

6/5/2024

Stewart Builders

Lot 81 Summit View Farms 2315 SW Serena Place Lee's Summit, MO

Our firm has been asked to make structural clarifications to the plans of the house to be built at the address listed above. During the permit review process the AHJ has questioned items. Below is a list of our recommendations along with the corresponding city item.

The builder has asked us to change the plan to I joist floors, this change has added code requirements for fire protection of the I joist floor system. See clouded plan change for I joist floors and fire protection.

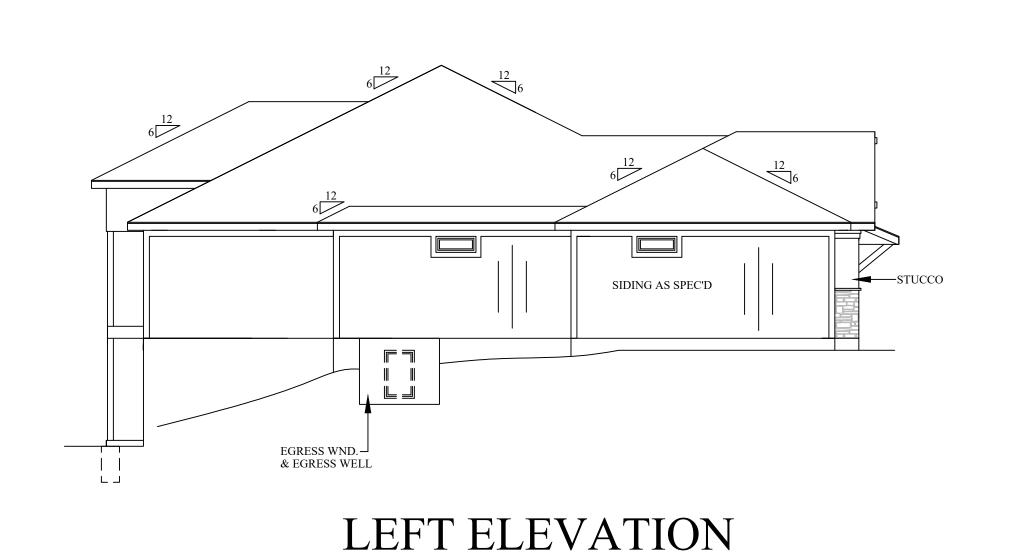
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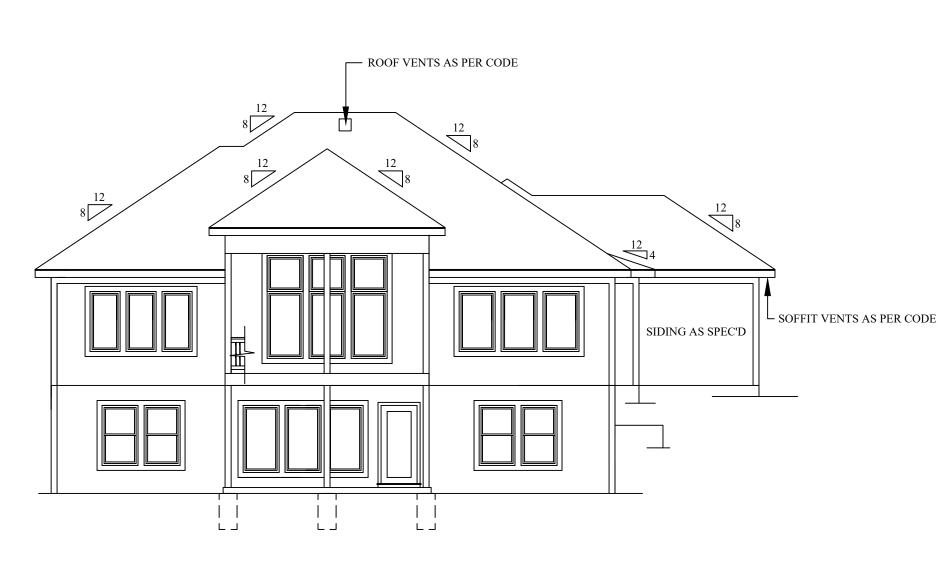
We appreciate the opportunity to be of service to you on this project. If you have any questions regarding this report, please contact us.

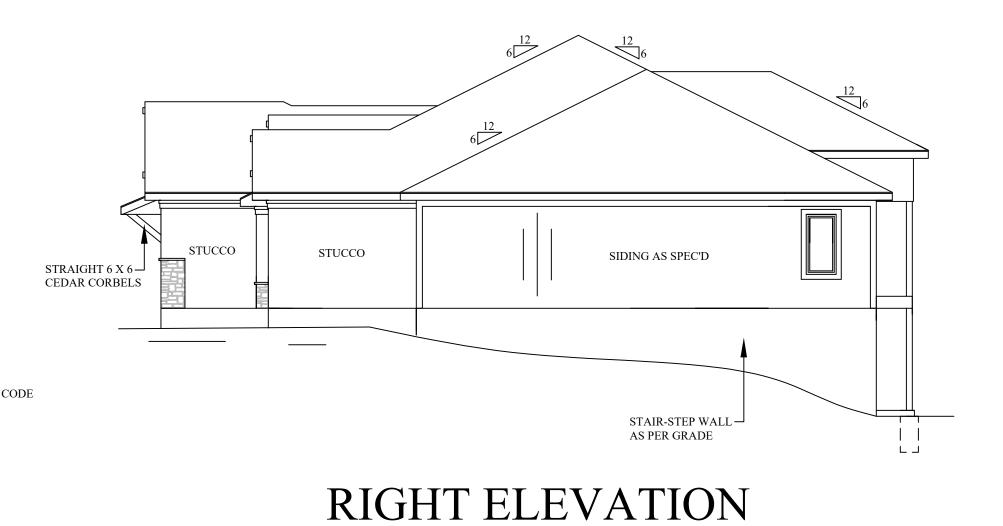




se vide@nsemmine@ascum







REAR ELEVATION

COMP. ROOF AS SPEC'D SHAKE 6 X 12 CEDAR BEAM METAL ROOF OR AS SPEC'D CULTURED STONE AS SPEC'D

RELEASE FOR CONSTRUCTION **AS NOTED ON PLANS REVIEW** DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 06/25/2024 8:29:48

FRONT ELEVATION 1/4" = 1'-0"

NOTE: THE CONTRACTOR IS RESPONSIBLE TO INSURE THAT THIS STRUCTURE IS BUILT IN STRICT COMPLIANCE WITH ALL GOVERNING CODES (COUNTY, STATE, & FED.).

ALL CONSTRUCTION TO MEET 2018 IRC AND LOCAL BUILDING PRACTICES.

ANY DEVIATION FROM PLANS MUST BE APPROVED BY OWNER PRIOR TO EXECUTION.

STEWART BUILDERS OR AFFILIATES OR ASSIGNS SHALL NOT BE LIABLE FOR STRUCTURAL DESIGN OR FUNCTION OF THESE HOUSE PLANS.

PLAN: LOT 81 - SUMMIT VIEW FARMS DATE: 7-12-2022

STEWART BUILDERS, LLC

"NOT A REGISTERED ARCHITECT"

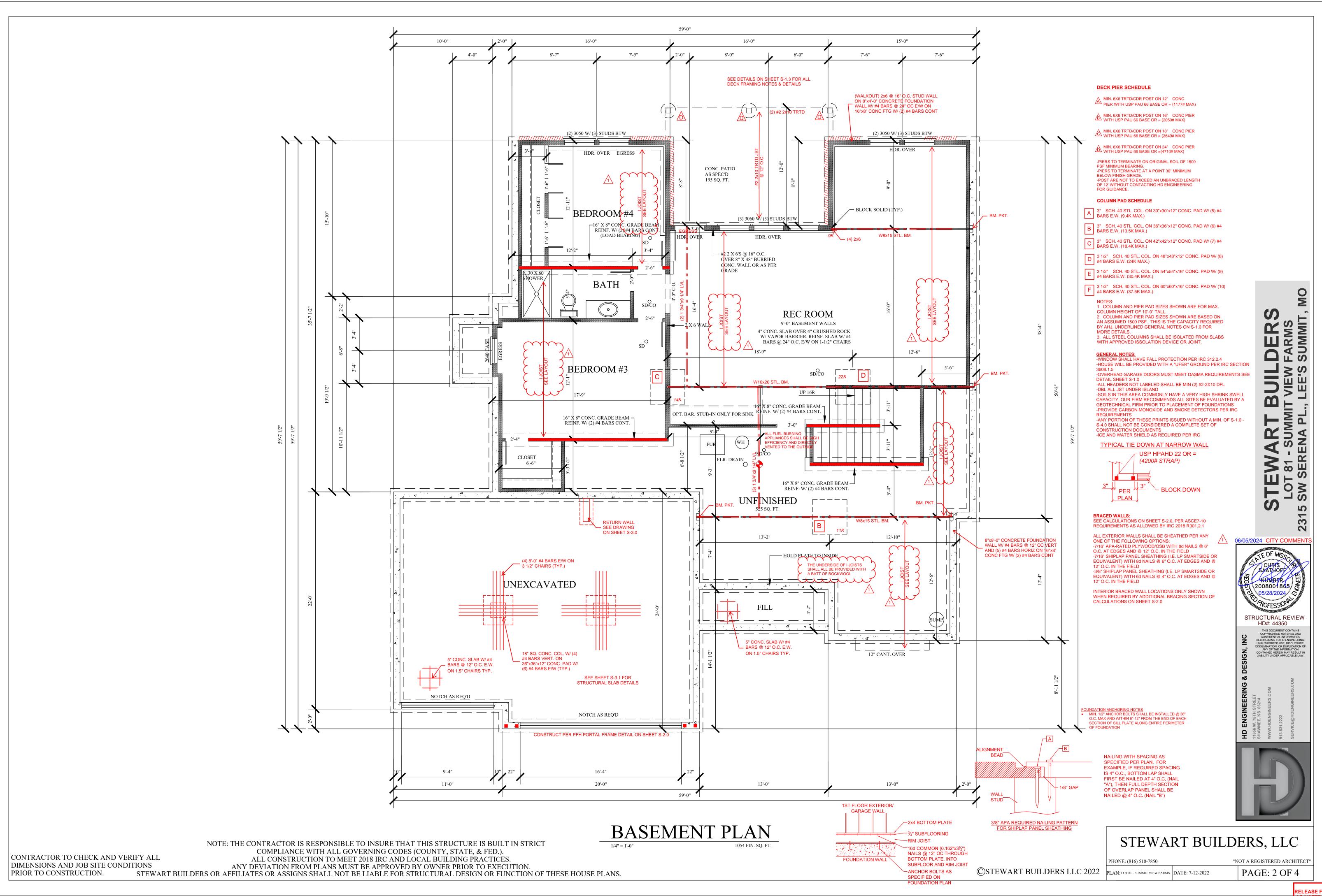
CONTRACTOR TO CHECK AND VERIFY ALL DIMENSIONS AND JOB SITE CONDITIONS PRIOR TO CONSTRUCTION.

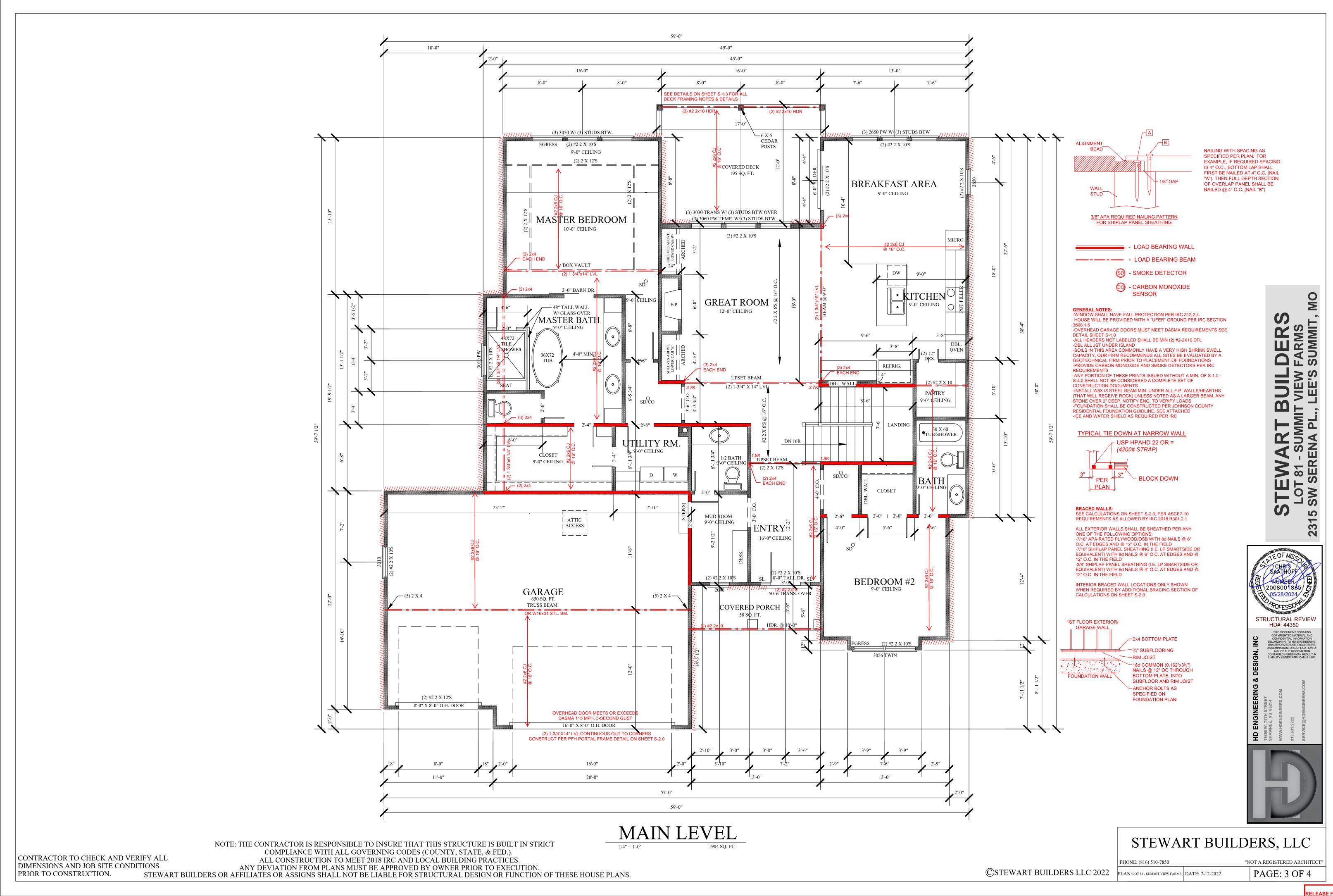
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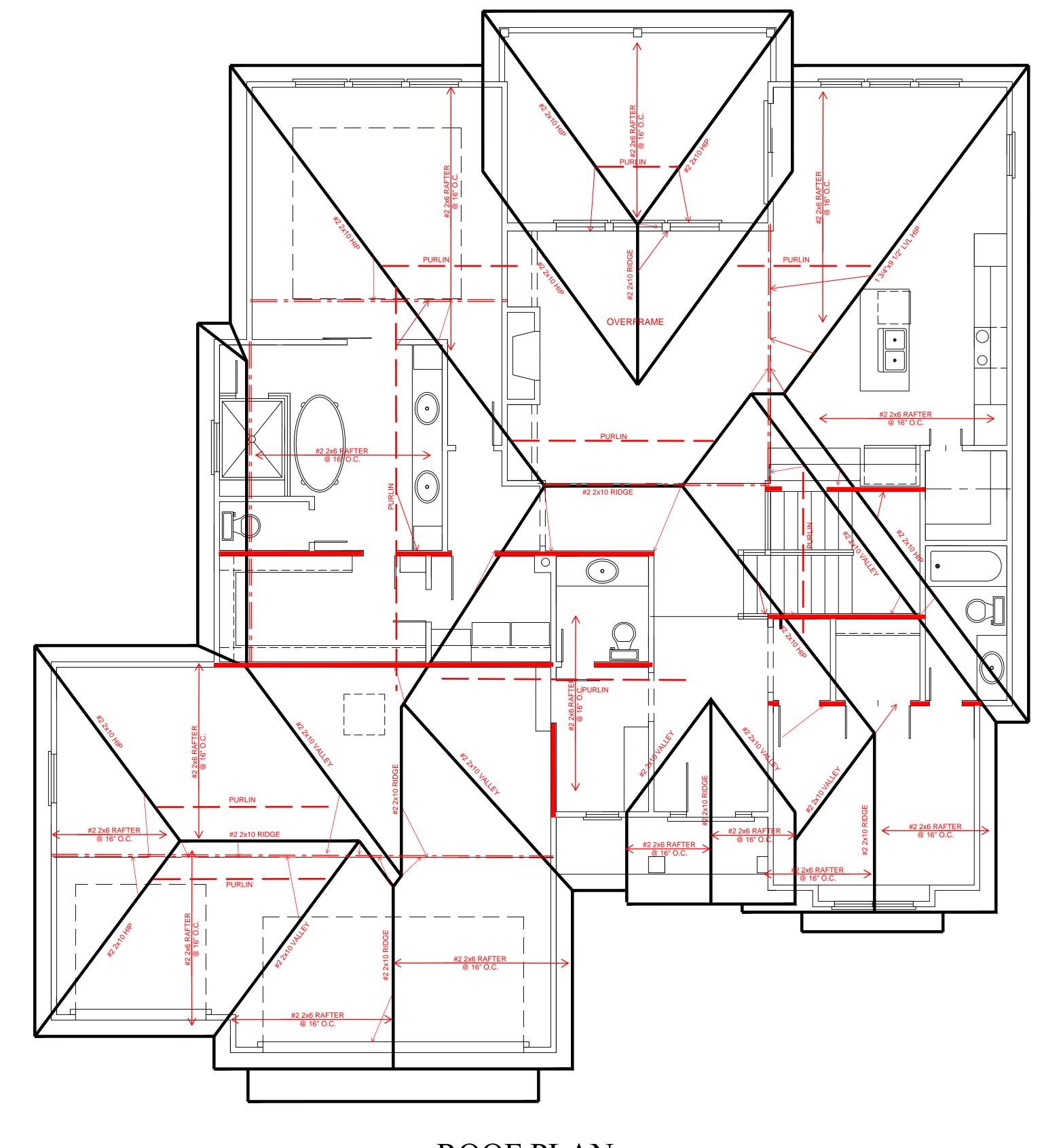
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STRUCTURAL REVIEW
HD#: 44350

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ROOF DESIGNED FOR LIGHT ROOF COVERING 30PSF TOTAL LOAD [10PSF DL, 20PSF LL (SL)]

RAFTERS (DOUG-FIR, OR EQUAL): SEE SPAN CHARTS BELOW

CODE MINIMUM		
RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN
#2-2x6	@24" O.C.	11'-11"
#2-2x6	@16" O.C.	14'-1"
#2 - 2x8	@24" O.C.	15'-1"
#2-2x8	@16" O.C.	18'-5"
#2 - 2x10	@24" O.C.	18'-5"
#2-2x10	@16" O.C.	22'-6"

NOTE: CODE MINIMUM L/240 DEFLECTION

GREATER THAN C	ODE	
RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN
#2-2x6	@24" O.C.	8'-6"
#2-2x6	@16" O.C.	9'-9"
#2 - 2x8	@24" O.C.	11'-3"
#2 - 2x8	@16" O.C.	12'-9"
#2 - 2x10	@24" O.C.	14'-3"
#2-2x10	@16" O.C.	16'-3"

DEFLECTION = L/360 LIVE LOAD, L/240 TOTAL LOAD VAULTS TO BE 2x10 DEPTH

ALL RIDGES, HIPS, AND VALLEYS NOT MARKED SHALL BE (1) NOMINAL SIZE LARGER THAN THE INTERSECTING RAFTERS

PURLINS ARE 2x6 MIN. PURLIN STRUTS ARE AT 4'-0" O.C. PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 DEGREE ANGLE WITH THE HORIZONTAL ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED LENGTH OF 8'-0" PURLINS STRUTS SHALL BE CONSTRUCTED IN A "T"

CONFIGURATION AND PER THE FOLLOWING CHART PURLIN STRUT MAX PURLIN STRUT LENGTH

(2) 2x4	8'-0"
(1) 2x4 & (1) 2x6	12'-0"
(1) 2x6 & (1) 2x8	20'-0"
(2) 2x6 & (1) 2x8	30'-0"
CONSULT ARCH./ENGR.	>30'-0"

SEE DETAILS 1, 5, 6, 7, 11, 12, 13, & 14 ON S-1.2 FOR ROOF FRAMING AND INSULATION OPTIONS

—— —— - PURLIN

- LOAD BEARING WALL ==== - LOAD BEARING BEAM/

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ROOF PLAN

NOTE: THE CONTRACTOR IS RESPONSIBLE TO INSURE THAT THIS STRUCTURE IS BUILT IN STRICT COMPLIANCE WITH ALL GOVERNING CODES (COUNTY, STATE, & FED.).

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STEWART BUILDERS LLC 2022

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PLAN: LOT 81 - SUMMIT VIEW FARMS DATE: 7-12-2022

"NOT A REGISTERED ARCHITECT" PAGE: 4 OF 4

CONTRACTOR TO CHECK AND VERIFY ALL DIMENSIONS AND JOB SITE CONDITIONS PRIOR TO CONSTRUCTION.

STEWART BUILDERS, LLC

ALLOWABLE LOADS FOR PNEUMATIC OR MECHANICALLY DRIVEN NAILS AND STAPLES

	NAIL GUN		PENETRATION	Al	LLOWABLE LO	DADS (POUND	S)
FASTENER DESCRIPTION	NAILS/ WIRE	WIRE GAGE	REQUIRED INTO MAIN MEMBER FOR LATERAL	LATERAL STRENGTH		WITHDRAWA	L STRENGTH
	DIAMETER	5115	STRENGTH (INCHES)	SP	DF/L	SP	DF/L
16 GA. STAPLE	.063	16	1	51		36	32
15 GA. STAPLE	.072	15	1	64		42	37
14 GA. STAPLE	.080	14	1	75		46	41
6d COOLER NAIL	000	40	_	40		07	00
6d SINKER NAIL	.092	13	1	46		27	23
6d BOX NAIL							
6d CASING NAIL	.099	12-1/2	1-1/8	61	55	31	24
7d COOLER NAIL							
6d COMMON NAIL							
8d COOLER NAIL							
8d SINKER NAIL	.113	11-1/2	1-1/4	79	72	35	28
8d BOX NAIL							
8d CASING NAIL							
6d RING SHANK NAIL							
6d SCREW SHANK NAIL							
8d RING SHANK NAIL	.120	11	1-3/8	89	81	41	32
8d SCREW SHANK NAIL							
10d COOLER NAIL							
10d SINKER NAIL	.128	10-1/2	1-1/2	89	81	36	31
12d SHORT	-						
10d BOX NAILS							
12d BOX NAILS	128	10-1/2	1-1/2	101	93	40	31
10d CASING NAILS		10 1/2	,				0.
8d COMMON NAILS							
16d SHORT	.131	10-1/4	1-1/2	106	97	41	32
12d SINKERS							
16d BOX NAILS	.135	10	1-1/2	113	103	42	33
10d RING SHANK NAILS							
0d SCREW SHANK NAILS							
12d RING SHANK NAILS	.135	10	1-5/8	113	103	46	36
2d SCREW SHANK NAILS							
10d COMMON NAILS							
12d COMMON NAILS							
16d SINKER NAILS	.148	9	1-5/8	128	118	46	36
20d BOX NAILS		Ü	1 5/5	.20			
30d BOX NAILS							
16d RING SHANK NAILS							
6d SCREW SHANK NAILS	.148	9	1-3/4	128	118	50	40
16d COMMON NAILS							
40d BOX NAILS	.162	8	1-3/4	154	141	50	40
20d RING SHANK NAILS							
20d SCREW SHANK NAILS	.177	7	2-1/8	178	163	59	47
20d SINKER NAILS	.177	7	2-1/8	178	163	54	43
20d COMMON NAILS	.1//		Z-1/0	170	103	J 4	43
ZUU GOIVIIVION NAILS	.148	9	2-1/8	170	166	59	47

MINIMUM SHEATHING REQUIREMENTS

30d SINKER NAILS

BUILDING COMPONENT	MATERIAL
ROOF SHEATHING	7/16" PLYWOOD
ROOF SHEATHING	1 x 4 #3 FURRING
FLOOR SHEATHING	3/4" T&G YELLOW PINE PLYWOOD
WALL COVERING	1/2" GYPSUM SHEATHING
CEILING COVERING	1/2" GYPSUM SHEATHING
EXTERIOR WALL	7/16" APA RATED SHEATHING
SHEATHING	RATED PANEL SIDING, RATED 16" O.C. 7/16" THICK

ALL SHEATHING MATERIALS TO BE APPLIED PERPENDICULAR TO JOISTS AND ENDS STAGGERED REFER TO TABLE R602.3(1) ON S-1.1 FOR FASTENING SCHEDULE

TYPE	MAX. UNSUPPORTED SPAN				
ITE	2x8	2x10	2x12	1 3/4"x9 1/2" LVL	1 3/4"x11 7/8" LVL
HIP RAFTER	11'-3"	13'-3"	15'-2"	15'-8"	18'-2"
VALLEY RAFTER	8'-11"	10'-6"	12'-0"	13'-2"	15'-3"

HIP/ VALLEY ALLOWABLE SPAN TABLE

FRAME FASTENING SCHEDULE

BUILDING COMPONENT	FASTEN TO	FASTEN WITH
	RIDGE / VALLEY / HIP	TOENAIL W/ (4) 16D, FACENAIL W/ (3) 16D
DAETEDS	PLATE	TOENAIL W/ (3) 10D
	LEDGER STRIPS SUPPORTING JOISTS OR RAFTERS	FACENAIL W/ (3) 16D
	COLLAR TIE TO RAFTERS	FACENAIL W/ (3) 10D
	TOP PLATE	TOENAIL W/ (3) 8D @ EACH END
	WHERE CLG JST RUN PARALLEL TO RAFTERS FAC	ENAIL TO RAFTERS W/ (3) 10D MINIMUM
CEILING JOISTS	LAPS OVER PARTITIONS	FACENAIL W/ (3) 10D
RAFTERS BEAMS FLOOR JOISTS	BLOCKING BETWEEN JOISTS/RAFTERS TO TOP PLATE	TOENAIL W/ (3) 8D
	BUILT-UP BEAMS, 2" LUMBER LAYERS, FACENAIL OPPOSITE SIDES, (2) @ EACH END PLUS	10D @ 32" O.C. STAGGERED, TOP & BOTTOM, OPPOSITE SIDES
BEAMS	BUILT-UP BEAMS OF ENGINEERED LUMBER, FACE NAIL OPPOSITE SIDES	(2) ROWS @ 12" O.C.
	BUILT-UP HEADER, TWO PIECES W/ A 1/2" SPACER	16D @ 16" O.C. ALONG EDGES
	BUILT-UP HEADER, TWO PIECES W/ NO 1/2" SPACER	3" x 0.131" NAILS @ 12" O.C. ALONG EDGES
	BEARING	TOENAIL W/ (2) 18D @ EACH END
	RIM JOIST TO SILL OR TOP PLATE	TOENAIL W/ 8D COMMON OR 10D BOX @ 6" O.C.
	JOIST TO SILL OR GIRDER	TOENAIL W/ (3) 8D
	JOIST TO RIM JOIST	FACENAIL W/ (3) 16D
	BRIDGING TO JOIST	TOENAIL W/ (2) 8D
FLOOR JOISTS	I-JOIST TO BEARING PLATE	TOENAIL W/ (2) 8D - ONE INTO EACH SIDE AT LEAST 1 1/2" FROM THE END
	RIM JOIST TO I-JOIST	FACENAIL W/ (2) 10D BOX - ONE INTO EACH FLANGE
	SOLE PLATE TO LSL RIM BOARD	16D BOX @ 12" O.C.
	SINGLE JOIST HANGERS*	10D FACENAILS AND TOENAILS
	DOUBLE JOIST HANGERS*	16D FACENAILS AND TOENAILS
	TOP AND SOLE PLATE TO STUD	END NAIL W/ (2) 16D
	STUD TO SOLE AND TOP PLATE	TOENAIL W/ (4) 8D
	DOUBLE TOP PLATES	FACENAIL W/ 16D @ 16" O.C.
	DOUBLE TOP PLATE LAP SPLICE	FACENAIL W/ (8) 16D
	TOP PLATE LAPS AND INTERSECTIONS	FACENAIL W/ (2) 16D
	DOUBLE STUDS	FACENAIL W/ 16D @ 24" O.C.
	BUILT-UP CORNER STUDS	FACENAIL W/ 16D - 2 ROWS @ 24" O.C.
	STEEL "X" BRACING	FACENAIL W/ (2) 16D IN EACH TOP AND BOTTOM PLATE AND (1) 8D PER STUD
	SOLE PLATE TO JOIST OR BLOCKING	FACENAIL W/ 16D @ 16" O.C.
WALLS	SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL LINES, PERPENDICULAR TO FRAMING	FACENAIL W/ (3) 16D @ 16" O.C. ALONG BRACED WALL PANEL
	TOP PLATE TO JOIST OR BLOCKING AT BRACED WALL LINES, PERPENDICULAR TO FRAMING	TOENAIL W/ 8D @ 6" O.C. ALONG BRACED WALL PANEL
	SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL LINES, PARALLEL TO FRAMING, BLOCKING @ 16" O.C.	FACENAIL W/ (3) 16D @ 16" O.C. ALONG BRACED WALL PANEL AND AT EACH BLOC
	TOP PLATE TO JOIST OR BLOCKING AT BRACED WALL LINES, PARALLEL TO FRAMING, BLOCKING @ 16" O.C.	TOENAIL W/ 8D @ 6" O.C. ALONG BRACED WALL PANEL AND AT EACH BLOCK
	NON-STRUCT. SIDING OVER STRUCT. SHEATHING	(1) 6D BOX IN EACH STUD
	FIBER-CEMENT PLANK SIDING	(1) 6D GALVANIZED IN EACH STUD
	WINDOW INSTALLATION NAILING	1 ³ / ₄ " - 2" ROOFING NAILS @ 12" O.C. MAX.

NO JOIST HANGER NAILS ALLOWED FOR TOENAILS.

NO GUN NAILS OR SCREWS ALLOWED IN CONNECTORS. TOENAILS SHALL ALWAYS BE A FULL 3" OR 3.5" NAIL.

COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. 1/2" x 2" BOLTS SHOULD THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

DUCT SEALING METHOD, PER 2018 IRC W1103.3.2

N1103.2.2 (R403.2.2) SEALING (MANDATORY) DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF THIS CODE.

1. AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT 2. WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL

BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT. 3. CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURE LESS THAN 2 INCHES OF WATER COLUMN (500 Pa) PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.

DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING: 1. POST CONSTRUCTION TEST: TOTAL LEAKAGE SHALL NOT BE LESS THAN OR EQUAL TO 4 CFM (113.3 L/MIN) PER 100FT² (9.29m²) OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. (25 Pa) ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS

SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. 2. ROUGH-IN TEST: TOTAL AIR LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM (113.3 L/MIN) PER 100FT2 (9.29m²) OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. (25 Pa) ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST. TOTAL AIR LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM (85 L/MIN) PER 100FT² (9.29m²) OF CONDITIONED FLOOR

EXCEPTION: THE TOTAL LEAKAGE IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

PLANS SHALL COMPLY WITH THE 2018 INTERNATIONAL RESIDENTIAL CODE, ICC AS ADOPTED BY AHJ, AND ALL AMENDMENTS AS ADOPTED BY THE AHJ. IF ANY CHANGES MAKE ANY APPROPRIATE MODIFICATIONS TO THE PLANS

WHERE DISCREPANCIES EXIST BETWEEN THE STANDARD COMMENTS, NOTES FOR THE DESIGN PROFESSIONAL OR THE CODE, THE MOST RESTRICTIVE SHALL APPLY. THE CONTRACTUAL OBLIGATION OF THESE PLANS IS TO PROVIDE THE OWNER/BUILDER AND THE AHJ WITH A SET OF PLANS THAT MEET AHJ AND CODE REQUIREMENTS FOR A SINGLE SITE CONSTRUCTION PROJECT. UNLESS REQUESTED BY OUR CLIENT, CODE/AHJ MINIMUM DESIGNS WILL BE UTILIZED. ALSO, UNLESS REQUESTED BY THE OWNER, OUR FIRM CAN NOT AND WILL NOT BE AUTHORIZED TO VISIT THE SITE TO EVALUATE THE SITE OR ANY CONSTRUCTION FOR THIS PROJECT. IMPLEMENTATION OF ALTERNATES TO THE DESIGNS INCLUDING BUT NOT LIMITED TO PIER DESIGNS, FOUNDATION ALTERATIONS, OR ANY STRUCTURAL CHANGES NOT PROVIDED BY HD ENGINEERING OR A PROFESSIONAL REFERRED BY HD ENGINEERING SHALL RELEASE HD ENGINEERING FROM ALL LIABILITY ASSOCIATED WITH THIS DESIGN.

OUR FIRM HIGHLY RECOMMENDS THAT ANY SITE WITH GREATER THAN A 15% GRADE, ANY SITE WHERE A PREVIOUS STRUCTURE WAS LOCATED, OR ANY SITE WITH POTENTIAL FILL MATERIAL OR A POTENTIAL SOIL BEARING CAPACITY BELOW 1500 PSF SHOULD BE EVALUATED BY OUR FIRM OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACING FOOTINGS. THE ATTACHED PLANS HAVE BEEN DESIGNED WITH THE UNDERSTANDING THAT <u>OUR FIRM HAS NOT AND CAN NOT VISIT OR INSPECT THE SITE</u> WITHOUT WRITTEN CONSENT/REQUEST OF THE OWNER/BUILDER. DUE TO THIS FACT, OUR FIRM CAN ONLY DESIGN THE ATTACHED PLANS TO CERTAIN CODE REQUIREMENTS WHICH ARE DETAILED THROUGHOUT THE PLAN AND ATTACHED DETAIL SHEETS, IF THE OWNER DESIRES GREATER THAN CODE

DESIGNS THAT REQUEST MUST BE MADE CLEARLY AND IN WRITING PRIOR TO ENGINEERING OF THE PLAN. DUE TO THE WIDE VARIETY OF SOIL CONDITIONS, PLASTICITY INDEXES, AND SOIL BEARING CAPACITIES IN OUR AREA, OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY HD ENGINEERING OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF ANY "STANDARD" FOUNDATIONS.

THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION RESIDENTIAL FOUNDATION STANDARD IN LIEU OF ENGINEERING REPORT REQUIREMENTS

BASED ON ACTUAL SITE CONDITIONS. FOUNDATION WALLS SHALL BE DAMP-PROOFED PER IRC SECTION R406. PROVIDE A MINIMUM 4" PERFORATED DRAIN AROUND USABLE SPACE BELOW GRADE OR OTHER EQUIVALENT MATERIALS PER IRC SECTION 405.1. THE PIPE SHALL BE

COVERED WITH NOT LESS THAN 6" OF WASHED GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT TO THE EXTERIOR BELOW THE FLOOR LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT. FOUNDATION DESIGN SHALL BE BASED ON A MINIMUM SOIL BEARING CAPACITY OF 1500 PSF.

FOOTINGS SHALL BE A MINIMUM OF 16" WIDE AND 8" DEEP WITH (2) #4 BARS CONTINUOUS, LOCATED A MINIMUM OF 3" CLEAR FROM THE BOTTOM. FOOTINGS SHALL BE A

COLUMN PADS SHALL BE A MINIMUM OF 24"x24"x8" WITH (3) #4 BARS EACH WAY.

FOUNDATION WALLS SHALL BE A MINIMUM OF 8" THICK WITH MINIMUM #4 BARS @ 24" O.C. HORIZONTAL AND VERTICAL WITH THE TOP BAR WITHIN 8" OF THE TOP OF THE WALL UNLESS NOTED OTHERWISE ON PLAN. REINFORCEMENT SHALL LAP A MINIMUM OF 24".

INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB.

INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING, SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE BY A SEPARATION CONCRETE FLOOR SLABS ON GRADE SHALL BE A MINIMUM OF 4" THICK OVER A MINIMUM 4" BASE OF SAND, GRAVEL, OR CRUSHED STONE. BASEMENT SLABS SHALL HAVE A MINIMUM 6 MIL POLYETHYLENE OR APPROVED VAPOR RETARDER WITH JOINTS LAPPED NOT LESS THAN 6" AND SHALL BE PLACED BETWEEN THE FLOOR SLAB

FLOOR SLABS SUPPORTED BY FILL CONSISTING OF MORE THAN 24" OF GRANULAR FILL OR 8" OF EARTH SHALL BE REINFORCED PER A SEPARATE ENGINEERING DESIGN. BASEMENT FOUNDATION SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WITH MINIMUM 1/2" DIAMETER ANCHOR BOLTS EMBEDDED AT LEAST 7" INTO THE CONCRETE AND SPACED NOT MORE THAN 3' ON CENTER AND WITHIN 12" OF EACH END OF THE PLATE SECTION PER IRC SECTION R403.1.6.

FOUNDATION WINDOW WELLS FOR SECONDARY MEANS OF EGRESS SHALL PROVIDE A MINIMUM 3'x3' HORIZONTAL AREA. THE BASE OF ALL FOOTING EXCAVATIONS SHOULD BE FREE OF ALL WATER AND LOOSE MATERIAL PRIOR TO PLACING CONCRETE. CONCRETE SHOULD BE PLACED AS SOON AS POSSIBLE AFTER EXCAVATING SO THAT EXCESSIVE DRYING OR DISTURBANCE OF BEARING MATERIALS DOES NOT OCCUR. SHOULD THE MATERIALS AT

BEARING LEVEL BECOME EXCESSIVELY DRY OR SATURATED, WE RECOMMEND THAT THE AFFECTED MATERIAL BE REMOVED PRIOR TO PLACING CONCRETE. IT IS RECOMMENDED THAT ALL FOOTING EXCAVATIONS BE EVALUATED AND TESTED BY A GEOTECHNICAL ENGINEER IMMEDIATELY PRIOR TO PLACEMENT OF FOUNDATION CONCRETE. UNSUITABLE AREAS IDENTIFIED AT THIS TIME SHOULD BE CORRECTED. CORRECTIVE PROCEDURES WOULD BE DEPENDENT UPON CONDITIONS ENCOUNTERED AND MAY INCLUDE THE DEEPENING OF FOUNDATION ELEMENTS, OR THE UNDERCUTTING OF UNSUITABLE MATERIALS AND REPLACEMENT

WITH ENGINEERED FILL.

STAIRWAYS SHALL PROVIDE A MAXIMUM 7 3/4" RISE AND A MINIMUM 10" RUN. PROVIDE MINIMUM 36" GUARDRAILS ON THE OPEN SIDES OF RAISED FLOORS, PORCHES, AND BALCONIES. PROVIDE MINIMUM 34" GUARDRAILS ON THE OPEN SIDES OF STAIRWAYS LOCATED MORE THAN 30" ABOVE THE FLOOR OR GRADE BELOW. GUARDRAIL ENCLOSURES SHALL HAVE INTERMEDIATE RAILS OR ORNAMENTAL PATTERNS THAT DO NOT ALLOW PASSAGE OF A 4" DIAMETER SPHERE

EACH STAIRWAY OF 3 OR MORE RISERS SHALL PROVIDE A CONTINUOUS HANDRAIL ON AT LEAST ONE SIDE BETWEEN 34" AND 38" ABOVE THE NOSING OF THE TREADS. HANDRAILS SHALL HAVE A CIRCULAR CROSS-SECTION OF 1 1/4" MINIMUM TO 2" MAXIMUM OR ANOTHER APPROVED GRASPABLE SHAPE PER IRC SECTION R311.7.8.5. PROVIDE A MINIMUM 6'-8" OF HEADROOM CLEARANCE IN STAIRWAYS.

ENCLOSED ACCESSIBLE SPACE UNDER STAIRWAYS SHALL HAVE WALLS AND THE UNDERSIDE OF THE STAIR AND LANDING PROTECTED WITH 1/2" GYPSUM BOARD ON THE

WINDERS SHALL PROVIDE A MINIMUM TREAD OF 6" AT ANY POINT WITHIN CLEAR WIDTH OF STAIRS. WINDER TREAD PROPORTION IS TO COMPLY WITH IRC SECTION R311.7.5.2.1.

GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPERABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 24" ARCH OF THE DOOR IN A CLOSED POSITION AND WHOSE BOTTOM EDGE IS WITHIN 60" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 60" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS AND WHIRLPOOLS, GLAZING IN FIXED OR OPERABLE PANELS EXCEEDING 9 S.F. AND WHOSE BOTTOM EDGE IS LESS THAN 18" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 36".

IN DWELLING UNITS WHERE THE OPENING OF AN OPERABLE WINDOW IS LOCATED MORE THAN 72" ABOVE THE FINISHED GRADE OR SURFACE BELOW, THE LOWEST PART OF THE CLEAR OPENING OF THE WINDOW SHALL BE A MINIMUM OF 24" ABOVE THE FINISHED FLOOR OF THE ROOM IN WHICH THE WINDOW IS LOCATED. OPERABLE SECTIONS OF WINDOWS SHALL NOT PERMIT OPENINGS THAT ALLOW PASSAGE OF A 4" DIAMETER SPHERE WHERE SUCH OPENINGS ARE LOCATED WITHIN 24" OF THE FINISHED FLOOR.

ALL LUMBER SIZES ARE FOR DOUGLAS FIR-LARCH UNLESS NOTED OTHERWISE

ALL HEADERS ARE TO BE A MINIMUM OF (2) #2 2x10'S UNLESS NOTED OTHERWISE. BLOCK CANTILEVERS, DOOR JAMBS, AND OVER BEAMS.

ALL HEADERS/BEAMS ARE TO BEAR ON A MINIMUM OF (2) 2x4 POSTS UNLESS NOTED OTHERWISE

INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING, SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE WHERE JOISTS RUN PARALLEL TO FOUNDATION WALLS, SOLID BLOCKING FOR A MINIMUM OF (2) JOIST SPACES SHALL BE PROVIDED AT A MAXIMUM OF 4' ON CENTER TO TRANSFER LATERAL LOADS ON THE WALL TO THE FLOOR DIAPHRAGM. THE BLOCKING SHALL BE SECURELY NAILED TO THE JOISTS AND FLOORING. NAIL JOISTS AND

BLOCKING TO SILL PLATE WITH (4) 10D NAILS. IF DUCTS ARE INSTALLED IN THE FIRST JOIST SPACE(S), NAIL 2x4'S FLAT AT 4' ON CENTER WITHIN THE JOIST SPACE(S) AND THEN PROVIDE SOLID BLOCKING, INSTALLED UPRIGHT, IN THE NEXT TWO JOIST SPACES. SECURE THE 2x4'S TO THE SILL PLATE WITH (4) 10D NAILS.

ALL SILLS AND SLEEPERS SUPPORTED ON CONCRETE OR MASONRY AND FURRING ATTACHED TO CONCRETE OR MASONRY SHALL BE OF DECAY RESISTANT MATERIALS. JOISTS UNDER BEARING PARTITIONS SHALL BE SIZED TO CARRY THE DESIGN LOAD IN ACCORDANCE WITH IRC SECTION R502.4. JOISTS FRAMING FROM OPPOSITE SIDES OVER BEARING SUPPORTS SHALL LAP A MINIMUM OF 3" AND SHALL BE NAILED TOGETHER WITH MINIMUM 10D FACE NAILS.

JOISTS FRAMING INTO A WOOD GIRDER OR BEAM SHALL BE SUPPORTED BY APPROVED FRAMING ANCHORS OR ON MINIMUM 2"x2" LEDGER STRIPS. HEADER AND TRIMMERS SHALL BE OF SUFFICIENT CROSS SECTION TO SUPPORT THE FLOOR FRAMING. TRIMMER JOISTS SHALL BE DOUBLED WHEN THE HEADER IS SUPPORTED MORE THAN 3' FROM THE TRIMMER JOIST BEARING. WHEN THE HEADER SPAN EXCEEDS 4', THE HEADER AND TRIMMER SHALL BE DOUBLED. JOISTS AT SUPPORTS SHALL BE SUPPORTED LATERALLY AT THE ENDS BY FULL-DEPTH SOLID BLOCKING NOT LESS THAN 2" IN NOMINAL THICKNESS OR BY ATTACHMENT

TO A HEADER, BAND, OR RIM JOIST OR TO AN ADJOINING STUD OR OTHERWISE PROVIDED WITH LATERAL SUPPORT TO PREVENT ROTATION. ALL WALL COVERINGS ARE TO COMPLY WITH IRC SECTIONS 702 AND 703.

ALL RAFTER / COLLAR TIES ARE TO COMPLY WITH IRC SECTION 802. ALL RAFTERS ARE TO HAVE 2x4 COLLAR TIES @ 48" O.C. IN THE UPPER 1/3 OF DISTANCE BETWEEN THE CEILING AND ROOF

BLOCKING BETWEEN JOISTS UNDER A PERPENDICULAR LOAD-BEARING WALL IS NOT REQUIRED.

THE BOTTOM OF ALL FLOOR ASSEMBLIES SHALL BE PROVIDED WITH A 1/2" GYPSUM WALLBOARD MEMBRANE (IF REQUIRED BY LOCAL CODE). I-JOIST AND FLOOR TRUSS SYSTEMS SHALL BE FIRE PROTECTED PER IRC AS ADOPTED BY AHJ

STUDS SHALL BE CONTINUOUS FROM THE FLOOR TO THE ROOF / CEILING DIAPHRAGM PER IRC SECTION 602.3

CONCRETE SHALL BE AIR-ENTRAINED (5%-7%) WITH A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS, 3000 PSI FOR BASEMENT AND FOUNDATION WALLS, AND 3500 PSI FOR PORCHES, CARPORTS AND GARAGE FLOOR SLABS.

EMERGENCY EGRESS AND RESCUE NOTES:

PROVIDE ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 S.F. WITH A MINIMUM OPENABLE HEIGHT OF 24" AND WIDTH OF 21". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 44" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP.

PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA, AND ON EACH FLOOR INCLUDING BASEMENTS. ALARMS SHALL BE INTERCONNECTED IN SUCH A MANNER THAT THE ACTIVATION OF ONE ALARM WILL ACTIVATE ALL OF THE ALARMS IN THE DWELLING.

PROVIDE CARBON MONOXIDE ALARMS AS REQUIRED PER IRC. CARBON MONOXIDE ALARMS SHALL BE INSTALLED OUTSIDE OF EACH SEPARATE SLEEPING AREA. WHERE FUEL-BURNING APPLIANCES ARE LOCATED WITHIN A BEDROOM OR ITS ATTACHED BATHROOM, A CARBON MONOXIDE ALARM SHALL BE INSTALLED IN THE BEDROOM.

THE GARAGE FLOOR SHALL SLOPE TOWARDS THE GARAGE DOORWAYS OR SLOPE TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES DIRECTLY TO THE EXTERIOR ABOVE GRADE.

DOORS BETWEEN THE GARAGE AND DWELLING - MINIMUM 1 3/8" THICK SOLID WOOD, MINIMUM 1 3/8" THICK SOLID OR HONEY-COMB-CORE STEEL DOOR, OR 20-MINUTE

FIRE-RATED EQUIPPED WITH A SELF-CLOSING DEVICE PER IRC SECTION R302.5.1. GARAGE VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER

THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 5/8" GYPSUM BOARD APPLIED TO THE GARAGE SIDE. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE. THE FLOOR/CEILING ASSEMBLY SHALL BE PROTECTED WITH MINIMUM 5/8" TYPE X GYPSUM BOARD ON THE GARAGE CEILING. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE, COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH 5/8"

GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING ATTACHED WITH 1 3/4"x0.120" NAILS AT 7" ON CENTER STAGGERED WITH (7) 3 1/4"x0.120" NAILS THROUGH THE JAMB INTO THE HEADER, MINIMUM 2x8 HEADER FOR ATTACHMENT OF THE COUNTER BALANCE SYSTEM.

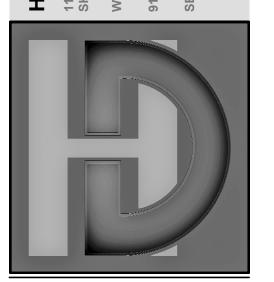
ANY ATTACHED GARAGE TO THE MAIN HOUSE SHALL BE PROVIDED WITH A SINGLE HEAT DETECTOR. THE HEAT DETECTOR SHALL BE HARDWIRED AND INTERCONNECTED WITH THE HOUSEHOLD SMOKE ALARM SYSTEM. THE HEAT DETECTOR SHALL BE LISTED FOR THE AMBIENT ENVIRONMENT AND INSTALLED PER

BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.2 OR THE 2018 IECC. (SEE S-6.0 FOR MORE DETAILS)

ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH 1/8" TO 1/4" OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/150th OF THE AREA OF SPACE VENTILATED. WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED, THE

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AINED HEREIN MAY RESULT IN LIABILITY UNDER APPLICABLE LAW.





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GENERAL NOTES

TABLE R602.3(1) FASTENING SCHEDULE

ITEM	DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER ^{a, b, c}	SPACING AND LOCATION
		4-8D BOX (2 1/2" x 0.113"); OR	
2	BLOCKING BETWEEN CEILING JOISTS OR RAFTERS TO TOP PLATE CEILING JOISTS TO PLATE	3-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 3-10D BOX (3" x 0.128"); OR	TOE NAIL PER JOIST, TOE NAIL
3	CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER	3-3" x 0.131" NAILS 4-10D BOX (3" x 0.128"); OR 3-16D COMMON (3 ½" x 0.162"); OR	FACE NAIL
4	PARTITIONS (SEE SECTION R802.5.2 AND TABLE R802.5.2) CEILING JOIST ATTACHED TO PARALLEL RAFTER (HEEL JOINT)	4-3" x 0.131" NAILS TABLE R802.5.2	FACE NAIL
5	(SEE SECTION R802.5.2 AND TABLE R802.5.2) COLLAR TIE TO RAFTER, FACE NAIL OR	4-10D BOX (3" x 0.128"); OR 3-10D COMMON (3" x 0.148"); OR	FACE NAIL EACH RAFTER
	1 ¹ / ₄ " x 20 GA. RIDGE STRAP TO RAFTER	4-3" x 0.131" NAILS 3-16D BOX NAILS (3 1/2" x 0.135"); OR 3-10D COMMON NAILS (3" x 0.148"); OR	2 TOE NAILS ON ONE SIDE AND 1 TOE
6	RAFTER OR ROOF TRUSS TO PLATE	4-10D BOX (3" x 0.128"); OR 4-3" x 0.131" NAILS 4-16D (3 1/2" x 0.135"); OR 3-10D COMMON (3" x 0.148"); OR	NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS ⁱ
7	ROOF RAFTERS TO RIDGE, VALLEY OR HIP RAFTERS OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM	4-10D BOX (3" x 0.128"); OR 4-3" x 0.131" NAILS 3-16D BOX (3 ¹ / ₂ " x 0.135"); OR 2-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 3-10D BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	TOE NAIL END NAIL
		WALL	
8	STUD TO STUD (NOT BRACED WALL PANELS)	16D COMMON (3 ¹ / ₂ " x 0.162")	24" O.C. FACE NAIL
· · · · · · · · · · · · · · · · · · ·	STOD TO STOD (NOT BRACED WALL PANELS)	10D BOX (3" x 0.128"); OR 3" x 0.131" NAILS	16" O.C. FACE NAIL
9	STUD TO STUD AND ABUTTING STUDS AT INTERSECTING	16D BOX (3 ¹ / ₂ " x 0.135"); OR 3" x 0.131" NAILS	12" O.C. FACE NAIL
	WALL CORNERS (AT BRACED WALL PANELS)	16D COMMON (3 ¹ / ₂ " x 0.162")	16" O.C. FACE NAIL
10	BUILT-UP HEADER (2" TO 2" HEADER WITH 1/2" SPACER)	16D COMMON (3 ¹ / ₂ " x 0.162")	16" O.C. EACH EDGE FACE NAIL
	, = = /2 5. //SLIV	16D BOX (3 ¹ / ₂ " x 0.135")	12" O.C. EACH EDGE FACE NAIL
11	CONTINUOUS HEADER TO STUD	5-8D BOX (2 ¹ / ₂ " x 0.113"); OR 4-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 4-10D BOX (3" x 0.128")	TOE NAIL
10	TOD DI ATE TO TOD DI ATE	16D COMMON (3 ¹ / ₂ " x 0.162")	16" O.C. FACE NAIL
12	TOP PLATE TO TOP PLATE	10D BOX (3" x 0.128"); OR 3" x 0.131" NAILS	12" O.C. FACE NAIL
13	DOUBLE TOP PLATE SPLICE	8-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 12-16D BOX (3 ¹ / ₂ " x 0.135"); OR 12-10D BOX (3" x 0.128"); OR 12-3" x 0.131" NAILS	FACE NAIL ON EACH SIDE OF END JOINT (MINIMUM 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT)
	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING	16D COMMON (3 ¹ / ₂ " x 0.162")	16" O.C. FACE NAIL
14	(NOT AT BRACED WALL PANELS)	16D BOX (3 ¹ / ₂ " x 0.135"); OR 3" x 0.131" NAILS	12" O.C. FACE NAIL
15	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING (AT BRACED WALL PANEL)	3-16D BOX (3 ¹ / ₂ " x 0.135"); OR 2-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 4-3" x 0.131" NAILS	3 EACH 16" O.C. FACE NAIL 2 EACH 16" O.C. FACE NAIL 4 EACH 16" O.C. FACE NAIL
16	TOP OR BOTTOM PLATE TO STUD	4-8D BOX (2 ¹ / ₂ " x 0.113"); OR 3-16D BOX (3 ¹ / ₂ " x 0.135"); OR 4-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 4-10D BOX (3" x 0.128"); OR 4-3" x 0.131" NAILS	TOE NAIL
		3-16D BOX (3 ¹ / ₂ " x 0.135"); OR 2-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 3-10D BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	END NAIL
17	TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	3-10D BOX (3" x 0.128"); OR 2-16D COMMON (3 1/2" x 0.162"); OR 3-3" x 0.131" NAILS	FACE NAIL
18	1" BRACE TO EACH STUD AND PLATE	3-8D BOX (2 ¹ / ₂ " x 0.113"); OR 2-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 2-10D BOX (3" x 0.128"); OR	FACE NAIL
		2 STAPLES 1 ³ / ₄ " 3-8D BOX (2 ¹ / ₂ " x 0.113"); OR 2-8D COMMON (2 ¹ / ₂ " x 0.131"); OR	
19	1" x 6" SHEATHING TO EACH BEARING	2-10D BOX (3" x 0.128"); OR 2 STAPLES, 1" CROWN, 16 GA., 1 ³ / ₄ " LONG	FACE NAIL
20	1" x 8" AND WIDER SHEATHING TO EACH BEARING	3-8D BOX (2 ¹ / ₂ " x 0.113"); OR 3-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 3-10D BOX (3" x 0.128"); OR 3 STAPLES, 1" CROWN, 16 GA., 1 ³ / ₄ " LONG	FACE NAIL
		WIDER THAN 1" x 8" 4-8D BOX (2 ¹ / ₂ " x 0.113"); OR 3-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 3-10D BOX (3" x 0.128"); OR 4 STAPLES, 1" CROWN, 16 GA., 1 ³ / ₄ " LONG	
		FLOOR	
21	JOIST TO SILL, TOP PLATE OR GIRDER	4-8D BOX (2 ¹ / ₂ " x 0.113"); OR 3-8D COMMON (2 ¹ / ₂ " x 0.131"); OR 3-10D BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	TOE NAIL
22	RIM JOIST, BAND JOIST OR BLOCKING TO SILL OR TOP PLATE (ROOF APPLICATIONS ALSO)	8D BOX (2 ¹ / ₂ " x 0.113") 8D COMMON (2 ¹ / ₂ " x 0.131"); OR 10D BOX (3" x 0.128"); OR	4" O.C. TOE NAIL
		3" x 0.131" NAILS 3-8D BOX (2 ½" x 0.131"); OR 3-8D BOX (2 ½" x 0.113"); OR 2-8D COMMON (2 ½" x 0.131"); OR	6" O.C. TOE NAIL
23	1" x 6" SUBFLOOR OR LESS TO EACH JOIST	3-10D BOX (3" x 0.128"); OR 2 STAPLES, 1" CROWN, 16 GA., 1 ³ / ₄ " LONG	FACE NAIL
2.		FLOOR 3-16D BOX (3 ¹ / ₂ " x 0.135"); OR	DIND AND THE
24	2" SUBFLOOR TO JOIST OR GIRDER	2-16D COMMON (3 ½ x 0.162") 3-16D BOX (3 ½" x 0.162")	BLIND AND FACE NAIL
25	2" PLANKS (PLANK & BEAM-FLOOR AND ROOF) BAND OR RIM JOIST TO JOIST	2-16D COMMON (3 ¹ / ₂ " x 0.162") 3-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 4-10D BOX (3" x 0.128"); OR 4-3" x 0.131" NAILS; OR	AT EACH BEARING, FACE NAIL END NAIL
		4-3" x 14 GA. STAPLES, ⁷ / ₁₆ " CROWN 20D COMMON (4" x 0.192"); OR	NAIL EACH LAYER AS FOLLOWS: 32" O.C AT TOP AND BOTTOM AND STAGGERED
27	BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS	10D BOX (3" x 0.128"); OR 3" x 0.131" NAILS AND: 2-20D COMMON (4" x 0.192"); OR 3-10D BOX (3" x 0.128"); OR 3-3" x 0.131" NAILS	24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES FACE NAIL AT ENDS AND AT EACH SPLICI
28	LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	4-16D BOX (3 ¹ / ₂ " x 0.135"); OR 3-16D COMMON (3 ¹ / ₂ " x 0.162"); OR 4-10D BOX (3" x 0.128"); OR	AT EACH JOIST OR RAFTER, FACE NAIL
20		4-3" x 0.131" NAILS	

- NAILS ARE SMOOTH-COMMON, BOX OR DEFORMED SHANKS EXCEPT WHERE OTHERWISE STATED. NAILS USED FOR FRAMING AND SHEATHING CONNECTIONS SHALL HAVE MINIMUM AVERAGE BENDING YIELD STRENGTHS AS SHOWN: 80 KSI FOR
- SHANK DIAMETER OF 0.192 INCH (20D COMMON NAIL), 90 KSI FOR SHANK DIAMETERS LARGER THAN 0.142 INCH BUT NOT LARGER THAN 0.177 INCH, AND 100 KSI FOR SHANK DIAMETERS OF 0.142 INCH OR LESS. STAPLES ARE 16 GAGE WIRE AND HAVE A MINIMUM 7/16-INCH ON DIAMETER CROWN WIDTH.
- NAILS SHALL BE SPACED AT NOT MORE THAN 6 INCHES ON CENTER AT ALL SUPPORTS WHERE SPANS ARE 48 INCHES OR GREATER. FOUR-FOOT BY 8-FOOT OR 4-FOOT BY 9-FOOT PANELS SHALL BE APPLIED VERTICALLY. SPACING OF FASTENERS NOT INCLUDED IN THIS TABLE SHALL BE BASED ON TABLE R602.3(2).
- FOR WOOD STRUCTURAL PANEL ROOF SHEATHING ATTACHED TO GABLE END ROOF FRAMING AND TO INTERMEDIATE SUPPORTS WITHIN 48 INCHES OF ROOF EDGES AND RIDGES, NAILS SHALL BE SPACED AT 6 INCHES ON CENTER WHERE THE
- ULTIMATE DESIGN WIND SPEED IS LESS THAN 130 MPH AND SHALL BE SPACED 4 INCHES ON CENTER WHERE THE ULTIMATE DESIGN WIND SPEED IS 130 MPH OR GREATER BUT LESS THAN 140 MPH.

 GYPSUM SHEATHING SHALL CONFORM TO ASTM C1396 AND SHALL BE INSTALLED IN ACCORDANCE WITH GA 253. FIBERBOARD SHEATHING SHALL CONFORM TO ASTM C208.
- SPACING OF FASTENERS ON FLOOR SHEATHING PANEL EDGES APPLIES TO PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING AND AT FLOOR PERIMETERS ONLY. SPACING OF FASTENERS ON ROOF SHEATHING PANEL EDGES APPLIES TO PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING. BLOCKING OF ROOF OR FLOOR SHEATHING PANEL EDGES PERPENDICULAR TO THE FRAMING MEMBERS NEED NOT BE PROVIDED EXCEPT AS
- REQUIRED BY OTHER PROVISIONS OF THIS CODE. FLOOR PERIMETER SHALL BE SUPPORTED BY FRAMING MEMBERS OR SOLID BLOCKING.

 WHERE A RAFTER IS FASTENED TO AN ADJACENT PARALLEL CEILING JOIST IN ACCORDANCE WITH THIS SCHEDULE, PROVIDE TWO TOE NAILS ON ONE SIDE OF THE RAFTER AND TOE NAILS FROM THE CEILING JOIST TO TOP PLATE IN ACCORDANCE WITH THIS SCHEDULE. THE TOE NAIL ON THE OPPOSITE SIDE OF THE RAFTER SHALL NOT BE REQUIRED.

 RSRS-01 IS A ROOF SHEATHING RING SHANK NAIL MEETING THE SPECIFICATIONS IN ASTM F1667.

CONTINUED TABLE R602.3(1) FASTENING SCHEDULE

ITEM	DECORIDATION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FACTENERS b.c.	SPACING OF FASTENERS				
ITEM	DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER ^{a, b, c}	EDGES (INCHES)h	INTERMEDIATE SUPPORTS ^{c, e} (INCHES)			
	WOOD STRUCTURAL PANELS, SUBFLOOR, ROOF AND INTERIOR WALL SHEATHING TO FRAMING AND PARTICLEBOARD WALL SHEATHING TO FRAMING [SEE TABLE R602.3(3) FOR WOOD STRUCTURAL PANEL <i>EXTERIOR</i> WALL SHEATHING TO WALL FRAMING]						
30	³ /8" - ¹ / ₂ "	6D COMMON (2" x 0.113") NAIL (SUBFLOOR, WALL) ⁱ 8D COMMON (2 ¹ / ₂ " x 0.131") NAIL (ROOF); OR RSRS-01 (2 ³ / ₈ " x 0.113") NAIL (ROOF)	6	12 ^f			
31	¹⁹ / ₃₂ " - 1"	8D COMMON NAIL (2 ¹ / ₂ " x 0.131"); OR RSRS-01 (2 ³ / ₈ " x 0.113") NAIL (ROOF) ^j	6	12 ^f			
32	1 ¹ /8" - 1 ¹ /4"	10D COMMON (3" x 0.148") NAIL; OR 8D (2 ½" x 0.131") DEFORMED NAIL	6	12			
	OTHER WALL SHEATHING ⁹						
33	1/2" STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	1 ¹ / ₂ " GALVANIZED ROOFING NAIL, ⁷ / ₁₆ " HEAD DIAMETER, OR 1 ¹ / ₄ " LONG 16 GA. STAPLE WITH ⁷ / ₁₆ " OR 1" CROWN	3	6			
34	²⁵ / ₃₂ " STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	1 3/4" GALVANIZED ROOFING NAIL, 7/16" HEAD DIAMETER, OR 1 1/2" LONG 16 GA. STAPLE WITH 7/16" OR 1" CROWN	3	6			
35	1/2" GYPSUM SHEATHING ^d	1 1/2" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1 1/2" LONG; 1 1/4" SCREWS, TYPE W OR S	7	7			
36	5/8" GYPSUM SHEATHING ^d	1 ³ / ₄ " GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1 ⁵ / ₈ " LONG; 1 ⁵ / ₈ " SCREWS, TYPE W OR S	7	7			
	WOOD STRUCTURAL PANELS, COMBINATION SUBFLOOR UNDERLAYMENT TO FRAMING						
37	³ / ₄ " AND LESS	6D DEFORMED (2" x 0.120") NAIL; OR 8D COMMON (2 1/2" x 0.131") NAIL	6	12			
38	⁷ / ₈ " - 1"	8D COMMON (2 ¹ / ₂ " x 0.131") NAIL; OR 8D DEFORMED (2 ¹ / ₂ " x 0.120") NAIL	6	12			
39	1 1/8" - 1 1/4"	10D COMMON (3" x 0.148") NAIL; OR 8D DEFORMED (2 1/2" x 0.120") NAIL	6	12			

TABLE R602.3(2) ALTERNATE ATTACHMENTS TO TABLE R602.3(1)

NOMINAL MATERIAL		SPACING° OF FASTENERS			
THICKNESS (INCHES)	DESCRIPTION ^{a, b} OF FASTENER AND LENGTH (INCHES)	EDGES (INCHES) INTERMEDIATE SUPPORTS (INCHES)			
WOOD STRUCT	JRAL PANELS SUBFLOOR, ROOF ⁹ AND WALL SHEATHING TO FRAMING AND PART	CLEBOARD WALL SH	HEATHING TO FRAMING ^f		
	STAPLE 15 GA. 1 ³ / ₄	4	8		
UP TO ¹ / ₂	0.097 - 0.099 NAIL 2 ¹ / ₄	3	6		
	STAPLE 16 GA. 1 ³ / ₄	3	6		
	0.113 NAIL 2	3	6		
¹⁹ / ₃₂ AND ⁵ / ₈	STAPLE 15 AND 16 GA. 2	4	8		
	0.097 - 0.099 NAIL 2 ¹ / ₄	4	8		
	STAPLE 14 GA. 2	4	8		
227 AND 27	STAPLE 15 GA. 1 ³ / ₄	3	6		
²³ / ₃₂ AND ³ / ₄	0.097 - 0.099 NAIL 2 ¹ / ₄	4	8		
	STAPLE 16 GA. 2	4	8		
	STAPLE 14 GA. 2 1/4	4	8		
,	0.113 NAIL 2 ¹ / ₄	3	6		
1	STAPLE 15 GA. 2 ¹ / ₄	4	8		
	0.097 - 0.099 NAIL 2 ¹ / ₂	4	8		
NOMINAL MATERIAL		SPACING° OF FASTENERS			
THICKNESS (INCHES)	DESCRIPTION ^{a, b} OF FASTENER AND LENGTH (INCHES)	EDGES (INCHES)	BODY OF PANEL ^d (INCHES)		
	FLOOR UNDERLAYMENT; PLYWOOD-HARDBOARD-PARTICLEBOARDf-	FIBER-CEMENT ^h			
	FIBER-CEMENT				
	3D, CORROSION-RESISTANT, RING SHANK NAILS (FINISHED FLOORING OTHER THAN TILE)	3	6		
41	STAPLE 18 GA., ⁷ / ₈ LONG, ³ / ₄ CROWN (FINISHED FLOORING OTHER THAN TILE)	3	6		
1/4	1 1/4 LONG x .121 SHANK x .375 HEAD DIAMETER CORROSION-RESISTANT (GALVANIZED OR STAINLESS STEEL) ROOFING NAILS (FOR TILE FINISH)	8	8		
	1 ¹ / ₄ LONG, NO. 8 x .375 HEAD DIAMETER, RIBBED WAFER-HEAD SCREWS (FOR TILE FINISH)	8	8		
	PLYWOOD				
44 433 54	1 ¹ / ₄ RING OR SCREW SHANK NAIL-MINIMUM 12 ¹ / ₂ GA. (0.099") SHANK DIAMETER	3	6		
¹ / ₄ AND ⁵ / ₁₆	STAPLE 18 GA., ⁷ / ₈ , ³ / ₁₆ CROWN WIDTH	2	5		
¹¹ / ₃₂ , ³ / ₈ , ¹⁵ / ₃₂ AND ¹ / ₂	1 ¹ / ₄ RING OR SCREW SHANK NAIL-MINIMUM 12 ¹ / ₂ GA. (0.099") SHANK DIAMETER	6	8e		
	1 ½ RING OR SCREW SHANK NAIL-MINIMUM 12 ½ GA (0.099") SHANK DIAMETER	6	8		
¹⁹ / _{32,} ⁵ / _{8,} ²³ / ₃₂ AND ³ / ₄	1 1/2 RING OR SCREW SHANK NAIL-MINIMUM 12 1/2 GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 1/2	6	8		
¹⁹ / ₃₂ , ⁵ / ₈ , ²³ / ₃₂ AND ³ / ₄	12 ¹ / ₂ GA. (0.099") SHANK DIAMETER				
¹⁹ / ₃₂ , ⁵ / ₈ , ²³ / ₃₂ AND ³ / ₄	12 ¹ / ₂ GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 ¹ / ₂				
¹⁹ / ₃₂ , ⁵ / ₈ , ²³ / ₃₂ AND ³ / ₄	12 ¹ / ₂ GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 ¹ / ₂ HARDBOARD ^f	6	8		
	12 ¹ / ₂ GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 ¹ / ₂ HARDBOARD ^f 1 ¹ / ₂ LONG RING-GROOVED UNDERLAYMENT NAIL	6	6		
	12 ¹ / ₂ GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 ¹ / ₂ HARDBOARD ^f 1 ¹ / ₂ LONG RING-GROOVED UNDERLAYMENT NAIL 4D CEMENT-COATED SINKER NAIL	6 6	8 6 6		
0.200	12 ¹ / ₂ GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 ¹ / ₂ HARDBOARD ^f 1 ¹ / ₂ LONG RING-GROOVED UNDERLAYMENT NAIL 4D CEMENT-COATED SINKER NAIL STAPLE 18 GA., ⁷ / ₈ LONG (PLASTIC COATED)	6 6	8 6 6		
	12 ¹ / ₂ GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 ¹ / ₂ HARDBOARD ^f 1 ¹ / ₂ LONG RING-GROOVED UNDERLAYMENT NAIL 4D CEMENT-COATED SINKER NAIL STAPLE 18 GA., ⁷ / ₈ LONG (PLASTIC COATED) PARTICLEBOARD	6 6 3	6 6 6		
0.200	12 ¹ / ₂ GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 ¹ / ₂ HARDBOARD ^f 1 ¹ / ₂ LONG RING-GROOVED UNDERLAYMENT NAIL 4D CEMENT-COATED SINKER NAIL STAPLE 18 GA., ⁷ / ₈ LONG (PLASTIC COATED) PARTICLEBOARD 4D RING-GROOVED UNDERLAYMENT NAIL	6 6 6 3	8 6 6 6		
0.200	12 1/2 GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 1/2 HARDBOARD ^f 1 1/2 LONG RING-GROOVED UNDERLAYMENT NAIL 4D CEMENT-COATED SINKER NAIL STAPLE 18 GA., 7/8 LONG (PLASTIC COATED) PARTICLEBOARD 4D RING-GROOVED UNDERLAYMENT NAIL STAPLE 18 GA., 7/8 LONG, 3/16 CROWN	6 6 6 3 3 3 3	6 6 6 6		
0.200	12 1/2 GA. (0.099") SHANK DIAMETER STAPLE 16 GA.1 1/2 HARDBOARD 1 1/2 LONG RING-GROOVED UNDERLAYMENT NAIL 4D CEMENT-COATED SINKER NAIL STAPLE 18 GA., 7/8 LONG (PLASTIC COATED) PARTICLEBOARD 4D RING-GROOVED UNDERLAYMENT NAIL STAPLE 18 GA., 7/8 LONG, 3/16 CROWN 6D RING-GROOVED UNDERLAYMENT NAIL	6 6 6 3 3 3 3 6	6 6 6 6 6 10		

- NAIL IS A GENERAL DESCRIPTION AND SHALL BE PERMITTED TO BE T-HEAD, MODIFIED ROUND HEAD OR ROUND HEAD. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16-INCH ON DIAMETER EXCEPT AS NOTED.
- NAILS OR STAPLES SHALL BE SPACED AT NOT MORE THAN 6 INCHES ON CENTER AT ALL SUPPORTS WHERE SPANS ARE 48 INCHES OR GREATER. NAILS OR STAPLES SHALL BE SPACED AT NOT MORE THAN 12 INCHES ON CENTER AT INTERMEDIATE
 - FASTENERS SHALL BE PLACED IN A GRID PATTERN THROUGHOUT THE BODY OF THE PANEL.
 FOR 5-PLY PANELS, INTERMEDIATE NAILS SHALL BE SPACED NOT MORE THAN 12 INCHES ON CENTER EACH WAY.
- HARDBOARD UNDERLAYMENT SHALL CONFORM TO CPA/ANSI A135.4

 SPECIFIED ALTERNATE ATTACHMENTS FOR ROOF SHEATHING SHALL BE PERMITTED WHERE THE ULTIMATE DESIGN WIND SPEED IS LESS THAN 130 MPH. FASTENERS ATTACHING WOOD STRUCTURAL PANEL ROOF SHEATHING TO GABLE END WALL
- FRAMING SHALL BE INSTALLED USING THE SPACING LISTED FOR PANEL EDGES.
 FIBER-CEMENT UNDERLAYMENT SHALL CONFORM TO ASTM C1288 OR ISO 8336, CATEGORY C.

DESIGN LOADS (PSF)

THE DWELLING SHALL COMPLY WITH THE FOLLOWING LOAD CONDITIONS

AREA	MIN. DEAD LOAD	MIN. LIVE LOAD
EXTERIOR BALCONIES	10	60
DECKS, STAIRS	10	40
CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE 3:12 OR LESS	10	10
CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE OVER 3:12	10	10
CEILING JOISTS / ATTICS WITH STORAGE - DOOR PULL DOWN LADDER ACCESS	10	20
ROOMS: NON-SLEEPING	10	40
ROOMS: SLEEPING	10	30
ROOF: LIGHT ROOF COVERING	10	20
ROOF: HEAVY ROOF COVERING / CONCRETE / TILE / SLATE	20	20
GUARDRAILS, HANDRAILS	200# LL 1	NORMAL

HEAVY ROOF COVERING MATERIAL (TILE, CONCRETE, SLATE, ETC.) SHALL NOT BE USED UNLESS 20 PSF DEAD LOAD AND HEAVY ROOF IS NOTED ON THE ROOF PLAN. IF HEAVY ROOFING IS TO BE USED AND IS NOT NOTED ON THE ROOF PLAN, NOTIFY ENGINEER PRIOR TO ANY CONSTRUCTION, INCLUDING FOUNDATION AND SITE WORK. IF THE PLAN HAS BEEN DESIGNED FOR HEAVY

COLUMN SCHEDULE

BASED ON FOOTING SIZE (ASSUME 1500 PSF SOIL)

REINFORCEMENT	COL. MIN.	COL. TYPE	MAX. LOAD
(4) #4 BARS E/W	3"	SCH40	6K
(5) #4 BARS E/W	3"	SCH40	9.4K
(6) #4 BARS E/W	3"	SCH40	13.5K
(7) #4 BARS E/W	3 1/2"	SCH40	18.4K
(8) #4 BARS E/W	3 1/2"	SCH40	24.0K
(9) #4 BARS E/W	3 1/2"	SCH40	30.4K
(10) #4 BARS E/W	3 1/2"	SCH40	37.5K
	(4) #4 BARS E/W (5) #4 BARS E/W (6) #4 BARS E/W (7) #4 BARS E/W (8) #4 BARS E/W (9) #4 BARS E/W	REINFORCEMENT MIN. (4) #4 BARS E/W 3" (5) #4 BARS E/W 3" (6) #4 BARS E/W 3" (7) #4 BARS E/W 3 1/2" (8) #4 BARS E/W 3 1/2" (9) #4 BARS E/W 3 1/2"	REINFORCEMENT MIN. TYPE (4) #4 BARS E/W 3" SCH40 (5) #4 BARS E/W 3" SCH40 (6) #4 BARS E/W 3" SCH40 (7) #4 BARS E/W 3 1/2" SCH40 (8) #4 BARS E/W 3 1/2" SCH40 (9) #4 BARS E/W 3 1/2" SCH40

COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. 1/2" x 2" BOLTS SHOULD THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE

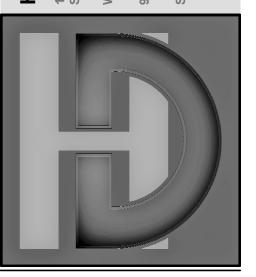
ENGINEERED LUMBER

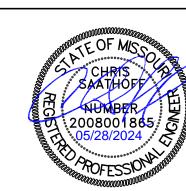
MIN. DESIGN REQUIREMENTS

	F _b (psi)	E (psi)	F _v (psi)
LVL	2600	1.8x10	285
GLULAM	2400	1.8x10	190
PARALAM	2600	2.0x10	290

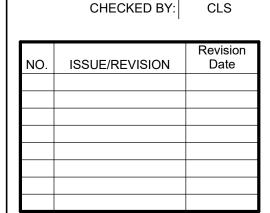
<u>BUILDER'S PLANS:</u> THE TERM "BUILDER'S PLANS" REFERS TO A CERTAIN LEVEL OF DEVELOPMENT OF THE DRAWINGS. AS THE NAME IMPLIES, THESE PLANS REQUIRE THAT THE CONTRACTOR POSSESSES COMPETENCE IN RESIDENTIAL CONSTRUCTION AND A THOROUGH UNDERSTANDING OF THE INTERNATIONAL RESIDENTIAL CODE (IRC). THE CONTRACTOR WARRANTS TO HD ENGINEERING & DESIGN THAT THEY POSSESSES THE PARTICULAR COMPETENCE AND SKILL IN CONSTRUCTION NECESSARY TO BUILD THIS PROJECT WITHOUT FULL ENGINEERING AND DESIGN SERVICES, AND FOR THAT REASON THE CONTRACTOR OR HOME OWNER HAS RESTRICTED THE SCOPE OF PROFESSIONAL SERVICES. THE CONSTRUCTION DOCUMENTS PROVIDED BY THE LIMITED SERVICES SHALL BE TERMED "BUILDER'S PLANS" IN RECOGNITION OF THE CONTRACTOR'S SOPHISTICATION. ALTHOUGH HD ENGINEERING & DESIGN HAVE PERFORMED THEIR SERVICES WITH DUE CARE AND DILIGENCE, WE CANNOT GUARANTEE PERFECTION. ANY AMBIGUITY OR DISCREPANCY DISCOVERED BY THE USE OF THESE PLANS SHALL BE REPORTED IMMEDIATELY TO HD ENGINEERING. CONSTRUCTION MAY REQUIRE THAT THE CONTRACTOR ADAPT THE "BUILDER'S PLANS" TO THE FIELD CONDITIONS ENCOUNTERED AND MAKE LOGICAL ADJUSTMENTS IN FIT, FORM, DIMENSION AND QUANTITY. CHANGES MADE FROM THE PLANS WITHOUT THE CONSENT OF HD ENGINEERING & DESIGN ARE UNAUTHORIZED. IT IS ALSO UNDERSTOOD THAT THE CONTRACTOR WILL BE RESPONSIBLE FOR MEETING ALL APPLICABLE BUILDING CODES INCLUDING BUT NOT LIMITED TO MECHANICAL, ELECTRICAL, AND PLUMBING CODE REQUIREMENTS (WHICH IS EXCLUDED FROM THESE PLANS). IN THE EVENT ADDITIONAL DETAIL OR GUIDANCE IS NEEDED BY THE CONTRACTOR OR HOMEOWNER FOR CONSTRUCTION OF ANY ASPECT OF THE PROJECT, HD ENGINEERING & DESIGN OR A QUALIFIED ENGINEER SHALL IMMEDIATELY BE RETAINED. FAILURE TO NOTIFY US OF THESE NEEDS OR OF CHANGES TO THE PLANS SHALL RELIEVE HD ENGINEERING & DESIGN OF ALL RESPONSIBILITIES OF THE CONSEQUENCES.

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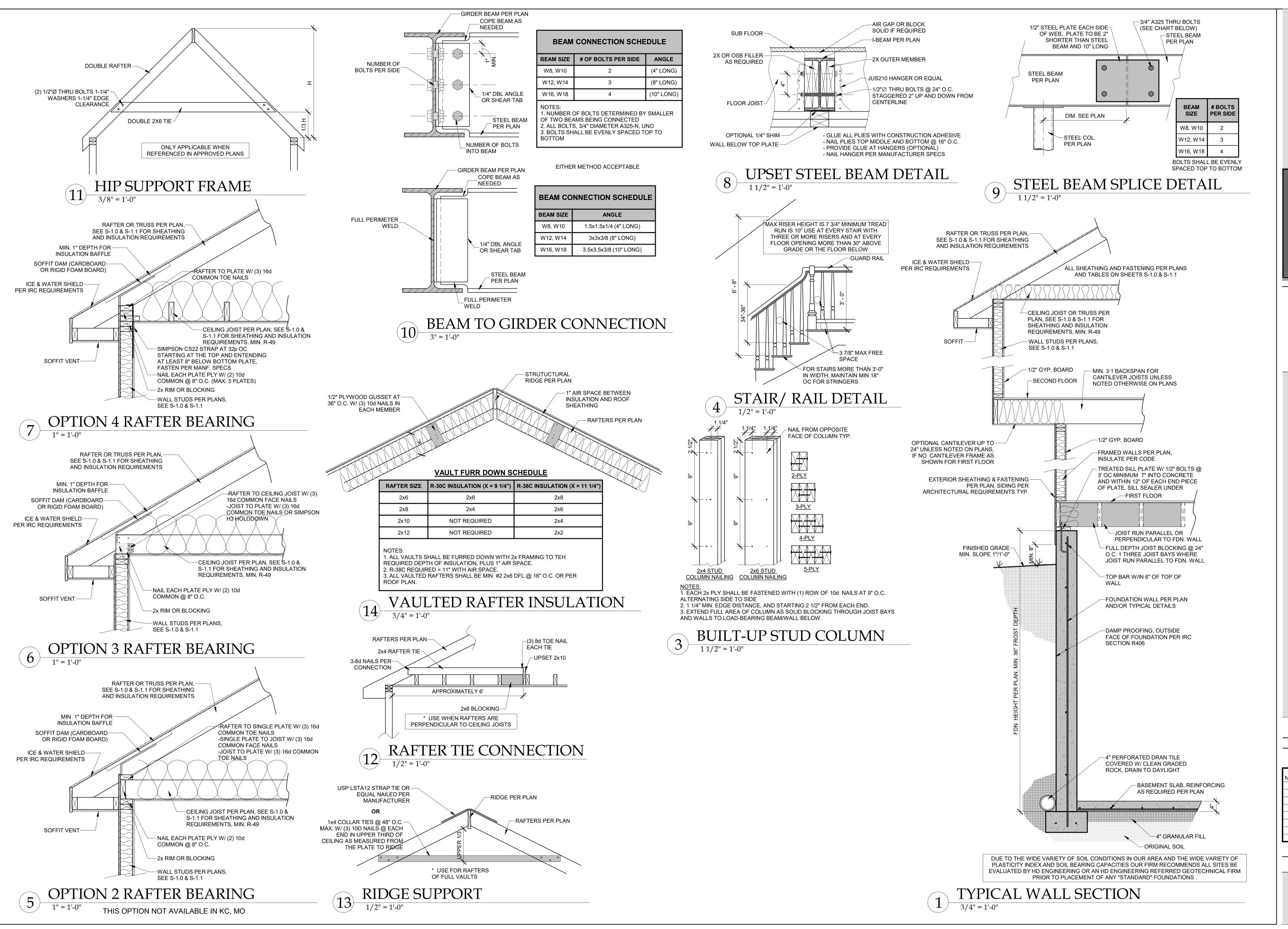




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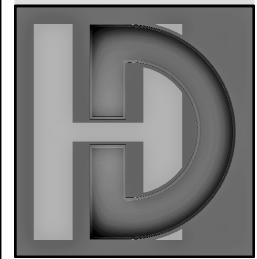
GENERAL NOTES

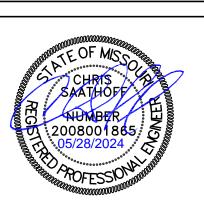


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N.HDENGINEERS.COM
S31.2222





WART BUILDERS
81 - SUMMIT VIEW FARMS
SERENA PL., LEE'S SUMMIT, M

#: 44350

CHECKED BY: CLS

05/28/2024

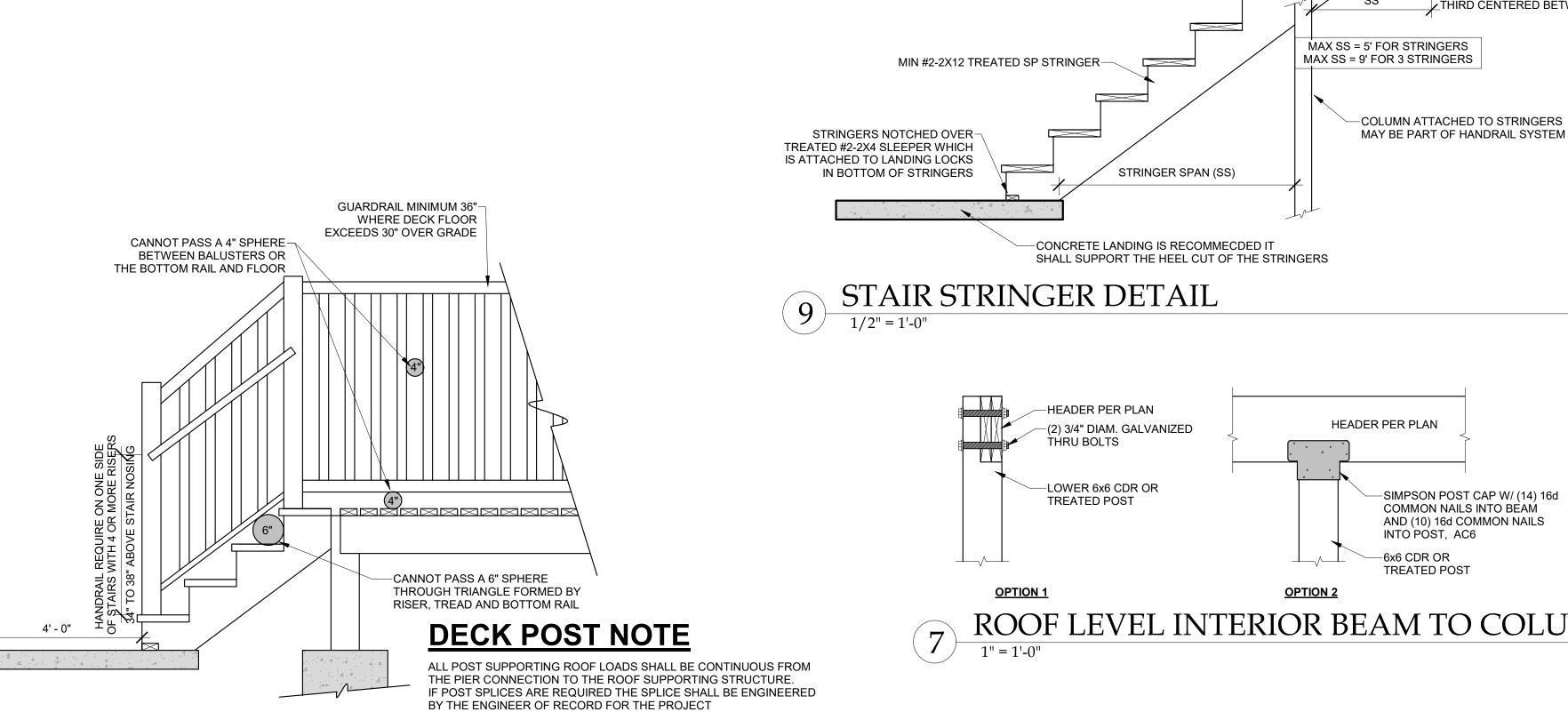
NO. ISSUE/REVISION Date

DATE:

FRAMING SECTIONS

S-1.2

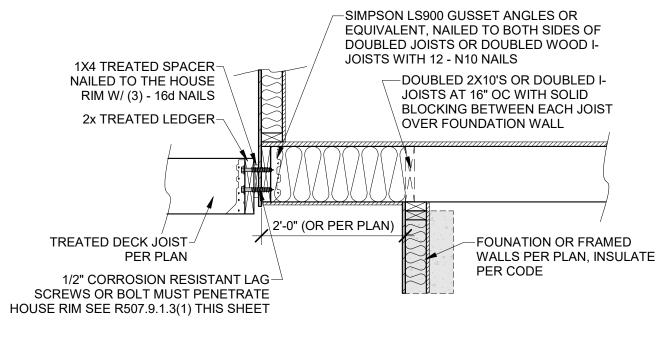
RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
06/25/2024 8:29:49



-UPPER 6x6 CDR OR TREATED POST -UPPER 6x6 CDR OR UPPER TO LOWER TREATED POST COLUMN SPLICE LINE SIMPSON POST BASE W/ (6) 16d COMMON NAILS -(6) 1/4" LEDGER-LOK'S INTO BEAM AND POST, BC6 (3) EACH SIDE BEAM PER PLAN BEAM PER PLAN -(2) 3/4" DIAM. GALVANIZED THRU BOLTS -LOWER 6x6 CDR OR -SIMPSON POST CAP W/ (14) 16d TREATED POST COMMON NAILS INTO BEAM OPTIONS FOR USE IF POST IS NOT AND (10) 16d COMMON NAILS CONTINUOUS WITH LET-IN BEAM, IF POST INTO PÓST, AC6 IS CONTINUOUS LET IN BEAM AND BOLT -6x6 CDR OR PER OPTION 1 TREATED POST DECK LEVEL INTERIOR BEAM TO COLUMN

UPPER 6x6 CDR OR TREATED POST UPPER TO LOWER COLUMN SPLICE LINE -SIMPSON POST BASE W/ -(6) 1/4" LEDGER-LOK'S (6) 16d COMMON NAILS INTO BEAM AND POST, BC6 (3) EACH SIDE BEAM PER PLAN BEAM PER PLAN -(2) 3/4" DIAM. GALVANIZED THRU BOLTS -SIMPSON END POST CAP -LOWER 6x6 CDR OR W/ (14) 16d COMMON NAILS TREATED POST INTO BEAM AND (10) 16d COMMON NAILS INTO POST OPTIONS FOR USE IF POST IS NOT LCE6 CONTINUOUS WITH LET-IN BEAM, IF POST IS CONTINUOUS LET IN BEAM -6x6 CDR OR AND BOLT PER OPTION 1 TREATED POST

DECK LEVEL EXTERIOR BEAM TO COLUMN



DECK LEDGER TO CANTILEVER

shall be permitted to substitute for you to 1/2" of allowable sheathing thickness where combined with wood structural panel or lumbers sheathing. **TABLE IRC2018 R507.9.1.3(2)** PLACEMENT OF LAG SCEWS AND BOLT IN **DECK LEDGERS AND BAND JOISTS**

e. Sheathing shall be permitted to be wood structural panel, gypsum board, fiberboard lumber or foam sheathing. Up to 1/2" thinckness of stacked washers

a. Ledges shall be flashed in accordance with Section R703.4 to prevent water from contacting the house band joist.

For SI: 1 inch = 25.4mm, 1 foot = 304.8mm, 1 pound per square foot = 0.0479 kPa

c. The tip of the lag screw shall fully extend beyond the inside face of the band joist.

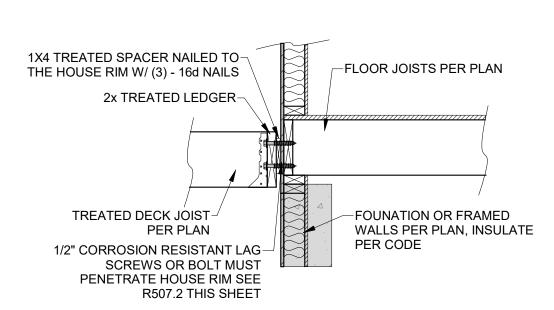
b. Snow load shall not be assumed to act concurrently with live load.

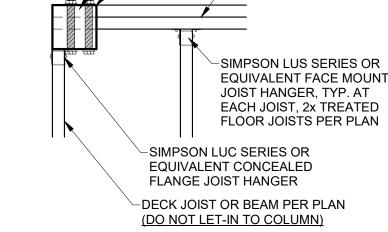
d. Sheathing shall be wood structural panel or solid sawn lumber.

MINIMUM EN	ND AND EDGE D	ISTANCES AND S	PACING BETWEEN	I ROWS
	TOP EDGE	BOTTOM EDGE	ENDS	ROW SPACING
LEDGER ^a	2 inches ^d	3/4 inches	2 inches ^b	1 5/8 inches ^b
BAND JOIST ^c	3/4 inches	2 inches	2 inches	1 5/8 inches ^b

For SI: 1 inch = 25.4mm. a. Lag screws of bolts shal lbe staggered from the top to the bottom along the horizontal run of the deck ledger in accordance with Figure R507.9.1.3(1) b. Maximum 5 inces

c. For engineered rim joists, the manufacturer's recommendations shall govern. d. The minimum distances from bottom row of lag screws or bolts to the top of the ledger shall be in accordance with Figure R507.9.1.3(1)

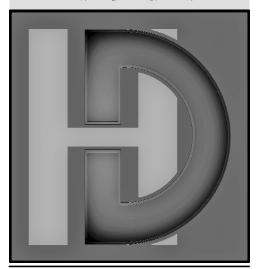




DECK CORNER COLUMN

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44350 05/28/2024 DATE: CHECKED BY: CLS

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DECK DETAILS

-LET IN BEAM PER DETAILS THIS SHEET BEAM PER PLAN **EQUIVALENT FACE MOUNT**

-6x6 CDR OR TREATED POST

OR POST PER PLAN

GUARD RAIL

HEADER PER PLAN SIMPSON POST CAP W/ (14) 16d COMMON NAILS INTO BÈAM AND (10) 16d COMMON NAILS INTO POST, AC6 -6x6 CDR OR TREATED POST

ROOF LEVEL INTERIOR BEAM TO COLUMN

TOP OF EACH STRINGER IS-TOE-NAILED (TYP) AND

ON ONE SIDE, OR SLOPED

-MIN (2) STRINGERS 24" OC TREÀÓS OVERHANGE 5" EACH

SIDE (3) STRINGERS 36" OC THE

THIRD CENTERED BETWEEN

SUPPORTED BY SIMPSON LS70

GUSSET ANGLE OR EQUIVALENT

TABLE IRC2018 R507.9.1.3(1) DECK LEDGER CONNECTION TO BAND JOIST a.b (DECK LIVE LOAD = 40 PSF, DECK HEAD LOAD = 10 PSF, SNOW LOAD < 40 PSF)

JOIST SPAN	6' AND LESS	6'-1" TO 8'	8'-1" TO 10'	10'-1" TO 12'	12'-1" TO 14'	14'-1" TO 16'	16'-1" TO 18'
CONNECTION DETAILS			ON-CENTE	R SPACING OF F	ASTENERS d, e		
1/2" LAG SCREW WITH 15/32" MAX. SHEATHING ^{c,d}	30	23	18	15	13	11	10
1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING ^d	36	36	34	29	24	21	19
1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING & 1/2" STACKED WASHERS ^e	36	36	29	24	21	18	16

TABLE R602.3(5) SIZE, HEIGHT AND SPACING OF WOOD STUDS

			BEARING WALLS			NON-BEAR	ING WALLS
STUD SIZE (INCHES)	LATERALLY UNSUPPORTED STUD HEIGHT ^a (FEET)	MAXIMUM SPACING WHERE SUPPORTING A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY, ONLY (INCHES)	MAXIMUM SPACING WHERE SUPPORTING ONE FLOOR, PLUS A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY (INCHES)	MAXIMUM SPACING WHERE SUPPORTING TWO FLOORS, PLUS A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY (INCHES)	MAXIMUM SPACING WHERE SUPPORTING ONE FLOOR HEIGHT ^a (INCHES)	LATERALLY UNSUPPORTED STUD HEIGHT ^a (FEET)	MAXIMUM SPACING (INCHES)
2 x 3 ^b						10	16
2 x 4	10	24°	16°		24	14	24
3 x 4	10	24	24	16	24	14	24
2 x 5	10	24	24		24	16	24
2 x 6	10	24	24	16	24	20	24

LISTED HEIGHTS ARE DISTANCES BETWEEN POINTS OF LATERAL SUPPORT PLACED PERPENDICULAR TO THE PLANE OF THE WALL. BEARING WALLS SHALL BE SHEATHED ON NOT LESS THAN ONE SIDE OR BRIDGING SHALL BE INSTALLED NOT GREATER THAN 4 FEET APART MEASURED VERTICALLY FROM EITHER END OF THE STUD. INCREASES IN UNSUPPORTED HEIGHT ARE PERMITTED WHERE IN COMPLIANCE WITH EXCEPTION 2 OF SECTION R602.3.1 OR DESIGNED IN ACCORDANCE WITH

SHALL NOT BE USED IN EXTERIOR WALL A HABITABLE ATTIC ASSEMBLY SUPPORTED BY 2 x 4 STUDS IS LIMITED TO A ROOF SPAN OF 32 FEET. WHERE THE ROOF SPAN EXCEEDS 32 FEET, THE WALL STUDS SHALL BE INCREASED TO 2 x 6 OR THE STUDS SHALL BE DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICE.

RESIDENTIAL SEISMIC & WIND ANALYSIS

 q_{z10} =0.00256 $K_z K_{zt} K_d V^2$ (ASCE7-16 Velocity Pressure) $q_{z10_ASD} = 0.6 q_{z10} \quad \text{(Design Velocity Pressure for ASD analysis under ASCE7-16 and IRC/IBC 2018)}$

1ST FLOOR TRIBUTARY WEIGH $S_{\text{S}}\left(\text{SITE GROUND MOTION - }\%g \text{ - FROM ASCE7 SEISMIC MAP}\right)$ F_a (from ASCE7 Table 11.4-1)

59730 72530 72530 10.0% S_{DS} (= 2/3 * S_S * F_a) R (from ASCE7 Table 12.2-1)

2ND FLOOR				842	
1ST FLOOR				1023	
BASEMENT				1023	
Observations I constitute	Min Objection Oak adula	Factoria a Oaka dula	Allamakia Oh	(44) 5	lo
Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable Sh	near (#/LF)	Code Reference
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	159	5	per IBC, Table 2306.3(1)
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	230	()	per IBC, Table 2306.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	310		per IBC, Table 2306.3(1)
Exterior <u>(Option #4)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220	0	AF&PA SDPWS Table 4.3A
Exterior <u>(Option #5)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	32(0	AF&PA SDPWS Table 4.3A
Exterior <u>(Option #6)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing	6d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field	200	0	AF&PA SDPWS Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60		per IBC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)	329	5	

EXTERIOR SHEAT	THING OPTION FOR SEC	OND FLOOR	4		WIDTH OF 1ST STORY (FT.)	59	WIDTH OF 2ND STORY (FT.)	1
EXTERIOR SHEAT	THING OPTION FOR FIRS	T FLOOR	4		DEPTH OF 1ST STORY (FT.)	59.5	DEPTH OF 2ND STORY (FT.)	1
EXTERIOR SHEAT	THING OPTION FOR BASE	EMENT WALLS	4		BACK WALL OF GARAGE (FT.)	31		
				•	GAR. WALL: 1=F-B, 2=S-S	2		
			EX	TERIOR STRUCTURAL WA	ALL LENGTHS (ft.) & RESISTANCES			
		S	EISMIC			WIND		
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs
1ST FLOOR	135	37800	55.5	15540	135	52920	55.5	21756
BASEMENT	17.5	4900	47	13160	17.5	6860	47	18424
				Ī				
			STANCE REQUIRED		Anchor Bolt Spacing	(in.)	16d Nail Spacing req'd at I	ottom plate (in.)
2ND FLOOR FROM	NT TO BACK	SEISMIC 842	WIND 4670		diameter (in.) Shear value (per NDS)	944	2nd Floor F-B 2nd Floor S-S	-
2ND FLOOR FROM		842	4851		Spacing F-B (inches)	169.5	1st Floor F-B	
1ST FLOOR FROM		042	0		spacing S-S (inches)	164.9	1st Floor S-S	
1ST FLOOR SIDE-		0	0		spacing 0-0 (inches)	104.5	1311 1001 0-0	
BASEMENT FROM		0	11855					
BASEMENT SIDE-	TO-SIDE	0	0					
			RESISTANCE RE	QUIRED IN ADDITION TO	RESISTANCE PROVIDED BY EXTERIOR	OR WALLS**		
		ADDITIONAL	PORTAL FRAMES OR	INTERIOR X-BRACES	INTERIOR WALL LENGTH W/ 1/2"	BURIED CONCRETE FOUNDATION WALL (MIN.	RESISTANCE PROVIDED BY	OKS

BASEMENT FRONT-TO-BACK	0	11855					
BASEMENT SIDE-TO-SIDE	0	0					
		RESISTANCE RE	QUIRED IN ADDITION TO	RESISTANCE PROVIDED BY EXTERIO	OR WALLS**		
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	BURIED CONCRETE FOUNDATION WALL (MIN. WALL RESISTANCE = 1500#/FT)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)	OK?
1ST FLOOR FRONT-TO-BACK	0					0	YES
1ST FLOOR SIDE-TO-SIDE	0					0	YES
BASEMENT FRONT-TO-BACK	11855				40	15680	YES
BASEMENT SIDE-TO-SIDE	0					0	YES
**NOTES: 1) SEE ATTACHED CALCULA					=/NAILING		_

PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER

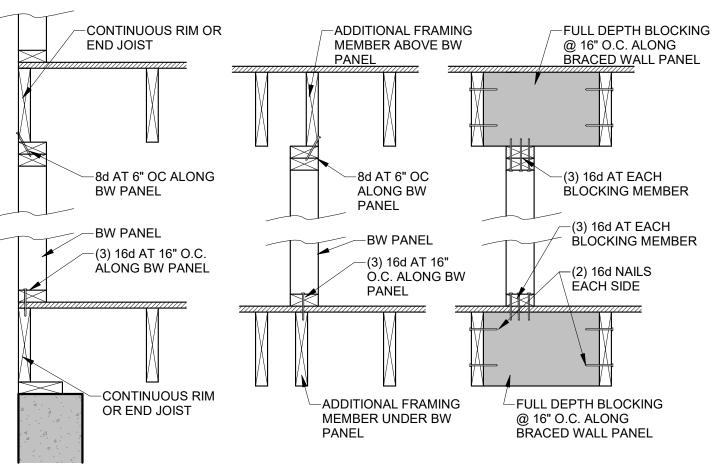
				WIND UPI	LIFT ANALYSIS		
	X/12	DEGREES					
ROOF PITCH (MAX)	8	33.7	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2			
		ASCE 7					
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)			
OVERHANG	1	-1.08	239	-1.08			
	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)
MAIN ROOF**	3510.5	-491.84	4002.34	-1.08	-0.36	-910	-3.8
*ALONG PERIMETER		TOTAL UPLIFT PER LINEAL	FOOT ALONG EXTERIOR (PO	UNDS)	-4.9	UPLIFT OK	
**INSIDE EXTERIOR V	VALLS	RESISTANCE DUE TO DEAD	WEIGHT & (3) 10d TOENAILS		251.6		

'HE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2018 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 220 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC) NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION

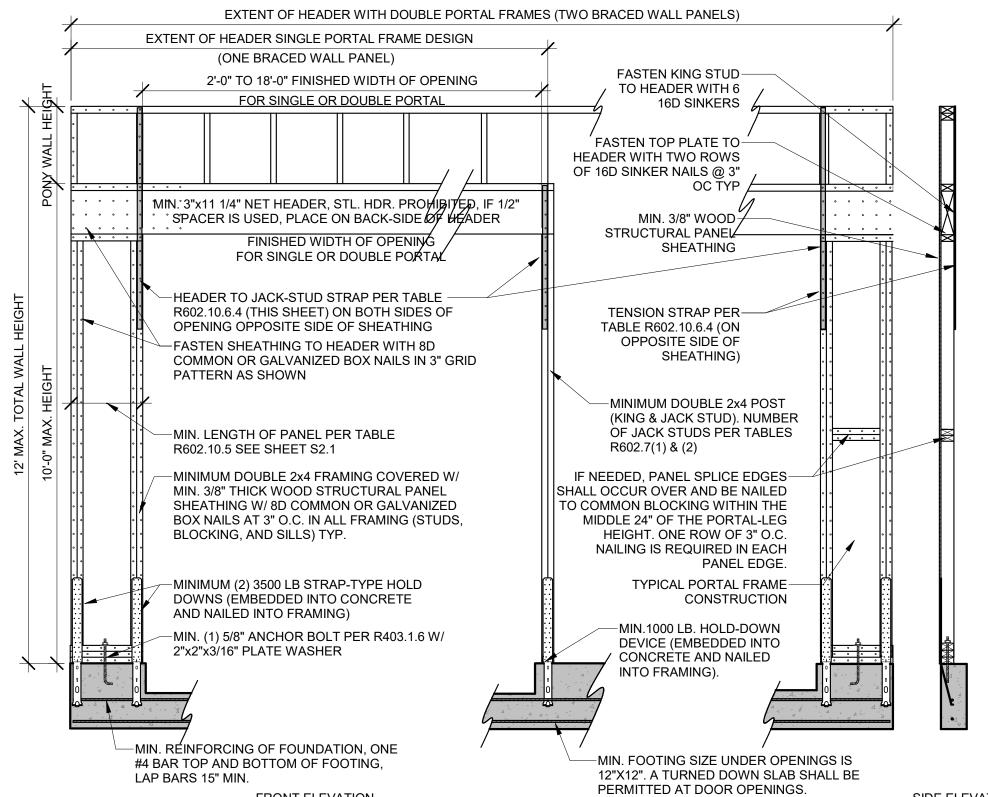
CONTINUOUS RIM OR **END JOIST** -PERPENDICULAR JOISTS -FULL DEPTH BLOCKING @ 16" O.C. ALONG **BRACED WALL PANEL** -8d AT 6" OC ALONG -8d AT 6" OC **BW PANEL** ALONG BW PANEL —BW PANEL -(3) 16d AT 16" O.C. -(3) 16d AT 16" **ALONG BW PANEL** O.C. ALONG BW PANEL -PERPENDICULAR -PERPENDICULAR JOISTS -CONTINUOUS RIM FULL DEPTH BLOCKING OR END JOIST @ 16" O.C. ALONG BRACED WALL PANEL

BRACED WALL PANEL CONNECTION WHEN PERPENDICULAR TO FLOOR/CEILING JOISTS



BRACED WALL PANEL CONNECTION WHEN PARALLEL TO FLOOR/CEILING JOISTS

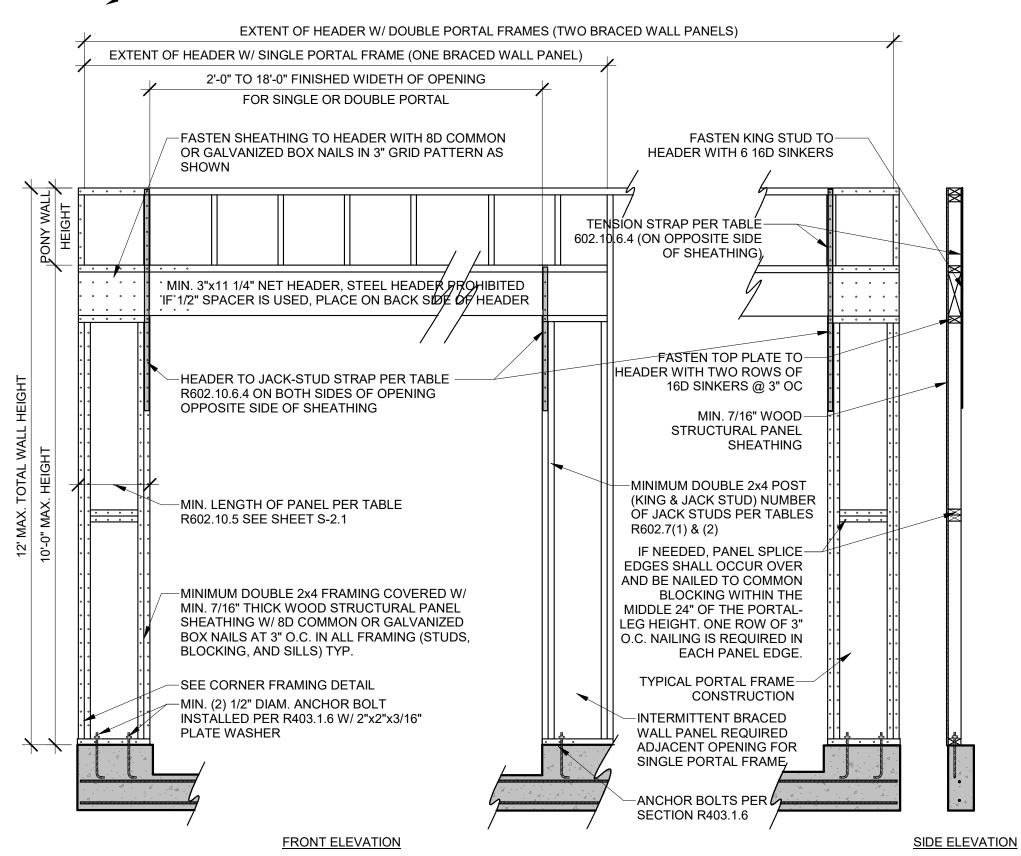
BRACED WALL PANEL CONNECTIONS



SIDE ELEVATION PFH PORTAL FRAME W/ HOLD DOWNS (R602.10.6.2)

1/2" = 1'-0"

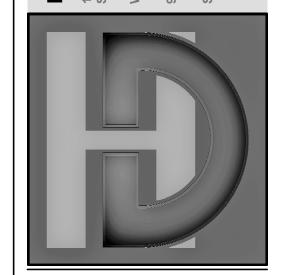
-FASTEN TOP PLATE TO HEADER WITH TWO ROWS OF 16D SINKERS @ 3" OC



PFG PORTAL FRAME W/OUT HOLD DOWNS (R602.10.6.3)

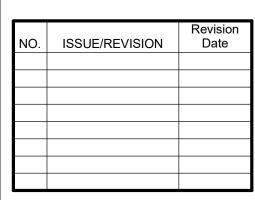
1/2" = 1'-0"

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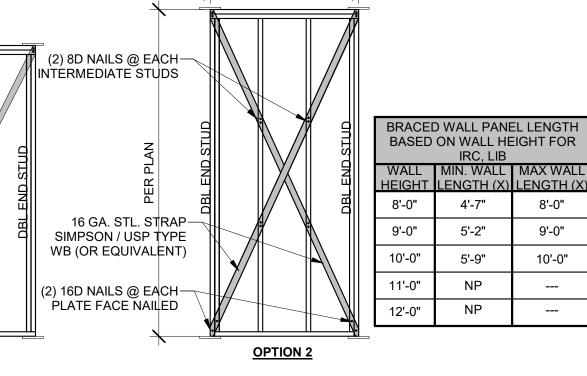


44350 05/28/2024 DATE: CHECKED BY: CLS



BRACED WALL NOTES & DETAILS

TENSION STRAP CAPACITY REQUIRED FOR RESISTING WIND PRESSURES PERPENDICULAR TO METHOD PFH, PFG AND CS-PF BRACED WALL PANELS IRC2018 TABLE R602.10.6.4



6 LIB BRACING 3/8" = 1'-0"

(2) 16 NAIL\$ @ EACH

PLATE FACE NAILED

(2)|BD NAILS @ EACH-

INTÉRMEDIAITE STUDS

FOR IRC CODE PRESCRIPTIVE METHOD

TABLE R602.10.5 MINIMUM LENGTH OF BRACED WALL PANELS

	METHOD			LENGTH	CONTRIBUTING LENGTH			
(SEE TABLE R602.10.4)			WALL HEIGHT				(INCHES)	
			9 FEET	10 FEET	11 FEET	12 FEET		
DWB	WSP,SFB,PBS,PCP,HPS,BV-WSP	48	48	48	53	58	ACTUAL ^b	
GB		48	48	48	53	58	DOUBLE SIDED = ACTUAL SINGLE SIDED=.5xACTUAL	
	LIB	55	62	69	NP	NP	ACTUAL ^b	
A DIA/	SDC A, B, AND C ULTIMATE DESIGN WIND SPEED<140	28	32	34	38	42	40	
ABW	SDC D ₀ ,D ₁ ,D ₂ ULTIMATE DESIGN WIND SPEED<140	32	32	34	NP	NP	48	
DELL	SUPPORTING ROOF ONLY	16	16	16	NOTE C	NOTE C	48	
PFH	SPTNG. ONE STORY & ROOF	24	24	24	NOTE C	NOTE C	48	
	PFG	24	27	30	NOTE D	NOTE D	1.5 x ACTUAL ^b	
	CS-G	24	27	30	33	36	ACTUAL ^b	
	CS-PF	16	18	20	NOTE E	NOTE E	ACTUAL ^b	
	ADJACENT CLEAR OPENING HEIGHT (INCHES)							
	≤64	24	27	30	33	36		
	68	26	27	30	33	36		
	72	27	27	30	33	36		
	76	30	29	30	33	36		
	80	32	30	30	33	36		
	84	35	32	32	33	36		
	88	38	35	33	33	36		
	92	43	37	35	35	36		
CS-WSP,	96	48	41	38	36	36	ACTUAL ^b	
CS-SFB	100	-	44	40	38	38	AOTOAL	
	104	-	49	43	40	39		
	108	-	54	46	43	41		
	112	-	-	50	45	43		
	116	-	-	55	48	45		
	120	-	-	60	52	48		
	124	-	-	-	56	51		
	128	-	-	-	61	54		
	132	-	-	-	66	58		
	136	-	-	-	-	62		
	140	-	-	-	-	66		
	144	-	-	-	-	72		

A. LINEAR INTERPOLATION SHALL BE PERMITTED
 b. USE THE ACTUAL LENGTH WHEN IT IS GREATER THAN OR EQUAL TO THE MINIMUM LENGTH
 MAX_HEADER HEIGHT FOR PEH IS 10' IN ACCORDANCE WITH R602 10.6.2. WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY

c. MAX. HEADER HEIGHT FOR PFH IS 10' IN ACCORDANCE WITH R602.10.6.2, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.
d. MAX. OPENING HEIGHT FOR PFG IS 10' IN ACCORDANCE WITH R602.10.6.3, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.
e. MAX. OPENING HEIGHT FOR CS-PF IS 10' IN ACCORDANCE WITH R602.10.6.4, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.

BRACED WALL PRESCRIPTIVE METHOD:

CONTINOUS EXTERIOR SHEATHING (CS-WSP) PER WSP METHOD (BELOW) UNLESS OTHERWISE NOTED ON THE PLAN

EXTERIOR BRACED WALL METHOD: (SEE ON THIS SHEET) · WSP METHOD:

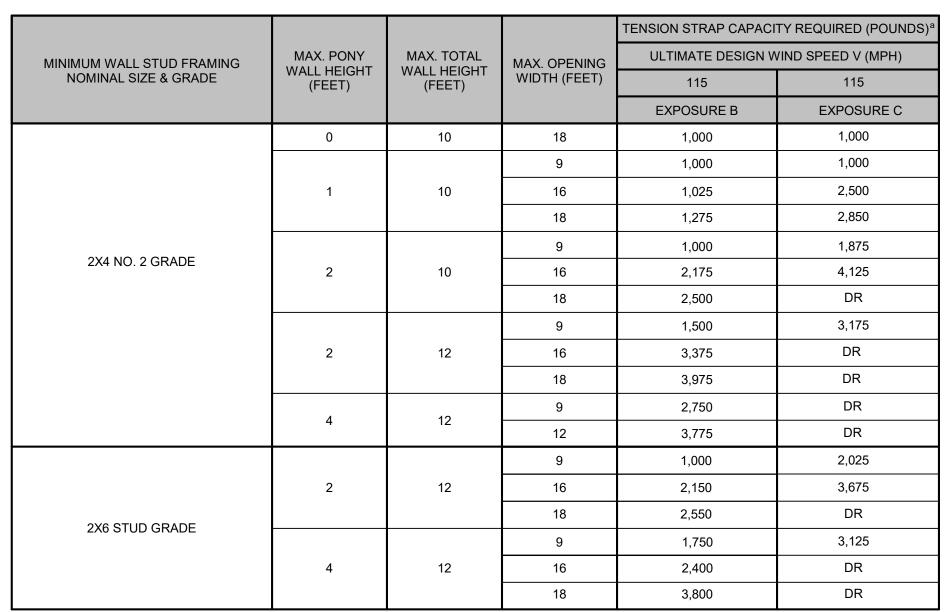
WOOD STRUCUTRAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 3/8" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" O.C. STUD SPACING WITH 6d NAILS COMMON NAILS @ 6" O.C. EDGES AND 12" O.C. FIELD OR SHEATHING THICKNESS NOT LESS THANK 7/16" WITH MINIMUM SPAN RATING OF 24/16 FOR 24" O.C. SPACING WITH 8d COMMON NAILS @ 6" O.C. EDGES AND 12" O.C. IN FIELD (NOTE: FRAMING MEMBERS 16" O.C. MAX, UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS)

INTERIOR BRACED WALLS (SEE ON THIS SHEET)

1/2" MINIMUM GYPSUM BOARD OVER STUDS SPACED @ 24" MAXIMUM FASTENED W/ #6- 1 1/4" TYPE "W" OR "S" DRYWALL SCREWS @ 7" O.C. EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES)

OR LIB METHOD:

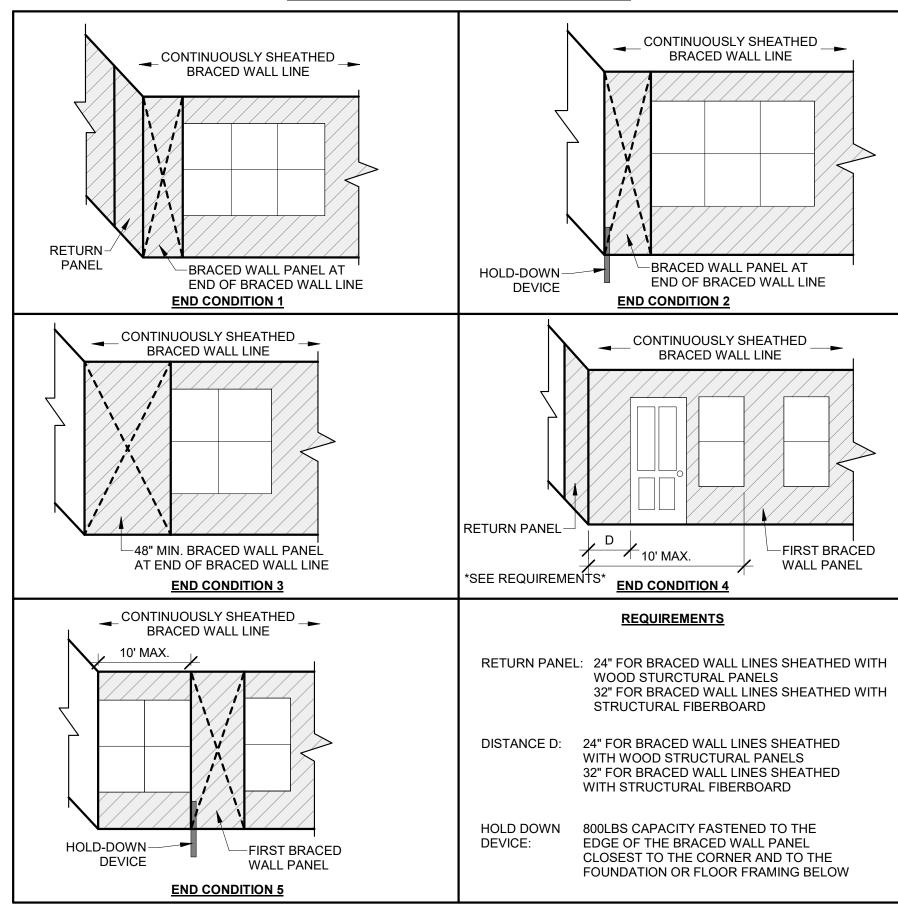
1X4 WOOD FASTENED W/ (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUIVALENT) STL. X-BRACE(S) @ 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUDS FASTENED PER MANUF. SPECS.

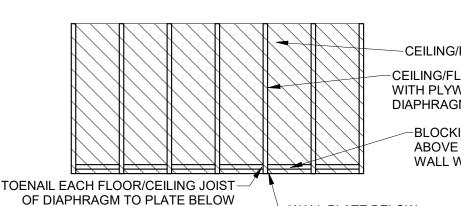


a. DR = DESIGN REQUIREDb. STRAP SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

END WALL CONDITIONS

FOR CONTINUOUSLY SHEATHED BRACED WALL LINES





WITH MIN. (3) 8d NAILS OR (2) 18d NAILS

CEILING/FLOOR DIAPHRAGM PER PLAN
CEILING/FLOOR JOISTS @ 16" OC
WITH PLYWOOD OR GYPSUM
DIAPHRAGM ATTACHED PER PLAN

BLOCKING BETWEEN JOISTS ABOVE WALL, TOENAILED TO WALL W/ (3) 8d NAILS

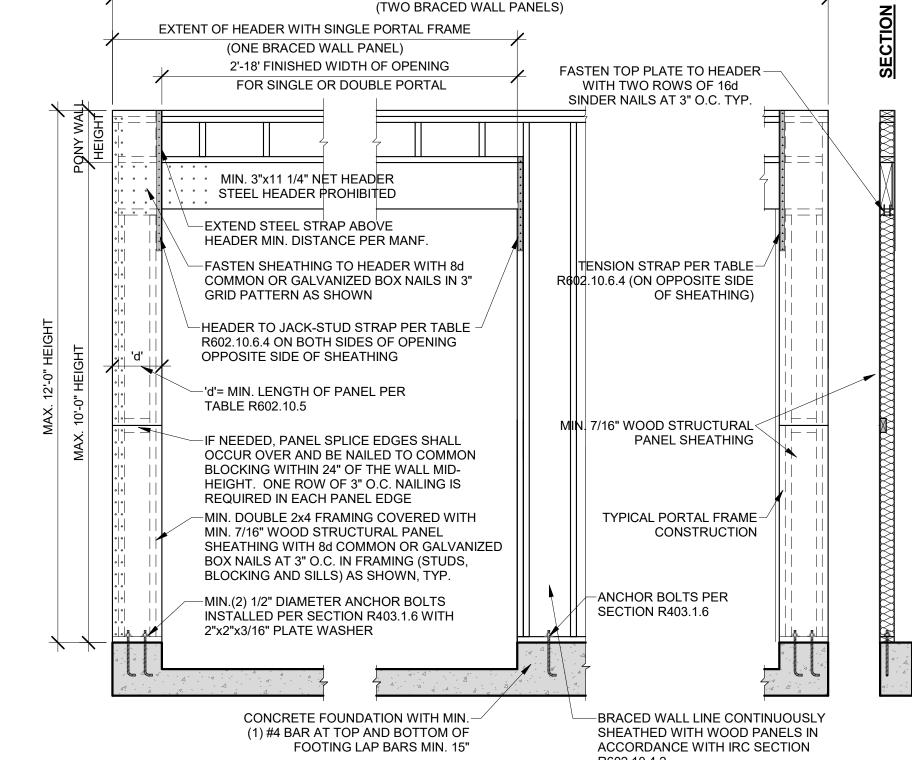
WALL PLATE BELOW FLOOR/CEILING JOISTS

DIAPHRAGM CONNECTION TO INTERIOR WALL

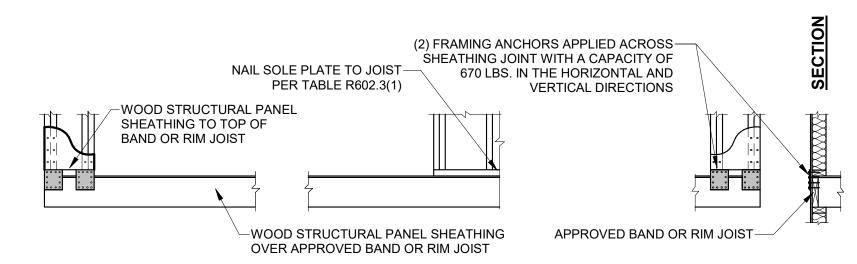
T/2" GYPSUM BOARD W/ NO6 - 1 1/4" TYPE "W" OR "S" SCREWS @ 7" O.C. MIN. 4"-0" GYP BOARD BOTH SIDES

FRONT ELEVATION

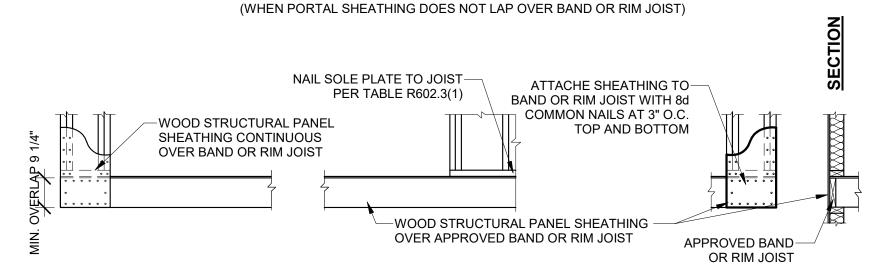
EXTENT OF HEADER WITH DOUBLE PORTAL FRAMES



OVER CONCRETE OR MASONRY BLOCK FOUNDATION

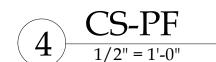


OVER RAISED WOOD FLOOR - FRAMING ANCHOR OPTION



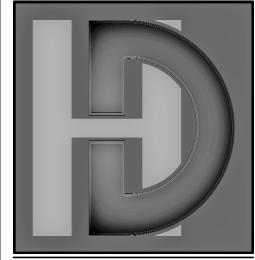
OVER RAISED WOOD FLOOR - OVERLAP OPTION

(WHEN PORTAL SHEATHING LAPS OVER BAND OR RIM JOIST)



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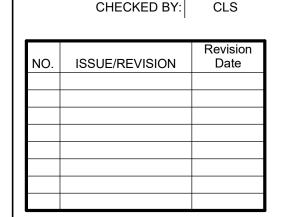
DERS FARMS SUMMIT, MO

OT 81 - SUMMIT VIEW FARM W SERENA PL., LEE'S SUMN

HD#: 44350

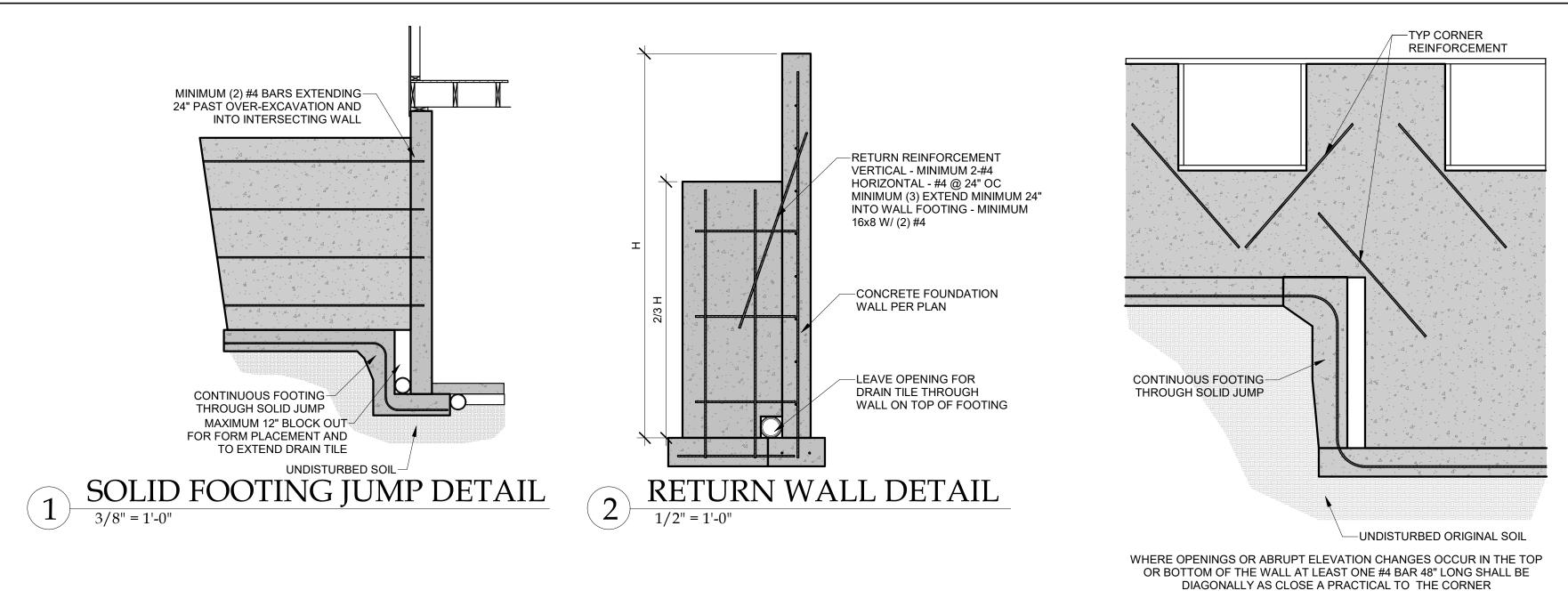
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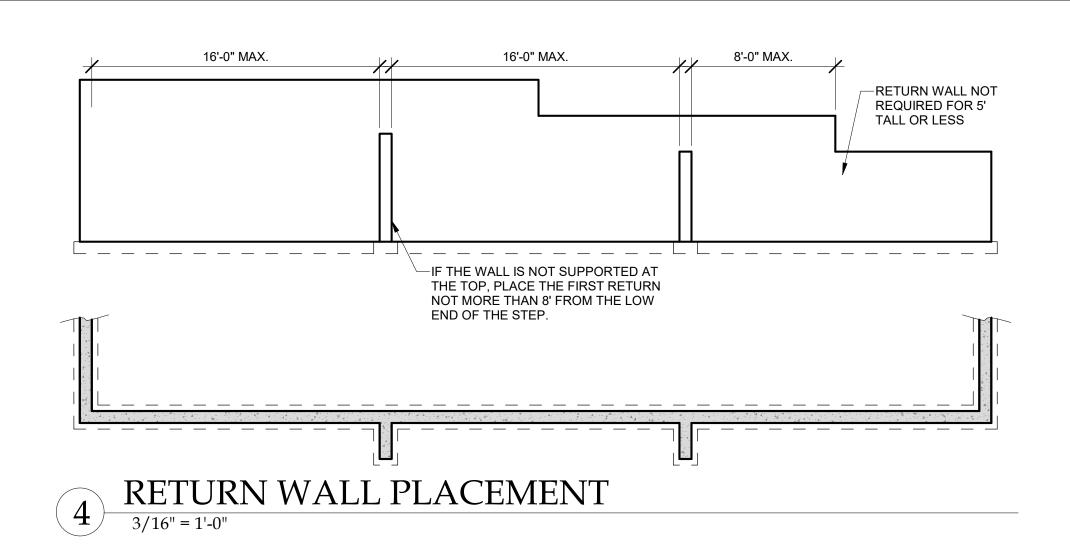
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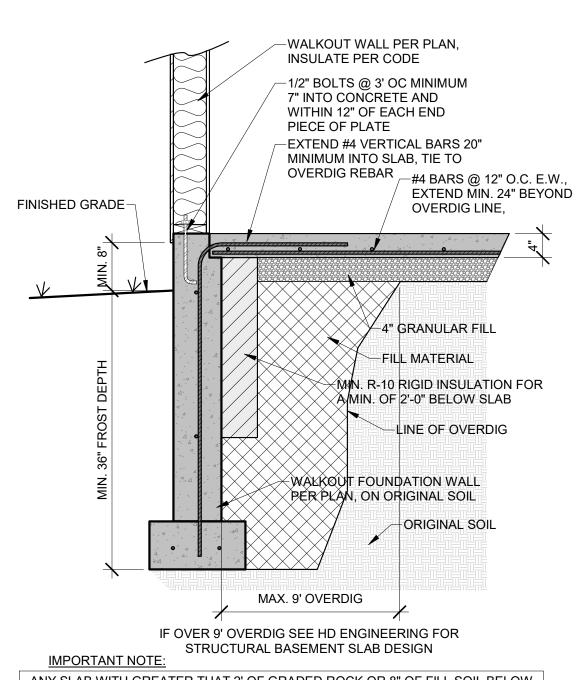
BRACED WALLS NOTES & DETAILS

S-2.1



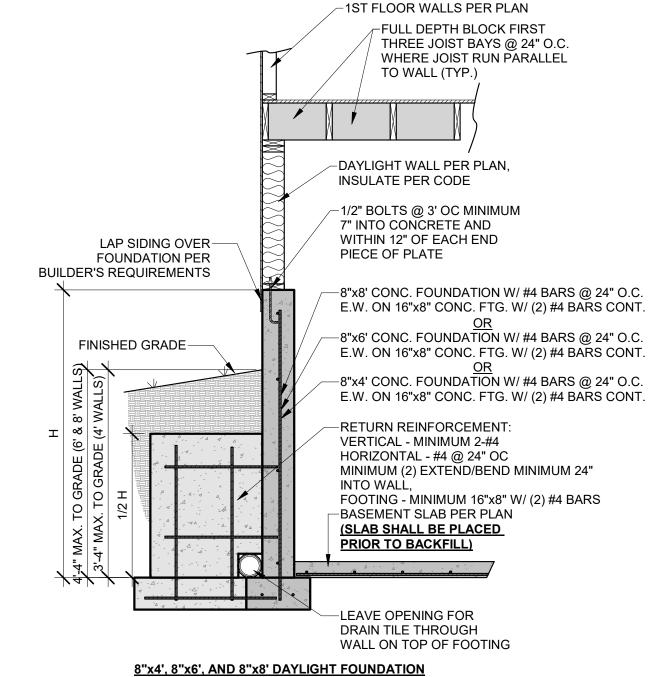


REINFORCEMENT AT CORNERS AND STEPS



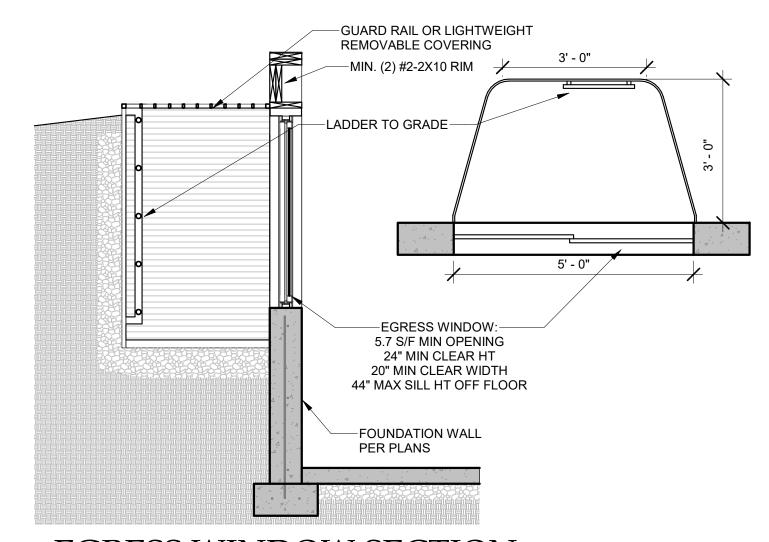
ANY SLAB WITH GREATER THAT 2' OF GRADED ROCK OR 8" OF FILL SOIL BELOW SHALL BE DESIGNED AS STRUCTURAL PER PLAN. OUR FIRM SHOULD BE CONTACTED IMMEDIATELY FOR DESIGN RECOMMENDATIONS. DESIGN MUST BE COMPLETED PRIOR TO PLACEMENT OF PIERS OR FOOTINGS.

WALKOUT DETAIL
3/4" = 1'-0"



IF SLAB IS NOT PLACED PRIOR TO BACKFILL CONTRACTOR IS RESPONSIBLE FOR BRACING THE FOUNDATION AS REQUIRED

UNRESTRAINED FOUNDATION WALL



EGRESS WINDOW SECTION

1/2" = 1'-0"

3000 PSI/ 40 KSI 3500 PSI/ 40 KSI 3000 PSI/ 60 KSI	8' 16 16 24	9' 12 12	8' 24 24	9' 16 24	10
3500 PSI/ 40 KSI 3000 PSI/ 60 KSI	16				-
3000 PSI/ 60 KSI		12	24	24	4.0
	24				12
0500 001/001/01		16	24	20	16
3500 PSI/ 60 KSI	24	16	24	24	16
HORIZONTAL REINFORCEMENT**					_

* CONCRETE SHALL HAVE AIR ENTRAINMENT OF 5-7%.

* MINIMUM REQUIREMENT FOR VERTICAL REBAR IN PLAIN CONCRETE WALLS IS #4 @ 36" ON CENTER (ACI 332). * VERTICÀL BARS SHALL BE CONTINUED UP TO WITHIN 8" OF THE TOP OF THE WALL.

* REBAR SHALL BE POSITIONED AT THE TENSION FACE OF THE WALL (2" FROM THE INSIDE * REINFORCEMENT SHALL LAP A MINIMUM OF 24 INCHES AT ENDS, SPLICES, AND AROUND

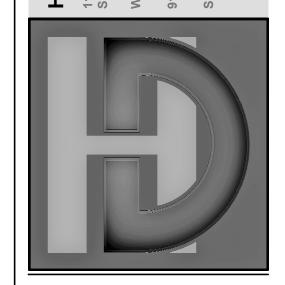
CORNERS. ** #4 BARS @ 24" ON CENTER.

** #4 BAR WITHIN 12 OF TOP AND BOTTOM OF WALL. ** MINIMUM GRADE 40 (40ksi) STEEL (PER ACI 332).

** HORIZONTAL REINFORCEMENT SHALL BE INSTALLED ON THE COMPRESSION SIDE (SOIL

SIDE) OF THE VERTICAL REINFORCEMENT

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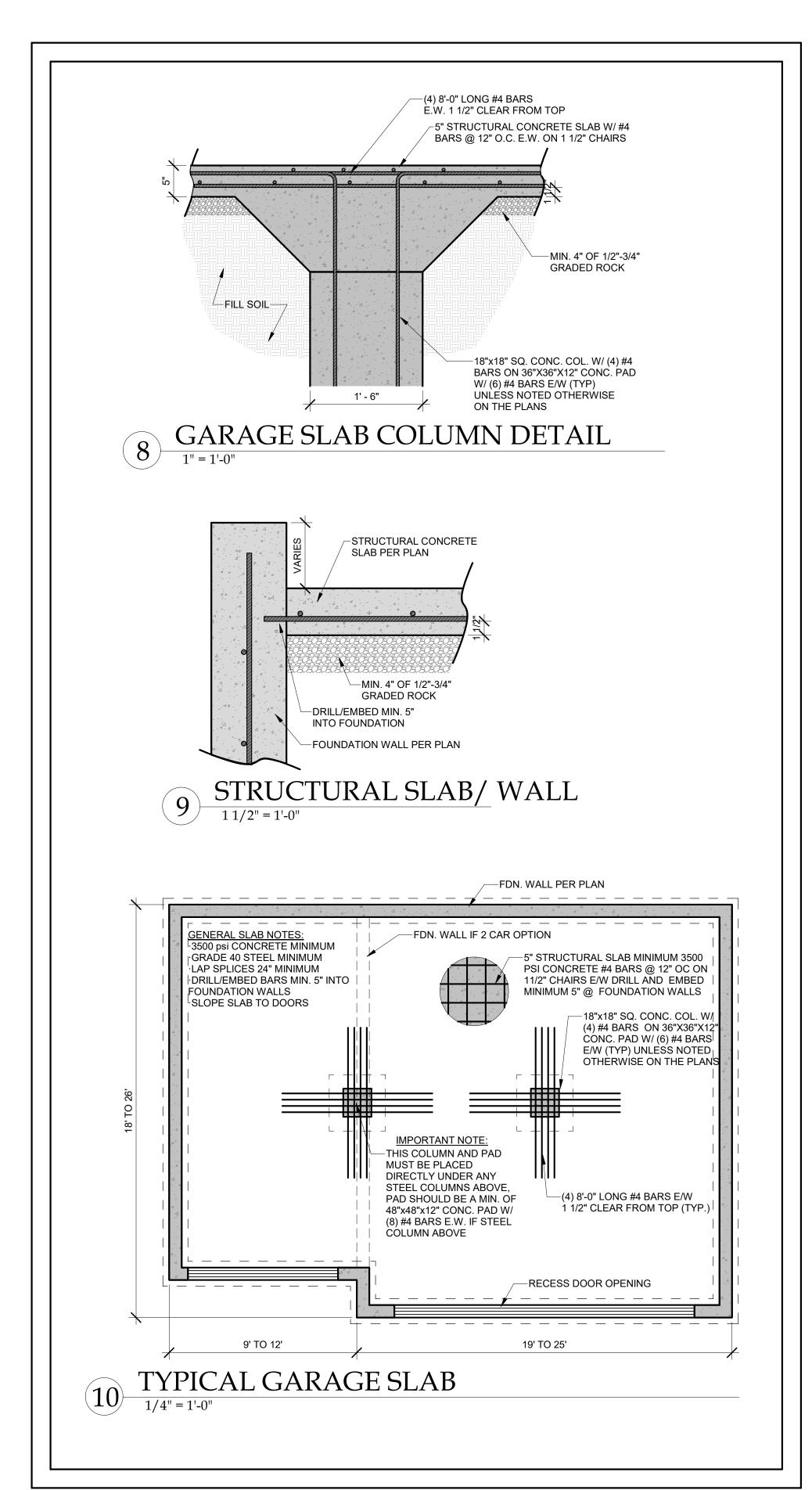
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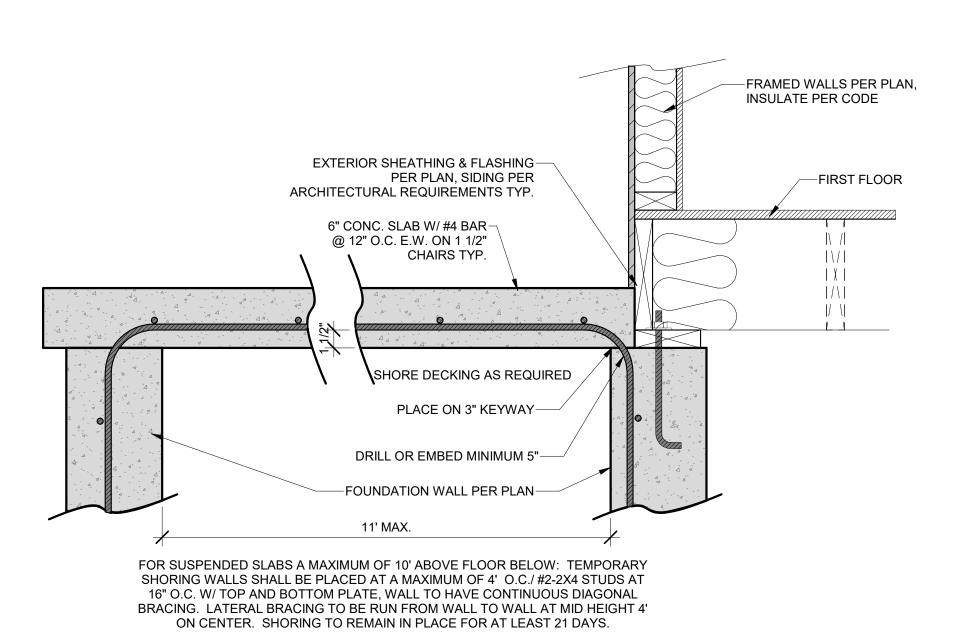
44350 05/28/2024 DATE:

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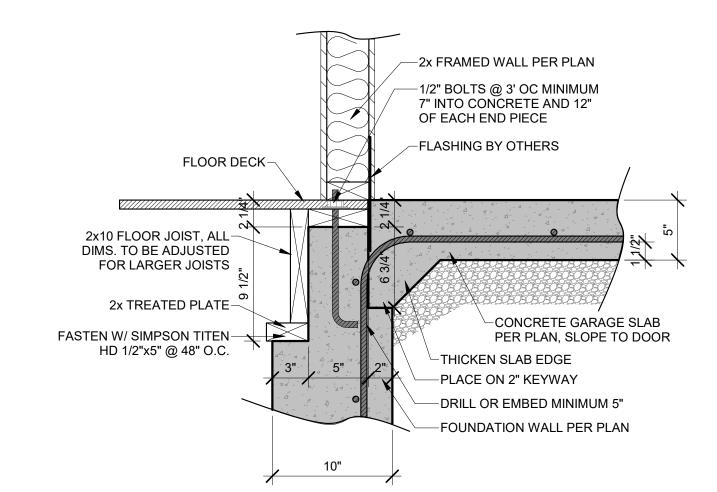
	ı	
NO.	ISSUE/REVISION	Revision Date

CONCRETE DETAILS



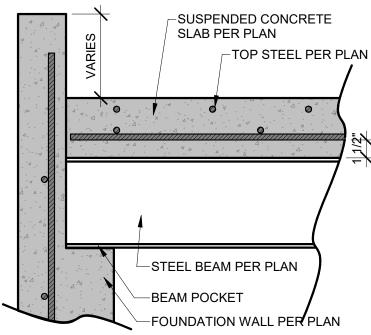


SUSPENDED PORCH STOOP SLAB



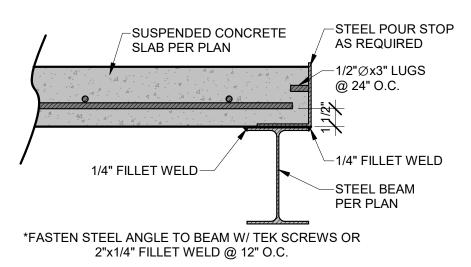
ZERO ENTRY GARAGE DETAIL

1 1/2" = 1'-0"



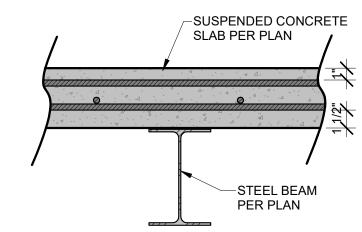
SUSPENDED SLAB BEAM/WALL CONNECTION

1 1/2" = 1'-0"



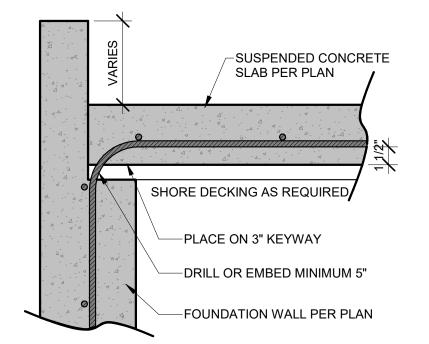
SUSPENDED SLAB POUR STOP

1 1/2" = 1'-0"

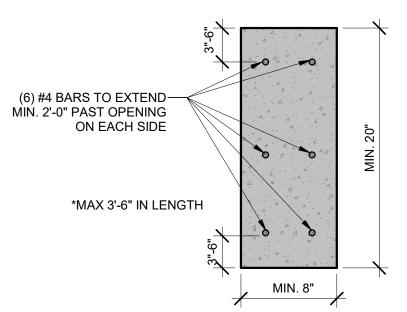


SUSPENDED SLAB/STEELBEAM CROSS SECTION

1 1/2" = 1'-0"



SUSPENDED SLAB/WALL CONNECTION

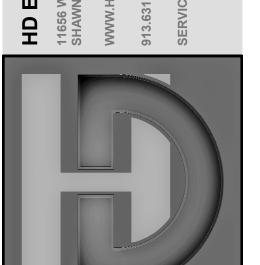


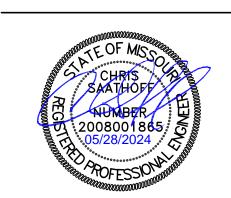
CONCRETE HEADER DETAIL

IMPORTANT NOTE:

FOR SUSPENDED SLABS A MAXIMUM OF 10' ABOVE FLOOR BELOW: TEMPORARY SHORING WALLS SHALL BE PLACED AT A MAXIMUM OF 4'
O.C. / #2-2X4 STUDS AT 16" O.C. W/ TOP AND BOTTOM PLATE, WALL TO HAVE CONTINUOUS DIAGONAL BRACING. LATERAL BRACING TO BE
RUN FROM WALL TO WALL AT MID HEIGHT 4' ON CENTER. SHORING TO REMAIN IN PLACE FOR AT LEAST 21 DAYS.
-ANY CAST IN PLACE SLABS FORMED MORE THAN 10' ABOVE THE FLOOR BELOW SHALL HAVE A SITE SPECIFIC SHORING DESIGN DONE. OUR
FIRM SHOULD BE CONSULTED FOR THIS DESIGN ONCE FOUNDATION WALLS ARE IN PLACE TO EVALUATE ALL FIELD CONDITIONS. IT SHOULD

STREET





SERS ARMS UMMIT, MO

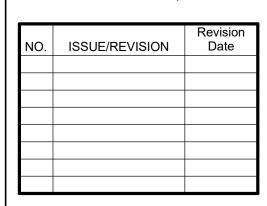
SERENA PL., LEE'S SUM

2315 SW SERE

HD#: 44350

DATE: 05/28/2024

CHECKED BY: CLS



SUSPENDED SLAB DETAILS

S-3.1

ENGINEERING & DESIGN

RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
06/25/2024 8:29:50

HD ENGINEERING STRUCTURAL

CO.C./#2-2X4 STUDES AT 16" O.C. W/ TOP IN PLACE SLABS FORMED MALL AT MID HEIGHT STORY OF THE GRANGE SLAB DETAILS

GARAGE SLAB DETAILS

MPORTANT NOTE:

1-FOR SUSPENDED SLABS A MAXIMUM OF THE CONTROL OF THE

MINIMUM INSULATION & FENSTRATION VALUES BY COMPONENT, PER IRC2018 N1102.1.2

CLIMATE ZONE	FENSTRATION U-FACTOR	SKYLIGHT U-FACTOR	GLAZED SHGC FENSTRATION	HNSULATED WETAL	INSULATED WOOD DOOR U-VALUE	CEILING R-VALUE	WOOD FRAMED WALL R-VALUE	FLOOR R-VALUE	BASEMENT WALL R-VALUE	SLAB R-VALUE & DEPTH		DUCTWORK OVER OUTSIDE R-VALUE	
4 EXCEPT MARINE	0.32	0.55	0.40	0.60	0.50	49	20 OR 13 CAV. +5	19	10 CONTINUOUS OR 13 CAVITY	R-10, 2 FT.	10 CONTINUOUS OR 13 CAVITY	8	6

NOTES: 1) BUILDING THERMAL ENVELOPE IS REQUIRED TO BE SEALED WITH AN AIR BARRIER AS PER N1102.4.1 OF THE 2018 IRC 2) RECESSED LIGHTING SHALL BE SEALED TO PREVENT LEAKAGE BETWEEN THE CONDITIONED SPACE AND UNCONDITIONED SPACE

VALUES BELOW ARE PER 2018 IECC. ACTUAL VALUES MAY VARY BASED ON ALTERNATE ENERGY COMPLIANCE PATH CHOSEN (IN JURISDITIONS WHERE ALTERNATIVE PATHS ARE AVAILABLE)

3) ALL DUCTS, AIR HANDLERS, FILTER BOXES, AND BUILDING CAVITIES USED AS DUCTS SHALL BE SEALED AS PER N1103.2 OF THE 2018 IRC

FAN LOCATION

HRV OR ERV

RANGE HOODS

IN-LINE FAN

BATHROOM, UTILITY ROOM

BATHROOM, UTILITY ROOI

CATHEDRAL / VAULTED CEILING FRAMING AND INSULATION

MINIMUM R-38 INSULATION REQUIRED, SEE DETAIL 14/S-1.2

WHERE THE CEILING IS APPLIED DIRECTLY TO THE BOTTOM OF THE RAFTERS, A MINIMUM 1" AIR SPACE SHALL BE PROVIDED BETWEEN THE TOP OF THE INSULATION AND THE SHEATHING FOR VENTILATION (R806.3) NOTE: RAFTER SIZES SPECIFIED ON PLANS ARE THE MINIMUM REQUIRED FOR STRUCTURAL PURPOSES ONLY.

TABLE N1103.6.1 (R403.6.1) WHOLE-HOUSE

MECHANICAL VENTILATION SYSTEM FAN EFFICACY

MINIMUM EFFICACY

(CFM/WATT)

1.2 CFM/WATT

2.8 CFM/WATT

2.8 CFM/WATT

1.4 CFM/WATT

2.8 CFM/WATT

AIR FLOW RATE

ANY

ANY

ANY

< 90

ANY

MAXIMUM (CFM)

IF FULL RAFTER DEPTH IS NOT ADEQUATE FOR MINIMUM INSULATION VALUE, RAFTER SIZES WILL NEED TO BE INCREASED. OR ADEQUATE FURRING SHALL BE USED TO OBTAIN THE MINIMUM JOIST DEPTH FOR THE REQUIRED INSULATION. IN ADDITION, IF THE RAFTER SIZE IS INCREASED IT SHALL BE VERIFIED THAT THE RIDGE BE A MINIMUM OF ONE NOMINAL SIZE LARGER THAN THE RAFTERS BEING RECEIVED. (SEE CHART BELOW)

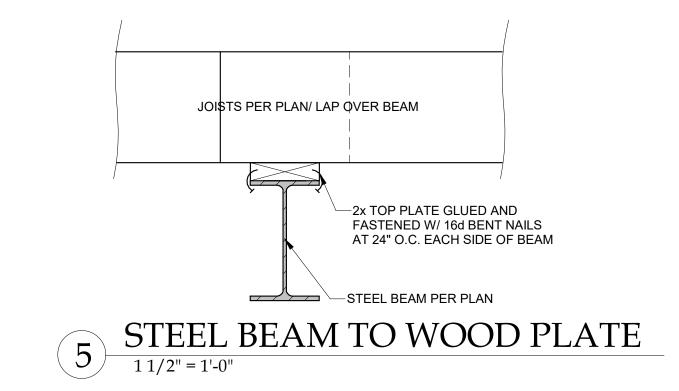
MAXIMUM INSULATION VALUE	2x6	2x8	2x10	2x12	
1" AIR SPACE (FIBERGLASS)	R-13, 3 1/2"	R-19, 6 1/4"	CONDENSED R-38, 8 1/4"	R-38, 10 1/4"	

AIR FLOW RATE

90

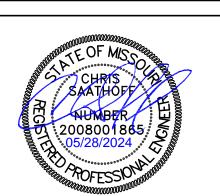
WHEN TESTED IN ACCORDANCE WITH HVI STANDARD 916

-JOIST SECTION TO BE REMOVED FOR SHOWER PAN -APA SHEATHING FILLER -JOIST SECTION TO BE REMOVED -1.75"x9.25" LVL (11-7/8" I-JOIST FLOOR) FOR SHOWER PAN 1.75"x7.25" LVL (9-1/2" I-JOIST FLOOR) -1.75"x9.25" LVL (2X12 FLOOR) * SISTER TO RUN FULL 1.75"x7.25" LVL (2X10 FLOOR) LENGTH OF FLOOR JOIST TO BE ALTERED SISTER TO RUN FULL LENGTH OF FLOOR JOIST TO BE ALTERED -12D 16" O.C FROM THIS SIDE -12D 16" O.C FROM THIS SIDE -JOIST PER PLAN ZERO ENTRY SHOWER DETAIL



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-(3) 8d TOE NAILS

FLUSH WOOD BEAM CONNECTION

-SIMPSON A34 FRAMING ANGLE

EXTERIOR WALL STEEL BEAM BEARING

ON EACH SIDE OF BEAM

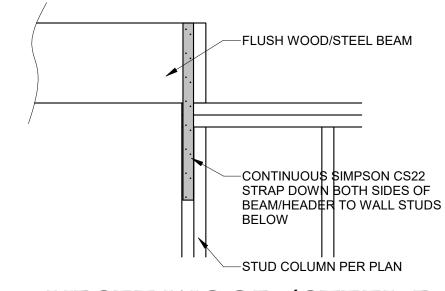
-STEEL BEAM

TOP PLATE

FLUSH WOOD BEAM

-JOISTS PER PLAN

WOOD TO WOOD STACKED CONNECTION



-FLUSH BEAM

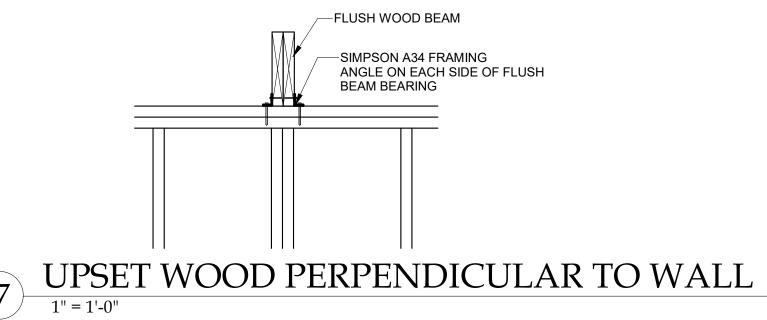
DOWN WOOD BEAM

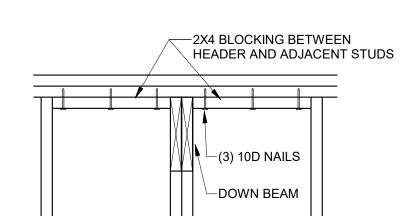
BEAM BEARING

-SIMPSON A34 FRAMING

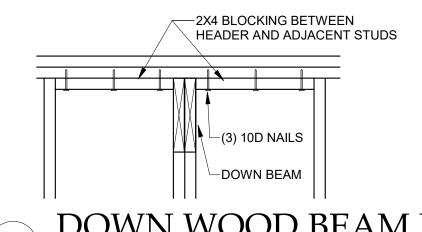
ANGLE ON EACH SIDE OF FLUSH

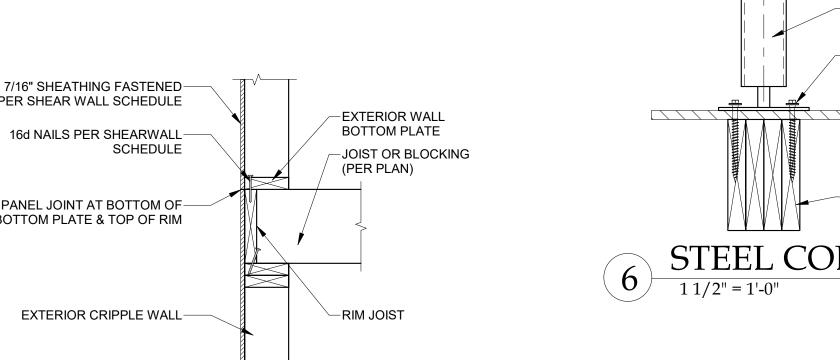


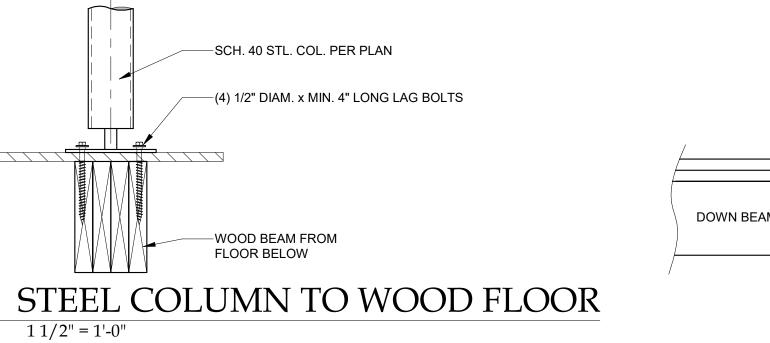


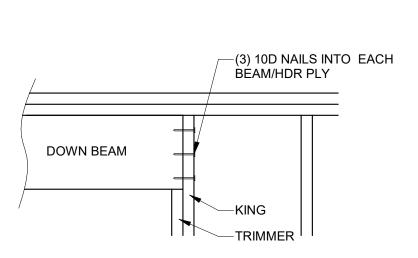


DOWN WOOD BEAM PERPENDICULAR









DOWN WOOD BEAM PARALLEL

1" = 1'-0"

S-4.0

44350

CHECKED BY: CLS

NO. ISSUE/REVISION

05/28/2024

SHEATHING JOINT LOCATION

1" = 1'-0"

RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 06/25/2024 8:29:50

PER SHEAR WALL SCHEDULE 16d NAILS PER SHEARWALL PANEL JOINT AT BOTTOM OF-**BOTTOM PLATE & TOP OF RIM** EXTERIOR CRIPPLE WALL-

GENERAL DETAILS