

MiTek, Inc. 16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200

Re: 240113 Lot 179 HT

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Wheeler - Waverly.

Pages or sheets covered by this seal: I67297520 thru I67297521

My license renewal date for the state of Missouri is December 31, 2025.

Missouri COA: Engineering 001193



August 4,2024

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

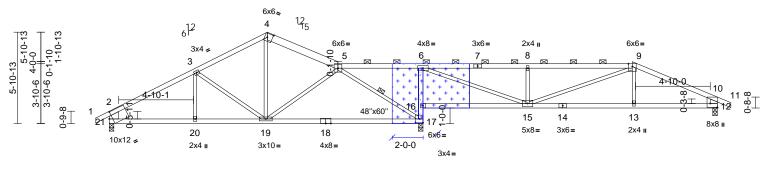
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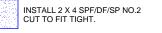






REPAIR REPLACE MEMBER 6-17





ATTACH 7/16" OSB GUSSET (7/16" RATED SHEATHING 24/16 EXP 1) TO EACH FACE OF TRUSS WITH (0.131" X 2.5" MIN.) NAILS PER THE FOLLOWING NAIL SCHEDULE: 2 X 3'S - 2 ROWS, 2 X 4'S - 3 ROWS, 2 X 6'S - 4 ROWS: SPACED @ 4" O.C.NAILS TO BE DRIVEN FROM BOTH FACES. STAGGER SPACING FROM FRONT TO BACK FACE FOR A NET 2" O.C. SPACING IN EACH COVERED TRUSS MEMBER. USE 2" MEMBER END DISTANCE.

		5	-6-9 ₁	10-1-	7		20-3-8		27-0)-12		33	-11-4	. 39	9-4-0
Scale = 1:74.5	ŀ		5-6-9	4-6-14			10-2-1	1	6-9		1		10-8		4-12
Plate Offsets (X, Y): [4:0-3-	15,0-3-0]], [12:Edge,0-5	5-8], [21:0-	-2-7,0-4	-14]									
Loading TCLL (roof) TCDL BCLL		(psf) 25.0 10.0 0.0*	Spacing Plate Grip D0 Lumber DOL Rep Stress II	OL 1.	-0-0 .15 .15 ES		CSI TC BC WB	0.78 0.65 0.57	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.23 -0.49 0.02	(loc) 17-19 17-19 17	l/defl >999 >492 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 197/144
BCDL		10.0	Code			/TPI2014	Matrix-S	0.01	Wind(LL)		13-15	>999	240	Weight: 139 lb	FT = 10%
	2x4 SPF 210 No.2, 14-12, 2x3 SPF No 2.0E, 12-10: Structural w 3-6-12 oc pu 2-0-0 oc pu Rigid ceiling bracing, Ex 6-0-0 oc bra 1 Row at mi (size) 1: Max Horiz 2 Max Uplift 1: 2	00F 1.8E 14-16:2) .2 'Exce 2x6 SP 2 ood shea urlins, ex- rlins (3-9) directly xcept: ucing: 15- dpt 2=0-3-8, 1=101 (L 2=-176 (1 1=-136 (1)	athing directly xcept end verti -5 max.): 5-9. applied or 10-0 -16. 5-17 17=0-3-8, 21= .C 8) LC 9), 17=-286	6:2x3 SPF 2400F applied or icals, and 0-0 oc =0-3-8 6 (LC 9),	- 3) 4) r 5) 6)	Vasd=91mp II; Exp C; E cantilever le right expose Provide ade This truss h chord live le * This truss on the botto 3-06-00 tall chord and a Bearings ar Joint 17 SP Provide me bearing plat 21, 176 lb u This truss is	bh; TCDL=6.0 nclosed; MWF ift and right ey ad; Lumber Dd quate drainag as been desig and nonconcu has been desig and nonconcu has been desig m chord in all by 2-00-00 w ny other men e assumed to F 2100F 1.8E chanical conn e capable of a plift at joint 12 s designed in a state of the stat	be: Joint 21 S , Joint 12 SP ection (by oth withstanding 1 2 and 286 lb u accordance w	Dipsf; h=25ft; a) exterior zc ertical left ai grip DOL=1 water pondir b) psf bottom other live load e load of 20. a rectangle veen the bott SPF 2100F 1 F No.2. ers) of truss 36 lb uplift at plift at joint 1 ith the 2018	one; nd .60 ng. ads. .0psf tom .8E, to to ti joint 17.					
FORCES TOP CHORD	21=936 (LC 1) CES (Ib) - Maximum Compression/Maximum Tension				,	International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. LOAD CASE(S) Standard									
BOT CHORD	8-9=-1396/3 2-21=-820/1 20-21=-170/ 17-19=-70/7 6-16=-1055/	805, 9-10 70, 10-1 ⁄967, 19- ′77, 16-1 ⁄291, 15-	=-1349/252, 1	0-11=0/30	_{),} LO	AD CASE(S	Standard						A start	STATE OF M	Г М. ТЕМ
WEBS		8, 3-19=-2 1, 5-17= 662, 8-1	279/169, 4-19= -1117/151, 5=-526/215,										×.	PE-2001	118807 E

NOTES

1) Unbalanced roof live loads have been considered for this design.

August 4,2024

SSIONAL

 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to dury with with where outputs into design is based only door parameters shown, and is for an individual building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH1 Quality Criteria**, and **DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



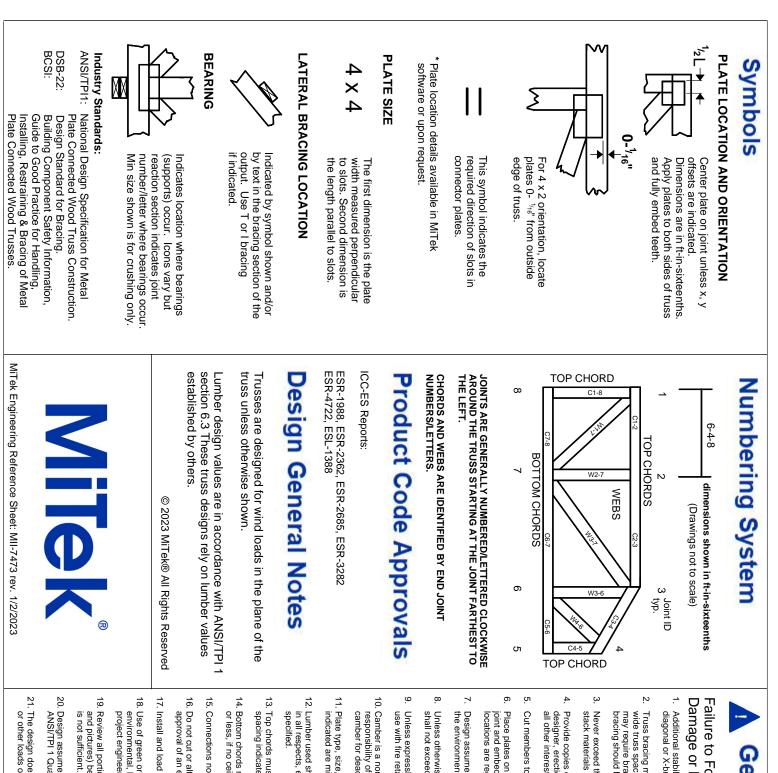
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NULL # ATTACH THAT DO GRAUGET FUND PATED SHATT HUR ALTED SHATT HUR ALT									25 M18	⊠ AHS 5x	:14 =	5x8=		3X	10=		
In the result of the resu		1	0x12 ≠		+ + +				6" RATED SH	IEATHII	NG 24/	16 EXP 1) ⁻	TO EACH FA	CE OF T	RUSS W	/ITH	
Image: State = 174.5 5-6-9 10-10-12 20-3 32-2 32-10-14 39-4-0 39-4-12 99-4-14 99-4-12 99-4-14 99-4-12 99-4-14 99-4-14 99-4-12 99-4-14 99-4-12					+ + + + + + + + + + + + + + + + + + + +	(0.131" 2 X 6'S	X 2.5" MIN.) NA - 4 ROWS: SPA	ALS PER T	HE FOLLOWI O.C.NAILS TO	NG NA O BE D	IL SCH RIVEN	EDULE: 2 FROM BO	X 3'S - 2 ROV TH FACES. S	NS, 2 X 4 STAGGEI	'S - 3 RO R SPACI	OWS, ING FROM FROM	
Link PackED 8/ + 0.2. STANGER NALL PRACING FROM FROM FROM FROM FROM FROM FROM FROM	I	ISTALL 2 X 4 S	SPF/DF/SF	P NO.2		F A	APPLY 2 X 4 X	4' SPF/DF/S	SP NO.2 SCAI	B(S) TC		FACE OF	TRUSS AS S	SHOWN.		.	
State = 174.5 5-6-9 10-10-12 20-3-8 29-11-4 9-4-12 9-4-12 Plate Offsets (X, Y): [r1:03-33.0-22:1] (r1:0-3:15.0-116]. [r1:0-3:15.0-116]. [r1:0-3:15.0-16]. [r1:0-3:15.0-1	C	UT TO FIT TIG	GHT.			5	SPACED @ 4" (D.C. STAGO	GER NAIL SP	ACING	FROM	FRONT FA	ACE AND BA	CK FACE		5.	
State -17.4.5 6-6-9 5-4-3 9-4-12 9-7.12 9-4.12 Landing TCLL (rod) (16.0-3-6,0-2.1] (16.0-3-6,0-4.14] (22-0-2.7,0-4.14] (22-0-2.7,0-4.14] Landing TCLL (rod) (16.0-3-6,0-2.1] (16.0-3-6,0-4.14] (22-0-2.7,0-4.14] (22-0-2.7,0-4.14] Landing TCLL (rod) (16.0) (16.0-3-6,0-4.14] (22-0-2.7,0-4.14] (22-0-2.7,0-4.14] Lumber DOL BCLL (10.0) (11.0-3-6,0-4.14) (22-0-2.7,0-4.14] (22-0-2.7,0-4.14] BCLL (10.0) (25.0) (16.0-2.7,0-4.14) (22-0.2,7,0-4.14) BCL (10.0) (25.0) (16.0-2.7,0-4.14) (22-0.7,0-4.14) (22-0.7,0-4.14) BCL (10.0) (26.0-2,0-2.7,0-4.14) (26.0-2,0-2.7,0-4.14) (26.0-2,0-2.7,0-4.14) (26.0-2,0-2.7,0-4.14) BCL (10.0) (26.0-2,0-2.7,0-2.14) (26.0-2,0-2.7,0-2.14) (26.0-2,0-2.7,0-2.14) (26.0-2,0-2.7,0-2.14) LUMBER (27.0-2,0-2.7,0-2.14) (28.0-2.7,0-2.7,0-2.14) (28.0-2.7,0-2.14) (28.0-2.14,0-2.2,0-2,0-2.2,0-2.2,0-2.2,0-2.2,0-2.2,0-2.			-	.	10						0022			IOL.		00.4.0	
Loading TCLL (rod) (pst) 25.0 (Col Spacing Pite Gip DCL 2-0-0 1.15 (Dumber DCL CSI TC 0.96 (Or Pite Gip DCL DEFL (Vert(C) in (loc) Udel U/ Pite Sign BCL 0.0° Spreading (Col 1.15 (Cod BC 0.96 (Vert(C) 0.45 (Vert(C) 0.42 (Vert(C) 0.43 (Vert(C) 0.43 (Vert(C) 0.43 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.66 (Vert(C) 0.44 (Vert(C) 0.66 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.66 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.45 (Vert(C) 0.45 (Vert(C) 0.45 (Vert(C) 0.45 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.45 (Vert(C) 0.44 (Vert(C) 0.44 (Vert(C) 0.44 (Vert(C) 0.46 (Vert(C) 0.46 (Vert(C) 0.44 (Vert(C) 0.46 (Vert(C) 0.46 (Vert(C) 0.46 (Vert(C) 0.46 (Vert(C)	Scale = 1:74.5	F															
TCLL (root) 25.0 (mode) Piete Gip DOL (mode) 1.15 (mode) TC (mode) 0.98 (mode) Vert(T) (mode) 0.25 (mode) 19-20 (mode) 19-20 (mode) 19-77+44 (mode) BCLL 0.00 (mode) 0.00 (mode) 0.00 (mode) 1.15 (mode) TC 0.98 (mode) Vert(T) 0.42 (mode) 19-20 (mode) 14-20 (mode) 14-21-20 (mode)	Plate Offsets (X	, Y): [4:0-3-3	8,0-2-2], [′	14:0-3-15,	,0-4-14], [2	22:0-2-7,0	0-4-14]	-									
TCDL 10.0 Lumber DOL 1.15 BC 0.04 Ver(CT) -0.44 18-20 >539 240 Mt8AHS 142/136 BCDL 10.0 Code IRC2018/TF12014 Mt8AHS 0.41 Horz (CT) -0.03 14 n/a n/a n/a BCDL 10.0 Code IRC2018/TF12014 Mt8AHS 142/136 Weight: 147 lb FT = 10% LUMBER Code Ver(CT) -0.04 IR-200 539 240 Weight: 147 lb FT = 10% LUMBER Code Ver(CT) -0.04 IR-200 539 240 Weight: 147 lb FT = 10% LUMBER Code Ver(CT) -0.04 IR-200 539 240 No	-		·· /	•									. ,				
BCDL 10.0 Code IRC2018/TPI2014 Matrix:S Wind(L) 0.06 15-17 >999 240 Weight: 147 Ib FT = 10% LUMBER TOP CHORD 2x4 SPF Nu.2 * Except* 4-52:rS SPF Nu.2 BOT CHORD 2x4 SPF Nu.2 * Except* 4-52:rS SPF Nu.2 BOT CHORD 2x4 SPF Nu.2 * Except* 22-2:x8 SPF Nu.2 BOT CHORD 2x4 SPF Nu.2 * Except* 22-2:x8 SP 2407 SUG Line 11:2:r2 Karpt* 22-2:x8 SP 2407 SUG Line 2:r2 Karpt* 22-2:x8 SP 2407 SUG Line 2:r2 Karpt* 22-2:r2 Karpt* 2	· · /											,					
 LUMBER TOP CHORD Zx4 SPF No.2 "Except 4-52x6 SPF No.2. 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vas4=91mph; TCDL=6.0pd; BCDL=6.0pd; B-25tr; Cat. II; Exp C: Enclosed; MWRS (BCDL=6.0pd; B-25tr; Cat. II; Exp C: Enclosed; MWRS (Bcnevlope) exterior zone; canliever left and right exposed; :ed vertical left and right exposed				•	ss Incr		8/TPI2014			0.45	•	,				Weight: 147 lb) FT = 10%
 TOP CHORD 2: 24 SPF No.2 "Except" 4:5:2x6 SPF No.2 BOT CHORD 2: 43 SPF No.1 B:E "Except" 16:8:2x3 SPF No.2 WEES 2: 35 SPF No.2 "Except" 2:2:2x8 SP 2400F 2:0E BAC LING 2: 0.0E; 14:16:17:2x4 SPF 200F 2:0E CHORD 2: 0.0E; 14:16:2x6 SP 2400F 2:0E TOP CHORD 3: Tructural wood sheathing directly applied or 10:0-0 oc braiding. BOT CHORD 1: 14:0:3x8 1:5:10. BOT CHORD 2: 14:0-3:8, 118-0:38, 22=0-38. Max Horiz 2:2:10:1 (LC 8) Max Horiz 1:1:1:1:1:1:1:1:1:1:1:1:1:1:1:1:1:1:1:	-		10.0	0000						3-5000		,		2000	210	Wolght. The	
 WEBS No.2, 16-14; 16-17; 2x4 SPF No.2 WEBS 2x3 SPF No.2 Fixcept 22-2:28 SP 2400F 2.0E BRACING TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2:0-0 oc puting (5:1-12 max); 5:1-0. BOT CHORD Rigid celling directly applied or 10:0-0 oc bracking. WEBS 1 Row at midpt 9-17, 6-18 (LC 9); 18-268 (LC 9), 22-143 (LC 8) Wax Grav 14-887 (LC 24), 18-1934 (LC 2), 22-937 (LC 8) FORCES (b) - Maximum Compression/Maximum ToP CHORD 12-2007, 23-1204/174, 34-a-1141/289, 4-22-937 (LC 2) FORCES (b) - Maximum Compression/Maximum ToP Sim Sha Seed 12 - 2007 1.8E, 23-937 (LC 2), 22-937 (LC 9) BOT CHORD 12-2037, 2:3-1204/174, 34-a-1141/289, 4-23-93429, 6:20-15,670, 9:15-903, 22-22-809/175, 12-14-7802/28, 11-13-1039, 22-22-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-22-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-22-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-22-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-22-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-22-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-22-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-2204124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-204124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-204124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-204124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-204124, 12-15-2031124 WEBS 3-12-2807/124, 12-13-030, 22-23-809/175, 12-14-7802/28, 11-14-15-204124, 12-15-2031124 WEBS 3-12-2807/124, 12-13-1680-125, 12-14-14-124, 12-14, 12-14-14-142, 12-14-14-14-14, 12-14-14-14, 12-14-14-14-14, 12-14-1	TOP CHORD					2	Vasd=91m	oh; TCDL=	=6.0psf; BCD	L=6.0	psf; h=	25ft; Cat.					
 2.0E, 14-12:2x6 SP 2400F 2.0E 3) Provide adequate drainage to prevent water ponding. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates unless otherwise indicated. 4) All plates are MT20 plates are summed in the badd of 20.0psf. 5) This truss has been designed for a live load of 20.0psf. 5) This truss has been designed for a live load of 20.0psf. 6) "This truss has been designed for a live load of 20.0psf. 6) "This truss has been designed for be: Joint 22 SPF 2100F 1.6E. 6) "Max forg 14-as6 (LC 9), 18286 (LC 9), 22-ar-13 (LC 8). 6) "Max forg 14-as6 (LC 9), 18286 (LC 9), 22-ar-13 (LC 8). 6) "This truss is designed in accordance with the 2018 international Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANS/TP1 1. 7) PC HOR 1: 2-2-073 (9, 2-1-14) (12-130, 2-14) (14-18-2020) 80T C HOR 0: 21-22-2-T7898, 0.201-5-47789, 15-17-36/1254, 15-18-9103/225, 8:17-36-13/14, 14-15-2303/124, 12-13-030, 2-22-809/T5, 12-14-7802, 3/148, 6:13-950/141, 11-15-266/120, 9:15-00523 NOTES NOTES NOTES NOTES NOTES NOTES NOTES NOTES Notal edsign. 						SPF	cantilever le	eft and righ	nt exposed ;	end ve	ertical I	left and					
 TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 op puring (5-11-2 max, 15-10. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD Reactions (size) 14-0-3-8, 18=0-3-8, 82-20-3-8 Max Horiz 22-101 (LC 8) Max Upit 14-188 (LC 9), 18=-268 (LC 9), 22-24-143 (LC 8) Care 2-433 (LC 2) Forces (lb) - Maximum Compression/Maximum ToP CHORD 1-2-0137, 2-3-1204/174, 3-4=-1141/289, 4-5-8684/179, 5-6-828/161, 6-8-0/206, 8-9-0/219, 9-10-9-38/248, 10-111-1061/224, 11-121309/344, 12-13-0/30, 8-17-346/138, 15-17-91/548, 4-22-9-38/456, 6-20-1-15/570, 9-172-976/205, 10-115-20157, 12-14-786/2025 BOT CHORD 21-22-178/998, 20-21-5/5770, 9-172-976/205, 10-115-2053 NOTES 1) Unbalanced roof live loads have been considered for this design.)F 3)											
 chord live load normaling and 2-0-0 oc purines (5-11-2 max): 5-10. BOT CHORD Ridd certificats, and 2-0-0 oc purines (5-11-2 max): 5-10. BOT CHORD Ridd certificats, and 2-0-0 oc purines (5-11-2 max): 5-10. BOT CHORD Size) 14-0-3-8, 18=0-3-8, 22=0-3-8. Max Upiti 14-186 (LC 9), 18=-268 (LC 9), 22=-133 (LC 8). Max Upiti 14-186 (LC 9), 18=-268 (LC 9), 22=-378 (LC 9), 22=-373 (LC 2). Max Grav 14-887 (LC 24), 18=-1934 (LC 2), 22=937 (LC 2). FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1-2e-0/37, 2-3=-1204/174, 3-4=-1141/29, 4-3-638, 21-3-300, 2-22=-809/175, 12-14=-782/228. BOT CHORD 21-22=-178/998, 20-21=-54/788, 14-15=-263/1124. BOT CHORD 21-22=-178/998, 20-21=-54/788, 14-15=-263/1124. BOT CHORD 21-22=-178/998, 20-21=-54/788, 14-15=-263/1124. MVEBS 3-21=-282/202, 4-21=-194/391, 4-20=348/146, 6-18=-960/141, 11-15=-266/190, 9.15=-0/6225, 10-15=-29/156, 5-20=-438/146, 6-18=-960/141, 11-15=-266/190, 9.15=-0/6225. NOTES 1) Unbalanced roof live loads have been considered for this design. 	BRACING					E,											
BOT CHORD Rigid celling directly applied or 10-0-0 c bracing. WEBS I Row at midpt 9-17, 6-18 REACTIONS (size) 14-0-38, 18=0-38, 22=0-38 Max Horiz 22=011 (LC 8) Max Grav 14=887 (LC 24), 18=1934 (LC 2), 22=134 (LC 8) Max Grav 14=887 (LC 24), 18=1934 (LC 2), 22=937 (LC 2) FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 12=0037, 2-3=-1204/174, 3-4=-1141/299, 4-5=-884/179, 5-6=-822/151, 6-8=0/202, 8-9-0219, 9-10=-938/246, 10-11=-1061/224, 11-12=-130/244, 12-13-10/30, 2-22=-809/175, 12-14=-782/228 BOT CHORD 21-22=-178/988, 6-20=-15/70, 9-17=-976/205, 10-15=-29/156, 5-20=-438/146, 6-18=-950/141, 11-15=-266/190, 9-15=0/523 NOTES 1) Unbalanced roof live loads have been considered for this design.							chord live lo	oad nonco	ncurrent with	n any c	other liv	ve loads.					
 WEBS 1 Row at midp 9-17, 6-18 REACTIONS (size) 14-03-83, 18-03-8, 22=03-8 Max Hoiz 22=101 (LC 8) Max Diplit 14=-186 (LC 9), 18=-268 (LC 9), 22=-143 (LC 8) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2e-0/37, 2-3=-1204/174, 3-4=-1141/289, 4-5=-884/179, 5-6=-828/151, 6-8=-0/206, 8-9=-0/219, 9-10=-938/246, 10-11=-1061/224, 11-12=-130/344, 12-13-0/30, 2-22=-809/175, 12-14=-782/228 BOT CHORD 2-12-22-178/989, 20-21-5-47R89, 18-20=-53/452, 17-18=-108/3225, 8-17=-346/138, 15-17=-91/548, 4-20=-93/456, 6-20-1-291/50, 9-15=-0/523 NOTES 1) Unbalanced roof live loads have been considered for this design. 	BOT CHORD			pplied or	10-0-0 oc	0)	on the botto	om chord i	n all areas w	here a	recta	ngle					
 REACTIONS (size) 14=0-3-8, 18=0-3-8, 22=0-3-8 Max Horiz 22=101 (LC 8) Max Upiit 14=186 (LC 9), 18=-268 (LC 9), 22=-143 (LC 8) Max Grav 14=887 (LC 24), 18=1934 (LC 2), 22=937 (LC 2) FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/37, 2-3=-1204/174, 3-4=-1141/289, 4-5=-888/179, 5-6=-828/151, 6-8=0/2026, 8-9-0/219, 9-10=-938/246, 10-11=-1061/224, 11-12=-1/309/344, 12-13=0/30, 4-5=-888/179, 5-6=-828/151, 6-8=0/2026, 8-9-0/219, 9-10=-938/246, 10-11=-1061/224, 11-12=-1/309/344, 12-13=0/30, 4-5=-838/179, 5-6=-828/151, 6-8=0/2026, 8-9-0/219, 9-10=-938/254, 10-11=-1061/224, 11-12=-1/309/344, 12-13=0/30, 4-5=-283/1124 BOT CHORD 21-22=-178/998, 20-21=-54/789, 18-20=-53/452, 17-18=-1083/225, 8-17=-346/138, 15-17=-91/5/48, 14-15=-263/1124 WEBS 3-21=-28/2020, 4-21=-191/548, 14-15=-263/1124 WEBS 3-21=-28/200, 141, 11-15=-266/190, 9-15=0/523 NOTES 1) Unbalanced roof live loads have been considered for this design. 		•	dot 9	-17. 6-18													
 Nikk Holiz 22=101 (UC 8) Max Upiiti 14=-186 (LC 9), 18=-268 (LC 9), 22=-413 (LC 8) Max Grav 14=887 (LC 24), 18=1934 (LC 2), 22=937 (LC 2) FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1-2=/037, 2-3=-1204/174, 3-4=-1141/289, 4-5=-884/179, 5-6=-828/151, 6-8=0/206, 8-9=/0/219, 9-10=-938/246, 10-11=-1061/224, 11-12=-1309/344, 12-13=0/30, 2-22=-809/175, 12-14=-782/228 BOT CHORD 21-22=-178/988, 20-21=-54/789, 18-20=-53/452, 17-18=-1083/225, 8-17=-346/138, 15-17=-91/548, 14-15=-263/1124 WEBS 3-21=-282/202, 4-21=-194/391, 4-20=-93/459, 6-20=-15/570, 9-17=-976/205, 10-15=-29/156, 5-20=-438/146, 6-18=-950/141, 11-15=-266/190, 9-15=0/523 NOTES 1) Unbalanced roof live loads have been considered for this design. 	REACTIONS (size) 14	=0-3-8, 1	8=0-3-8, 2	22=0-3-8	7)											
 22, 186 lb uplift at joint 14 and 268 lb uplift at joint 18. 22, 186 lb uplift at joint 14. 22, 186 lb uplift at joint 14. 22, 186 lb uplift at joint 18. 22, 186 lb uplift at joint 14. 23, 184 and referenced standard ANS/ITP1. 10, Graphical puttin representation does not depict the size or the orientation of the purtin along the top and/or bottom chord. 22, 228-809/175, 12-14-789, 2021-5479, 13. 24, 184 and 268 lb uplift at joint 14. 25, 201, 124 26, 182-950/141, 11-15-266/190, 9-15=0/523 27, 282 and 28 and 28			· ·	,	268 (LC 9), ⁸⁾) Provide me	chanical c	onnection (b	y othe	rs) of t	truss to	+				
 22=937 (LC 2) FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/37, 2-3=-1204/174, 3-4=-1141/289, 4-5=-884/179, 5-6=-828/151, 6-8=-0/206, 8-9=-0/19, 9-10] 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANS/TPI 1. 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. LOAD CASE(S) Standard WEBS 3-21=-282/202, 4-21=-194/391, 4-20=-93/459, 6-20=-15/570, 9-17=-976/205, 10-15=-29/156, 5-20=-438/146, 6-18=-950/141, 11-15=-266/190, 9-15=0/523 NOTES 1) Unbalanced roof live loads have been considered for this design. 	Ν		· ·	,	1934 (LC :	2).							·				
Tension TOP CHORD 1-2=0/37, 2-3=-1204/174, 3-4=-1141/289, 4-5=-884/179, 5-6=-828/151, 6-8=0/206, 8-9=0/219, 9-10=-938/246, 10-11=-1061/224, 11-12=-1309/344, 12-13=0/30, 2-22=-809/175, 12-14=-782/228 BOT CHORD 21-22=-178/998, 20-21=-54/789, 18-20=-53/452, 17-18=-1083/225, 8-17=-346/138, 15-17=-91/548, 14-15=-263/1124 WEBS 3-21=-282/202, 4-21=-194/391, 4-20=-93/459, 6-20=-15/570, 9-17=-976/205, 10-15=-29/156, 5-20=-438/146, 6-18=-950/141, 11-15=-266/190, 9-15=0/523 NOTES 1) Unbalanced roof live loads have been considered for this design.		22	2=937 (LC	2)		9)											
TOP CHORD 1-2=0/37, 2-3=-1204/174, 3-4=-1141/289, 4-5=-884/179, 5-6=-828/151, 6-8=0/206, 8-9=0/219, 9-10=-938/246, 10-11=-1061/224, 11-12=-1309/344, 12-13=0/30, 2-22=-809/175, 12-14=-782/228 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. BOT CHORD 21-22=-178/998, 20-21=-54/789, 18-20=-53/452, 17-18=-1083/225, 8-17=-346/138, 15-17=-91/548, 14-15=-263/1124 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. WEBS 3-21=-282/202, 4-21=-194/391, 4-20=-93/459, 6-20=-15/570, 9-17=-976/205, 10-15=-29/156, 5-20=-438/146, 6-18=-950/141, 11-15=-266/190, 9-15=0/523 Standard NOTES 1) Unbalanced roof live loads have been considered for this design. PE-2001018807		Tension															
8-9=0/219, 9-10=-938/246, 10-11=-1061/224, 11-12=-1309/344, 12-13=0/30, 2-22=-809/175, 12-14=-782/228 bottom chord. BOT CHORD 21-22=-178/998, 20-21=-54/789, 18-20=-53/452, 17-18=-1083/225, 8-17=-346/138, 15-17=-91/548, 14-15=-263/1124 LOAD CASE(S) Standard WEBS 3-21=-282/202, 4-21=-194/391, 4-20=-93/459, 6-20=-15/570, 9-17=-976/205, 10-15=-291/156, 5-20=-438/146, 6-18=-950/141, 11-15=-266/190, 9-15=0/523 Scott M. NOTES 1) Unbalanced roof live loads have been considered for this design. PE-2001018807						[,] 10											
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NOTES 1) Unbalanced roof live loads have been considered for this design.		10-15=-29/1	56, 5-20=	-438/146,										-		NUN	1018907 AL
1) Unbalanced roof live loads have been considered for this design.		o-18=-950/14	+1, 11-15	=-266/190	J, 9-15=0/	523									N.	PE-200	101880/
All and a second s	1) Unbalanced	l roof live load	ds have b	een consi	dered for											SSION	IL ENGI
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August 4,2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling or individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor1 bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.