

RE: P240539-01
Roof - Osage Lot 79

MiTek, Inc.
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200

Site Information:

Customer: Clayton Properties Project Name: P240539-01
Lot/Block: 79 Model:
Address: 3716/3718 SW Knoxville Ct OS 1179
City: Lee's Summit Subdivision: Osage
State: MO

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014
Wind Code: ASCE 7-16
Roof Load: 45.0 psf

Design Program: MiTek 20/20 8.6
Wind Speed: 115 mph
Floor Load: N/A psf

This package includes 17 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	I61197917	A1	10/6/2023
2	I61197918	A2	10/6/2023
3	I61197919	A3	10/6/2023
4	I61197920	A4	10/6/2023
5	I61197921	B1	10/6/2023
6	I61197922	B2	10/6/2023
7	I61197923	C1	10/6/2023
8	I61197924	C2	10/6/2023
9	I61197925	C3	10/6/2023
10	I61197926	C4	10/6/2023
11	I61197927	E1	10/6/2023
12	I61197928	E2	10/6/2023
13	I61197929	G1A	10/6/2023
14	I61197930	G2A	10/6/2023
15	I61197931	V5	10/6/2023
16	I61197932	V6	10/6/2023
17	I61197933	V7	10/6/2023

The truss drawing(s) referenced above have been prepared by
MiTek USA, Inc. under my direct supervision
based on the parameters provided by .

Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of Missouri is December 31, 2025.

Missouri COA: 001193

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek. Any project specific information included is for MiTek customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



October 06, 2023

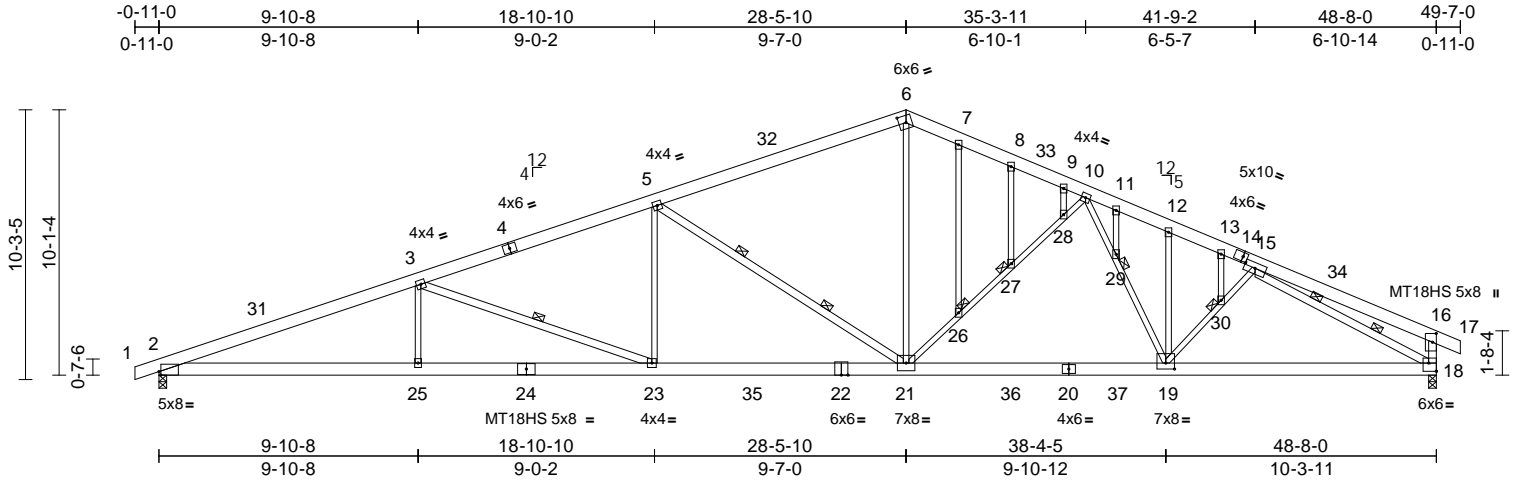
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197917
P240539-01	A1	Roof Special Structural Gable	1	1	Job Reference (optional)	

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Scale = 1:87.8

Plate Offsets (X, Y): [2:0-0-14,Edge], [6:0-3-5,0-3-4], [14:0-2-2,0-2-0], [16:0-4-0,0-1-12], [18:Edge,0-3-12], [19:0-4-0,0-2-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.60	Vert(LL)	-0.45	19-21	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.50	Vert(CT)	-0.73	19-21	>794	180	MT18HS	197/144
BCLL	0.0*	Rep Stress Incr	YES	WB	0.81	Horz(CT)	0.15	18	n/a	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S								
											Weight: 319 lb	FT = 20%

LUMBER			WEBS			6-21=-276/1706, 3-25=0/424, 5-21=-1669/450, 3-23=-1305/354, 5-23=-18/828, 19-30=0/338, 15-30=0/330, 21-26=-705/282, 26-27=-693/274, 27-28=-679/268, 10-28=-791/316, 15-18=-3502/610, 10-29=-25/454, 19-29=-26/423, 7-26=-20/15, 8-27=-25/12, 9-28=-65/155, 11-29=-34/0, 12-19=-107/61, 13-30=0/10			10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 18 and 449 lb uplift at joint 2.		
TOP CHORD	2x6 SP 2400F 2.0E *Except*	6-14,14-17:2x6 SPF No.2							11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.		
BOT CHORD	2x6 SP 2400F 2.0E								LOAD CASE(S) Standard		
WEBS	2x3 SPF No.2 *Except*	18-16,21-5,23-3,18-15:2x4 SP No.2									
OTHERS	2x3 SPF No.2										
BRACING											
TOP CHORD	Structural wood sheathing directly applied or 3-7-12 oc purlins, except end verticals.										
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.										
WEBS	1 Row at midpt	3-23									
WEBS	2 Rows at 1/3 pts	5-21, 15-18									
JOINTS	1 Brace at Jt(s):	26, 27, 29, 30									
REACTIONS (size)											
	Max Horiz	2=184 (LC 12)									
	Max Uplift	2=-449 (LC 8), 18=-328 (LC 13)									
	Max Grav	2=2348 (LC 2), 18=2366 (LC 2)									
FORCES (lb) - Maximum Compression/Maximum Tension											
TOP CHORD	1-2=0/11, 2-3=-5861/1164, 3-5=-4575/983, 5-6=-3139/777, 6-7=-3091/796, 7-8=-3130/785, 8-9=-3164/773, 9-10=-3239/782, 10-11=-3748/840, 11-12=-3780/835, 12-13=-3795/802, 13-15=-3833/793, 15-16=-460/213, 16-17=0/29, 16-18=-480/303										
BOT CHORD	2-25=-1036/5472, 23-25=-1036/5472, 21-23=-755/4260, 19-21=-563/3330, 18-19=-624/3316										

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 28-5-10, Exterior(2R) 28-5-10 to 33-5-10, Interior (1) 33-5-10 to 49-7-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) All plates are 3x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) All bearings are assumed to be SP 2400F 2.0E crushing capacity of 805 psi.



October 6,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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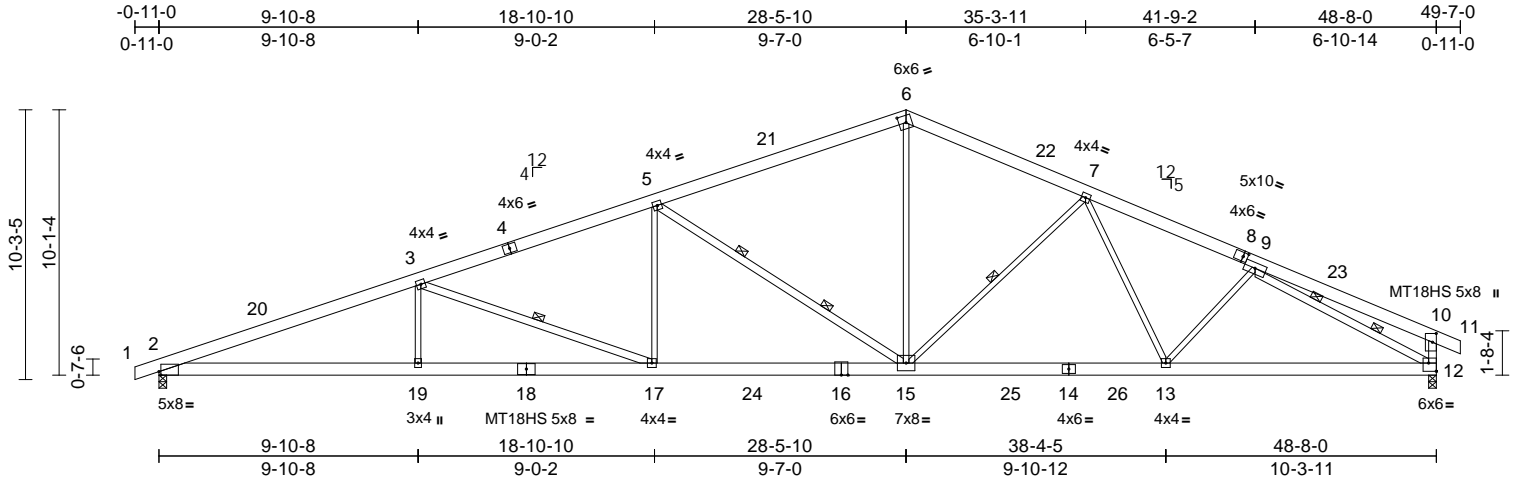
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197918
P240539-01	A2	Roof Special	2	1	Job Reference (optional)	

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Scale = 1:87.8

Plate Offsets (X, Y): [2:0-0-14,Edge], [6:0-3-5,0-3-4], [8:0-2-2,0-2-0], [10:0-4-0,0-1-12], [12:Edge,0-3-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.59	Vert(LL)	-0.43	13-15	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.49	Vert(CT)	-0.70	17-19	>833	180	MT18HS	197/144
BCLL	0.0*	Rep Stress Incr	YES	WB	0.81	Horz(CT)	0.15	12	n/a	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S								
											Weight: 303 lb	FT = 20%

LUMBER

TOP CHORD 2x6 SP 2400F 2.0E *Except* 8-11,8-6:2x6 SPF No.2
 BOT CHORD 2x6 SP 2400F 2.0E *Except* 0-0:2x6 SPF No.2
 WEBS 2x3 SPF No.2 *Except* 12-10,15-5,17-3,12-9:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-6-2 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 3-17, 7-15
 WEBS 2 Rows at 1/3 pts 5-15, 9-12

REACTIONS

(size) 2=0-3-8, 12=0-3-8
 Max Horiz 2=184 (LC 16)
 Max Uplift 2=449 (LC 8), 12=328 (LC 13)
 Max Grav 2=2348 (LC 2), 12=2366 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/11, 2-3=-5857/1163, 3-5=-4577/984, 5-6=-3135/775, 6-7=-3198/796, 7-9=-3818/813, 9-10=-456/207, 10-11=0/29, 10-12=-475/299
 BOT CHORD 2-19=-1035/5468, 17-19=-1035/5468, 15-17=-756/4263, 13-15=-568/3334, 12-13=-630/3313
 WEBS 3-19=0/422, 6-15=-283/1724, 9-13=0/317, 5-17=-17/827, 7-13=0/390, 5-15=-1677/454, 3-17=-1299/352, 7-15=-712/287, 9-12=-3497/622

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 28-5-10, Exterior(2R) 28-5-10 to 33-5-10, Interior (1) 33-5-10 to 49-7-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP 2400F 2.0E crushing capacity of 805 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 449 lb uplift at joint 2 and 328 lb uplift at joint 12.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



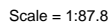
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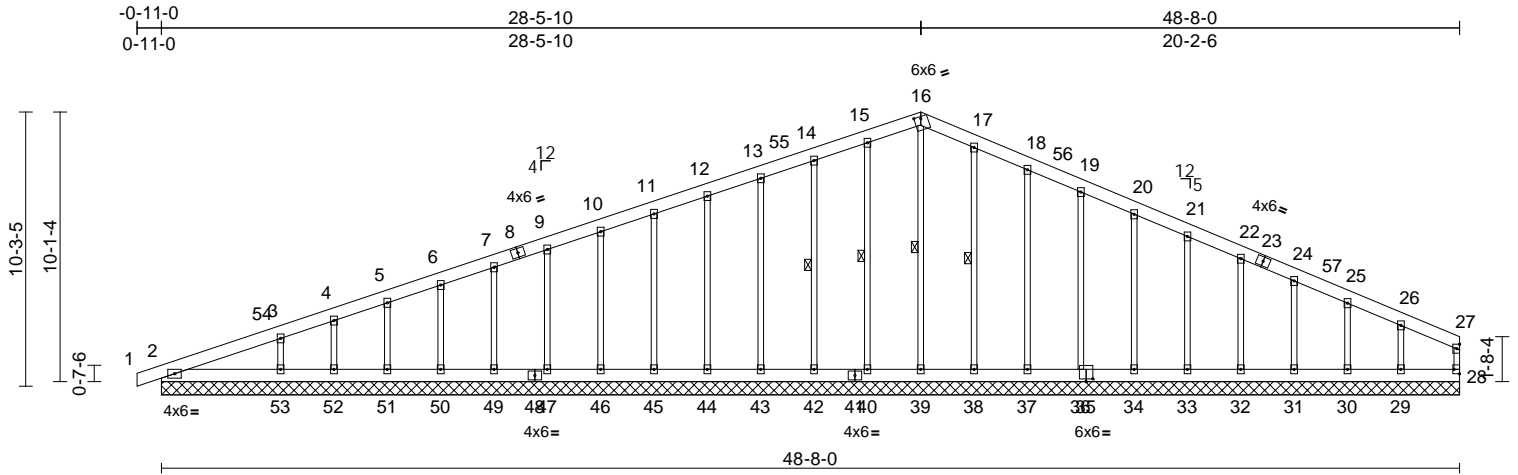
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79
P240539-01	A4	Roof Special Supported Gable	1	1	Job Reference (optional)
					I61197920

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Scale = 1:86.4

Plate Offsets (X, Y): [16:0-3-0,0-0-15], [28:Edge,0-2-8], [36:0-3-0,0-1-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	n/a	-	n/a	999	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	n/a	-	n/a	999	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.21	Horz(CT)	0.00	28	n/a	n/a	
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S							
										Weight: 275 lb	FT = 20%

LUMBER

TOP CHORD	2x6 SPF No.2
BOT CHORD	2x6 SPF No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt

REACTIONS (size)	2=48-8-0, 28=48-8-0, 29=48-8-0, 30=48-8-0, 31=48-8-0, 32=48-8-0, 33=48-8-0, 34=48-8-0, 36=48-8-0, 37=48-8-0, 38=48-8-0, 39=48-8-0, 40=48-8-0, 42=48-8-0, 43=48-8-0, 44=48-8-0, 45=48-8-0, 46=48-8-0, 47=48-8-0, 49=48-8-0, 50=48-8-0, 51=48-8-0, 52=48-8-0, 53=48-8-0
	2=192 (LC 12)
Max Horiz	2=44 (LC 13), 29=94 (LC 13), 30=43 (LC 13), 31=58 (LC 13), 32=59 (LC 13), 33=51 (LC 13), 34=53 (LC 13), 36=58 (LC 13), 37=64 (LC 13), 38=20 (LC 13), 40=26 (LC 12), 42=57 (LC 8), 43=50 (LC 8), 44=49 (LC 12), 45=49 (LC 12), 46=46 (LC 8), 47=51 (LC 8), 49=54 (LC 12), 50=45 (LC 8), 51=52 (LC 12), 52=30 (LC 8), 53=111 (LC 12)

FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/11, 2-3=-194/131, 3-4=-144/145, 4-5=-124/159, 5-6=-102/176, 6-7=-81/193, 7-9=-84/211, 9-10=-96/229, 10-11=-107/246, 11-12=-119/263, 12-13=-131/287, 13-14=-143/317, 14-15=-156/348, 15-16=-161/365, 16-17=-165/367, 17-18=-157/334, 18-19=-140/285, 19-20=-125/245, 20-21=-109/209, 21-22=-96/173, 22-24=-79/134, 24-25=-63/96, 25-26=-50/62, 26-27=-55/34, 27-28=-66/22
BOT CHORD	2-53=-35/58, 52-53=-35/58, 51-52=-35/58, 50-51=-35/58, 49-50=-35/58, 47-49=-35/58, 46-47=-35/58, 45-46=-35/58, 44-45=-35/58, 43-44=-35/58, 42-43=-35/58, 40-42=-35/58, 39-40=-35/58, 38-39=-35/58, 37-38=-35/58, 36-37=-35/58, 34-36=-35/58, 33-34=-35/58, 32-33=-35/58, 31-32=-35/58, 30-31=-35/58, 29-30=-35/58, 28-29=-35/58

WEBS

16-39=-164/19, 15-40=-144/56, 14-42=-143/97, 13-43=-140/74, 12-44=-140/73, 11-45=-140/73, 10-46=-134/70, 9-47=-144/75, 7-49=-149/78, 6-50=-131/69, 5-51=-149/77, 4-52=-84/47, 3-53=-286/172, 17-38=-144/59, 18-37=-143/105, 19-36=-140/80, 20-34=-140/78, 21-33=-133/74, 22-32=-149/84, 24-31=-143/80, 25-30=-132/93, 26-29=-158/135
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NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TC DL=6.0psf; BC DL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 28-5-10, Exterior(2R) 28-5-10 to 33-5-10, Interior (1) 33-5-10 to 48-6-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



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Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79
P240539-01	A4	Roof Special Supported Gable	1	1	I61197920
					Job Reference (optional)

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 3x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 2, 26 lb uplift at joint 40, 57 lb uplift at joint 42, 50 lb uplift at joint 43, 49 lb uplift at joint 44, 49 lb uplift at joint 45, 46 lb uplift at joint 46, 51 lb uplift at joint 47, 54 lb uplift at joint 49, 45 lb uplift at joint 50, 52 lb uplift at joint 51, 30 lb uplift at joint 52, 111 lb uplift at joint 53, 20 lb uplift at joint 38, 64 lb uplift at joint 37, 58 lb uplift at joint 36, 53 lb uplift at joint 34, 51 lb uplift at joint 33, 59 lb uplift at joint 32, 58 lb uplift at joint 31, 43 lb uplift at joint 30 and 94 lb uplift at joint 29.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

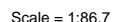
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[illegible]

BOT CHORD 2-30=-716/3855, 28-30=-716/3855,
26-28=-406/2660, 25-26=-104/1054,
23-25=-104/1054, 22-23=-1129/309,
21-22=-1129/309, 20-21=-1129/309,
18-20=-1129/309

WEBS 3-30=0/413, 5-28=-12/805, 3-28=-1282/332,
7-26=-130/772, 23-37=-293/1901,
15-37=-325/1858, 11-35=-2007/391,
35-36=-2075/396, 23-36=-2054/399,
5-31=-1421/356, 26-31=-1487/379,
26-32=-136/740, 32-33=-127/695,
33-34=-130/702, 11-34=-138/757,
6-31=-104/35, 8-32=-14/73, 9-33=-22/12,
10-34=0/127, 25-34=0/178, 24-35=-6/85,
12-36=0/32, 13-37=-15/98, 16-21=-115/61,
17-20=-224/148, 15-22=-2291/474

- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 8) Bearings are assumed to be: Joint 2 SPF No.2 crushing capacity of 425 psi, Joint 22 SP 2400F 2.0E crushing capacity of 805 psi, Joint 22 SP 2400F 2.0E crushing capacity of 805 psi.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 431 lb uplift at joint 18, 365 lb uplift at joint 2, 180 lb uplift at joint 21, 94 lb uplift at joint 20 and 384 lb uplift at joint 22.
 - 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- LOAD CASE(S)** Standard

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust)
Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft;
Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope)
exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0,
Interior (1) 4-1-0 to 26-9-3, Exterior(2R) 26-9-3 to
31-9-3, Interior (1) 31-9-3 to 48-11-0 zone; cantilever left
and right exposed ; end vertical left and right
exposed; C-C for members and forces & MWFRS for
reactions shown; Lumber DOL=1.60 plate grip
DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss
only. For studs exposed to wind (normal to the face),
see Standard Industry Gable End Details as applicable,
or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 3x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom
chord live load nonconcurrent with any other live loads.



October 6.2023



WARNING – Verify design parameters and/or notes on this and included literature reference page. (M-747-16V, 1/2/2023) BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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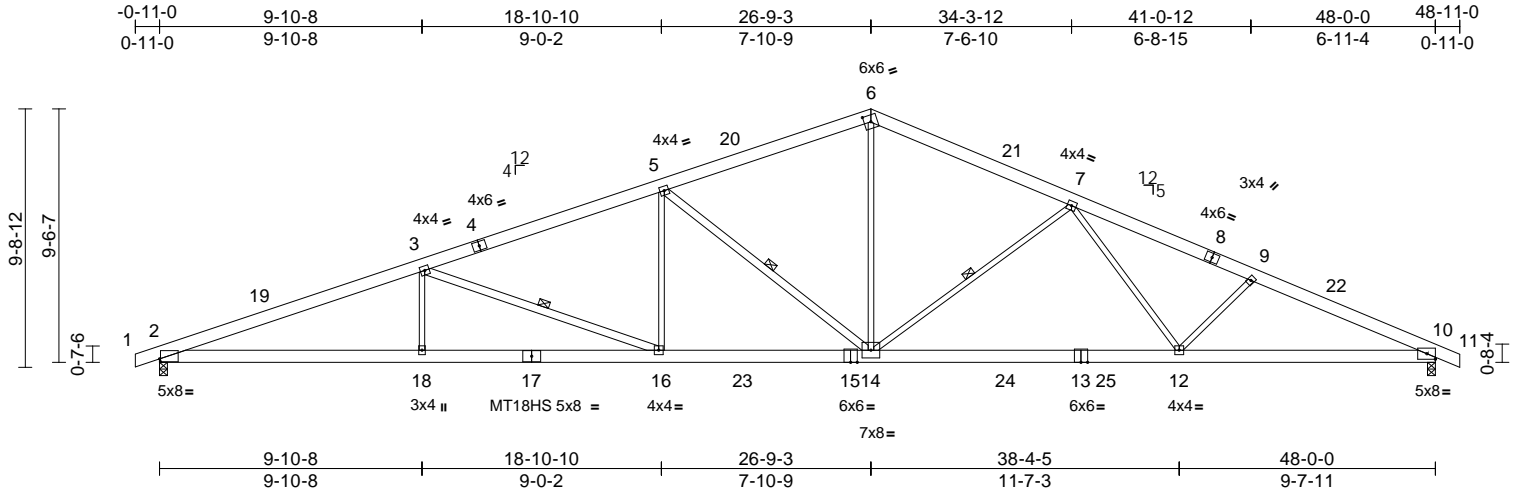
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197922
P240539-01	B2	Roof Special	4	1	Job Reference (optional)	

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Scale = 1:86.7

Plate Offsets (X, Y): [2:0-0-6,0-1-5], [6:0-3-1,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.82	Vert(LL)	-0.57	12-14	>997	240	MT20	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.85	Vert(CT)	-0.99	12-14	>579	180	MT18HS	244/190
BCLL	0.0*	Rep Stress Incr	YES	WB	0.87	Horz(CT)	0.17	10	n/a	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S								
Weight: 262 lb											FT = 20%	

LUMBER

TOP CHORD 2x6 SPF No.2
 BOT CHORD 2x6 SP 2400F 2.0E *Except* 13-15:2x6 SPF No.2
 WEBS 2x3 SPF No.2 *Except* 14-5,16-3:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 9-0-8 oc bracing.
 WEBS 1 Row at midpt 7-14, 5-14, 3-16

REACTIONS

(size) 2=0-3-8, 10=0-3-8
 Max Horiz 2=-174 (LC 17)
 Max Uplift 2=-437 (LC 8), 10=-339 (LC 13)
 Max Grav 2=2311 (LC 2), 10=2327 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/11, 2-3=-5737/1125, 3-5=-4431/954, 5-6=-3224/808, 6-7=-3321/819, 7-9=-4720/933, 9-10=-4954/981, 10-11=0/12
 BOT CHORD 2-18=-950/5331, 16-18=-950/5331, 14-16=-661/4121, 12-14=-648/3781, 10-12=-789/4440
 WEBS 3-18=0/433, 6-14=-312/1825, 9-12=-352/251, 5-16=-16/820, 7-14=-1037/335, 7-12=-20/912, 5-14=-1484/379, 3-16=-1307/345

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 26-9-3, Exterior(2R) 26-9-3 to 31-9-3, Interior (1) 31-9-3 to 48-11-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP 2400F 2.0E crushing capacity of 805 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 339 lb uplift at joint 10 and 437 lb uplift at joint 2.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



October 6, 2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of the design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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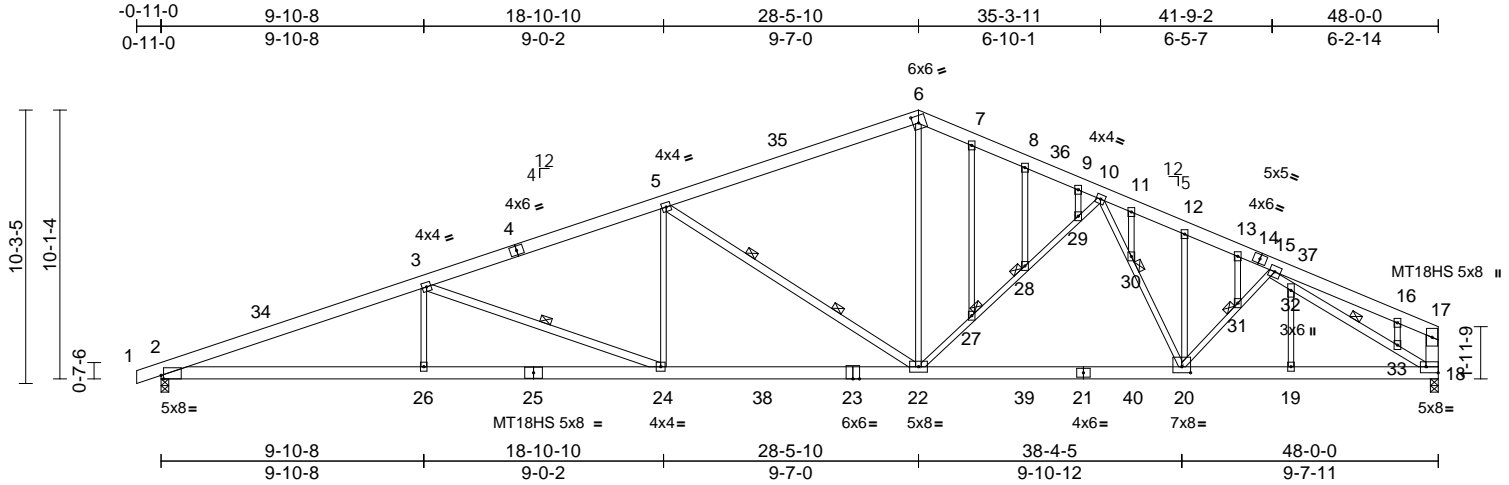
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197923
P240539-01	C1	Roof Special Structural Gable	1	1	Job Reference (optional)	

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Scale = 1:86.6

Plate Offsets (X, Y): [2:0-1-2,Edge], [6:0-2-9,0-3-4], [18:Edge,0-2-12], [20:0-4-0,0-2-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.49	Vert(LL)	-0.37	24-26	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.50	Vert(CT)	-0.67	24-26	>851	180	MT18HS	197/144
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.93	Horz(CT)	0.14	18	n/a	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S								
											Weight: 317 lb	FT = 20%

LUMBER
TOP CHORD 2x6 SP 2400F 2.0E *Except* 14-17,14-6:2x6 SPF No.2
BOT CHORD 2x6 SP 2400F 2.0E *Except* 0-0:2x6 SPF No.2
WEBS 2x3 SPF No.2 *Except* 18-17:2x6 SPF No.2, 24-3,22-5:2x4 SP No.2, 18-15:2x4 SP 2400F 2.0E
OTHERS 2x3 SPF No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-24, 15-18
WEBS 2 Rows at 1/3 pts 5-22
JOINTS 1 Brace at Jt(s): 27, 28, 30, 31

REACTIONS (size) 2=0-3-8, 18=0-3-8
Max Horiz 2=193 (LC 16)
Max Uplift 2=445 (LC 8), 18=292 (LC 13)
Max Grav 2=2313 (LC 2), 18=2270 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/11, 2-3=-5747/1140, 3-5=-4472/964, 5-6=-3029/755, 6-7=-2978/778, 7-8=-3017/767, 8-9=-3052/755, 9-10=-3119/763, 10-11=-3492/802, 11-12=-3527/798, 12-13=-3526/761, 13-15=-3584/752, 15-16=-784/259, 16-17=-928/249, 17-18=-760/253
BOT CHORD 2-26=-1070/5365, 24-26=-1070/5365, 22-24=-793/4163, 20-22=-587/3147, 19-20=-586/2823, 18-19=-586/2823

WEBS
3-26=0/421, 6-22=-260/1626, 10-30=-9/336, 20-30=-11/314, 5-24=-19/833, 20-31=-12/672, 15-31=-12/630, 3-24=-1298/352, 22-27=-612/265, 27-28=-600/258, 28-29=-586/252, 10-29=-686/297, 15-32=-2739/509, 32-33=-2595/472, 18-33=-2448/463, 5-22=-1675/452, 7-27=-17/17, 8-28=-25/11, 9-29=-61/138, 11-30=-26/0, 12-20=-149/70, 13-31=-1/56, 19-32=-350/80, 16-33=-27/275

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 28-5-10, Exterior(2R) 28-5-10 to 33-5-10, Interior (1) 33-5-10 to 47-9-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 3x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) All bearings are assumed to be SP 2400F 2.0E crushing capacity of 805 psi.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 18 and 445 lb uplift at joint 2.
11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
LOAD CASE(S) Standard



October 6,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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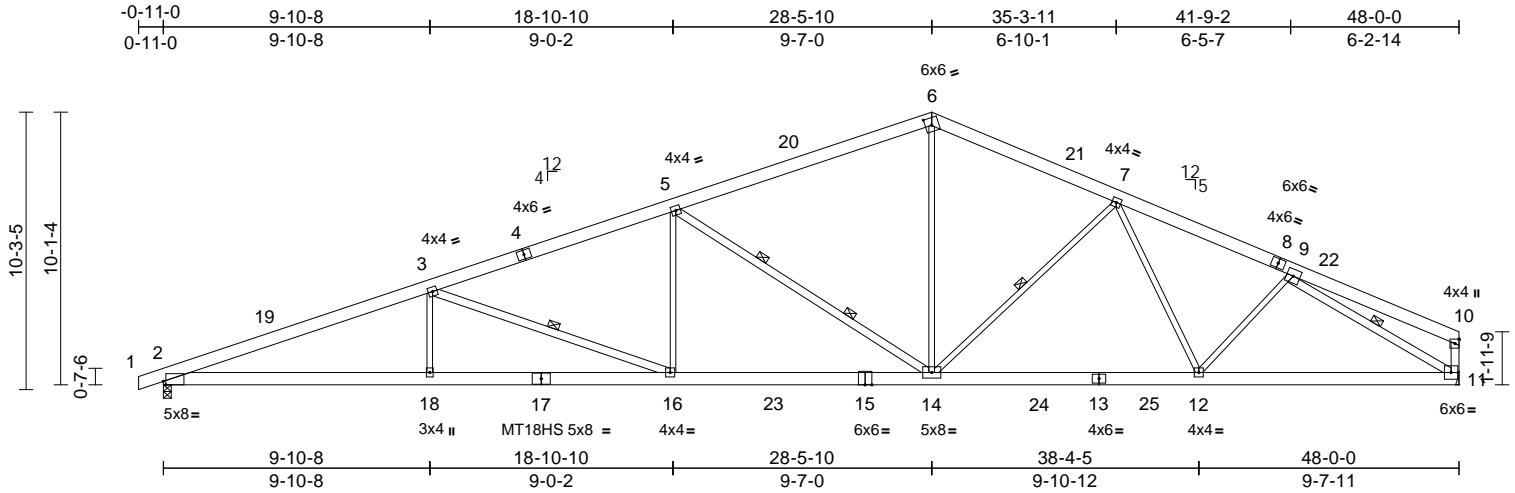
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Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197924
P240539-01	C2	Roof Special	2	1	Job Reference (optional)	

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Page: 1



Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.47	Vert(LL)	-0.40	12-14	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.48	Vert(CT)	-0.68	16-18	>847	180	MT18HS	244/190
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.88	Horz(CT)	0.15	11	n/a	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S							Weight: 297 lb	FT = 20%

LUMBER		
TOP CHORD	2x6 SP 2400F 2.0E	*Except* 8-10,8-6:2x6 SPF No.2
BOT CHORD	2x6 SP 2400F 2.0E	
WEBS	2x3 SPF No.2	*Except* 11-10:2x4 SPF No.3, 14-5,16-3,11-9:2x4 SP No.2
BRACING		
TOP CHORD	Structural wood sheathing directly applied or 3-7-11 oc purlins, except end verticals.	
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	
WEBS	1 Row at midpt	3-16, 7-14, 9-11
WEBS	2 Rows at 1/3 pts	5-14
REACTIONS (size) 2=0-3-8, 11= Mechanical		
	Max Horiz	2=193 (LC 16)
	Max Uplift	2=-446 (LC 8), 11=-293 (LC 13)
	Max Grav	2=2317 (LC 2), 11=2274 (LC 2)
FORCES (lb) - Maximum Compression/Maximum Tension		
TOP CHORD	1-2=0/11, 2-3=-5764/1144, 3-5=-4482/966, 5-6=-3039/756, 6-7=-3099/781, 7-9=-3586/778, 9-10=-247/137, 10-11=-275/165	
BOT CHORD	2-18=-1071/5380, 16-18=-1071/5380, 14-16=-793/4173, 12-14=-594/3173, 11-12=-631/2987	
WEBS	3-18=0/423, 6-14=-270/1656, 9-12=0/439, 5-16=-17/828, 7-12=-27/287, 5-14=-1678/454, 3-16=-1302/352, 7-14=-631/272, 9-11=-3430/682	

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 28-5-10, Exterior(2R) 28-5-10 to 33-5-10, Interior (1) 33-5-10 to 47-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 2 SP 2400F 2.0E crushing capacity of 805 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 446 lb uplift at joint 2 and 293 lb uplift at joint 11.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

- NOTES**
- Unbalanced roof live loads have been considered for this design.



October 6, 2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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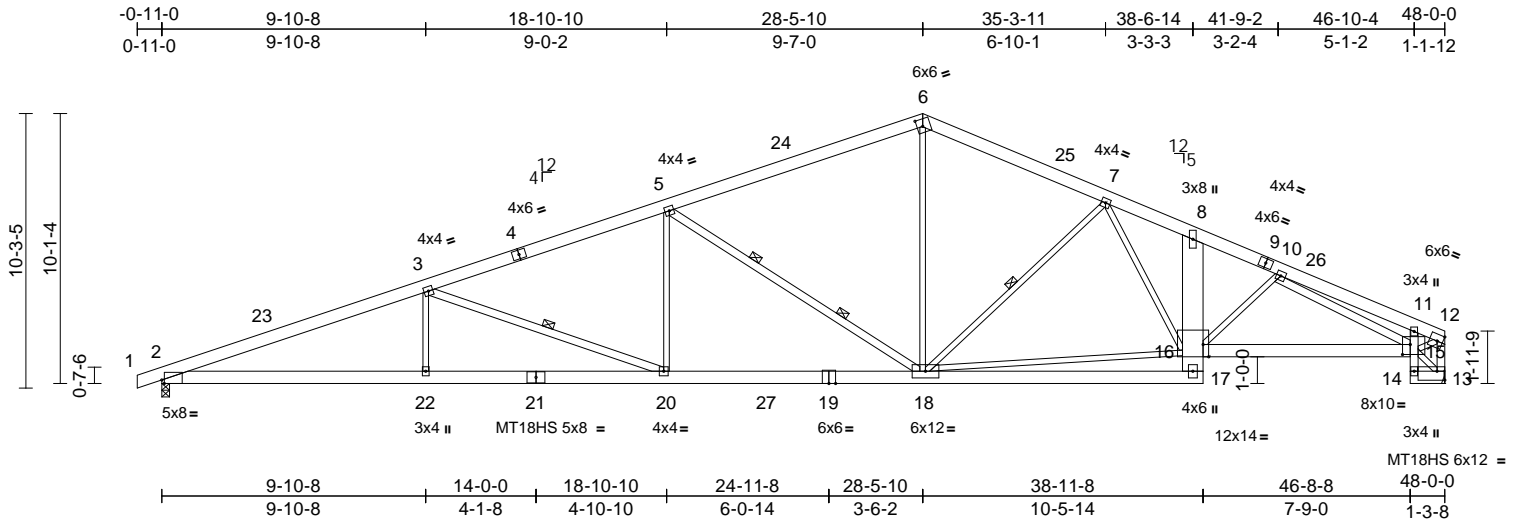
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197925
P240539-01	C3	Roof Special	5	1	Job Reference (optional)	

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Scale = 1:86.2

Plate Offsets (X, Y): [2:0-1-2,Edge], [6:0-2-9,0-3-4], [13:Edge,0-4-0], [15:0-3-8,0-4-8], [16:0-2-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.36	Vert(LL)	-0.34	20-22	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.87	Vert(CT)	-0.63	20-22	>903	180	MT18HS	197/144
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.92	Horz(CT)	0.21	13	n/a	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S								
Weight: 320 lb											FT = 20%	

LUMBER
TOP CHORD 2x6 SP 2400F 2.0E *Except* 9-12,9-6:2x6 SPF No.2
BOT CHORD 2x6 SPF No.2 *Except* 17-8:2x10 SP 2400F 2.0E, 11-14:2x4 SP No.2, 21-2,19-17,19-21:2x6 SP 2400F 2.0E
WEBS 2x3 SPF No.2 *Except* 13-12:2x4 SPF No.3, 20-3,15-10,18-5:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
8-1-1 oc bracing: 15-16.
WEBS 1 Row at midpt 3-20, 7-18
WEBS 2 Rows at 1/3 pts 5-18

REACTIONS (size) 2=0-3-8, 13= Mechanical
Max Horiz 2=193 (LC 12)
Max Uplift 2=446 (LC 8), 13=293 (LC 13)
Max Grav 2=2300 (LC 2), 13=2230 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/11, 2-3=5692/1145, 3-5=4441/965, 5-6=2989/751, 6-7=3056/776, 7-8=3633/879, 8-10=4162/936, 10-11=2716/705, 11-12=2337/552, 12-13=2126/475
BOT CHORD 16-17=0/176, 8-16=0/95, 15-16=834/3775, 14-15=0/7, 11-15=319/242, 13-14=67/119, 2-22=1072/5313, 20-22=1072/5313, 18-20=792/4134, 17-18=384/1922
WEBS 3-22=0/408, 5-20=19/832, 3-20=1273/356, 6-18=262/1617, 7-16=58/382, 10-16=86/151, 10-15=1549/340, 7-18=715/299, 5-18=1684/457, 13-15=135/112, 12-15=608/2397, 16-18=242/1271

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 28-5-10, Exterior(2R) 28-5-10 to 33-5-10, Interior (1) 33-5-10 to 47-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 2 SP 2400F 2.0E crushing capacity of 805 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 293 lb uplift at joint 13 and 446 lb uplift at joint 2.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



October 6, 2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of the design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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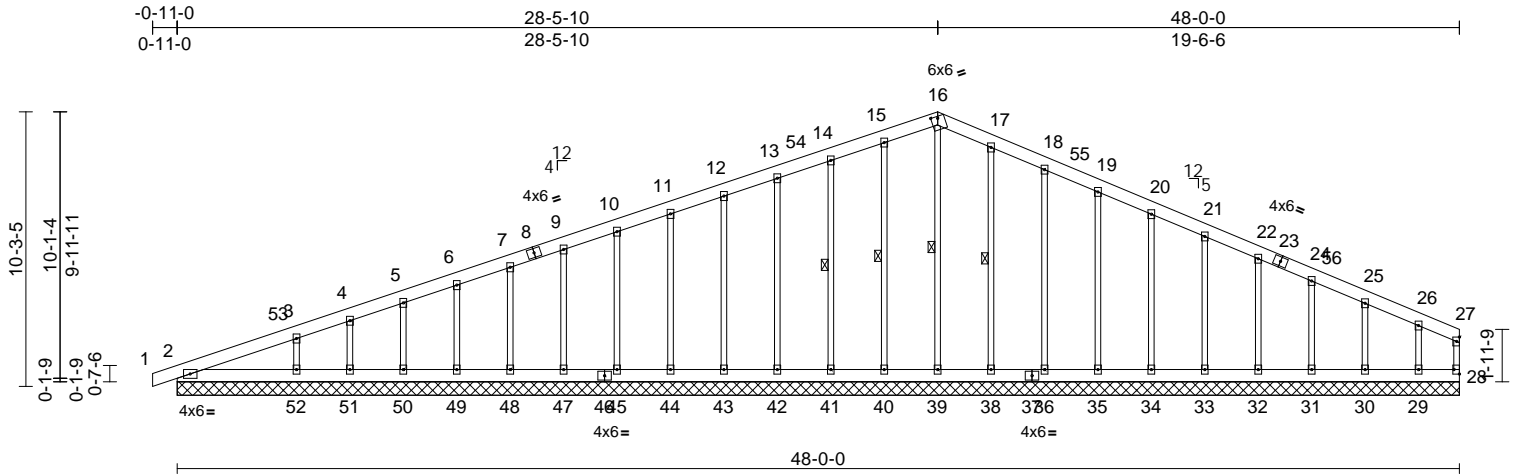
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197926
P240539-01	C4	Roof Special Supported Gable	1	1	Job Reference (optional)	

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Scale = 1:86.2

Plate Offsets (X, Y): [16:0-3-0,0-0-15], [28:Edge,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	n/a	-	n/a	999	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	n/a	-	n/a	999	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.21	Horz(CT)	0.00	28	n/a	n/a	
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S							
Weight: 273 lb FT = 20%											

LUMBER

TOP CHORD	2x6 SPF No.2
BOT CHORD	2x6 SPF No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 16-39, 15-40, 14-41, 17-38

REACTIONS (size)

Max Horiz	2=193 (LC 12)
Max Uplift	2=44 (LC 13), 29=81 (LC 13), 30=49 (LC 13), 31=57 (LC 13), 32=59 (LC 13), 33=50 (LC 13), 34=54 (LC 13), 35=56 (LC 13), 36=65 (LC 13), 38=20 (LC 13), 40=26 (LC 12), 41=57 (LC 8), 42=50 (LC 8), 43=49 (LC 12), 44=49 (LC 12), 45=46 (LC 8), 47=51 (LC 8), 48=54 (LC 12), 49=45 (LC 8), 50=52 (LC 12), 51=30 (LC 8), 52=111 (LC 12)

FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/11, 2-3=-195/129, 3-4=-145/143, 4-5=-125/157, 5-6=-103/175, 6-7=-82/192, 7-9=-88/210, 9-10=-100/228, 10-11=-111/245, 11-12=-123/264, 12-13=-135/293, 13-14=-147/323, 14-15=-160/354, 15-16=-165/371, 16-17=-169/372, 17-18=-161/340, 18-19=-144/291, 19-20=-129/251, 20-21=-114/215, 21-22=-100/179, 22-24=-83/140, 24-25=-67/102, 25-26=-53/66, 26-27=-45/36, 27-28=-41/19
BOT CHORD	2-52=-38/53, 51-52=-38/53, 50-51=-38/53, 49-50=-38/53, 48-49=-38/53, 47-48=-38/53, 45-47=-38/53, 44-45=-38/53, 43-44=-38/53, 42-43=-38/53, 41-42=-38/53, 40-41=-38/53, 39-40=-38/53, 38-39=-38/53, 36-38=-38/53, 35-36=-38/53, 34-35=-38/53, 33-34=-38/53, 32-33=-38/53, 31-32=-38/53, 30-31=-38/53, 29-30=-38/53, 28-29=-38/53

WEBS

16-39=-164/21, 15-40=-144/57, 14-41=-143/97, 13-42=-140/74, 12-43=-140/73, 11-44=-140/73, 10-45=-134/70, 9-47=-144/75, 7-48=-149/78, 6-49=-131/69, 5-50=-149/77, 4-51=-84/47, 3-52=-286/172, 17-38=-144/59, 18-36=-143/105, 19-35=-140/80, 20-34=-140/78, 21-33=-133/74, 22-32=-149/84, 24-31=-143/84, 25-30=-138/105, 26-29=-129/112

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TC DL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 28-5-10, Exterior(2R) 28-5-10 to 33-5-10, Interior (1) 33-5-10 to 47-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



October 6, 2023

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79
P240539-01	C4	Roof Special Supported Gable	1	1	I61197926
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Page: 2

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 3x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 2, 26 lb uplift at joint 40, 57 lb uplift at joint 41, 50 lb uplift at joint 42, 49 lb uplift at joint 43, 49 lb uplift at joint 44, 46 lb uplift at joint 45, 51 lb uplift at joint 47, 54 lb uplift at joint 48, 45 lb uplift at joint 49, 52 lb uplift at joint 50, 30 lb uplift at joint 51, 111 lb uplift at joint 52, 20 lb uplift at joint 38, 65 lb uplift at joint 36, 56 lb uplift at joint 35, 54 lb uplift at joint 34, 50 lb uplift at joint 33, 59 lb uplift at joint 32, 57 lb uplift at joint 31, 49 lb uplift at joint 30 and 81 lb uplift at joint 29.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

 **WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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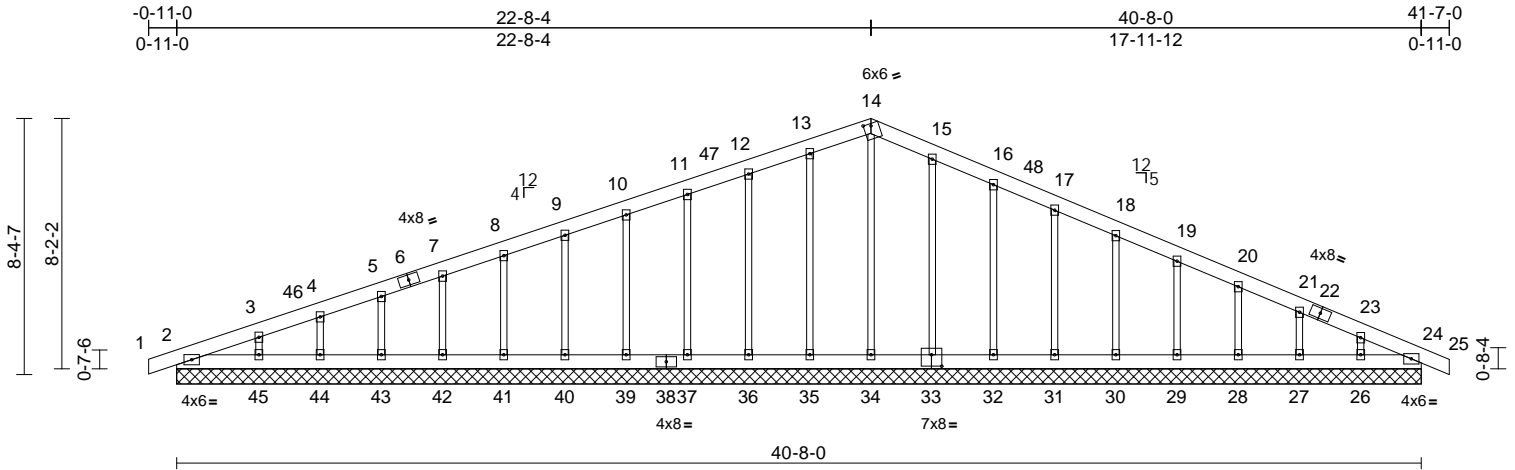
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197927
P240539-01	E1	Roof Special Supported Gable	2	1	Job Reference (optional)	

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Scale = 1:75.3

Plate Offsets (X, Y): [14:0-3-0,0-0-15], [33:0-4-0,0-4-8]

Loading	(psf)	Spacing	1-11-4	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20
TCDL	10.0	Lumber DOL	1.15	BC	0.02	Vert(CT)	n/a	-	n/a	999	197/144
BCLL	0.0*	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.01	24	n/a	n/a	
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S							
Weight: 213 lb FT = 20%											

LUMBER
TOP CHORD 2x6 SPF No.2
BOT CHORD 2x6 SPF No.2
OTHERS 2x3 SPF No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size)
2=40-8-0, 24=40-8-0, 26=40-8-0, 27=40-8-0, 28=40-8-0, 29=40-8-0, 30=40-8-0, 31=40-8-0, 32=40-8-0, 33=40-8-0, 34=40-8-0, 35=40-8-0, 36=40-8-0, 37=40-8-0, 39=40-8-0, 40=40-8-0, 41=40-8-0, 42=40-8-0, 43=40-8-0, 44=40-8-0, 45=40-8-0
Max Horiz 2=144 (LC 13)
Max Uplift 2=20 (LC 13), 24=14 (LC 9), 26=62 (LC 13), 27=58 (LC 13), 28=49 (LC 13), 29=53 (LC 13), 30=52 (LC 13), 31=54 (LC 13), 32=57 (LC 13), 33=39 (LC 13), 35=34 (LC 12), 36=54 (LC 8), 37=46 (LC 8), 39=48 (LC 12), 40=48 (LC 8), 41=44 (LC 12), 42=49 (LC 8), 43=53 (LC 12), 44=42 (LC 8), 45=69 (LC 12)
Max Grav 2=178 (LC 1), 24=155 (LC 1), 26=170 (LC 26), 27=184 (LC 26), 28=167 (LC 1), 29=175 (LC 26), 30=175 (LC 1), 31=173 (LC 1), 32=182 (LC 26), 33=169 (LC 1), 34=170 (LC 22), 35=177 (LC 1), 36=175 (LC 25), 37=177 (LC 1), 39=174 (LC 1), 40=174 (LC 25), 41=169 (LC 25), 42=177 (LC 25), 43=187 (LC 1), 44=151 (LC 25), 45=226 (LC 25)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/11, 2-3=-171/68, 3-4=-127/73, 4-5=-100/82, 5-7=-83/95, 7-8=-67/112, 8-9=-50/129, 9-10=-53/146, 10-11=-65/163, 11-12=-76/180, 12-13=-88/206, 13-14=-97/230, 14-15=-100/236, 15-16=-88/207, 16-17=-72/169, 17-18=-57/133, 18-19=-44/97, 19-20=-44/61, 20-21=-53/28, 21-23=-72/18, 23-24=-114/39, 24-25=0/11
BOT CHORD 2-45=-35/125, 44-45=-35/125, 43-44=-35/125, 42-43=-35/125, 41-42=-35/125, 40-41=-35/125, 39-40=-35/125, 37-39=-35/125, 36-37=-35/125, 35-36=-35/125, 34-35=-35/125, 32-34=-35/125, 31-32=-35/125, 30-31=-35/125, 29-30=-35/125, 28-29=-35/125, 27-28=-35/125, 26-27=-35/125, 24-26=-35/125
WEBS 14-34=-127/0, 13-35=-139/67, 12-36=-138/92, 11-37=-135/71, 10-39=-136/71, 9-40=-136/71, 8-41=-130/68, 7-42=-139/72, 5-43=-146/76, 4-44=-118/64, 3-45=-169/116, 15-33=-139/75, 16-32=-138/98, 17-31=-135/76, 18-30=-136/76, 19-29=-137/77, 20-28=-128/72, 21-27=-146/97, 23-26=-130/103

NOTES
1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 22-8-4, Exterior(2R) 22-8-4 to 27-8-4, Interior (1) 27-8-4 to 41-7-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 3x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.



October 6, 2023

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79
P240539-01	E1	Roof Special Supported Gable	2	1	I61197927
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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- 9) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 2, 34 lb uplift at joint 35, 54 lb uplift at joint 36, 46 lb uplift at joint 37, 48 lb uplift at joint 39, 48 lb uplift at joint 40, 44 lb uplift at joint 41, 49 lb uplift at joint 42, 53 lb uplift at joint 43, 42 lb uplift at joint 44, 69 lb uplift at joint 45, 39 lb uplift at joint 33, 57 lb uplift at joint 32, 54 lb uplift at joint 31, 52 lb uplift at joint 30, 53 lb uplift at joint 29, 49 lb uplift at joint 28, 58 lb uplift at joint 27, 62 lb uplift at joint 26 and 14 lb uplift at joint 24.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

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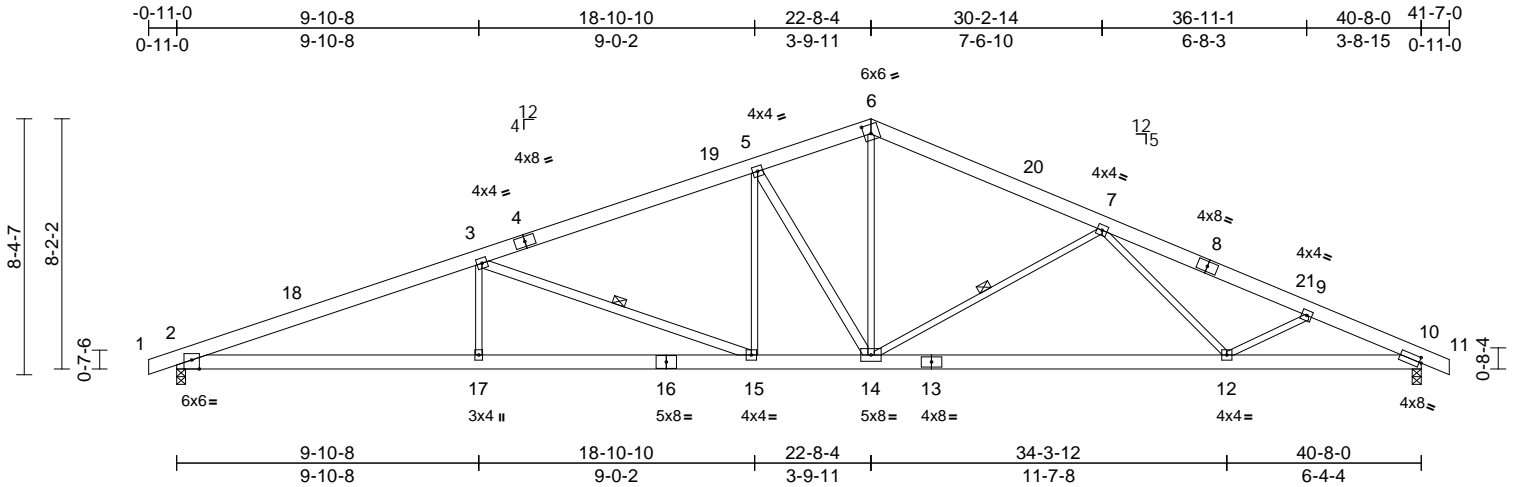
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197928
P240539-01	E2	Roof Special	4	1	Job Reference (optional)	

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Scale = 1:75.3

Plate Offsets (X, Y): [2:0-3-0,Edge], [6:0-2-13,0-3-8], [10:0-0-13,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.92	Vert(LL)	-0.26	15-17	>999	240	MT20	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.96	Vert(CT)	-0.55	15-17	>874	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.98	Horz(CT)	0.15	10	n/a	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S							Weight: 206 lb	FT = 20%

LUMBER

TOP CHORD 2x6 SPF No.2
BOT CHORD 2x6 SPF No.2
WEBS 2x3 SPF No.2 *Except* 14-5,15-3:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-15, 7-14

REACTIONS

(size) 2=0-3-8, 10=0-3-8
Max Horiz 2=-149 (LC 17)
Max Uplift 2=-377 (LC 8), 10=-291 (LC 13)
Max Grav 2=1891 (LC 1), 10=1891 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/11, 2-3=-4410/924, 3-5=-3163/749, 5-6=-2570/697, 6-7=-2707/687, 7-9=-3717/822, 9-10=-3910/910, 10-11=0/12
BOT CHORD 2-17=-758/4059, 15-17=-758/4059, 14-15=-466/2902, 12-14=-625/3123, 10-12=-762/3488
WEBS 3-17=0/404, 6-14=-333/1488, 9-12=-212/226, 5-15=-49/573, 7-12=0/484, 5-14=-1045/305, 3-15=-1256/348, 7-14=-885/362

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 22-8-4, Exterior(2R) 22-8-4 to 27-8-4, Interior (1) 27-8-4 to 41-7-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 377 lb uplift at joint 2 and 291 lb uplift at joint 10.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



October 6, 2023

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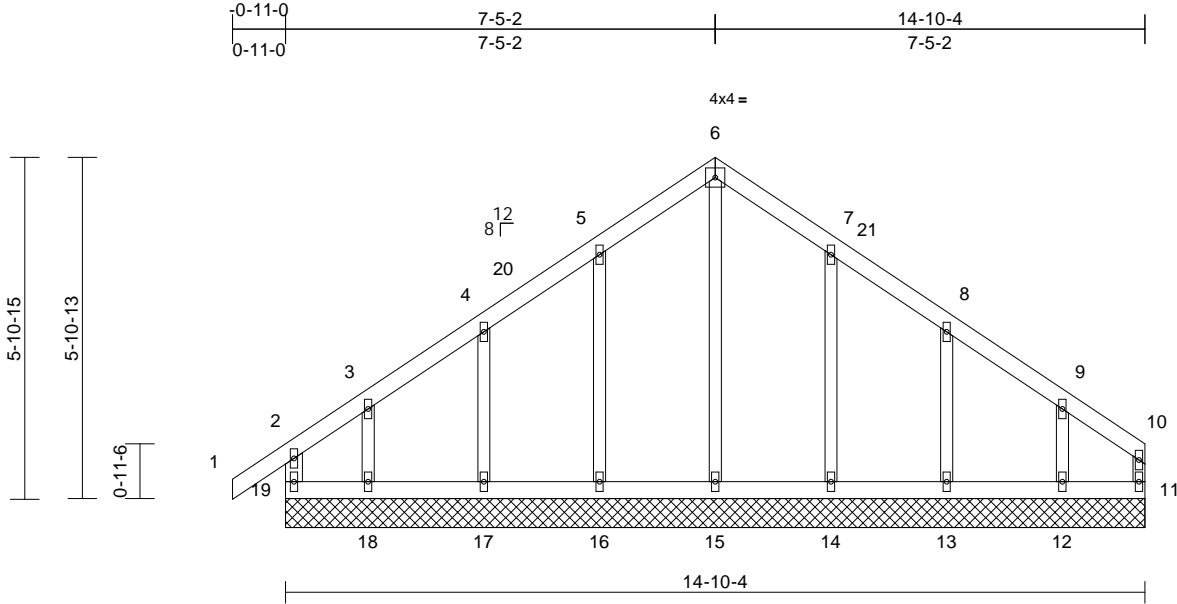
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197929
P240539-01	G1A	Common Supported Gable	1	1	Job Reference (optional)	

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Scale = 1:39.8

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.09	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.06	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.14	Horz(CT)	0.00	11	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-R						Weight: 69 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.2 *Except* 10-11:2x3 SPF No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	11=14-10-4, 12=14-10-4, 13=14-10-4, 14=14-10-4, 15=14-10-4, 16=14-10-4, 17=14-10-4, 18=14-10-4, 19=14-10-4
Max Horiz	19=178 (LC 9)
Max Uplift	11=44 (LC 9), 12=109 (LC 13), 13=76 (LC 13), 14=78 (LC 13), 16=80 (LC 12), 17=74 (LC 12), 18=120 (LC 12), 19=97 (LC 8)
Max Grav	11=84 (LC 19), 12=202 (LC 20), 13=187 (LC 20), 14=198 (LC 20), 15=191 (LC 22), 16=199 (LC 19), 17=188 (LC 19), 18=189 (LC 19), 19=197 (LC 20)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	2-19=-159/94, 1-2=0/41, 2-3=-130/119, 3-4=-96/96, 4-5=-89/170, 5-6=-129/251, 6-7=-130/252, 7-8=-88/169, 8-9=-63/81, 9-10=-71/67, 10-11=-60/34
BOT CHORD	18-19=-66/71, 17-18=-66/71, 16-17=-66/71, 15-16=-66/71, 14-15=-66/71, 13-14=-66/71, 12-13=-66/71, 11-12=-66/71
WEBS	6-15=-188/42, 5-16=-159/123, 4-17=-151/160, 3-18=-133/129, 7-14=-157/133, 8-13=-149/164, 9-12=-149/156

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-11-0 to 4-1-0, Exterior(2N) 4-1-0 to 7-5-2, Corner(3R) 7-5-2 to 12-5-2, Exterior(2N) 12-5-2 to 14-9-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 19, 44 lb uplift at joint 11, 80 lb uplift at joint 16, 74 lb uplift at joint 17, 120 lb uplift at joint 18, 78 lb uplift at joint 14, 76 lb uplift at joint 13 and 109 lb uplift at joint 12.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



October 6, 2023

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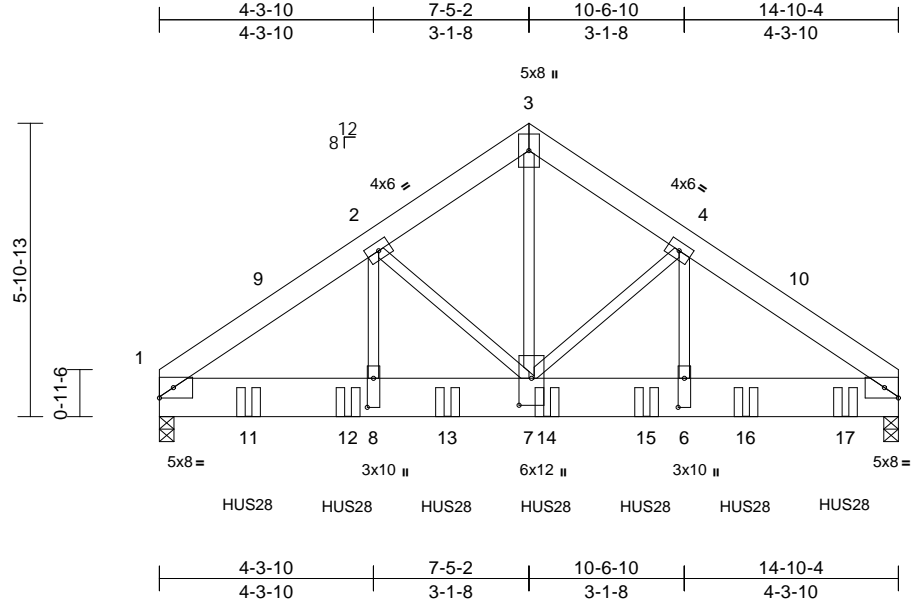
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	
P240539-01	G2A	Common Girder	1	3	Job Reference (optional)	I61197930

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Scale = 1:46.3									
Plate Offsets (X, Y): [6:0-7-0,0-1-8], [7:0-6-8,0-3-0], [8:0-7-0,0-1-8]									
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.22	Vert(LL)	-0.05	6-7	>999
TCDL	10.0	Lumber DOL	1.15	BC	0.24	Vert(CT)	-0.09	6-7	>999
BCLL	0.0*	Rep Stress Incr	NO	WB	0.90	Horz(CT)	0.02	5	n/a
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S					
PLATES MT20 GRIP 197/144 Weight: 317 lb FT = 20%									

LUMBER
TOP CHORD 2x6 SPF No.2
BOT CHORD 2x10 SP 2400F 2.0E
WEBS 2x3 SPF No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=0-3-8, 5=0-3-8
Max Horiz 1=-146 (LC 10)
Max Uplift 1=-1105 (LC 12), 5=-1212 (LC 13)
Max Grav 1=7954 (LC 2), 5=8734 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-9871/1429, 2-3=-7507/1164, 3-4=-7508/1164, 4-5=-9913/1435
BOT CHORD 1-8=-1108/7790, 7-8=-1111/7813, 6-7=-1074/7850, 5-6=-1071/7825
WEBS 2-8=-416/3290, 3-7=-1166/7827, 2-7=-2163/406, 4-6=-422/3347, 4-7=-2214/416

NOTES

- 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x10 - 5 rows staggered at 0-7-0 oc.
Web connected as follows: 2x3 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust)
Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft;
Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 5-1-12, Interior (1) 5-1-12 to 7-5-2, Exterior(2R) 7-5-2 to 12-5-2, Interior (1) 12-5-2 to 14-8-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP 2400F 2.0E crushing capacity of 805 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1105 lb uplift at joint 1 and 1212 lb uplift at joint 5.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Use Simpson Strong-Tie HUS28 (22-10d Girder, 4-10d Truss) or equivalent spaced at 12-0-0 oc max. starting at 1-9-8 from the left end to 13-9-8 to connect truss(es) to back face of bottom chord.
- Use Simpson Strong-Tie HUS28 (22-16d Girder, 4-16d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 3-9-8 from the left end to 11-9-8 to connect truss(es) to back face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-3=-70, 3-5=-70, 1-5=-20
Concentrated Loads (lb)

Vert: 11=-2126 (B), 12=-2126 (B), 13=-2126 (B), 14=-2126 (B), 15=-2126 (B), 16=-2126 (B), 17=-2127 (B)



October 6, 2023

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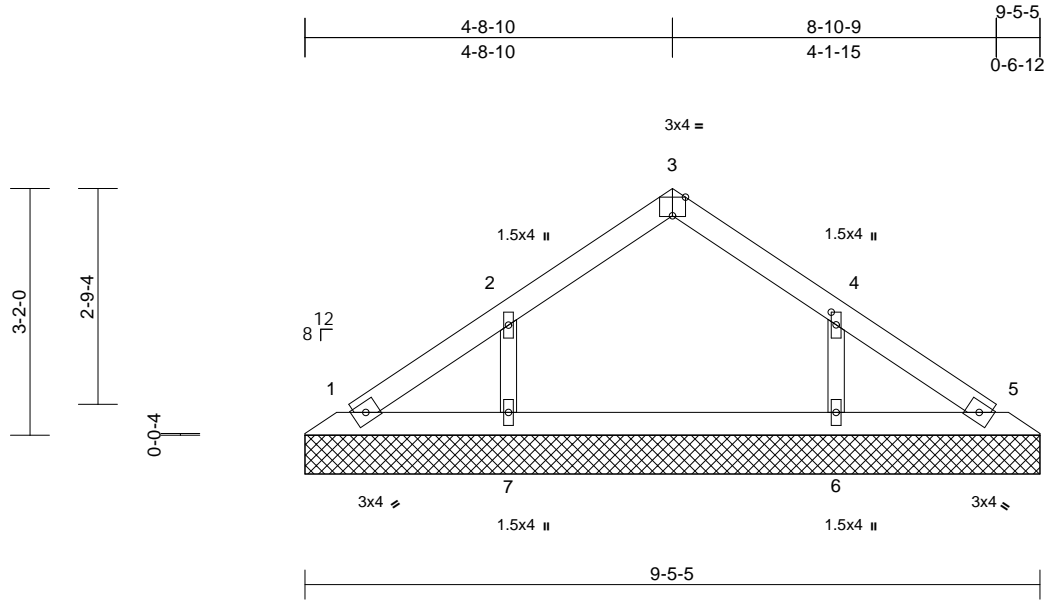
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197931
P240539-01	V5	Valley	1	1	Job Reference (optional)	

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Scale = 1:29.6

Plate Offsets (X, Y): [3'-0"-2'-0", Edge], [4'-0"-1'-15", 0'-0"-12"]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	n/a	-	n/a	999	MT20
TCDL	10.0	Lumber DOL	1.15	BC	0.11	Vert(TL)	n/a	-	n/a	999	244/190
BCLL	0.0*	Rep Stress Incr	YES	WB	0.04	Horiz(TL)	0.00	5	n/a	n/a	
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-S							Weight: 31 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x3 SPF No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0'-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0'-0 oc bracing.

REACTIONS

(size) 1=9'-5'-5, 5=9'-5'-5, 6=9'-5'-5, 7=9'-5'-5
Max Horiz 1=-80 (LC 8)
Max Uplift 1=-5 (LC 13), 5=-4 (LC 12), 6=-82 (LC 13), 7=-83 (LC 12)
Max Grav 1=127 (LC 1), 5=127 (LC 1), 6=256 (LC 20), 7=257 (LC 19)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-151/26, 2-3=-159/76, 3-4=-159/76, 4-5=-149/23
BOT CHORD 1-7=-14/109, 6-7=-14/109, 5-6=-14/109
WEBS 2-7=-178/151, 4-6=-177/151

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4'-0'-0 oc.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0'-0 tall by 2'-0'-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 1, 4 lb uplift at joint 5, 83 lb uplift at joint 7 and 82 lb uplift at joint 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



October 6, 2023

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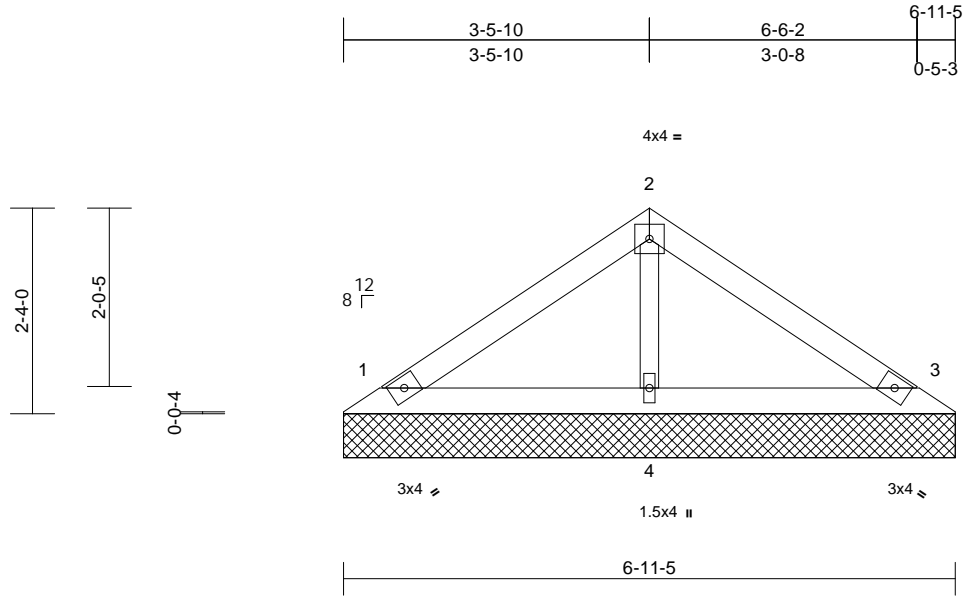
Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197932
P240539-01	V6	Valley	1	1	Job Reference (optional)	

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Scale = 1:26.2

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	n/a	-	n/a	999	MT20
TCDL	10.0	Lumber DOL	1.15	BC	0.09	Vert(TL)	n/a	-	n/a	999	244/190
BCLL	0.0*	Rep Stress Incr	YES	WB	0.03	Horiz(TL)	0.00	3	n/a	n/a	
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-P							Weight: 23 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x3 SPF No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 1=6-11-5, 3=6-11-5, 4=6-11-5
Max Horiz 1=-56 (LC 10)
Max Uplift 1=-39 (LC 12), 3=-46 (LC 13)
Max Grav 1=153 (LC 1), 3=153 (LC 1), 4=238 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-96/61, 2-3=-91/61
BOT CHORD 1-4=-12/45, 3-4=-12/45
WEBS 2-4=-162/92

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 1 and 46 lb uplift at joint 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



October 6, 2023

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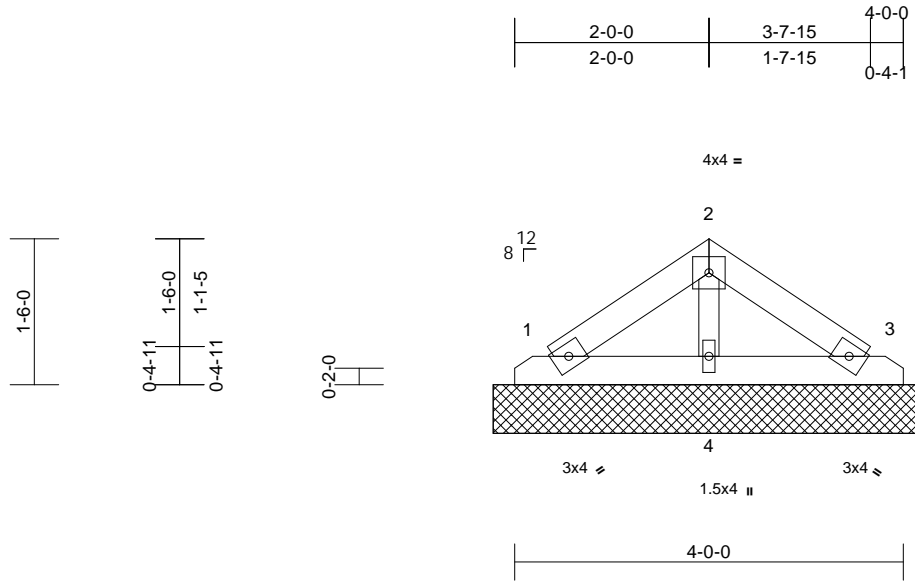
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Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 79	I61197933
P240539-01	V7	Valley	1	1	Job Reference (optional)	

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Page: 1



Scale = 1:23.7

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	n/a	-	n/a	999	MT20
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(TL)	n/a	-	n/a	999	244/190
BCLL	0.0*	Rep Stress Incr	YES	WB	0.01	Horiz(TL)	0.00	3	n/a	n/a	
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-P							Weight: 13 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x3 SPF No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-6-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 1=4-5-5, 3=4-5-5, 4=4-5-5
Max Horiz 1=33 (LC 9)
Max Uplift 1=-21 (LC 12), 3=-25 (LC 13)
Max Grav 1=83 (LC 1), 3=83 (LC 1), 4=120 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-56/38, 2-3=-53/38
BOT CHORD 1-4=-7/26, 3-4=-7/26
WEBS 2-4=-81/51

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 1 and 25 lb uplift at joint 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



October 6, 2023

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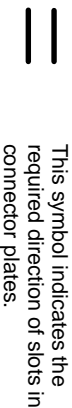
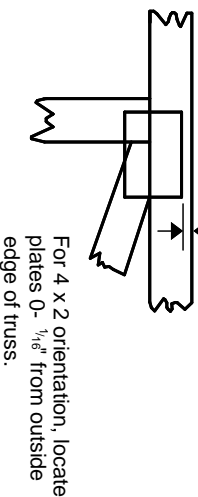
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

MiTek®

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Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

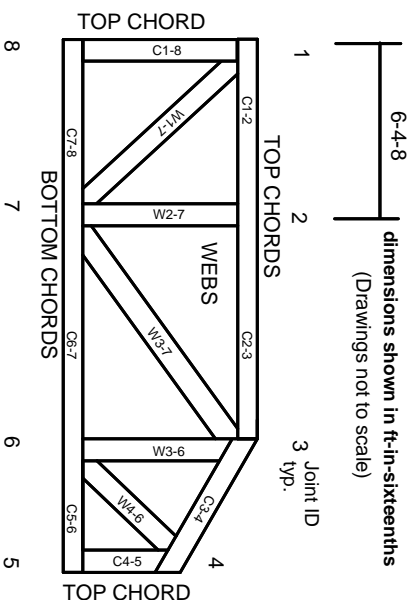


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3. These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MIL-7473 rev. 1/2/2023

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.