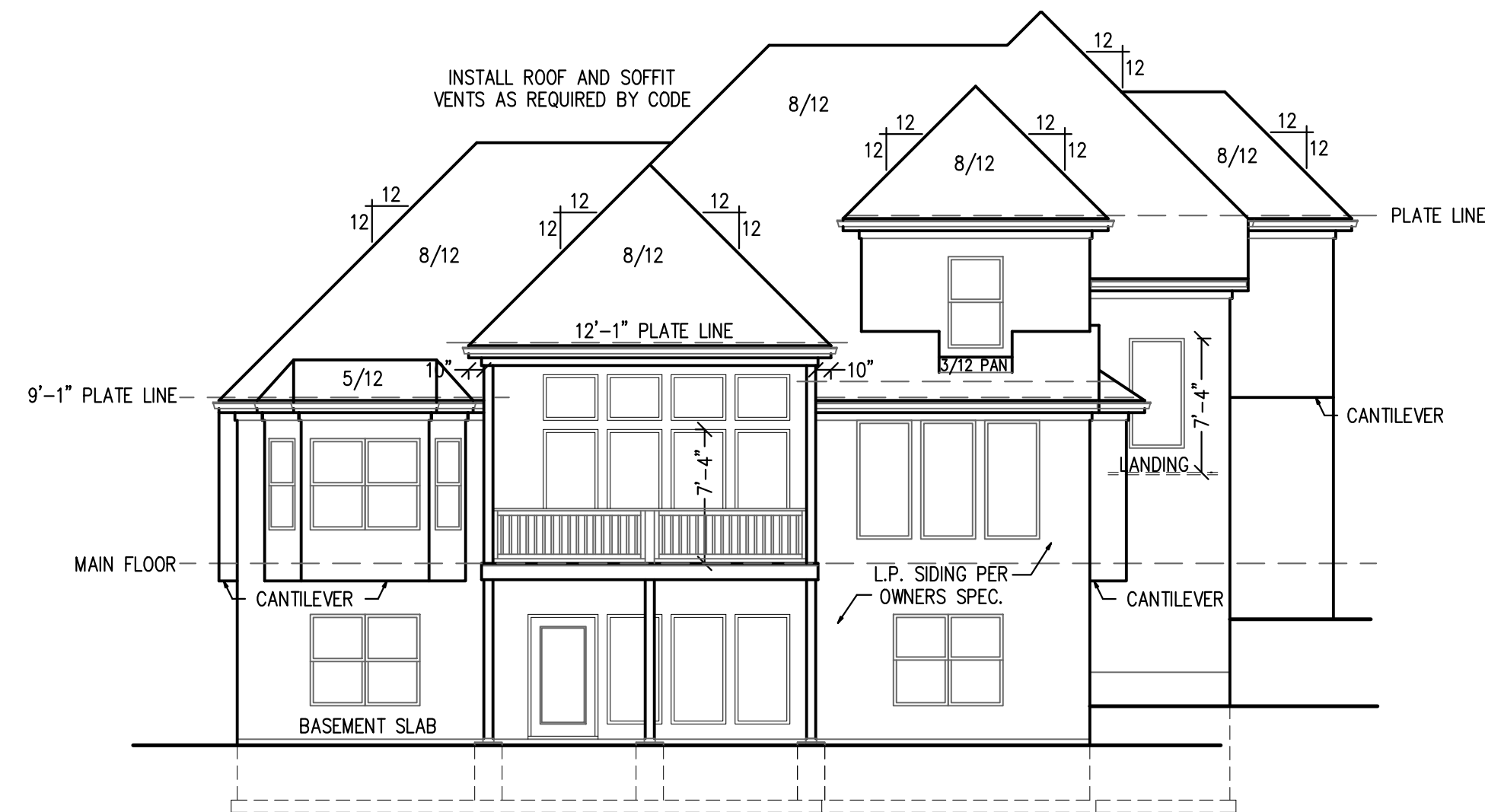


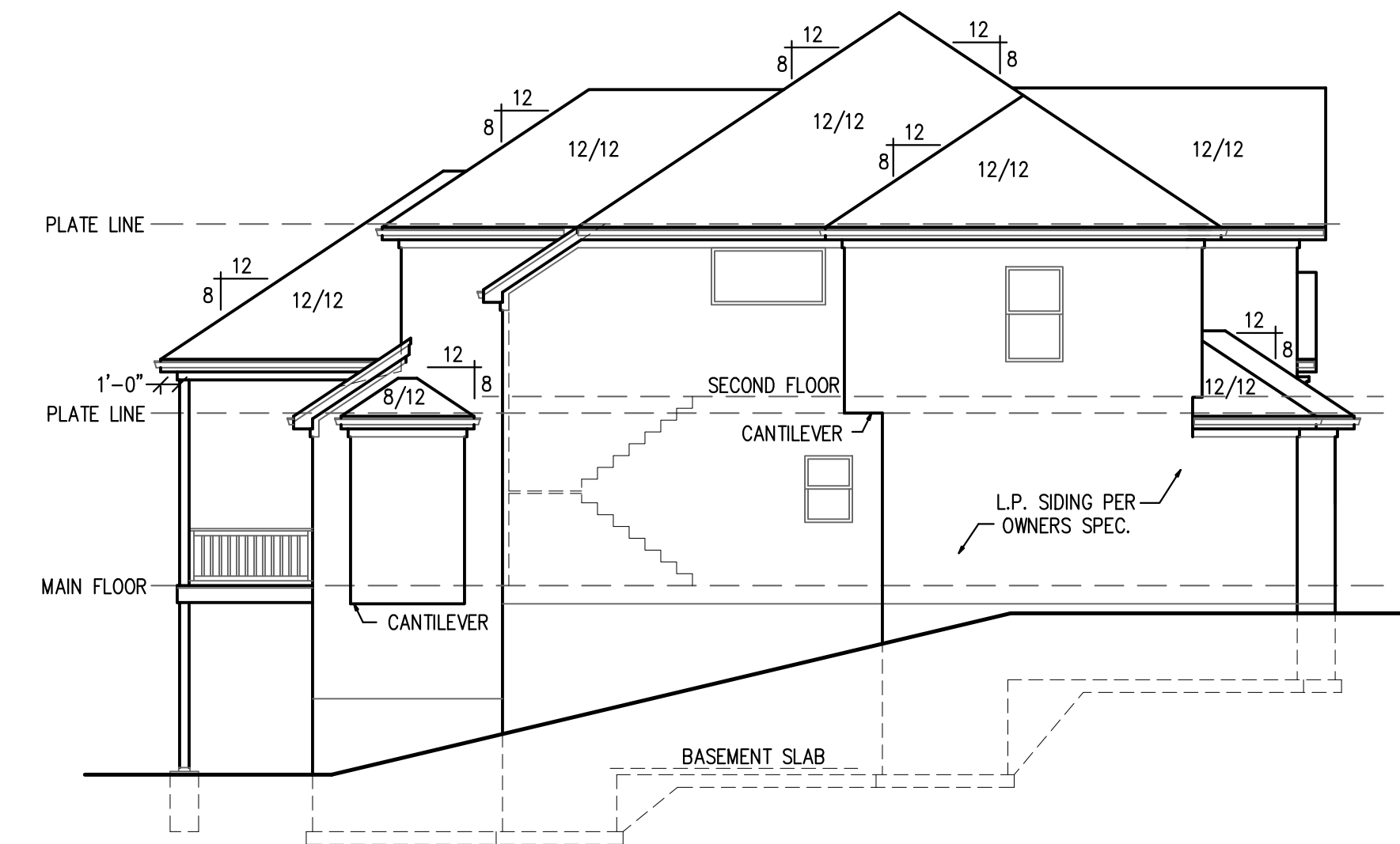
RIGHT ELEVATION

SCALE: 1/8" = 1'-0"



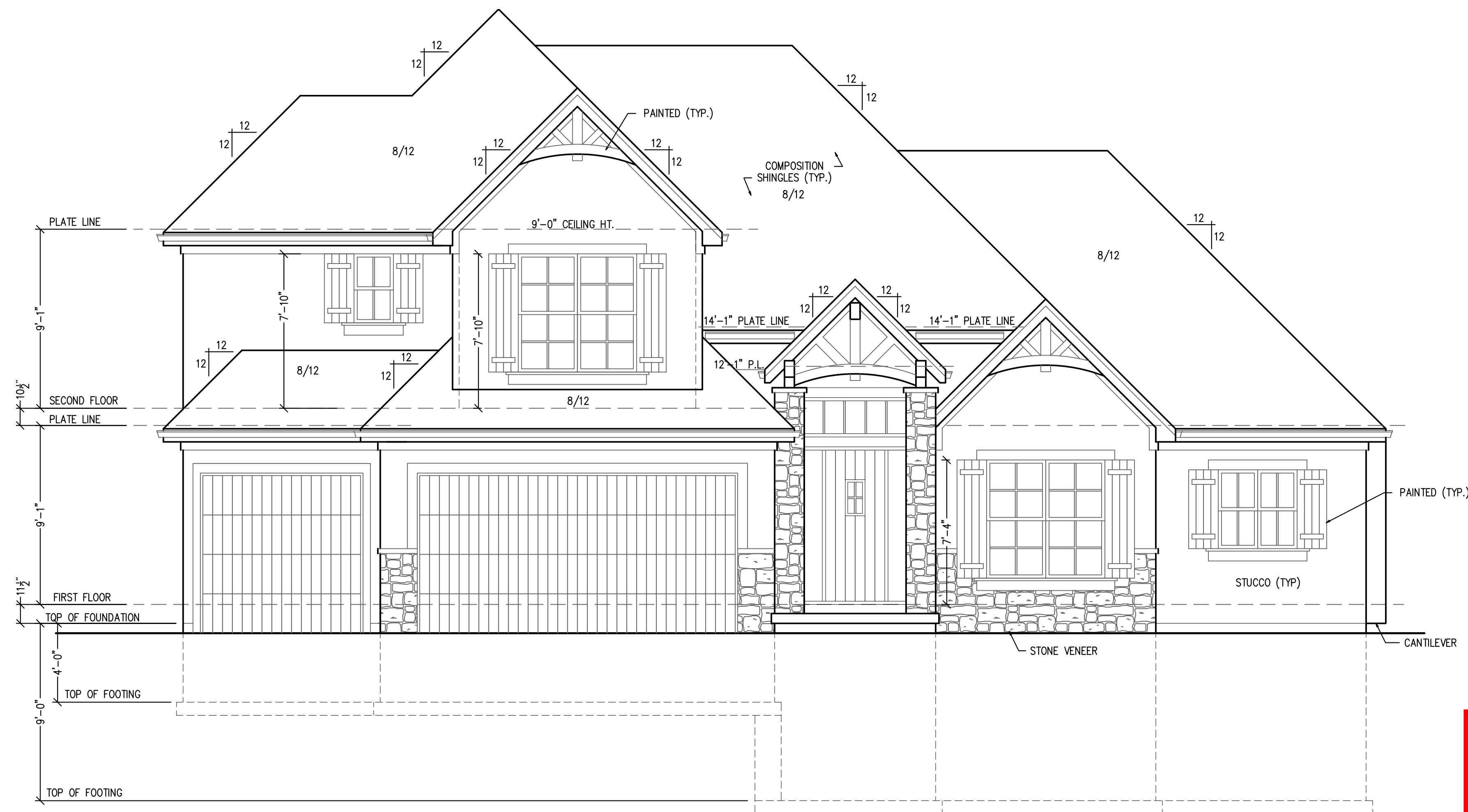
REAR ELEVATION

SCALE: 1/8" = 1'-0"



LEFT ELEVATION

SCALE: 1/8" = 1'-0"



FRONT ELEVATION

SCALE: 1/4" = 1'-0"

MAIN FLOOR - 1,980 SQ. FT.
SECOND FLOOR - 977 SQ. FT.
LOWER LEVEL - 25 SQ. FT.
TOTAL 2,982 SQ. FT.

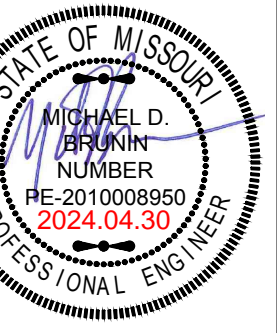
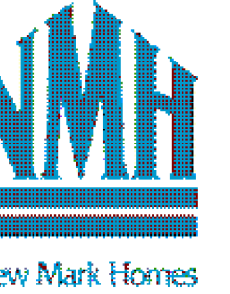
UNFINISHED - 1,745 SQ. FT.
GARAGE - 644 SQ. FT.
DECK - 216 SQ. FT.

DISCLAIMER
ACTUAL PLANS AND ELEVATIONS MAY VARY FROM ARCHITECTURAL DRAWINGS. DUE TO TERRAIN/BACKFILL PROCESS. FRONT ELEVATIONS ARE ARCHITECTURAL DRAWINGS AND MAY VARY DUE TO MATERIAL AVAILABILITY.

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NOTE:
PLANS DESIGNED PER IRC AS ADOPTED BY GOVERNING JURISDICTION

New Mark Homes
P.O. Box 12025
Parkville, Missouri 64152
Ph. (816) 969-9010



APEX ENGINEERS, INC.
1628 LOCUST ST.
KANSAS CITY, MO 64108
816-421-3222

STRUCTURAL DESIGN REVIEW
KANSAS ENGINEERING LICENSE: 992
MISSOURI ENGINEERING LICENSE: 2003004673

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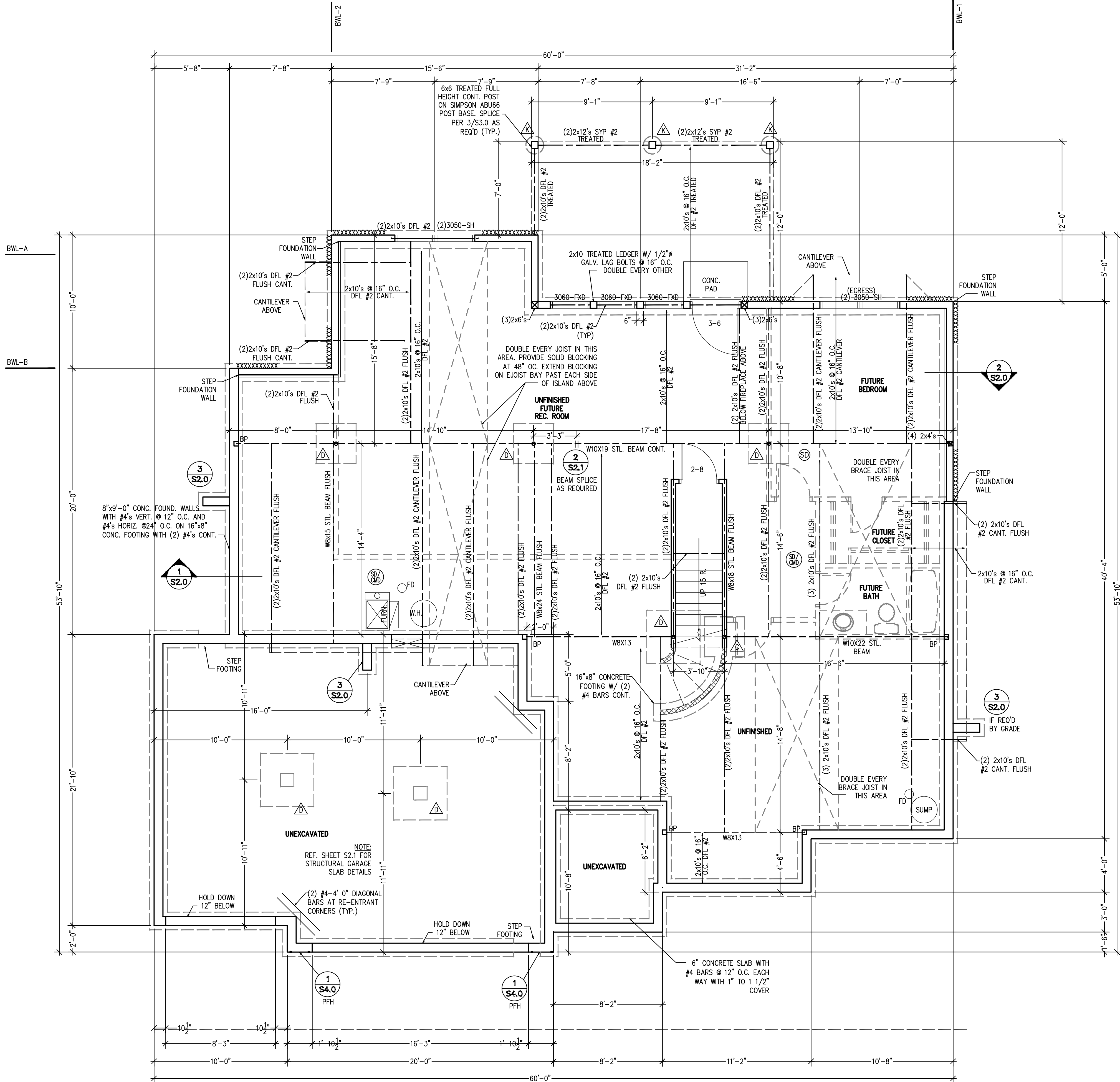
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DATE: 4/30/2024

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PROJ. 24-317



BASEMENT/FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

ALL WINDOWS SIZES ARE EXPRESSED
IN FEET AND INCHES TO THE UNIT
SIZE.

NOTE:
PLANS DESIGNED PER IRC AS
ADOPTED BY GOVERNING JURISDICTION

STRUCTURAL NOTES:
- ALL UNMARKED HEADERS MIN
(2)#2-2x10
- ALL HEADERS AND BEAMS MIN #2
GRADE DFL (OR EQ.)
- BEARING WALL.
- STRUCTURE NOTED AS FLUSH TO BE
FLUSH WITH SUB-FLOOR ABOVE.

BRACED WALL METHODOLOGY
CONTINUOUS EXTERIOR SHEATHING PER WSP METHOD (BELOW)
UNLESS OTHERWISE NOTED ON THE PLAN

XXXX EXTERIOR BRACED WALLS:

WSP METHOD: WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN
3/4" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" OC STUD SPACING WITH 6d
COMMON NAILS AT 6" OC EDGES AND 12" OC FIELD OR SHEATHING
THICKNESS NOT LESS THAN 7/8" WITH MINIMUM SPAN RATING OF 24/0 FOR 24"
OC SPACING WITH 8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN FIELD.
(NOTE: FRAMING MEMBERS 16" OC MAX UNBLOCKED, AND WITH SHEATHING APPLIED DIRECTLY TO FRAMING
MEMBERS)

IIII INTERIOR BRACED WALLS (REF 2-S4.0):

GB METHOD: 1/2" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX. FASTENED WITH
No 6 - 1/2" TYPE 'W' OR 'S' DRYWALL SCREWS AT 7" OC EDGES AND FIELD
(MIN. 4'-0" SECTION FOR BOTH SIDES.)

OR

LIB METHOD: 1x4 WOOD FASTENED WITH (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA.
TYPE WB (OR EQUAL) STL. X-BRACE(S) AT 45° TO 60° ANGLES, MAXIMUM 16"
O.C. STUD FASTENED PER MANUFACTURER'S SPECIFICATIONS.

COLUMN & PIER PAD SCHEDULE (REF. 5/S2.0)

COLUMN MARK	PAD SIZE	REINFORCEMENT	COLUMN SIZE
△	30" x 30" x 12"	(4) #4 BAR E.W.	3" SCH 40 (3.5" OD)
△	36" x 36" x 12"	(4) #4 BAR E.W.	3" SCH 40 (3.5" OD)
△	42" x 42" x 12"	(5) #4 BAR E.W.	3" SCH 40 (3.5" OD)
△	48" x 48" x 12"	(6) #4 BAR E.W.	3 1/2" SCH 40 (4" OD)
△	54" x 54" x 16"	(8) #4 BAR E.W.	REF PLAN
△	60" x 60" x 16"	(10) #4 BAR E.W.	REF PLAN

- COLUMN & PAD SIZES SHOWN ARE FOR MAXIMUM ADJUSTABLE COLUMN HEIGHT
OF 9'-1". REQUIRES SEPARATE ENGR'D DESIGN IF GREATER THAN 9'-1" TALL.
COLUMNS SIZED AS QWIK-ADJUST COLUMN, BY QUALITY WAY PRODUCTS, LLC.
REFER TO SAFE LOADING CAPACITIES PER MANUF. SPECS, OR SUBSTITUTION TO
ANOTHER PRODUCT ONLY WITH PRIOR APPROVAL BY APEX ENGINEERS.
- COLUMN & PIER PAD SIZES SHOWN ARE BASED ON AN ASSUMED MINIMUM
ALLOWABLE SOIL BEARING CAPACITY OF 2,000PSF.

COLUMN & PIER SCHEDULE

MARK	COLUMN SIZE	PIER DIA.
△	6x6	12"
△	6x6	16"
△	6x6	18"
△	6x6	24"
△	6x6	28"

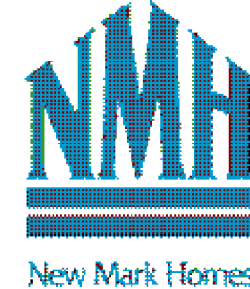
- ALL PIERS TO BEAR ON ORIGINAL
UNDISTURBED SOIL OF 2,000 PSF BEARING
CAPACITY OR FILL COMPACTED AND TESTED
TO CONFORM TO THE RECOMMENDATIONS
OF A GEOTECHNICAL ENGINEER.
- PIERS SHALL EXTEND BELOW THE FROST
LINE: MIN. DEPTH OF 36" BELOW GRADE.
- POST SHALL BE TREATED OR CEDAR WITH
SIMPSON ABU66 POST BASE

DETAIL REFERENCES

- | | | | |
|-----------|--|-----------|--|
| 1
S2.0 | TYPICAL FOUNDATION WALL DETAIL | 2
S2.1 | STRUCTURAL GARAGE SLAB
PIER PAD DETAIL |
| 2
S2.0 | TYPICAL "UNRESTRAINED"
FOUNDATION WALL DETAIL | 3
S2.1 | STRUCTURAL GARAGE SLAB /
WALL SECTION |
| 3
S2.0 | TYPICAL DEAD MAN DETAIL | 6
S2.1 | TYPICAL OVERDIG DETAIL AT
BASEMENT SLAB |
| 4
S2.0 | FOUNDATION WALL JUMP DETAIL | 1
S4.0 | ALTERNATE BRACED WALL PANEL
DETAIL |
| 5
S2.0 | COLUMN PAD DETAIL | 1
S4.0 | APA NARROW WALL BRACING
METHOD WITHOUT HOLD-DOWNS
ALT. |
| 1
S2.1 | TYPICAL STRUCTURAL GARAGE
SLAB PLAN | △ | COLUMN AND PIER PAD SCHEDULE
(SHEET S2.0) |

ADVANCE SOILS DISCLOSURE:
THESE PLANS HAVE BEEN PREPARED BASED ON A PRESUMPTIVE ALLOWABLE BEARING CAPACITY AS ALLOWED
BY IRC CODE AND THE LOCAL ENGINEERING JURISDICTION.
APEX ENGINEERS, INC. (APEX) RECOMMENDS THAT ALL FOOTING EXCAVATIONS BE EVALUATED BY A LICENSED
GEOTECHNICAL ENGINEER PRIOR TO THE PLACEMENT OF ANY FOUNDATION ELEMENTS. HYDROLOGICAL,
INVESTIGATION AND/OR TESTING IS NOT A SERVICE PROVIDED OR OFFERED BY APEX.
APEX HAS NOT BEEN RETAINED TO DETERMINE THE EXISTING SOIL CHARACTERISTICS OF THE SUBGRADE SOIL,
AND THEREFORE CANNOT BE HELD RESPONSIBLE FOR THE VOLUNTARY CHANGES OF THE SOIL, INCLUDING
BELOW THE BASEMENT SLAB, OR USE OF SUCH PLANS WITHOUT AN APPROPRIATE GEOTECHNICAL
INVESTIGATION REPORT. APEX WILL NOT BE HELD RESPONSIBLE FOR THE VOLUNTARY AND/OR PERMANENT
MOVEMENT OF THE PROPOSED STRUCTURE AND THE POSSIBLE DAMAGE THAT MAY BE CAUSED AS A RESULT OF
SOIL BEING MOVED. THE FOLLOWING BASEMENT SLAB BEARING, SHEET PILE, AND/OR SOIL NAIL, AND/OR SOIL NAIL
BEARING, THE FOLLOWING BASEMENT SLAB BEARING, SHEET PILE, AND/OR SOIL NAIL, AND/OR SOIL NAIL, AND/OR SOIL NAIL
OF PILES AND STICKING ANCHOR NOT OFFERED, DAMAGE TO TILE, MOLDING, AND OTHER COSMETIC FINISHES.

New Mark Homes
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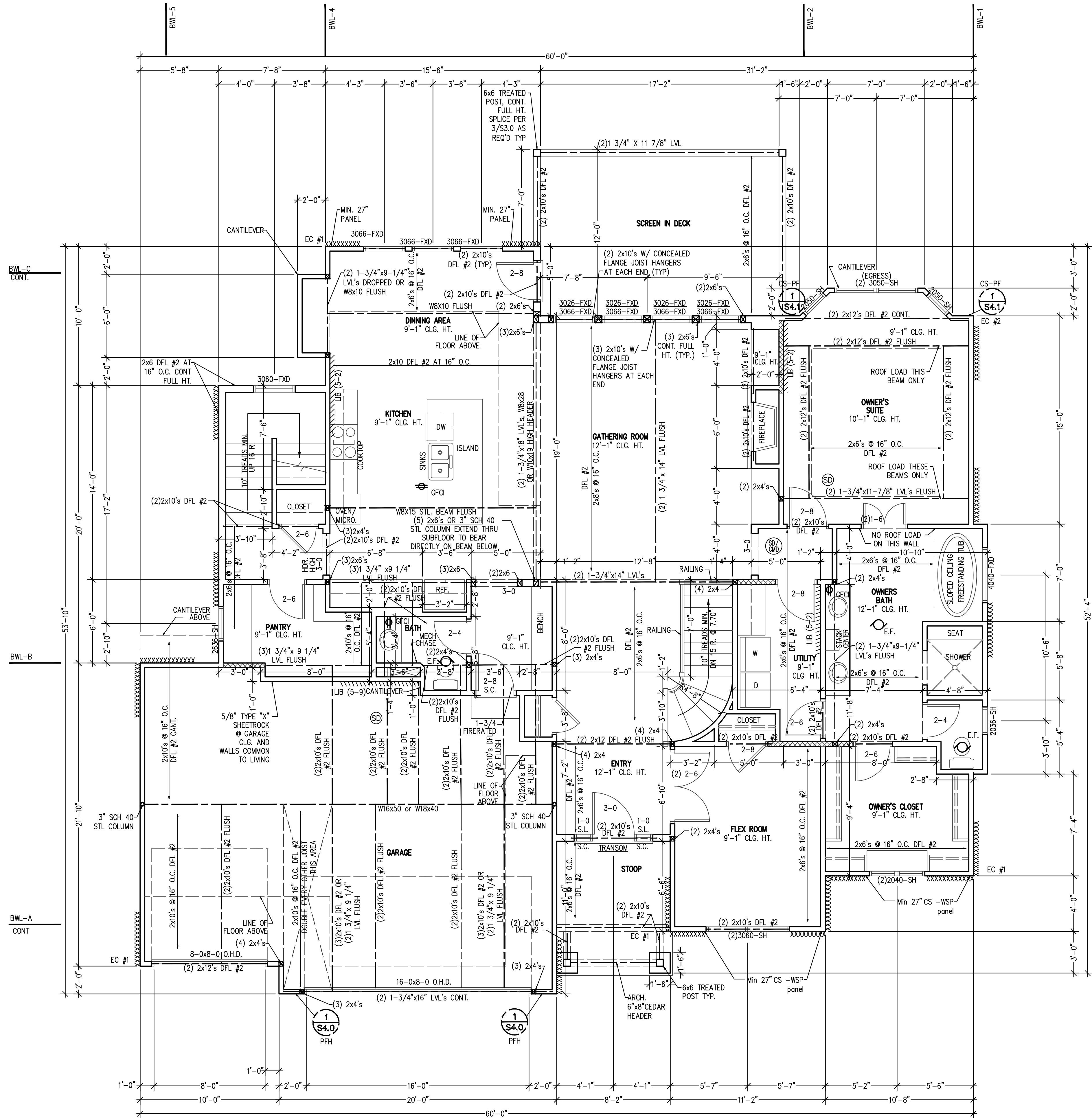


APEX ENGINEERS, INC.
6025 CLOUT ST.
KANSAS CITY, MO 64108
816-421-3222
STRUCTURAL DESIGN REVIEW
KANSAS ENGINEERING LICENSE:
692
MISSOURI ENGINEERING LICENSE:
2003004673

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MAIN FLOOR PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL NOTES:
- ALL UNMARKED HEADERS MIN (2)#2-2x10
- ALL HEADERS AND BEAMS MIN #2 GRADE DFL (OR EQ.)
- = BEARING WALL
- STRUCTURE NOTED AS FLUSH TO BE FLUSH WITH SUB-FLOOR ABOVE.

BRACED WALL METHODOLOGY
CONTINUOUS EXTERIOR SHEATHING PER WSP METHOD (BELOW)
UNLESS OTHERWISE NOTED ON THE PLAN

XXXX EXTERIOR BRACED WALLS:

WSP METHOD: WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 5/8" WITH MINIMUM SPAN RATING OF 240 FOR 16" OC STUD SPACING WITH 6d COMMON NAILS AT 6" OC EDGES AND 12" OC FIELD OR SHEATHING THICKNESS NOT LESS THAN 3/4" WITH MINIMUM SPAN RATING OF 240 FOR 24" OC SPACING WITH 8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN FIELD.
(NOTE: FRAMING MEMBERS 16" OC MAX UNBLOCKED, AND WITH SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS)

//// INTERIOR BRACED WALLS (REF 2-S4.0):

GB METHOD: 1/2" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX. FASTENED WITH No 6 - 1 1/2" TYPE 'W' OR 'S' DRYWALL SCREWS AT 7" OC EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES.)

OR

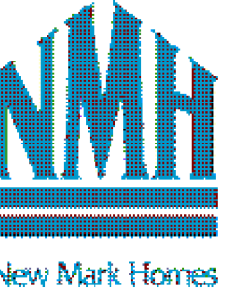
LJB METHOD: 1x4 WOOD FASTENED WITH (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUAL) STL. X-BRACE(S) AT 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUD FASTENED PER MANUFACTURER'S SPECIFICATIONS.

*ALL WINDOWS TO HAVE U = 0.4 OR LESS.

ALL WINDOWS SIZES ARE EXPRESSED IN FEET AND INCHES TO THE UNIT SIZE.

NOTE:
PLANS DESIGNED PER IRC AS
ADOPTED BY GOVERNING JURISDICTION

New Mark Homes
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Parkville, Missouri 64152
Ph. (816) 969-9010



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6025 LOUIST ST
KANSAS CITY, MO 64108
816-421-3222
STRUCTURAL DESIGN REVIEW
KANSAS ENGINEERING LICENSE: 992
MISSOURI ENGINEERING LICENSE: 2003004673

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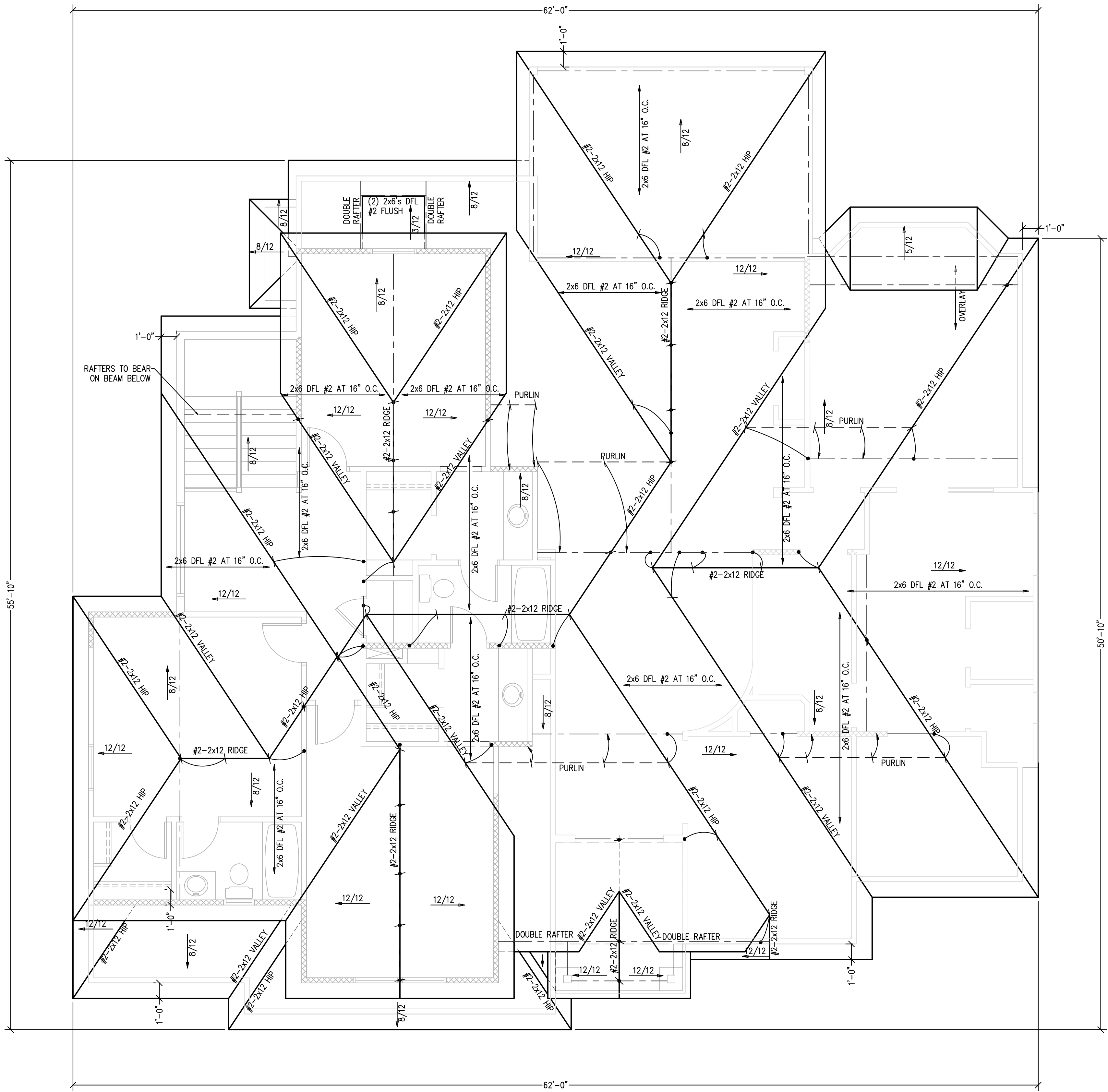
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LIB METHOD: 1x4 WOOD FASTENED WITH (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUAL) STL. X-BRACE(S) AT 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUD FASTENED PER MANUFACTURER'S SPECIFICATIONS.

NOTE:
PLANS DESIGNED PER IRC AS
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ROOF PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL NOTES:
- ALL UNMARKED HEADERS MIN (2)W2-2x10
- ALL HEADERS AND BEAMS MIN #2 GRADE DFL (OR EQ.)
- [XXXXXX] = BEARING WALL
- STRUCTURE NOTED AS FLUSH TO BE FLUSH WITH SUB-FLOOR ABOVE.

ROOF FRAMING NOTES

ROOF DESIGNED FOR LIGHT ROOF COVERING
30psf TOTAL LOAD [10psf DL, 20psf LL (SL)]

ROOF SYSTEM IS DESIGNED TO MEET REQUIREMENTS OF IRC 802

*RAFTERS (HEM-FIR, DOUG-FIR, OR EQUAL):
SEE SPAN CHARTS BELOW

CODE MINIMUM

RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN
#2-2x6	AT 24" OC	11'-7"
#2-2x6	AT 16" OC	14'-2"
#2-2x8	AT 24" OC	14'-8"
#2-2x8	AT 16" OC	17'-11"
#2-2x10	AT 24" OC	17'-10"
#2-2x10	AT 16" OC	21'-11"

NOTE: CODE MINIMUM ALLOWS FOR A RAFTER DEFLECTION OF L/180 TOTAL LOAD

HIGHER PERFORMANCE

RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN
#2-2x6	AT 24" OC	8'-6"
#2-2x6	AT 16" OC	9'-9"
#2-2x8	AT 24" OC	11'-3"
#2-2x8	AT 16" OC	12'-9"
#2-2x10	AT 24" OC	14'-3"
#2-2x10	AT 16" OC	16'-3"

APEX ENGINEERS, INC. RECOMMENDED
DEFLECTION = L/360 LIVE LOAD, L/240 TOTAL LOAD

*RIDGE BOARDS ARE (UNLESS OTHERWISE NOTED)

#2-2x10 UP TO 9:12 PITCH

#2-2x12 OVER 9:12 PITCH

*ALL HIP AND VALLEYS ARE (UNLESS OTHERWISE NOTED)

#2-2x10 UP TO 9:12 PITCH

#2-2x12 OVER 9:12 PITCH

#2-2x10 HIP/VALLEY, MAX CLEAR SPAN: 11'-11"

#2-2x12 HIP VALLEY, MAX HORIZ. CLEAR SPAN: 12'-11"

*PURLINS ARE 2x6 MIN

- PURLIN STRUTS ARE AT 4'-0" OC

- PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS

THAN A 45 DEGREE ANGLE WITH THE HORIZONTAL

- ALL PURLIN STRUTS SHALL HAVE A MAX UNBRACED

LENGTH OF 8'-0"

- PURLIN STRUTS SHALL BE CONSTRUCTED IN A "T"

CONFIGURATION AND PER THE FOLLOWING CHART:

PURLIN STRUT	MAX PURLIN STRUT LENGTH
(2)2x4	8'-0"
(1)2x4 AND (1)2x6	12'-0"
(1)2x6 AND (1)2x8	20'-0"
(2)2x6 AND (1)2x8	30'-0"
CONSULT ARCH ENGR	>30'-0"

*EACH END OF STRUT SHALL BE FASTENED WITH MIN (3)8d

OR (2)16d NAILS

*RIDGE BRACES ARE SAME AS PURLIN BRACES-SPACING,

SIZE, CONFIGURATION, AND INSTALLATION (SEE PURLIN

BRACE NOTES ABOVE)

*HIP AND VALLEY BRACES ARE THE SAME AS PURLINS SIZE,

CONFIGURATION, AND INSTALLATION (SEE PURLIN BRACE

NOTES ABOVE)

= ROOF BRACE/STRUT (PER CHART)

-SLASH IS TOP END OF BRACE

-CIRCLE IS BOTTOM END OF BRACE

= PURLIN STRUTS AT 48" OC (PER CHART) U.N.O.

-SLASH IS TOP END OF BRACE

-ARROW IS BEARING LOCATION

[XXXXXX] DENOTES BEARING WALL

--- DENOTES PURLIN

--- DENOTES BEARING STRUCTURE

1. THIS IS AN ENGINEERED ROOF STRUCTURE DESIGNED FOR COMPLIANCE WITH IRC 802.3, BUILD AS SHOWN WITH NO DEVIATIONS.
2. ALL HIPs ARE DESIGNED TO BE CONTROLLED BY BENDING.
3. SHEAR AT BEARING WITH MIN 5 1/2" DEPTH DOES NOT CONTROL DESIGN. FOR VALLEYS REF 4/S3.2

NOTE:
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Parkville, Missouri 64152
Ph. (816) 969-9010



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STRUCTURAL DESIGN REVIEW

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CURTAINS PENETRATING THE THERMAL ENVELOPE SHALL BE ICE-
 GATED, AND SEALED TO THE GYPSUM WALLBOARD AS
 PER M1102.4.5
 THERMOSTATS SHALL BE INSTALLED AS REQUIRED PER
 M1102.4.5
 SHALL BE RATED FOR MAXIMUM 2% AIR LEAKAGE RATE PER
 M1103.3.5
 CAVITIES SHALL NOT BE USED AS DUCTS OR
 PER M1103.3.5
 PIPES SHALL BE INSULATED AS REQUIRED PER M1103.4.
 FANS SHALL TERMINATE TO THE BUILDING EXTERIOR AS
 PER M1601.6
 SYSTEMS SHALL BE INSTALLED FOR KITCHEN EXHAUST
 EXCEED 400 CFM AS REQUIRED PER M1503.6.
 SYSTEMS SHALL NOT SERVE BOTH THE LIVING SPACE AND
 PER M1601.6.

1. PROVIDE ONE WINDOW FROM EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 24 INCHES AND WIDTH OF 20 INCHES.
2. SMOKE ALARMS SHALL MEET THE REQUIREMENTS OF IRC SECTION 310.
3. SMOKE ALARMS SHALL BE INSTALLED AS REQUIRED PER IRC 2018 SECTION R314.
4. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA, ON EACH FLOOR INCLUDING BASEMENTS AND HABITABLE ATTICS, AND NOT LESS THAN 3'-0" HORIZONTALLY FROM DOOR OR OPENING OF A BATHROOM THAT CONTAINS A BATHTUB OR SHOWER. ALARMS SHALL BE INTERCONNECTED SUCH THAT IF SUCH A BATHROOM IS THE LOCATION OF ONE ALARM WILL ACTIVATE ALL OF THE ALARMS IN THE DWELLING.
5. CARBON MONOXIDE ALARMS SHALL BE INSTALLED AS REQUIRED PER IRC 2018 SECTION R315.
6. CARBON MONOXIDE ALARMS SHALL BE INSTALLED OUTSIDE OF EACH SLEEPING AREA, WHERE A FUEL-BURNING APPLIANCE IS LOCATED WITHIN A BEDROOM OR ITS ATTACHED BATHROOM. A CARBON MONOXIDE ALARM SHALL BE INSTALLED WITHIN THE BEDROOM.

1. PLANS SHALL COMPLY WITH THE 2018 INTERNATIONAL RESIDENTIAL CODE WITH AMENDMENTS AS ADOPTED BY THE GOVERNING JURISDICTION. IF ANY CHANGES OR DEVIATIONS FROM THE PLANS ARE MADE DURING CONSTRUCTION, CONTRACTOR SHALL NOTIFY THE APPROPRIATE AUTHORITY AND ENGINEER OF RECORD, EITHER BEFORE OR AFTER THE WORK, FOR WHOM MAY REQUIRE REVISED DRAWINGS OR CALCULATIONS AT ITS DISCRETION.

2. REPRODUCTION, ALTERATION, OR RE-USE BY ANY METHOD OF ALL OR PORTIONS OF THESE STRUCTURAL PLANS OR VARIATIONS THEREOF WITHOUT WRITTEN PERMISSION FROM APEX ENGINEERS, INC. IS STRICTLY PROHIBITED. THE DRAWINGS AND DETAILS OF THIS SHEET SET, BEING INSTRUMENTS OF SERVICE, ARE THE PROPERTY OF APEX ENGINEERS, INC. ANY REPRODUCTION, ALTERATION, REVISION, OR A VERSION VOID OF APEX ENGINEER'S LOGO AND/OR TITLE BLOCK, SHALL BE CONSIDERED AN UNAUTHORIZED REPRODUCTION.

3. WHERE DISCREPANCIES EXIST BETWEEN THE STANDARD COMMENTS, NOTES FROM THE DESIGN PROFESSIONAL OR THE CODE, THE MOST RESTRICTIVE SHALL APPLY. THE DWELLING SHALL COMPLY WITH THE FOLLOWING LOAD CONDITIONS:

NOTE: HEAVY ROOF COVERING WILL NOT BE INSTALLED OR USED IN THE DESIGN CALCULATIONS UNLESS IT IS SPECIFICALLY NOTED ON THE PLANS THAT THE DESIGN IS FOR HEAVY ROOF COVERINGS.

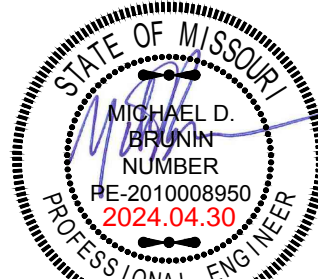
1. THE FOUNDATION DESIGN SHALL BE BASED ON A MINIMUM SOIL BEARING CAPACITY OF 2000 PSF, UNLESS OTHERWISE INDICATED ON THE PLANS OR IF MORE FAVORABLE SOIL ENGINEERING DATA BASED ON ACTUAL SITE CONDITIONS.
2. REFERENCE CONCRETE SCHEDULE 1 ON THIS SHEET, FOR APPLICABLE FOUNDATION CONCRETE MIX DESIGNS.
3. FOOTINGS SHALL EXTEND BELOW THE FROST LINE; MINIMUM DEPTH 36 INCHES BELOW GRADE.
4. UNLESS OTHERWISE NOTED ON THE PLANS OR IF SITE CONDITIONS REQUIRE OTHERWISE, FOOTINGS SHALL BE A MINIMUM OF 16" WIDE AND 8" DEEP WITH (2) #4 BARS CONTINUOUS.
5. COLUMN PADS SHALL BE A MINIMUM 30"x30"x12" WITH (4) #4 BARS EACH WAY UNLESS OTHERWISE NOTED.
6. UNLESS NOTED OTHERWISE ON THE PLANS, FOUNDATION WALLS SHALL BE MINIMUM 8" THICK X 8'-0" (OR 9'-0") TALL AND REINFORCED PER DETAIL 1-S2.0 (AND 2-S2.0 WHERE APPLICABLE); FOUNDATION WALLS GREATER THAN 10'-0" TALL REQUIRE A SEPARATE ENGINEERING DESIGN. PROVIDE A 2'-0" LONG INTERIOR EXTERIOR DEAD AIR SPACE ON EITHER STRAIGHT WALL PANELS EXCEEDING 20'-0" IN LENGTH (REF 3-S2.0).
7. REINFORCEMENT SHALL BE MINIMUM GRADE 40 UNLESS NOTED OTHERWISE; REINFORCEMENT SHALL LAP A MINIMUM OF 24" AT ENDS, SPLICES, AND AROUND CORNERS.
8. FOUNDATION WALLS SHALL BE BACKFILLED WITH A CLEAN LEAN CLAY (OR BETTER) LOW VOLUME CHANGE MATERIAL. ON-SITE MATERIAL MAY BE USED IF DEEMED ACCEPTABLE BY THE GEOTECHNICAL ENGINEER OF RECORD.
9. FOUNDATION WALLS WILL NOT ACHIEVE FULL STRENGTH UNTIL THE BASEMENT SLAB AND THE FIRST FLOOR DECK HAVE BEEN PROPERLY PLACED.
10. BACKFILL TO THE EXTERIOR OF THE FOUNDATION SHALL BE GREATER THAN 8" BARS EITHER 12" OR 24" OF GRANULAR FILL, A STRUCTURAL BASEMENT SLAB, OR ALTERNATE ENGINEERING SOLUTION (i.e. ENGINEERED FILL) WILL BE REQUIRED.

EXPANSIVE SOILS DISCLAIMER:

THESE PLANS HAVE BEEN PREPARED BASED ON A PRESUMPTIVE ALLOWABLE BEARING CAPACITY AS ALLOWED BY IRC CODE AND THE LOCAL ENFORCING JURISDICTION.

APEX ENGINEERS, INC. (APEX) RECOMMENDS THAT ALL FOOTING EXCAVATIONS BE EVALUATED BY A LICENSED GEOTECHNICAL ENGINEER PRIOR TO THE REPLACEMENT OF ANY FOUNDATION ELEMENTS. GEOTECHNICAL INVESTIGATION AND/OR TESTING IS NOT A SERVICE PROVIDED OR OFFERED BY APEX.

APEX HAS NOT BEEN RETAINED TO DETERMINE THE EXPANSIVE SOIL CHARACTERISTICS OF THE SUBGRADE SOIL AND THEREFORE CANNOT BE HELD RESPONSIBLE FOR THE VOLUMETRIC CHANGES OF THE SOIL (INCLUDING BELOW THE BASEMENT SLAB). BY USE OF THESE PLANS WITHOUT AN ACCOMPANYING GEOTECHNICAL ENGINEERING REPORT, APEX SHALL NOT BE HELD LIABLE FOR ANY FUTURE MOVEMENT AND/OR DIFFERENTIAL MOVEMENT OF THE PROPOSED STRUCTURE AND THE POSSIBLE DAMAGE THAT MAY BE CAUSED AS A RESULT OF SUCH MOVEMENT. DAMAGE FROM EXPANSIVE SOILS AND/OR SETTLEMENT CAN RESULT IN AMONGST OTHER THINGS, THE FOLLOWING: BASEMENT SLAB HEAVE, SHEETROCK CRACKS, WINDOWS AND DOOR BECOMING OUT OF PLUMB AND STICKING AND/OR NOT OPENING, DAMAGE TO TILE, MOULDING, AND OTHER COSMETIC FINISHES.



STRUCTURAL DESIGN REVIEW

KANSAS ENGINEERING LICENSE:
E-992

MISSOURI ENGINEERING LICENSE:
2003004673

THE ENERGY EFFICIENCY OF THE DWELLING SHALL COMPLY WITH THE FOLLOWING TABLE(S) (WHERE THERE ARE DISCREPANCIES BETWEEN THIS TABLE AND THE PLANS, THE MOST RESTRICTIVE SHALL APPLY). IF TABLE 1 IS NOT COMPLETED AND ACCOMPANIED BY RESCHECK CALCULATIONS, THEN TABLE 2 SHALL BE APPLIED.

NOTE: FOR USE OF TABLE 1 A ResCheck COMPLIANCE FORM MUST BE SUBMITTED WITH PLANS.

NOTE:
TABLE 2 PER IRC TABLE N1102.1.2
*DEFAULT U-FACTOR FOR DOUBLE PANE, ARGON FILLED LOW-E
TREATMENT IS U=0.35
**LIMITED TO AREAS LESS THAN 500 SQ-FT OR 20% OF CEILING AREA.

1. THE ARCHITECT OR ENGINEER OF RECORD SHALL LIST THE DEFERRED SUBMITTALS ON THE PLANS FOR REVIEW BY THE BUILDING OFFICIAL. THE DEFERRED SUBMITTALS SHALL BE IDENTIFIED BY THE ARCHITECT OR ENGINEER OF RECORD WHO SHALL REVIEW THEM AND FORWARD THEM TO THE BUILDING OFFICIAL WITH A NOTATION INDICATING THE DEFERRED SUBMITTALS. THE DEFERRED SUBMITTALS SHALL BE FOUND TO BE IN THE GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL. THE DEFERRED SUBMITTALS SHALL BE IDENTIFIED BY NOTATIONS OF THE DESIGN THAT ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION AND THAT ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITH A SPECIFIED DATE OF SUBMISSION. THE DEFERRED SUBMITTALS SHALL HAVE THE PRIORITY OF THE BUILDING OFFICIAL.

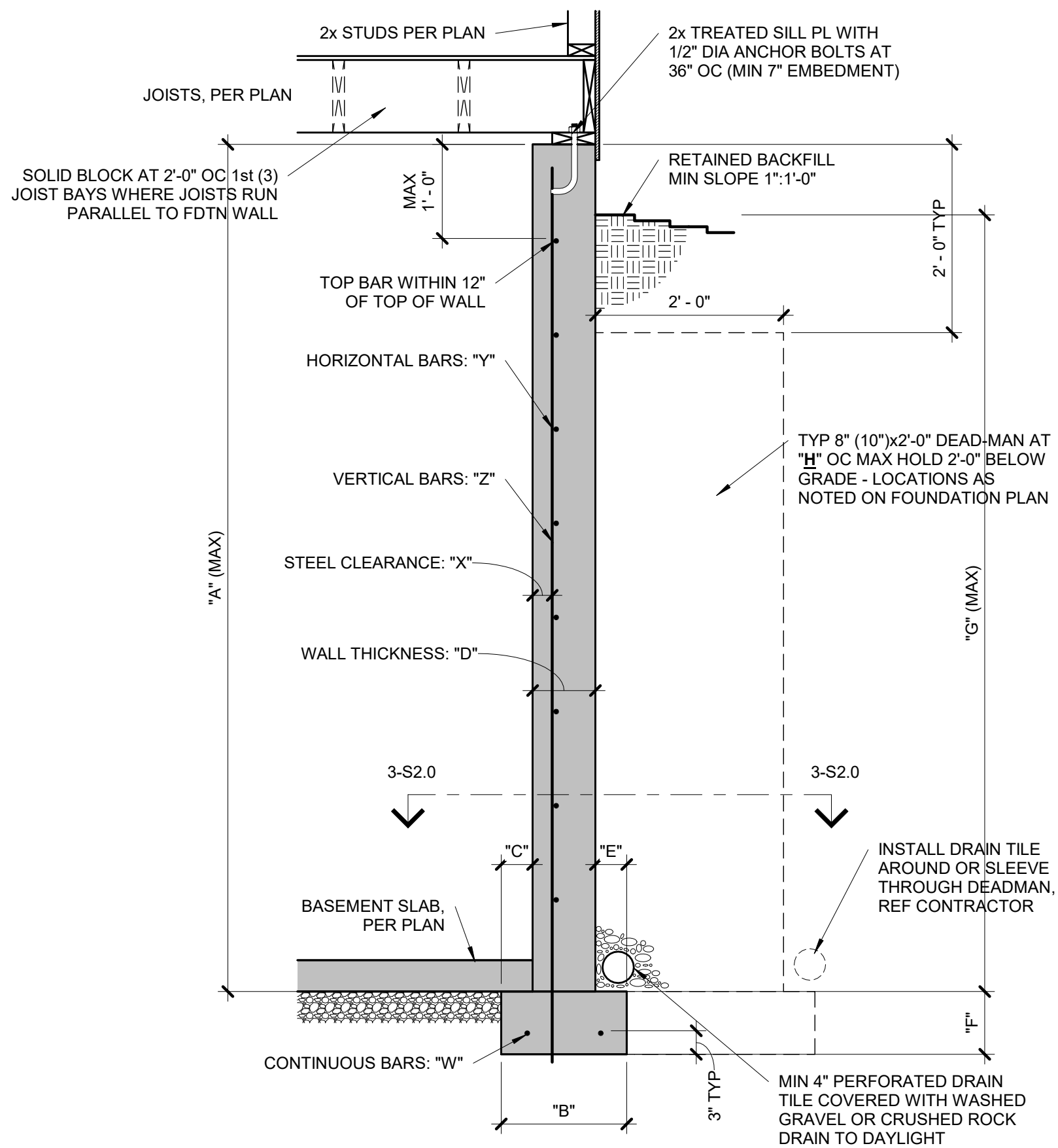
GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS: GLASS IN STORM DOORS; INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 24" ARCH OF THE DOOR IN A CLOSED POSITION AND WHOSE BOTTOM EDGE IS WITHIN 60" OF THE FLOOR; WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 60" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPools; GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING 9 SQUARE FEET AND WHOSE BOTTOM EDGE IS LESS THAN 18" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 36".

MINIMUM STRUCTURAL CONCRETE COVER					COVER
FORMED SURFACES EXPOSED TO GROUND OR WEATHER					2"
UNFORMED SURFACE IN CONTACT WITH THE GROUND					3"
WALLS AND SLABS NOT EXPOSED TO GROUND OR WEATHER					1"
INTERIOR BEAMS AND COLUMNS (TO TIES OF STIRRUPS)					1 1/2"
EPOXY GRouting APPLICATIONS					
THREADED ROD ANCHORS		HILTI HIT-HY 200 A OR SIMONS SET XP			
REINFORCING BARS		HILTI HIT-HY 200 R OR SIMONS SET XP			
CONCRETE USE	28 DAY STRENGTH	CEMENT TYPE	W/C RATIO	SUMP SLUMP (IN)	% AIR ENTRAINED
FOOTINGS/PIERS	3000 psi	N/A	0.55 (MAX)	5" (+/-1")	6% +/- 1%
FOUNDATION WALLS	3500 psi	N/A	0.50 (MAX)	4" (+/-1")	6% +/- 1%
INTERIOR SLABS	4000 psi	N/A	0.50 (MAX)	3" (+/-1")	3% MAX
SUSPENDED SLABS	4000 psi	N/A	0.50 (MAX)	3" (+/-1")	3% MAX

1. THE GARAGE FLOOR SHALL SLOPE TOWARDS THE GARAGE DOORWAYS.
2. DOORS BETWEEN THE GARAGE AND THE DWELLING - MINIMUM 1-3/8" SOLID CORE OR HONEY COMBED STEEL DOOR OR 20-MINUTE FIRE RATED.
3. THE GARAGE SHALL BE SEPARATED FROM THE RESIDENCE AND ITS ATTACHED GARAGE BY A MINIMUM 1-3/8" SOLID CORE OR HONEY COMBED STEEL DOOR FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION, APPLIED TO GARAGE SIDE, WHERE THE SEPARATION IS A FLOOR-CEILING ASSEMBLY, THE STRUCTURE SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED BY 5/8", TYPE X GYPSUM BOARD, OR MATERIALS APPROVED FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION OR EQUIVALENT, APPLIED TO THE GARAGE SIDE. PULL DOWN STAIRS LOCATED WITHIN GARAGE SHALL BE RATED TO BE ADEQUATELY PROTECTED BY MATERIALS APPROVED FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION. ATTIC ACCESS PANELS LOCATED WITHIN GARAGE SHALL BE OF 5/8", TYPE X GYPSUM BOARD, OR MATERIALS FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION.
4. GARAGE DOOR AND FRAME- THE H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL, JAMBS RUNNING FROM THE FLOOR TO CEILING ATTACHED WITH 1-3/4" X 4" X 8" MINIMUM 2X8 HEADER WITH (7)-1/4" X 0.120" NAILS THRU THE JAMB INTO THE HEADER, MINIMUM 2X8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.
5. SELF-CLOSING DEVICES SHALL BE INSTALLED FOR GARAGE AND/OR DWELLING SEPARATION DOORS PER R302.5.1.

1. STAIRWAYS SHALL PROVIDE A MAXIMUM 7'-3/4" RISE AND MINIMUM 10' RUN.
2. PROVIDE MINIMUM 36" GUARDRAILS ON THE OPEN SIDES OF RAISED FLOORS, PORCHES, AND BALCONIES; MINIMUM 34" GUARDRAILS ON THE OPEN SIDES OF STAIRWAYS LOCATED MORE THAN 30" ABOVE THE FLOOR OR GRADE BELOW. GUARDRAIL ENCLOSURES SHALL HAVE INTERMEDIATE RAILS OR ORNAMENTAL BALUSTRADES THAT DO NOT ALLOW PASSAGE OF A SPHERE 4" IN DIAMETER.
3. EACH STAIRWAY OF THREE OR MORE RISERS SHALL PROVIDE A CONTINUOUS HANDRAIL ON AT LEAST ONE SIDE BETWEEN 34" AND 38" ABOVE THE NOSING OF THE TREADS.
4. HANDRAILS SHALL HAVE A CIRCULAR CROSS SECTION OF 1-1/4" MINIMUM TO 2" MAXIMUM OR OTHER APPROVED GRASPABLE SHAPE PER IRC SECTION 311.7.6.5.
5. PROVIDE A MINIMUM 6'-8" OF HEADROOM CLEARANCE IN STAIRWAYS.
6. PROVIDE ACCESSIBLE SPACE UNDER STAIRWAYS SHALL HAVE WALLS AND THE UNDERSIDE OF THE STAIR AND LANDING PROTECTED WITH 1/2" GYPSUM BOARD ON ENCLOSURE SIDE PER IRC SECTION 302.7.
7. SPIRAL STAIRS TO BE CONSTRUCTED PER IRC SECTION 311.7.10.1.
8. SPACE STRINGERS AT 16" OC MAX.

LEE'S SUMMIT, MISSOURI
06/04/2024 3:58:51



CONCRETE DIMENSIONS

"A"	"B"	"C"	"D"	"E"	"F"	"G"	"H"
8'-0"	1'-4"	4"	8"	4"	8"	7'-6"	20'-0"
9'-0"	1'-4"	4"	8"	4"	8"	8'-6"	20'-0"
10'-0"	1'-8"	5"	10"	5"	10"	9'-6"	20'-0"

NOTES:

- DIMENSION SHOWN IS FOR MAXIMUM UNINTERRUPTED WALL PANEL LENGTH BEFORE A DEAD-MAN SHALL BE INSTALLED. NOTE: A MINIMUM 2'-0" RETURN OR OFFSET IN THE FOUNDATION WALL SHALL SUBSTITUTE AS A DEAD-MAN AND/OR BREAK IN THE WALL PANEL LENGTH.
- VERTICAL REINFORCING STEEL TO EXTEND TO WITHIN 8" OF TOP WALL. MINIMUM (1) #4 HORIZONTAL BAR WITHIN 12" OF TOP AND BOTTOM OF WALL.
- BURIED CONCRETE FOUNDATION WALLS UP TO 9'-0" TALL MAY BE 8" NOMINAL THICKNESS WITH #4 BARS AT 24" OC BOTH WAYS OVER 16"x8" CONCRETE FOOTINGS WITH (2) #4 BARS CONTINUOUS, UNLESS OTHERWISE REQUIRED BY ENGINEERING REPORT BASED ON ACTUAL SITE CONDITIONS.
- WALL WILL NOT ACHIEVE FULL STRENGTH UNTIL FIRST FLOOR DECK AND BASEMENT SLAB HAVE BEEN PLACED.

1 TYPICAL FOUNDATION WALL DETAIL

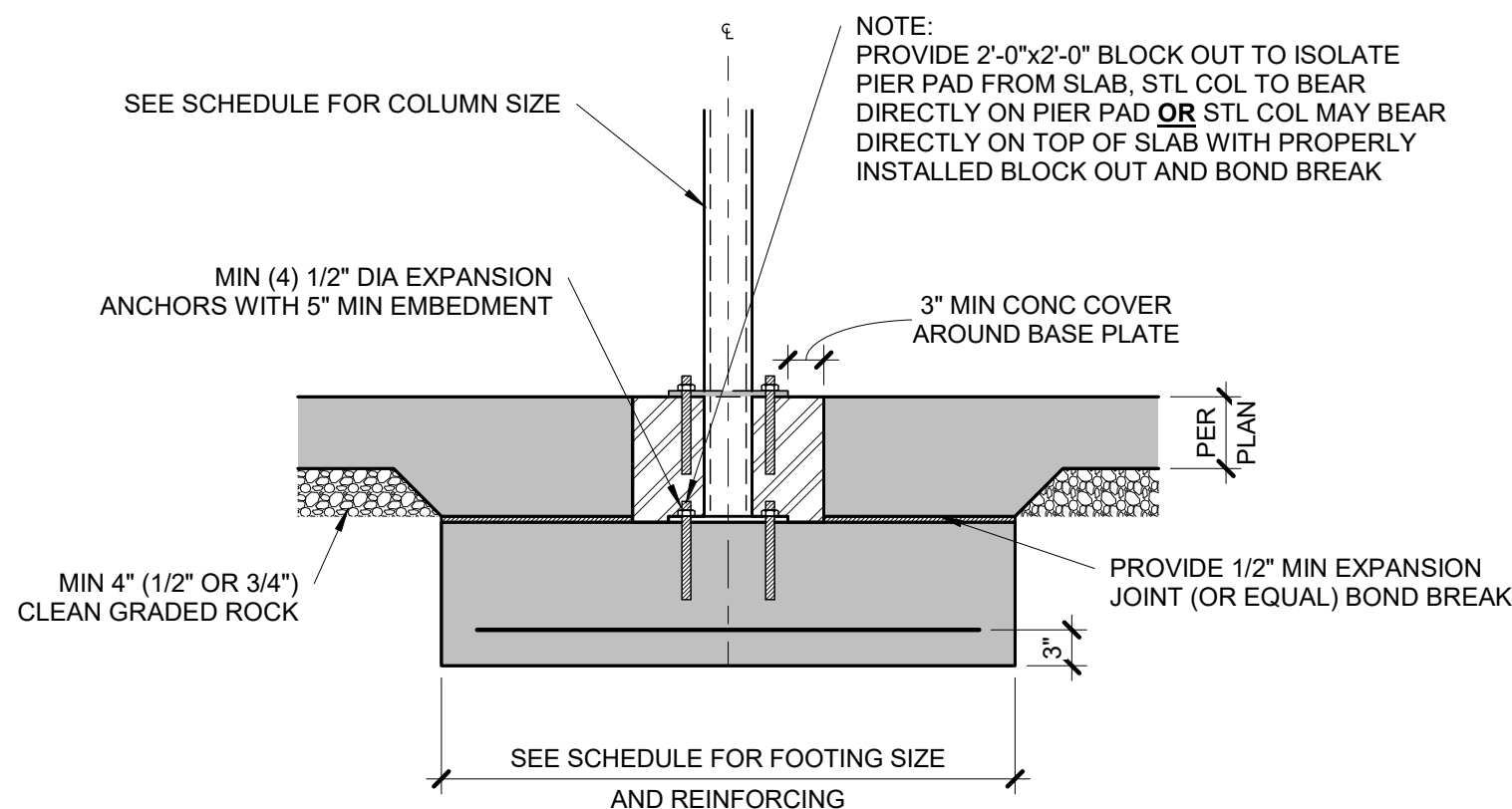
S2.0 3/4" = 1'-0"

COLUMN AND PIER PAD SCHEDULE

COLUMN MARK	PAD SIZE	REINFORCING	COL SIZE
A	30"x30"x12"	(4) #4 BARS E-W	3" SCH 40 (3.5" OD)
B	36"x36"x12"	(4) #4 BARS E-W	3" SCH 40 (3.5" OD)
C	42"x42"x12"	(5) #4 BARS E-W	3" SCH 40 (3.5" OD)
D	48"x48"x12"	(6) #4 BARS E-W	3 1/2" SCH 40 (4" OD)
E	54"x54"x16"	(8) #4 BARS E-W	REF PLAN
F	60"x60"x16"	(10) #4 BARS E-W	REF PLAN

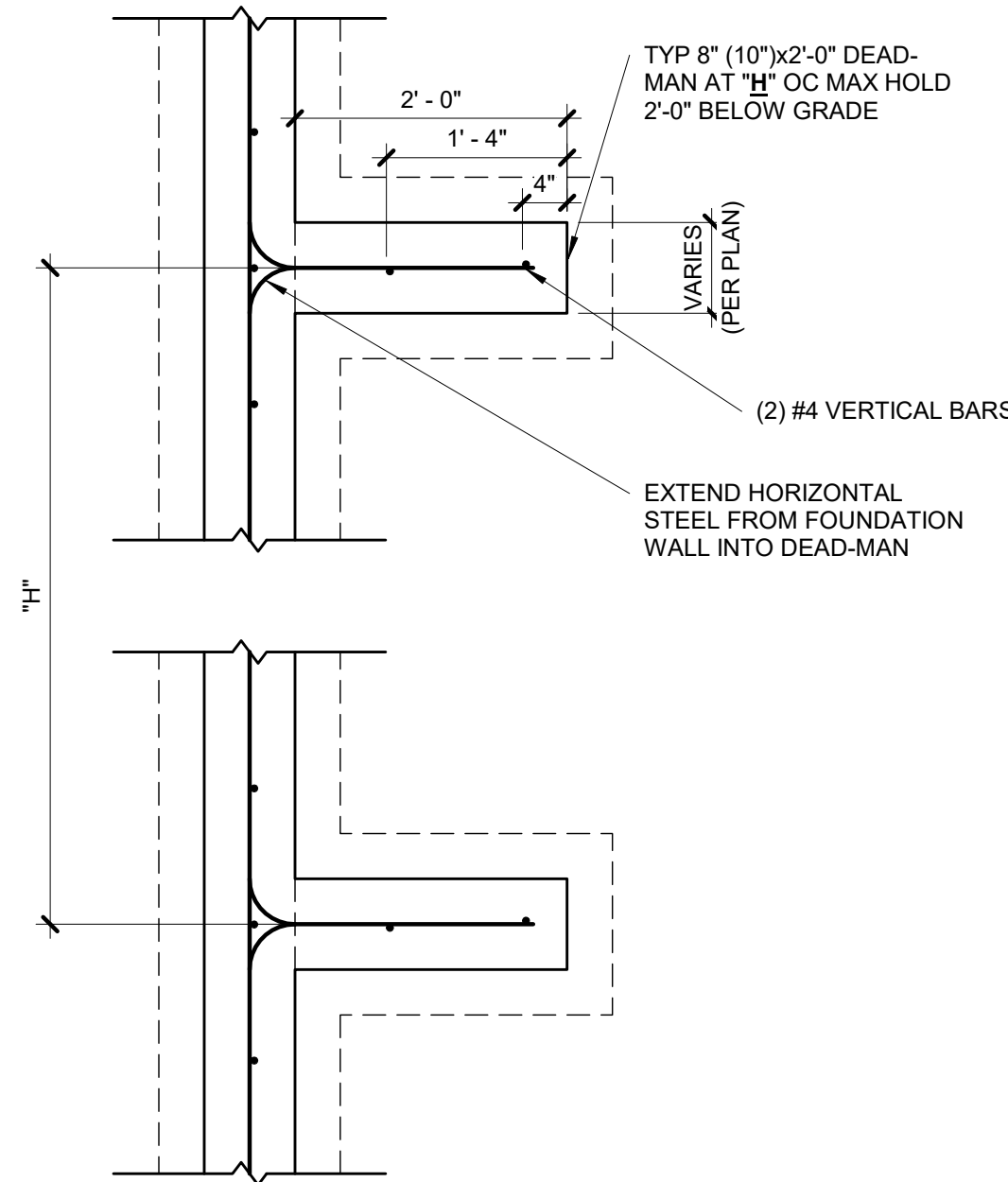
NOTES:

- COLUMN AND PIER PAD SIZES SHOWN ARE FOR MAXIMUM ADJUSTABLE COLUMN HEIGHT OF 9'-1". REQUIRES SEPARATE ENGINEERED DESIGN IF GREATER THAN 9'-1" TALL. COLUMNS SIZED AS QWIK-ADJUST COLUMN, BY QUALITY WAY PRODUCTS, LLC. REFER TO SAFE LOADING CAPACITIES PER MANUF SPECS, OR SUBSTITUTION TO ANOTHER PRODUCT ONLY WITH PRIOR APPROVAL BY APEX ENGINEERS.
- COLUMN AND PIER PAD SIZES SHOWN ARE BASED ON AN ASSUMED MINIMUM ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF.



5 COLUMN PAD DETAIL

S2.0 3/4" = 1'-0"

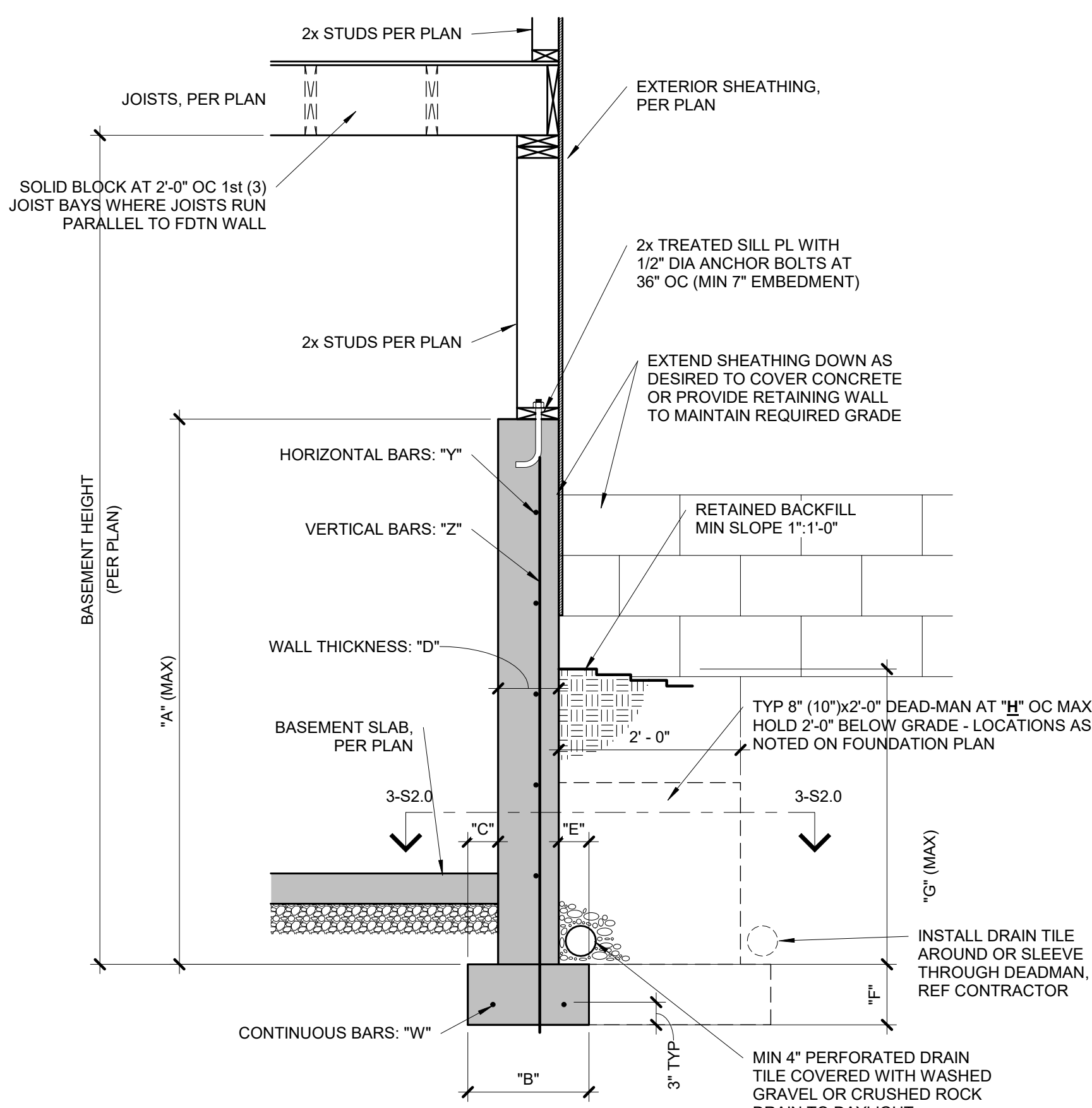


NOTES:

- MIN 3000 PSI FOOTING COMPRESSIVE CONCRETE STRENGTH.
- MIN 3000 PSI WALL COMPRESSIVE CONCRETE STRENGTH.
- AIR ENTRAINMENT BETWEEN 5% & 7% OF CONCRETE VOLUME.
- GRADE 40 REINFORCING STEEL UNLESS OTHERWISE NOTED.
- LAP SPLICES 24" MIN.
- WALL SHALL BE BACK-FILLED WITH CLEAN, LEAN CLAY (OR BETTER) LOW VOLUME CHANGE MATERIAL. ON-SITE MATERIAL MAY BE USED IF DEEMED ACCEPTABLE BY THE GEOTECHNICAL ENGINEER.
- ASSUMED 2,000 PSF BEARING (TO BE VERIFIED BY GEOTECHNICAL ENGINEER).

3 TYPICAL DEAD-MAN SECTION

S2.0 3/4" = 1'-0"



CONCRETE DIMENSIONS

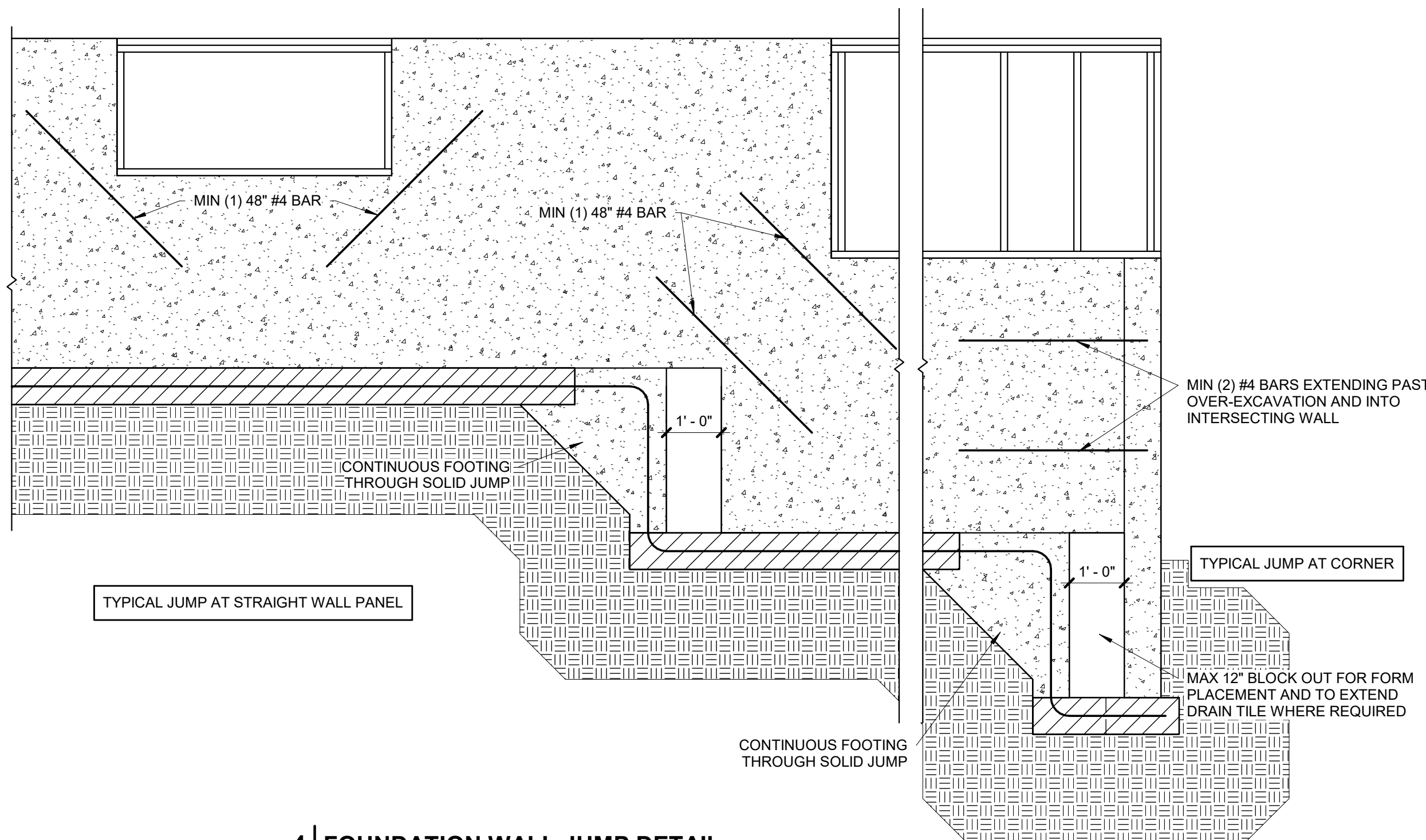
"A"	"B"	"C"	"D"	"E"	"F"	"G"	"H"
4'-0"	1'-4"	4"	8"	4"	8"	3'-4"	20'-0"
6'-0"	1'-4"	4"	8"	4"	8"	4'-4"	20'-0"
9'-0"	1'-8"	5"	8"	4"	8"	4'-4"	20'-0"

NOTES:

- DIMENSION SHOWN IS FOR MAXIMUM UNINTERRUPTED WALL PANEL LENGTH BEFORE A DEAD-MAN SHALL BE INSTALLED. NOTE: A MINIMUM 2'-0" RETURN OR OFFSET IN THE FOUNDATION WALL SHALL SUBSTITUTE AS A DEAD-MAN AND/OR BREAK IN THE WALL PANEL LENGTH.
- VERTICAL REINFORCING STEEL TO EXTEND TO WITHIN 8" OF TOP WALL. MINIMUM (1) #4 HORIZONTAL BAR WITHIN 12" OF TOP AND BOTTOM OF WALL.
- THE BASEMENT SLAB IS AN INTEGRAL PART OF THE 'UNRESTRAINED' FOUNDATION WALL DESIGN THEREFORE, IF THE WALL IS BACKFILLED PRIOR TO PLACEMENT OF THE BASEMENT SLAB, THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPERLY BRACING THE WALL UNTIL THE BASEMENT SLAB HAS BEEN PLACED.

2 TYPICAL 'UNRESTRAINED' FOUNDATION WALL DETAIL

S2.0 3/4" = 1'-0"



4 FOUNDATION WALL JUMP DETAIL

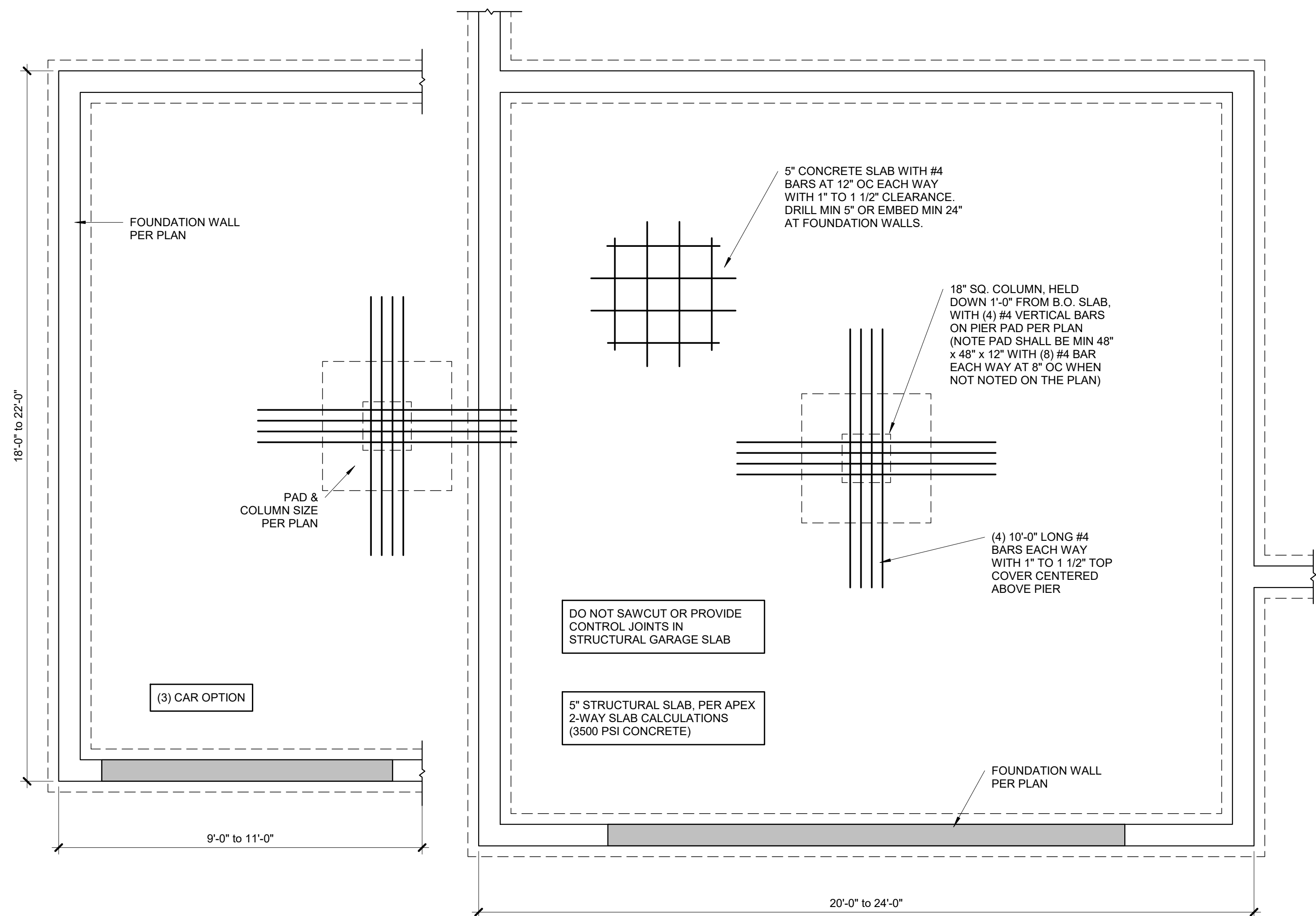
S2.0 1/2" = 1'-0"

EXPANSIVE SOILS DISCLAIMER:

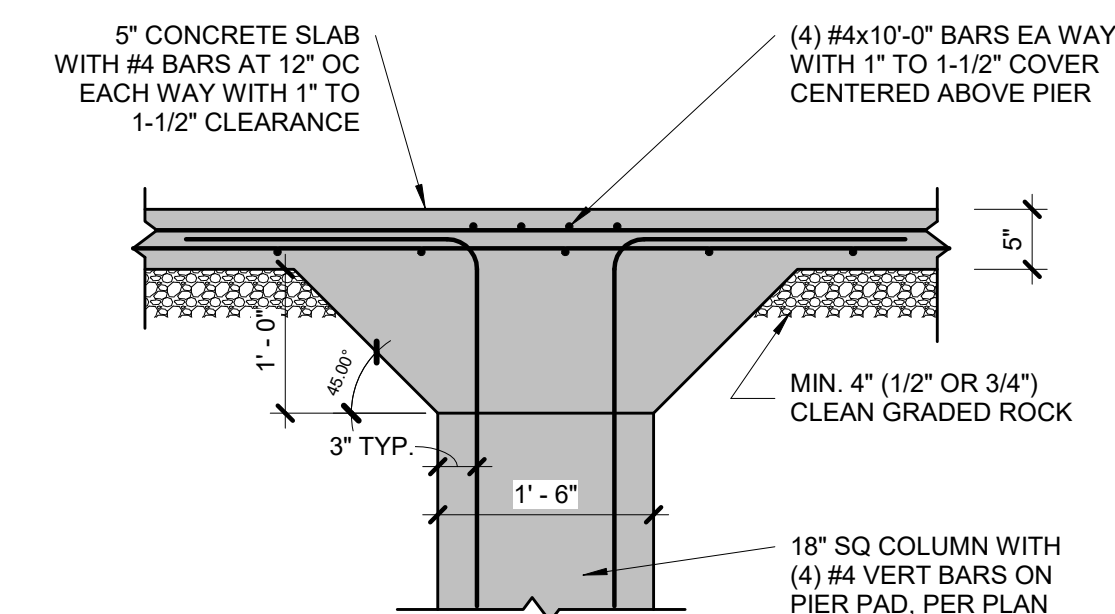
THESE PLANS HAVE BEEN PREPARED BASED ON A PRESUMPTIVE ALLOWABLE BEARING CAPACITY AS ALLOWED BY IRC CODE AND THE LOCAL ENFORCING JURISDICTION.

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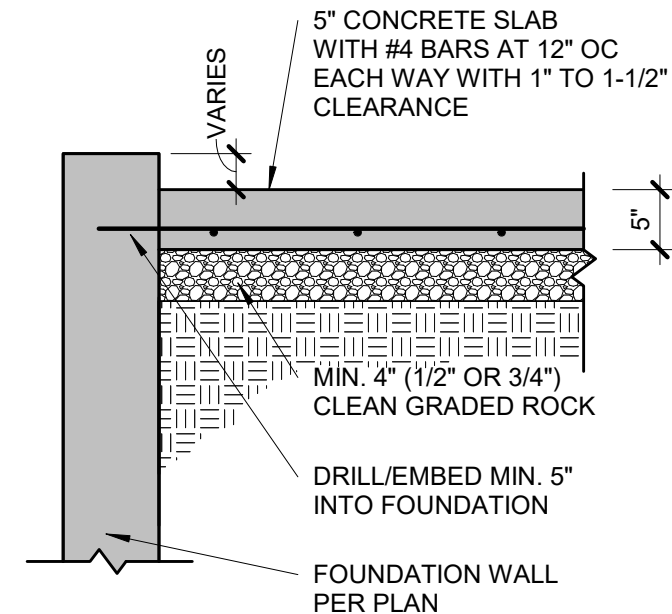
APEX HAS NOT BEEN RETAINED TO DETERMINE THE EXPANSIVE SOIL CHARACTERISTICS OF THE SUBGRADE SOIL AND THEREFORE CANNOT BE HELD RESPONSIBLE FOR THE VOLUMETRIC CHANGES OF THE SOIL (INCLUDING BELOW THE BASEMENT SLAB). BY USE OF THESE PLANS WITHOUT AN ACCOMPANYING GEOTECHNICAL ENGINEERING REPORT, APEX SHALL NOT BE HELD LIABLE FOR ANY FUTURE MOVEMENT AND/OR DIFFERENTIAL MOVEMENT OF THE PROPOSED STRUCTURE AND THE POSSIBLE DAMAGE THAT MAY BE CAUSED AS A RESULT OF SUCH MOVEMENT. DAMAGE FROM EXPANSIVE SOILS AND/OR SETTLEMENT CAN RESULT IN AMONGST OTHER THINGS, THE FOLLOWING: BASEMENT SLAB HEAVE, SHEETROCK CRACKS, WINDOWS AND DOOR BECOMING OUT OF PLUMB AND STICKING AND/OR NOT OPENING, DAMAGE TO TILE, MOULDING, AND OTHER COSMETIC FINISHES.



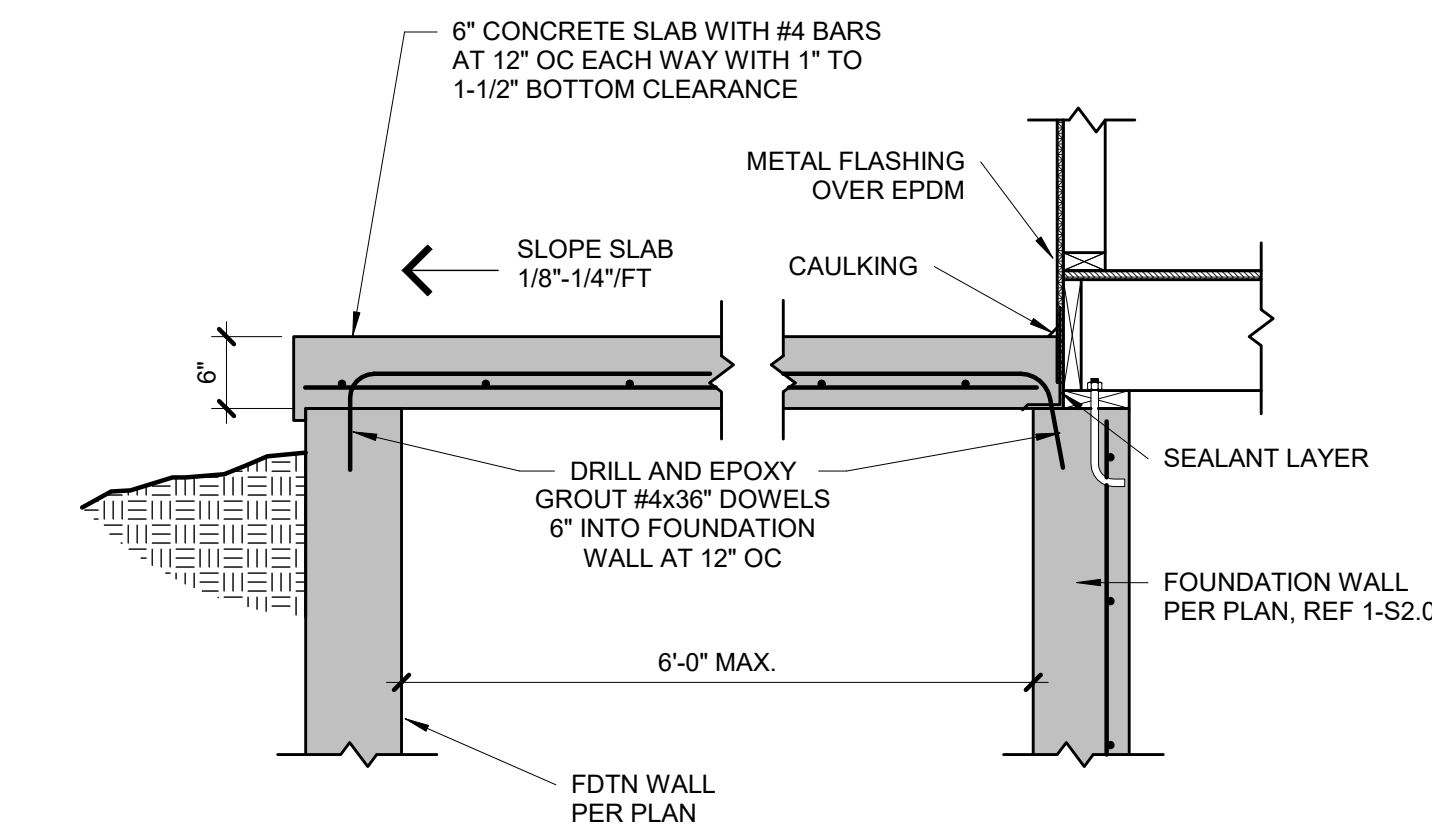
1 TYPICAL STRUCTURAL GARAGE SLAB PLAN
S2.1 3/4" = 1'-0"



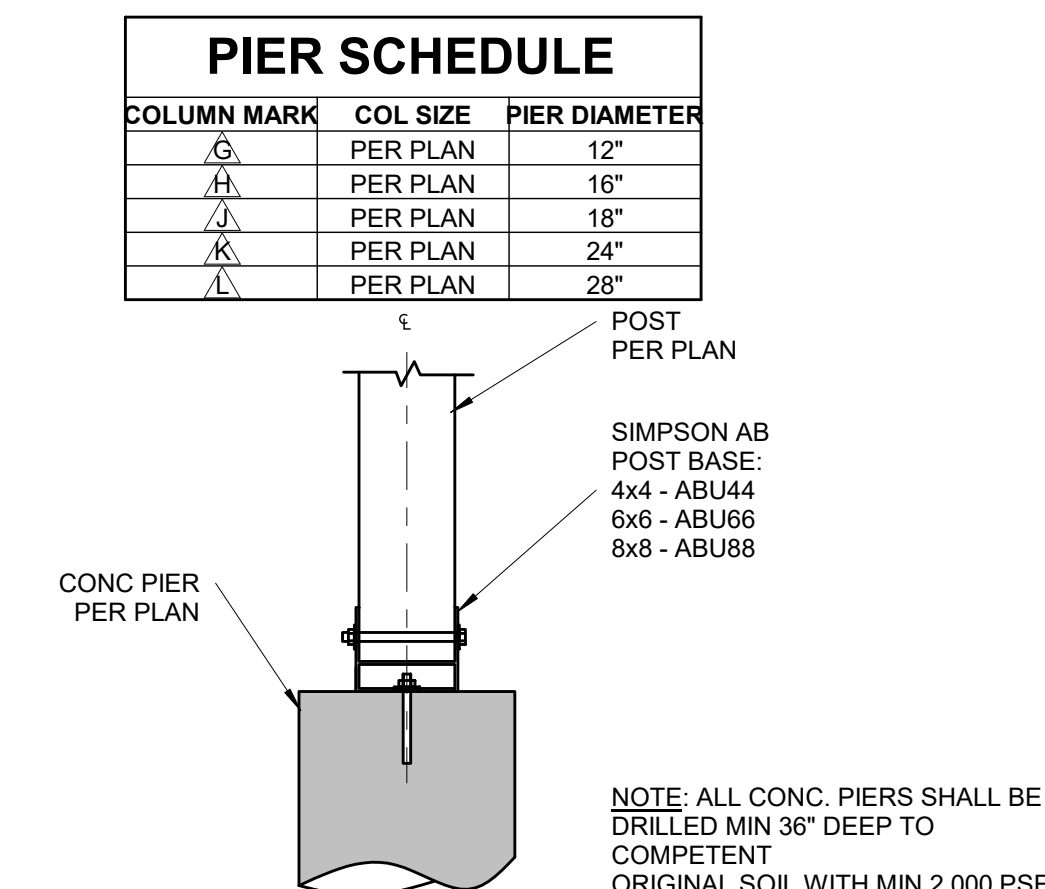
2 STRUCTURAL GARAGE SLAB PIER PAD DETAIL
S2.1 3/4" = 1'-0"



3 STRUCTURAL GARAGE SLAB/WALL SECTION
S2.1 3/4" = 1'-0"

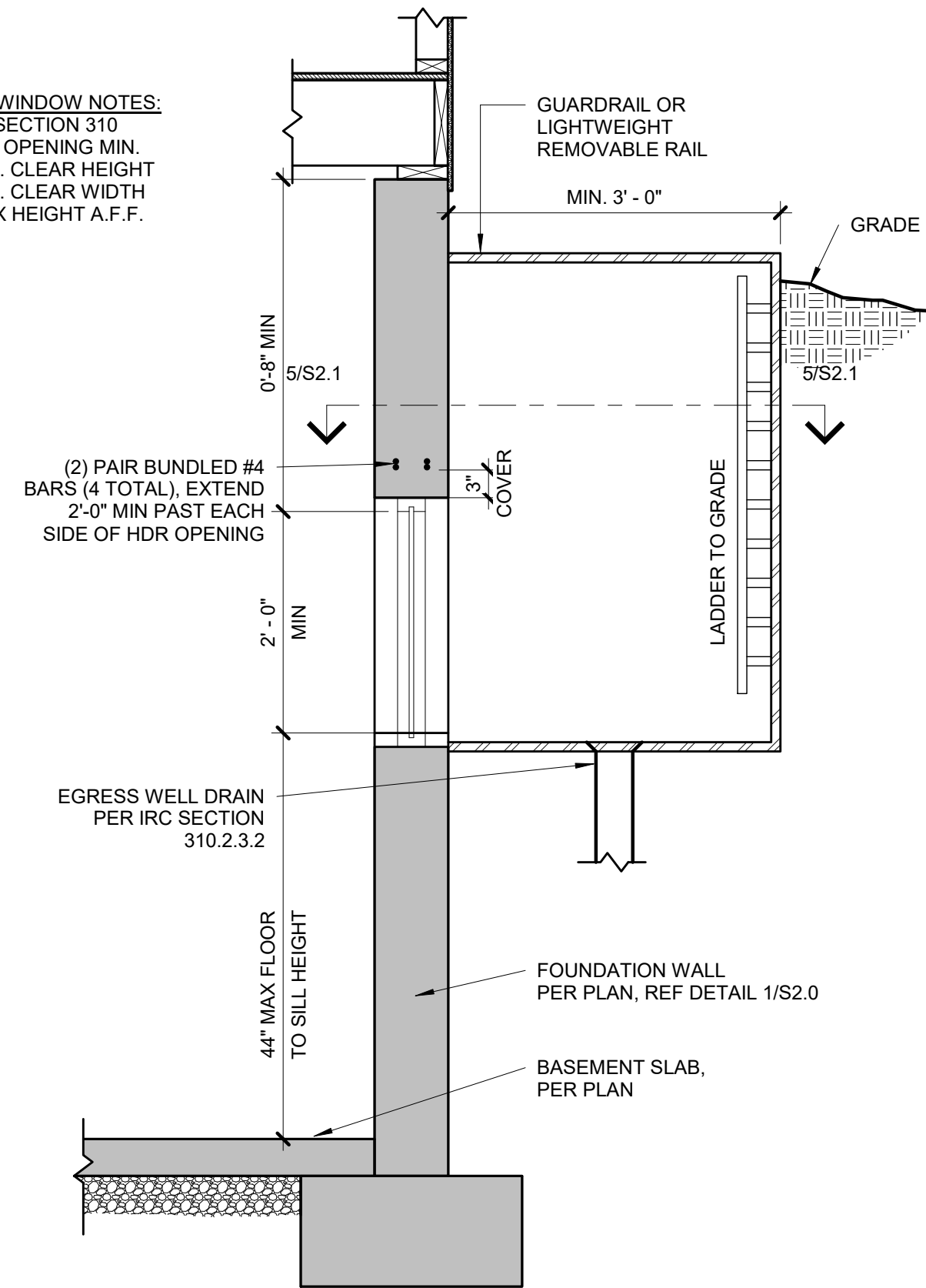


7 SUSPENDED PORCH STOOP DETAIL
S2.1 3/4" = 1'-0"

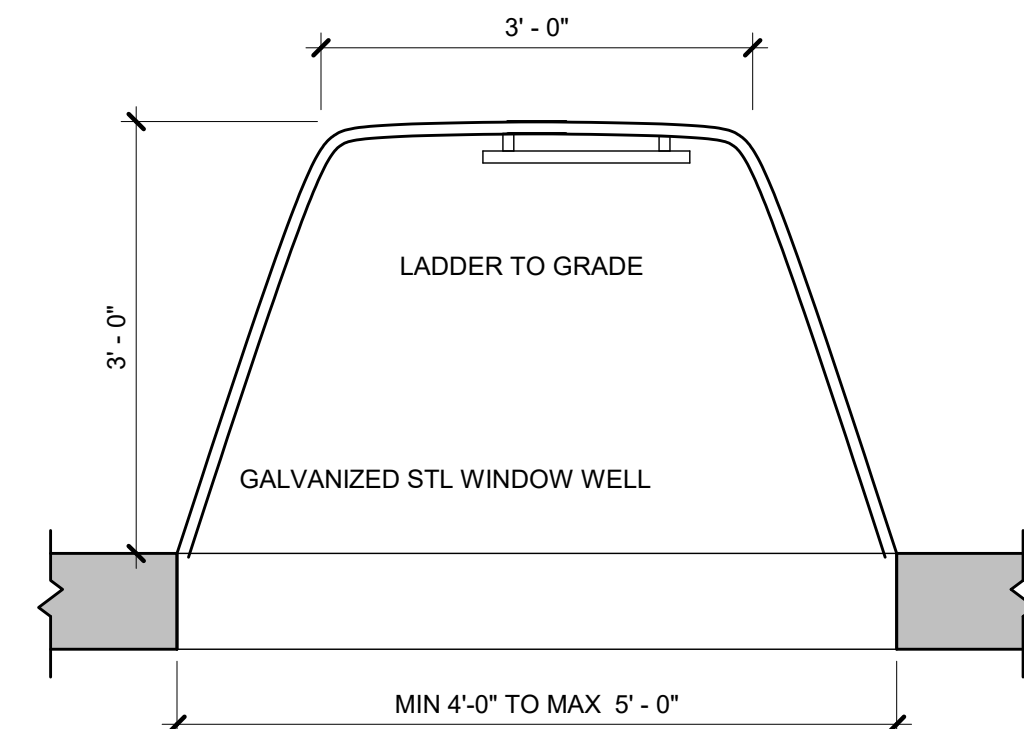


8 POST BASE DETAIL
S2.1 3/4" = 1'-0"

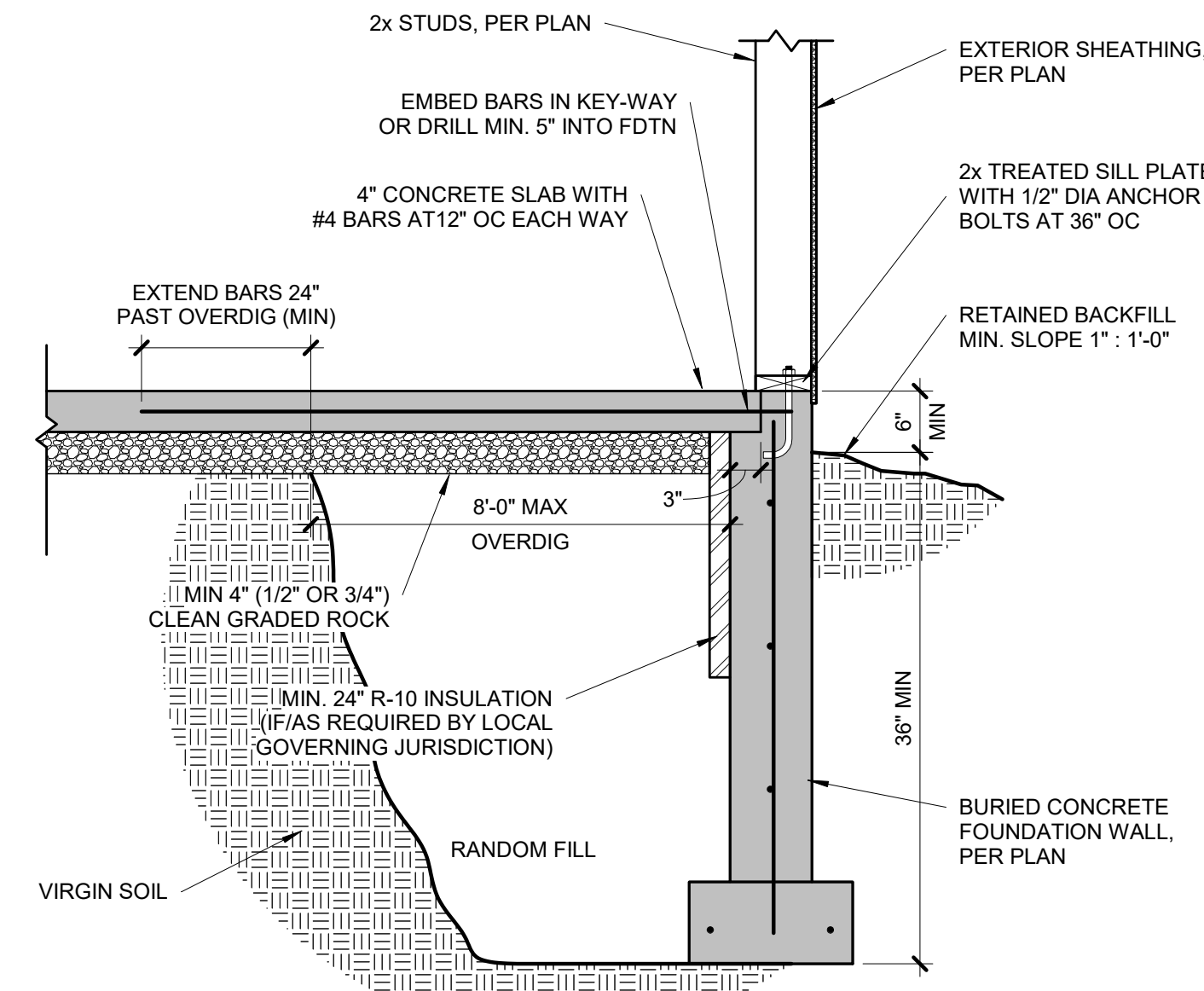
EGRESS WINDOW NOTES:
PER IRC SECTION 310
1. 5.7 S.F. OPENING MIN.
2. 24" MIN. CLEAR HEIGHT
3. 20" MIN. CLEAR WIDTH
4. 44" MAX HEIGHT A.F.F.



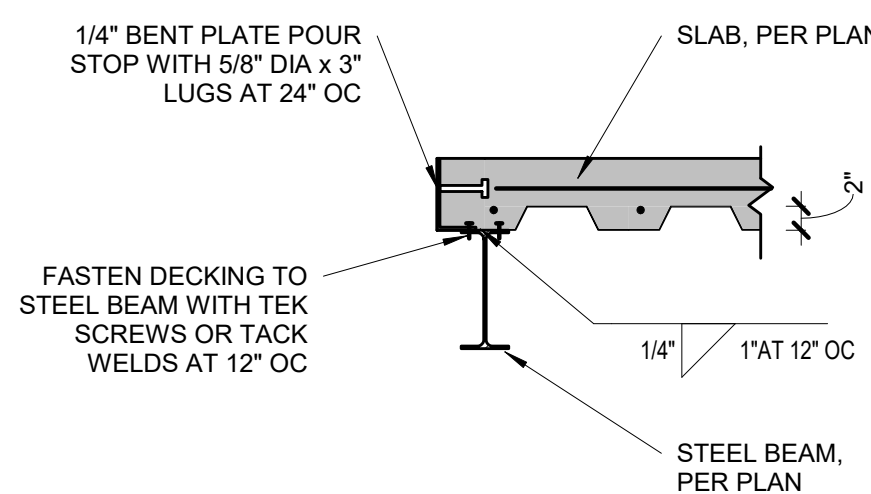
4 TYPICAL EGRESS WINDOW SECTION DETAIL
S2.1 3/4" = 1'-0"



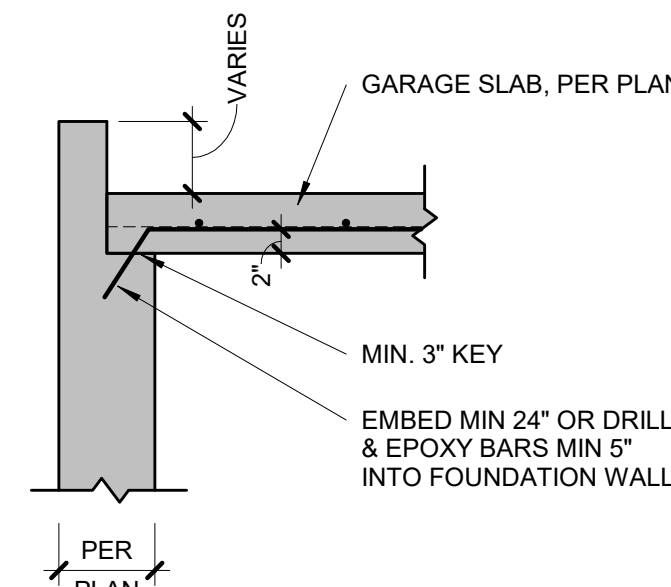
5 TYPICAL EGRESS WINDOW PLAN
S2.1 3/4" = 1'-0"



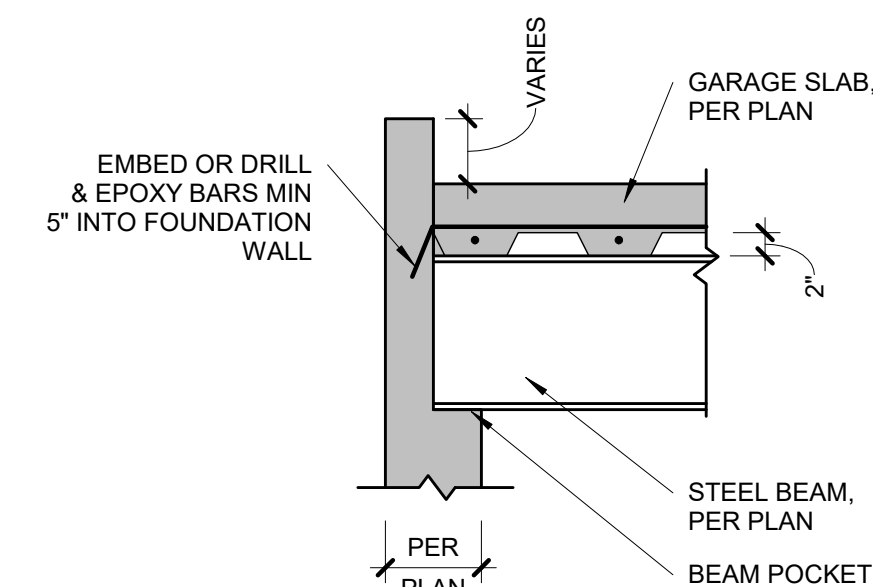
6 TYPICAL OVERDIG DETAIL AT BASEMENT SLAB
S2.1 3/4" = 1'-0"



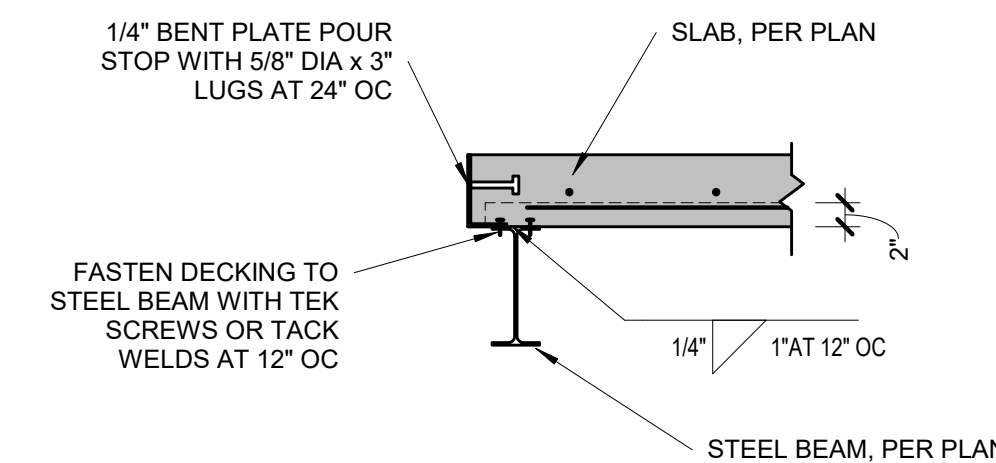
9 POUR STOP DETAIL
S2.1 3/4" = 1'-0"



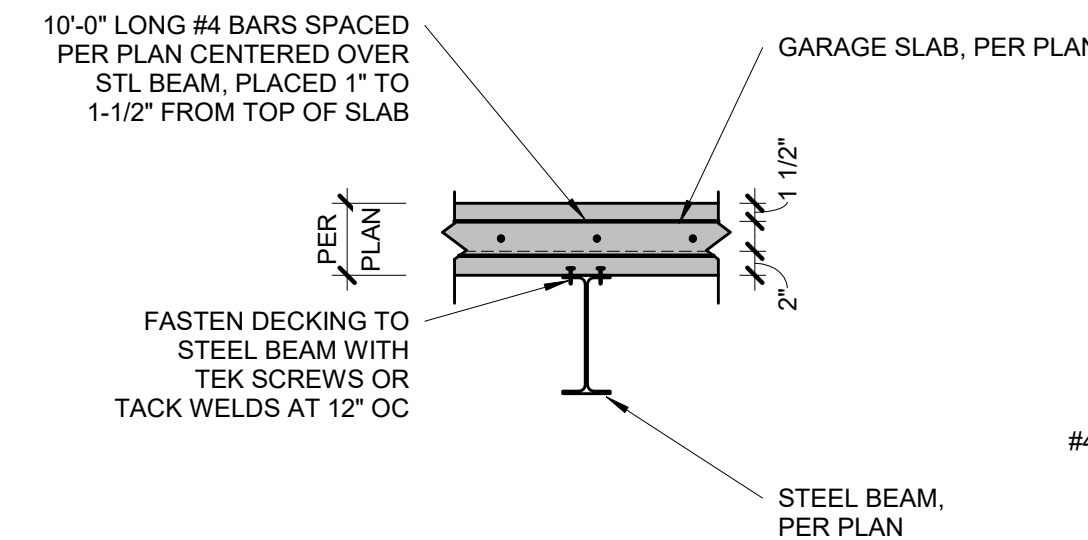
10 GARAGE SLAB BEARING
S2.1 3/4" = 1'-0"



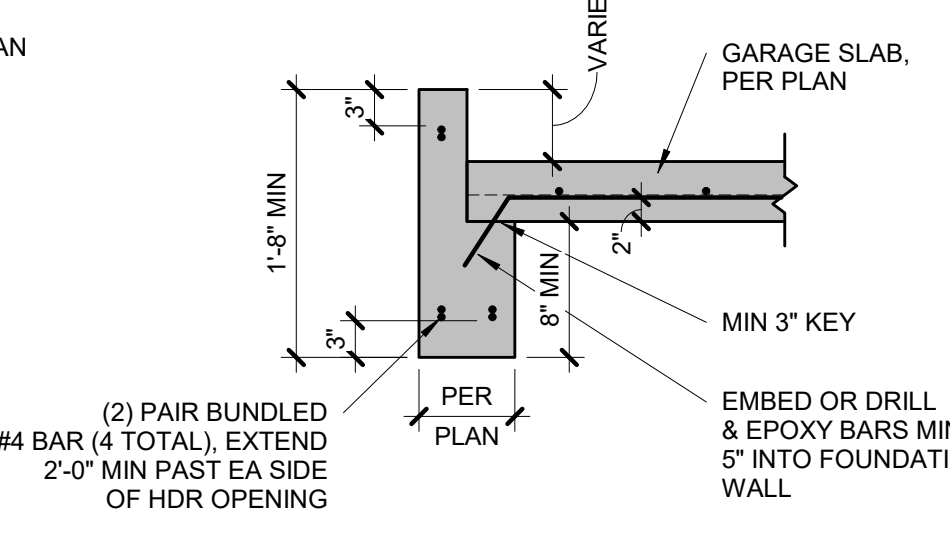
11 GARAGE SLAB BEAM BEARING
S2.1 3/4" = 1'-0"



12 POUR STOP DETAIL
S2.1 3/4" = 1'-0"



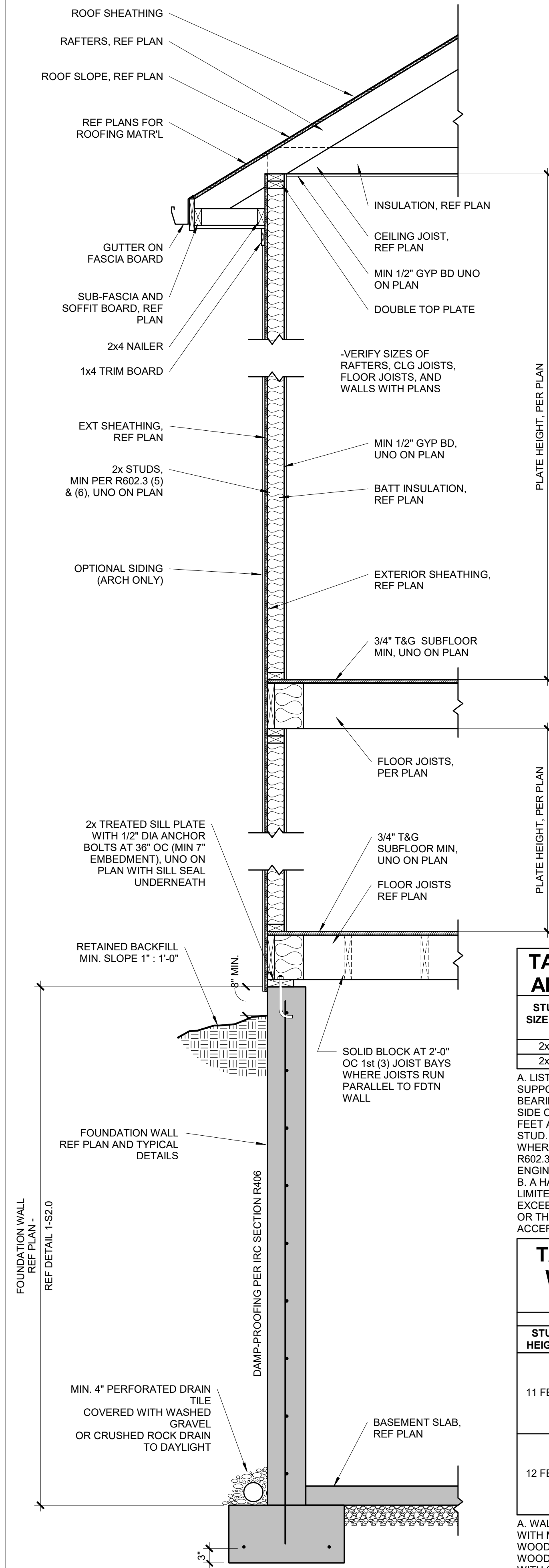
13 GARAGE SLAB BEAM BEARING
S2.1 3/4" = 1'-0"



14 CONCRETE HEADER DETAIL
S2.1 3/4" = 1'-0"

TYPICAL SUSPENDED SLAB DETAIL

STEEL DECKING NOTES:
• MINIMUM 1-1/2" BEARING
• FASTEN TO SUPPORT STEEL WITH 5/8" VISIBLE PUDDLE WELDS AT EDGE RIBS AND 12" CENTERS ALONG END BEARING
• FASTEN SIDE LAPS AND PERIMETER EDGES AT 36" CENTERS WITH #10 TEK SCREWS OR 5/8" PUDDLE WELDS
• MAX UNSUPPORTED CONSTRUCTION SPAN 6'-0", UNO ON PLANS BY APEX



12 | TYPICAL WALL CROSS-SECTION
S3.0 3/4" = 1'-0"

TABLE R602.3 (5) - SIZE, HEIGHT, AND SPACING OF WOOD STUDS*					
STUD SIZE (IN)	LATERALLY UNSUPPORTED STUD HEIGHT*	STRUCTURE SUPPORTED			
		ROOF ONLY	ROOF AND (1) FLOOR	ROOF AND (2) FLOORS	
2x4	10 FEET	24" OC*	16" OC*	N/A	
2x6	10 FEET	24" OC	24" OC	16" OC	

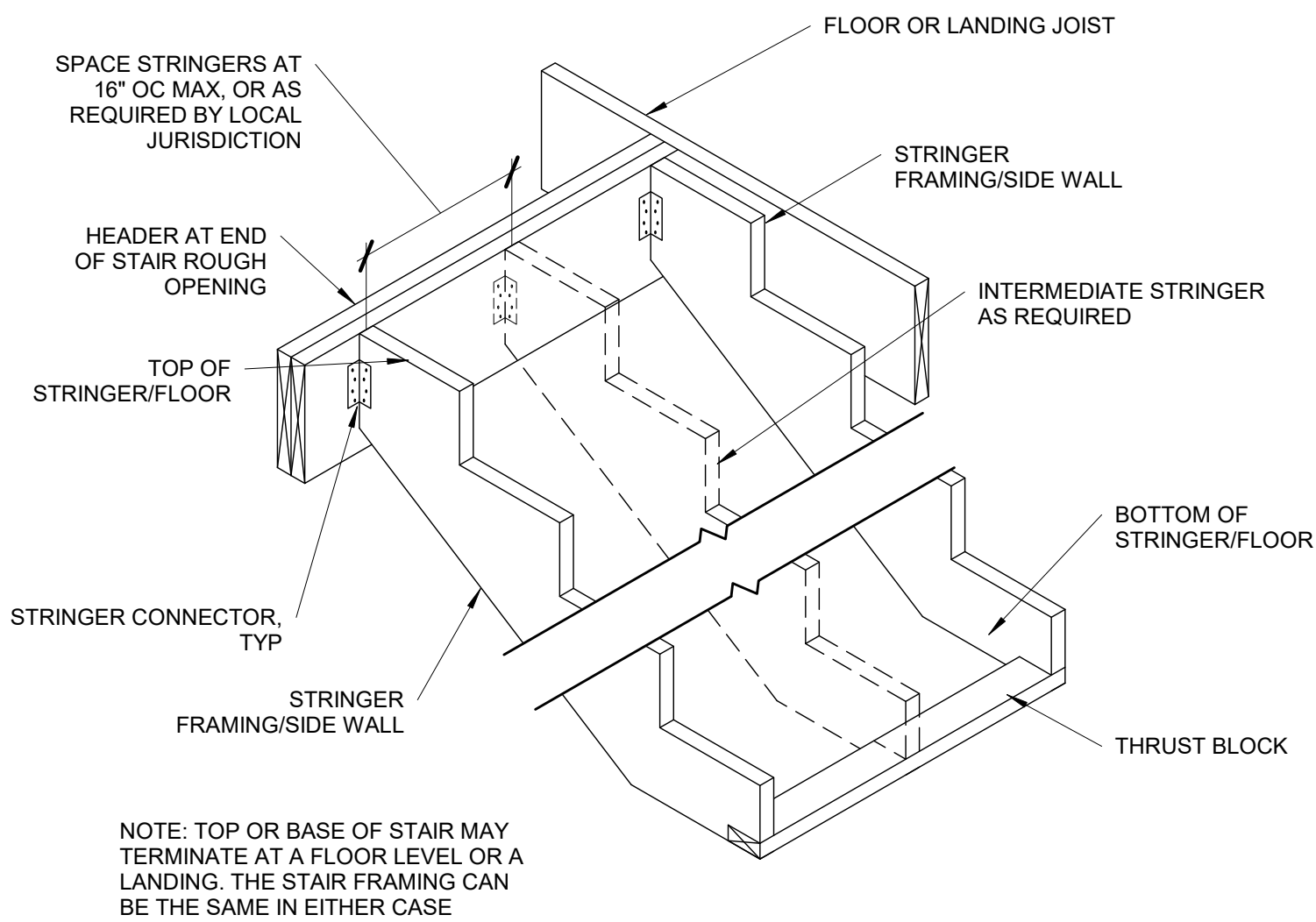
A. LISTED HEIGHTS ARE DISTANCES BETWEEN POINTS OF LATERAL SUPPORT PLACED PERPENDICULAR TO THE PLANE OF THE WALL. BEARING WALLS SHALL BE SHEATHED ON NOT LESS THAN ONE SIDE OR BRIDGING SHALL BE INSTALLED NOT GREATER THAN 4 FEET APART MEASURED VERTICALLY FROM EITHER END OF THE STUD. INCREASES IN UNSUPPORTED HEIGHT ARE PERMITTED WHERE IN THE COMPLIANCE WITH EXCEPTION 2 OF SECTION R602.3.1 OR DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICE.

B. A HABITABLE ATTIC ASSEMBLY SUPPORTED BY 2x4 STUDS IS LIMITED TO A ROOF SPAN OF 32 FEET. WHERE THE ROOF SPAN EXCEEDS 32 FEET, THE WALL STUDS SHALL BE INCREASED TO 2x6 OR THE STUDS SHALL BE DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICE.

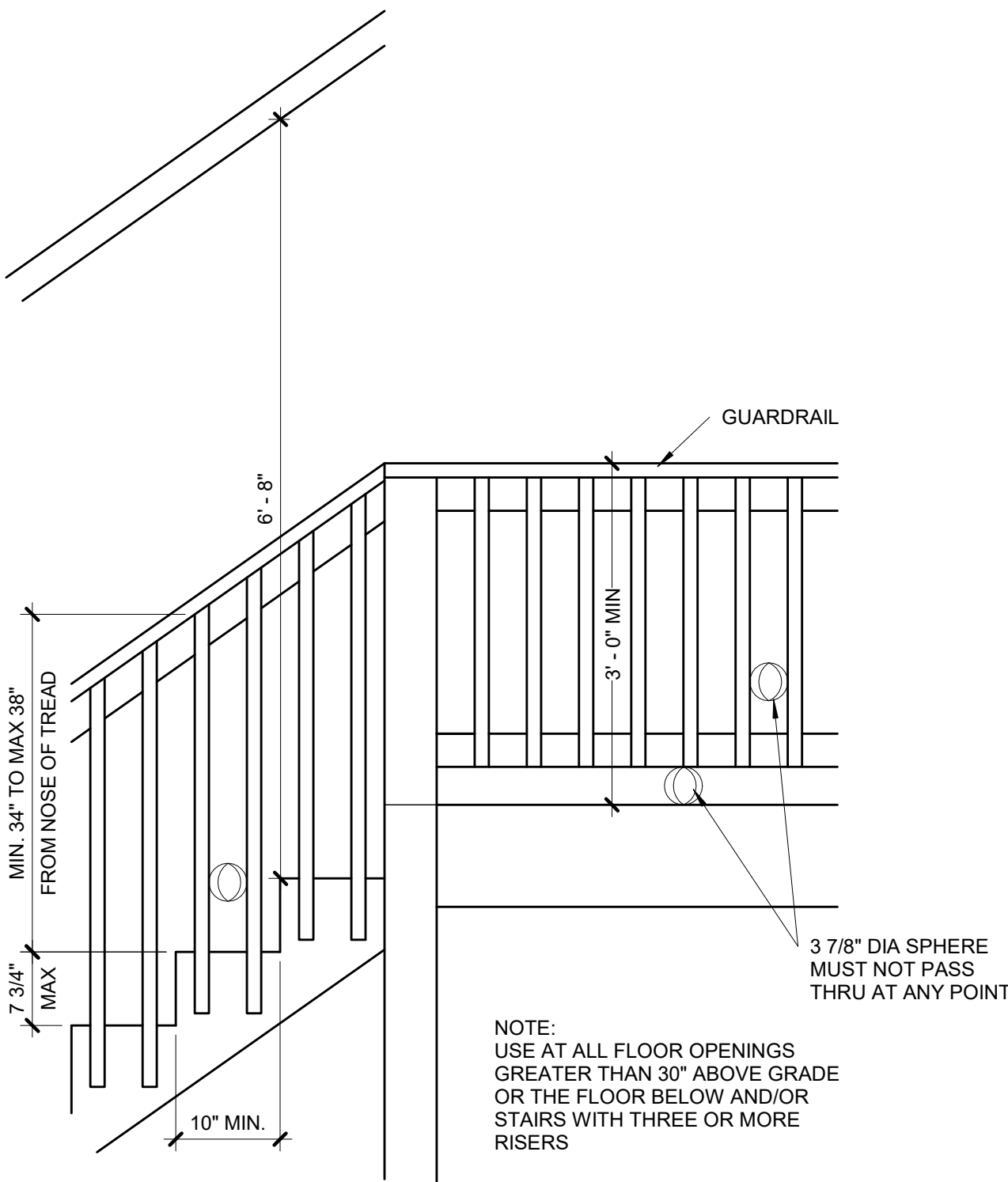
TABLE R602.3 (6) - ALTERNATE WOOD BEARING WALL STUD SIZE, HEIGHT AND SPACING					
ULTIMATE DESIGN WIND SPEED = 115 MPH					
STUD HEIGHT	SUPPORTING	STUD SPACING	MAX ROOF/FLOOR SPAN		
			12 FEET	24 FEET	
11 FEET	ROOF ONLY	12 IN	2x4	2x4	
		16 IN	2x4	2x4	
	ROOF AND ONE FLOOR	24 IN	2x6	2x6	
		12 IN	2x4	2x6	
12 FEET	ROOF ONLY	12 IN	2x4	2x4	
		16 IN	2x4	2x6	
	ROOF AND ONE FLOOR	24 IN	2x6	2x6	
		12 IN	2x4	2x6	

A. WALL STUDS NOT EXCEEDING 16" OC SHALL BE SHEATHED WITH MINIMUM 1/2" GYPSUM BOARD ON THE INTERIOR AND 3/8" WOOD STRUCTURAL PANEL SHEATHING ON THE EXTERIOR. WOOD STRUCTURAL PANEL SHEATHING SHALL BE ATTACHED WITH 8d (2.5" x 0.131") NAILS NOT GREATER THAN 6" OC ALONG PANEL EDGES AND 12" OC AT INTERMEDIATE SUPPORTS, AND ALL PANEL JOINTS SHALL OCCUR OVER STUDS OR BLOCKING.

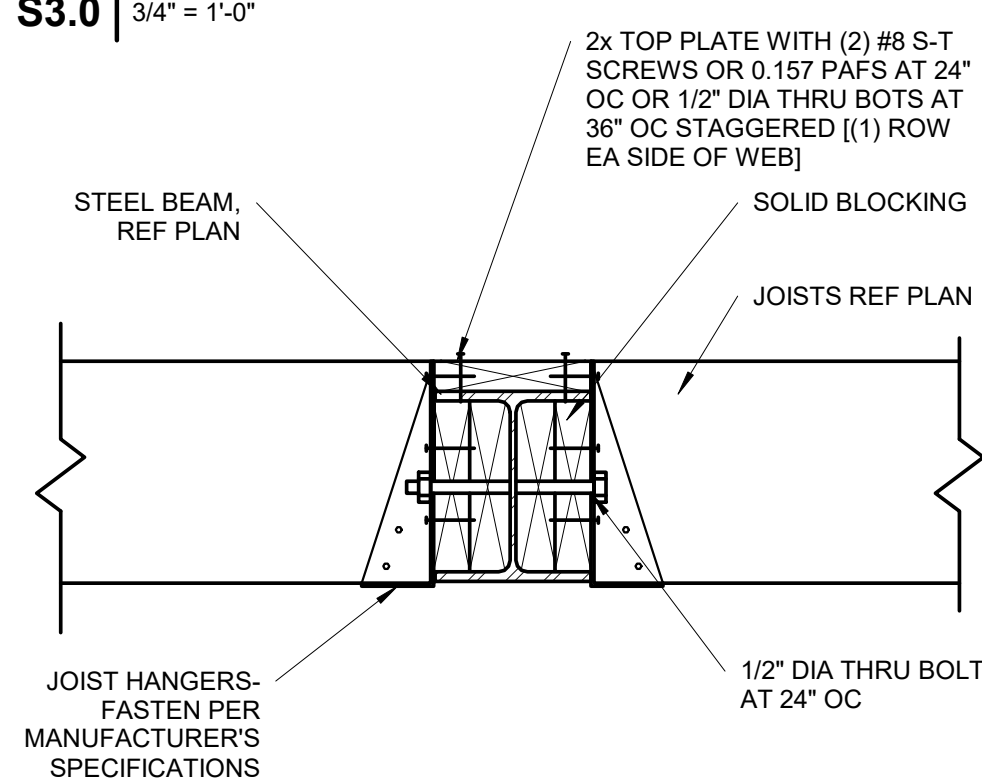
B. THE MAXIMUM SPAN IS APPLICABLE TO BOTH SINGLE AND MULTIPLE SPAN ROOF AND FLOOR CONDITIONS. THE ROOF ASSEMBLY SHALL NOT CONTAIN A HABITABLE ATTIC.



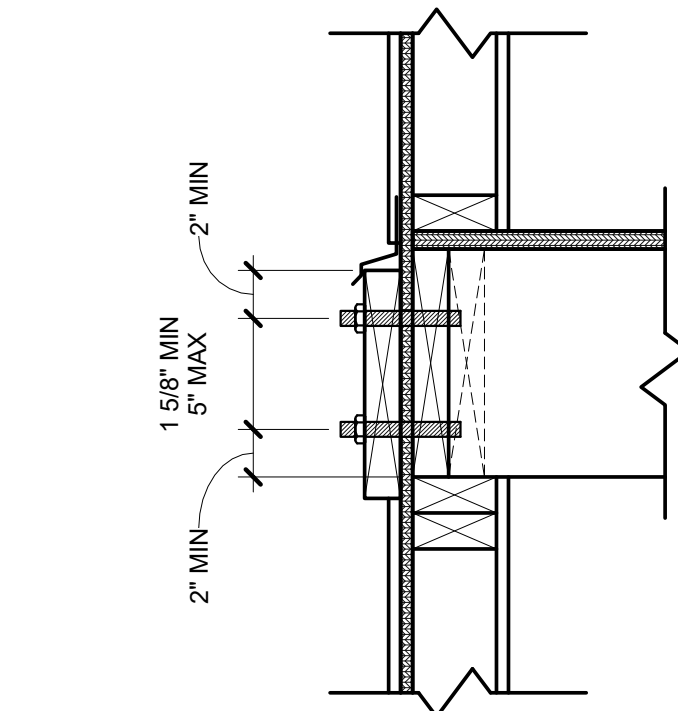
11 | TYPICAL STRINGER DETAIL
S3.0 3/4" = 1'-0"



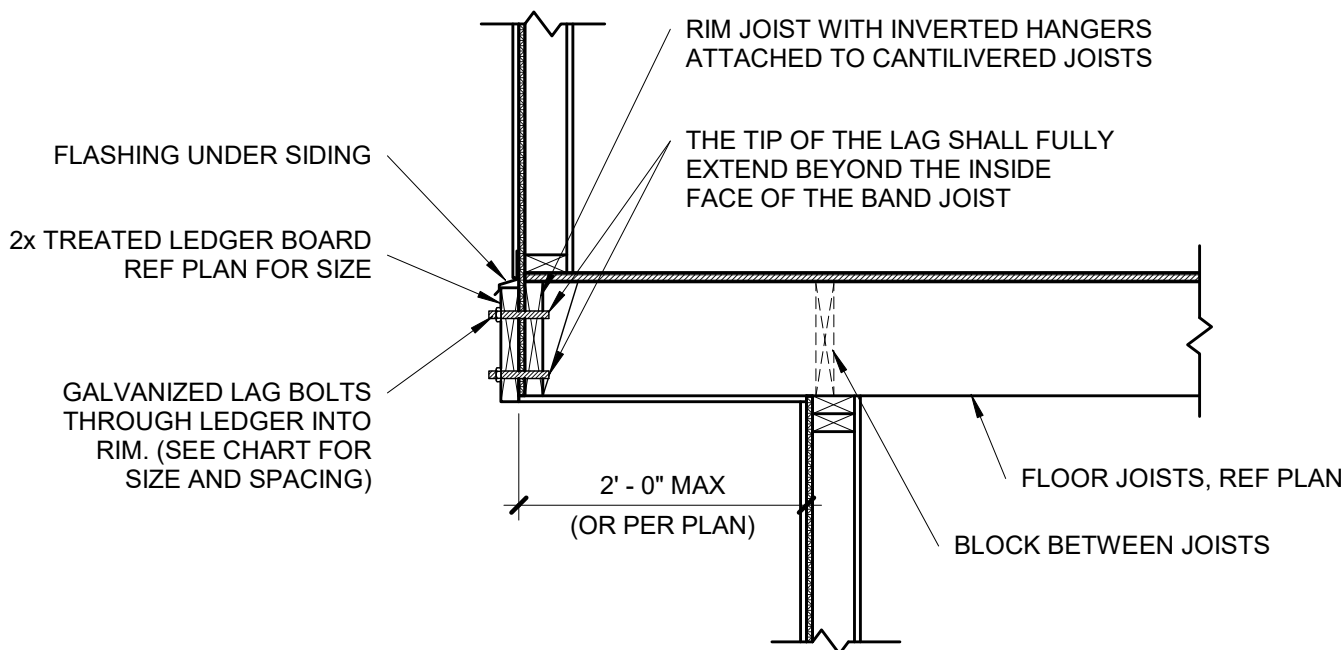
10 | TYPICAL STAIR/RAIL DETAIL
S3.0 3/4" = 1'-0"



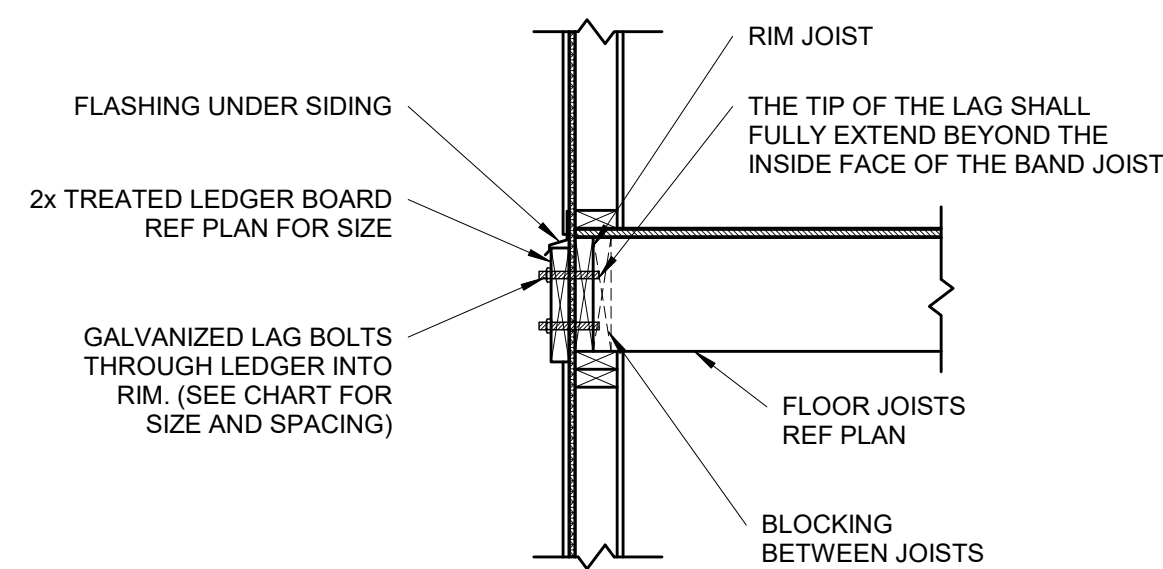
9 | UPSET STEEL BEAM/JOIST CONNECTION
S3.0 1 1/2" = 1'-0"



8 | LEDGER FASTENER PLACEMENT
S3.0 1 1/2" = 1'-0"



7 | TYPICAL CANTILEVER FRAMING WITH DECK ATTACHMENT
S3.0 3/4" = 1'-0"

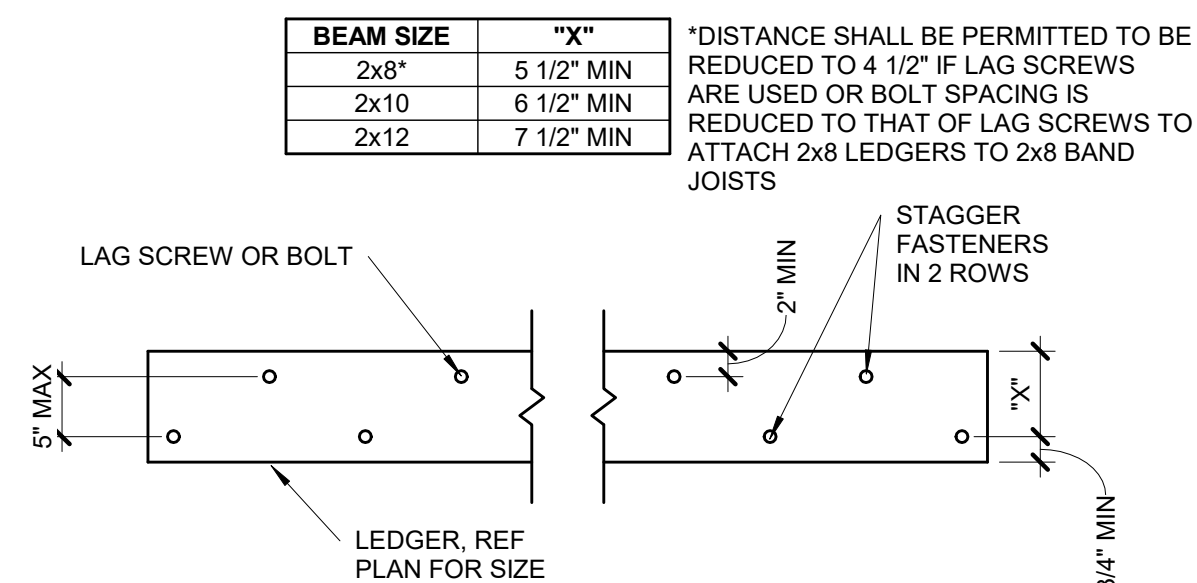


DECK LEDGER ATTACHMENT CHART

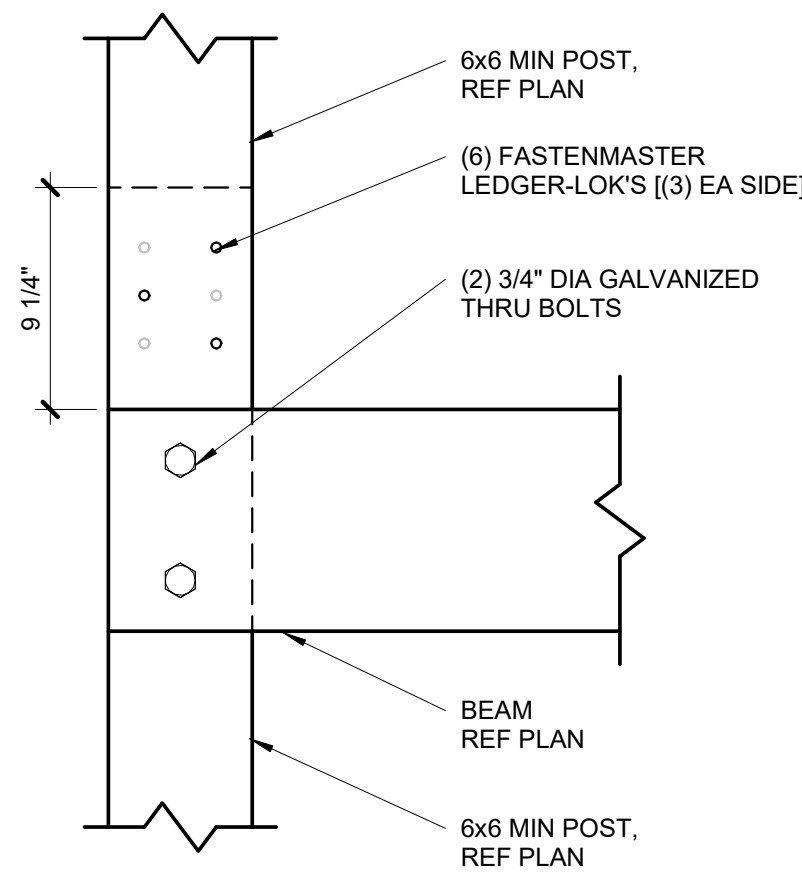
DECK JOIST SPAN	1/2" DIA LAG SPACING	EQUIVALENT SPACING FOR 16" OC JOIST BAYS
UP TO 10'-0"	16" OC	N/A
10'-1" TO 12'-0"	15" OC	16" OC DBL EVERY OTHER
12'-1" TO 14'-0"	13" OC	16" OC DBL EVERY OTHER
14'-1" TO 16'-0"	11" OC	16" OC DBL EVERY JOIST BAY
16'-1" TO 18'-0"	10" OC	16" OC DBL EVERY JOIST BAY

NOTE: CHART IS APPLICABLE ONLY WHEN DECK IS SHOWN ON APPROVED PLAN.

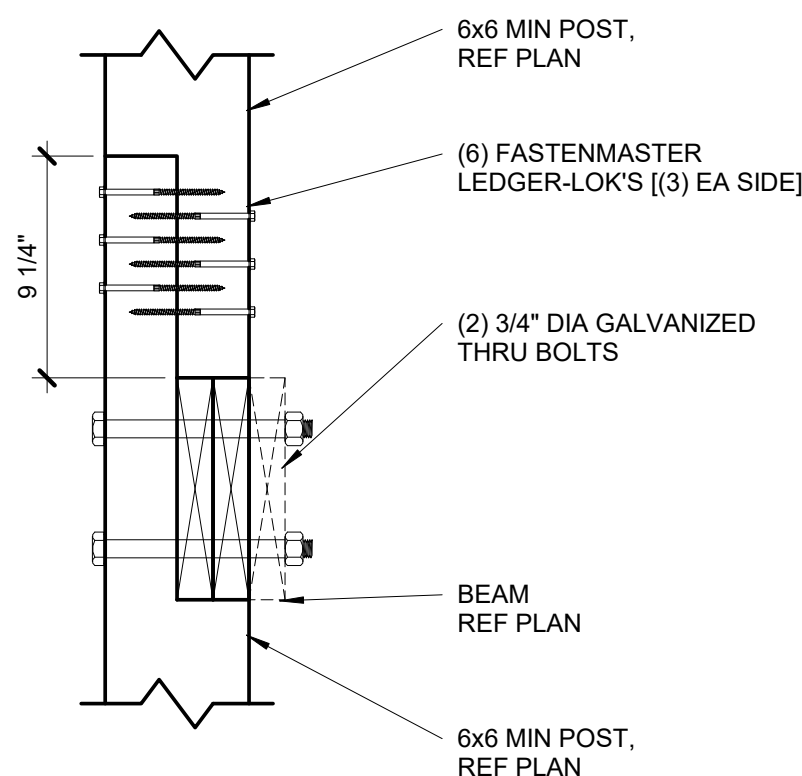
6 | TYPICAL LEDGER ATTACHMENT
S3.0 3/4" = 1'-0"



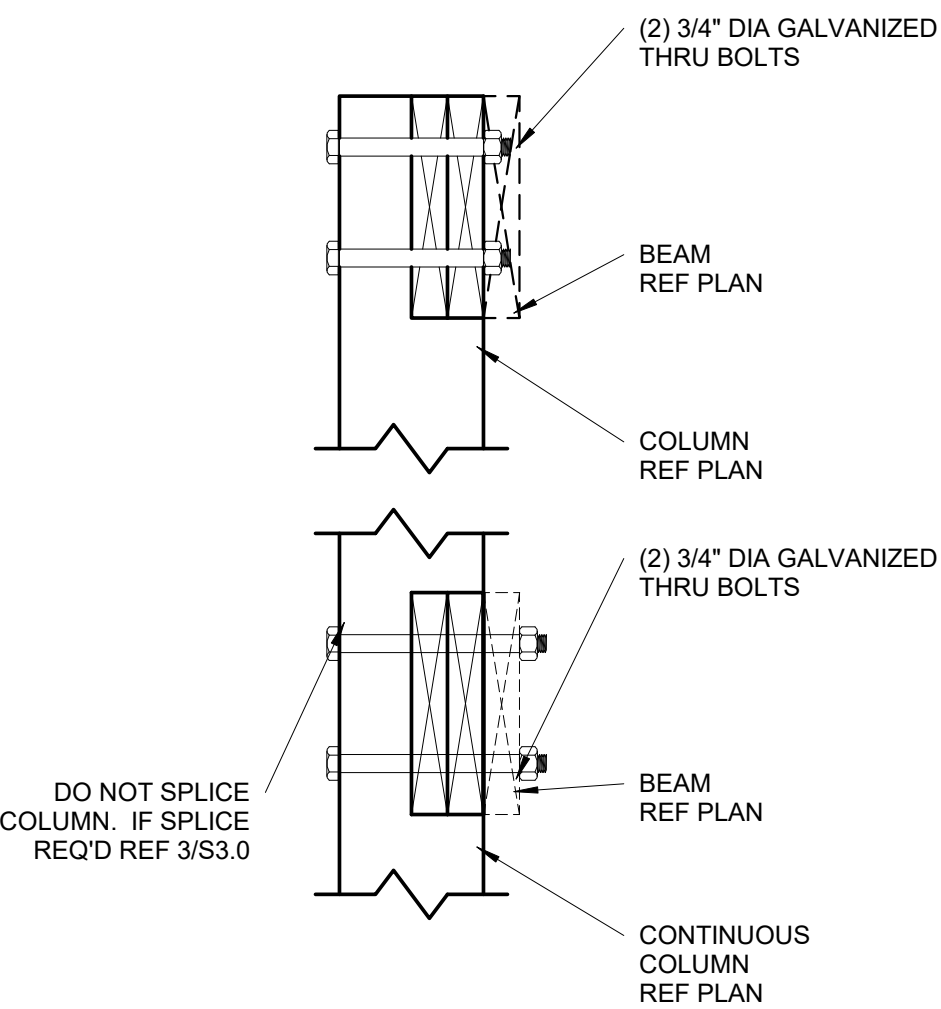
5 | TYPICAL LEDGER BOLT SPACING
S3.0 3/4" = 1'-0"



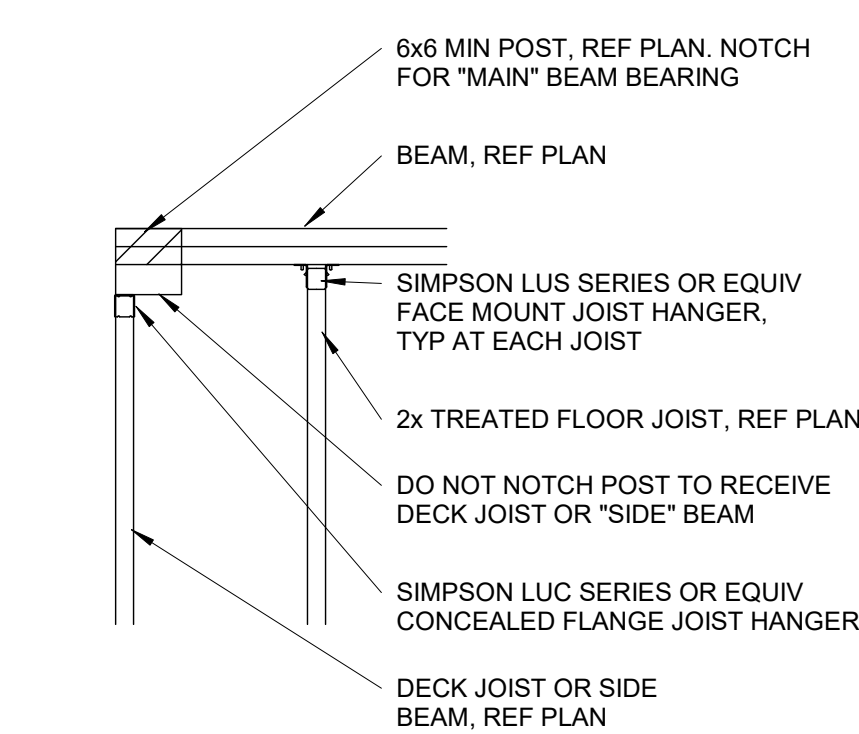
4 | SPLICED DECK COLUMN CONNECTION
S3.0 1 1/2" = 1'-0"



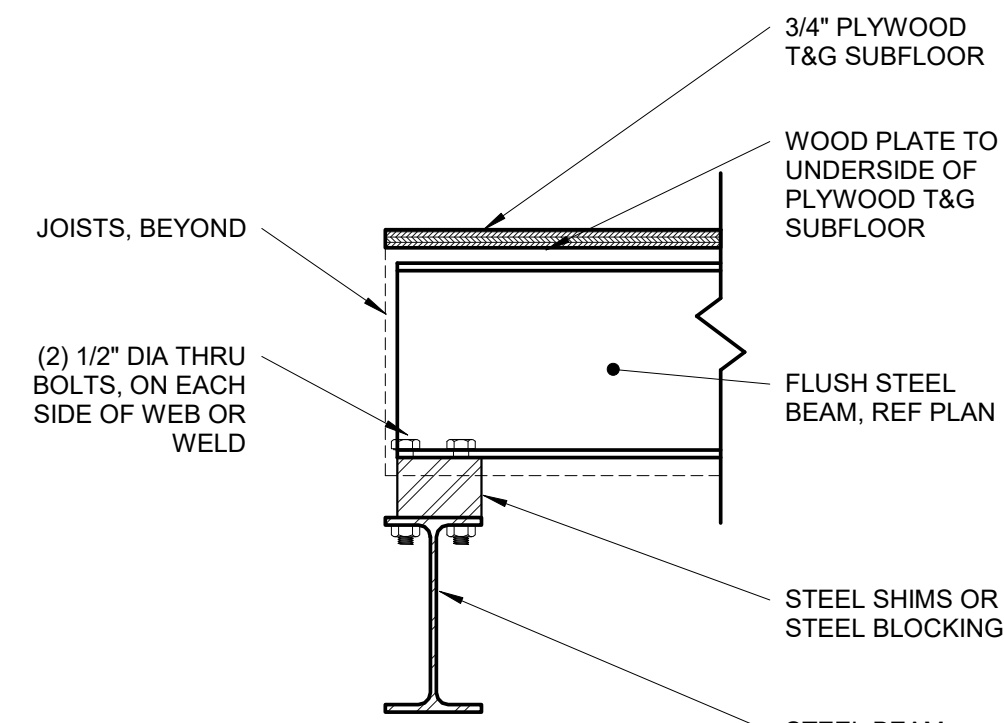
3 | SPLICED DECK COLUMN CONNECTION
S3.0 1 1/2" = 1'-0"



2 | DECK BEAM/COLUMN CONNECTION
S3.0 1 1/2" = 1'-0"

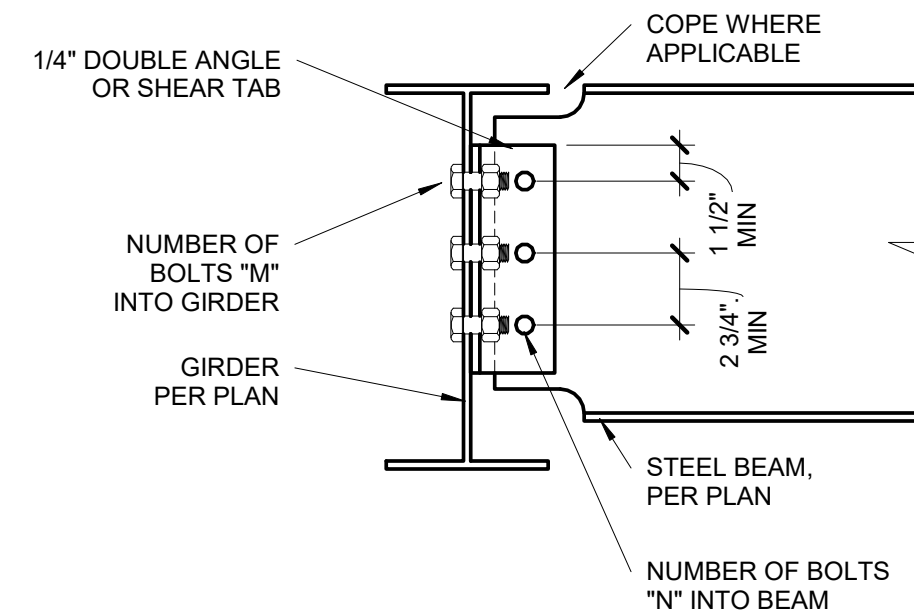


1 | DECK BEAM/COLUMN CORNER CONDITION
S3.0 3/4" = 1'-0"



4 FLUSH STEEL BEAM TO STEEL BEAM

S3.1 1 1/2" = 1'-0"



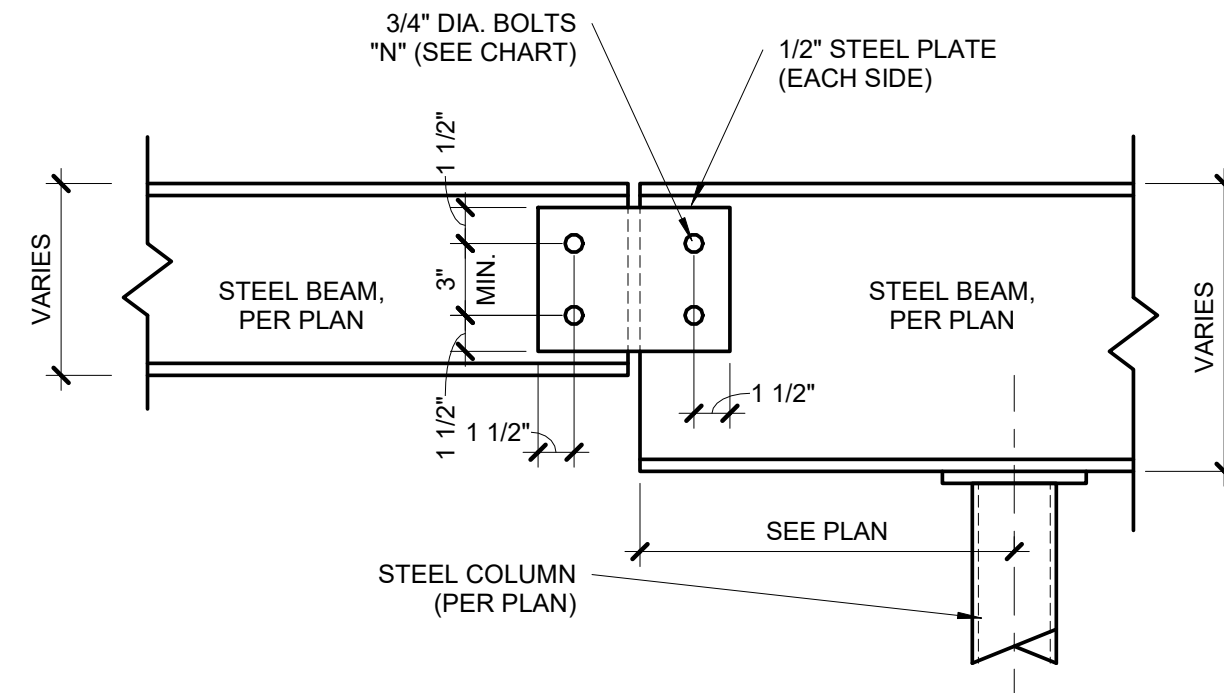
3 BEAM TO GIRDER CONNECTION

S3.1 1 1/2" = 1'-0"

BEAM CONNECTION SCHEDULE

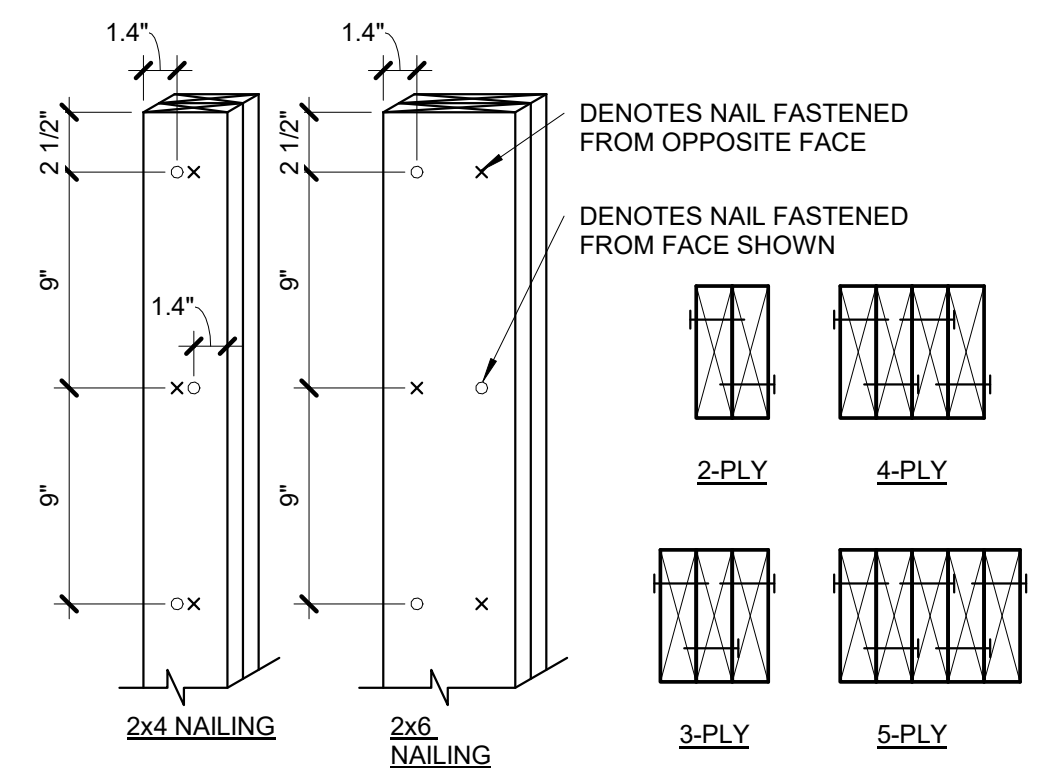
BEAM SIZE	# OF BOLTS "N"	# OF BOLTS "M"
W8, W10	2	4
W12, W14	3	6
W16, W18	4	8

NOTES:
1. THESE CONNECTIONS ARE TYPICAL, UNO.
2. NUMBER OF BOLTS IN UPSET BEAM CONNECTIONS DETERMINED BY SMALLER OF TWO BEAMS AT CONNECTION.
3. ALL AROUND 1/4" FILLET WELD MAY BE SUBSTITUTED FOR EITHER BOLTED CONNECTION.
4. ALL BOLTS 3/4" DIAMETER, A325-N, UNO.



2 BEAM SPLICE DETAIL

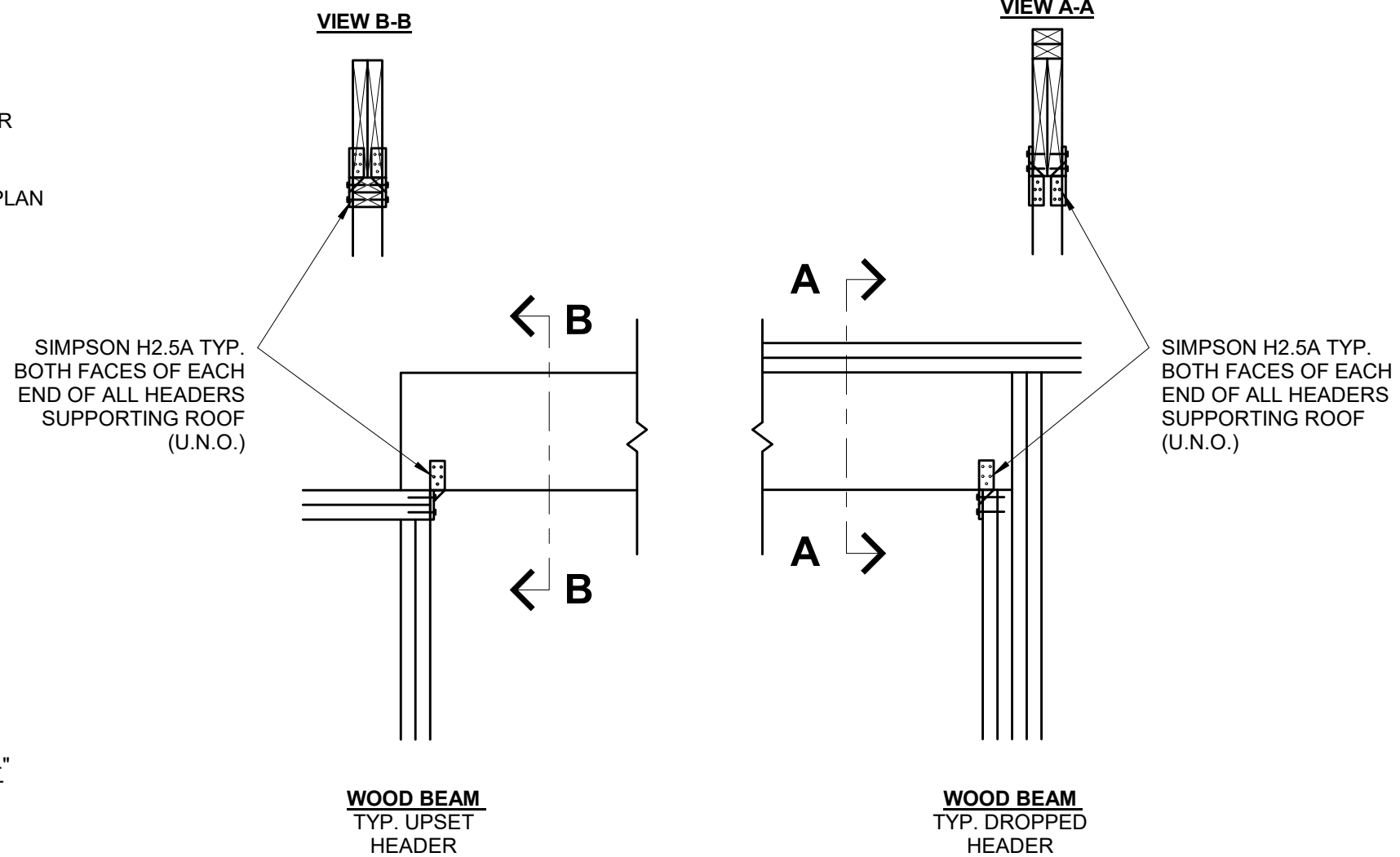
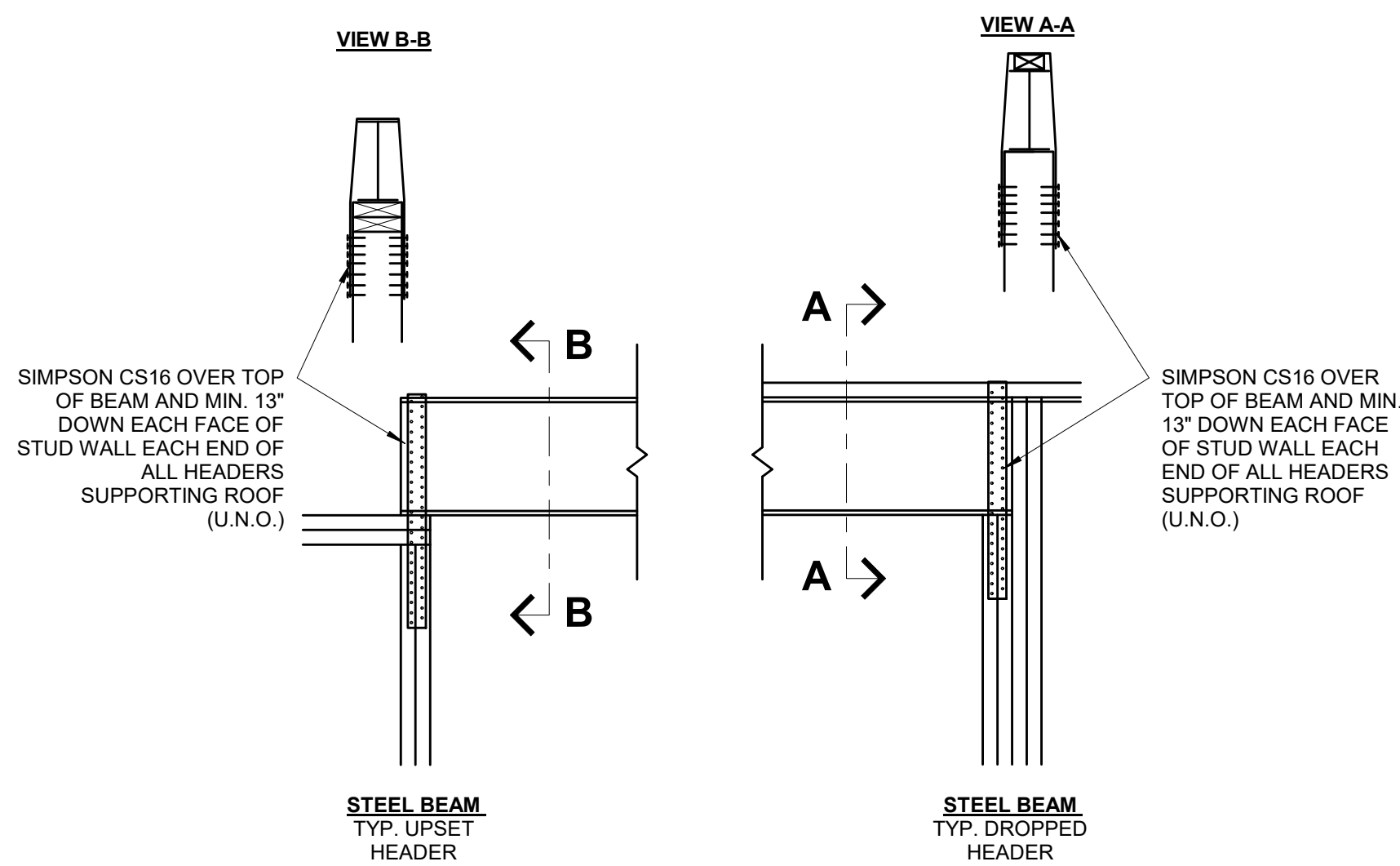
S3.1 1 1/2" = 1'-0"



NOTES:
1. EACH 2x PLY SHALL BE FASTENED WITH (1) ROW OF 10d NAILS AT 9" OC, ALTERNATING SIDE TO SIDE.
2. 1.4" MIN EDGE DISTANCE, AND STARTING 2 1/2" FROM EACH END.
3. EXTEND FULL AREA OF COLUMN AS SOLID BLOCKING THROUGH JOIST BAYS AND WALLS TO LOAD-BEARING BEAM/WALL BELOW.

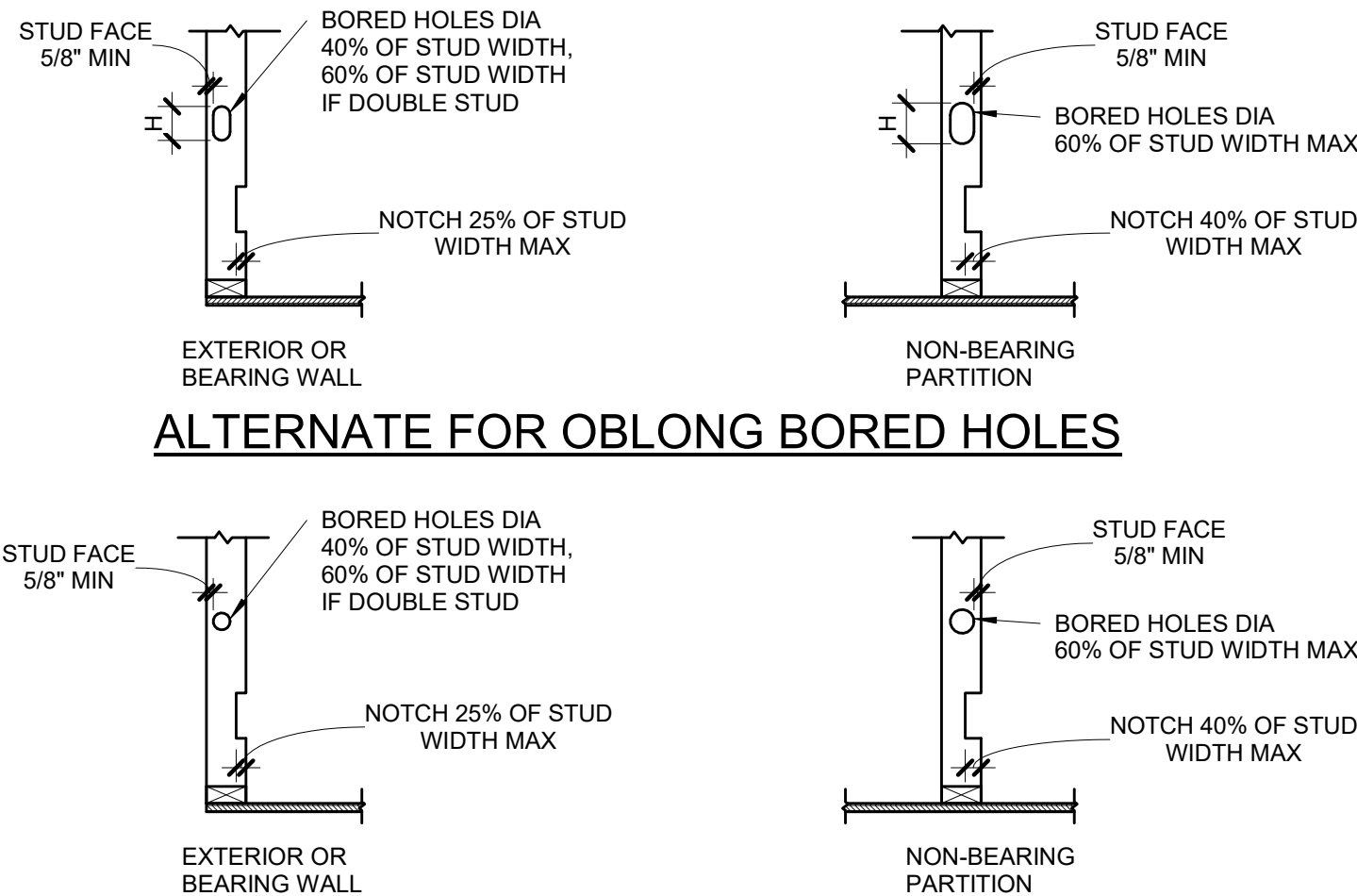
1 BUILT-UP STUD COLUMN

S3.1 1 1/2" = 1'-0"



6 ROOF SUPPORTING BEAM HOLD DOWN

S3.1 3/4" = 1'-0" (COMPLIANCE WITH IRC R802.11)



ALTERNATE FOR OBLONG BORED HOLES

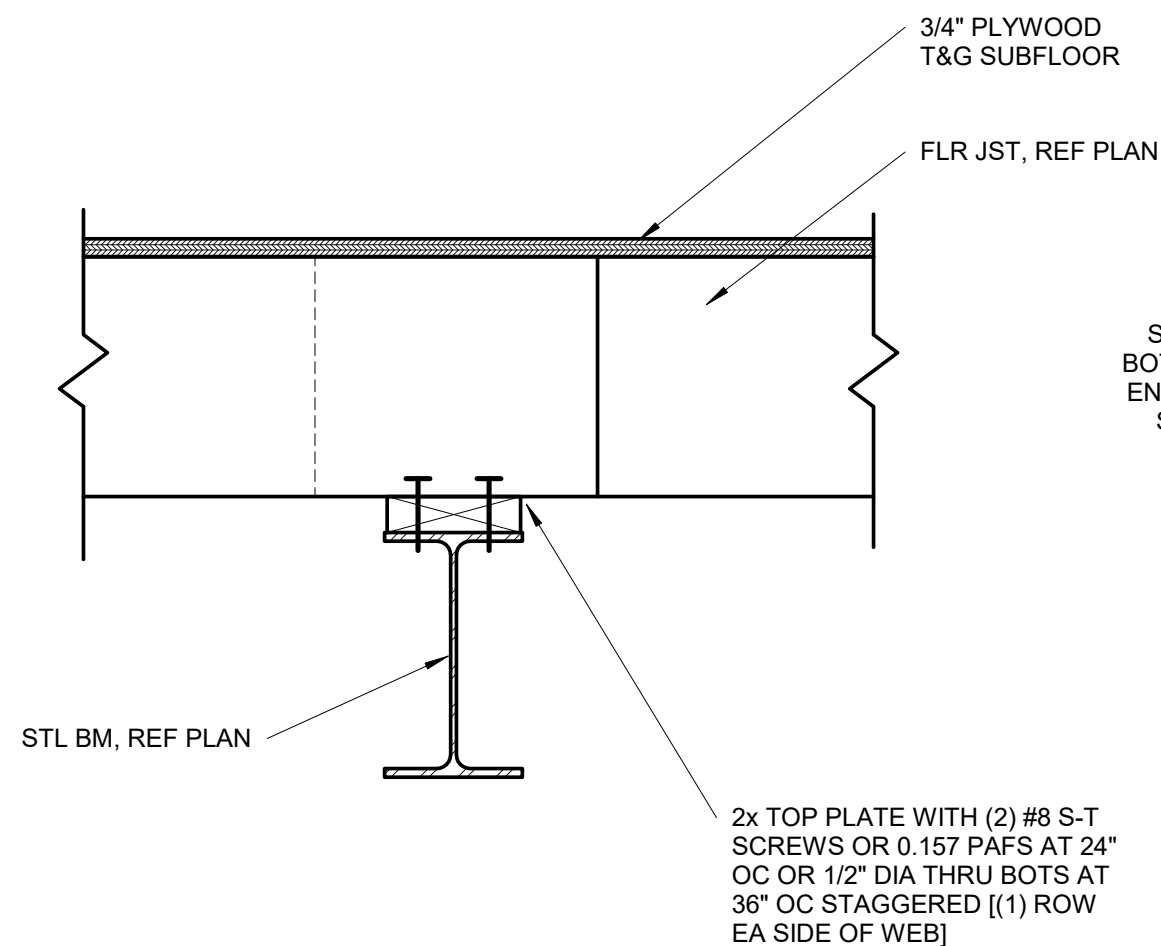
WALL SIZE	BORED HOLE SIZE				WALL NOTCH	
	STUDS LOAD BEARING OR EXTERIOR WALL	NON LOAD BEARING WALL	LOAD BEARING WALL	NON LOAD BEARING WALL	LOAD BEARING WALL	NON LOAD BEARING WALL
2x4	1 3/8"	2 1/8"	2 1/8"	7/8"	1 3/8"	1 3/8"
(2) 2x4	-	2 1/8"	2 1/8"	7/8"	1 3/8"	1 3/8"
2x6	2 1/4"	-	3 15/16"	1 3/8"	2 1/4"	2 1/4"
(2) 2x6	-	3 5/16"	3 15/16"	1 3/8"	2 1/4"	2 1/4"
2x8	2 7/8"	-	4 3/8"	1 13/16"	2 7/8"	2 7/8"
(2) 2x8	-	4 3/8"	4 3/8"	1 13/16"	2 7/8"	2 7/8"

PLATES:
TOP AND BOTTOM PLATE HOLE, CUT OR NOTCH THAT IS 50% MORE OF WIDTH MUST BE REPAIRED USING 16 GA (MIN) METAL TIE THAT IS AT LEAST 1-1/2" WIDE IF WALL IS A SHEAR WALL IT MUST BE REPAIRED USING HARDY FRAME SADDLE (HFS).

NOTE:
SEE SECTION R602.6 AND FIGURES R602.6.1 AND R602.6.2

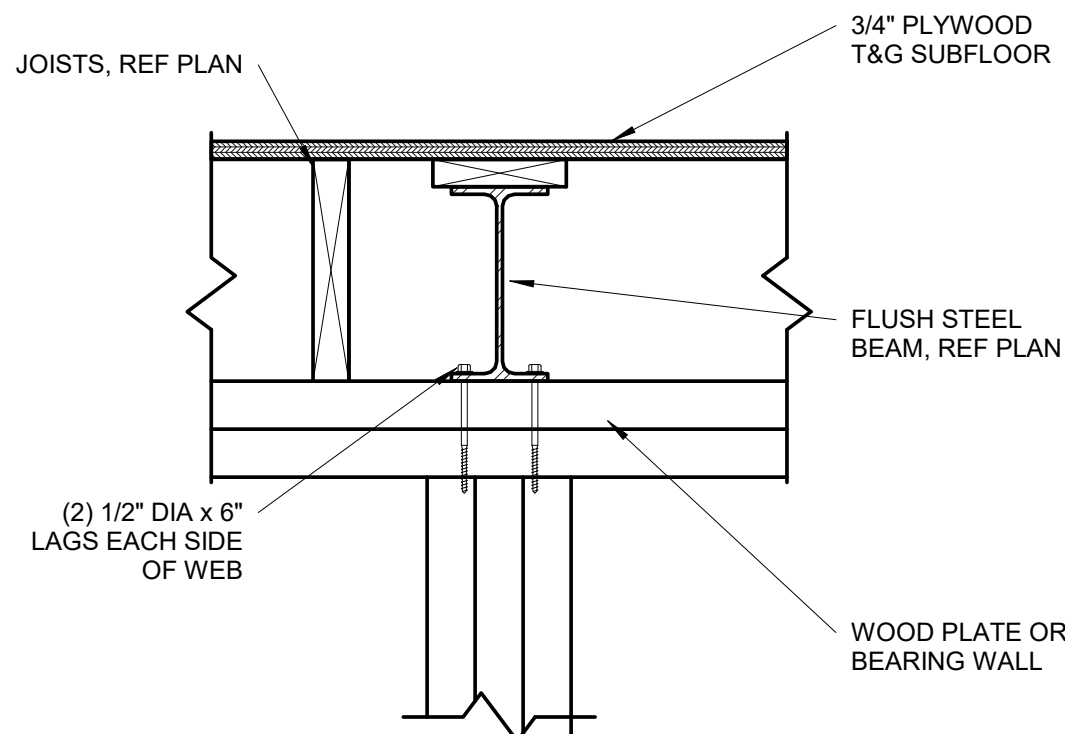
5 DRILLING & NOTCHING DETAIL

S3.1 3/4" = 1'-0"



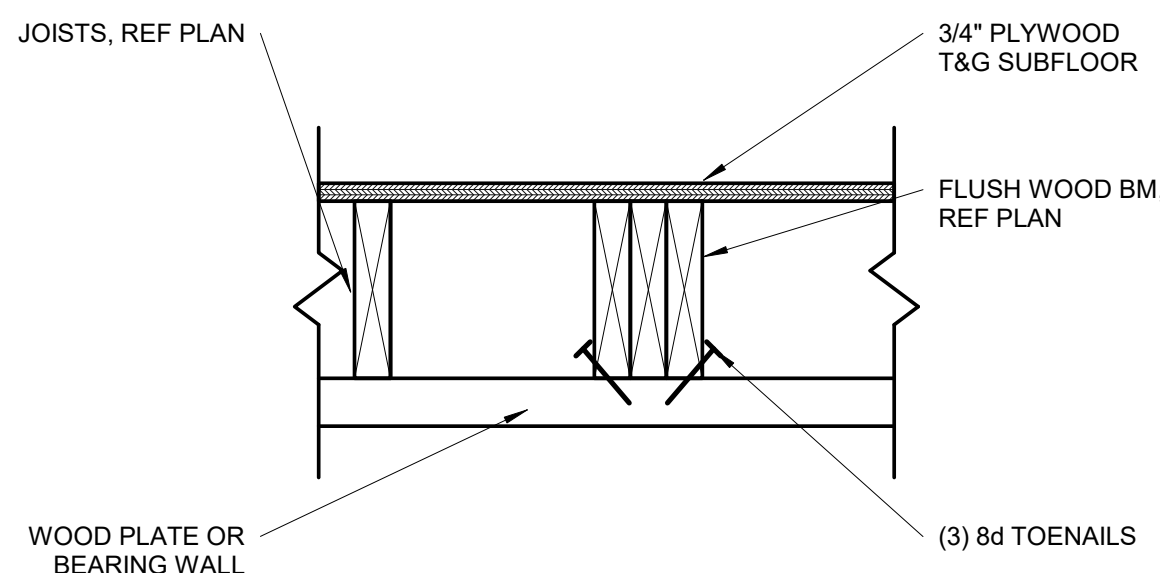
9 WOOD PLATE TO STEEL BEAM CONNECTION

S3.1 1 1/2" = 1'-0"



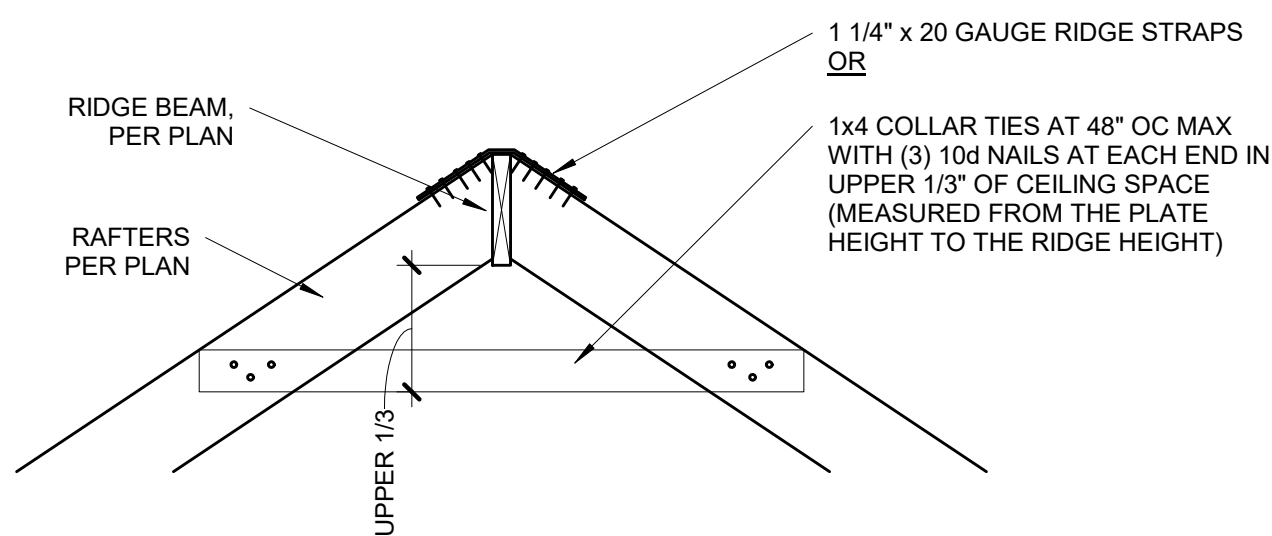
8 FLUSH STEEL BEAM CONNECTION

S3.1 1 1/2" = 1'-0"



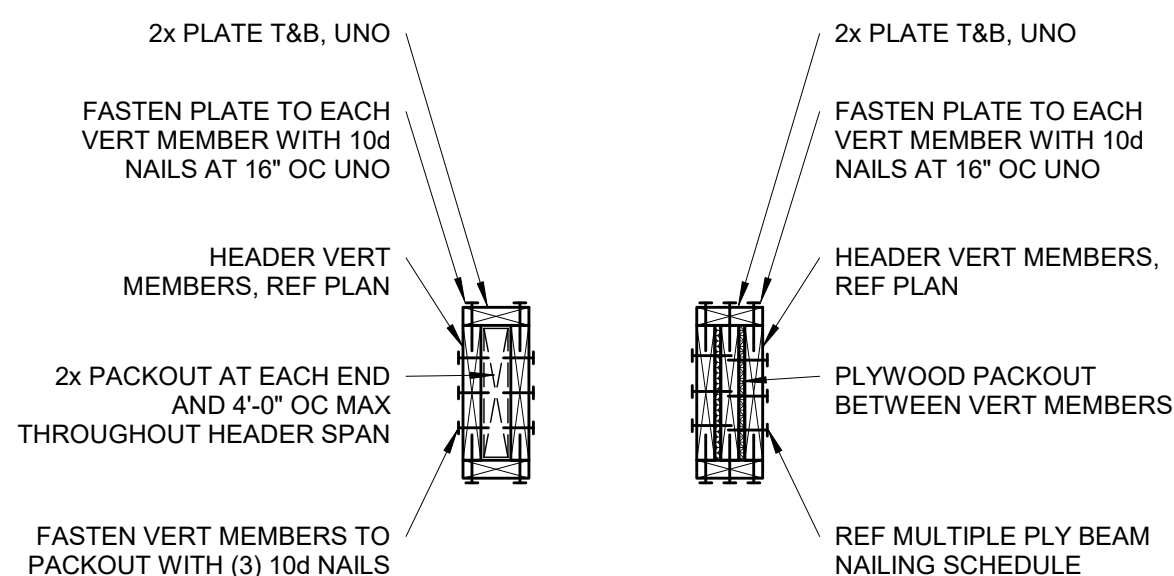
7 FLUSH WOOD BEAM CONNECTION

S3.1 1 1/2" = 1'-0"



12 RIDGE BEAM DETAIL

S3.1 3/4" = 1'-0"



11 TYPICAL WOOD HEADER DETAIL

S3.1 NOT TO SCALE

2 - PLY	3 - PLY	4 - PLY
(3) ROWS OF 16d x 3-1/2" NAILS AT 6" OC	(3) ROWS OF 16d x 3-1/2" NAILS AT 4" OC	(2) ROWS OF 1/2" DIA. A307 THRU-BOLTS AT 12" OC STAGGERED
NOTES: 1. NAILING SHOWN APPLIES UNLESS SPECIFICALLY NOTED IN DETAILS. 2. SPACE NAILS EVENLY THROUGHOUT DEPTH OF BEAM.		

10 MULTIPLE PLY BEAM NAILING SCHEDULE

S3.1 NOT TO SCALE

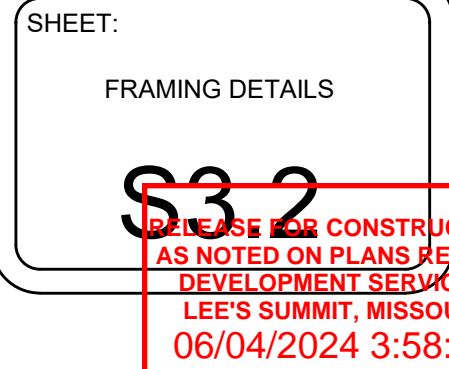
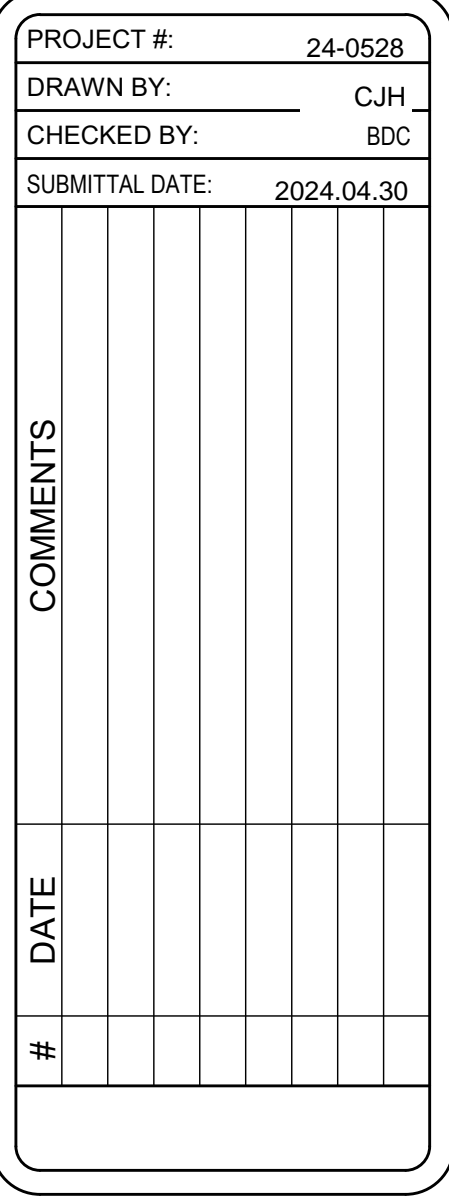
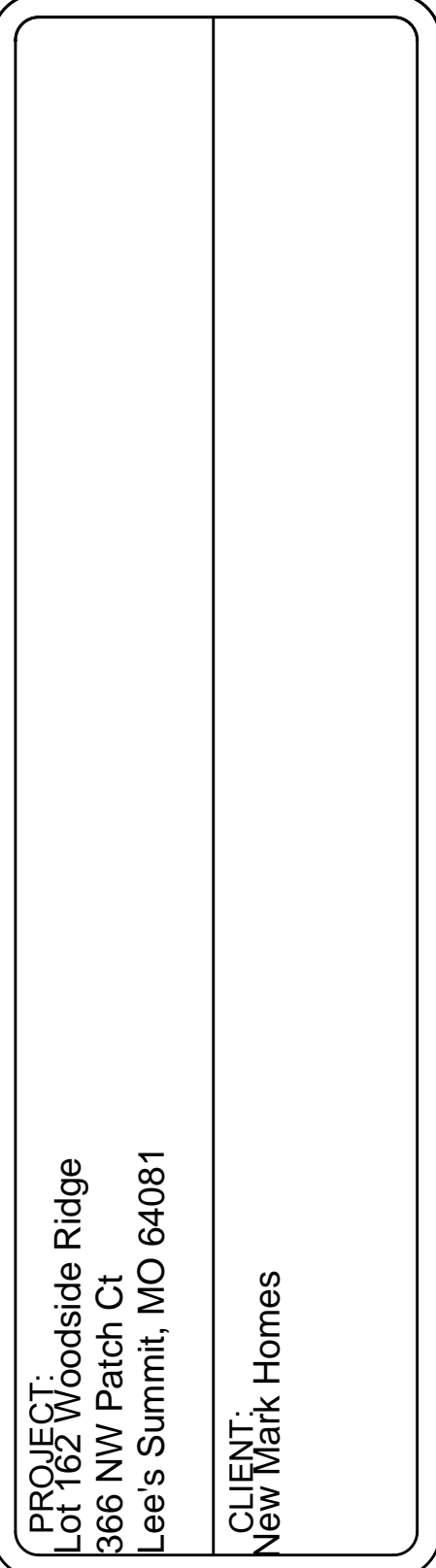


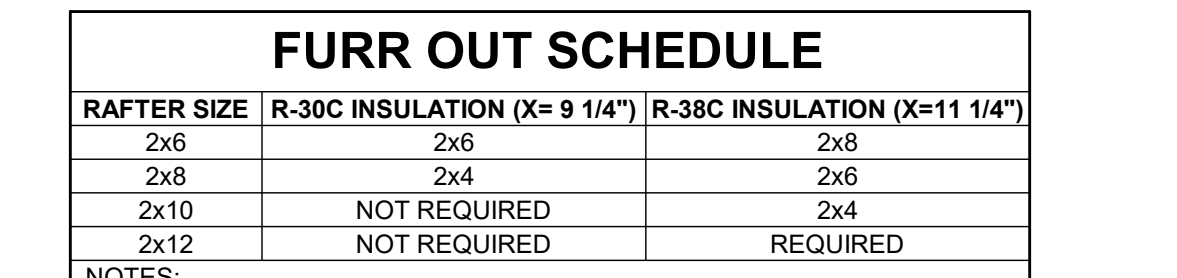
Diagram illustrating a rafter beam supporting an outlooker backspan. The beam is shown as a horizontal line with vertical supports at each end. The text "RAFTER BEAM SUPPORTING OUTLOOKER BACKSPAN" is positioned above the beam, with an arrow pointing to the beam itself.

5 | OUTLO
FRAMIN
S3.2 NOT TO SCALE

1. VALLEYS OF A LENGTH GREATER THAN THAT FOUND IN THE CELL ABOVE ARE CONTROLLED BY THE MAXIMUM ALLOWABLE NUMBER OF SISTER PLIES.
2. THIS TABLE IS INTENDED TO BE USED IN CONJUNCTION WITH THE STAMPED, ENGINEERED PLANS AS THEY ARE DRAWN BY APEX. BRACING LOCATIONS SHALL DETERMINE HORIZONTAL, UNSUPPORTED SPAN FROM VALLEY BEARING AND BE USED TO DETERMINE THE NUMBER OF SISTERS REQUIRED. BRACING LOCATIONS ARE **NOT** TO BE INFERRED USING THIS TABLE.
3. TABLE VALUES ARE BASED ON A DEPTH OF MEMBER REMAINING, d , EQUAL TO THE DEPTH OF THE RAFTERS. IF d IS OBSERVED TO BE LESS THAN THE DEPTH OF THE RAFTER, THE VALUE OF d SHALL BE USED TO BE USED TO BE DETERMINED BY APEX.
4. TABLE VALUES ARE VALID FOR TAPERED CUTS ONLY. REF DETAIL 4/33.2.
5. IF MULTI-PLY VALLEY IS SPECIFIED ON PLAN TABLE EACH ADDITIONAL PLY AS A SISTER PLY WHEN LOOKING UP MAX SPAN.
6. MAX 14'-0" HORIZONTAL RAFTER SPAN IN BOTH DIRECTIONS FROM VALLEY.
7. ALL HIPS ARE DESIGNED TO BE CONTROLLED BY BENDING. SHEAR AT BEARING WITH MIN 5 1/2" DEPTH DOES NOT CONTROL DESIGN.

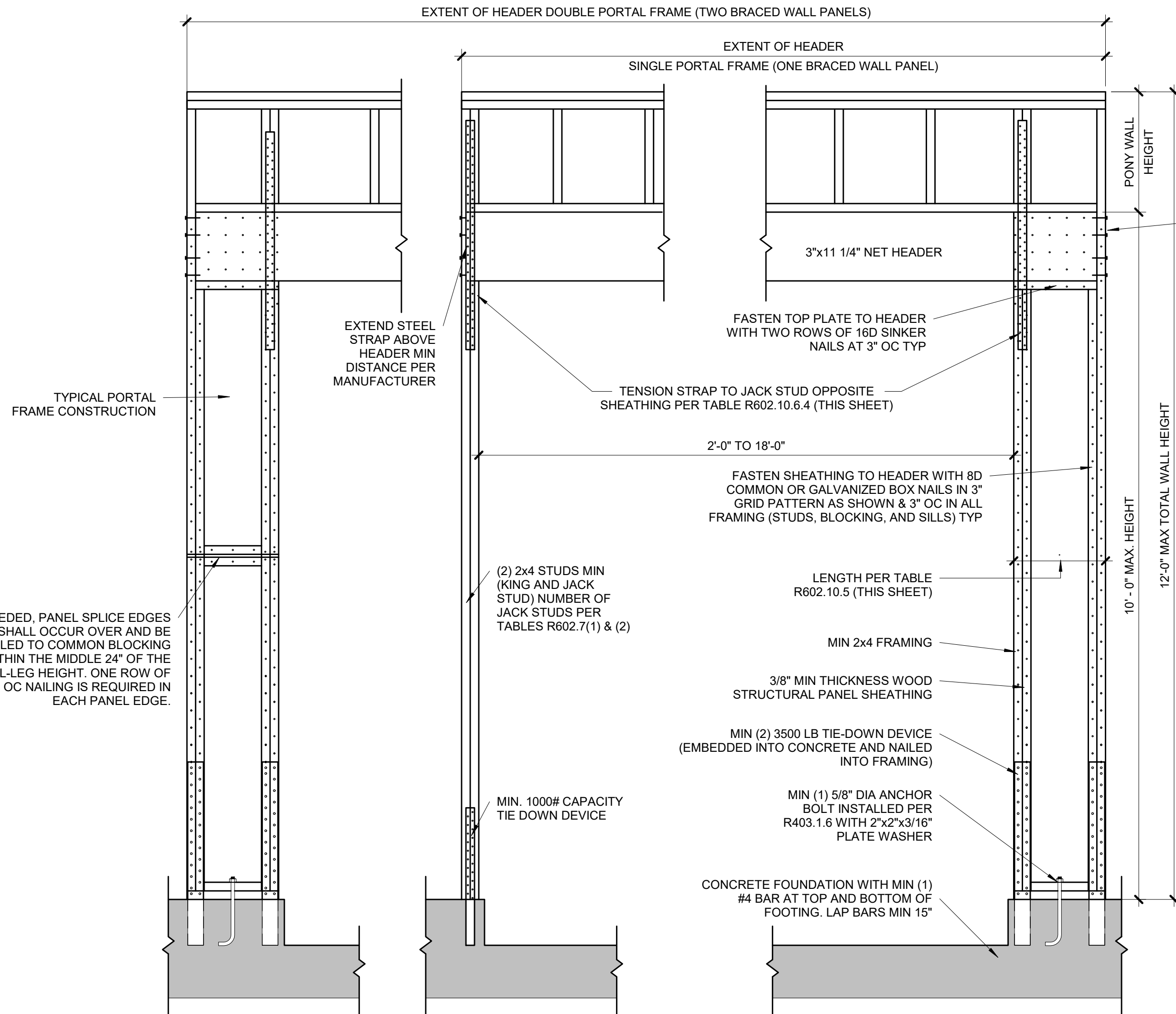


NOTES:
-CHART IS ONLY APPLICABLE IF NO
RAFTER BEAM SHOWN ON PLAN.
-CONTACT EOR IF OVERHANG LENGTH
EXCEEDS CHART OPTIONS.
-ALTERNATE, REF BARGE RAFTER DETAIL
FOR OVERHANGS 1'-0" OR LESS.



1. ALL VAULTED RAFTERS SHALL BE 2x26 DFL, MINIMUM, AT 16" OC, PER SPAN CHART, UNLESS NOTED OTHERWISE.
2. ALL VAULTS SHALL BE FURRED DOWN WITH 2x FRAMING TO THE REQUIRED DEPTH OF INSULATION, PLUS 1" AIR SPACE.
3. R-30 INSULATION = 8 1/4" THICK
4. R-38 INSULATION = 10 1/4" THICK
5. INSULATION REQUIREMENTS MAY BE REDUCED TO R30 IF ROOF/CEILING ASSEMBLY DOES NOT ALLOW SUFFICIENT SPACE BUT IS LIMITED TO VAULTED CEILING AREAS THAT ARE LESS THAN 500 SQUARE FEET OR 20 PERCENT OF THE TOTAL INSULATED CEILING AREA, WHICHEVER IS LESS. (PER N1102.2.2)





SECTION

FASTEN KING STUD
TO HEADER WITH (8)
16d SINKER NAILS

FASTEN TOP PLATE TO
HEADER WITH TWO
ROWS OF 16d SINKER
NAILS AT 3" OC, TYP

MIN 7/16" WOOD
STRUCTURAL PANEL
SHEATHING

IF NEEDED, PANEL SPLICE EDGES
SHALL OCCUR
OVER AND BE NAILED TO
COMMON BLOCKING
WITHIN THE MIDDLE 24" OF
THE PORTAL-LEG HEIGHT. ONE ROW OF 3" OC NAILING
IS REQUIRED IN EACH
PANEL EDGE.

EXTENT OF HEADER DOUBLE PORTAL FRAME (TWO BRACED WALL PANELS)

EXTENT OF HEADER
SINGLE PORTAL FRAME (ONE BRACED WALL PANEL)

TOP PLATE CONTINUITY
REQUIRED

TENSION STRAP OPPOSITE
SHEATHING, PER TABLE R602.10.6.4
(THIS SHEET)

FASTEN SHEATHING TO HEADER WITH 8D
COMMON OR GALVANIZED BOX NAILS IN 3" GRID
PATTERN AS SHOWN AND 3" OC IN ALL FRAMING
(STUDS, BLOCKING, AND SILLS) TYP

LENGTH PER TABLE
R602.10.5 (THIS SHEET)

MIN 2x4 FRAMING

7/16" MIN THICKNESS WOOD
STRUCTURAL PANEL SHEATHING

1/2" DIA. ANCHOR BOLT WITH 2"x2"x3/16"
PLATE WASHER 7" MIN EMBEDMENT, TYP.

CONCRETE FOUNDATION WITH MIN
(1) #4 BAR AT TOP AND BOTTOM OF
FOOTING. LAP BARS MIN 15"

PORTAL FRAME AT GARAGE DOOR WITHOUT HOLD DOWNS (METHOD PFG)

S4.0 3/4" = 1'-0" (ALT ALLOWED AT GARAGE DOOR ONLY)
ALT (PER IRC R602.10.6.3)

TABLE R602.10.5 (PARTIAL)					
MINIMUM LENGTH OF BRACED WALL PANELS					
METHOD	MIN LENGTH (INCHES)				
	WALL HEIGHT				
	8 FEET	9 FEET	10 FEET	11 FEET	12 FEET
SUPPORTING ROOF ONLY	16	16	16	16	16
ONE STORY AND ROOF	24	24	24	24	24
PFG	24	27	30	30	30

NOTE: MAX HEADER HEIGHT IS 10'-0", BUT WALL HEIGHT SHALL BE
PERMITTED TO BE INCREASED TO 12'-0" WITH PONY WALL

TABLE R602.10.6.4

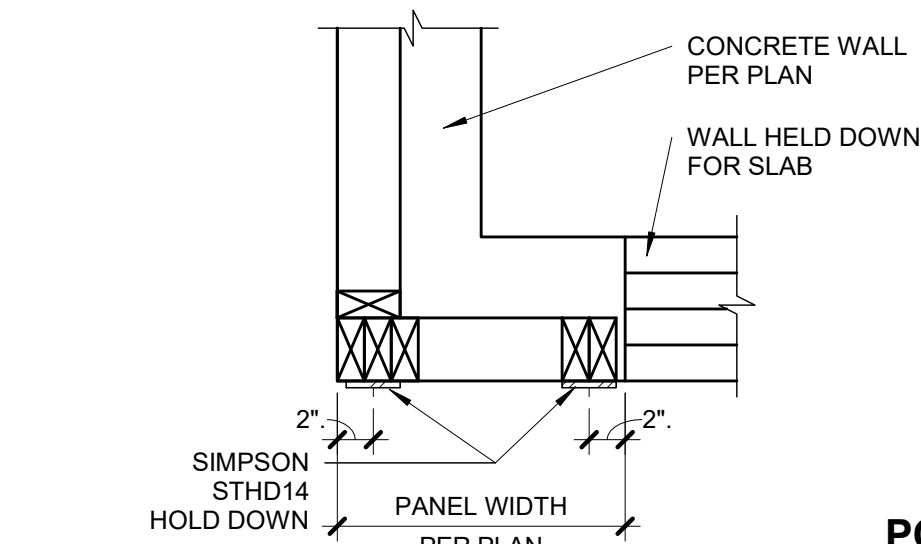
TENSION CAPACITY STRAP TABLE				
MIN WALL STUD FRAMING NOMINAL SIZE AND GRADE	MAX PONY WALL HEIGHT (FEET)	MAX TOTAL WALL HEIGHT (FEET)	MAX OPENING WIDTH (FEET)	TENSION STRAP CAPACITY REQ (LBS) 115 MPH, EXP B
2x4 #2 GRADE	0	10	18	1,000
			9	1,000
			16	1,025
	1	10	18	1,275
			9	1,000
			16	2,175
	2	10	18	2,500
			9	1,500
			16	3,375
	4	12	18	3,975
			9	2,750
			12	3,775
2x6 STUD GRADE	2	12	9	1,000
			16	2,150
			18	2,550
	4	12	9	1,750
			16	2,400
			18	3,800

BRACED WALL METHODOLOGY
CONTINUOUS EXTERIOR SHEATHING (CS-WSP) PER WSP METHOD (BELOW)
UNLESS OTHERWISE NOTED ON THE PLAN

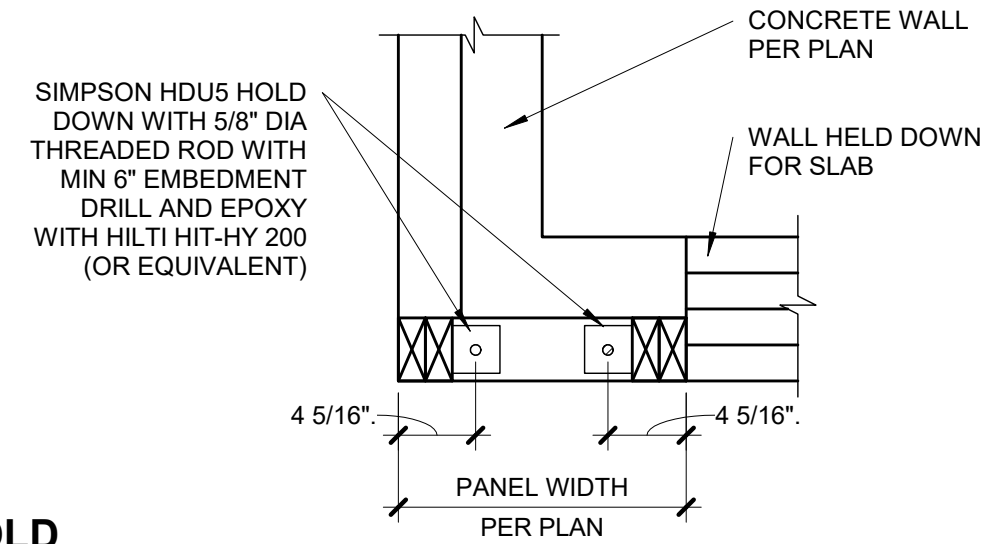
XXXX EXTERIOR BRACED WALLS:
WSP METHOD:
WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 3/8" WITH MINIMUM SPAN RATING OF
2400 FOR 16" OC STUD. SPACING WITH 6d COMMON NAILS AT 6" OC EDGES AND 12" OC FIELD OR SHEATHING
THICKNESS NOT LESS THAN 7/16" WITH MINIMUM SPAN RATING OF 24/16 FOR 24" OC SPACING WITH 8d COMMON
NAILS AT 6" OC EDGES AND 12" OC IN FIELD.
(NOTE: FRAMING MEMBERS 16" OC MAX, UNBLOCKED, AND WITH SHEATHING APPLIED DIRECTLY TO FRAMING
MEMBERS)

///// INTERIOR BRACED WALLS (REF 2/S4.0):
GB METHOD:
1/2" MIN GYPSUM BOARD OVER STUDS SPACED 24" MAX FASTENED WITH #6 - 1 1/4" TYPE 'W' OR 'S' DRYWALL
SCREWS AT 7" OC EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES.)

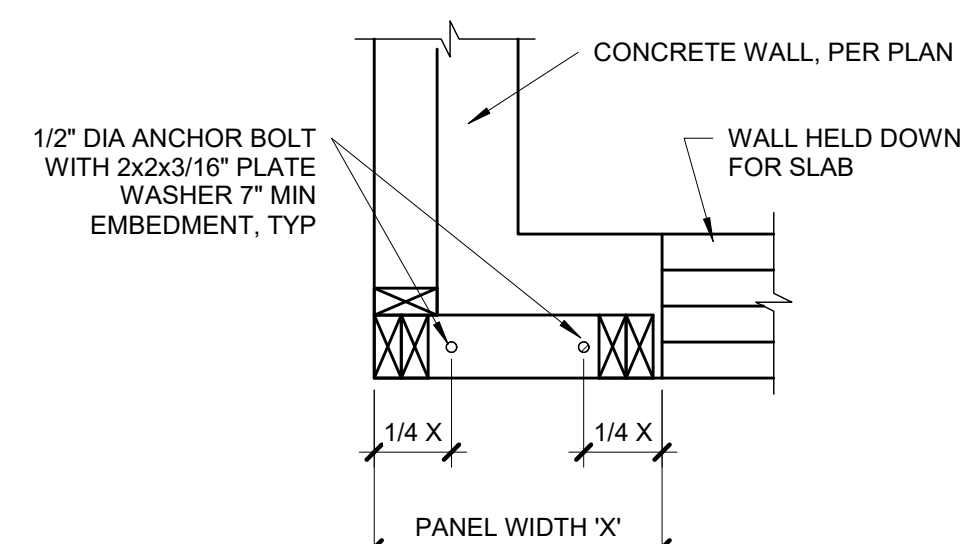
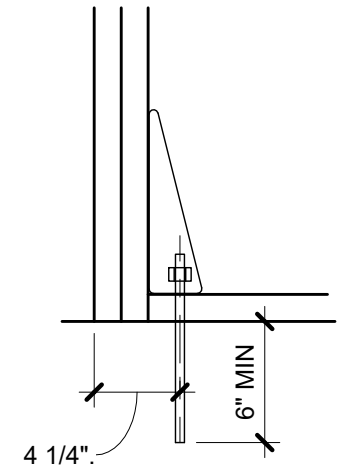
LIB METHOD:
1x4 WOOD FASTENED WITH (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA TYPE WB (OR EQUAL) STL X-BRACE(S)
AT 45° TO 60° ANGLES, MAXIMUM 16" OC STUD FASTENED PER MANUFACTURER'S SPECIFICATIONS.



PLAN VIEW - ALTERNATE BRACED WALL PANEL



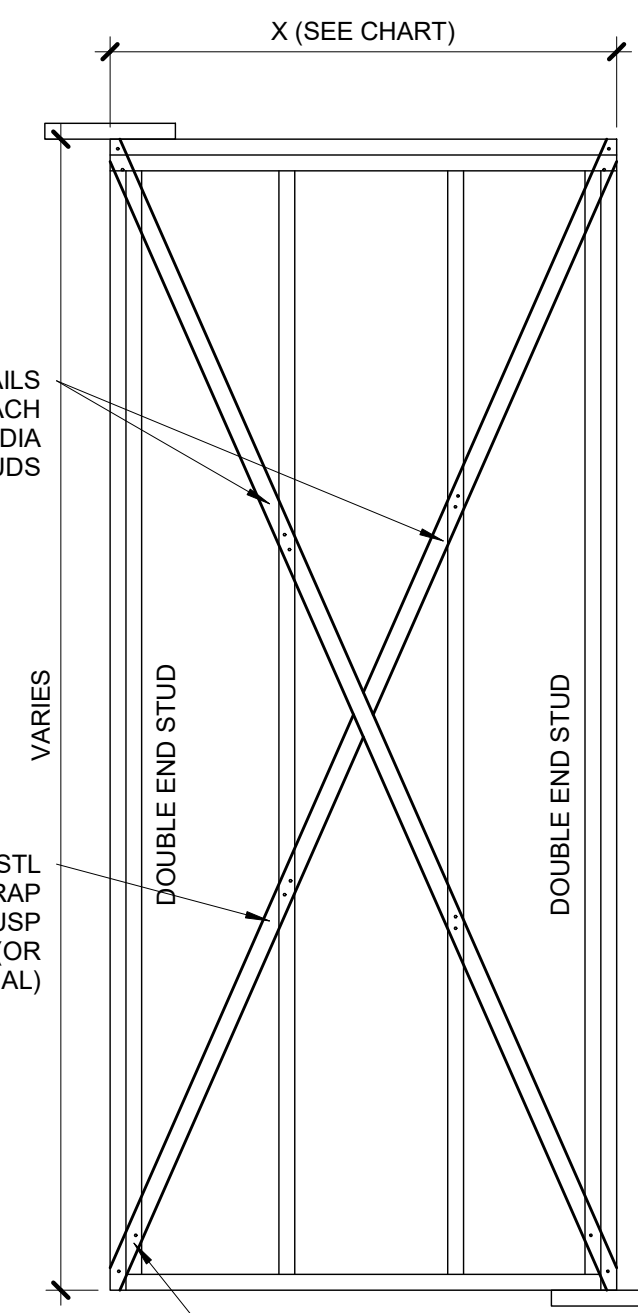
PLAN VIEW - ALTERNATE BRACED WALL
PANEL DRILL AND EPOXY OPTION



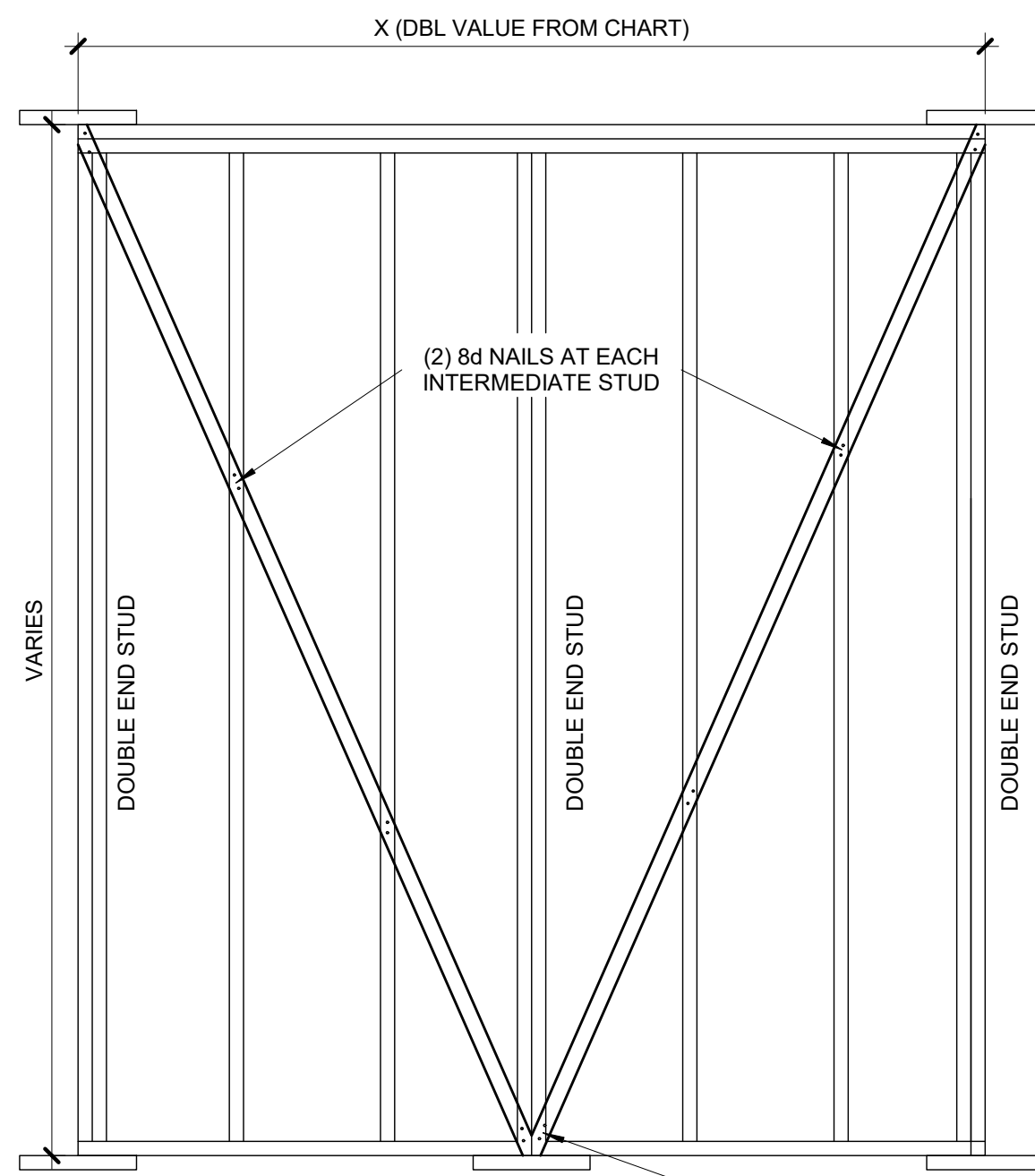
PLAN VIEW - APA NARROW WALL BRACING
METHOD WITHOUT HOLD-DOWNS

1 PORTAL FRAME WITH HOLD DOWNS (METHOD PFH)

S4.0 3/4" = 1'-0" (PER IRC R602.10.6.2)



LIB

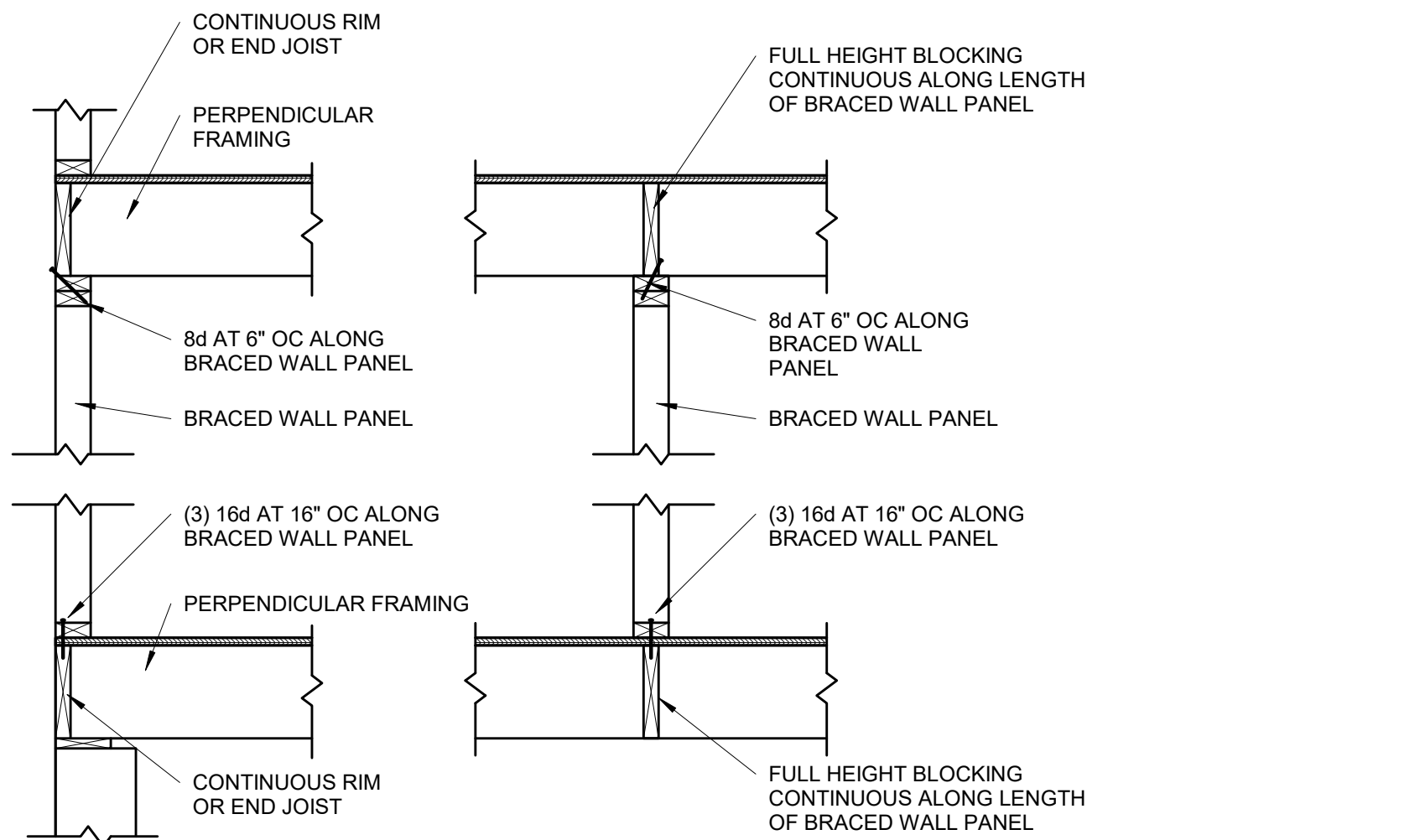


BRACED WALL PANEL SCHEDULE		
WALL HEIGHT	MIN WALL LENGTH (X)	MAX WALL LENGTH (X)
8'-0"	4'-7"	8'-0"
9'-0"	5'-2"	9'-0"
10'-0"	5'-9"	10'-0"
11'-0"	NP	-
12'-0"	NP	-

NOTE: BRACED WALL PANEL LENGTHS BASED ON WALL HEIGHT
FOR IRC, LIB

2 BRACED WALL PANEL-IRC METHODS LIB AND GB

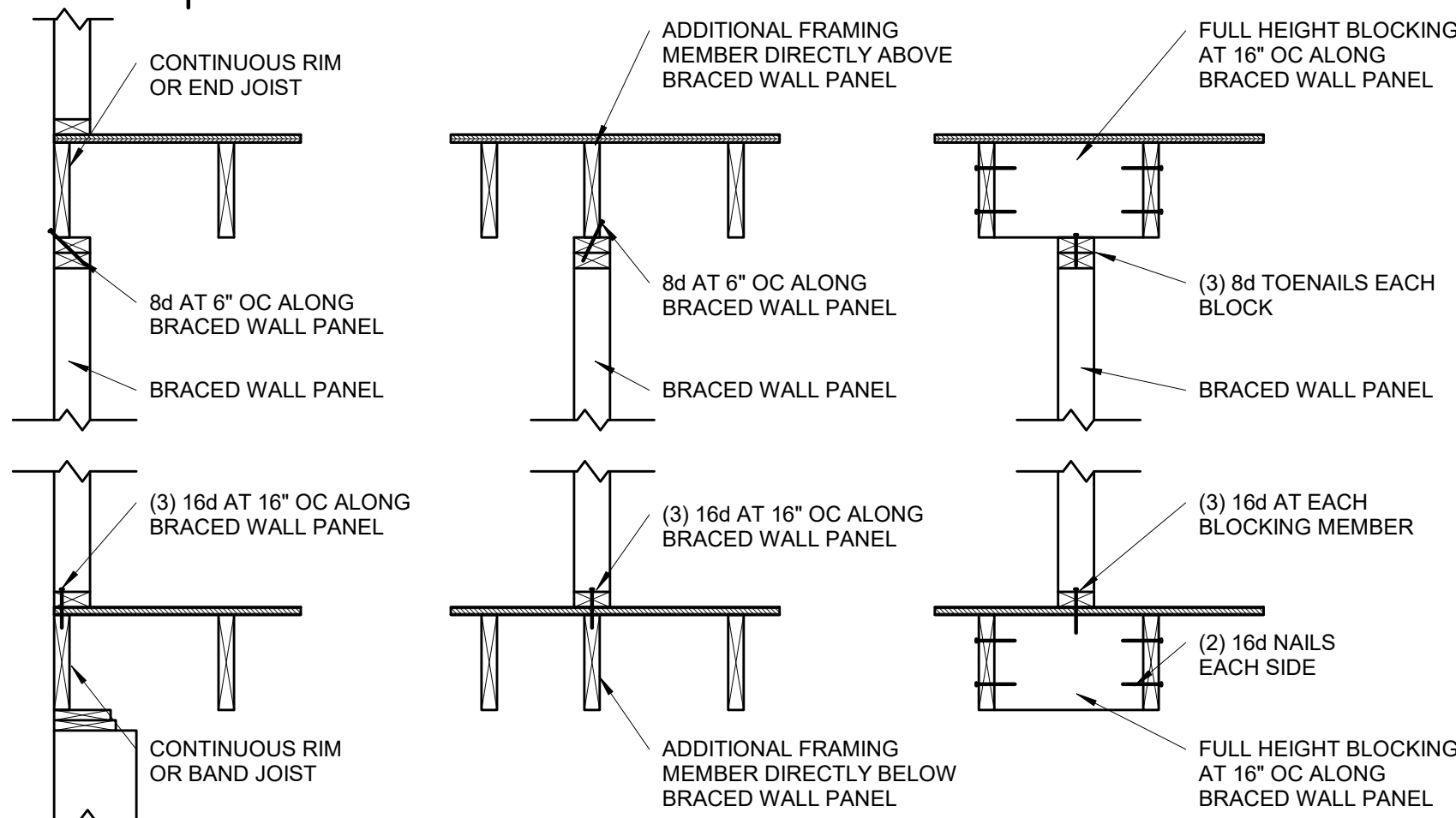
S4.0 3/4" = 1'-0"



**BRACED WALL PANEL
CONNECTION WHEN
PERPENDICULAR TO
FLOOR/CEILING FRAMING**

S4.1

3/4" = 1'-0"

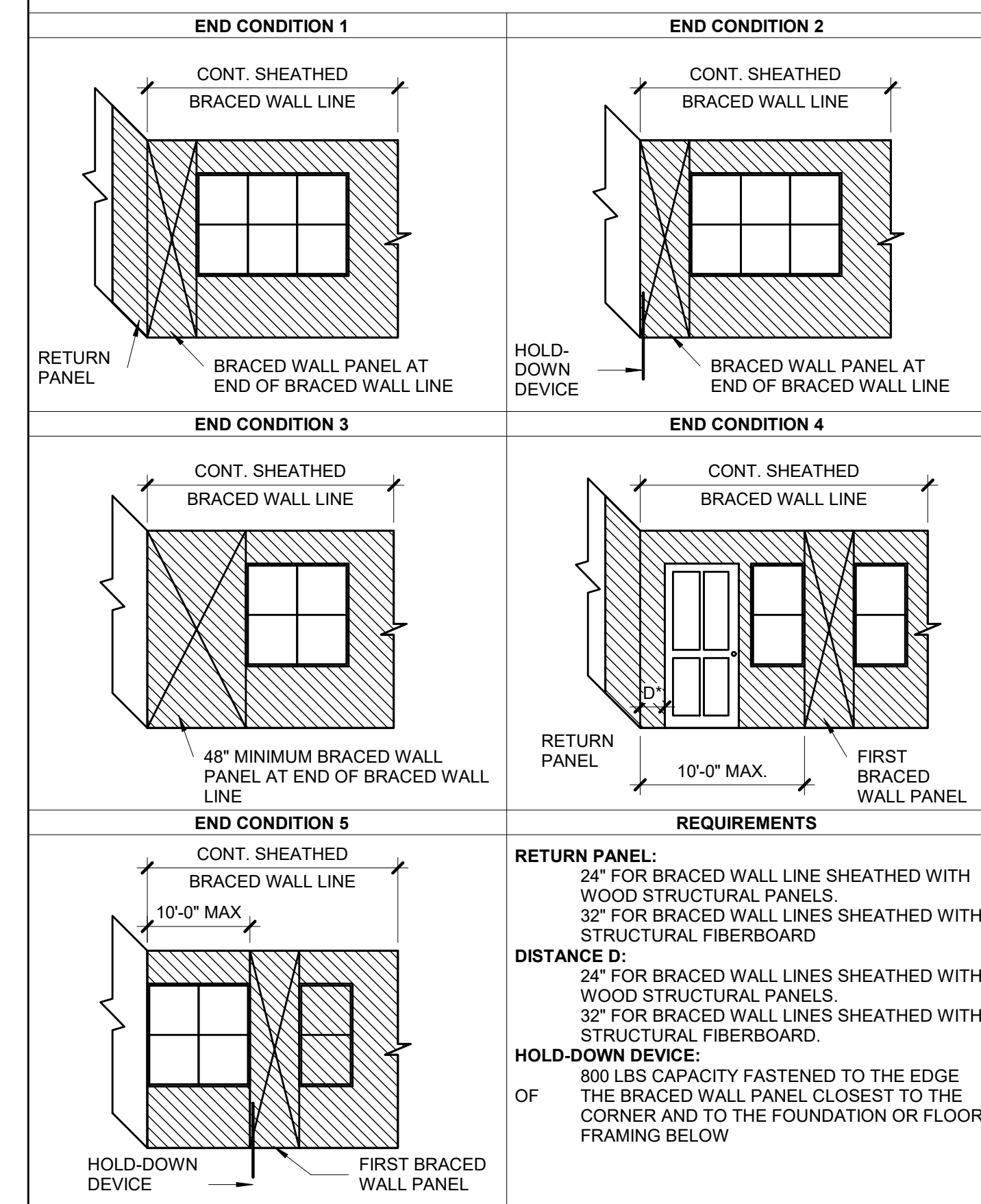


**BRACED WALL PANEL
CONNECTION WHEN PARALLEL
TO FLOOR/CEILING FRAMING**

S4.1

3/4" = 1'-0"

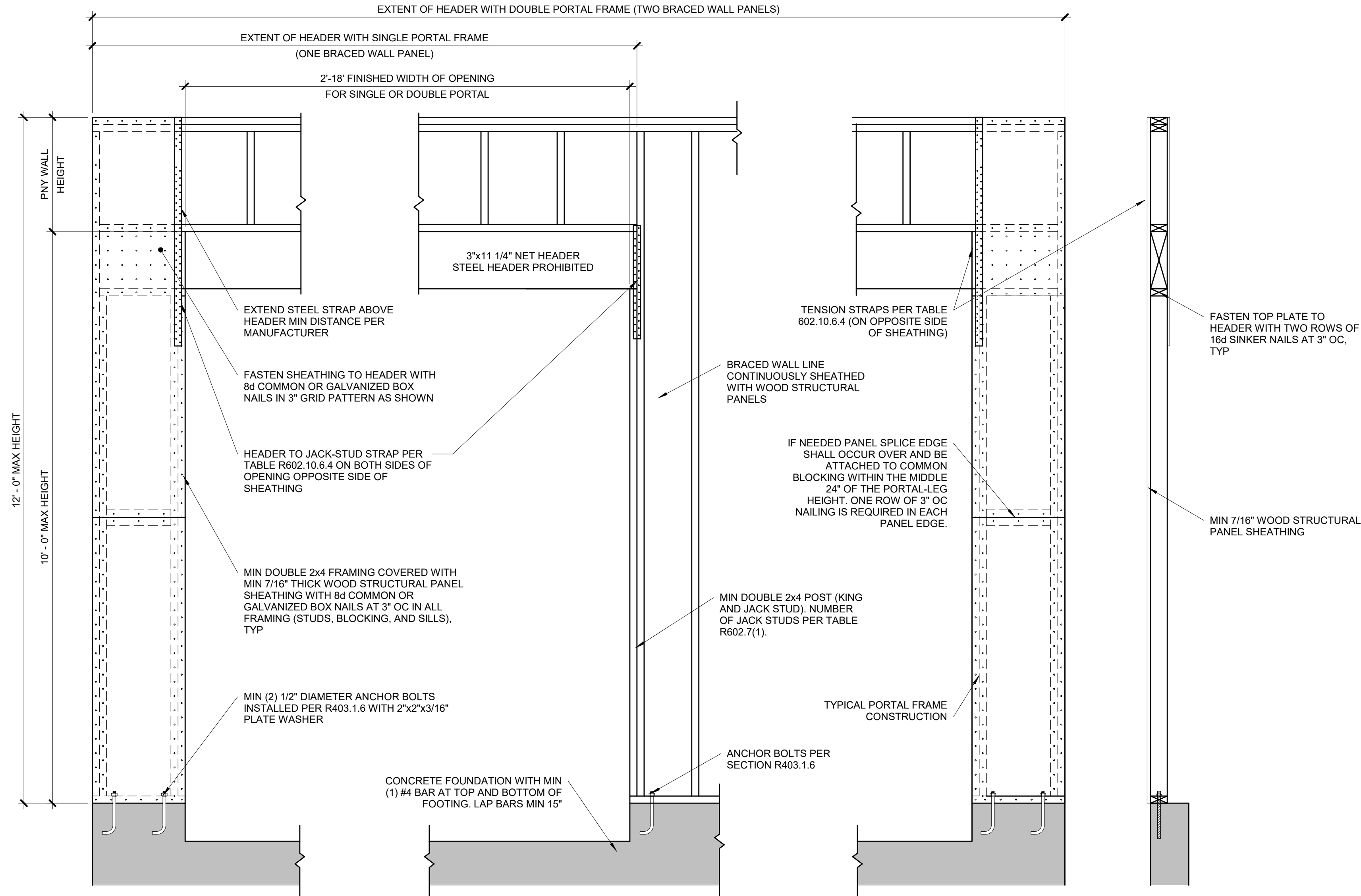
CONT. SHEATHED BRACED WALL END CONDITIONS



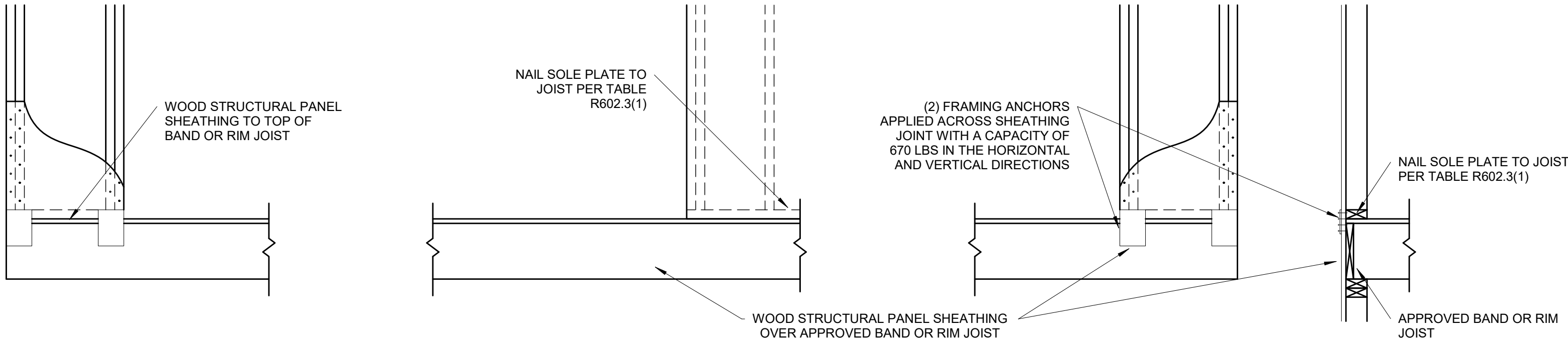
**CONTINUOUS SHEATHED BRACED
WALL END CONDITIONS**

S4.1

NOT TO SCALE (COMPLIANCE WITH IRC R602.10.7)

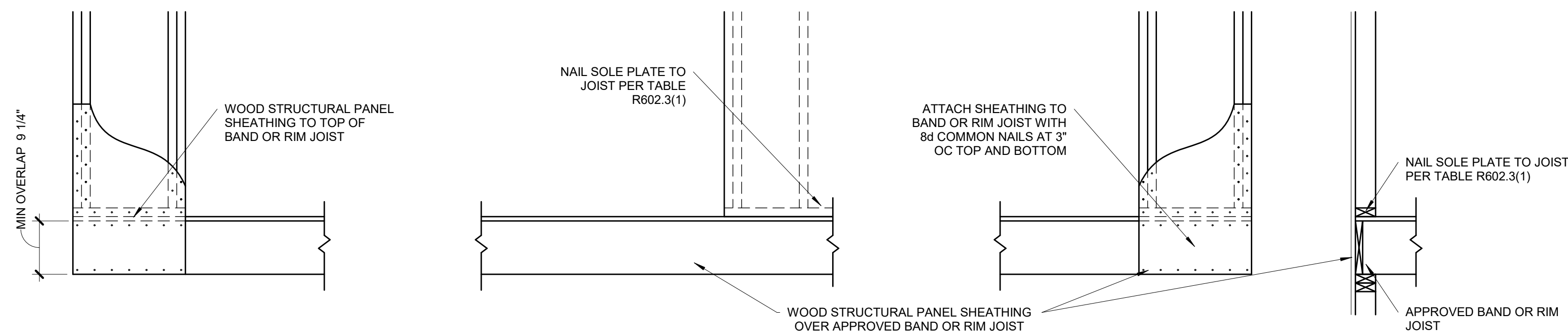


OVER CONCRETE OR MASONRY BLOCK FOUNDATION



OVER RAISE WOOD FLOOR - FRAMING ANCHOR OPTION

(WHEN PORTAL SHEATHING DOES NOT LAP OVER BAND OR RIM JOIST)



OVER RAISE WOOD FLOOR - OVERLAP OPTION

(WHEN PORTAL SHEATHING LAPS OVER BAND OR RIMBOARD)

**BRACED WALL PANEL-IRC
METHOD CS-PF CONTINUOUSLY
SHEATHED PORTAL FRAME
1 PANEL CONSTRUCTION**

S4.1

3/4" = 1'-0"

(PER IRC R602.10.6.4)