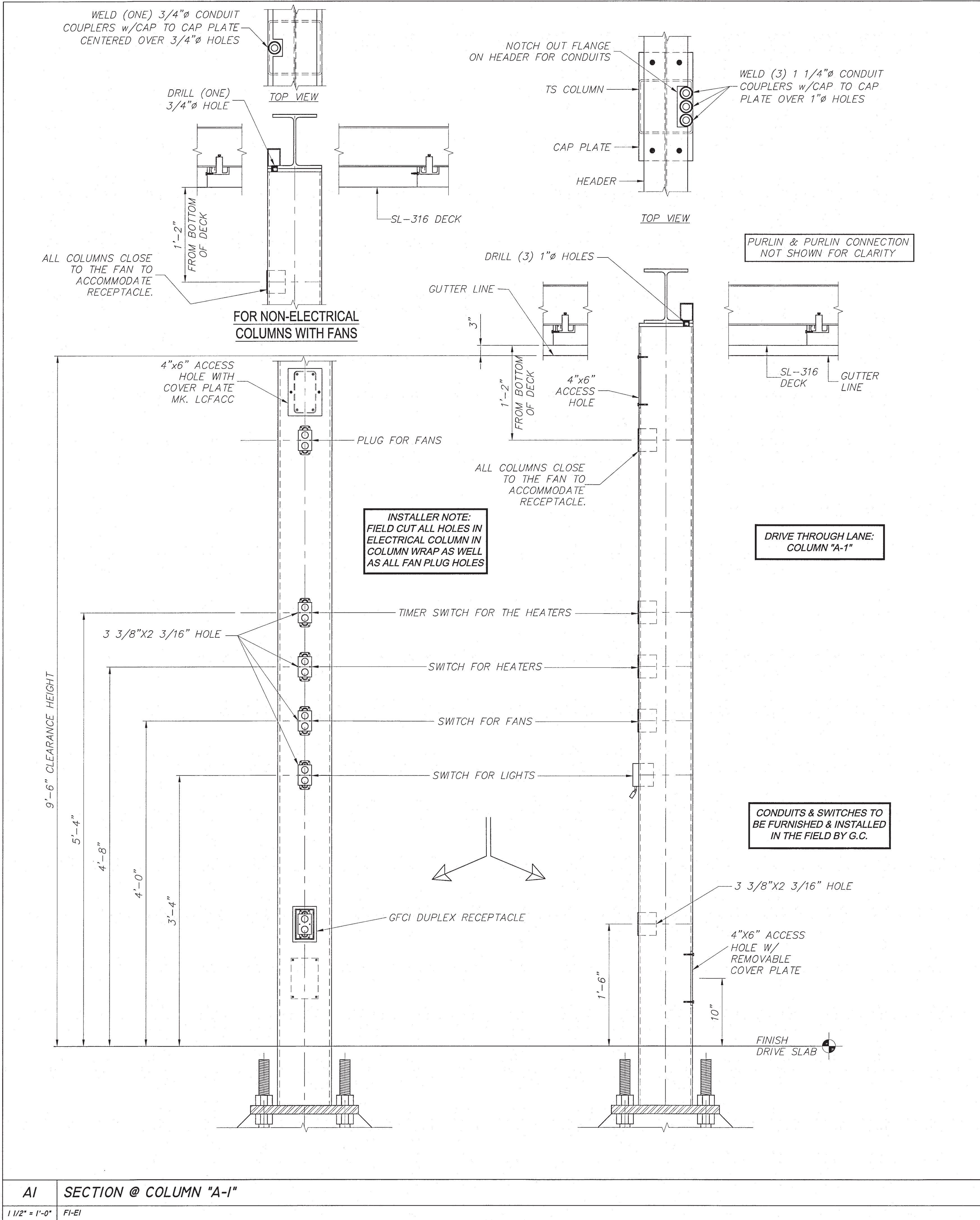
STORE
 Chick-fil-A #02859
 690 NW BLUE
 PARKWAY,
 LEE'S SUMMIT,MO
 64086

SHEET TITLE
 CANOPY SECTIONS

25'-10 1/2" X 54'-1 1/2"

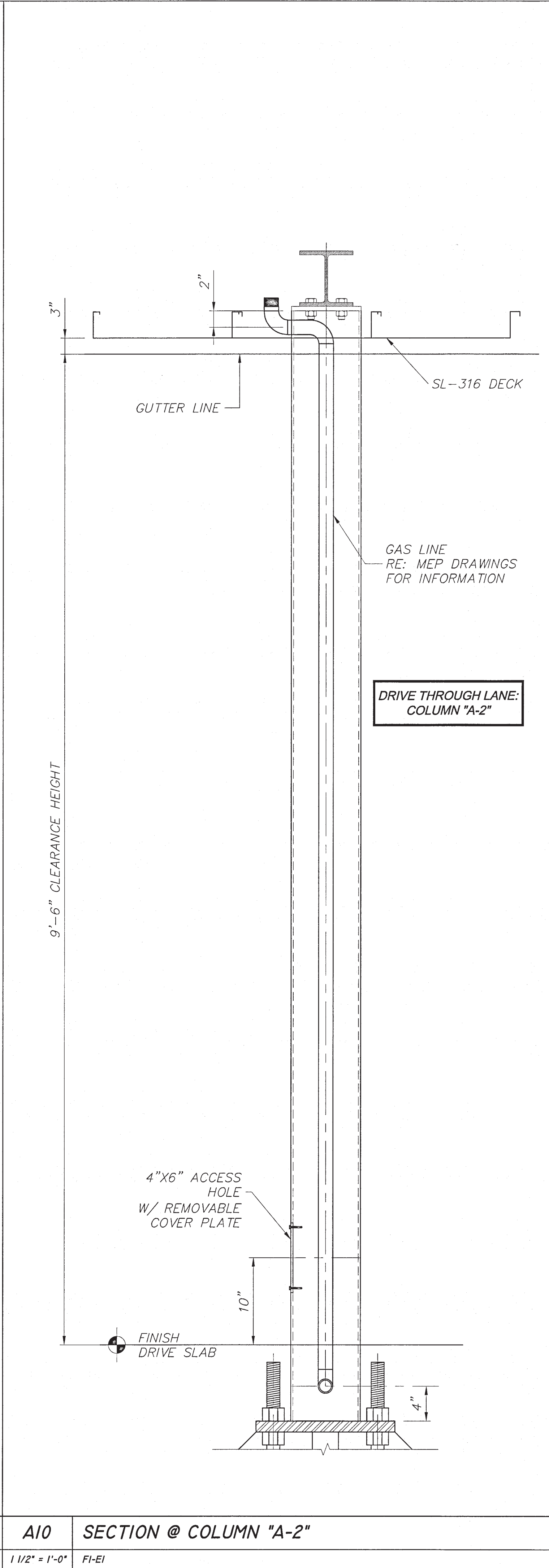
Job No.: LSC: 75966
 Store : 02859
 Date : 8.30.23
 Drawn By : RED
 Checked By: RM

Sheet
 F2FC-5
 E3 OF 4



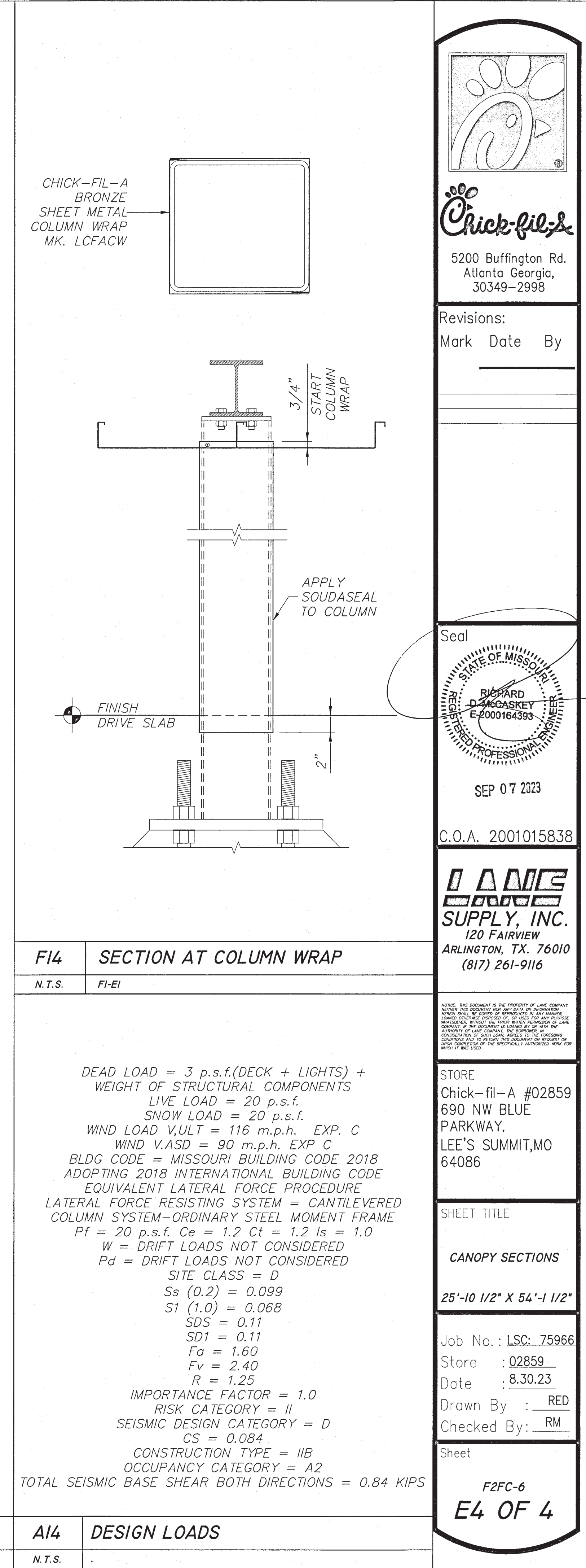
AI SECTION @ COLUMN "A-1"

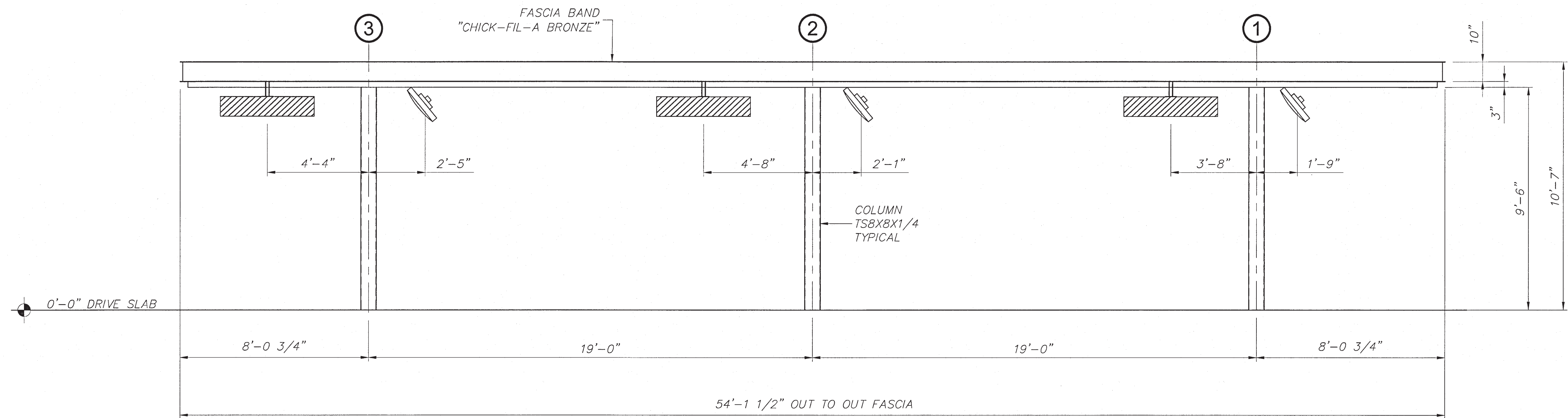
1 1/2" = 1'-0" FI-EI



A10 SECTION @ COLUMN "A-2"

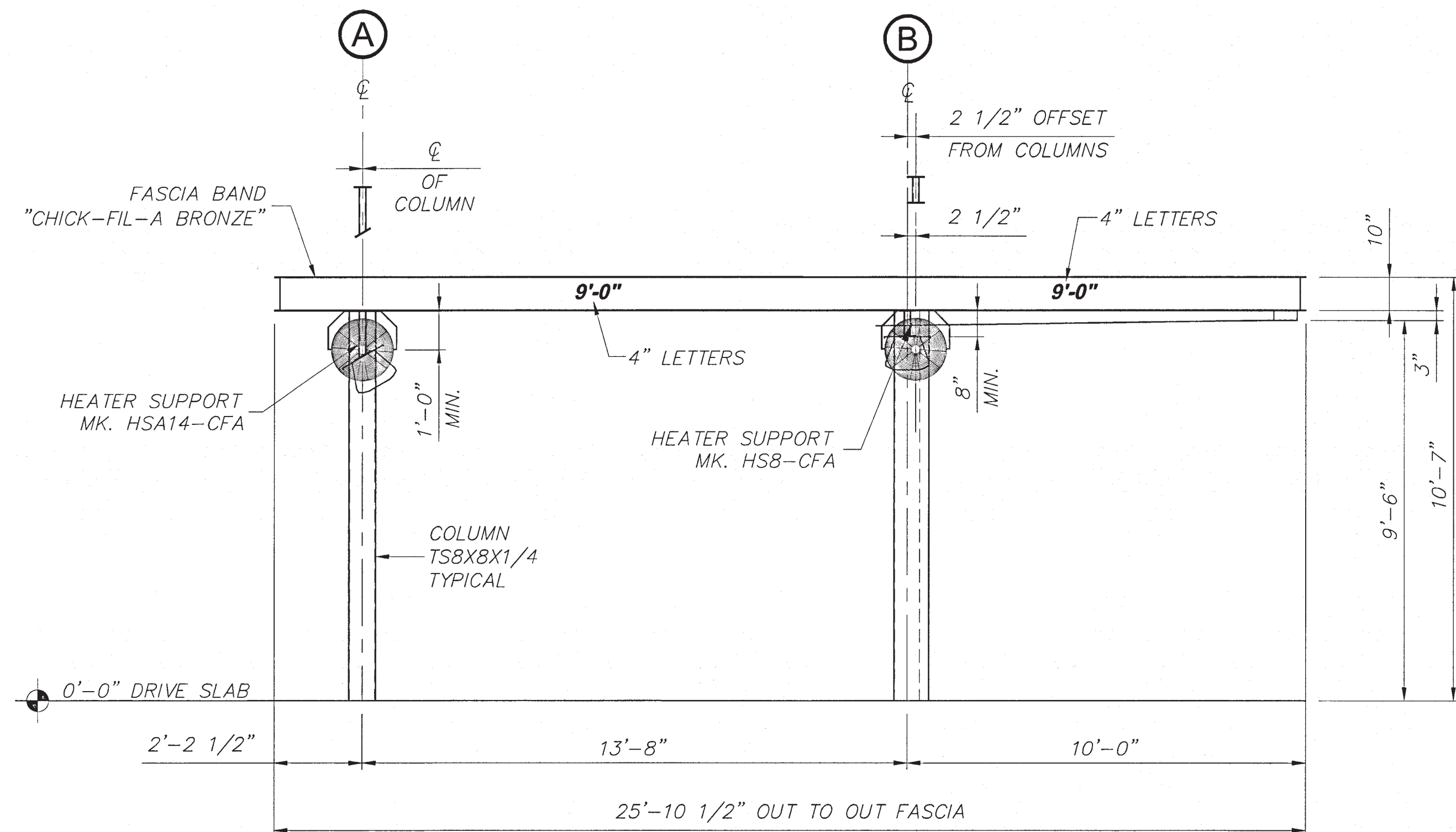
1 1/2" = 1'-0" FI-EI





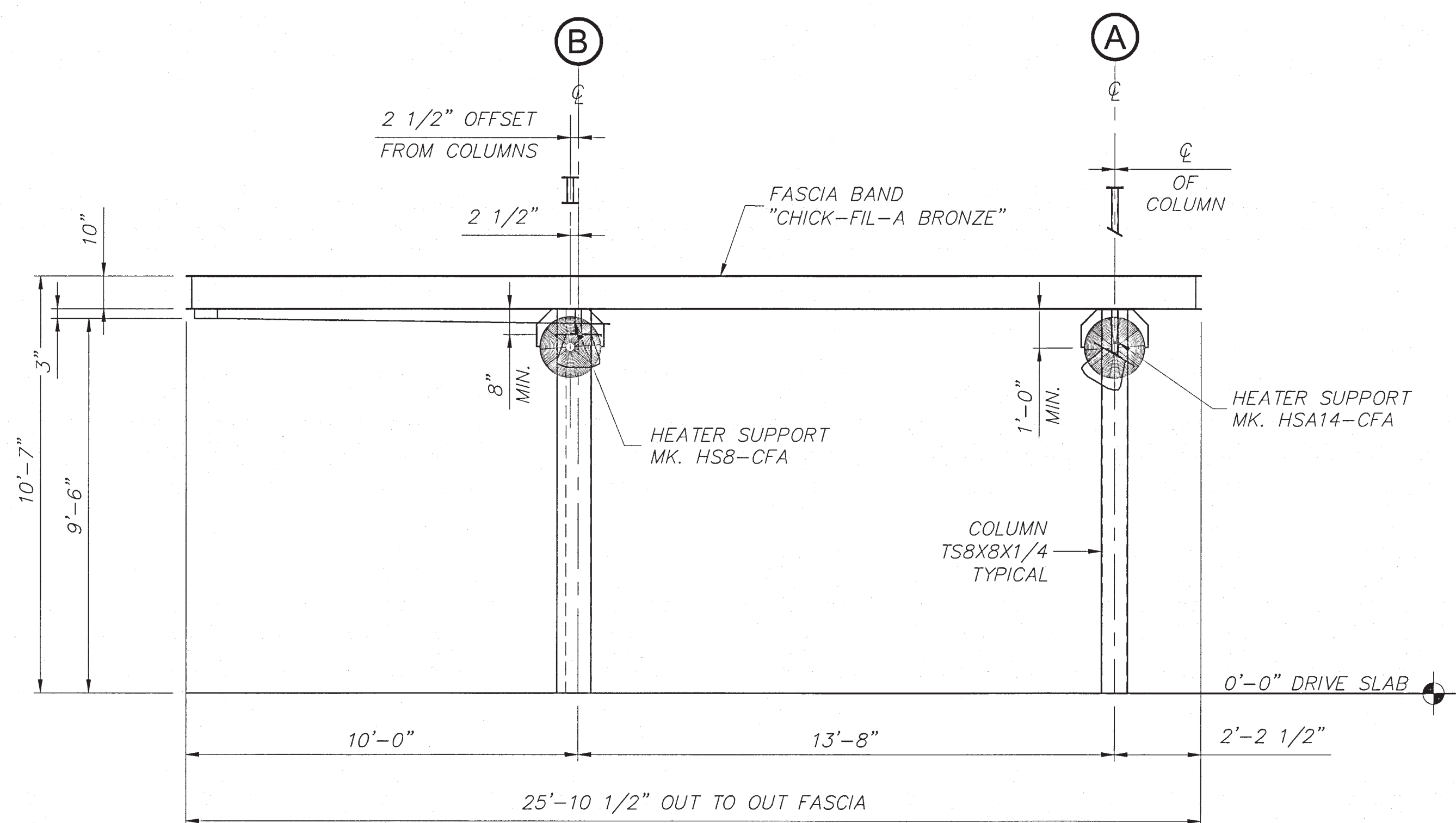
HI CANOPY SIDE ELEVATION

3/8" = 1'-0" FI-ABI, FI-EI, FI-LLI



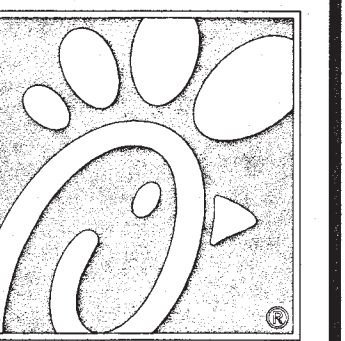
AI CANOPY END ELEVATION

3/8" = 1'-0" FI-ABI, FI-EI, FI-LLI



A9 CANOPY END ELEVATION

3/8" = 1'-0" FI-ABI, FI-EI, FI-LLI



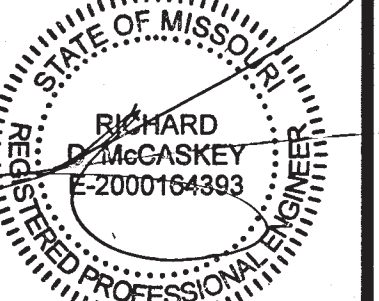
Chick-fil-A

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:

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Seal



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120 FAIRVIEW
ARLINGTON, TX. 76010
(817) 261-9116

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STORE
Chick-fil-A #02859
690 NW BLUE
PARKWAY,
LEE'S SUMMIT, MO
64086

SHEET TITLE

CANOPY ELEVATION
PLAN

25'-10 1/2" X 54'-1 1/2"

Job No.: LSC: 75966

Store : 02859

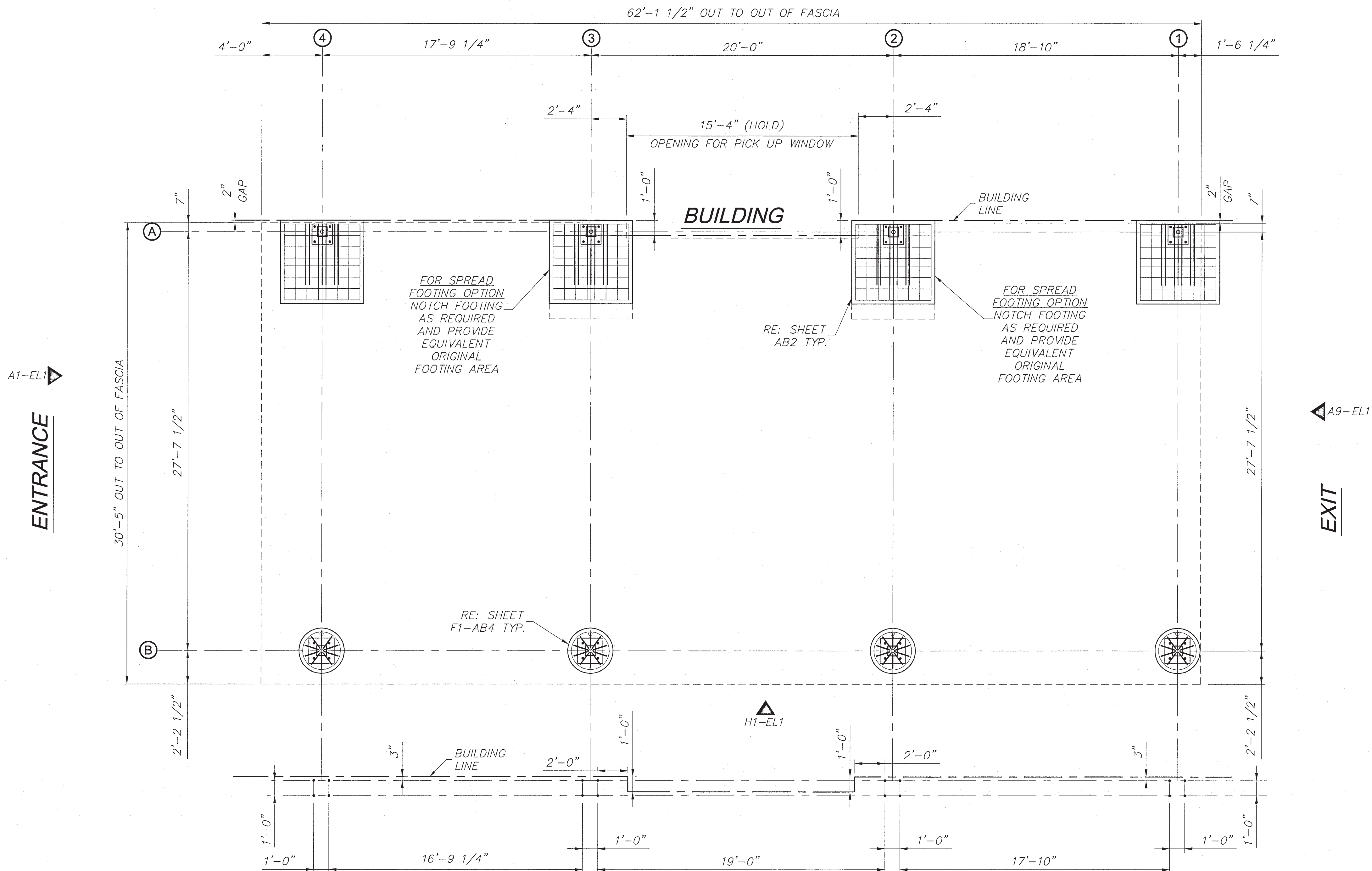
Date : 8.30.23

Drawn By : RED

Checked By: RM

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F2FC-7
ELI OF 1



FI COLUMN AND FOOTING LOCATIONS

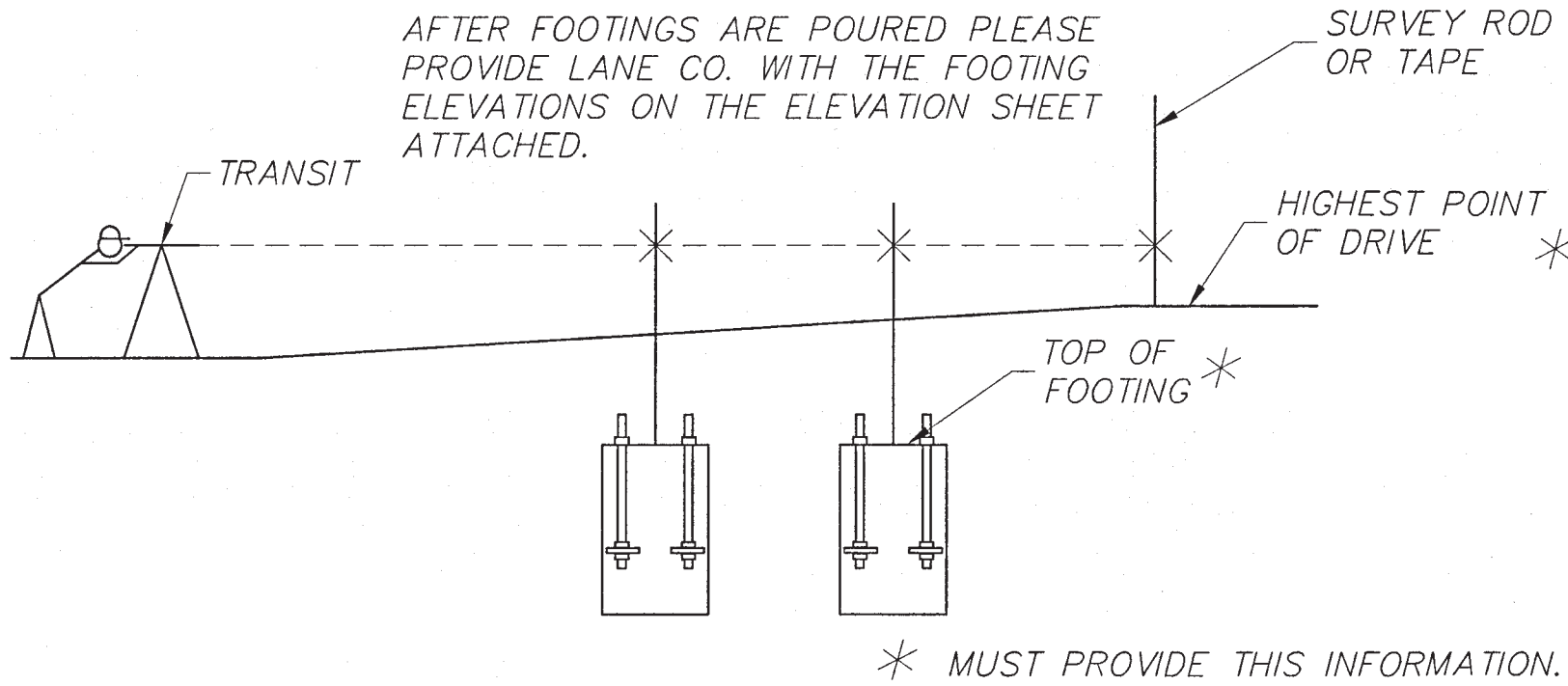
1/4" = 1'-0"

- ALL FOUNDATION WORK BY OTHERS AND SUBJECT TO LOCAL APPROVAL.
- THE FOUNDATION DESIGN IS BASED UPON SECTION 1807.3.2.2-IBC 2018 EDITION. THE DESIGN CRITERIA SELECTED ASSUMES: SITE CLASS D MATERIAL OR BETTER, SOIL BEARING CAPACITY OF 1,500 p.s.f. AND A PASSIVE SOIL PRESSURE OF 100 p.s.f. PER FOOT OF DEPTH.
- DRILLED SHAFT FOOTINGS SHALL BE INSTALLED PER ACI STD. 336.
- CONCRETE DESIGN AND CONSTRUCTION SHALL CONFORM TO ACI STANDARD 318-14 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE."
- MINIMUM COMPRESSIVE STRENGTH OF CONCRETE (F'C) AT THE END OF 28 DAYS SHALL BE 3,000 PSI MIN.
- REINFORCING STEEL SHALL BE GRADE 60 AND CONFORM TO ASTM A615 LATEST REVISION.
- DETAILING, FABRICATION AND PLACEMENT OF REINFORCING BARS SHALL COMPLY WITH ACI 315, ACI 318 AND CRSI STANDARDS.
- ANCHOR BOLTS SHALL CONFORM TO ASTM F1554-GR36.
- LANE IS NOT RESPONSIBLE FOR FOOTING POURED PRIOR TO PERMITTING.
- FOOTINGS ARE DESIGNED TO BE CONSTRAINED AT THE TOP BY A 6" SLAB. IF THEY ARE NOT, PLEASE NOTIFY LANE SUPPLY CO.
- POUR FOOTINGS TO SAME TOP ELEVATION.
- USE MASTER FLOW 928 NON-SHRINK GROUT OR EQUIVALENT F'm=5000 p.s.i.
- G.C. TO ENSURE THAT FOOTINGS DO NOT INTERFERE WITH UNDERGROUND UTILITIES

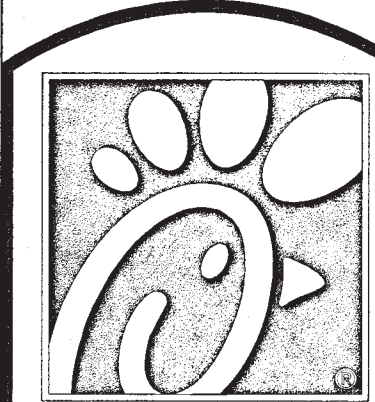
- TOP OF ALL CANOPY FOOTINGS ARE TO BE POURED A MINIMUM OF 24" BELOW FINISHED GRADE OR AS REQUIRED BY LOCAL CODES AND ORDINANCES.
- IT IS THE OWNERS RESPONSIBILITY TO CONVEY TO ALL CONTRACTORS THAT IT IS THEIR RESPONSIBILITY TO INSURE THAT THE SITE IS PROPERLY EXCAVATED AND GRADED. DURING CONCRETE FORMING PRIOR TO AND AFTER THE POUR, THE CONCRETE SHOULD BE CHECKED FOR PROPER ELEVATION, SQUARE AND CORRECT DIMENSIONS.
- MEASUREMENTS FOR ANCHOR BOLTS ARE EXACT AND SHOULD BE RECHECKED TO INSURE PROPER LOCATION.
- CORRECTION OF LOCATION, OF ELEVATION AND OF DIMENSIONAL ERRORS MUST BE MADE PRIOR TO THE ARRIVAL OF THE ERECTION CREW AND PRIOR TO THE ERECTION OF THE STRUCTURE.
- AFTER THE FORMS HAVE BEEN REMOVED, ALL TRENCHES, HOLES AND UNEVEN SITE CONDITIONS MUST BE LEVELED TO INSURE A SAFE WORKING AND ACCESS AREA ACCEPTABLE TO LOCAL, STATE, FEDERAL AND OSHA AGENCIES.

VERY IMPORTANT:

AFTER FOOTINGS ARE POURED PLEASE PROVIDE LANE CO. WITH THE FOOTING ELEVATIONS ON THE ELEVATION SHEET ATTACHED.



DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) +
WEIGHT OF STRUCTURAL COMPONENTS
LIVE LOAD = 20 p.s.f.
SNOW LOAD = 20 p.s.f.
WIND LOAD V.U.L.T. = 116 m.p.h. EXP. C
WIND V.A.S.D. = 90 m.p.h. EXP. C
BLDG CODE = MISSOURI BUILDING CODE 2018
ADOPTING 2018 INTERNATIONAL BUILDING CODE
EQUIVALENT LATERAL FORCE PROCEDURE
LATERAL FORCE RESISTING SYSTEM =
CANTILEVERED COLUMN SYSTEM-
ORDINARY STEEL MOMENT FRAME
Pf = 20 p.s.f.
Ce = 1.2
Ct = 1.2
Is = 1.0
W = 4.92
Pd = 20.44
SITE CLASS = D
Ss (0.2) = 0.099
S1 (1.0) = 0.068
SDS = 0.11
SD1 = 0.11
Fa = 1.60
Fv = 2.40
R = 1.25
IMPORTANCE FACTOR = 1.0
RISK CATEGORY = II
SEISMIC DESIGN CATEGORY = D
CS = 0.084
CONSTRUCTION TYPE = IIB
OCCUPANCY CATEGORY = A2
TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 1.43 KIPS

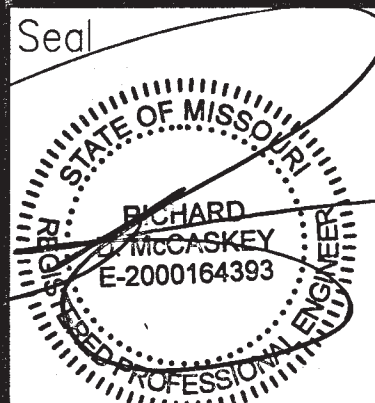


Chick-fil-A

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:

Mark Date By
5 7/15/24 IP
ADDED OFFSET
FOOTING



JUL 17 2024

C.O.A. 2001015838

LANE
SUPPLY, INC.
120 FAIRVIEW
ARLINGTON, TX. 76010
(817) 261-9116

STORE
Chick-fil-A #02859
690 NW BLUE PKWY
LEE'S SUMMIT, MO
64086

SHEET TITLE
CANOPY FOOTING
LOCATIONS

30-5" X 62'-1 1/2"

Job No.: LSC: 75967
Store : 02859
Date : 09.01.23
Drawn By : KLM
Checked By: RM

Sheet
OMD-I
ABI OF 4

AI FOUNDATION NOTES

N.T.S.

A5 GENERAL NOTES

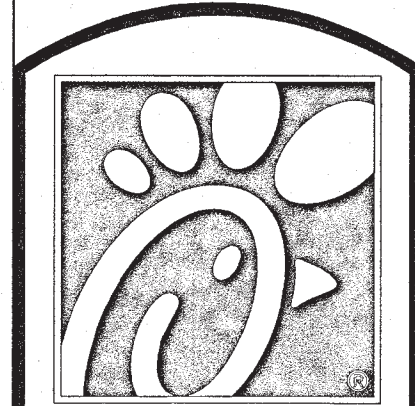
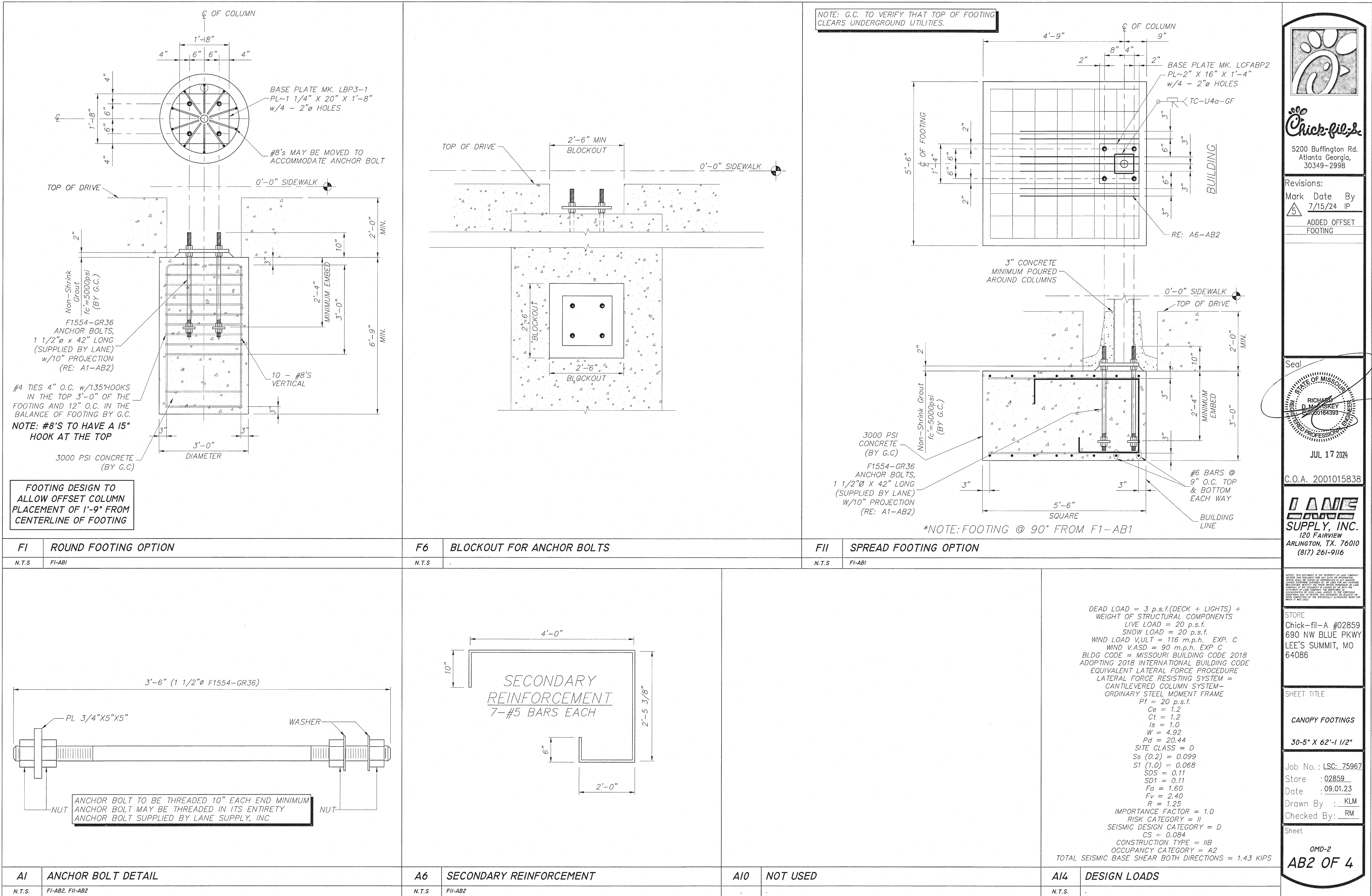
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A9 FOOTING ELEVATIONS

N.T.S.

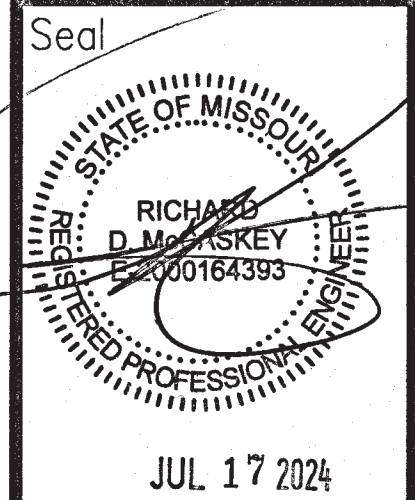
A14 DESIGN LOADS

N.T.S.



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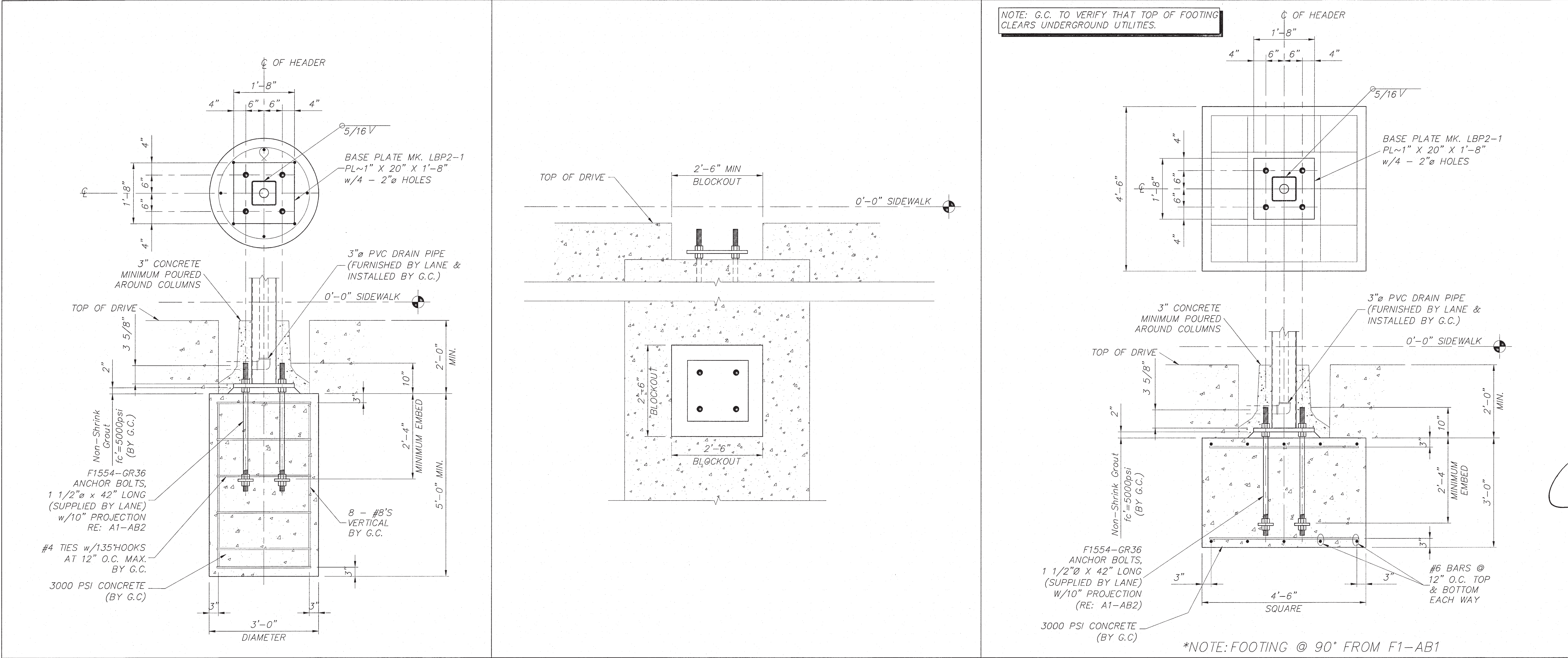
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7/15/24 IP
ADDED OFFSET
FOOTING



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ARLINGTON, TX. 76010
(817) 261-9116

STORE
Chick-fil-A #02859
690 NW BLUE PKWY
LEE'S SUMMIT, MO
64086

SHEET TITLE
CANOPY FOOTINGS
30-5' X 62'-1 1/2"
Job No.: LSC: 75967
Store : 02859
Date : 09.01.23
Drawn By : KLM
Checked By: RM
Sheet
OMD-2
AB2 OF 4



FI	ROUND FOOTING OPTION	F6	BLOCKOUT FOR ANCHOR BOLTS	FII	SPREAD FOOTING OPTION
N.T.S.	FI-ABI	N.T.S.		N.T.S.	FI-ABI

DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) +
WEIGHT OF STRUCTURAL COMPONENTS
LIVE LOAD = 20 p.s.f.
SNOW LOAD = 20 p.s.f.
WIND LOAD V_{ULT} = 116 m.p.h. EXP. C
WIND V_{ASD} = 90 m.p.h. EXP. C
BLDG CODE = MISSOURI BUILDING CODE 2018
ADOPTING 2018 INTERNATIONAL BUILDING CODE
EQUIVALENT LATERAL FORCE PROCEDURE
LATERAL FORCE RESISTING SYSTEM =
CANTILEVERED COLUMN SYSTEM-
ORDINARY STEEL MOMENT FRAME
P_f = 20 p.s.f.
C_e = 1.2
C_t = 1.2
I_s = 1.0
W = 4.92
P_d = 20.44
SITE CLASS = D
S_s (0.2) = 0.099
S₁ (1.0) = 0.068
SDS = 0.11
SD1 = 0.11
F_a = 1.60
F_v = 2.40
R = 1.25
IMPORTANCE FACTOR = 1.0
RISK CATEGORY = II
SEISMIC DESIGN CATEGORY = D
CS = 0.084
CONSTRUCTION TYPE = IIB
OCCUPANCY CATEGORY = A2
TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 1.43 KIPS

ANCHOR BOLT TO BE THREADED 10" EACH END MINIMUM
ANCHOR BOLT MAY BE THREADED IN ITS ENTIRETY
ANCHOR BOLT SUPPLIED BY LANE SUPPLY, INC

AI	ANCHOR BOLT DETAIL	A6	NOT USED	A10	NOT USED	A14	DESIGN LOADS
N.T.S.	FI-AB2, FII-AB2					N.T.S.	

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:
Mark Date By
7/15/24 IP
ADDED OFFSET
FOOTING

Seal
STATE OF MISSOURI
REGISTERED PROFESSIONAL ENGINEER
RICHARD D. KOCASKEY
No. E2000164393
JUL 17 2024
C.O.A. 2001015838

LANE
SUPPLY, INC.
120 FAIRVIEW
ARLINGTON, TX. 76010
(817) 261-9116

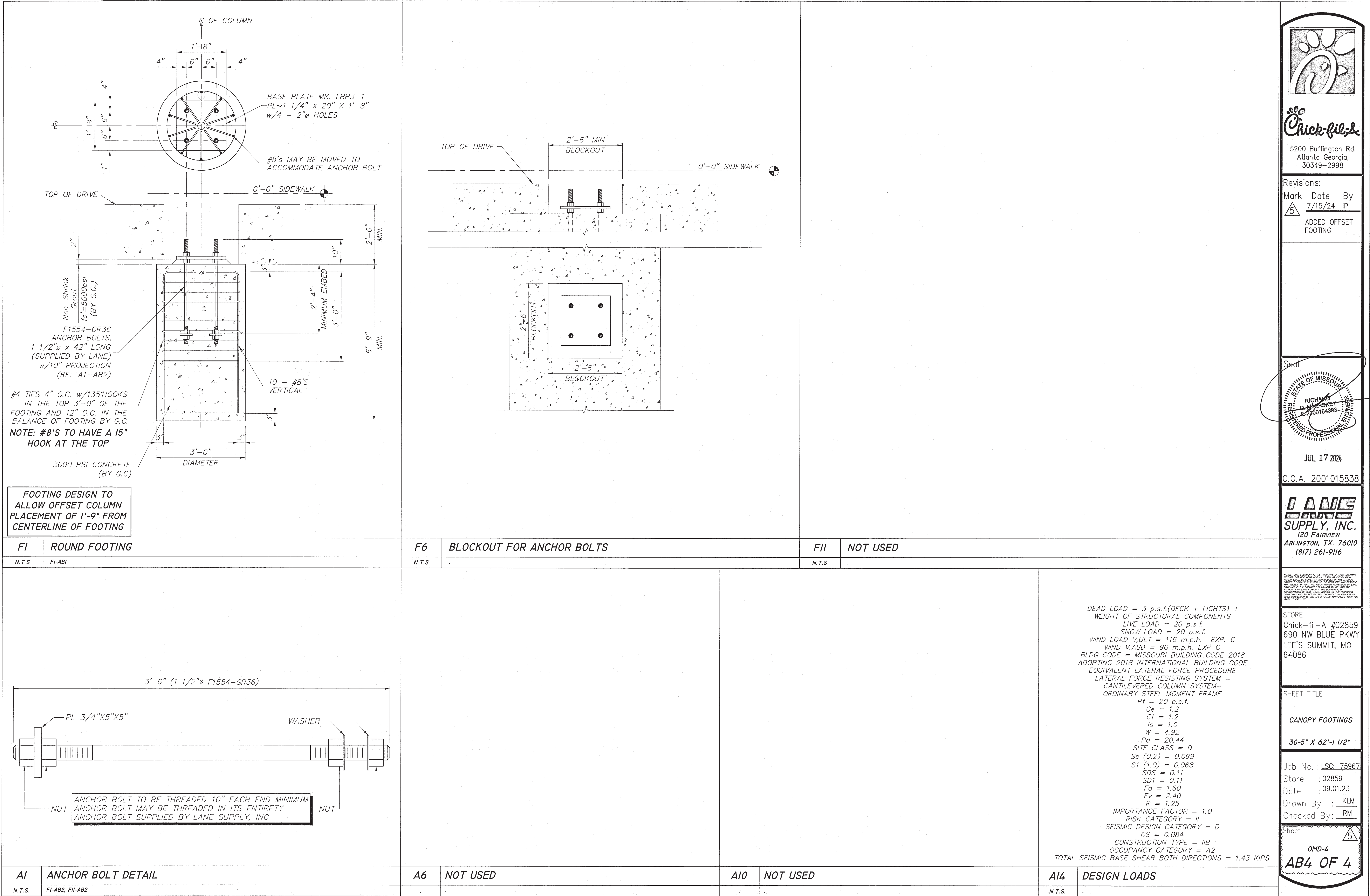
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STORE
Chick-fil-A #02859
690 NW BLUE PKWY
LEE'S SUMMIT, MO
64086

SHEET TITLE
CANOPY FOOTINGS
30-5' X 62'-1 1/2"

Job No.: LSC: 75967
Store : 02859
Date : 09.01.23
Drawn By : KLM
Checked By: RM

Sheet
OMD-3
AB3 OF 4



5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:

Mark	Date	By
5	7/15/24	IP

ADDED OFFSET FOOTING

Seal

JUL 17 2024

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ARLINGTON, TX. 76010
(817) 261-9116

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Chick-fil-A #02859
690 NW BLUE PKWY
LEE'S SUMMIT, MO
64086

SHEET TITLE

CANOPY FOOTINGS

30'-5" X 62'-1 1/2"

Job No.: LSC: 75967

Store : 02859

Date : 09.01.23

Drawn By : KLM

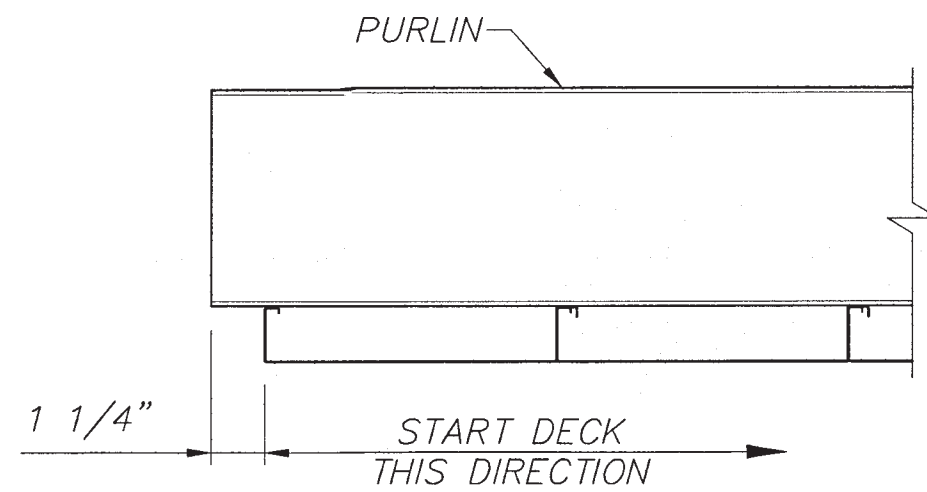
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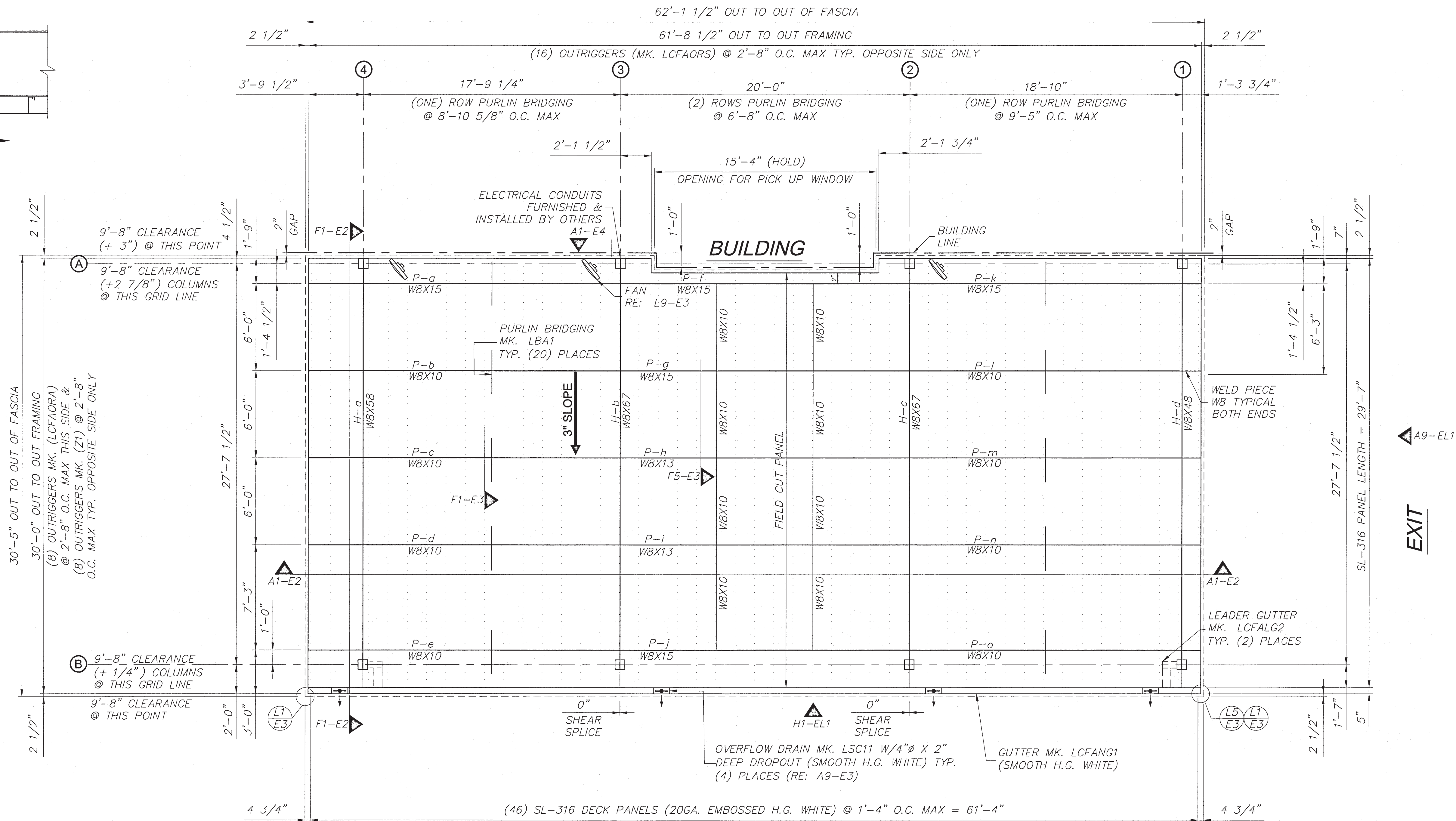
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OMD-4

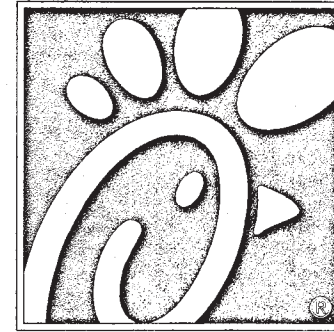
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ENTRANCE



EXIT



Chick-fil-A

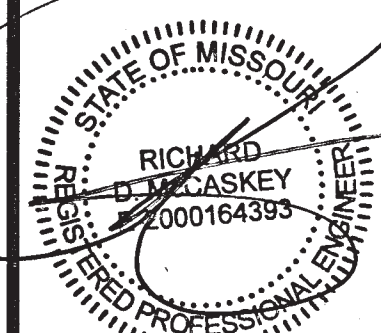
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5 7/15/24 IP

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FOOTING

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STORE
Chick-fil-A #02859
690 NW BLUE PKWY
LEE'S SUMMIT, MO
64086

SHEET TITLE

CANOPY FRAMING
PLAN

30'-5" X 62'-1 1/2"

Job No.: LSC- 75967

Store : 02859

Date : 09.01.23

Drawn By : KLM

Checked By: RM

Sheet

OMD-5
EI OF 4

FI CANOPY FRAMING LAYOUT

1/4" = 1'-0"

STRUCTURAL STEEL SHALL MEET THE AISC 2017 SPECIFICATION 15TH EDITION AND THE AISC CODE OF STANDARD PRACTICE, CURRENT VERSION.
COLUMNS TO BE ASTM A500, GRADE B
BOLTS TO BE ASTM A325 OR ASTM F1852 (A325-TC)
INSTALLATION OF BOLTS SHALL BE BROUGHT TO A SNUG TIGHT CONDITION-DEFINED AS THE CONDITION THAT EXISTS WHEN ALL OF THE PLIES IN A CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT BY THE BOLTS IN THE JOINT AND ALL OF THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

WIDE FLANGE BEAMS TO BE ASTM A992
ANGLES, & PLATES TO BE ASTM A36
REINFORCING STEEL TO BE ASTM 615, GRADE 60
DECK PANELS TO BE ASTM 653, GRADE C MINIMUM
WELD FILLER METALS SHALL MEET THE MINIMUM CHARPY V-NOTCH REQUIREMENT OF 20 FT-LB AT 0°F WELDING SHALL MEET THE REQUIREMENTS OF THE AWS FOR BUILDING CONSTRUCTION USING E70XX ELECTRODES
ALL STRUCTURAL STEEL TO BE PAINTED WITH ONE SHOP COAT PRIMER
CANOPY FABRICATOR SHALL BE AISC CERTIFIED
LANE SUPPLY INC. IS AN AISC CERTIFIED FABRICATOR (AISC # C-00022431)

1. REFERENCE SEALANT SCHEDULE FOR ALL APPLICATIONS
2. SEAL ALL JOINTS WITH A SMOOTH, CLEAN APPLICATION
3. APPLY CAULK CLEAR AROUND THE COLUMNS ON THE TOP SIDE OF THE DECK AFTER BOTTOM SIDE HAS BEEN CAULKED.
4. DECK PANELS AND TRIM WILL BE WIPED CLEAN AFTER INSTALLATION
5. ALL TRASH AND EXTRA MATERIALS WILL BE HAULED OFF JOBSITE
6. CHECK WITH GENERAL CONTRACTOR FOR DRAIN ORIENTATION
7. FURNISH & INSTALL LANE DESIGNED AND ENGINEERED "HUNG" DECK
8. FURNISH & INSTALL SUPPORT FRAMING FOR (3) FANS (FANS FURNISHED & INSTALLED BY OTHERS.)
9. FURNISH & INSTALL (6) LSI CRUS-CS-LED-LW-30-UE-WHT CANOPY DECK LIGHTS.
10. FURNISH & INSTALL SHEET METAL FASCIA "CHICK-FIL-A BRONZE".

SEALANT SCHEDULE		
SEALANT	COLOR	APPLICATION
Soudaseal FC	WHITE	DECK TO COLUMN @ BOTTOM
Soudaseal FC	WHITE	GUTTER JOINTS
Soudaseal FC	WHITE	DECK TO COLUMN @ TOP
Soudaseal FC	WHITE	SEAL @ OVERFLOW DROPOUTS
Soudaseal FC	WHITE	SEAL BOLTS @ HEATER SUPPORTS
Soudaseal FC	WHITE	DAM UP DECK @ DECK CLOSURE
Soudaseal FC	WHITE	SEAL FASCIA @ DECK CLOSURE

AI GENERAL NOTES

N.T.S.

A6 ERECTOR'S NOTES

N.T.S.

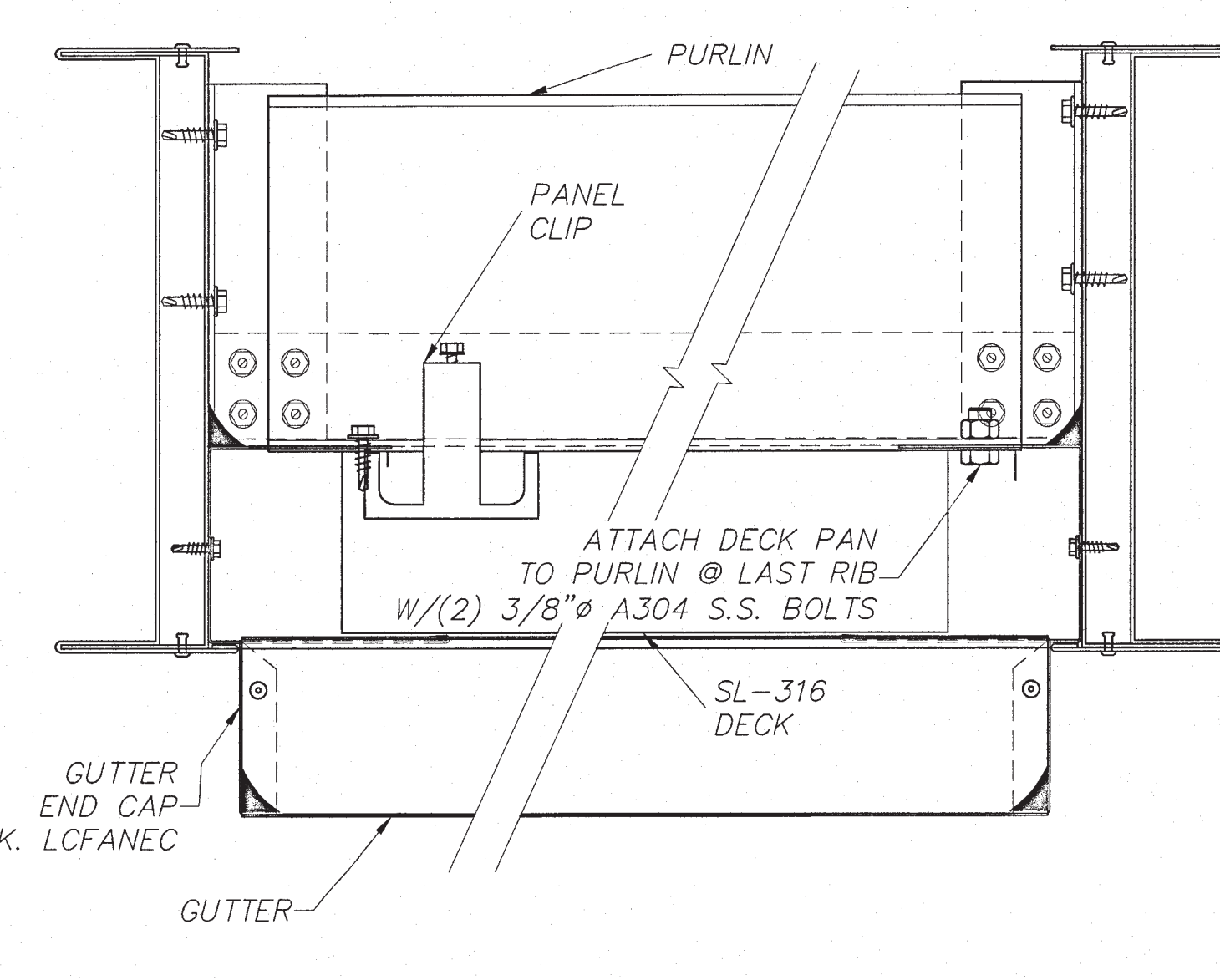
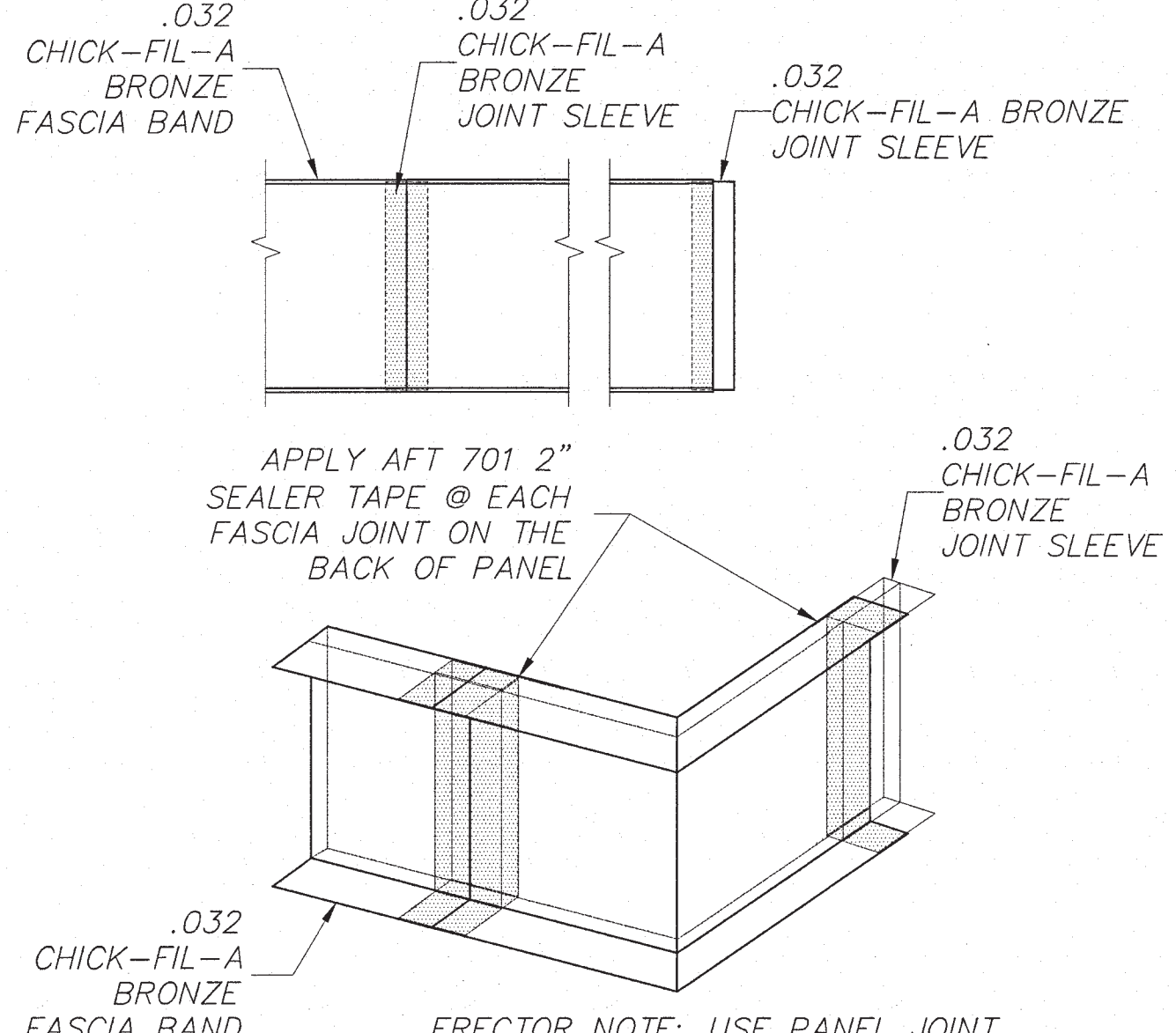
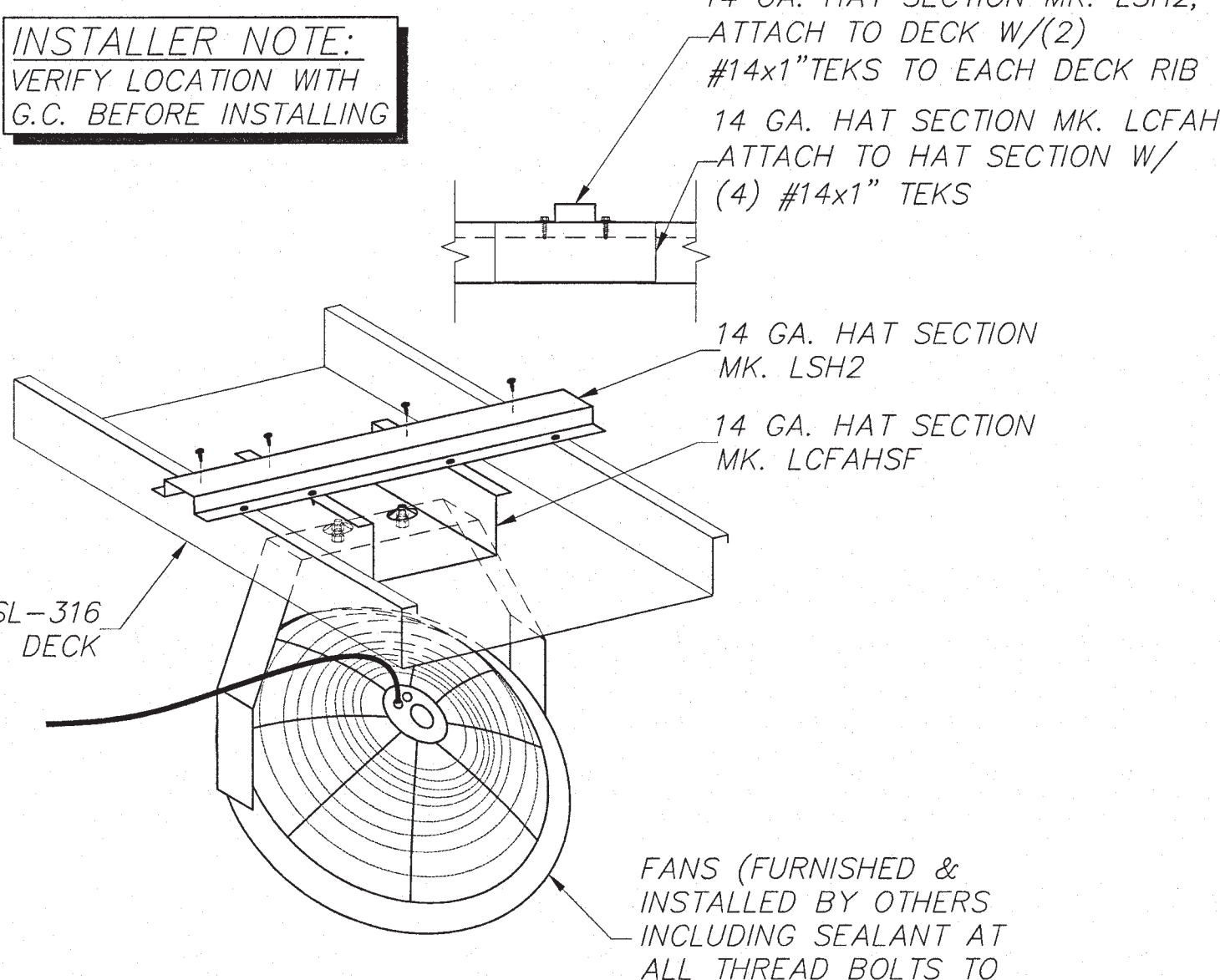
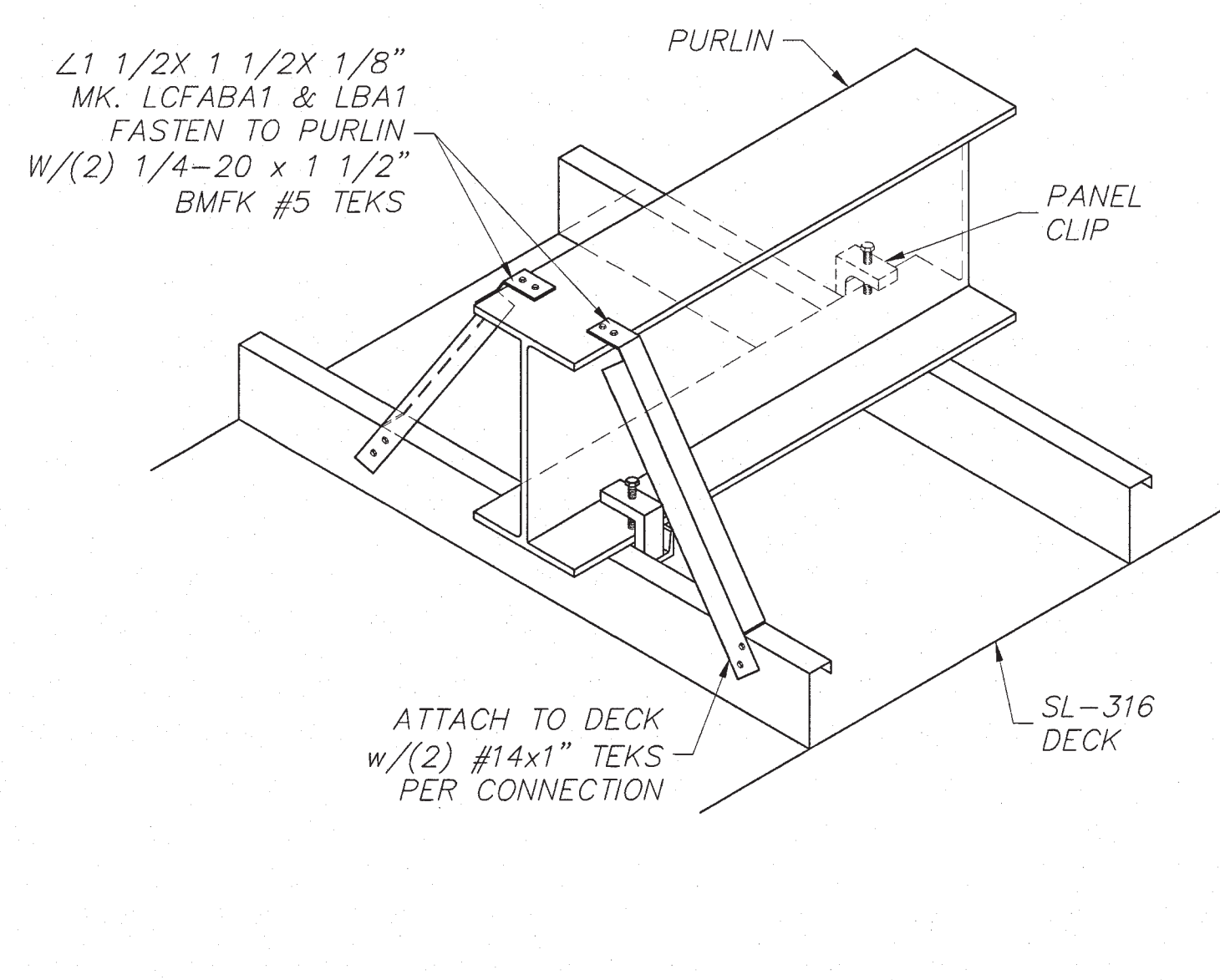
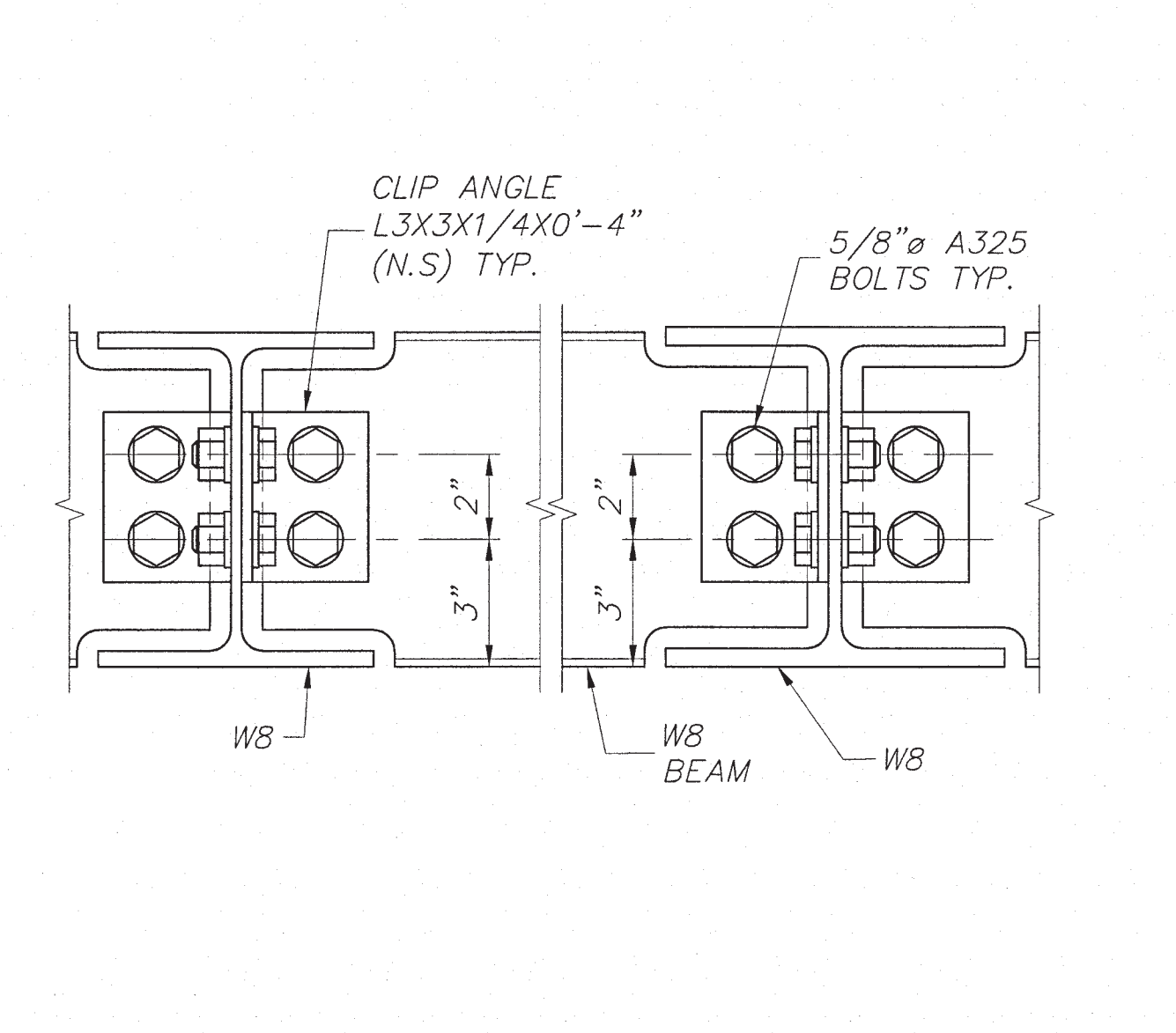
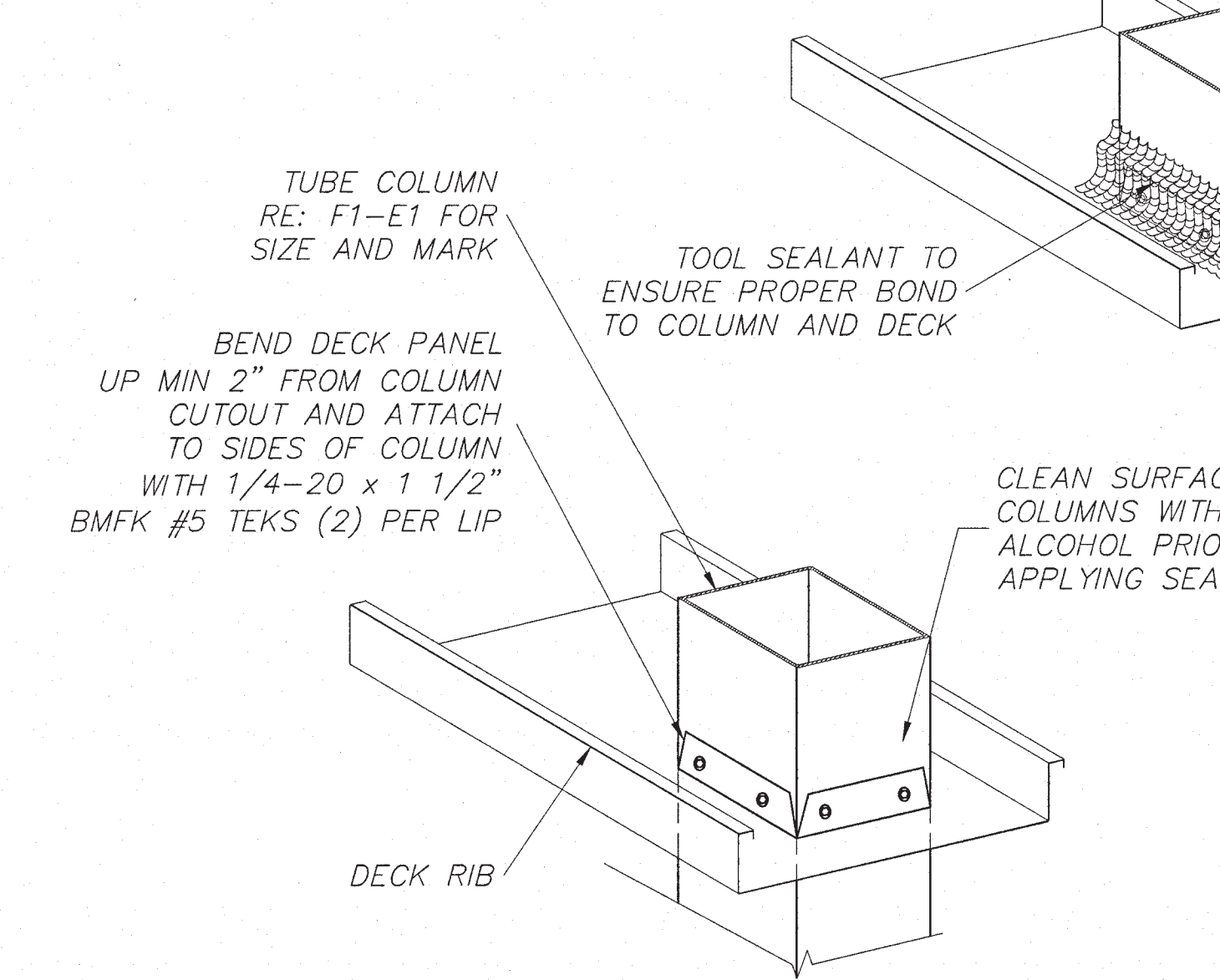
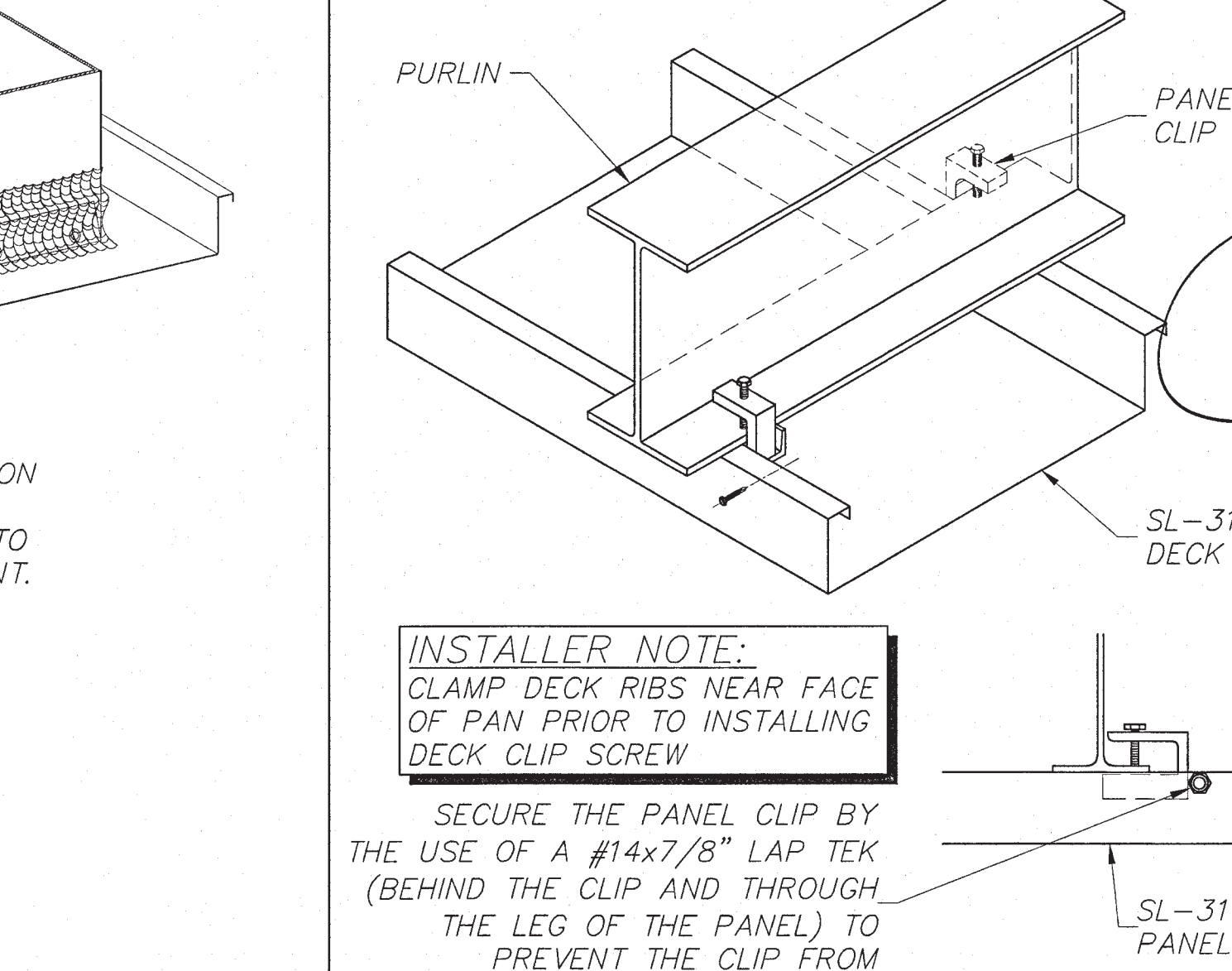
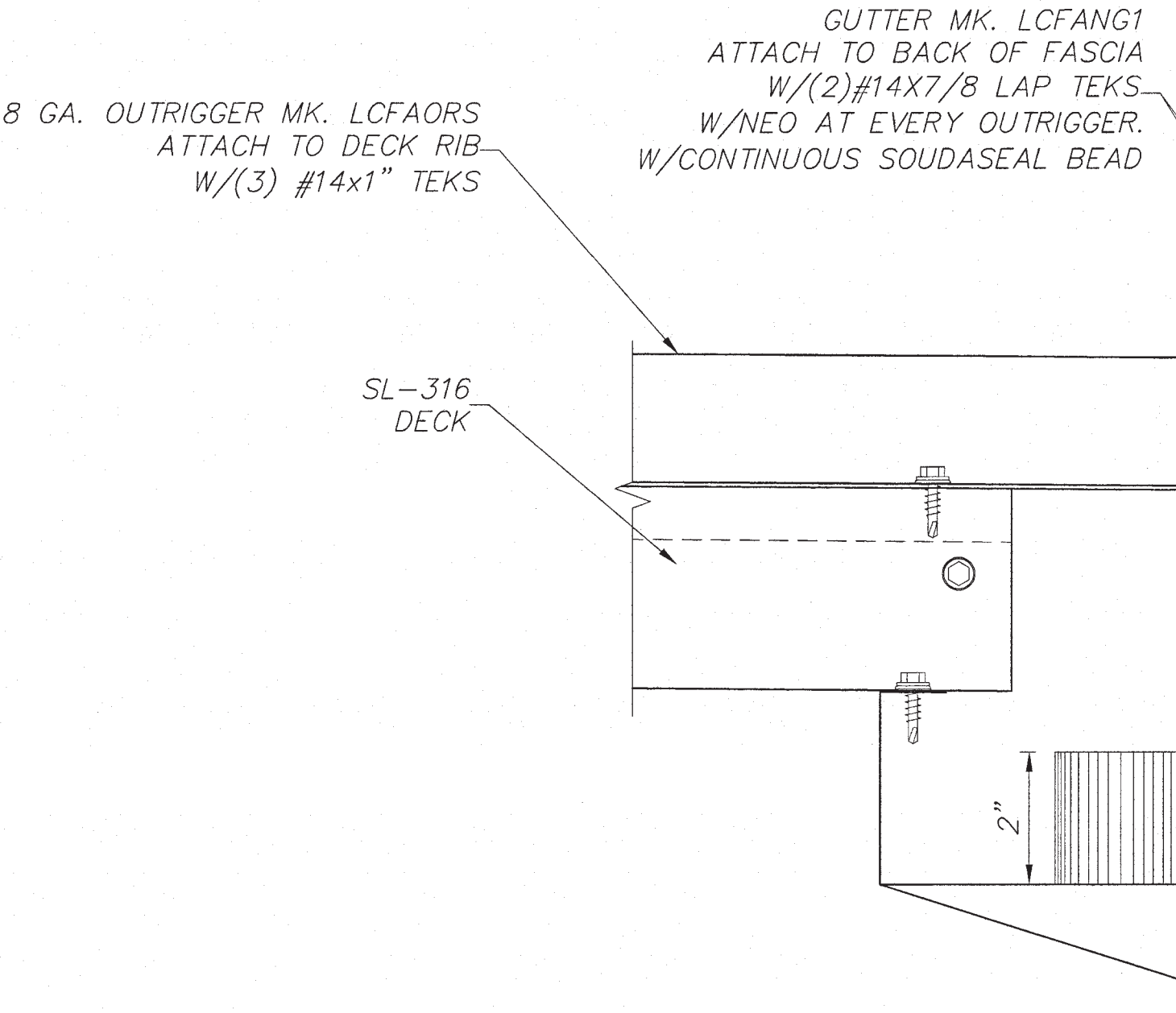
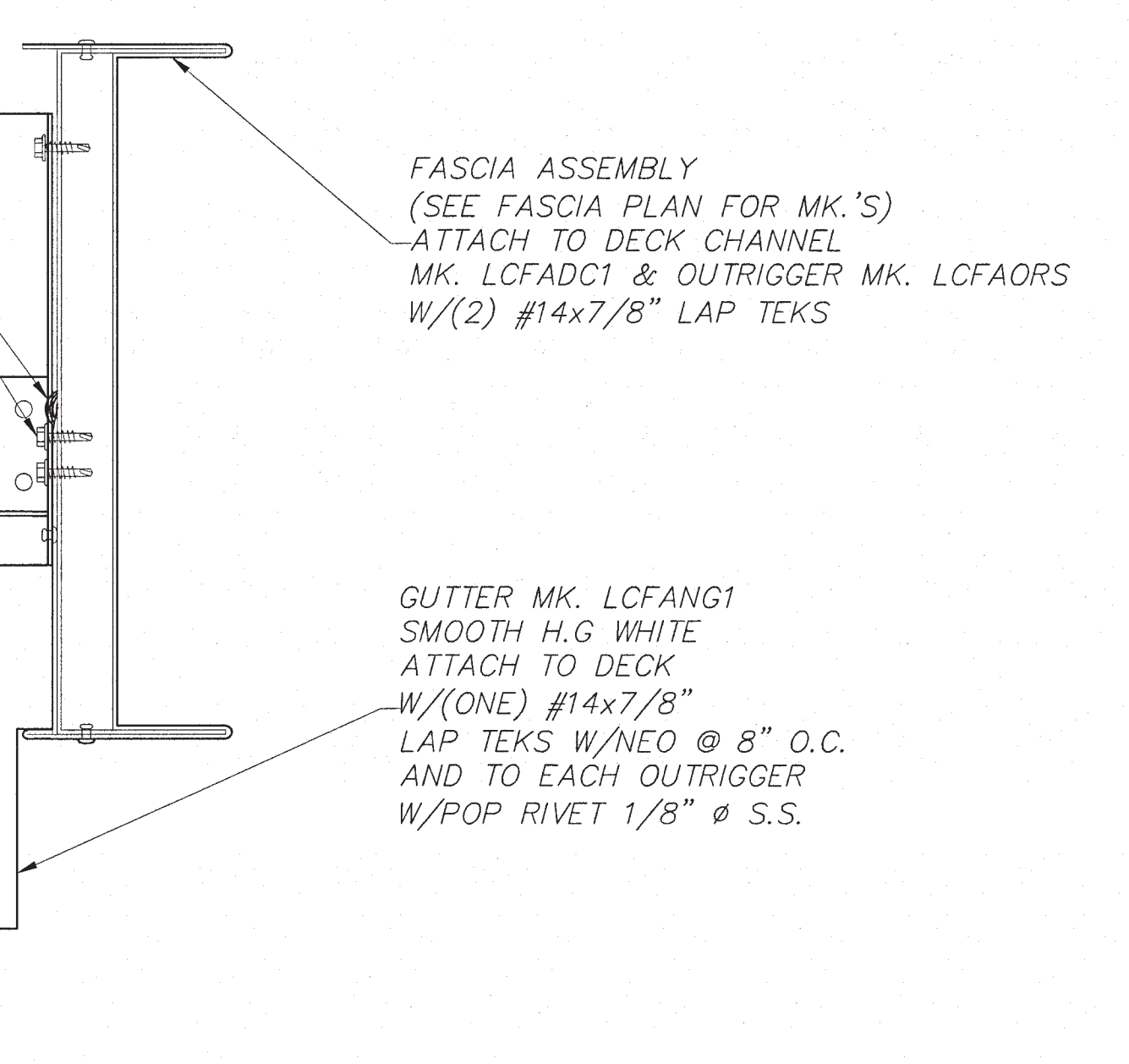
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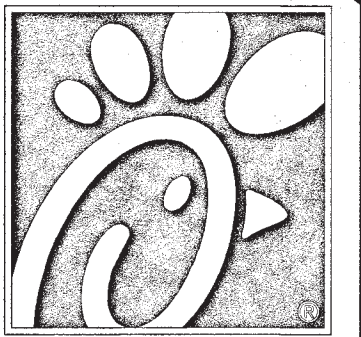
N.T.S.

DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) +
WEIGHT OF STRUCTURAL COMPONENTS
LIVE LOAD = 20 p.s.f.
SNOW LOAD = 20 p.s.f.
WIND LOAD V_{ULT} = 116 m.p.h. EXP. C
WIND V_{ASD} = 90 m.p.h. EXP. C
BLDG CODE = MISSOURI BUILDING CODE 2018
ADOPTING 2018 INTERNATIONAL BUILDING CODE
EQUIVALENT LATERAL FORCE PROCEDURE
LATERAL FORCE RESISTING SYSTEM =
CANTILEVERED COLUMN SYSTEM-
ORDINARY STEEL MOMENT FRAME
P_t = 20 p.s.f.
C_e = 1.2
C_t = 1.2
I_s = 1.0
W = 4.92
P_d = 20.44
SITE CLASS = D
S_s (0.2) = 0.099
S₁ (1.0) = 0.068
SDS = 0.11
SD1 = 0.11
F_a = 1.60
F_v = 2.40
R = 1.25
IMPORTANCE FACTOR = 1.0
RISK CATEGORY = II
SEISMIC DESIGN CATEGORY = D
CS = 0.084
CONSTRUCTION TYPE = IIB
OCCUPANCY CATEGORY = A2
TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 1.43 KIPS

A14 DESIGN LOADS

N.T.S.

				<p>INSTALLER NOTE: VERIFY LOCATION WITH G.C. BEFORE INSTALLING</p> 			
L1	DETAIL AT SIDES OF CANOPY	L5	DETAIL AT FASCIA CORNER AND SPLICE	L9	SECTION AT FAN SUPPORT	L13	NOT USED
N.T.S.	AI-E2, FI-EI	N.T.S.	FI-EI	N.T.S.	FI-EI		
							
FI	SECTION AT PURLIN BRIDGING	F5	SECTION AT PURLIN BRIDGING	F9	DETAIL AT DECK SUPPORT	FI4	DETAIL AT DECK CLIP
N.T.S.	FI-EI	N.T.S.	FI-EI	1 1/2" = 1'-0"	FI-E2	N.T.S.	AI-E2, FI-E2
						<p>DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) + WEIGHT OF STRUCTURAL COMPONENTS LIVE LOAD = 20 p.s.f. SNOW LOAD = 20 p.s.f. WIND LOAD V_{ULT} = 116 m.p.h. EXP. C WIND V_{ASD} = 90 m.p.h. EXP. C BLDG CODE = MISSOURI BUILDING CODE 2018 ADOPTING 2018 INTERNATIONAL BUILDING CODE EQUIVALENT LATERAL FORCE PROCEDURE LATERAL FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEM - ORDINARY STEEL MOMENT FRAME P_f = 20 p.s.f. C_e = 1.2 C_t = 1.2 I_s = 1.0 W = 4.92 P_d = 20.44 SITE CLASS = D S_s (0.2) = 0.099 S₁ (1.0) = 0.068 S_{DS} = 0.11 S_{D1} = 0.11 F_a = 1.60 F_v = 2.40 R = 1.25 IMPORTANCE FACTOR = 1.0 RISK CATEGORY = II SEISMIC DESIGN CATEGORY = D CS = 0.084 CONSTRUCTION TYPE = IIB OCCUPANCY CATEGORY = A2 TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 1.43 KIPS</p>	
AI	DETAIL AT END OF CANOPY	A9	DETAIL OF OVERFLOW DRAIN	AI4	DESIGN LOADS		
N.T.S.	FI-EI, FI-E2	N.T.S.	FI-EI	N.T.S.			



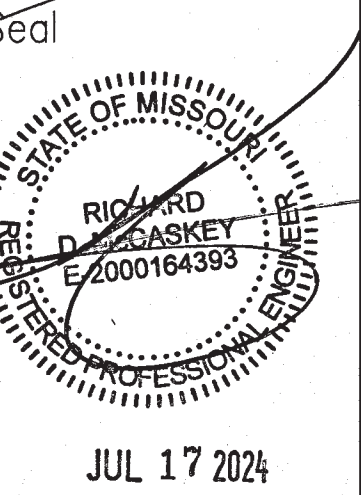
5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:

Mark	Date	By
5	7/15/24	IP

ADDED OFFSET FOOTING

Seal



JUL 17 2024

C.O.A. 2001015838

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ARLINGTON, TX. 76010
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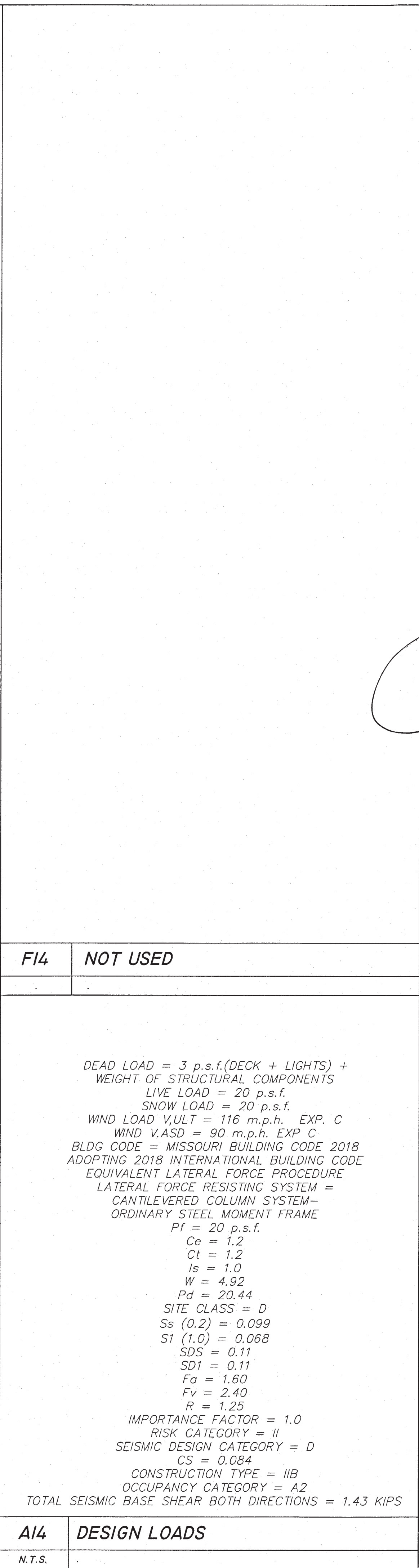
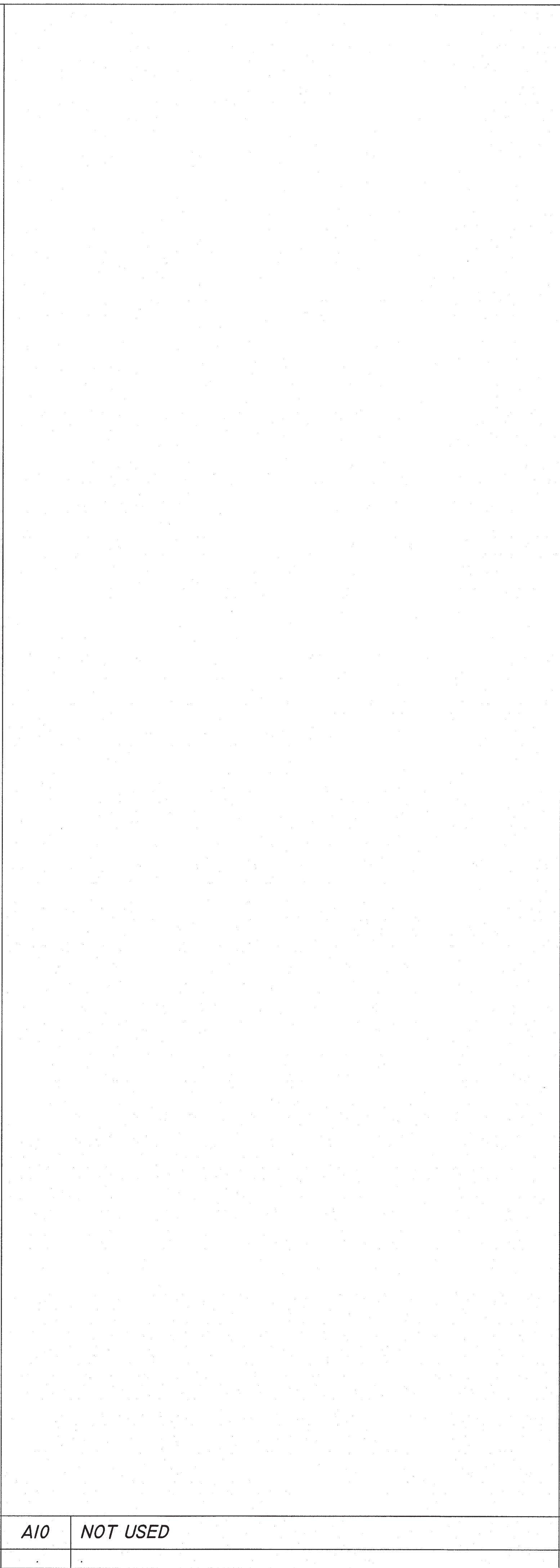
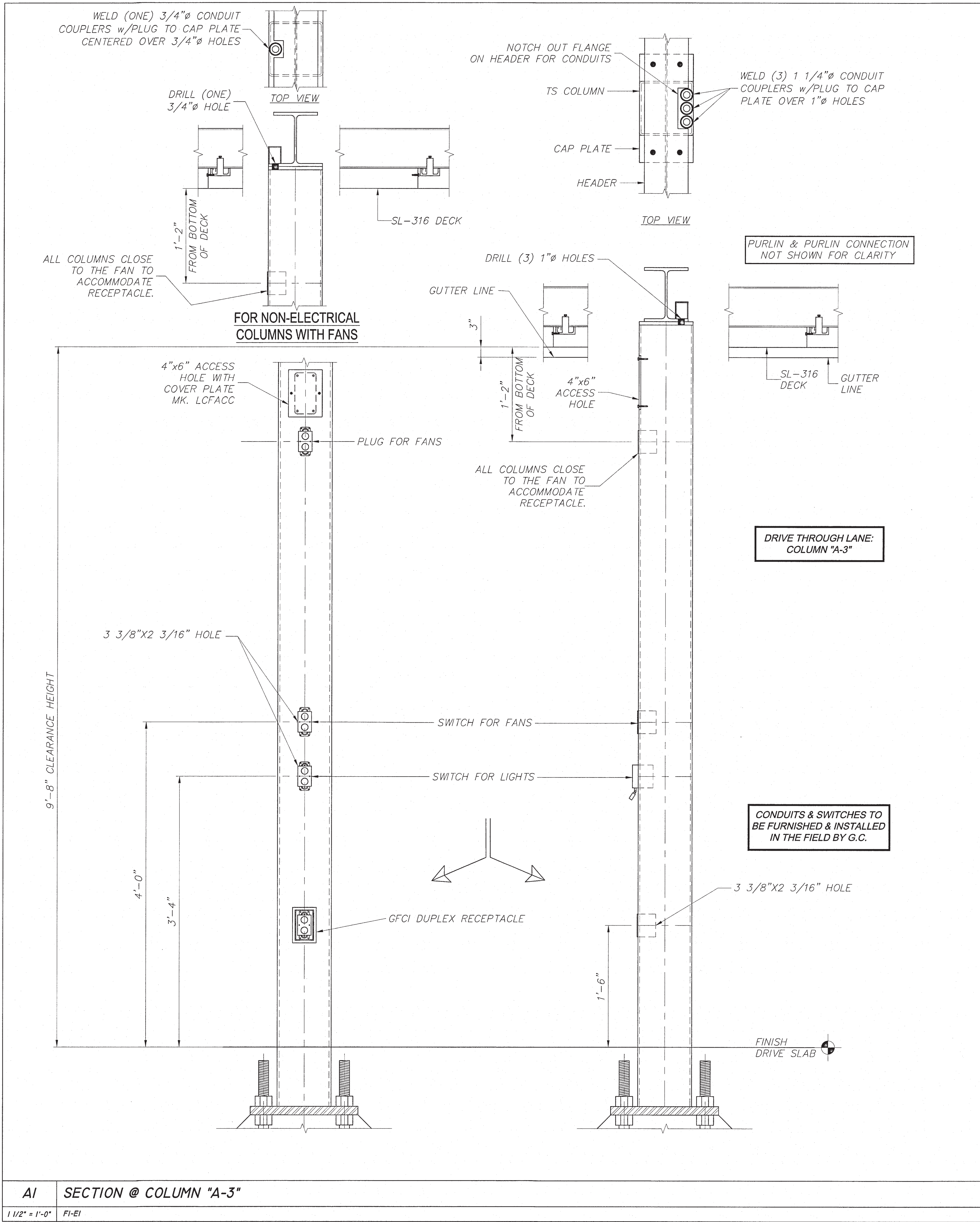
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STORE
Chick-fil-A #02859
690 NW BLUE PKWY
LEE'S SUMMIT, MO
64086

SHEET TITLE
CANOPY SECTIONS
30'-5" X 62'-1 1/2"

Job No.: LSC: 75967
Store : 02859
Date : 09.01.23
Drawn By : KLM
Checked By: RM

Sheet
OMD-7
E3 OF 4





Chick-fil-A

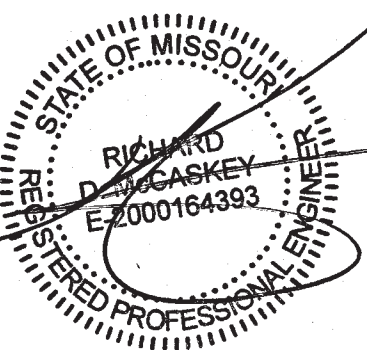
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5	7/15/24	IP

ADDED OFFSET FOOTING

Seal



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SUPPLY, INC.
120 FAIRVIEW
ARLINGTON, TX. 76010
(817) 261-9116

STORE

Chick-fil-A #02859
690 NW BLUE PKWY
LEE'S SUMMIT, MO
64086

SHEET TITLE

CANOPY SECTIONS

30'-5" X 62'-1 1/2"

Job No.: LSC: 75967

Store : 02859

Date : 09.01.23

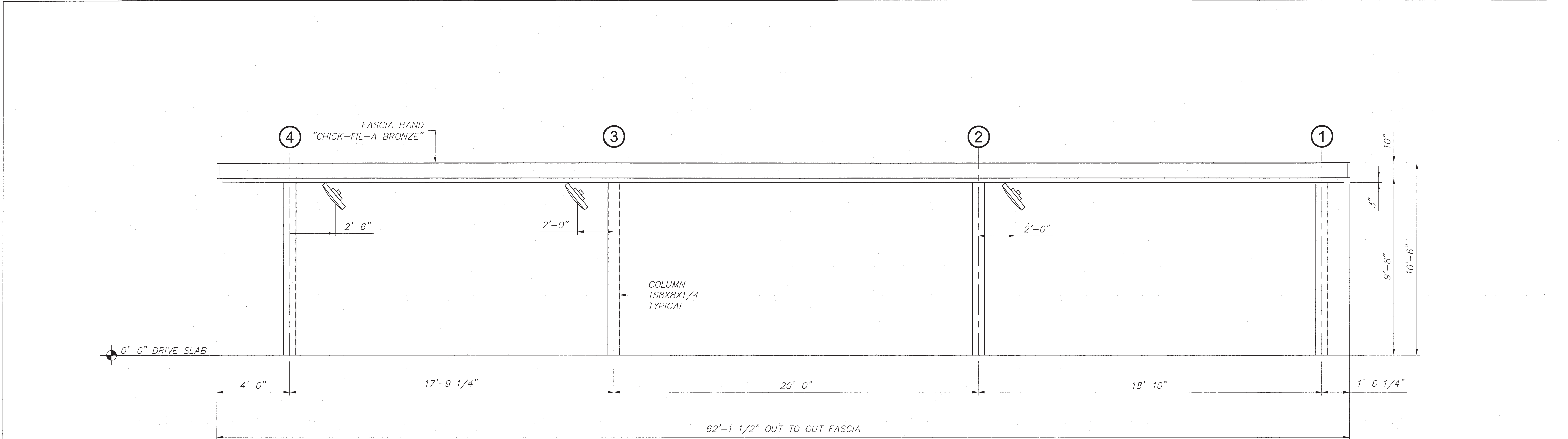
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Checked By: RM

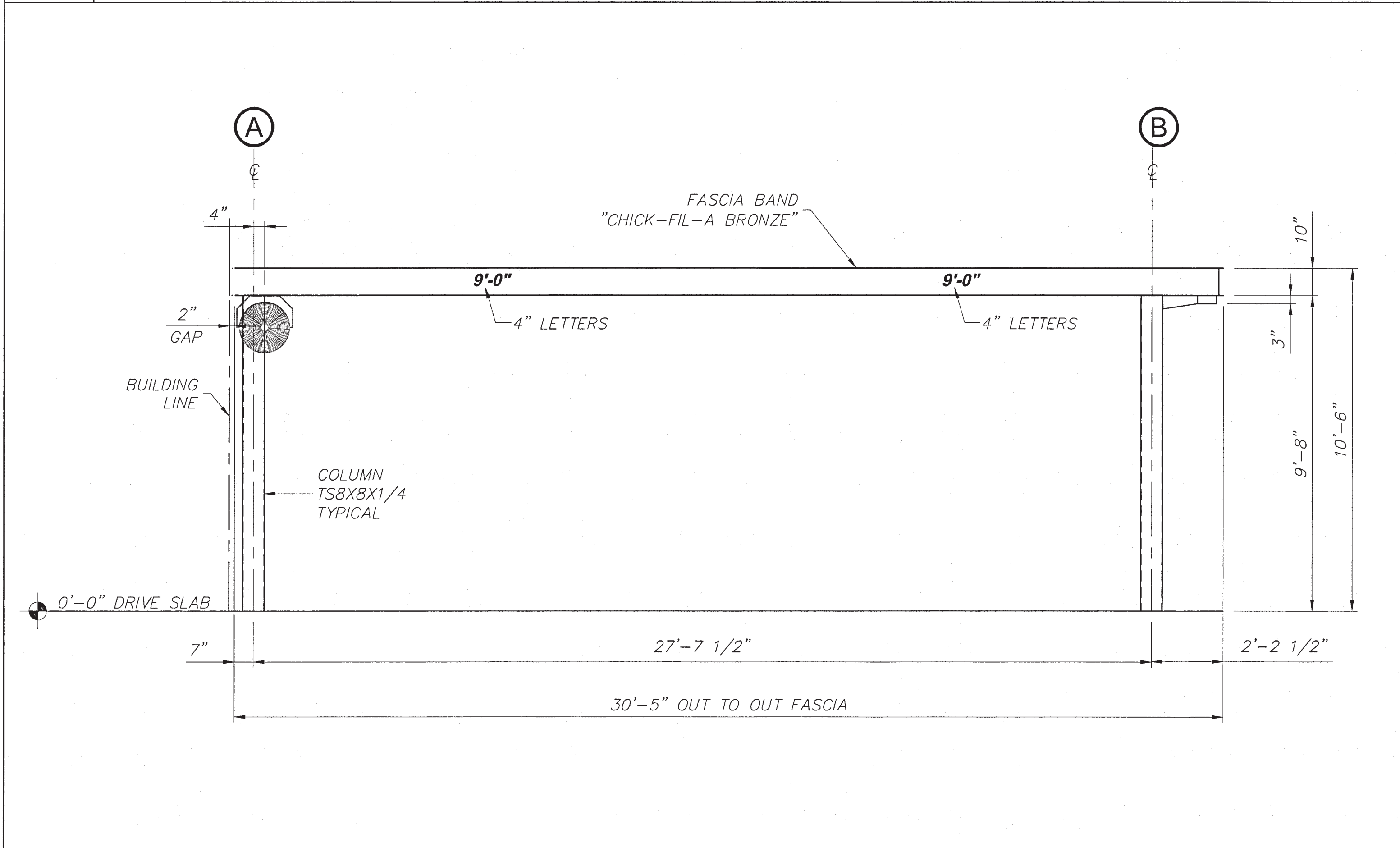
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OMD-8

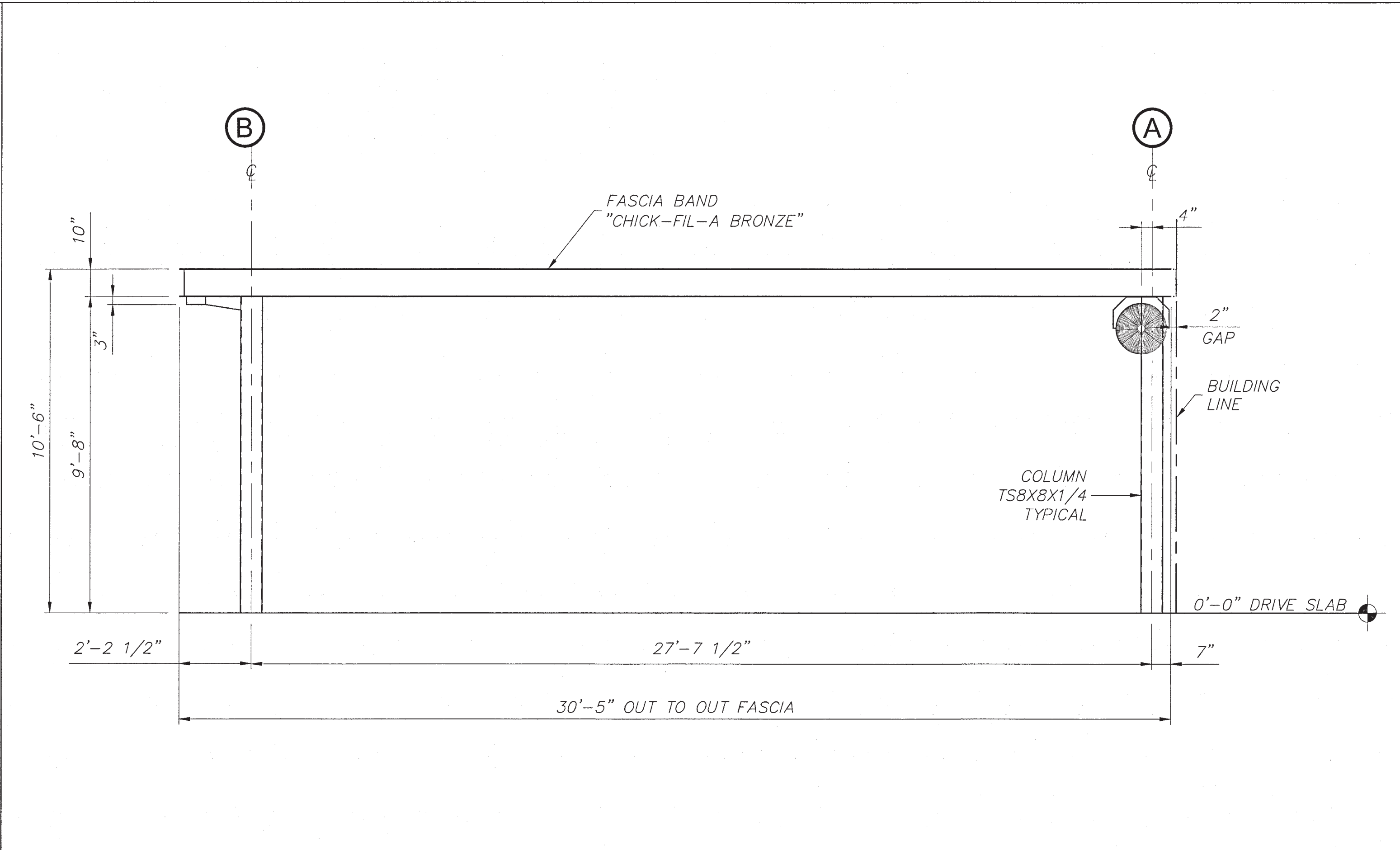
E4 OF 4



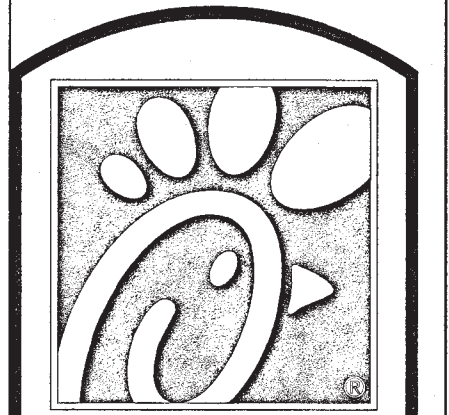
HI CANOPY SIDE ELEVATION
3/8" = 1'-0" FI-ABI, FI-EI, FI-LLI



AI CANOPY END ELEVATION
3/8" = 1'-0" FI-ABI, FI-EI, FI-LLI



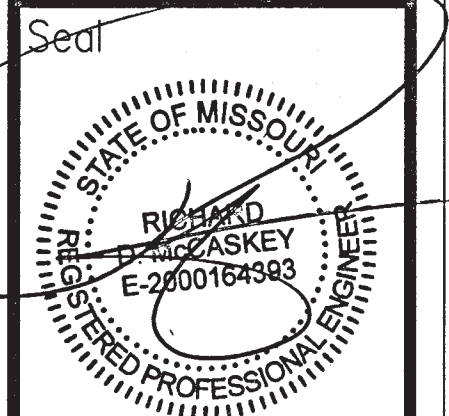
A9 CANOPY END ELEVATION
3/8" = 1'-0" FI-ABI, FI-EI, FI-LLI



Chick-fil-A

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:
Mark Date By
5 7/15/24 IP
ADDED OFFSET
FOOTING



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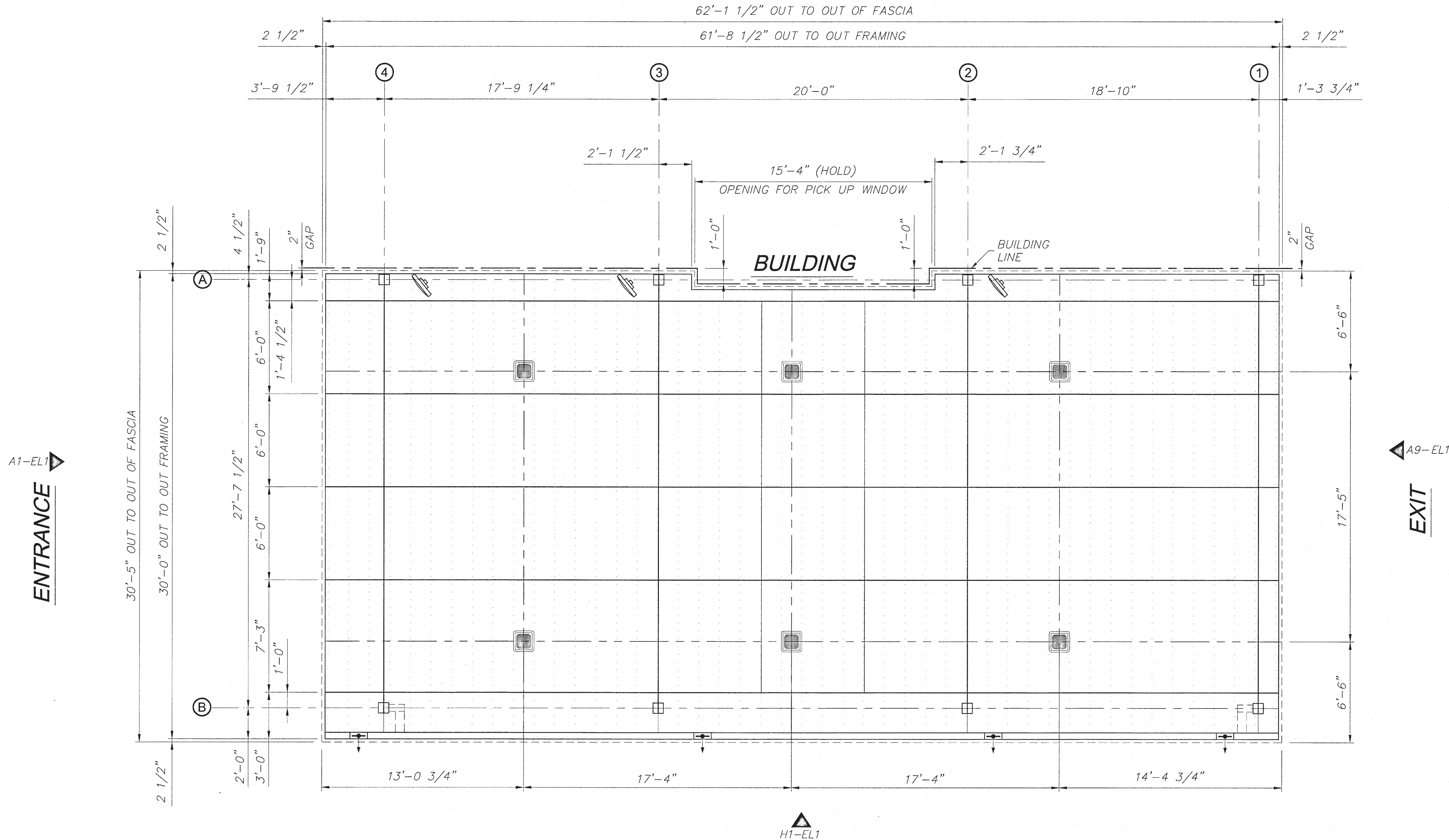
STORE
Chick-fil-A #02859
690 NW BLUE PKWY
LEE'S SUMMIT, MO
64086

SHEET TITLE
CANOPY ELEVATION
PLAN

30'-5" X 62'-1 1/2"

Job No.: LSC: 75967
Store : 02859
Date : 09.01.23
Drawn By : KLM
Checked By: RM

Sheet
OMD-9
ELI OF 1



FI CANOPY LIGHT LAYOUT

1/4" = 1'-0"

1. THE LIGHT LAYOUT IS A SUGGESTED PLAN ONLY. IT IS PROVIDED BY LANE AS A SERVICE TO ITS CUSTOMER AND IS TO BE USED AS A GUIDE ONLY.
2. THESE LIGHT FIXTURES NEED TO BE CENTERED IN DECK PANELS.
3. ENSURE ADEQUATE CLEARANCE FROM STRUCTURAL MEMBERS PRIOR TO CUTTING DECK.
4. INSTALLATION OF FIXTURES TO BE DONE IN ACCORDANCE WITH MANUFACTURES INSTRUCTIONS AND RECOMMENDATIONS.
5. IF LIGHTS INTERFERE WITH LEADER GUTTERS MOVE ROW LIGHTS TO THE NEXT DECK.

LEGEND:

INDICATES LSI CRUS-SC-LED-LW-30-CW-UE-WHT CANOPY DECK LIGHTS.

INDICATES DECK STITCHING.

MOUNTED FANS
(BY OTHERS)
RE: SHEET E3 FOR
MOUNTING SUPPORT

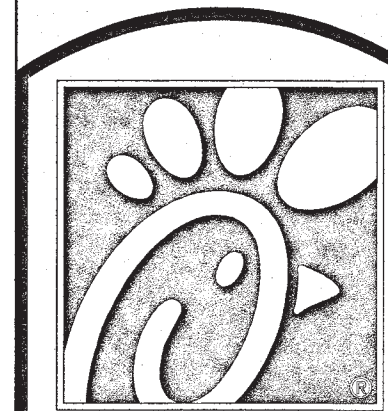
INDICATES MOUNTED FAN

AI GENERAL NOTES

N.T.S.

A6 NOT USED

A10 NOT USED



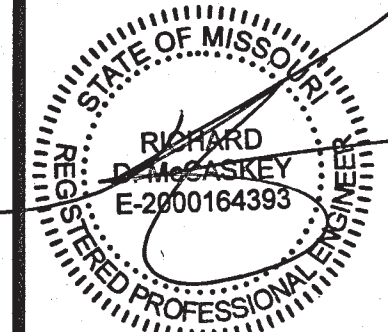
Chick-fil-A

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:

Mark Date By
5 7/15/24 IP
ADDED OFFSET
FOOTING

Seal



JUL 17 2024

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STORE
Chick-fil-A #02859
690 NW BLUE PKWY
LEE'S SUMMIT, MO
64086

SHEET TITLE

CANOPY LIGHT
LAYOUT

30-5" X 62'-1 1/2"

Job No.: LSC: 75967

Store : 02859

Date : 09.01.23

Drawn By : KLM

Checked By: RM

Sheet

OMD-10
LLI OF 1



LANE SUPPLY, INC.

120 Fairview
Arlington, TX 76010
817-261-9116

DESIGN CALCULATIONS FOR :

Chick-fil-A #02859 Order Canopy
690 NW Blue Parkway
Lee's Summit, MO

Six-Column Canopy :	25'-5" X 53'-9" Canopy
Lane Reference Number :	LSC-75966
Date :	01-Sep-23

TABLE OF CONTENTS :

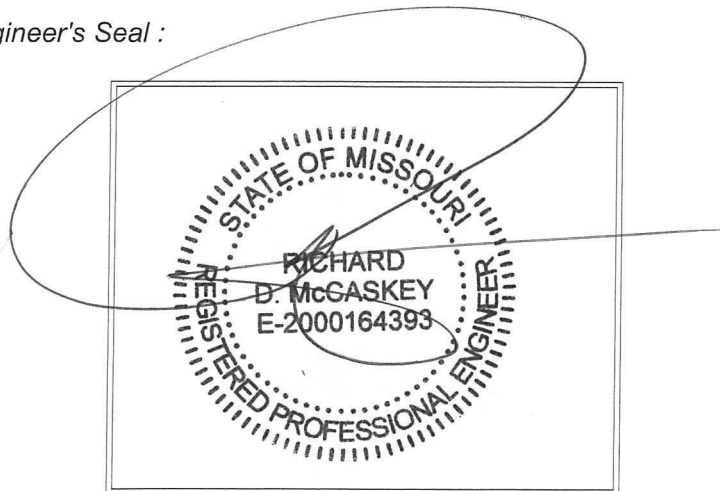
Canopy Calculations :

Design Loads :	1-2
Deck Design :	3
Purlin Design :	4-8
Header Design :	9-10
Column & Foundation Design :	11-12

Attachments :

Lane SL-316 Deck Panel Properties
Lane Standard Base Plate Design
Design Sketch

Engineer's Seal :



C.O.A. 2001015838

SEP 07 2023

Calculations By:
Customer:
Project:

Lane Supply, Inc.
Chick-fil-A #02859 Order Canopy
25'-5" X 53'-9" Canopy

LSC - 75966
By: JO
Check:

Code:

Missouri Building Code 2018
2018 International Building Code

Roof Loads:

Dead Load = 3.00 psf (SL-316 Deck)
Live/Snow Load = 20.00 psf
TOTAL = 23.00 psf

Fascia Load:

Height = 10.00 in.
Dead Load = 5.83 plf

Wind Loads:

Risk Category = II
V, ULT Speed = 116 m.p.h. Exp C
V, ASD Speed = 90 m.p.h. Exp C
Height = 15 ft
Kd = 0.85
Kh = 0.85
G = 0.85
qz = 14.93 psf

Lateral Load = 1.0 (H)•qz = 16.00 psf
Deck Uplift = -1.7 (V)•G•qz = -21.58 psf
Frame Uplift = -1.1 (V)•G•qz = -13.96 psf

Base Shear : V = CS • W = 0.084 • W

Site Class = D
Ss(0.2) = 0.099
S1(1.0) = 0.068
Fa = 1.60
Fv = 2.40
SM1 = Fv•S1 = 0.16
SMS = Fa•Ss = 0.16
SD1 = 2/3•SM1 = 0.11
SDS = 2/3•SMS = 0.11
R = 1.25
Risk Category = II
CS = (SDS/R) = 0.084 (12.8-2)

Seismic Design Category Based on SDS : A
Seismic Design Category Based on SD1 : B

Design Category : B

Section 7.1--Symbols & Notation

C_e =	1.2	Exposure Factor as determined from Table 7-2
C_t =	1.2	Thermal factor as determined from Table 7-3
D =		Snow Density in pcf as determined from Eq. 7-4
h_b =		Height of balanced snow load determined by dividing P_f by D , in feet.
h_d =		Height of snow drift, in feet
h_c =		Clear height from top of balanced snow to top of parapet, ft
h_r =	0.83	= Fascia height, ft
I_s =	1.0	= Importance factor (see Table 7-4).
P_f =		Snow load on flat roofs, psf.
P_g =	20	=ground snow, psf.
P_d =		Maximum intensity of drift surcharge load, psf.
l_u =	25.875	= Length of roof upwind of the drift, feet
w =		Width of snow drift, in feet

Section 7.3--Flat-Roof Snow Loads, P_f

The snow load, P_f , on a roof with a slope equal to or less than 15° shall be calculated in psf using equation 7.3-1, but not less than the following minimum values for low slope roofs: where P_g is 20 psf or less $P_f = I(P_g)$, where P_g exceeds 20 psf, $P_f = 20 (I)$.

Section 7.7 & Section 7.8

The geometry of the surcharge load due to snow drifting shall be approximated by a triangle as shown in figure 7-8. Drift loads shall be superimposed on the balanced snow load. If h_c/h_b is less than 0.2, drift loads are not required to be applied. The height of such drifts shall be taken as $0.75 \times h_d$ as determined from Fig 7-9, with l_u equal to the length of the roof upwind of the projection or parapet wall. If the side of a roof projection is less than 15 ft long, a drift load is not required to be applied to that side. If the height, h_d , is equal to or less than h_c , the drift width shall equal $4h_d$ and the drift height shall equal h_d . If this height exceeds h_c , the drift width, w , shall equal $4h_d^2/h_c$ and the drift height shall equal h_c . However, the drift width w shall not exceed $8h_c$. The maximum intensity of the drift surcharge load, p_d , equals $h_d \times D$ where the snow density, D , is defined by Eq 7.7-

Section 7.10--Rain-On-Snow Surcharge Load

For locations where P_g is 20 psf or less but not zero, all roofs with a slope less than $W/50$, shall have a 5 psf rain-on-snow surcharge load applied to establish the design snow loads. This rain-on-snow augmented design load applies only to the balanced load case and need not be used in combination with drift, sliding, unbalanced, or partial loads.

$$P_f = 0.7 \times C_e \times C_t \times I_s \times P_g \quad \text{Eq 7.3-1}$$

$$P_f = 20.0 \text{ psf}$$

$$h_d = 0.75 \times (0.43(l_u)^{1/3} \times (P_g + 10)^{1/4 - 1.5})$$

$$h_d = 1.11 \text{ ft}$$

$$D = 0.13P_g + 14 < 30 \text{ psf} \quad \text{Eq 7.7-1}$$

$$D = 16.60 \text{ psf}$$

$$h_b = 1.20 \text{ ft}$$

$$h_c = h_r - h_b = -0.37 \text{ ft}$$

$$h_c/h_b = -0.31$$

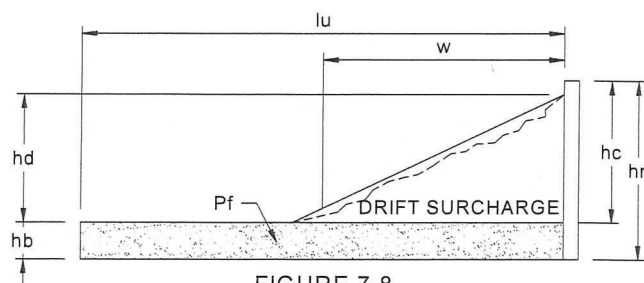
Drift Loads Not Considered

$$w = -13.20 \text{ ft}$$

$$P_d = D \times h_d < D \times h_c$$

$$P_d = -6.17 \text{ psf}$$

Ref. Fig. 7-9

FIGURE 7-8
Configuration of Snow Drifts on Lower Roofs

| P1 | P2 |
 v o 23.00 psf o v
 ^A B^ ^C D^
 |<-- X1 ----> |<----- L1 ----->| |<----- L2 ----->| |<----- L3 ----->| <--- X2 ---->|

USE 20 GAUGE GRADE C DECK

Page 3 of 12

BEAM DESIGN: P-a

Wd=	30.32	plf	P	
Wl=	140.42	plf	v	170.74 plf
Ww=	-97.47	plf	^A	B^
Pd=	40.72	lbs	<-----X----->	<----- L ----->
Pl=	0.00	lbs		
Pw=	0.00	lbs	Deflections: (inches)	Overhang Midspan
L=	5.92	ft	DL=	0.000 0.000
X=	1.94	ft	DL+LL=	-0.002 0.004
			(+downward, -upward)	
MA(d) =	136	ft-lbs	RAd=	212 lbs
MA(l) =	264	ft-lbs	RAI=	732 lbs
MA(w) =	-183	ft-lbs	RAw=	-508 lbs
MA(d+l) =	399	ft-lbs lu=	1.33 ft	RA(d+l)= 944 lbs
MA(d+w) =	-47	ft-lbs lu=	1.94 ft	RA(d+w)= -296 lbs
			RBd=	67 lbs
			RBI(Max)=	415 lbs
			RBw=	-257 lbs
MAB(d+l)=	681	ft-lbs lu=	5.92 ft	RB(d+l)= 482 lbs
MAB(d+w)=	-271	ft-lbs lu=	1.33 ft	RB(d+w)= -191 lbs

USE: W8X10 Fy = 50 ksi

BEAM DESIGN: P-b

Wd=	30.32	plf		
Wl=	140.42	plf		
Ww=	-97.47	plf		
L=	19.00	ft		
			170.74 plf	
			^	^
			<----- L ----->	
Md=	1368.2	ft-lbs	Rd=	288 lbs
MI=	6336.3	ft-lbs	RI=	1334 lbs
Mw=	-4398.2	ft-lbs	Rw=	-926 lbs
			R(d+l)=	1622 lbs
			R(d+w)=	-638 lbs
M(d+l)=	7704.5	ft-lbs	Lu=	9.50 ft
M(d+w)=	-3030.1	ft-lbs	Lu=	1.33 ft

OK
OK

USE: W8X10 Fy = 50 ksi

Deflections: (inches)	Midspan
DL=	0.100
DL+LL=	0.560
(+downward, -upward)	

	P		
Wl=	167.74 plf	v	
Pd=		A	B^
X=		L	
(+downward, -upward)			
MA(d) =	119 ft-lbs	RAd=	183 lbs
MA(l) =	266 ft-lbs	RAI=	740 lbs
MA(w) =	-164 ft-lbs	RAw=	-454 lbs
MA(d+l) =	385 ft-lbs lu=	1.33 ft	RA(d+l)= 923 lbs
MA(d+w) =	-44 ft-lbs lu=	1.94 ft	RA(d+w)= -271 lbs
MAB(d+l)=	676 ft-lbs lu=	5.92 ft	RB(d+l)= 476 lbs
MAB(d+w)=	-246 ft-lbs lu=	1.33 ft	RB(d+w)= -174 lbs

USE: W8X10 $F_y = 50$ ksi

Wd=	25.87 plf
WI=	141.88 plf
Ww=	-87.11 plf
L=	19.00 ft
167.74 plf Λ-----Λ	
<-----	L ----->
Md=	1167.3 ft-lbs
MI=	6402.1 ft-lbs
Mw=	-3930.9 ft-lbs
<hr/>	
Rd=	246 lbs
RI=	1348 lbs
Rw=	-828 lbs
R(d+l)=	1594 lbs
R(d+w)=	-582 lbs
<hr/>	
M(d+l)=	7569.4 ft-lbs
M(d+w)=	-2763.6 ft-lbs
Lu=	9.50 ft OK
Lu=	1.33 ft OK

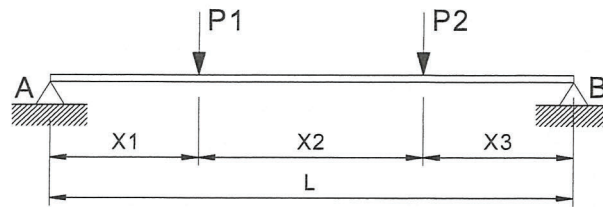
USE: W8X10 $F_y =$ 50 ksi

Deflections: (inches)	Midspan
DL=	0.085
DL+LL=	0.551
(+downward, -upward)	

BEAM DESIGN: P-e

P1d = 212.11 lbs
P1l = 732.00 lbs
P1w = -508.10 lbs
P2d = 183.17 lbs
P2l = 739.60 lbs
P2w = -454.12 lbs

Wd = 16.00 plf
Wl = 0.00 plf
Ww = 0.00 plf



X1 = 7.02 ft
X2 = 7.02 ft
X3 = 7.17 ft
L = 21.21 ft

RA_d = 373 lbs
RA_l = 740 lbs
RA_w = -493 lbs
RA(d+l) = 1113 lbs
RA(d+w) = -120 lbs

RB_d = 361 lbs
RB_l = 732 lbs
RB_w = -469 lbs
RB(d+l) = 1093 lbs
RB(d+w) = -108 lbs

MAB(dl) = 2301 ft-lbs
MAB(ll) = 5246 ft-lbs
MAB(wl) = -3464 ft-lbs
MAB(dl+ll) = 7520 ft-lbs lu = 7.17 ft
MAB(dl+wl) = -1236 ft-lbs lu = 7.17 ft

Deflection

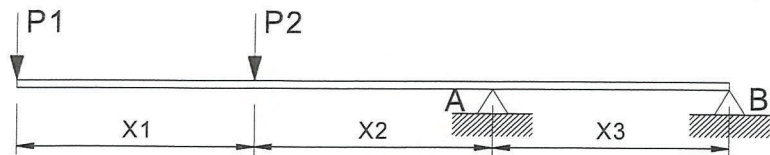
dl = 0.202 in
dl+ll = 0.665 in

USE: W6X16 F_y = 50 ksi

BEAM DESIGN: P-f

P1d= 25.09 lbs
P1l= 0.00 lbs
P1w= 0.00 lbs
P2d= 373.46 lbs
P2l= 739.60 lbs
P2w= -493.36 lbs
Wd = 39.36 plf
Wl = 86.01 plf
Ww = -60.04 plf

X1= 1.94 ft
X2= 5.92 ft
X3= 19.00 ft



RAd= 1272.12 lbs
RAI= 2602.17 lbs
RAw= -1786.49 lbs
RAd+l= 3874.29 lbs
RAd+w= -514.36 lbs

RBd= 183.34 lbs
RBI= 447.15 lbs
RBw= -319.32 lbs
RBd+l= 630.49 lbs
RBd+w= -135.97 lbs

MA(d)= 3621 ft-lbs
MA(l)= 7029 ft-lbs
MA(w)= -4771 ft-lbs
MA(d+l)= 10649 ft-lbs lu = 1.33 ft
MA(d+w)= -1150 ft-lbs lu = 5.92 ft
MAB(d+l)= 1557 ft-lbs lu = 9.50 ft
MAB(d+w)= -439 ft-lbs lu = 1.33 ft

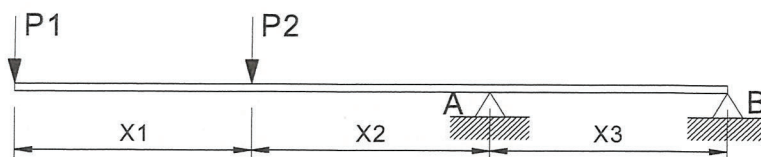
Deflections		
	OH	SPAN
dl (in)=	0.221	-0.023
dl+ll (in) =	0.616	-0.045

USE: W6X20 Fy = 50 ksi

BEAM DESIGN: P-g

P1d= 46.30 lbs
P1l= 0.00 lbs
P1w= 0.00 lbs
P2d= 361.16 lbs
P2l= 732.00 lbs
P2w= -468.87 lbs
Wd = 45.50 plf
Wl = 158.76 plf
Ww = -110.83 plf

X1= 1.94 ft
X2= 5.92 ft
X3= 19.00 ft



RAd= 1402.47 lbs
RAI= 3972.82 lbs
RAw= -2718.20 lbs
RAd+l= 5375.29 lbs
RAd+w= -1315.73 lbs

RBd= 226.75 lbs
RBI= 1022.54 lbs
RBw= -726.98 lbs
RBd+l= 1249.29 lbs
RBd+w= -500.23 lbs

MA(d)= 3904 ft-lbs
MA(l)= 9228 ft-lbs
MA(w)= -6193 ft-lbs
MA(d+l)= 13132 ft-lbs lu = 1.33 ft
MA(d+w)= -2289 ft-lbs lu = 5.92 ft
MAB(d+l)= 3803 ft-lbs lu = 9.50 ft
MAB(d+w)= -1915 ft-lbs lu = 1.33 ft

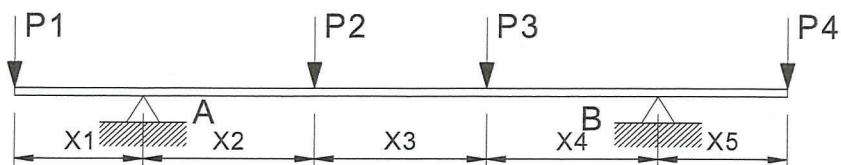
Deflections		
	OH	SPAN
dl (in)=	0.239	-0.018
dl+ll (in) =	0.624	0.067

USE: W8X13 Fy = 50 ksi

HEADER BEAM DESIGN:

H-a

P1d = 1272 lbs
P1l = 2602 lbs
P1w = -1786 lbs
P2d = 355 lbs
P2l = 1749 lbs
P2w = -1183 lbs
P3d = 302 lbs
P3l = 1768 lbs
P3w = -1058 lbs
P4d = 1402 lbs
P4l = 3973 lbs
P4w = -2718 lbs



X1 = 1.25 ft
X2 = 5.77 ft
X3 = 7.02 ft
X4 = 0.88 ft
X5 = 6.29 ft

Wd = 24.00 plf
Wl = 0.00 plf
Ww = 0.00 plf

RAd = 1128 lbs
RAI = 2135 lbs
RAw = -1450 lbs
RAd+l = 3263 lbs
RAd+w = -322 lbs

RBd = 2713 lbs
RBI = 7957 lbs
RBw = -5296 lbs
RBd+l = 10670 lbs
RBd+w = -2583 lbs

MA(DL): -1608.9 ft-lbs
MA(LL): -3252.7 ft-lbs
MA(WL): 2233.1 ft-lbs
MA(DL+LL): -4861.6 ft-lbs lu = 1.25 ft
MA(DL+WL): 624.2 ft-lbs lu = 1.25 ft

Deflection
DL = 0.02 in
DL+LL = 0.06 in

SPAN
M(DL+LL): -4861.6 ft-lbs lu = 7.02 ft
M(DL+WL): 624.2 ft-lbs lu = 7.02 ft

Deflection
DL = -0.06 in
DL+LL = -0.08 in

MB(DL): -9298.9 ft-lbs
MB(LL): -24995.6 ft-lbs
MB(WL): 17102.0 ft-lbs
MB(DL+LL): -34294.5 ft-lbs lu = 6.29 ft
MB(DL+WL): 7803.1 ft-lbs lu = 6.29 ft

Deflection
DL = 0.11 in
DL+LL = 0.38 in

USE: W8X24

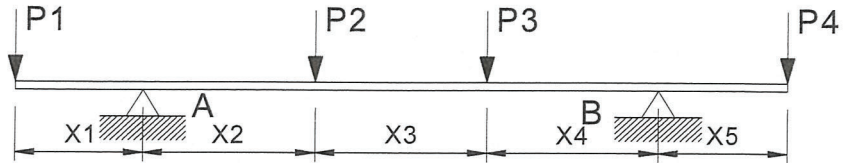
Fy = 50 ksi

HEADER BEAM DESIGN:

H-b

P1d = 367 lbs
P1l = 894 lbs
P1w = -639 lbs
P2d = 576 lbs
P2l = 2668 lbs
P2w = -1852 lbs
P3d = 492 lbs
P3l = 2696 lbs
P3w = -1655 lbs
P4d = 453 lbs
P4l = 2045 lbs
P4w = -1454 lbs

Wd = 20.00 plf
Wl = 0.00 plf
Ww = 0.00 plf



X1 = 1.25 ft
X2 = 5.77 ft
X3 = 7.02 ft
X4 = 0.88 ft
X5 = 6.29 ft

RAd = 690 lbs
RAI = 1749 lbs
RAw = -1204 lbs
RAd+l = 2438 lbs
RAd+w = -514 lbs

RBd = 1622 lbs
RBI = 6554 lbs
RBw = -4396 lbs
RBd+l = 8177 lbs
RBd+w = -2774 lbs

MA(DL): -474.0 ft-lbs
MA(LL): -1117.9 ft-lbs
MA(WL): 798.3 ft-lbs
MA(DL+LL): -1591.9 ft-lbs lu = 1.25 ft
MA(DL+WL): 324.3 ft-lbs lu = 1.25 ft

Deflection
DL = -0.00 in
DL+LL = -0.00 in

SPAN
M(DL+LL): 4724.0 ft-lbs lu = 7.02 ft
M(DL+WL): -1549.8 ft-lbs lu = 7.02 ft

Deflection
DL = -0.01 in
DL+LL = 0.01 in

MB(DL): -3249.1 ft-lbs
MB(LL): -12867.0 ft-lbs
MB(WL): 9147.8 ft-lbs
MB(DL+LL): -16116.1 ft-lbs lu = 6.29 ft
MB(DL+WL): 5898.7 ft-lbs lu = 6.29 ft

Deflection
DL = 0.06 in
DL+LL = 0.30 in

USE: W6X20

Fy = 50 ksi

Column Design

AISC 15th ed, Use First Order Analysis Criteria

P DL =	2.71 kips	Clr. Ht.=	9.50 ft
P LL =	7.96 kips	Fascia Ht.=	1.00 ft
P WL =	-5.30 kips	Col. Trib=	19.27 ft
Base Shear =	0.20 kips	Wind Load=	16.00 psf
Total Base Shear =	0.84 kips	# of COL.=	2
M WL =	w(Fascia Ht*2.5*Col Trib./# of col*L)+ w(Wrap*1/2 Clr. Ht^2)		Max All. Defl = 1.20 in
M Seis =	Base Shear x L		Max Defl Ratio = L/ 100
M Unbal =	Live Load x Col. Trib.x (Canopy Width/2)^2/2		Max Defl. = 0.13 in, OK
L =	Clr. Ht. + Fascia Ht/2		
Pr =	10.67 kips 1.6Pr<0.5Py First-Order Analysis Allowed (A-7-1)		
Py =	326.60 kips		
N =	0.00 •Yi (A-7-2)		
B2 =	1.05 OK, A-8-6		
M WL =	4.58 kip-ft		
M Seis =	1.98 kip-ft		
M DL(Nod) =	0.11 kip-ft		
M LL(Nod) =	0.33 kip-ft		
M Unbal DL=	0.00 kip-ft		
M Unbal LL=	0.00 kip-ft		
M Unbal WL=	0.00 kip-ft		

Use: TS8X8X1/4

Fy =	46.00 ksi
K =	1.00
L, Col =	10.00 ft
A =	7.10 in^2
I =	70.70 in^4
Cm =	1.00
Pe1 =	447.31 kips
B1 =	1.04 (A-8-3)
P, All =	177.38 kips
M, All =	44.10 kip-ft

Load Combination	Pr, Kips	Mr, Kip-ft	Equation	Result
D+L	10.67	0.47	0.04	OK
D+W	2.71	4.88	0.12	OK
D+0.7E	2.71	1.56	0.04	OK
D+0.75W+0.75L	8.68	3.95	0.11	OK
D+0.525E+0.75L	8.68	1.46	0.06	OK

Top Connection : Standard Cap Plate

Base Plate : LBP 8 - 20

Foundation: (Restrained at Grade)

$$d^2=(4.25*M)/(S3*b)$$

M(MAX)=	4877 ft-lbs
S3=	100 PCF X d
b=	3.000 ft
d=	4.103 ft

Pmax=	10.67 kips
Footing Area=	7.07 ft^2
Bearing=	1509.44 psf

Footing= Round

USE: 3.00 FT.RND. X 5.00 ft deep footing

$$As=12*M/(jd*24000)= 0.0653 \text{ in}^2$$

USE: 8 #8's (RND. Cage) w/ #4 Ties @ 12" O.C. w/135 hooks

$P_{dl} = 2713$ lbs
 $P_{ll} = 7957$ lbs
 $P_w = -5296$ lbs
 $M = 4877$ ft-lbs
 Soil Density = 110 pcf

Width = 4.50 ft
 Length = 4.50 ft
 Depth = 3.00 ft

$a = 0.00$ ft
 $b = 0.00$ ft
 $c = 0.00$ ft

Footing Weight = 9112.5 lbs
 Soil Weight = 0 lbs

Overturning : $OTM = 4877$ ft-lbs
 $RM = 12250$ ft-lbs
 $FS = 2.51 > 1.5$ Therefore OK

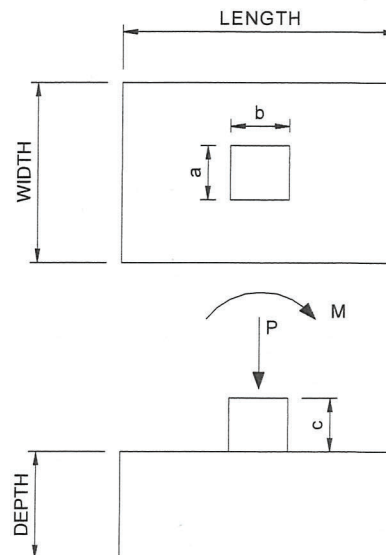
Soil Pressure : $q_{(dl+ll)} = 526.89$ psf Net
 $q(ALLOW) = 1500$ psf OK

For $dl+(wl, seismic)$:

$P = 11825$ lbs
 $e = M/P = 0.41$ ft
 $L/6 = 0.75$ ft
Resultant within middle 3rd
 $q_{(dl+wl, seismic)} = 455.06$ psf Net
 $q(ALLOW) = 1500.00$ psf OK

REINFORCING: $M_{(dl+ll)} = 1333.70$ ft-lbs/ft
 $M_{(dl+wl, seismic)} = 517.28$ ft-lbs/ft
dl+ll Controls
 Assume: $f'_c = 2500$ psi, $F_y = 40000$ psi
 $d = 32.63$ in
 $As(REQ'D) = 0.03$ in²
 $As(PROV.) = 0.44$ in²

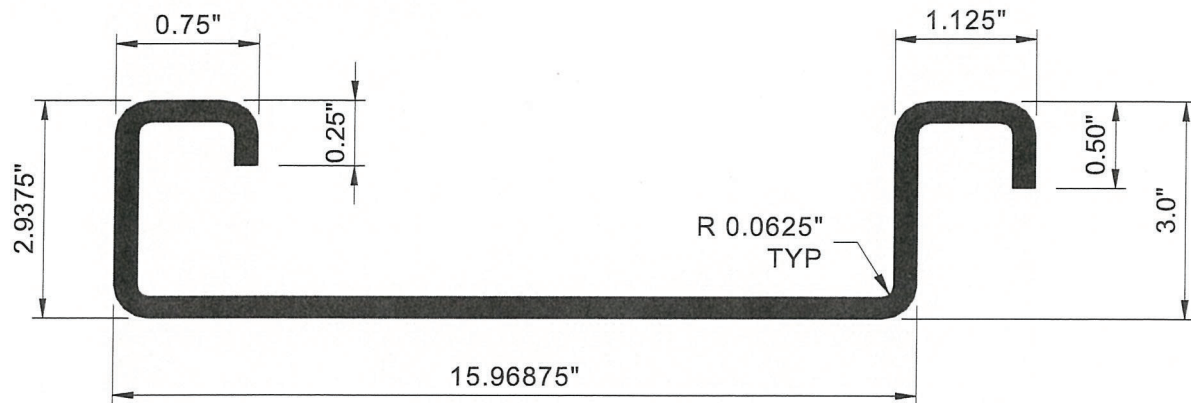
USE #6's AT 12" O.C. T&B, EACH WAY





LANE SUPPLY, INC.

120 Fairview
Arlington, Texas 76010
817-261-9116



SL-316 DECK PANEL

Section Properties

Gage	Wt, psf	Thickness, in	ASTM 653	+I, in ⁴	-I, in ⁴	+S, in ³	-S, in ³	+M, ft-lbs/ft	-M, ft-lbs/ft
20	2.20	0.0359	Grade 40	0.9346	0.4680	0.3961	0.3036	592.70	454.44
			Grade 50	0.9208	0.4522	0.3879	0.2880	725.86	538.92
18	2.93	0.0478	Grade 40	1.2486	0.6827	0.5329	0.4377	797.77	655.28
			Grade 50	1.2129	0.6518	0.5141	0.4296	962.09	803.92

Notes:

- 1 Designed per AISI Cold Formed Steel Manual, 2016 ed.
- 2 Complete calculations are available upon request.
- 3 $\pm M$ is allowable bending moment.

Issued 12-5-17



LANE SUPPLY, INC.

120 Fairview
Arlington, Texas 76010
817-261-9116
FAX 817-275-1660

STANDARD BASE PLATE DESIGN

LBP #	M	P _{BOLT}	Bolt Dia.	t _{REQ'D}	t _{ACTUAL}	Weld Req'd	Weld Actual	Base Plate
(D - M)	(ft-k)	(k)	(in)	(in)	(in)	(1/16 in)	(in)	Mark
8 - 10	10	5.58	1 1/2	0.72	3/4	1.52	1/4	LBP 1
8 - 20	20	10.91	1 1/2	0.99	1	3.03	5/16	LBP 2
8 - 30	30	16.00	1 1/2	1.17	1 1/4	4.55	5/16	LBP 3
8 - 40	40	20.87	1 1/2	1.32	1 1/2	6.06	F.P.	LBP 4
8 - 50	50	26.09	1 1/2	1.46	1 1/2	7.58	F.P.	LBP 5

TS 8 X 8 COLUMN:

D= 8 in.
e= 2 in.
b,d= 8 in.

CONSTANTS:

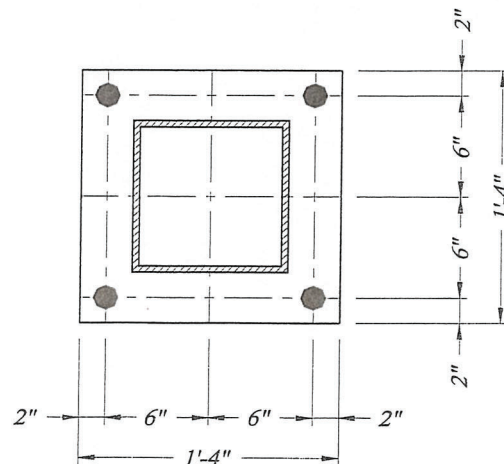
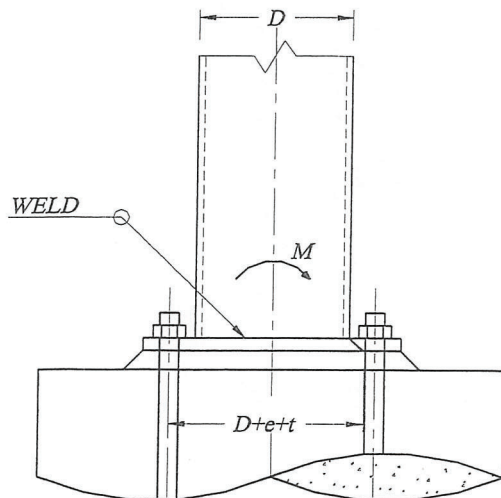
A36 Steel Plate
E70xx Electrode
A307 Anchor Bolts
F_y = 36 ksi
F_w = 0.928 k/in/16th
F_t = 20 ksi

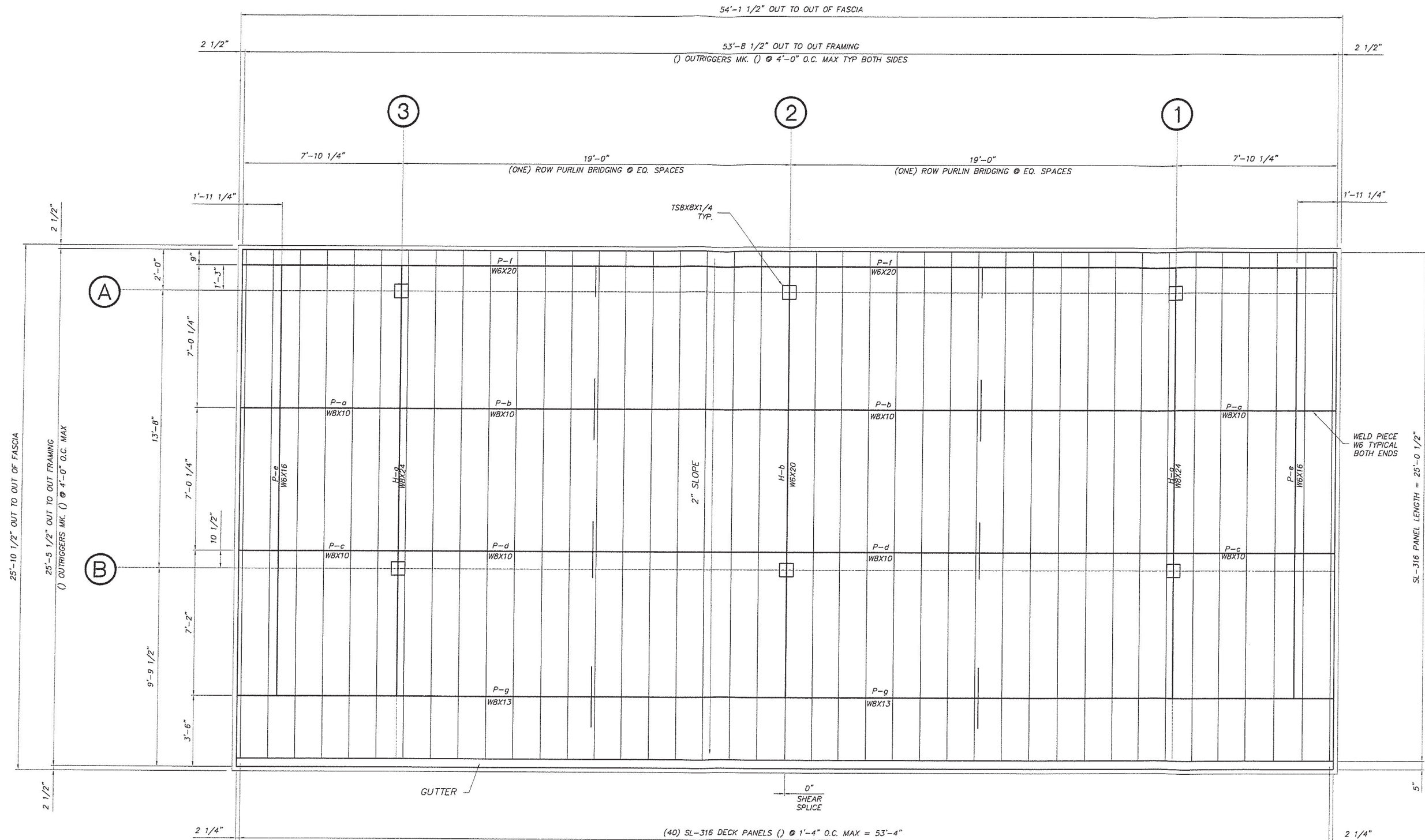
EQUATIONS:

$$P_{BOLT} = \frac{M \times 12 \text{ in/ft}}{2 \text{ bolts } (D+e+t)}$$

$$Weld = \frac{M \times 12 \text{ in/ft}}{S_{Weld} \times F_w} = \frac{M \times 12 \text{ in/ft}}{F_w (bd+d^2/3)}$$

$$t_{REQ'D} = \sqrt{\frac{6 \times P \times e \times 2 \text{ bolts}}{0.75 \times F_y \times (D+2t)}}$$





DESIGN LOADS:

DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) + WEIGHT OF STRUCTURAL COMPONENTS
 LIVE LOAD = 20 p.s.f.
 SNOW LOAD = 20 p.s.f.
 WIND LOAD VULT = 116 m.p.h. EXP. C
 WIND VASD = 90 m.p.h. EXP. C
 BLDG CODE = MISSOURI BUILDING CODE 2018
 ADOPTING 2018 INTERNATIONAL BUILDING CODE
 EQUIVALENT LATERAL FORCE PROCEDURE
 LATERAL FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEM-ORDINARY STEEL MOMENT FRAME
 Pf = 20 p.s.f.
 Ce = 1.2
 Cf = 1.2
 Ig = 1.0
 W = DRIFT LOADS NOT CONSIDERED
 Pd = DRIFT LOADS NOT CONSIDERED
 SITE CLASS = D
 Ss (0.2) = 0.099
 S1 (1.0) = 0.068
 SDS = 0.11
 SD1 = 0.11
 Fa = 1.60
 Fv = 2.40
 R = 1.25
 IMPORTANCE FACTOR = 1.0
 RISK CATEGORY = II
 SEISMIC DESIGN CATEGORY = D
 CS = 0.084
 CONSTRUCTION TYPE = IIB
 OCCUPANCY CATEGORY = A2
 TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 0.84 KIPS



LANE SUPPLY, INC.

120 Fairview
Arlington, TX 76010
817-261-9116

DESIGN CALCULATIONS FOR :

Chick-fil-A #02859 Outside Meal Delivery Canopy
690 NW Blue Parkway
Lee's Summit, MO

Eight-Column Canopy :	30'-0" X 61'-9" Canopy
Lane Reference Number :	LSC-75967 Rev 1
Date :	13-Jul-24

TABLE OF CONTENTS :

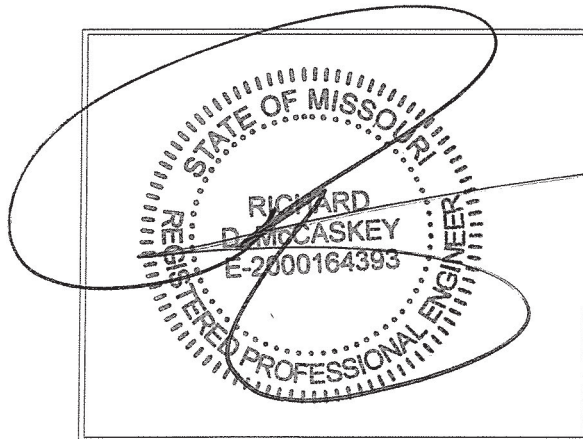
Canopy Calculations :

Design Loads :	1-2
Deck Design :	3-6
Purlin Design :	7-14
Header Design :	15-18
Column & Foundation Design :	19-24

Attachments :

Lane SL-316 Deck Panel Properties
Lane Offset Base Plate Design
Design Sketch

Engineer's Seal :



C.O.A. 2001015838

JUL 17 2024

Calculations By:

Lane Supply, Inc.

LSC - 75967 Rev 1

Customer:

Chick-fil-A #02859 Outside Meal Delivery Canopy

By: JO

Project:

30'-0" X 61'-9" Canopy

Check:

Code:

Missouri Building Code 2018

2018 International Building Code

Roof Loads:

Dead Load = 3.00 psf (SL-316 Deck)
 Live/Snow Load = 20.00 psf
 TOTAL = 23.00 psf

Fascia Load:

Height = 10.00 in.
 Dead Load = 5.83 plf

Wind Loads:

Risk Category = II
 V, ULT Speed = 116 m.p.h. Exp C
 V, ASD Speed = 90 m.p.h. Exp C
 Height = 15 ft
 Kd = 0.85
 Kh = 0.85
 G = 0.85
 qz = 14.93 psf

Lateral Load = 1.0 (H)•qz = 16.00 psf
 Deck Uplift = -1.7 (V)•G•qz = -21.58 psf
 Frame Uplift = -1.1 (V)•G•qz = -13.96 psf

Base Shear : V = CS • W = 0.084 • W

Site Class = D
 Risk Category = II
 Ss(0.2) = 0.099
 S1(1.0) = 0.068
 Fa = 1.60
 Fv = 2.40
 SMS = Fa•Ss = 0.16 (11.4-1)
 SM1 = Fv•S1 = 0.16 (11.4-2)
 SDS = 2/3•SMS = 0.11 (11.4-3)
 SD1 = 2/3•SM1 = 0.11 (11.4-4)
 R = 1.25
 CS = (SDS/R) = 0.084 (12.8-2)

Seismic Design Category Based on SDS : A

Seismic Design Category Based on SD1 : B

Design Category : B

Section 7.1.2--Symbols & Notation

C_e =	1.2	Exposure Factor as determined from Table 7.3-1
C_t =	1.2	Thermal factor as determined from Table 7.3-2
D =	Snow Density in pcf as determined from Eq. 7.7-1	
h_b =	Height of balanced snow load determined by dividing P_f by D , in feet.	
h_d =	Height of snow drift, in feet	
h_c =	Clear height from top of balanced snow to top of parapet, ft	
h_r =	6.00	= Fascia height, ft
I_s =	1.0	= Importance factor (see Table 1.5-2).
P_f =	Snow load on flat roofs, psf.	
P_g =	20	= ground snow, psf.
P_d =	Maximum intensity of drift surcharge load, psf.	
l_u =	30.42	= Length of roof upwind of the drift, feet
w =	Width of snow drift, in feet	

Section 7.3--Flat-Roof Snow Loads, P_f

The snow load, P_f , on a roof with a slope equal to or less than 5° shall be calculated in psf using equation 7.3-1, but not less than the following minimum values for low slope roofs: where P_g is 20 psf or less $P_f = I(P_g)$, where P_g exceeds 20 psf, $P_f = 20 (I)$.

Section 7.7 & Section 7.8

The geometry of the surcharge load due to snow drifting shall be approximated by a triangle as shown in figure 7.7-2. Drift loads shall be superimposed on the balanced snow load. If h_c/h_b is less than 0.2, drift loads are not required to be applied. The height of such drifts shall be taken as $0.75 \times h_d$ as determined from Fig 7.6-1, with l_u equal to the length of the roof upwind of the projection or parapet wall. If the side of a roof projection is less than 15 ft long, a drift load is not required to be applied to that side. If the height, h_d , is equal to or less than h_c , the drift width shall equal $4h_d$ and the drift height shall equal h_d . If this height exceeds h_c , the drift width, w , shall equal $4h_d^2/h_c$ and the drift height shall equal h_c . However, the drift width w shall not exceed $8h_c$. The maximum intensity of the drift surcharge load, p_d , equals $h_d \times D$ where the snow density, D , is defined by Eq 7.7-

Section 7.10--Rain-On-Snow Surcharge Load

For locations where P_g is 20 psf or less but not zero, all roofs with a slope less than $W/50$, shall have a 5 psf rain-on-snow surcharge load applied to establish the design snow loads. This additional load applies only to the sloped roof (balanced) load case and need not be used in combination with drift, sliding, unbalanced, or partial loads.

$$P_f = 0.7 \times C_e \times C_t \times I_s \times P_g \quad \text{Eq 7.3-1}$$

$$P_f = 20.0 \text{ psf}$$

$$h_d = 0.75 \times (0.43(l_u)^{1/3} \times (P_g + 10)^{1/4 - 1.5}) \quad \text{Ref. Fig. 7.6-1}$$

$$h_d = 1.23 \text{ ft}$$

$$D = 0.13P_g + 14 < 30 \text{ psf} \quad \text{Eq 7.7-1}$$

$$D = 16.60 \text{ psf}$$

$$h_b = 1.20 \text{ ft}$$

$$h_c = h_r - h_b = 4.80 \text{ ft}$$

$$h_c/h_b = 3.98$$

Consider Drift

$$w = 4.92 \text{ ft}$$

$$P_d = D \times h_d < D \times h_c$$

$$P_d = 20.44 \text{ psf}$$

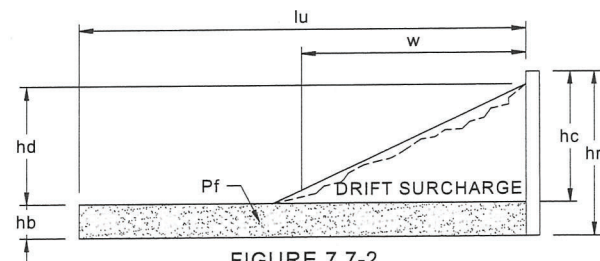
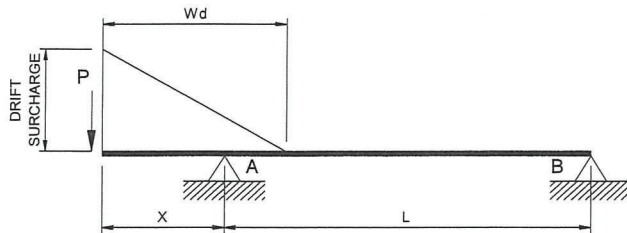


FIGURE 7.7-2
Configuration of Snow Drifts on Lower Roofs

Deck 1

$$+S=.3961 \text{ in}^3 \quad -S=.3036 \text{ in}^3 \quad FY=40 \text{ ksi}$$

$w_d = 3.00$ psf
 $w_s = 20.00$ psf
 Deck : $w_w = -21.58$ psf
 Frame : $w_w = -13.96$ psf
 $P = 5.83$ plf
 (Drift Surcharge) $P_m = 20.44$ psf
 $L = 6.00$ ft
 $X = 1.75$ ft
 (Drift Length) $W_d = 4.92$ ft



$R_{Ad} = 22.55$ plf	$R_{Bd} = 6.53$ plf
$R_{As} = 151.33$ plf	$R_{Bs} = 53.98$ plf
Frame : $R_{Aw} = -69.88$ plf	Frame : $R_{Bw} = -38.32$ plf
Deck : $R_{Aw} = -108.00$ plf	Deck : $R_{Bw} = -59.23$ plf
$R_{A(d+s)} = 173.88$ plf	$R_{B(d+s)} = 60.52$ plf
Frame : $R_{A(d+w)} = -47.33$ plf	Frame : $R_{B(d+w)} = -31.79$ plf
Deck : $R_{A(d+w)} = -85.45$ plf	Deck : $R_{B(d+w)} = -52.69$ plf

$M_{Ad} = -14.80$ ft-lbs/ft	
$M_{As} = -58.21$ ft-lbs/ft	
Deck : $M_{Aw} = 33.04$ ft-lbs/ft	
$M_{A(d+s)} = -73.01$ ft-lbs/ft	$M_{AB(d+s)} = 79.59$ ft-lbs/ft
Deck : $M_{A(d+w)} = 18.24$ ft-lbs/ft	Deck : $M_{AB(d+w)} = -74.73$ ft-lbs/ft

USE 20 GAUGE GRADE C DECK

$+S = .3961 \text{ in}^3$ $-S = .3036 \text{ in}^3$ $FY = 40 \text{ ksi}$

DECK DESIGN:

$W_d = 3.00$ psf	23.00 psf
$W_l = 20.00$ psf	^A
Deck : $W_w = -21.58$ psf	$ \text{-----} $
Frame : $W_w = -13.96$ psf	L
	$ \text{-----} $
$L = 6.00$ ft	B^
	$ \text{-----} $
$M(d+l) = 103.50$ ft-lbs/ft	$R_d = 9.00$ plf
Deck : $M(d+w) = -83.60$ ft-lbs/ft	$R_l = 60.00$ plf
	Frame : $R_w = -41.89$ plf
	Deck : $R_w = -64.73$ plf
	$R(d+l) = 69.00$ plf
	Frame : $R(d+w) = -32.89$ plf
	Deck : $R(d+w) = -55.73$ plf

USE 20 GAUGE GRADE C DECK

$+S = .3961 \text{ in}^3$ $-S = .3036 \text{ in}^3$ $FY = 40 \text{ ksi}$

DECK DESIGN:

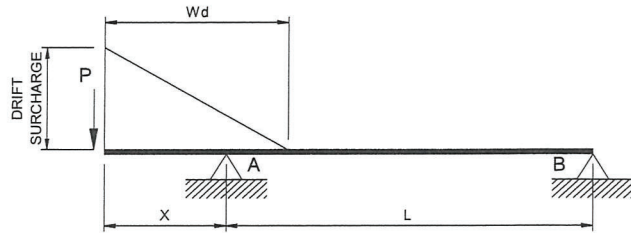
Deck 2

P1						P2	
v	23.00 PSF	o	o	o			v
^A		^B		^C		^D	
<--- X1 ---> <-----L1-----> <---L2---> <---L3---> <-----L4-----> <---X2--->							
	Wd=	3.00	psf		X1=	0.75	ft
	Wl=	20.00	psf		L1=	6.00	ft
Deck :	Ww=	-21.58	psf		L2=	6.00	ft
Frame :	Ww=	-13.96	psf		L3=	6.00	ft
	P1=	5.83	plf		L4=	7.25	ft
	P2=	5.83	plf		X2=	3.00	ft
	MAd=	5.22	ft-lbs/ft		RAd=	17.95	plf
	MAI=	5.63	ft-lbs/ft		RAI=	75.94	plf
Deck :	MAw=	-6.07	ft-lbs/ft		Frame : RAw=	-53.01	plf
	MA(d+l)=	10.84	ft-lbs/ft		RA(d+l)=	93.89	plf
Deck :	MA(d+w)=	-0.85	ft-lbs/ft		Frame : RA(d+w)=	-35.06	plf
	MAB(d+l)=	100.91	ft-lbs/ft		RBd=	17.13	plf
Deck :	MAB(d+w)=	-83.18	ft-lbs/ft		RBI=	120.00	plf
					Frame : RBw=	-83.12	plf
	MBC(d+l)=	103.50	ft-lbs/ft		RB(d+l)=	137.13	plf
Deck :	MBC(d+w)=	-83.60	ft-lbs/ft		Frame : RB(d+w)=	-65.99	plf
	MCD(d+l)=	103.50	ft-lbs/ft		RCd=	18.00	plf
Deck :	MCD(d+w)=	-83.60	ft-lbs/ft		RCI=	120.00	plf
					Frame : RCw=	-83.77	plf
	MDE(d+l)=	136.01	ft-lbs/ft		RC(d+l)=	138.00	plf
Deck :	MDE(d+w)=	-91.25	ft-lbs/ft		Frame : RC(d+w)=	-65.77	plf
	MEd=	31.00	ft-lbs/ft		RDd=	15.60	plf
	MEI=	90.00	ft-lbs/ft		RDI=	132.50	plf
Deck :	MEw=	-97.10	ft-lbs/ft		Frame : RDw=	-83.83	plf
	ME(d+l)=	121.00	ft-lbs/ft		RD(d+l)=	148.10	plf
Deck :	ME(d+w)=	-66.10	ft-lbs/ft		Frame : RD(d+w)=	-68.23	plf
					REd=	29.98	plf
					REI=	144.91	plf
					Frame : REw=	-101.17	plf
					RE(d+l)=	174.90	plf
					Frame : RE(d+w)=	-71.18	plf

USE 20 GAUGE GRADE C DECK

+S=.3961 in^3 -S=.3036 in^3 FY=40 ksi

$w_d = 3.00$ psf
 $w_s = 20.00$ psf
 Deck : $w_w = -21.58$ psf
 Frame : $w_w = -13.96$ psf
 $P = 5.83$ plf
 (Drift Surcharge) $P_m = 20.44$ psf
 $L = 6.00$ ft
 $X = 0.75$ ft
 (Drift Length) $W_d = 4.92$ ft



$R_{Ad} = 17.95$ plf	$R_{Bd} = 8.13$ plf
$R_{As} = 118.78$ plf	$R_{Bs} = 66.54$ plf
Frame : $R_{Aw} = -53.01$ plf	Frame : $R_{Bw} = -41.23$ plf
Deck : $R_{Aw} = -81.93$ plf	Deck : $R_{Bw} = -63.72$ plf
$R_{A(d+s)} = 136.73$ plf	$R_{B(d+s)} = 74.67$ plf
Frame : $R_{A(d+w)} = -35.06$ plf	Frame : $R_{B(d+w)} = -33.10$ plf
Deck : $R_{A(d+w)} = -63.98$ plf	Deck : $R_{B(d+w)} = -55.59$ plf
$M_{Ad} = -5.22$ ft-lbs/ft	
$M_{As} = -11.08$ ft-lbs/ft	
Deck : $M_{Aw} = 6.07$ ft-lbs/ft	
$M_{A(d+s)} = -16.30$ ft-lbs/ft	$M_{AB(d+s)} = 119.51$ ft-lbs/ft
Deck : $M_{A(d+w)} = 0.85$ ft-lbs/ft	Deck : $M_{AB(d+w)} = -83.04$ ft-lbs/ft

USE 20 GAUGE GRADE C DECK

$+S = .3961$ in³ $-S = .3036$ in³ $FY = 40$ ksi

DECK DESIGN:

$W_d = 3.00$ psf	23.00 psf
$W_l = 20.00$ psf	\hat{A} \hat{B}
Deck : $W_w = -21.58$ psf	<----- L ----->
Frame : $W_w = -13.96$ psf	
$L = 6.00$ ft	$R_d = 9.00$ plf
	$R_l = 60.00$ plf
	Frame : $R_w = -41.89$ plf
	Deck : $R_w = -64.73$ plf
$M(d+l) = 103.50$ ft-lbs/ft	$R(d+l) = 69.00$ plf
Deck : $M(d+w) = -83.60$ ft-lbs/ft	Frame : $R(d+w) = -32.89$ plf
	Deck : $R(d+w) = -55.73$ plf

USE 20 GAUGE GRADE C DECK

$+S = .3961$ in³ $-S = .3036$ in³ $FY = 40$ ksi

BEAM DESIGN: P-a

Wd=	37.55	plf	P	
Wl=	151.33	plf	v	188.88 plf
Ww=	-69.88	plf	^A	B^
Pd=	29.20	lbs	<-----X----->	<----- L ----->
Pl=	0.00	lbs		
Pw=	0.00	lbs		
L=	17.77	ft		
X=	3.79	ft		
Deflections: (inches)				
			DL=	-0.029
			DL+LL=	-0.160
				0.051
				0.268
(+downward, -upward)				
MA(d) =	381	ft-lbs	RA d=	527 lbs
MA(l) =	1088	ft-lbs	RA l=	1980 lbs
MA(w) =	-502	ft-lbs	RA w=	-914 lbs
MA(d+l) =	1468	ft-lbs lu=	RA(d+l)=	2506 lbs
MA(d+w) =	-122	ft-lbs lu=	RA(d+w)=	-388 lbs
			RB d=	312 lbs
			RBI(Max)=	1345 lbs
			RB w=	-593 lbs
MAB(d+l)=	7267	ft-lbs lu=	RB(d+l)=	1657 lbs
MAB(d+w)=	-1216	ft-lbs lu=	RB(d+w)=	-280 lbs

USE: W8X15 Fy = 50 ksi

BEAM DESIGN: P-b

Wd=	25.53	plf	P	
Wl=	120.00	plf	v	145.53 plf
Ww=	-80.21	plf	^A	B^
Pd=	33.51	lbs	<-----X----->	<----- L ----->
Pl=	0.00	lbs		
Pw=	0.00	lbs		
L=	17.77	ft		
X=	3.79	ft		
Deflections: (inches)				
			DL=	-0.028
			DL+LL=	-0.190
				0.052
				0.321
(+downward, -upward)				
MA(d) =	311	ft-lbs	RA d=	375 lbs
MA(l) =	863	ft-lbs	RA l=	1570 lbs
MA(w) =	-577	ft-lbs	RA w=	-1049 lbs
MA(d+l) =	1173	ft-lbs lu=	RA(d+l)=	1944 lbs
MA(d+w) =	-266	ft-lbs lu=	RA(d+w)=	-675 lbs
			RB d=	209 lbs
			RBI(Max)=	1066 lbs
			RB w=	-680 lbs
MAB(d+l)=	5591	ft-lbs lu=	RB(d+l)=	1276 lbs
MAB(d+w)=	-2027	ft-lbs lu=	RB(d+w)=	-471 lbs

USE: W8X10 Fy = 50 ksi

[illegible]

BEAM DESIGN:		P-d		
Wd=	25.60	plf	P	
WI=	132.50	plf	v	158.10 plf
Ww=	-83.83	plf		
Pd=	35.03	lbs		
Pl=	0.00	lbs		
Pw=	0.00	lbs		
L=	17.77	ft		
X=	3.79	ft		
			<div> <div>Deflections: (inches)</div> <div>Overhang</div> <div>Midspan</div> </div>	
			<div> <div>DL=</div> <div>-0.028</div> <div>0.052</div> </div>	
			<div> <div>DL+LL=</div> <div>-0.207</div> <div>0.349</div> </div>	
			<div>(+downward, -upward)</div>	
MA(d) =	317	ft-lbs	RA(d) =	377 lbs
MA(l) =	952	ft-lbs	RA(l) =	1733 lbs
MA(w) =	-603	ft-lbs	RA(w) =	-1097 lbs
MA(d+l) =	1269	ft-lbs	lu=	1.33 ft
MA(d+w) =	-286	ft-lbs	lu=	3.79 ft
			<div> <div>RB(d) =</div> <div>210 lbs</div> </div>	
			<div> <div>RBI(Max) =</div> <div>1177 lbs</div> </div>	
			<div> <div>RBw =</div> <div>-711 lbs</div> </div>	
MAB(d+l) =	6084	ft-lbs	lu=	8.89 ft
MAB(d+w) =	-2158	ft-lbs	lu=	1.33 ft
			<div> <div>RB(d+l) =</div> <div>1387 lbs</div> </div>	
			<div> <div>RB(d+w) =</div> <div>-501 lbs</div> </div>	
USE: W8X10			Fy =	50 ksi

BEAM DESIGN:

P-e

Wd=	39.98	plf	P	
Wl=	144.91	plf	v	184.90 plf
Ww=	-101.17	plf	^A	B^
Pd=	42.27	lbs	<-----X----->	<----- L ----->
Pl=	0.00	lbs		
Pw=	0.00	lbs	Deflections: (inches)	Overhang
L=	17.77	ft	DL=	-0.046
X=	3.79	ft	DL+LL=	-0.242
			(+downward, -upward)	Midspan
MA(d) =	448	ft-lbs		RA(d)= 574 lbs
MA(l) =	1042	ft-lbs		RA(l)= 1896 lbs
MA(w) =	-727	ft-lbs		RA(w)= -1323 lbs
MA(d+l) =	1489	ft-lbs lu=	1.33 ft	RA(d+l)= 2470 lbs
MA(d+w) =	-280	ft-lbs lu=	3.79 ft	RA(d+w)= -749 lbs
				RB(d)= 330 lbs
				RBI(Max)= 1288 lbs
				RBw= -858 lbs
MAB(d+l)=	7077	ft-lbs lu=	8.89 ft	RB(d+l)= 1618 lbs
MAB(d+w)=	-2277	ft-lbs lu=	1.33 ft	RB(d+w)= -528 lbs

USE: W8X10

Fy =

50 ksi

BEAM DESIGN:

P-f

Wd=	32.95	plf		
Wl=	118.78	plf		
Ww=	-53.01	plf		
L=	20.00	ft		
	151.73	plf		
	^		^	
	<----- L ----->			
Md=	1647.7	ft-lbs	Rd=	330 lbs
MI=	5938.8	ft-lbs	RI=	1188 lbs
Mw=	-2650.7	ft-lbs	Rw=	-530 lbs
			R(d+l)=	1517 lbs
			R(d+w)=	-201 lbs
M(d+l)=	7586.4	ft-lbs	Lu=	6.67 ft
M(d+w)=	-1003.0	ft-lbs	Lu=	1.33 ft

OK

OK

USE: W8X15

Fy =

50 ksi

Deflections: (inches)	Midspan
DL=	0.085
DL+LL=	0.392
(+downward, -upward)	

BEAM DESIGN: P-g

Wd= 32.13 plf
Wl= 126.54 plf
Ww= -83.12 plf
L= 20.00 ft

158.67 plf
^ ^

|<----- L ----->|

Md= 1606.5 ft-lbs
Ml= 6326.8 ft-lbs
Mw= -4156.0 ft-lbs

Rd= 321 lbs
Rl= 1265 lbs
Rw= -831 lbs
R(d+l)= 1587 lbs
R(d+w)= -510 lbs

M(d+l)= 7933.4 ft-lbs
M(d+w)= -2549.5 ft-lbs

Lu= 6.67 ft
Lu= 1.33 ft

OK
OK

USE: W8X15 Fy = 50 ksi

Deflections: (inches) Midspan
DL= 0.083
DL+LL= 0.410
(+downward, -upward)

BEAM DESIGN: P-h

Wd= 31.00 plf
Wl= 120.00 plf
Ww= -83.77 plf
L= 20.00 ft

151.00 plf
^ ^

|<----- L ----->|

Md= 1550.0 ft-lbs
Ml= 6000.0 ft-lbs
Mw= -4188.7 ft-lbs

Rd= 310 lbs
Rl= 1200 lbs
Rw= -838 lbs
R(d+l)= 1510 lbs
R(d+w)= -528 lbs

M(d+l)= 7550.0 ft-lbs
M(d+w)= -2638.7 ft-lbs

Lu= 6.67 ft
Lu= 1.33 ft

OK
OK

USE: W8X13 Fy = 50 ksi

Deflections: (inches) Midspan
DL= 0.097
DL+LL= 0.473
(+downward, -upward)

BEAM DESIGN: P-i

Wd= 28.60 plf
 Wl= 132.50 plf
 Ww= -83.83 plf
 L= 20.00 ft

161.10 plf
 ^ ^

|<----- L ----->|

Md= 1430.0 ft-lbs
 Ml= 6625.0 ft-lbs
 Mw= -4191.7 ft-lbs

 M(d+l)= 8055.0 ft-lbs
 M(d+w)= -2761.7 ft-lbs

Rd= 286 lbs
 Rl= 1325 lbs
 Rw= -838 lbs
 R(d+l)= 1611 lbs
 R(d+w)= -552 lbs

Lu= 6.67 ft OK
 Lu= 1.33 ft OK

USE: W8X13 Fy = 50 ksi

Deflections: (inches) Midspan
 DL= 0.090
 DL+LL= 0.505
 (+downward, -upward)

BEAM DESIGN: P-j

Wd= 44.98 plf
 Wl= 144.91 plf
 Ww= -101.17 plf
 L= 20.00 ft

189.90 plf
 ^ ^

|<----- L ----->|

Md= 2249.2 ft-lbs
 Ml= 7245.7 ft-lbs
 Mw= -5058.3 ft-lbs

 M(d+l)= 9494.9 ft-lbs
 M(d+w)= -2809.1 ft-lbs

Rd= 450 lbs
 Rl= 1449 lbs
 Rw= -1012 lbs
 R(d+l)= 1899 lbs
 R(d+w)= -562 lbs

Lu= 6.67 ft OK
 Lu= 1.33 ft OK

USE: W8X15 Fy = 50 ksi

Deflections: (inches) Midspan
 DL= 0.116
 DL+LL= 0.491
 (+downward, -upward)

BEAM DESIGN: P-k

Wd=	37.55	plf	P	
WI=	151.33	plf	v	188.88 plf
Ww=	-69.88	plf	^A	B^
Pd=	29.20	lbs	<----X---->	<----- L ----->
PI=	0.00	lbs		
Pw=	0.00	lbs		
L=	18.83	ft		
X=	1.31	ft		
Deflections: (inches)				
			DL=	-0.016
			DL+LL=	-0.083
				0.379
(+downward, -upward)				
MA(d) =	71	ft-lbs	RA d=	436 lbs
MA(l) =	130	ft-lbs	RA l=	1631 lbs
MA(w) =	-60	ft-lbs	RA w=	-753 lbs
MA(d+l) =	201	ft-lbs lu=	RA(d+l)=	2066 lbs
MA(d+w) =	10	ft-lbs lu=	RA(d+w)=	-317 lbs
			RB d=	350 lbs
			RBI(Max)=	1425 lbs
			RB w=	-655 lbs
MAB(d+l)=	8339	ft-lbs lu=	RB(d+l)=	1775 lbs
MAB(d+w)=	-1439	ft-lbs lu=	RB(d+w)=	-305 lbs
USE: W8X15 Fy = 50 ksi				

BEAM DESIGN: P-l

Wd=	25.53	plf	P	
WI=	120.00	plf	v	145.53 plf
Ww=	-80.21	plf	^A	B^
Pd=	33.51	lbs	<----X---->	<----- L ----->
PI=	0.00	lbs		
Pw=	0.00	lbs		
L=	18.83	ft		
X=	1.31	ft		
Deflections: (inches)				
			DL=	-0.017
			DL+LL=	-0.100
				0.454
(+downward, -upward)				
MA(d) =	66	ft-lbs	RA d=	311 lbs
MA(l) =	103	ft-lbs	RA l=	1293 lbs
MA(w) =	-69	ft-lbs	RA w=	-864 lbs
MA(d+l) =	169	ft-lbs lu=	RA(d+l)=	1604 lbs
MA(d+w) =	-3	ft-lbs lu=	RA(d+w)=	-553 lbs
			RB d=	237 lbs
			RBI(Max)=	1130 lbs
			RB w=	-752 lbs
MAB(d+l)=	6420	ft-lbs lu=	RB(d+l)=	1367 lbs
MAB(d+w)=	-2423	ft-lbs lu=	RB(d+w)=	-515 lbs
USE: W8X10 Fy = 50 ksi				

BEAM DESIGN:

P-m

Wd=	28.00	plf	P	
WI=	120.00	plf	v	148.00 plf
Ww=	-83.77	plf	^A	B^
Pd=	35.00	lbs	<-----X----->	<----- L ----->
PI=	0.00	lbs		
Pw=	0.00	lbs		
L=	18.83	ft		
X=	1.31	ft		
Deflections: (inches)				
		DL=	-0.019	0.086
		DL+LL=	-0.102	0.462
(+downward, -upward)				
MA(d) =	70	ft-lbs	RA d=	339 lbs
MA(l) =	103	ft-lbs	RA l=	1293 lbs
MA(w) =	-72	ft-lbs	RA w=	-903 lbs
MA(d+l) =	173	ft-lbs lu=	RA(d+l)=	1632 lbs
MA(d+w) =	-2	ft-lbs lu=	RA(d+w)=	-564 lbs
			RB d=	260 lbs
			RBI(Max)=	1130 lbs
			RB w=	-785 lbs
MAB(d+l)=	6527	ft-lbs lu=	RB(d+l)=	1390 lbs
MAB(d+w)=	-2472	ft-lbs lu=	RB(d+w)=	-525 lbs

USE: W8X10

Fy =

50 ksi

BEAM DESIGN:

P-n

Wd=	25.60	plf	P	
WI=	132.50	plf	v	158.10 plf
Ww=	-83.83	plf	^A	B^
Pd=	35.03	lbs	<-----X----->	<----- L ----->
PI=	0.00	lbs		
Pw=	0.00	lbs		
L=	18.83	ft		
X=	1.31	ft		
Deflections: (inches)				
		DL=	-0.017	0.078
		DL+LL=	-0.109	0.493
(+downward, -upward)				
MA(d) =	68	ft-lbs	RA d=	313 lbs
MA(l) =	114	ft-lbs	RA l=	1428 lbs
MA(w) =	-72	ft-lbs	RA w=	-903 lbs
MA(d+l) =	182	ft-lbs lu=	RA(d+l)=	1741 lbs
MA(d+w) =	-4	ft-lbs lu=	RA(d+w)=	-590 lbs
			RB d=	237 lbs
			RBI(Max)=	1248 lbs
			RB w=	-786 lbs
MAB(d+l)=	6976	ft-lbs lu=	RB(d+l)=	1485 lbs
MAB(d+w)=	-2580	ft-lbs lu=	RB(d+w)=	-548 lbs

USE: W8X10

Fy =

50 ksi

BEAM DESIGN: P-o

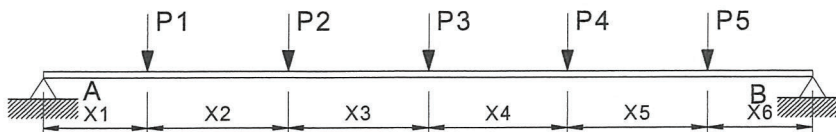
Wd=	39.98	plf		P	
WI=	144.91	plf		v	184.90 plf
Ww=	-101.17	plf		^A	B^
Pd=	42.27	lbs		<-----X----->	<----- L ----->
Pl=	0.00	lbs			
Pw=	0.00	lbs			
L=	18.83	ft			
X=	1.31	ft			
			Deflections: (inches)	Overhang	Midspan
			DL=	-0.027	0.123
			DL+LL=	-0.127	0.577
			(+downward, -upward)		
MA(d) =	90	ft-lbs		RA d=	476 lbs
MA(l) =	125	ft-lbs		RA l=	1561 lbs
MA(w) =	-87	ft-lbs		RA w=	-1090 lbs
MA(d+l) =	215	ft-lbs lu=	1.33 ft	RA(d+l)=	2037 lbs
MA(d+w) =	3	ft-lbs lu=	1.31 ft	RA(d+w)=	-614 lbs
				RB d=	372 lbs
				RB l(Max)=	1365 lbs
				RB w=	-948 lbs
MAB(d+l)=	8153	ft-lbs lu=	9.42 ft	RB(d+l)=	1736 lbs
MAB(d+w)=	-2714	ft-lbs lu=	1.33 ft	RB(d+w)=	-576 lbs

USE: W8X10 Fy = 50 ksi

HEADER BEAM DESIGN:

H-a

P1d= 527 lbs
P1l= 1980 lbs
P1w= -914 lbs
P2d= 375 lbs
P2l= 1570 lbs
P2w= -1049 lbs
P3d= 409 lbs
P3l= 1570 lbs
P3w= -1096 lbs
P4d= 377 lbs
P4l= 1733 lbs
P4w= -1097 lbs
P5d= 574 lbs
P5l= 1896 lbs
P5w= -1323 lbs



X1 = 1.38 ft
X2 = 6.00 ft
X3 = 6.00 ft
X4 = 6.00 ft
X5 = 7.25 ft
X6 = 1.00 ft
L = 27.63 ft

Wd= 58.00 plf
Wl= 0.00 plf
Ww= 0.00 plf

RA(d)= 1921 lbs
RA(l)= 4428 lbs
RA(w)= -2579 lbs
RA(d+l)= 6348 lbs
RA(d+w)= -658 lbs

Deflection

dl= 0.24 in
dl+l= 0.77 in

RB(d)= 1943 lbs
RB(l)= 4320 lbs
RB(w)= -2901 lbs
RB(d+l)= 6264 lbs
RB(d+w)= -957 lbs

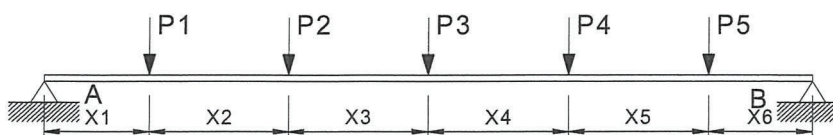
Mspan(d+l) = 37979.5 ft-lbs lu= 7.25 ft
Mspan(d+w) = -5290.7 ft-lbs lu= 7.25 ft

USE: W8X58 Fy = 50 ksi

HEADER BEAM DESIGN:

H-b

P1d= 642 lbs
P1l= 2532 lbs
P1w= -1123 lbs
P2d= 531 lbs
P2l= 2332 lbs
P2w= -1511 lbs
P3d= 540 lbs
P3l= 2266 lbs
P3w= -1548 lbs
P4d= 496 lbs
P4l= 2502 lbs
P4w= -1549 lbs
P5d= 780 lbs
P5l= 2737 lbs
P5w= -1870 lbs



X1 = 1.38 ft
X2 = 6.00 ft
X3 = 6.00 ft
X4 = 6.00 ft
X5 = 7.25 ft
X6 = 1.00 ft
L = 27.63 ft

Wd= 67.00 plf
Wl= 0.00 plf
Ww= 0.00 plf

RA(d)= 2379 lbs
RA(l)= 6131 lbs
RA(w)= -3504 lbs
RA(d+l)= 8510 lbs
RA(d+w)= -1125 lbs

RB(d)= 2460 lbs
RB(l)= 6238 lbs
RB(w)= -4098 lbs
RB(d+l)= 8698 lbs
RB(d+w)= -1638 lbs

Deflection

dl= 0.25 in
dl+ll= 0.89 in

Mspan(d+l) = 52564.0 ft-lbs lu= 7.25 ft
Mspan(d+w) = -9379.9 ft-lbs lu= 7.25 ft

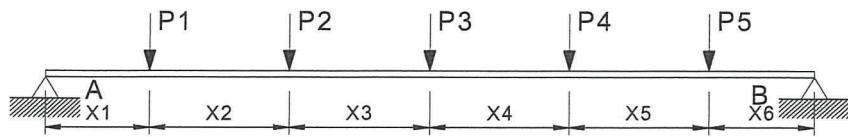
USE: W8X67

Fy = 50 ksi

HEADER BEAM DESIGN:

H-c

P1d= 679 lbs
P1l= 2613 lbs
P1w= -1185 lbs
P2d= 558 lbs
P2l= 2395 lbs
P2w= -1583 lbs
P3d= 570 lbs
P3l= 2330 lbs
P3w= -1623 lbs
P4d= 523 lbs
P4l= 2573 lbs
P4w= -1624 lbs
P5d= 822 lbs
P5l= 2814 lbs
P5w= -1960 lbs



X1 = 1.38 ft
X2 = 6.00 ft
X3 = 6.00 ft
X4 = 6.00 ft
X5 = 7.25 ft
X6 = 1.00 ft
L = 27.63 ft

Wd= 67.00 plf
Wl= 0.00 plf
Ww= 0.00 plf

RA(d)= 2460 lbs
RA(l)= 6311 lbs
RA(w)= -3679 lbs
RA(d+l)= 8771 lbs
RA(d+w)= -1219 lbs

Deflection

dl= 0.26 in
dl+ll= 0.91 in

RB(d)= 2543 lbs
RB(l)= 6414 lbs
RB(w)= -4295 lbs
RB(d+l)= 8957 lbs
RB(d+w)= -1752 lbs

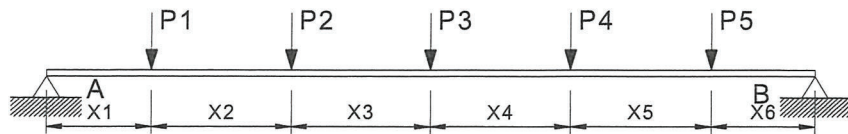
Mspan(d+l) = 54091.1 ft-lbs lu= 7.25 ft
Mspan(d+w) = -10082.3 ft-lbs lu= 7.25 ft

USE: W8X67 Fy = 50 ksi

HEADER BEAM DESIGN:

H-d

P1d= 436 lbs
P1l= 1631 lbs
P1w= -753 lbs
P2d= 311 lbs
P2l= 1293 lbs
P2w= -864 lbs
P3d= 339 lbs
P3l= 1293 lbs
P3w= -903 lbs
P4d= 313 lbs
P4l= 1428 lbs
P4w= -903 lbs
P5d= 476 lbs
P5l= 1561 lbs
P5w= -1090 lbs



X1 = 1.38 ft
X2 = 6.00 ft
X3 = 6.00 ft
X4 = 6.00 ft
X5 = 7.25 ft
X6 = 1.00 ft
L = 27.63 ft

Wd= 48.00 plf
Wl= 0.00 plf
Ww= 0.00 plf

RA(d)= 1591 lbs
RA(l)= 3647 lbs
RA(w)= -2124 lbs
RA(d+l)= 5238 lbs
RA(d+w)= -533 lbs

RB(d)= 1610 lbs
RB(l)= 3559 lbs
RB(w)= -2389 lbs
RB(d+l)= 5169 lbs
RB(d+w)= -779 lbs

Deflection

dl= 0.25 in
dl+l= 0.78 in

Mspan(d+l) = 31342.8 ft-lbs lu= 7.25 ft
Mspan(d+w) = -4297.5 ft-lbs lu= 7.25 ft

USE: W8X48 Fy = 50 ksi

Column Design

Col Line A

AISC 15th ed, Use First Order Analysis Criteria

P DL =	2.46 kips	Clr. Ht.=	10.17 ft
P LL =	6.31 kips	Fascia Ht.=	1.00 ft
P WL =	-3.68 kips	Col. Trib=	19.36 ft
Base Shear =	0.21 kips	Wind Load=	16.00 psf
Total Base Shear =	1.43 kips	# of COL=	2
M WL =	$w(\text{Fascia Ht} \cdot 2.5 \cdot \text{Col Trib} / \# \text{ of col} \cdot L) + w(\text{Wrap} \cdot 1/2 \text{ Clr. Ht}^2)$		Max All. Defl = 1.28 in
M Seis =	Base Shear x L		Max Defl Ratio = L/ 100
M Unbal =	Live Load x Col. Trib.x (Canopy Width/2)^2/2		Max Defl. = 0.16 in, OK
L =	Clr. Ht. + Fascia Ht/2		
Pr =	8.77 kips 1.6Pr<0.5Py First-Order Analysis Allowed (A-7-1)		
Py =	326.60 kips		
N =	0.00 •Yi (A-7-2)		
B2 =	1.05 OK, A-8-6		
M WL =	4.96 kip-ft		
M Seis =	2.25 kip-ft		
M DL(Nod) =	0.11 kip-ft		
M LL(Nod) =	0.28 kip-ft		
M Unbal DL=	0.00 kip-ft		
M Unbal LL=	0.00 kip-ft		
M Unbal WL=	0.00 kip-ft		

Use: TS8X8X1/4

Fy =	46.00 ksi
K =	1.00
L, Col =	10.67 ft
A =	7.10 in^2
I =	70.70 in^4
Cm =	1.00
Pe1 =	393.14 kips
B1 =	1.04 (A-8-3)

P, All =	175.01 kips
M, All =	44.10 kip-ft

Load Combination	Pr, Kips	Mr, Kip-ft	Equation	Result
D+L	8.77	0.41	0.03	OK
D+W	2.46	5.25	0.13	OK
D+0.7E	2.46	1.75	0.05	OK
D+0.75W+0.75L	7.19	4.19	0.12	OK
D+0.525E+0.75L	7.19	1.56	0.06	OK

Top Connection : Standard Cap Plate

Offset Base Plate : MODLBP 8 - 20

Spread Footing Design

Col Line A

From Column

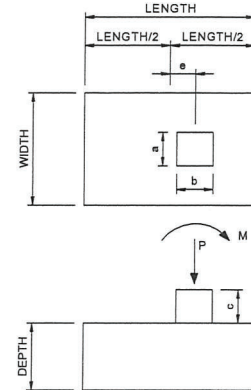
P dl = 2.46 kips
P ll = 6.31 kips
P wl = -3.68 kips

Soil Density = 110 pcf
Width = 5.50 ft
Length = 5.50 ft
Depth = 3.00 ft
e = 2.08 ft
a = 0.00 ft
b = 0.00 ft
c = 0.00 ft

Unbalanced Load to Column

M dl = 0.00 kip-ft
M ll = 0.00 kip-ft
M wl = 0.00 kip-ft
From Lateral
M wl = 4.96 kip-ft
M el = 2.82 kip-ft

Kern = L/6 = 0.92 ft
Footing Weight = 13.61 kips
Soil Weight = 0.00 kips



Total Loads to Spread Footing

PDL = 16.07 kips MDL = 5.13 kip-ft
PLL = 6.31 kips MLL = 13.15 kip-ft
PWL = -3.68 kips MWL = -2.71 kip-ft
MEL = 2.82 kip-ft

Load Combination	Pr, Kips	Mr, Kip-ft	ecc, ft	Soil Pressure psf
D+L	22.38	18.27	0.82	1068.92
D+W	12.39	2.42	0.19	166.84
0.6D+W	10.05	0.37	0.04	15.37
0.6D+0.7E	13.73	5.05	0.37	305.84
D+0.7E	16.07	7.10	0.44	457.31
D+0.75W+0.75L	18.05	12.95	0.72	733.74
D+0.75(0.7E)+0.75L	20.81	16.47	0.79	951.59

q(ALLOW)= 1500.00 psf OK

REINFORCING:

M = 17867.31 ft-lbs/ft

Assume: $f'_c=3000$ psi, $F_y=40000$ psi

d = 32.63 in

As(REQ'D)= 0.38 in²

As(PROV.)= 0.59 in² OK

USE #6's AT 9"O.C. T&B, EACH WAY

Foundation: (Restrained at Grade)

Col Line A

$$d^2 = (4.25 * M) / (S3 * b)$$

M(MAX)= 20603 ft-lbs
S3= 100 PCF X d
b= 3.000 ft
d= 6.633 ft

Pmax= 8.77 kips
Footing Area= 7.07 ft^2
Bearing= 1240.83 psf

Footing= Round

USE: 3.00 FT.RND. X 6.75 ft deep footing

$$A_s = 12 * M / (j * d * 24000) = 0.2759 \text{ in}^2$$

USE: 10 #8's (RND. Cage) w/ #4 Ties @ 4" O.C. w/135 degree hooks In The Top

3'-0" of The Footing, #4 Ties @ 12" o.c w/ 135 Degree Hooks In The Balance of Footing

Footing design to allow offset column placement of 1'-9" from centerline of footing.

Column Design

Col Line B

AISC 15th ed, Use First Order Analysis Criteria

P DL =	2.54 kips	Clr. Ht.=	10.17 ft
P LL =	6.41 kips	Fascia Ht.=	1.00 ft
P WL =	-4.29 kips	Col. Trib=	19.36 ft
Base Shear =	0.21 kips	Wind Load=	16.00 psf

$$M WL = w(\text{Fascia Ht} \cdot 2.5 \cdot \text{Col Trib} \cdot \# \text{ of col} \cdot L) + w(\text{Wrap} \cdot 1/2 \text{ Clr. Ht}^2)$$

$$\# \text{ of COL} = 2$$

$$M Seis = \text{Base Shear} \times L$$

$$\text{Max All. Defl} = 1.28 \text{ in}$$

$$M Unbal = \text{Live Load} \times \text{Col. Trib.} \times (\text{Canopy Width}/2)^2/2$$

$$\text{Max Defl.} = L/100$$

$$L = \text{Clr. Ht.} + \text{Fascia Ht}/2$$

$$Pr = 8.96 \text{ kips} \quad 1.6Pr < 0.5Py \text{ First-Order Analysis Allowed (A-7-1)}$$

$$Py = 326.60 \text{ kips}$$

$$N = 0.00 \text{ } \phi Y_i \text{ (A-7-2)}$$

$$B2 = 1.05 \text{ OK, A-8-6}$$

$$M WL = 4.96 \text{ kip-ft}$$

$$M Seis = 2.25 \text{ kip-ft}$$

$$M DL(\text{Nod}) = 0.11 \text{ kip-ft}$$

$$M LL(\text{Nod}) = 0.29 \text{ kip-ft}$$

$$M Unbal DL = 0.00 \text{ kip-ft}$$

$$M Unbal LL = 0.00 \text{ kip-ft}$$

$$M Unbal WL = 0.00 \text{ kip-ft}$$

Use: TS8X8X1/4

$$F_y = 46.00 \text{ ksi}$$

$$K = 1.00$$

$$L, \text{ Col} = 10.67 \text{ ft}$$

$$A = 7.10 \text{ in}^2$$

$$I = 70.70 \text{ in}^4$$

$$C_m = 1.00$$

$$P_{e1} = 393.14 \text{ kips}$$

$$B1 = 1.04 \text{ (A-8-3)}$$

$$P_{\text{All}} = 175.01 \text{ kips}$$

$$M_{\text{All}} = 44.10 \text{ kip-ft}$$

Load Combination	Pr, Kips	Mr, Kip-ft	Equation	Result
D+L	8.96	0.42	0.04	OK
D+W	2.54	5.26	0.13	OK
D+0.7E	2.54	1.76	0.05	OK
D+0.75W+0.75L	7.35	4.20	0.12	OK
D+0.525E+0.75L	7.35	1.57	0.06	OK

Top Connection : Standard Cap Plate

Base Plate : LBP 8 - 20

Foundation: (Restrained at Grade)

$$d^2 = (4.25 \cdot M) / (S_3 \cdot b)$$

$$M(\text{MAX}) = 5262 \text{ ft-lbs}$$

$$P_{\text{max}} = 8.96 \text{ kips}$$

$$S_3 = 100 \text{ PCF} \times d$$

$$\text{Footing Area} = 7.07 \text{ ft}^2$$

$$b = 3.000 \text{ ft}$$

$$\text{Bearing} = 1267.17 \text{ psf}$$

$$d = 4.209 \text{ ft}$$

Footing= Round

USE: 3.00 FT.RND. X 5.00 ft deep footing

$$A_s = 12 \cdot M / (j \cdot d \cdot 24000) = 0.0705 \text{ in}^2$$

USE: 8 #8's (RND. Cage) w/ #4 Ties @ 12" O.C. w/135 hooks

$P_{dl} = 2543$ lbs
 $P_{ll} = 6414$ lbs
 $P_w = -4295$ lbs
 $M = 5262$ ft-lbs
 Soil Density = 110 pcf

Width = 4.50 ft
 Length = 4.50 ft
 Depth = 3.00 ft

$a = 0.00$ ft
 $b = 0.00$ ft
 $c = 0.00$ ft

Footing Weight = 9112.5 lbs
 Soil Weight = 0 lbs

Overturning : $OTM = 5262$ ft-lbs
 $RM = 14273$ ft-lbs
 $FS = 2.71 > 1.5$ Therefore OK

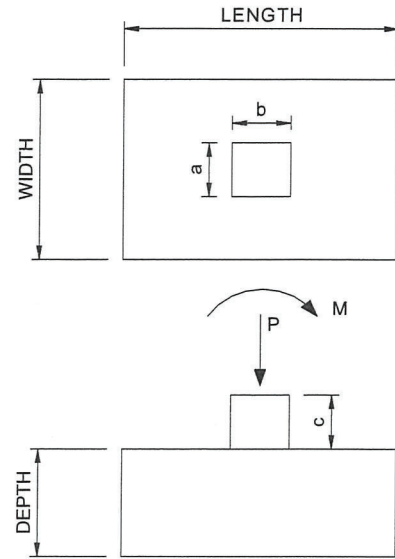
Soil Pressure : $q_{(dl+ll)} = 442.33$ psf Net
 $q_{(ALLOW)} = 1500$ psf OK

For $dl+(wl, seismic)$:

$P = 11656$ lbs
 $e = M/P = 0.45$ ft
 $L/6 = 0.75$ ft
Resultant within middle 3rd
 $q_{(dl+wl, seismic)} = 472.04$ psf Net
 $q_{(ALLOW)} = 1500.00$ psf OK

REINFORCING: $M_{(dl+ll)} = 1119.64$ ft-lbs/ft
 $M_{(dl+wl, seismic)} = 574.90$ ft-lbs/ft
dl+ll Controls
 Assume: $f'_c = 3000$ psi, $F_y = 40000$ psi
 $d = 32.63$ in
 $A_s(REQ'D) = 0.02$ in²
 $A_s(PROV.) = 0.44$ in²

USE #6's AT 12" O.C. T&B, EACH WAY



Foundation: (Restrained at Grade)

Col Line B

$$d^2 = (4.25 * M) / (S3 * b)$$

M(MAX)= 20937 ft-lbs
S3= 100 PCF X d
b= 3.000 ft
d= 6.669 ft

Pmax= 8.96 kips
Footing Area= 7.07 ft^2
Bearing= 1267.17 psf

Footing= Round

USE: 3.00 FT.RND. X 6.75 ft deep footing

$$A_s = 12 * M / (j * d * 24000) = 0.2804 \text{ in}^2$$

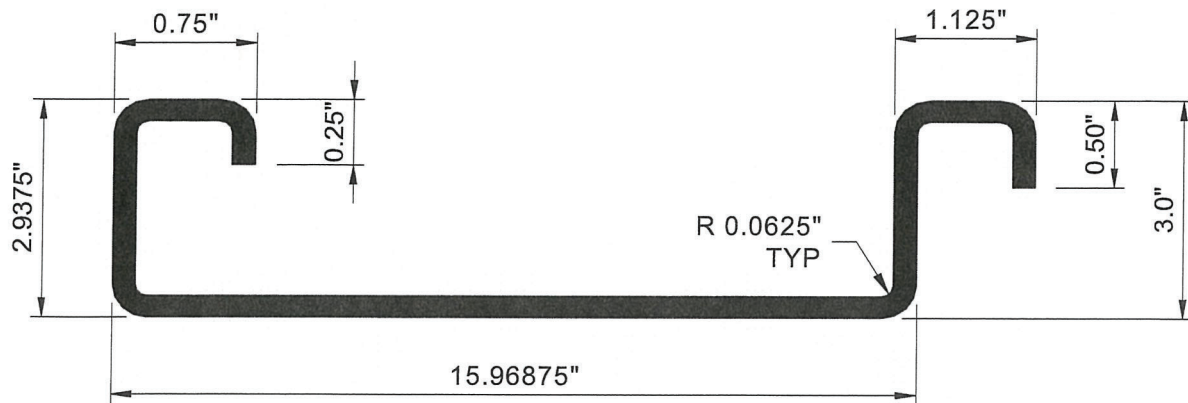
USE: 10 #8's (RND. Cage) w/ #4 Ties @ 4" O.C. w/135 degree hooks In The Top
3'-0" of The Footing, #4 Ties @ 12" o.c w/ 135 Degree Hooks In The Balance of Footing

Footing design to allow offset column placement of 1'-9" from centerline of footing.



LANE SUPPLY, INC.

120 Fairview
Arlington, Texas 76010
817-261-9116



SL-316 DECK PANEL

Section Properties

Gage	Wt, psf	Thickness, in	ASTM 653	+I, in ⁴	-I, in ⁴	+S, in ³	-S, in ³	+M, ft-lbs/ft	-M, ft-lbs/ft
20	2.20	0.0359	Grade 40	0.9346	0.4680	0.3961	0.3036	592.70	454.44
			Grade 50	0.9208	0.4522	0.3879	0.2880	725.86	538.92
18	2.93	0.0478	Grade 40	1.2486	0.6827	0.5329	0.4377	797.77	655.28
			Grade 50	1.2129	0.6518	0.5141	0.4296	962.09	803.92

Notes:

- 1 Designed per AISI Cold Formed Steel Manual, 2016 ed.
- 2 Complete calculations are available upon request.
- 3 $\pm M$ is allowable bending moment.

Issued 12-5-17

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Moment = 20.00 kip-ft
 Column = TS8X8X1/4
 D = 8 in.
 e1 = 8 in.
 e2 = 4 in.
 Anchor Bolts = 1 1/2 in
 t plate = 2.00 in

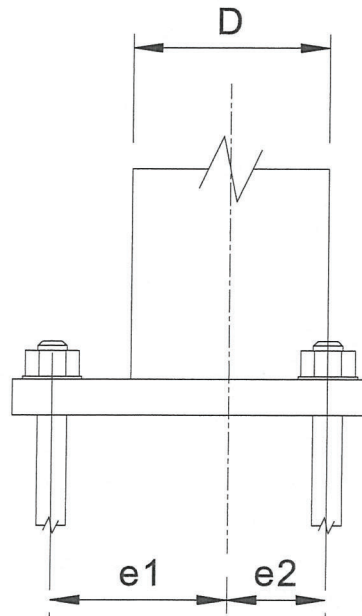
A36 Steel Plate $F_y = 36$ ksi
 E70 Electrode $F_w = 0.928$ kips / in / 16th
 A307 Anchor Bolts $F_t = 20$ ksi

Clockwise Moment

Pbolt = 8.57 kips < 35.3, OK
 t(req'd) = 1.04 in
 t(actual) = 2.00 in
 Weld(req'd) = 3.03 /16th's
 Weld(actual) = F.P. /16th's

Counter-Clockwise Moment

Pbolt = 12.00 kips < 35.3, OK





LANE SUPPLY, INC.

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STANDARD BASE PLATE DESIGN

LBP #	M	P _{BOLT}	Bolt Dia.	t _{REQ'D}	t _{ACTUAL}	Weld Req'd	Weld Actual	Base Plate
(D - M)	(ft-k)	(k)	(in)	(in)	(in)	(1/16 in)	(in)	Mark
8 - 10	10	5.58	1 1/2	0.72	3/4	1.52	1/4	LBP 1
8 - 20	20	10.91	1 1/2	0.99	1	3.03	5/16	LBP 2
8 - 30	30	16.00	1 1/2	1.17	1 1/4	4.55	5/16	LBP 3
8 - 40	40	20.87	1 1/2	1.32	1 1/2	6.06	F.P.	LBP 4
8 - 50	50	26.09	1 1/2	1.46	1 1/2	7.58	F.P.	LBP 5

TS 8 X 8 COLUMN:

D= 8 in.

e= 2 in.

b,d= 8 in.

CONSTANTS:

A36 Steel Plate

E70xx Electrode

A307 Anchor Bolts

Fy = 36 ksi

Fw = 0.928 k/in/16th

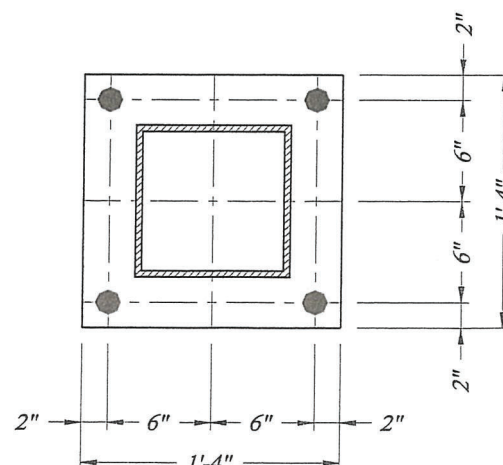
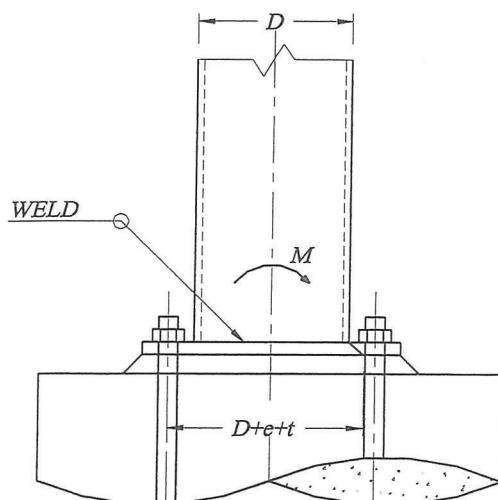
Ft = 20 ksi

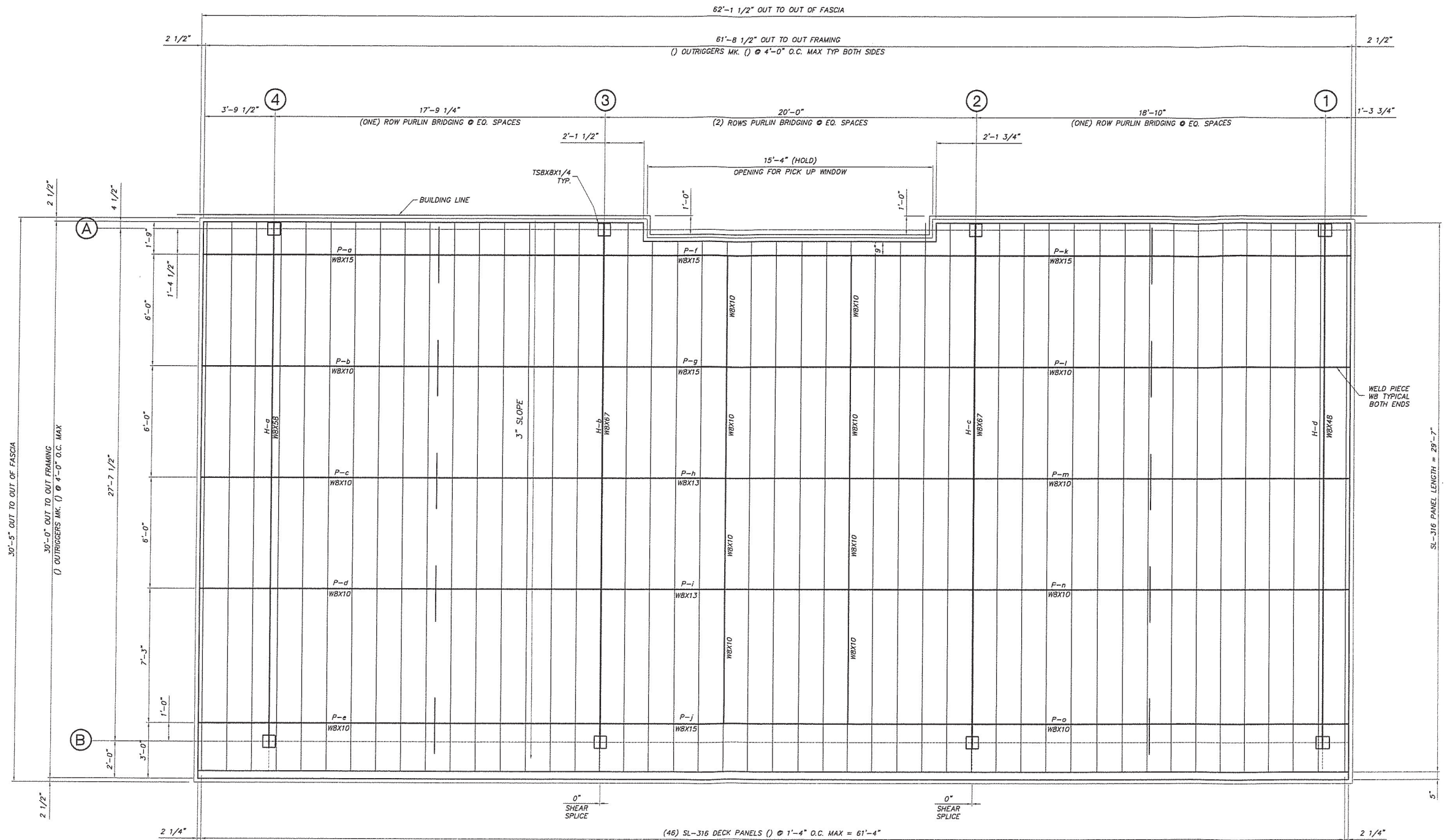
EQUATIONS:

$$P_{BOLT} = \frac{M \times 12 \text{ in/ft}}{2 \text{ bolts } (D+e+t)}$$

$$Weld = \frac{M \times 12 \text{ in/ft}}{S_{Weld} \times Fw} = \frac{M \times 12 \text{ in/ft}}{Fw (bd+d^2/3)}$$

$$t_{REQ'D} = \sqrt{\frac{6 \times P \times e \times 2 \text{ bolts}}{0.75 \times Fy \times (D+2t)}}$$





DESIGN LOADS:

DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) + WEIGHT OF STRUCTURAL COMPONENTS
 LIVE LOAD = 20 p.s.f.
 SNOW LOAD = 20 p.s.f.
 WIND LOAD VULT = 116 m.p.h. EXP. C
 WIND VASD = 90 m.p.h. EXP. C
 BLDG CODE = MISSOURI BUILDING CODE 2018
 ADOPTING 2018 INTERNATIONAL BUILDING CODE
 EQUIVALENT LATERAL FORCE PROCEDURE
 LATERAL FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEM-ORDINARY STEEL MOMENT FRAME
 PI = 20 p.s.f.
 Ce = 1.2
 Ct = 1.2
 Is = 1.0
 W = 4.92
 Pd = 20.44
 SITE CLASS = 0
 Ss (0.2) = 0.099
 SI (1.0) = 0.068
 SDS = 0.11
 SD1 = 0.11
 Fe = 1.60
 Fv = 2.40
 R = 1.25
 IMPORTANCE FACTOR = 1.0
 RISK CATEGORY = II
 SEISMIC DESIGN CATEGORY = D
 CS = 0.084
 CONSTRUCTION TYPE = IIB
 OCCUPANCY CATEGORY = A2
 TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 1.43 KIPS