

05/13/2024

RE: P240393

MiTek, Inc.  
16023 Swingley Ridge Rd.  
Chesterfield, MO 63017  
314.434.1200

**Site Information:**

Customer: Clayton Properties Project Name: P240393  
Lot/Block: 192 Model:  
Address: 3212 SW Arboridge Cir Subdivision: Hawthorne Ridge  
City: Lee's Summit State: MO

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code: IRC2018/TPI2014 Design Program: MiTek 20/20 8.6  
Wind Code: ASCE 7-16 Wind Speed: 115 mph  
Roof Load: 45.0 psf Floor Load: N/A psf

This package includes 24 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I65080517	A1	4/23/2024	21	I65080537	V6	4/23/2024
2	I65080518	A2	4/23/2024	22	I65080538	V07	4/23/2024
3	I65080519	A3	4/23/2024	23	I65080539	V7	4/23/2024
4	I65080520	A4	4/23/2024	24	I65080540	V8	4/23/2024
5	I65080521	A6	4/23/2024				
6	I65080522	A7	4/23/2024				
7	I65080523	B01	4/23/2024				
8	I65080524	B02	4/23/2024				
9	I65080525	C1	4/23/2024				
10	I65080526	C2	4/23/2024				
11	I65080527	C3	4/23/2024				
12	I65080528	C4	4/23/2024				
13	I65080529	C5	4/23/2024				
14	I65080530	PB1	4/23/2024				
15	I65080531	PB2	4/23/2024				
16	I65080532	V1	4/23/2024				
17	I65080533	V2	4/23/2024				
18	I65080534	V3	4/23/2024				
19	I65080535	V4	4/23/2024				
20	I65080536	V5	4/23/2024				

The truss drawing(s) referenced above have been prepared by  
MiTek USA, Inc. under my direct supervision  
based on the parameters provided by .

Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of Missouri is December 31, 2025.

Missouri COA: 001193

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek. Any project specific information included is for MiTek customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



April 23, 2024

AS NOTED FOR PLAN REVIEW  
DEVELOPMENT SERVICES  
P240893 SUMMIT, MISSOURI

Premier Building Supply (Spring Hill) KS, Spring Hills, KS - 66083,

Truss Type

Common Supported Gable

Qty

Ply

1

1

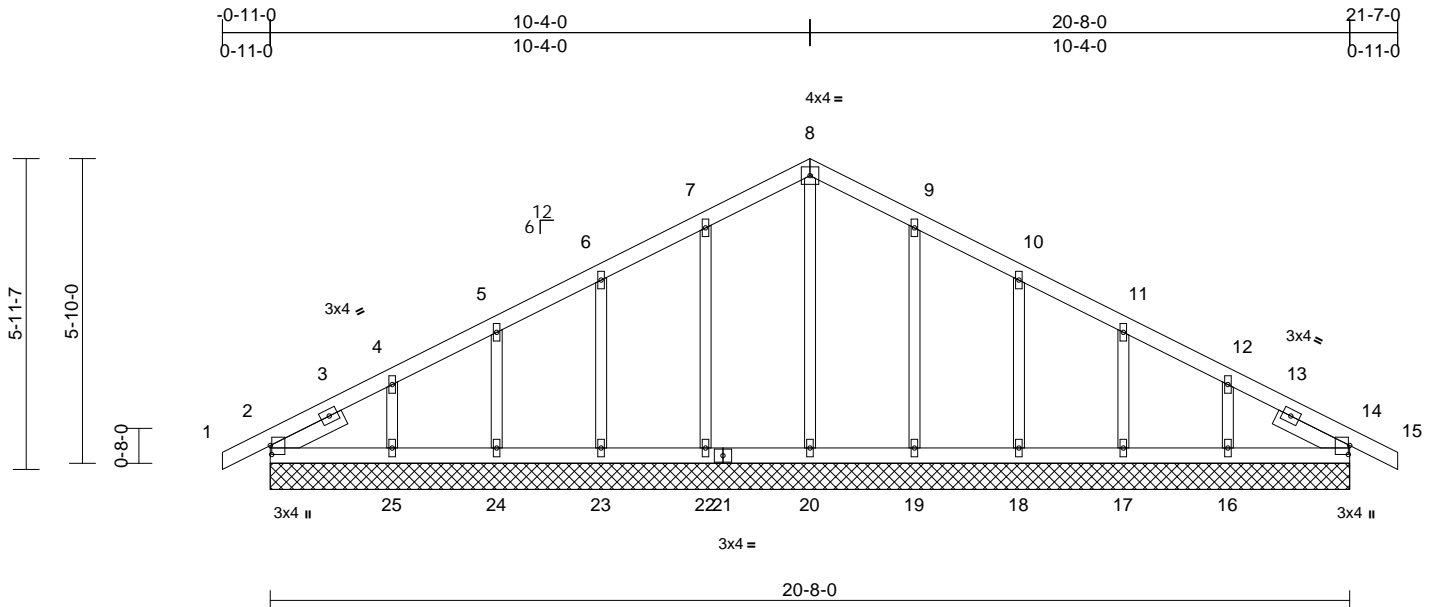
Job Reference (optional)

165080517

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Page: 1

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Scale = 1:44.1

Plate Offsets (X, Y): [2:0-2-1,0-0-5], [14:0-2-1,0-0-5]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	n/a	-	n/a	999	197/144
TCDL	10.0	Rep Stress Incr	YES	WB	0.12	Horz(CT)	0.00	14	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
Weight: 95 lb FT = 20%											

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x3 SPF No.2  
SLIDER Left 2x4 SP No.2 -- 1-6-7, Right 2x4 SP No.2 -- 1-6-7

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size) 2=20-8-0, 14=20-8-0, 16=20-8-0, 17=20-8-0, 18=20-8-0, 19=20-8-0, 20=20-8-0, 22=20-8-0, 23=20-8-0, 24=20-8-0, 25=20-8-0  
Max Horiz 2=106 (LC 20)  
Max Uplift 2=27 (LC 17), 14=-4 (LC 13), 16=-85 (LC 17), 17=-56 (LC 17), 18=-64 (LC 17), 19=-61 (LC 17), 22=-62 (LC 16), 23=-63 (LC 16), 24=-55 (LC 16), 25=-93 (LC 16)  
Max Grav 2=183 (LC 1), 14=183 (LC 1), 16=198 (LC 37), 17=218 (LC 24), 18=265 (LC 24), 19=276 (LC 24), 20=161 (LC 29), 22=276 (LC 23), 23=265 (LC 23), 24=218 (LC 23), 25=198 (LC 36)

#### FORCES

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/12, 2-4=-137/57, 4-5=-85/73, 5-6=-75/101, 6-7=-76/158, 7-8=-83/211, 8-9=-83/211, 9-10=-76/158, 10-11=-75/102, 11-12=-60/39, 12-14=-104/19, 14-15=0/12

BOT CHORD 2-25=-23/121, 24-25=-23/121, 23-24=-23/121, 22-23=-23/121, 20-22=-23/121, 19-20=-23/121, 18-19=-23/121, 17-18=-23/121, 16-17=-23/121, 14-16=-23/121  
WEBS 8-20=-120/77, 7-22=-236/96, 6-23=-224/102, 5-24=-180/117, 4-25=-149/185, 9-19=-236/96, 10-18=-224/102, 11-17=-180/118, 12-16=-149/181

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-11-0 to 4-4-0, Exterior(2N) 4-4-0 to 10-4-0, Corner(3R) 10-4-0 to 15-4-0, Exterior(2N) 15-4-0 to 21-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 2, 62 lb uplift at joint 22, 63 lb uplift at joint 23, 55 lb uplift at joint 24, 93 lb uplift at joint 25, 61 lb uplift at joint 19, 64 lb uplift at joint 18, 56 lb uplift at joint 17, 85 lb uplift at joint 16 and 4 lb uplift at joint 14.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



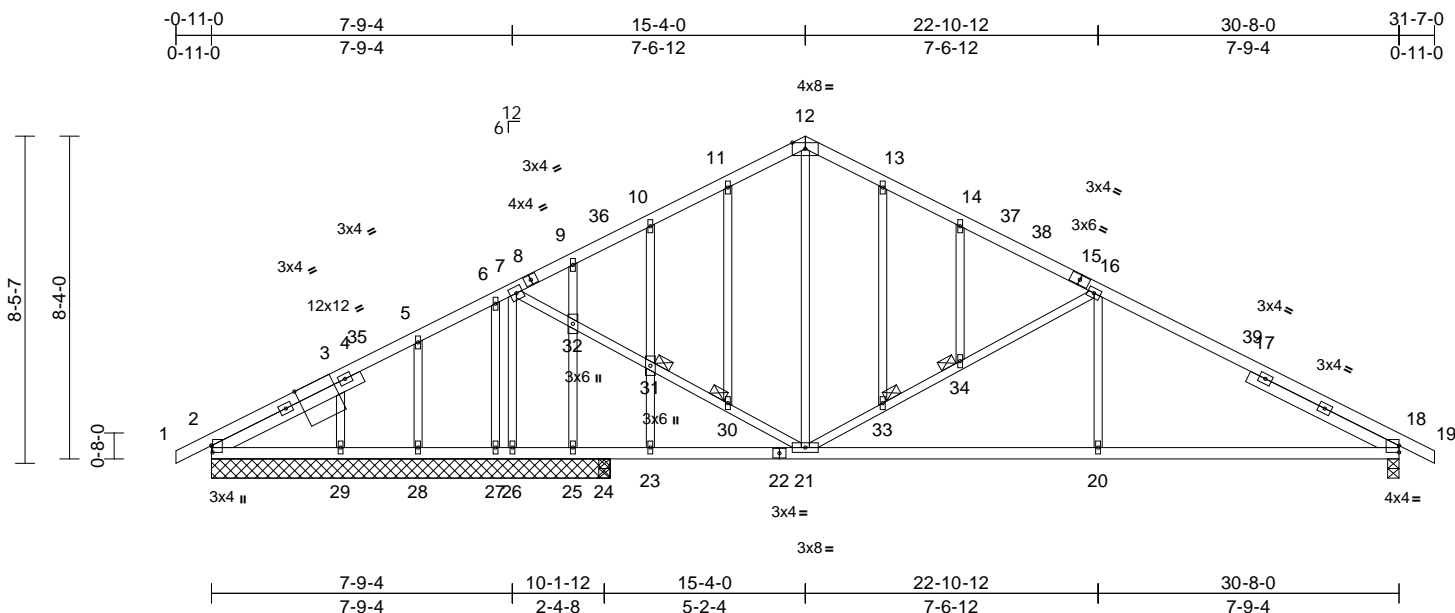
April 23, 2024

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of the design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcsccomponents.com](http://www.sbcsccomponents.com))

**MiTek®**

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Chesterfield, MO 63017  
314.434.1200 / MiTek-US.com



Scale = 1:59.5

Plate Offsets (X, Y): [2:0-2-1,0-0-5], [3:2-6-7,Edge], [18:Edge,0-2-1]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.85	Vert(LL)	-0.09	18-20	>999	240	244/190
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.63	Vert(CT)	-0.19	18-20	>999	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(CT)	0.03	18	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
Weight: 166 lb FT = 20%											

## LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2
SLIDER	Left 2x4 SP No.2 -- 4-3-11, Right 2x4 SP No.2 -- 4-3-11

## BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-1-9 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 20-21,18-20.
JOINTS	1 Brace at Jt(s): 30, 31, 33, 34

REACTIONS	(size)	2=10-3-8, 18=0-3-8, 24=0-3-8, 25=10-3-8, 26=10-3-8, 27=10-3-8, 28=10-3-8, 29=10-3-8
	Max Horiz	2=153 (LC 20)
	Max Uplift	2=42 (LC 17), 18=213 (LC 17), 24=161 (LC 16), 25=107 (LC 23), 26=22 (LC 17), 27=11 (LC 17), 28=62 (LC 16), 29=101 (LC 16)
	Max Grav	2=166 (LC 36), 18=1104 (LC 24), 24=686 (LC 23), 25=75 (LC 24), 26=607 (LC 24), 27=331 (LC 24), 28=153 (LC 36), 29=281 (LC 1)

## FORCES

TOP CHORD	(lb) - Maximum Compression/Maximum Tension
	1-2=0/12, 2-3=132/254, 3-5=42/232, 5-6=0/240, 6-7=13/159, 7-9=669/193, 9-10=721/221, 10-11=704/248, 11-12=646/255, 12-13=639/230, 13-14=707/213, 14-16=845/201, 16-18=1653/298, 18-19=0/12

BOT CHORD	2-29=-192/119, 28-29=-194/119, 27-28=-194/119, 26-27=-194/119, 25-26=-194/119, 24-25=-194/119, 23-24=-194/119, 21-23=-194/119, 20-21=-148/1376, 18-20=-148/1376
WEBS	12-21=-65/241, 21-33=-895/294, 33-34=-871/283, 16-34=-853/280, 16-20=0/335, 7-32=-42/935, 31-32=-44/930, 30-31=-41/918, 21-30=-44/947, 7-26=-570/45, 11-30=-23/60, 10-31=-283/116, 23-31=-301/120, 9-32=-274/85, 25-32=-276/91, 6-27=-314/52, 5-28=-122/83, 3-29=-216/135, 13-33=-47/22, 14-34=-38/12

## NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 15-4-0, Exterior(2R) 15-4-0 to 20-4-0, Interior (1) 20-4-0 to 31-7-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 2, 22 lb uplift at joint 26, 213 lb uplift at joint 18, 107 lb uplift at joint 25, 11 lb uplift at joint 27, 62 lb uplift at joint 28, 101 lb uplift at joint 29 and 161 lb uplift at joint 24.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

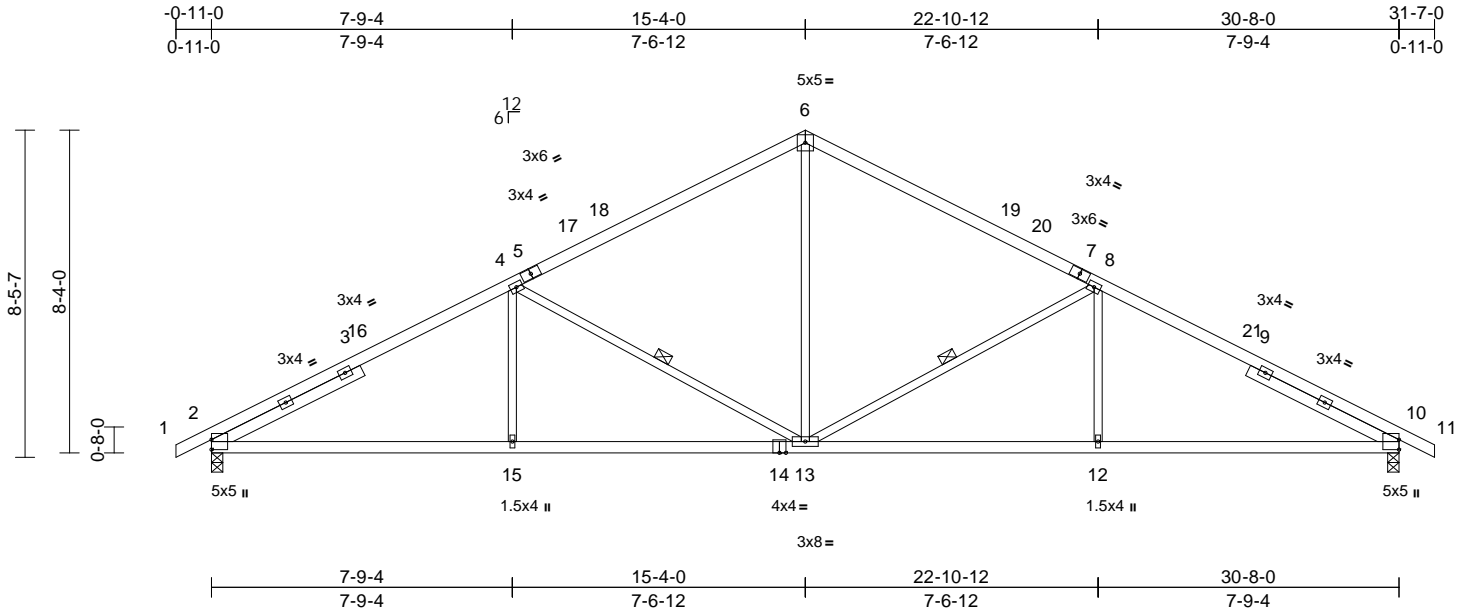


April 23, 2024

## WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of the design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcsccomponents.com)

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Scale = 1:59.5

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.76	-0.10	12-13	>999	240	MT20	244/190
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.71	-0.22	2-15	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.58	0.10	10	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
										Weight: 139 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP 2400F 2.0E \*Except\* 1-5,7-11:2x4 SP 1650F 1.5E  
 BOT CHORD 2x4 SP No.2  
 WEBS 2x3 SPF No.2  
 SLIDER Left 2x4 SP No.2 -- 4-3-11, Right 2x4 SP No.2 -- 4-3-11

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 4-0-8 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
 WEBS 1 Row at midpt 8-13, 4-13

**REACTIONS**

(size) 2=0-3-8, 10=0-3-8  
 Max Horiz 2=-153 (LC 17)  
 Max Uplift 2=-233 (LC 16), 10=-233 (LC 17)  
 Max Grav 2=1451 (LC 23), 10=1451 (LC 24)

**FORCES**

(lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/12, 2-4=-2366/375, 4-6=-1634/361, 6-8=-1634/361, 8-10=-2365/375, 10-11=0/12  
 BOT CHORD 2-15=-339/2003, 13-15=-339/2003, 12-13=-230/2003, 10-12=-230/2003  
 WEBS 6-13=-95/841, 8-13=-826/288, 8-12=0/322, 4-13=-826/287, 4-15=0/322

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 15-4-0, Exterior(2R) 15-4-0 to 20-4-0, Interior (1) 20-4-0 to 31-7-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 2 and 233 lb uplift at joint 10.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024

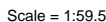
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Weight: 138 lb FT = 20%

2) Wind: ASCE 7-16; Vult=115mph (3-second gust)  
Vasd=91mph; TCDF=6.0psf; BCDF=6.0psf; h=35ft;  
Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope)  
Exterior zone and C-C Exterior(2E) 0-11-0 to 4-1-0,  
Interior (1) 4-1-0 to 15-4-0, Exterior(2R) 15-4-0 to  
20-4-0, Interior (1) 20-4-0 to 30-8-0 zone; cantilever left  
and right exposed ; end vertical left and right  
exposed; C-C for members and forces & MWFRS for  
reactions shown; Lumber DOL=1.60 plate grip  
DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 2 and 209 lb uplift at joint 10.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024



**WARNING – Verify design parameters and READ NOTES on this and INCLUDED MITER KEEF ELEMENTS (see MIT-1473 Rev. 1/2/2023) BEFORE USE.** Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria, and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcscomponents.com](http://www.sbcscomponents.com))

MiTek®

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Chesterfield, MO 63017  
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Job  
AS NOTED FOR PLAN REVIEW  
DEVELOPMENT SERVICES  
P240893 SUMMIT, MISSOURI A6

Premier Building Supply (Spring Hill), KS, Spring Hills, KS - 66083,

Truss Type  
Monopitch

Qty  
3

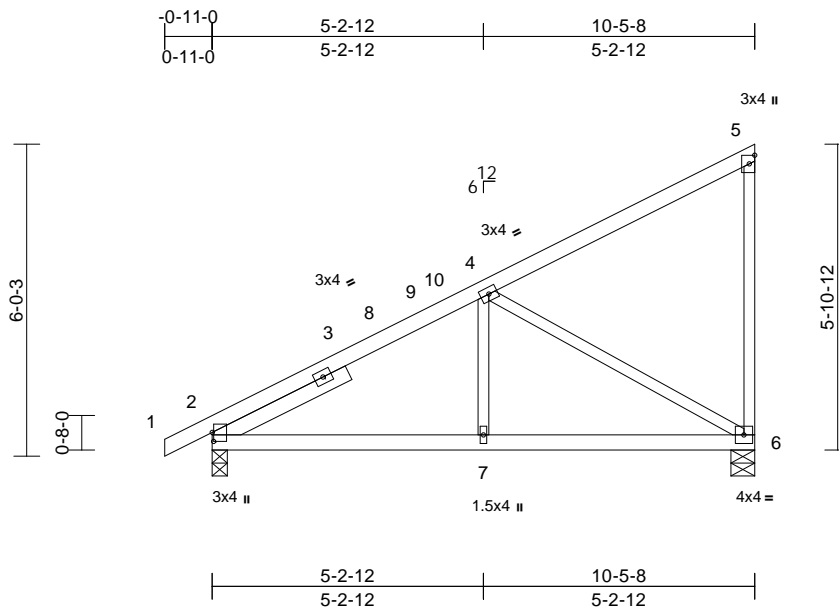
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Job Reference (optional)

I65080521

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Page: 1



Scale = 1:44.4

Plate Offsets (X, Y): [2:0-2-1,0-0-5]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	-0.02	6-7	>999	240	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.29	Vert(CT)	-0.04	6-7	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0 *	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
											Weight: 50 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x3 SPF No.2  
SLIDER Left 2x4 SP No.2 -- 2-10-10

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 9'-7-1 oc bracing.

#### REACTIONS

(size) 2=0-3-8, 6=0-5-8  
Max Horiz 2=248 (LC 13)  
Max Uplift 2=-93 (LC 16), 6=-134 (LC 16)  
Max Grav 2=599 (LC 23), 6=633 (LC 23)

#### FORCES

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/12, 2-4=-756/178, 4-5=-167/129, 5-6=-241/163  
BOT CHORD 2-7=-360/609, 6-7=-360/609  
WEBS 4-7=0/241, 4-6=-691/321

#### NOTES

- Wind: ASCE 7-16; Vult=115mph (3-second gust)  
Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft;  
Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope)  
exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0,  
Interior (1) 4-1-0 to 10-4-4 zone; cantilever left and right  
exposed; end vertical left and right exposed; C-C for  
members and forces & MWFRS for reactions shown;  
Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15  
Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate  
DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9;  
Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-06-00 tall by 2'-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 6 and 93 lb uplift at joint 2.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024

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Job  
AS NOTED FOR PLAN REVIEW  
DEVELOPMENT SERVICES  
P240893 SUMMIT, MISSOURI 7

Premier Building Supply (Spring Hill) KS, Spring Hills, KS - 66083,

Truss Type

Monopitch Supported Gable

Qty

1

Ply

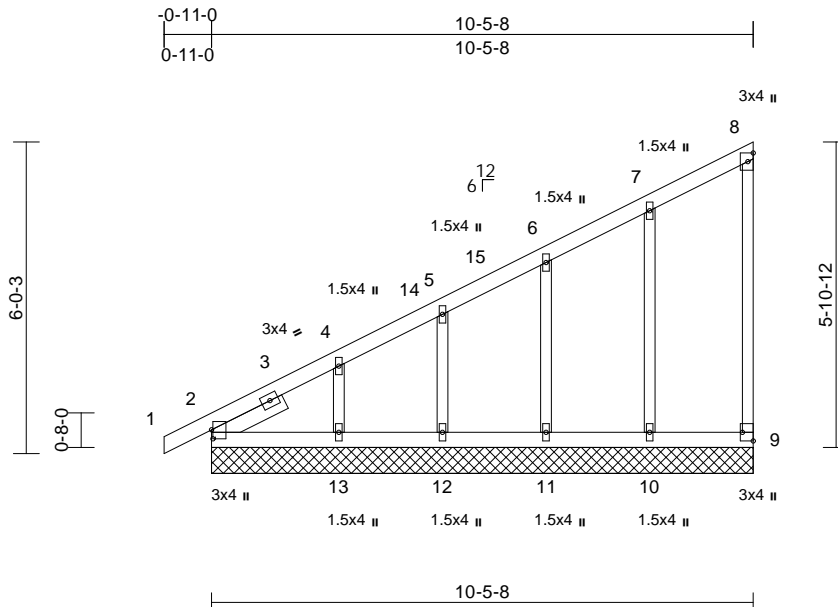
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Job Reference (optional)

I65080522

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:32  
ID:1ZrpYlpMcoNJXetA3u7DLIzkc9a-RfC?PsB70Hq3NSgPqnL8w3uITXbGKwRCDoi7J4zJC?f

Page: 1



Scale = 1:44.5

Plate Offsets (X, Y): [2:0-2-1,0-0-5], [9:Edge,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.39	Vert(LL)	n/a	-	n/a	999	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.12	Horz(CT)	0.00	9	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
										Weight: 50 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2
SLIDER	Left 2x4 SP No.2 -- 1-6-7

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size)	2=10-5-8, 9=10-5-8, 10=10-5-8, 11=10-5-8, 12=10-5-8, 13=10-5-8
Max Horiz	2=248 (LC 13)
Max Uplift	2=-7 (LC 12), 9=-33 (LC 13), 10=-65 (LC 16), 11=-62 (LC 16), 12=-52 (LC 16), 13=-106 (LC 16)
Max Grav	2=192 (LC 27), 9=101 (LC 23), 10=278 (LC 23), 11=266 (LC 23), 12=214 (LC 23), 13=211 (LC 1)

#### FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/12, 2-4=-419/244, 4-5=-313/190, 5-6=-260/170, 6-7=-194/147, 7-8=-109/99, 8-9=-85/61
BOT CHORD	2-13=-113/123, 12-13=-113/123, 11-12=-113/123, 10-11=-113/123, 9-10=-113/123
WEBS	7-10=-235/171, 6-11=-225/123, 5-12=-177/104, 4-13=-160/195

#### NOTES

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 10-4-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 9, 7 lb uplift at joint 2, 65 lb uplift at joint 10, 62 lb uplift at joint 11, 52 lb uplift at joint 12 and 106 lb uplift at joint 13.

13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



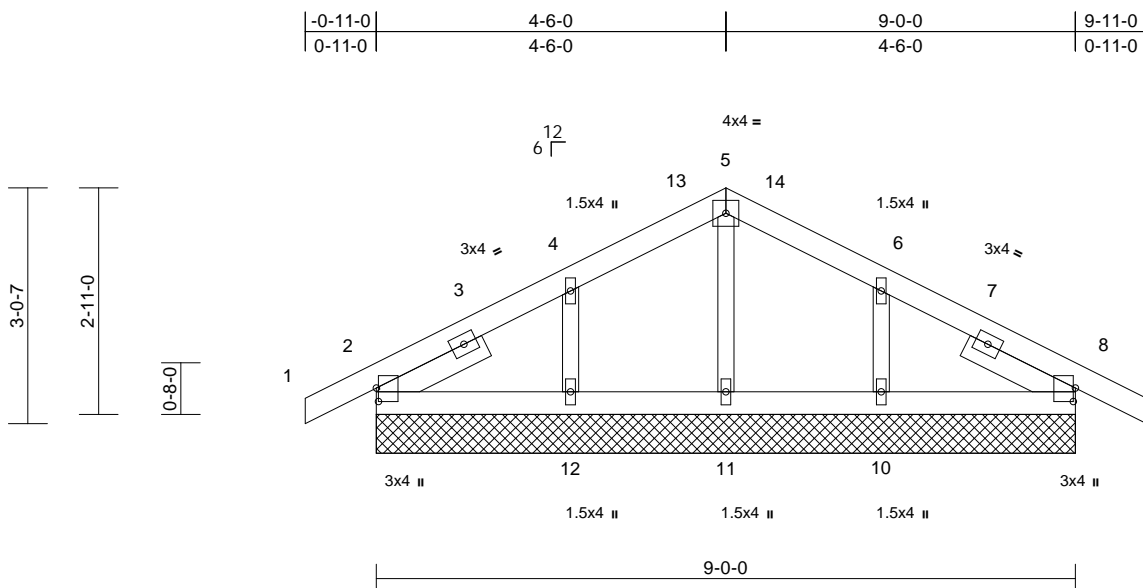
April 23, 2024

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Scale = 1:29.7

Plate Offsets (X, Y): [2:0-2-1,0-0-5], [8:0-2-1,0-0-5]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	n/a	999	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	8	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
										Weight: 40 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2  
 BOT CHORD 2x4 SP No.2  
 OTHERS 2x3 SPF No.2  
 SLIDER Left 2x4 SP No.2 -- 1-6-7, Right 2x4 SP No.2 -- 1-6-7

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS**

(size) 2=9-0-0, 8=9-0-0, 10=9-0-0, 11=9-0-0, 12=9-0-0  
 Max Horiz 2=-51 (LC 21)  
 Max Uplift 2=-38 (LC 16), 8=-49 (LC 17), 10=-80 (LC 17), 12=-84 (LC 16)  
 Max Grav 2=287 (LC 23), 8=287 (LC 24), 10=316 (LC 24), 11=116 (LC 24), 12=316 (LC 23)

**FORCES**

(lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/12, 2-4=-136/77, 4-5=-107/177, 5-6=-107/175, 6-8=-136/68, 8-9=0/12  
 BOT CHORD 2-12=-2/54, 11-12=-2/54, 10-11=-2/54, 8-10=-2/54  
 WEBS 5-11=-83/0, 4-12=-262/223, 6-10=-262/221

**NOTES**

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-11-0 to 4-1-0, Exterior(2N) 4-1-0 to 4-6-0, Corner(3R) 4-6-0 to 9-6-0, Exterior(2N) 9-6-0 to 9-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 2, 49 lb uplift at joint 8, 84 lb uplift at joint 12 and 80 lb uplift at joint 10.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 8.

14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard



April 23, 2024

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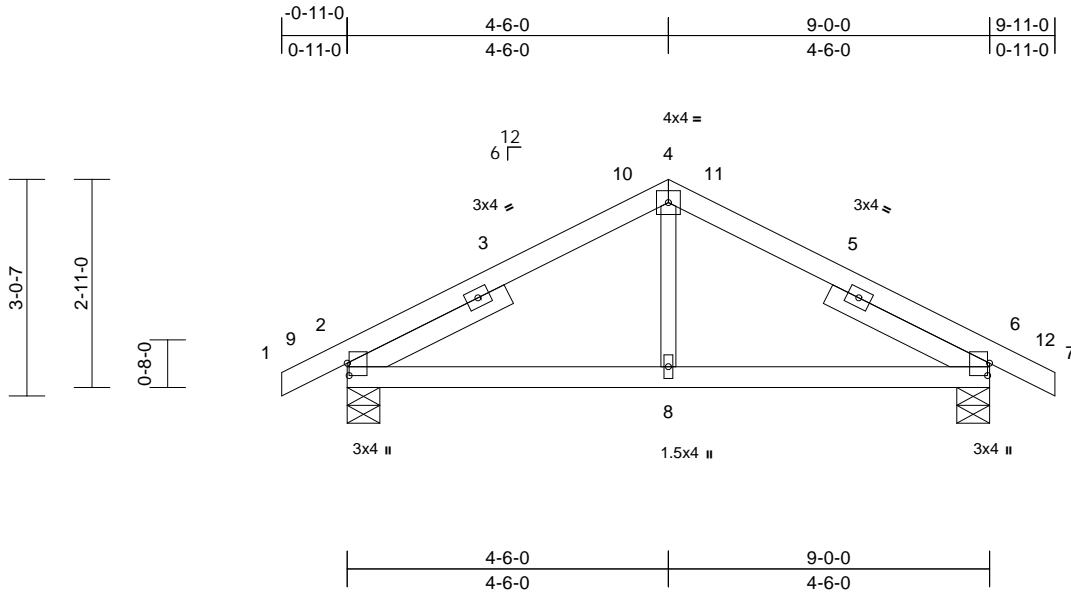
Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
P240893	0893 SUMMIT, MISSOURI 802	Common	2	1	I65080524

Premier Building Supply (Spring Hill), KS, Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:32

Page: 1

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Scale = 1:32.3

Plate Offsets (X, Y): [2:0-2-1,0-0-5], [6:0-2-1,0-0-5]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.41	Vert(LL)	-0.01	2-8	>999	240	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.22	Vert(CT)	-0.02	2-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
Weight: 41 lb											FT = 20%	

**LUMBER**

TOP CHORD 2x4 SP No.2  
 BOT CHORD 2x4 SP No.2  
 WEBS 2x3 SP No.2  
 SLIDER Left 2x4 SP No.2 -- 2-5-12, Right 2x4 SP No.2 -- 2-5-12

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

**REACTIONS**

(size) 2=0-5-8, 6=0-5-8  
 Max Horiz 2=-51 (LC 17)  
 Max Uplift 2=-84 (LC 16), 6=-84 (LC 17)  
 Max Grav 2=608 (LC 23), 6=608 (LC 24)

**FORCES**

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/12, 2-4=-590/257, 4-6=-590/257, 6-7=0/12  
 BOT CHORD 2-8=-111/389, 6-8=-111/389  
 WEBS 4-8=0/204

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust)  
 Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft;  
 Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope)  
 exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0,  
 Interior (1) 4-1-0 to 4-6-0, Exterior(2R) 4-6-0 to 9-6-0,  
 Interior (1) 9-6-0 to 9-11-0 zone; cantilever left and right  
 exposed; end vertical left and right exposed; C-C for  
 members and forces & MWFRS for reactions shown;  
 Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15  
 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate  
 DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9;  
 Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-06"-00 tall by 2'-00"-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 2 and 84 lb uplift at joint 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard

April 23, 2024

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Truss Type

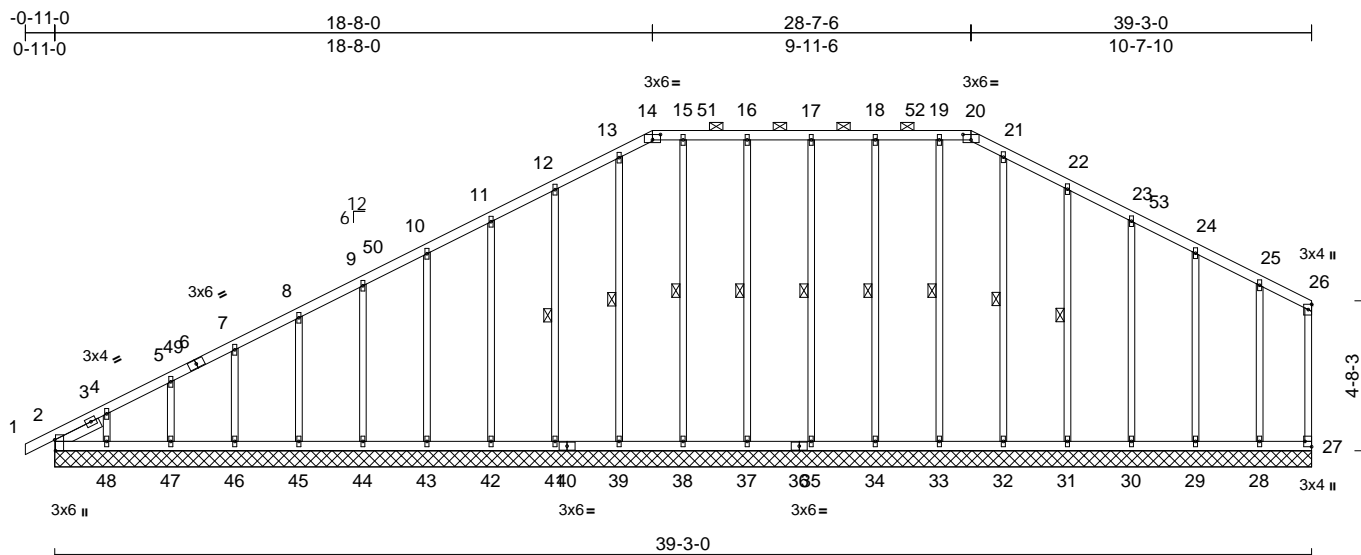
Qty

Ply

I65080525

Piggyback Base Supported Gable

Job Reference (optional)



Scale = 1:71.9

Plate Offsets (X, Y): [2:0-4-1,Edge], [14:0-3-0,0-2-0], [20:0-3-0,0-2-0], [27:Edge,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.32	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.11	Vert(CT)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.30	Horz(CT)	0.01	27	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
Weight: 232 lb FT = 20%											

## LUMBER

TOP CHORD 2x4 SP No.2  
 BOT CHORD 2x4 SP No.2  
 WEBS 2x3 SPF No.2  
 OTHERS 2x3 SPF No.2  
 SLIDER Left 2x4 SP No.2 -- 1-6-8

## BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 14-20.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS 1 Row at midpt 15-38, 13-39, 12-41, 16-37, 17-35, 18-34, 19-33, 21-32, 22-31

REACTIONS (size) 2=39-3-0, 27=39-3-0, 28=39-3-0, 29=39-3-0, 30=39-3-0, 31=39-3-0, 32=39-3-0, 33=39-3-0, 34=39-3-0, 35=39-3-0, 37=39-3-0, 38=39-3-0, 39=39-3-0, 41=39-3-0, 42=39-3-0, 43=39-3-0, 44=39-3-0, 45=39-3-0, 46=39-3-0, 47=39-3-0, 48=39-3-0

Max Horiz 2=272 (LC 13)

Max Uplift 2=-64 (LC 12), 27=-26 (LC 12), 28=-59 (LC 17), 29=-63 (LC 17), 30=-59 (LC 17), 31=-74 (LC 17), 33=-24 (LC 13), 34=-50 (LC 12), 35=-39 (LC 13), 37=-51 (LC 12), 38=-28 (LC 13), 39=-13 (LC 13), 41=-73 (LC 16), 42=-60 (LC 16), 43=-61 (LC 16), 44=-61 (LC 16), 45=-61 (LC 16), 46=-61 (LC 16), 47=-60 (LC 16), 48=-109 (LC 16)

Max Grav 2=200 (LC 51), 27=90 (LC 51), 28=264 (LC 41), 29=283 (LC 41), 30=277 (LC 41), 31=283 (LC 41), 32=259 (LC 41), 33=259 (LC 40), 34=282 (LC 40), 35=278 (LC 40), 37=282 (LC 40), 38=258 (LC 40), 39=261 (LC 41), 41=282 (LC 41), 42=278 (LC 41), 43=278 (LC 41), 44=278 (LC 41), 45=279 (LC 41), 46=274 (LC 41), 47=200 (LC 41), 48=155 (LC 43)

## FORCES

(lb) - Maximum Compression/Maximum Tension  
 1-2=0/12, 2-4=-276/199, 4-5=-212/165, 5-7=-193/146, 7-8=-177/132, 8-9=-161/153, 9-10=-145/207, 10-11=-152/261, 11-12=-170/314, 12-13=-194/376, 13-14=-190/378, 14-15=-179/375, 15-16=-179/375, 16-17=-179/375, 17-18=-179/375, 18-19=-179/375, 19-20=-179/375, 20-21=-189/377, 21-22=-194/377, 22-23=-171/315, 23-24=-153/262, 24-25=-132/206, 25-26=-125/172, 26-27=-98/126

## BOT CHORD

2-48=-92/124, 47-48=-92/124, 46-47=-92/124, 45-46=-92/124, 44-45=-92/124, 43-44=-92/124, 42-43=-92/124, 41-42=-92/124, 39-41=-92/124, 38-39=-92/124, 37-38=-92/124, 35-37=-92/124, 34-35=-92/124, 33-34=-92/124, 32-33=-92/124, 31-32=-92/124, 30-31=-92/124, 29-30=-92/124, 28-29=-92/124, 27-28=-92/124

## WEBS

15-38=-218/52, 13-39=-221/37, 12-41=-242/118, 11-42=-238/95, 10-43=-238/97, 9-44=-238/96, 8-45=-239/97, 7-46=-234/96, 5-47=-160/137, 4-48=-118/158, 16-37=-242/92, 17-35=-238/71, 18-34=-242/92, 19-33=-219/48, 21-32=-219/24, 22-31=-243/118, 23-30=-237/109, 24-29=-243/149, 25-28=-226/167

## NOTES

1) Unbalanced roof live loads have been considered for this design.



April 23, 2024

Continued on page 2

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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Job	Truss	Truss Type	Qty	Ply	I65080525
P240893	0893 SUMMIT, MISSOURI	Piggyback Base Supported Gable	1	1	
			Job Reference (optional)		

Premier Building Supply (Spring Hill) KS, Spring Hills, KS - 66083,

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ID:JGCWZQXmhQynsy71mfgcQKzwjJu-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrcDoi7J4zJC?f

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust)  
Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft;  
Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope)  
exterior zone and C-C Corner(3E) -0-11-0 to 4-1-0,  
Exterior(2N) 4-1-0 to 18-8-0, Corner(3R) 18-8-0 to  
23-7-8, Exterior(2N) 23-7-8 to 28-7-6, Corner(3R) 28-7-6  
to 33-7-8, Exterior(2N) 33-7-8 to 39-1-12 zone;  
cantilever left and right exposed ; end vertical left and  
right exposed;C-C for members and forces & MWFRS  
for reactions shown; Lumber DOL=1.60 plate grip  
DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss  
only. For studs exposed to wind (normal to the face),  
see Standard Industry Gable End Details as applicable,  
or consult qualified building designer as per ANSI/TPI 1.
- 4) TCELL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15  
Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate  
DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9;  
Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this  
design.
- 6) This truss has been designed for greater of min roof live  
load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on  
overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 1.5x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom  
chord live load nonconcurrent with any other live loads.
- 12) \* This truss has been designed for a live load of 20.0psf  
on the bottom chord in all areas where a rectangle  
3-06-00 tall by 2-00-00 wide will fit between the bottom  
chord and any other members.
- 13) All bearings are assumed to be SP No.2 crushing  
capacity of 565 psi.
- 14) Provide mechanical connection (by others) of truss to  
bearing plate capable of withstanding 26 lb uplift at joint  
27, 64 lb uplift at joint 2, 28 lb uplift at joint 38, 13 lb  
uplift at joint 39, 73 lb uplift at joint 41, 60 lb uplift at joint  
42, 61 lb uplift at joint 43, 61 lb uplift at joint 44, 61 lb  
uplift at joint 45, 61 lb uplift at joint 46, 60 lb uplift at joint  
47, 109 lb uplift at joint 48, 51 lb uplift at joint 37, 39 lb  
uplift at joint 35, 50 lb uplift at joint 34, 24 lb uplift at joint  
33, 74 lb uplift at joint 31, 59 lb uplift at joint 30, 63 lb  
uplift at joint 29 and 59 lb uplift at joint 28.
- 15) This truss is designed in accordance with the 2018  
International Residential Code sections R502.11.1 and  
R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size  
or the orientation of the purlin along the top and/or  
bottom chord.

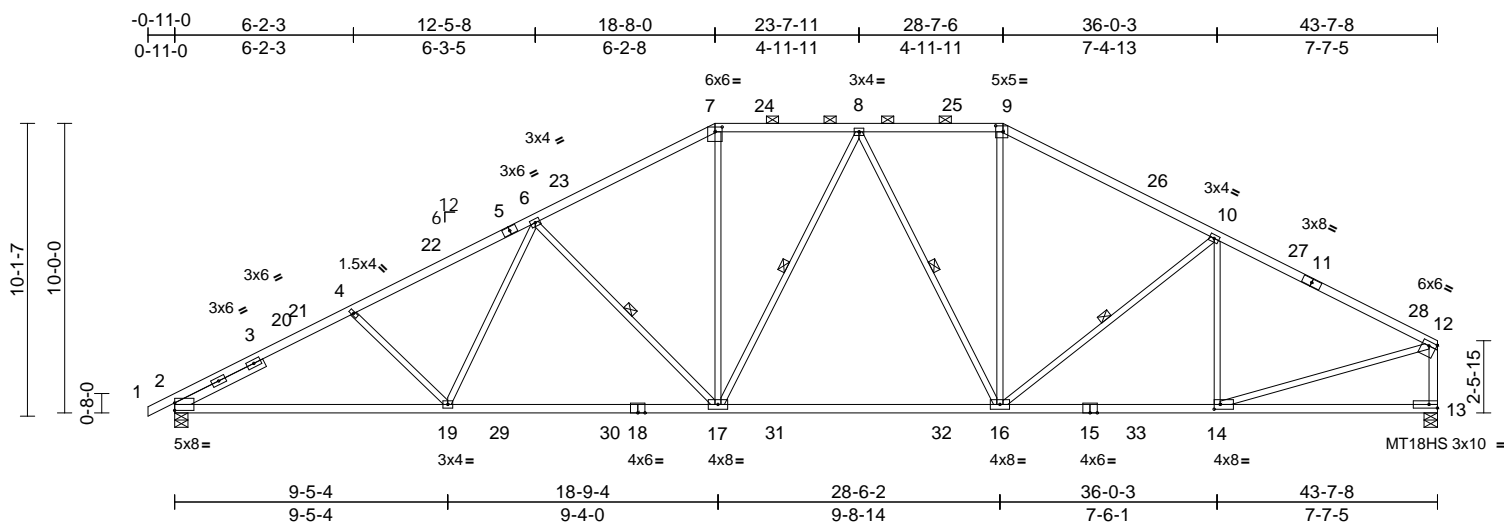
LOAD CASE(S) Standard

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Scale = 1:79.6

Plate Offsets (X, Y): [2:Edge,0-3-1], [7:0-3-0,0-2-0], [9:0-3-0,0-2-8], [12:Edge,0-1-12], [14:0-2-8,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.81	Vert(LL)	-0.39	16-17	>999	240	MT20	244/190
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.99	Vert(CT)	-0.62	16-17	>841	180	MT18HS	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(CT)	0.16	13	n/a	n/a		
BCLL	0.0 *	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
Weight: 213 lb											FT = 20%	

## LUMBER

TOP CHORD 2x4 SP 2400F 2.0E \*Except\* 5-7:2x4 SP 1650F 1.5E, 7-9:2x4 SP No.2  
 BOT CHORD 2x4 SP 1650F 1.5E \*Except\* 15-13:2x4 SP No.2, 18-15:2x4 SP 2400F 2.0E  
 WEBS 2x3 SPF No.2 \*Except\* 13-12:2x4 SP No.2  
 SLIDER Left 2x4 SP No.2 -- 3-5-1

## BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-1 oc purlins, except end verticals, and 2-0-0 oc purlins (3-3-12 max.): 7-9.  
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.  
 WEBS 1 Row at midpt 10-16, 6-17, 8-17, 8-16

## REACTIONS

(size) 2=0-5-8, 13=0-5-8  
 Max Horiz 2=205 (LC 13)  
 Max Uplift 2=-296 (LC 17), 13=-232 (LC 17)  
 Max Grav 2=2504 (LC 46), 13=2527 (LC 47)

## FORCES

(lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/12, 2-4=-4488/592, 4-6=-4176/575, 6-7=-3158/553, 7-8=-2698/542, 8-9=-2470/524, 9-10=-2934/518, 10-12=-3141/452, 12-13=-2439/370  
 BOT CHORD 2-19=-587/3840, 17-19=-494/3412, 16-17=-350/2610, 14-16=-365/2699, 13-14=-55/130  
 WEBS 7-17=-90/1000, 9-16=-66/832, 10-16=-407/222, 10-14=-626/188, 12-14=-324/2689, 4-19=-374/230, 6-19=-50/612, 6-17=-1142/309, 8-17=-245/310, 8-16=-583/171

## NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 18-8-0, Exterior(2R) 18-8-0 to 23-7-11, Interior (1) 23-7-11 to 28-7-6, Exterior(2R) 28-7-6 to 33-7-6, Interior (1) 33-7-6 to 43-5-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 2 SP 1650F 1.5E crushing capacity of 565 psi, Joint 13 SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2 and 232 lb uplift at joint 13.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 23, 2024

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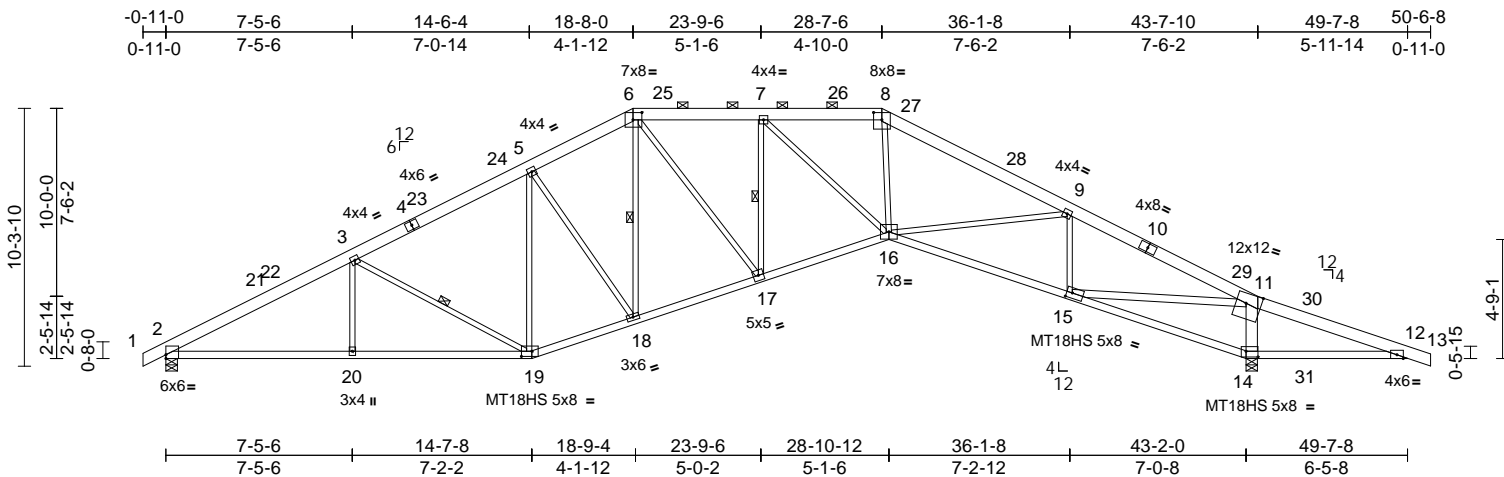
Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
P240893	0893 SUMMIT, MISSOURI 03	Piggyback Base	3	1	I65080527

Premier Building Supply (Spring Hill) KS, Spring Hills, KS - 66083,

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Scale = 1:92.1

Plate Offsets (X, Y): [2:Edge,0-1-14], [6:0-4-4,0-3-8], [8:0-4-0,0-3-8], [11:0-7-4,0-4-12], [14:0-5-12,0-2-12], [15:0-2-12,0-2-8], [19:0-5-0,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.41	16	>999	240	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.78	Vert(CT)	-0.66	15-16	>782	180	MT18HS	197/144
TCDL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(CT)	0.45	14	n/a	n/a		
BCLL	0.0 *	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
											Weight: 248 lb	FT = 20%

#### LUMBER

TOP CHORD 2x6 SPF No.2  
 BOT CHORD 2x4 SP 1650F 1.5E \*Except\* 14-12:2x4 SP No.2  
 WEBS 2x3 SPF No.2 \*Except\* 11-14:2x6 SPF No.2, 15-11:2x4 SP 1650F 1.5E

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (3-4-6 max.): 6-8.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:  
 5-7-10 oc bracing: 14-15  
 5-1-12 oc bracing: 12-14.  
 WEBS 1 Row at midpt 3-19, 6-18, 7-17

#### REACTIONS

(size) 2=0-5-8, 14=0-5-8  
 Max Horiz 2=-185 (LC 21)  
 Max Uplift 2=-298 (LC 16), 14=-388 (LC 17)  
 Max Grav 2=2449 (LC 43), 14=2927 (LC 43)

#### FORCES

(lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/23, 2-3=-4395/707, 3-5=-3551/665, 5-6=-3341/687, 6-7=-3490/684, 7-8=-4355/647, 8-9=-5206/651, 9-11=-4577/322, 11-12=-1178/1323, 12-13=0/23  
 BOT CHORD 2-20=-500/3774, 19-20=-500/3774, 18-19=-323/3172, 17-18=-233/3032, 16-17=-278/3693, 15-16=-127/4186, 14-15=-1571/1393, 12-14=-1149/1162  
 WEBS 3-20=0/326, 3-19=-882/246, 5-19=-463/54, 5-18=-313/294, 6-18=-205/404, 6-17=-82/1199, 8-16=-76/1700, 9-16=-221/988, 9-15=-1322/463, 7-17=-1085/145, 7-16=0/1193, 11-14=-2523/635, 11-15=-1143/4925

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 18-8-0, Exterior(2R) 18-8-0 to 23-9-6, Interior (1) 23-9-6 to 28-7-6, Exterior(2R) 28-7-6 to 33-7-6, Interior (1) 33-7-6 to 50-6-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP 1650F 1.5E crushing capacity of 565 psi, Joint 14 SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 298 lb uplift at joint 2 and 388 lb uplift at joint 14.

- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 23, 2024

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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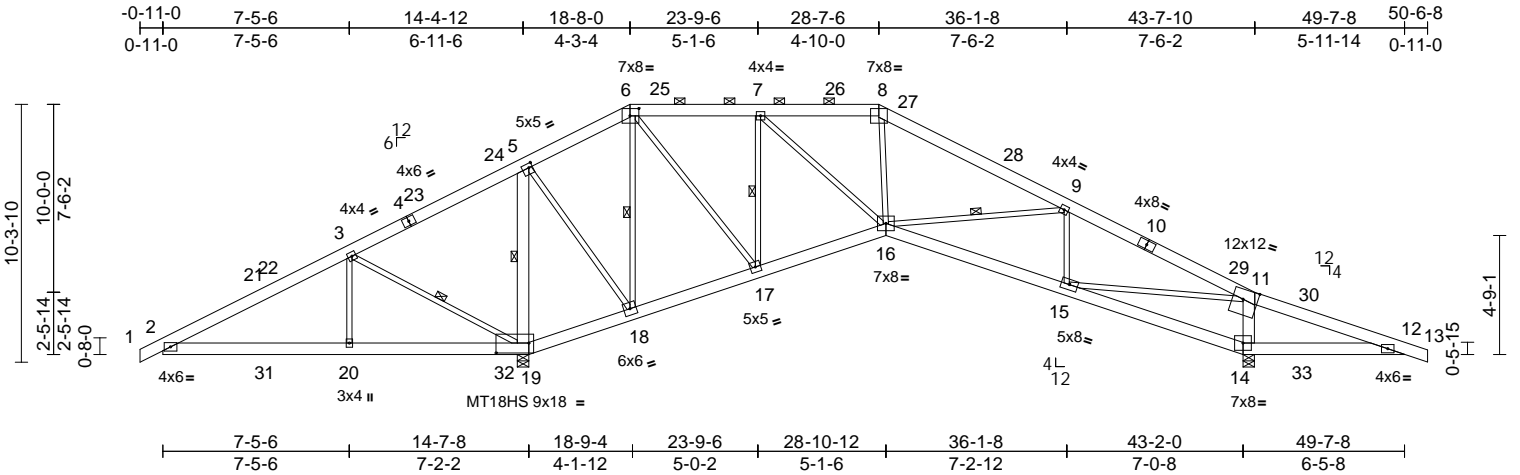
Job P240893 SUMMIT, MISSOURI 4	Truss Type Piggyback Base	Qty 6	Ply 1	Job Reference (optional) I65080528
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Premier Building Supply (Spring Hill) KS, Spring Hills, KS - 66083,

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Scale = 1:92.1

Plate Offsets (X, Y): [5:0-1-12,0-2-0], [6:0-4-4,0-3-8], [11:0-7-0,0-4-12], [19:1-3-12,0-4-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.70	Vert(LL)	-0.13	15-16	>999	240	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.40	Vert(CT)	-0.21	15-16	>999	180	MT18HS	197/144
TCDL	10.0	Rep Stress Incr	YES	WB	0.89	Horz(CT)	0.12	14	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
											Weight: 264 lb	FT = 20%

#### LUMBER

TOP CHORD 2x6 SPF No.2  
BOT CHORD 2x6 SPF No.2  
WEBS 2x3 SPF No.2 \*Except\* 19-5,11-14:2x6 SPF No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 4-11-13 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-8.  
BOT CHORD Rigid ceiling directly applied or 5-4-14 oc bracing.  
WEBS 1 Row at midpt 5-19, 6-18, 7-17, 9-16, 3-19

**REACTIONS** (size) 14=0-5-8, 19=0-5-8, (req. 0-5-13)  
Max Horiz 19=185 (LC 17)  
Max Uplift 14=350 (LC 17), 19=446 (LC 16)  
Max Grav 14=1846 (LC 24), 19=3691 (LC 43)

#### FORCES

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/32, 2-3=-874/932, 3-5=-1633/2034, 5-6=-1124/972, 6-7=-915/449, 7-8=-1231/94, 8-9=-1533/173, 9-11=-1942/124, 11-12=-1225/1352, 12-13=0/32  
BOT CHORD 2-20=-649/871, 19-20=-649/871, 18-19=-1845/1860, 17-18=-898/1483, 16-17=-489/1423, 15-16=0/1753, 14-15=-1488/1350, 12-14=-1191/1222  
WEBS 3-20=-529/342, 5-19=-2450/884, 6-18=-1492/695, 5-18=-607/1575, 7-17=-1128/153, 8-16=-252/738, 9-15=-565/176, 9-16=-785/442, 3-19=-1149/985, 6-17=-88/1232, 7-16=0/1208, 11-14=-1318/313, 11-15=-248/2408

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 18-8-0, Exterior(2R) 18-8-0 to 23-9-6, Interior (1) 23-9-6 to 28-7-6, Exterior(2R) 28-7-6 to 33-7-6, Interior (1) 33-7-6 to 50-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- WARNING: Required bearing size at joint(s) 19 greater than input bearing size.
- All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 446 lb uplift at joint 19 and 350 lb uplift at joint 14.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard



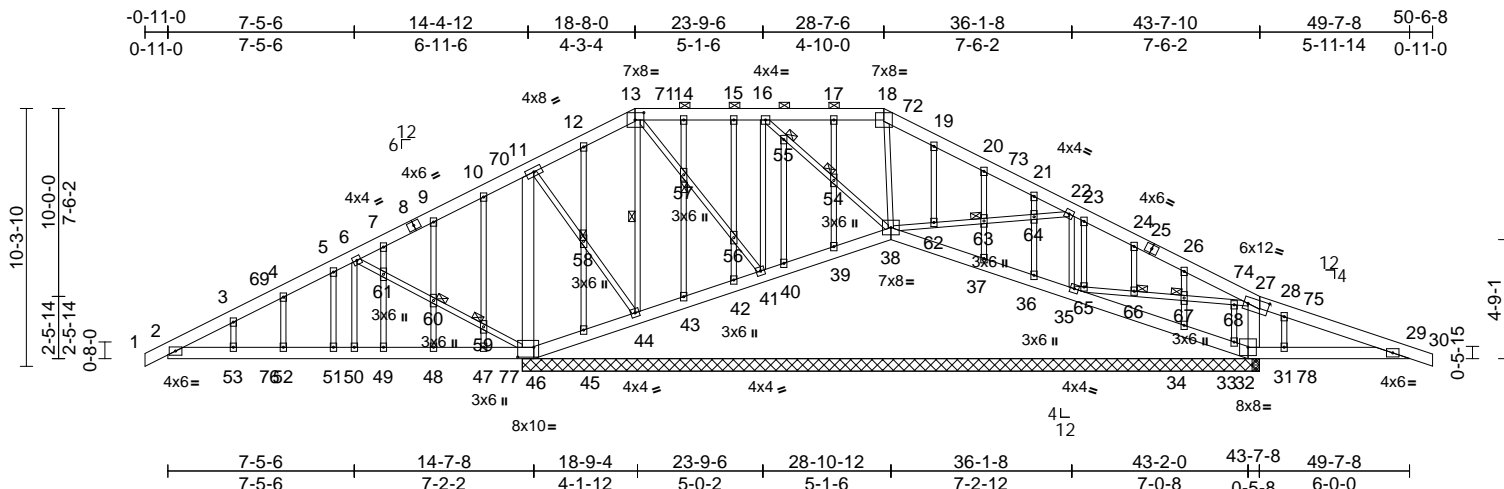
April 23, 2024

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of the design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcscomponents.com](http://www.sbcscomponents.com))

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Scale = 1:92.1

Plate Offsets (X, Y): [13:0-4-4,0-3-8], [27:0-10-0,0-3-0], [46:0-7-12,0-4-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.64	Vert(LL)	0.00	45-46	>999	240	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.36	Vert(CT)	0.00	34-35	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(CT)	0.03	32	n/a	n/a		
BCLL	0.0 *	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
Weight: 321 lb											FT = 20%	

## LUMBER

TOP CHORD 2x6 SPF No.2  
BOT CHORD 2x6 SPF No.2  
WEBS 2x3 SPF No.2 \*Except\* 46-11,27-32:2x6 SPF No.2  
OTHERS 2x3 SPF No.2

## BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-6 oc purlins, except 2-0-0 oc purlins (9-1-1 max.): 13-18.  
BOT CHORD Rigid ceiling directly applied or 5-2-6 oc bracing.  
WEBS 1 Row at midpt 13-44  
JOINTS 1 Brace at Jt(s): 54, 55, 57, 58, 59, 60, 63, 66, 67

## REACTIONS

(size) 32=29-5-8, 33=29-5-8, 34=29-5-8, 36=29-5-8, 37=29-5-8, 38=29-5-8, 39=29-5-8, 40=29-5-8, 41=29-5-8, 42=29-5-8, 43=29-5-8, 44=29-5-8, 45=29-5-8, 46=29-5-8, 65=29-5-8  
Max Horiz 46=185 (LC 17)  
Max Uplift 32=653 (LC 13), 33=740 (LC 49), 34=102 (LC 17), 36=99 (LC 49), 37=115 (LC 17), 38=294 (LC 46), 39=54 (LC 12), 40=11 (LC 43), 41=46 (LC 13), 42=34 (LC 12), 43=45 (LC 12), 44=101 (LC 60), 45=24 (LC 16), 46=496 (LC 16), 65=83 (LC 17)  
Max Grav 32=1704 (LC 49), 33=287 (LC 13), 34=507 (LC 43), 36=110 (LC 55), 37=515 (LC 43), 38=142 (LC 17), 39=379 (LC 42), 40=46 (LC 7), 41=201 (LC 42), 42=244 (LC 42), 43=324 (LC 42), 44=72 (LC 12), 45=196 (LC 43), 46=2904 (LC 43), 65=376 (LC 48)

## FORCES

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/32, 2-3=-858/720, 3-4=-836/742, 4-5=-823/809, 5-6=-875/888, 6-7=-1549/1729, 7-9=-1617/1876, 9-10=-1607/1925, 10-11=-1603/1994, 11-12=-993/1337, 12-13=-926/1318, 13-14=-652/997, 14-15=-651/996, 15-16=-651/996, 16-17=-368/619, 17-18=-369/619, 18-19=-476/811, 19-20=-504/727, 20-21=-595/808, 21-22=-587/727, 22-23=-610/709, 23-24=-599/713, 24-26=-610/680, 26-27=-662/672, 27-28=-1082/1225, 28-29=-1083/1255, 29-30=0/32  
BOT CHORD 2-53=-626/852, 52-53=-626/852, 51-52=-626/852, 50-51=-626/852, 49-50=-626/852, 48-49=-626/852, 47-48=-626/852, 46-47=-625/852, 45-46=-1857/1875, 44-45=-1804/1834, 43-44=-1257/1340, 42-43=-1252/1337, 41-42=-1252/1334, 40-41=-1051/1143, 39-40=-1056/1144, 38-39=-1056/1144, 37-38=-655/813, 36-37=-637/804, 35-36=-666/822, 34-35=-1267/1227, 33-34=-1253/1229, 32-33=-1489/1427, 31-32=-1115/1089, 29-31=-1116/1089



April 23, 2024

Continued on page 2

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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Job	Truss	Truss Type	Qty	Ply	
P240893	0893 SUMMIT, MISSOURI	Piggyback Base Structural Gable	1	1	165080529
Job Reference (optional)					

Premier Building Supply (Spring Hill) KS, Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:34

Page: 2

ID:Te\_6VV0tZR9We\_0Ly9vPsbzWjEv-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCD0i7J4zJC?f

WEBS 6-50=-346/357, 11-58=-802/894,  
44-58=-846/943, 13-44=-790/576,  
22-35=-257/242, 11-46=-1454/1076,  
16-41=-383/261, 18-38=-527/279,  
38-62=-299/201, 62-63=-291/196,  
63-64=-292/197, 22-64=-297/200,  
6-61=-1250/1051, 60-61=-1174/985,  
59-60=-1182/990, 46-59=-1176/987,  
13-57=-291/295, 56-57=-297/303,  
41-56=-305/310, 16-55=-403/538,  
54-55=-382/508, 38-54=-386/515,  
27-32=-327/315, 35-65=-416/614,  
65-66=-398/584, 66-67=-398/584,  
67-68=-398/584, 27-68=-397/582,  
17-54=-339/137, 39-54=-336/140,  
40-55=-31/43, 15-56=-202/108,  
42-56=-207/117, 14-57=-289/172,  
43-57=-292/179, 12-58=-263/122,  
45-58=-324/177, 10-59=-80/43,  
47-59=-92/46, 9-60=-128/65, 48-60=-112/61,  
7-61=-166/102, 49-61=-52/36, 5-51=-140/92,  
4-52=-103/60, 3-53=-83/73, 19-62=-62/36,  
20-63=-410/182, 37-63=-437/194,  
21-64=-81/19, 36-64=-98/36, 23-65=-228/55,  
24-66=-30/9, 26-67=-405/146,  
34-67=-422/152, 33-68=-16/26,  
28-31=-185/109

- 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard**NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCCL=6.0psf; BCCL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 18-8-0, Exterior(2R) 18-8-0 to 23-9-6, Interior (1) 23-9-6 to 28-7-6, Exterior(2R) 28-7-6 to 33-7-6, Interior (1) 33-7-6 to 50-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCCL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 3x4 MT20 unless otherwise indicated.
- 9) Gable studs spaced at 2'-0" oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0" tall by 2'-0" wide will fit between the bottom chord and any other members.
- 12) Bearings are assumed to be: Joint 33 SPF No.2 crushing capacity of 425 psi, Joint 32 SPF No.2 crushing capacity of 425 psi.
- 13) Bearing at joint(s) 65 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 496 lb uplift at joint 46, 294 lb uplift at joint 38, 653 lb uplift at joint 32, 101 lb uplift at joint 44, 46 lb uplift at joint 41, 54 lb uplift at joint 39, 11 lb uplift at joint 40, 34 lb uplift at joint 42, 45 lb uplift at joint 43, 24 lb uplift at joint 45, 115 lb uplift at joint 37, 99 lb uplift at joint 36, 83 lb uplift at joint 65, 102 lb uplift at joint 34 and 740 lb uplift at joint 33.

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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AS NOTED FOR PLAN REVIEW  
DEVELOPMENT SERVICES  
P240893 SUMMIT, MISSOURI PB1

Job Truss  
Piggyback

Truss Type  
Piggyback

Qty  
2

Ply  
1

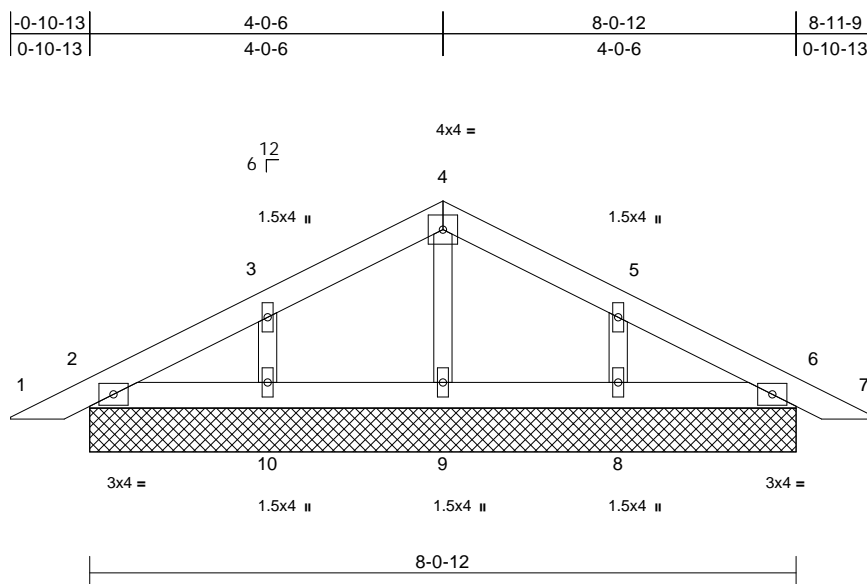
Job Reference (optional)

I65080530

Premier Building Supply (Spring Hill) KS, Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:35  
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Page: 1



Scale = 1:26.3

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.10	n/a	-	n/a	999	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.04	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	6	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-P							
BCDL	10.0										
										Weight: 31 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x3 SPF No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (size) 2=8-0-12, 6=8-0-12, 8=8-0-12,  
9=8-0-12, 10=8-0-12  
Max Horiz 2=-44 (LC 17)  
Max Uplift 2=-19 (LC 16), 6=-27 (LC 17),  
8=-75 (LC 17), 10=-75 (LC 16)  
Max Grav 2=185 (LC 23), 6=185 (LC 24),  
8=306 (LC 24), 9=134 (LC 23),  
10=306 (LC 23)

#### FORCES

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/29, 2-3=-60/42, 3-4=-76/93,  
4-5=-76/99, 5-6=-60/42, 6-7=0/29  
BOT CHORD 2-10=-13/49, 9-10=-13/49, 8-9=-13/49,  
6-8=-13/49  
WEBS 4-9=-97/13, 3-10=-260/213, 5-8=-260/173

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust)  
Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft;  
Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 2, 27 lb uplift at joint 6, 75 lb uplift at joint 10 and 75 lb uplift at joint 8.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

**LOAD CASE(S)** Standard



April 23, 2024

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Job  
AS NOTED FOR PLAN REVIEW  
DEVELOPMENT SERVICES  
P240893 SUMMIT, MISSOURI  
05/13/2024

Truss  
PB2

Premier Building Supply (Spring Hill), KS - Spring Hills, KS - 66083,

Truss Type  
Piggyback

Qty  
19

Ply  
1

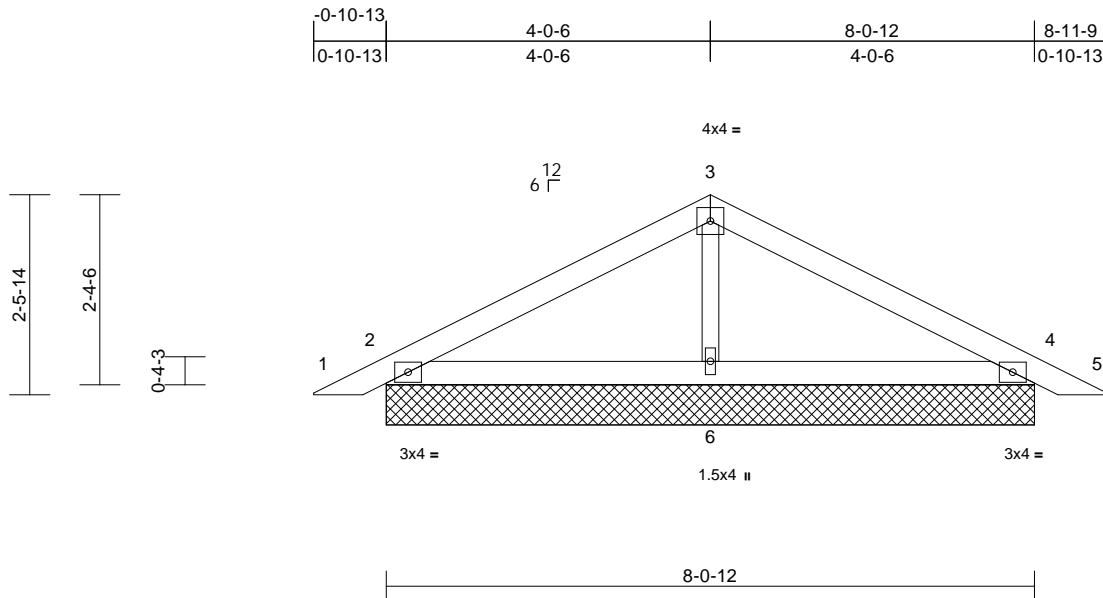
Job Reference (optional)

I65080531

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Page: 1

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Scale = 1:28.7

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.54	n/a	-	n/a	999	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.16	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	4	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-P							
BCDL	10.0									Weight: 30 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x3 SPF No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size) 2=8-0-12, 4=8-0-12, 6=8-0-12  
Max Horiz 2=44 (LC 20)  
Max Uplift 2=-67 (LC 16), 4=-75 (LC 17), 6=-8 (LC 16)  
Max Grav 2=340 (LC 23), 4=340 (LC 24), 6=348 (LC 24)

#### FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/29, 2-3=-123/76, 3-4=-123/87, 4-5=0/29  
BOT CHORD 2-6=0/46, 4-6=0/46  
WEBS 3-6=-248/180

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 4-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 2, 75 lb uplift at joint 4 and 8 lb uplift at joint 6.
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard



April 23, 2024

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Job  
AS NOTED FOR PLAN REVIEW  
DEVELOPMENT SERVICES  
P240893 SUMMIT, MISSOURI

Truss

Truss Type

Qty

Ply

I65080532

Valley

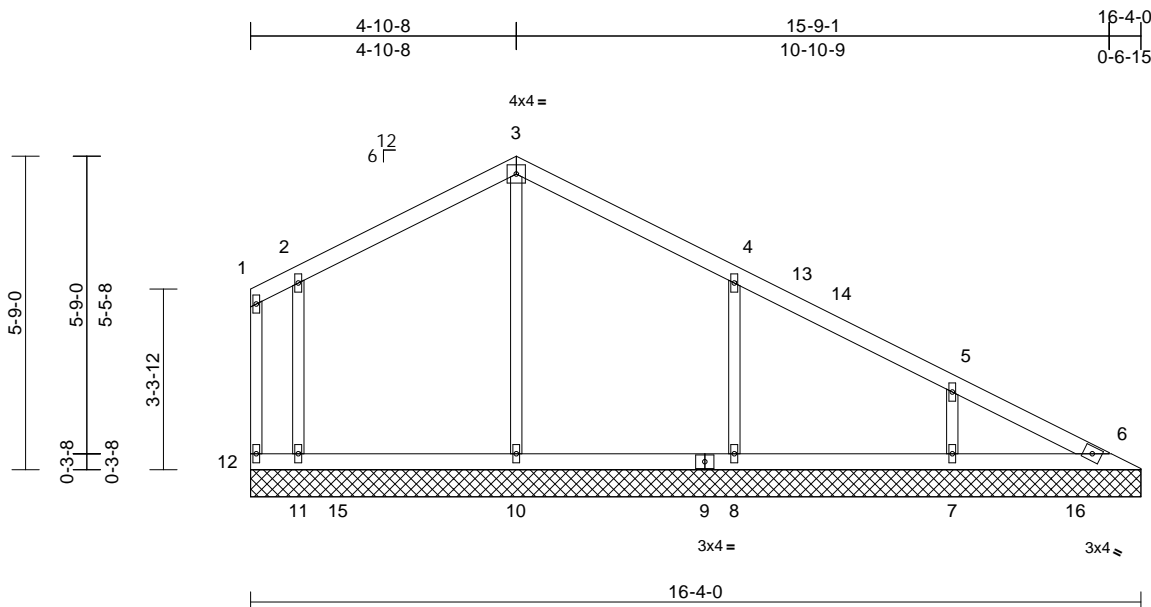
Job Reference (optional)

Premier Building Supply (Spring Hill) KS, Spring Hills, KS - 66083,

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Page: 1

05/13/2024



Scale = 1:42.3

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.36	n/a	-	n/a	999	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.17	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.20	Horiz(TL)	0.00	6	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
										Weight: 63 lb	FT = 20%

**LUMBER**

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2

**BRACING**

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS**

(size)	6=16-4-0, 7=16-4-0, 8=16-4-0, 10=16-4-0, 11=16-4-0, 12=16-4-0
Max Horiz	12=169 (LC 12)
Max Uplift	6=6 (LC 13), 7=113 (LC 17), 8=137 (LC 17), 11=131 (LC 16), 12=168 (LC 5)
Max Grav	6=120 (LC 25), 7=344 (LC 6), 8=543 (LC 23), 10=429 (LC 26), 11=549 (LC 22), 12=49 (LC 16)

**FORCES**

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-12=-93/145, 1-2=-78/102, 2-3=-117/179, 3-4=-118/191, 4-5=-123/96, 5-6=-196/118
BOT CHORD	11-12=-101/196, 10-11=-101/196, 8-10=-101/196, 7-8=-101/196, 6-7=-101/196
WEBS	3-10=-269/126, 2-11=-476/220, 4-8=-463/239, 5-7=-258/188

**NOTES**

- 1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-4 to 4-10-8, Exterior(2R) 4-10-8 to 9-10-8, Interior (1) 9-10-8 to 15-8-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 12, 6 lb uplift at joint 6, 131 lb uplift at joint 11, 137 lb uplift at joint 8 and 113 lb uplift at joint 7.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



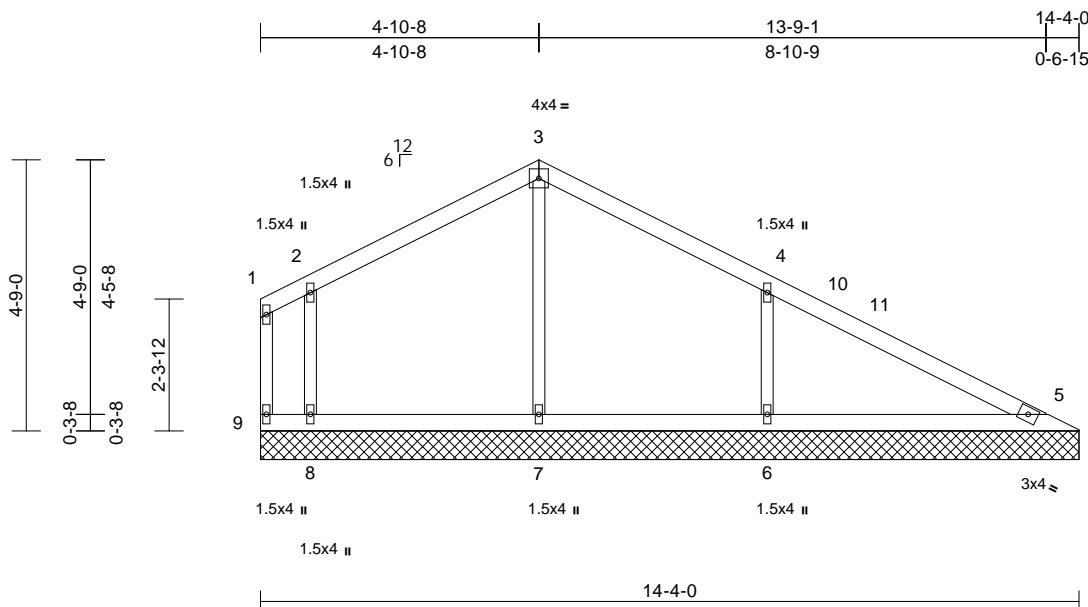
April 23, 2024

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of the design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcsccomponents.com](http://www.sbcsccomponents.com))

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Scale = 1:40.3

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.47	n/a	-	n/a	999	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.19	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.13	Horiz(TL)	0.00	5	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0									Weight: 53 lb	FT = 20%

**LUMBER**

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2

**BRACING**

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

<b>REACTIONS</b>	(size)	5=14-4-0, 6=14-4-0, 7=14-4-0, 8=14-4-0, 9=14-4-0
	Max Horiz	9=-124 (LC 12)
	Max Uplift	5=-3 (LC 17), 6=-170 (LC 17), 8=-138 (LC 16), 9=-177 (LC 22)
	Max Grav	5=177 (LC 23), 6=643 (LC 23), 7=330 (LC 22), 8=567 (LC 22), 9=52 (LC 16)

**FORCES**

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-9=-85/154, 1-2=-61/106, 2-3=-113/161, 3-4=-119/179, 4-5=-115/117
BOT CHORD	8-9=-70/143, 7-8=-70/143, 6-7=-70/143, 5-6=-70/143
WEBS	3-7=-258/110, 2-8=-488/269, 4-6=-528/298

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-4 to 4-10-8, Exterior(2R) 4-10-8 to 9-10-8, Interior (1) 9-10-8 to 13-8-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 177 lb uplift at joint 9, 3 lb uplift at joint 5, 138 lb uplift at joint 8 and 170 lb uplift at joint 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard

April 23, 2024

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

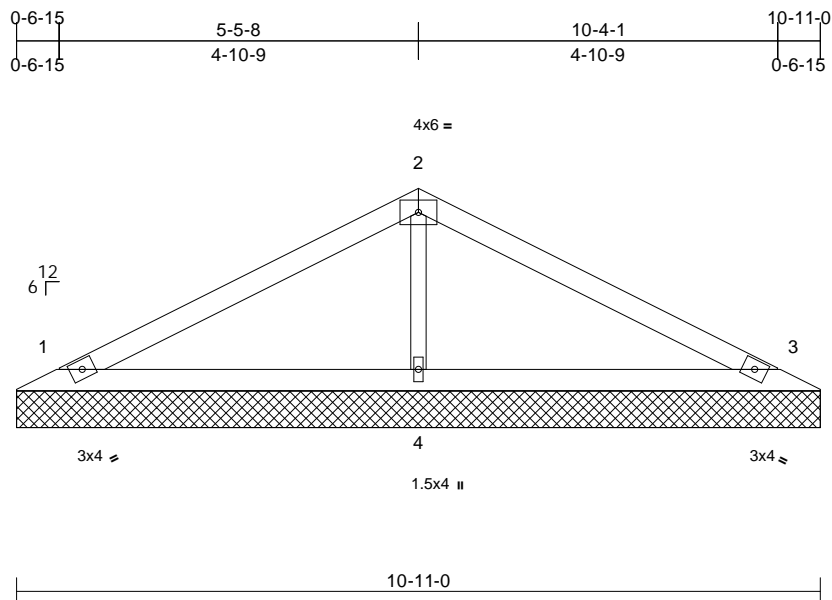
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of the design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcsccomponents.com](http://www.sbcsccomponents.com))

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Scale = 1:31.3

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.62	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.24	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 34 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x3 SPF No.2

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS**

(size) 1=10-11-0, 3=10-11-0, 4=10-11-0  
Max Horiz 1=-46 (LC 21)  
Max Uplift 1=-49 (LC 16), 3=-57 (LC 17),  
4=-37 (LC 16)  
Max Grav 1=304 (LC 22), 3=304 (LC 23),  
4=479 (LC 22)

**FORCES**

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-175/78, 2-3=-175/85  
BOT CHORD 1-4=-3/53, 3-4=-3/53  
WEBS 2-4=-330/219

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 57 lb uplift at joint 3 and 37 lb uplift at joint 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard

April 23, 2024

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Job  
AS NOTED FOR PLAN REVIEW  
DEVELOPMENT SERVICES  
P240893 SUMMIT, MISSOURI 6

Truss Type

Valley

Qty

1

Ply

1

I65080537

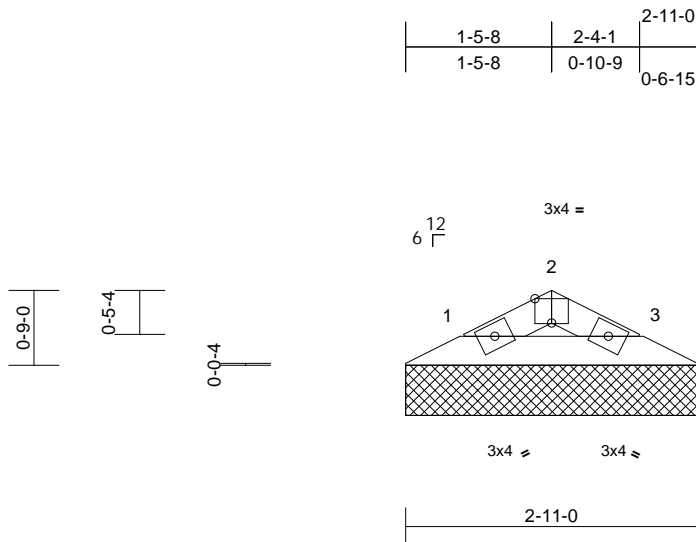
Job Reference (optional)

Premier Building Supply (Spring Hill) KS, Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:35

Page: 1

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Scale = 1:23

Plate Offsets (X, Y): [2:0-2-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.03	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.04	Vert(TL)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-P							
BCDL	10.0										
										Weight: 7 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (size) 1=2-11-0, 3=2-11-0

Max Horiz 1=8 (LC 16)

Max Uplift 1=12 (LC 16), 3=12 (LC 17)

Max Grav 1=82 (LC 22), 3=82 (LC 23)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-78/70, 2-3=-78/74

BOT CHORD 1-3=-48/60

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 1 and 12 lb uplift at joint 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard

April 23, 2024

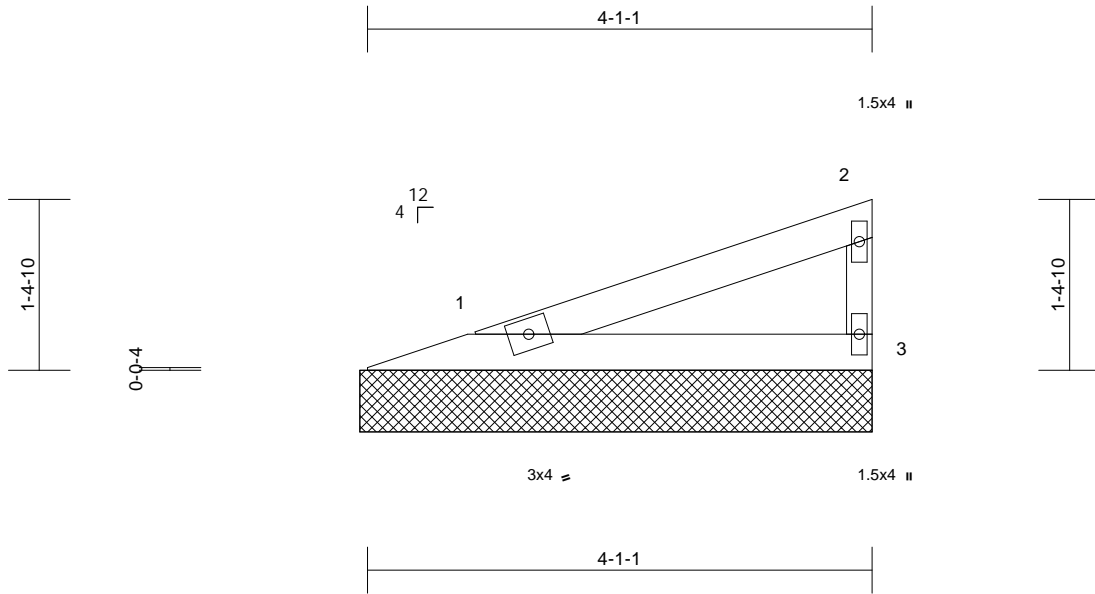
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Scale = 1:18.7

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.28	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.11	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-P								
BCDL	10.0											
											Weight: 12 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x3 SPF No.2

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 4-1-13 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (size) 1=4-1-13, 3=4-1-13

Max Horiz 1=50 (LC 15)  
Max Uplift 1=-27 (LC 12), 3=-34 (LC 16)  
Max Grav 1=181 (LC 22), 3=181 (LC 22)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-71/43, 2-3=-150/142  
BOT CHORD 1-3=-22/23

**NOTES**

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust)  
Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft;  
Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope)  
exterior zone and C-C Exterior(2E) zone; cantilever left  
and right exposed; end vertical left and right  
exposed; C-C for members and forces & MWFRS for  
reactions shown; Lumber DOL=1.60 plate grip  
DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss  
only. For studs exposed to wind (normal to the face),  
see Standard Industry Gable End Details as applicable,  
or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15  
Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate  
DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9;  
Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this  
design.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 4-0-0 oc.

- 7) This truss has been designed for a 10.0 psf bottom  
chord live load nonconcurrent with any other live loads.
  - 8) \* This truss has been designed for a live load of 20.0psf  
on the bottom chord in all areas where a rectangle  
3-06-00 tall by 2-00-00 wide will fit between the bottom  
chord and any other members.
  - 9) All bearings are assumed to be SP No.2 crushing  
capacity of 565 psi.
  - 10) Provide mechanical connection (by others) of truss to  
bearing plate capable of withstanding 27 lb uplift at joint  
1 and 34 lb uplift at joint 3.
  - 11) This truss is designed in accordance with the 2018  
International Residential Code sections R502.11.1 and  
R802.10.2 and referenced standard ANSI/TPI 1.
- LOAD CASE(S)** Standard



April 23, 2024

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AS NOTED FOR PLAN REVIEW  
DEVELOPMENT SERVICES  
P240893 SUMMIT, MISSOURI 7

Job Truss

Truss Type

Qty

Ply

I65080539

Valley

1

1

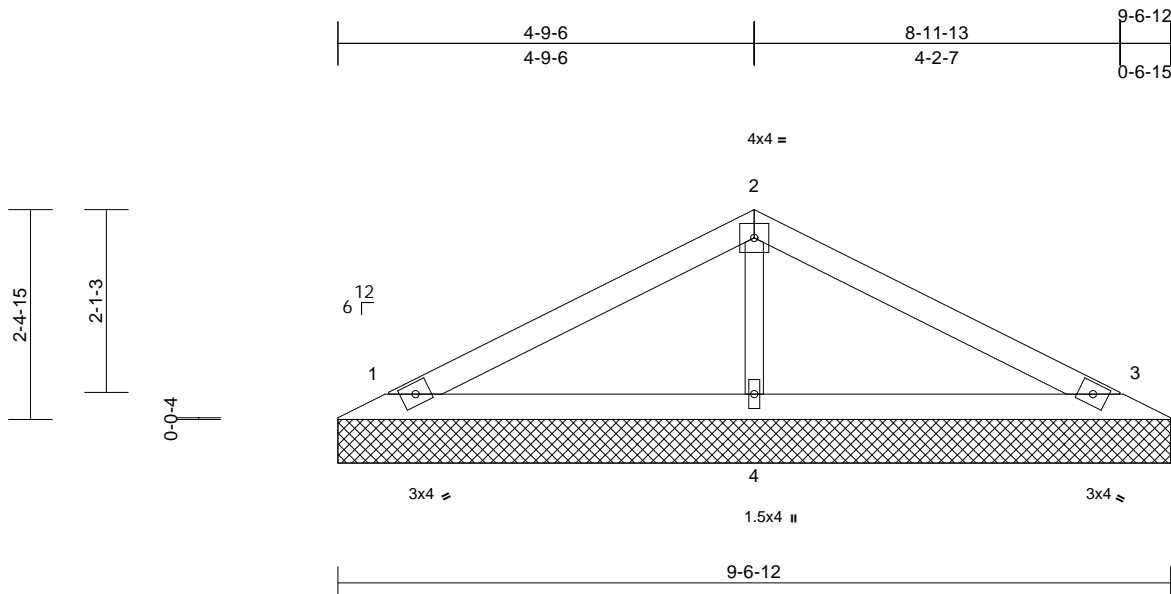
Job Reference (optional)

Premier Building Supply (Spring Hill), KS, Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:35

Page: 1

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Scale = 1:26.5

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.43	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.18	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horiz(TL)	0.00	3	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
										Weight: 30 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x3 SPF No.2

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS**

(size) 1=9-6-12, 3=9-6-12, 4=9-6-12  
Max Horiz 1=40 (LC 16)  
Max Uplift 1=-42 (LC 16), 3=-49 (LC 17), 4=-32 (LC 16)  
Max Grav 1=256 (LC 22), 3=256 (LC 23), 4=406 (LC 23)

**FORCES**

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-147/69, 2-3=-147/77  
BOT CHORD 1-4=-3/46, 3-4=-3/46  
WEBS 2-4=-278/200

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1, 49 lb uplift at joint 3 and 32 lb uplift at joint 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard

April 23, 2024

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcsccomponents.com](http://www.sbcsccomponents.com))

**MiTek®**

16023 Swingley Ridge Rd.  
Chesterfield, MO 63017  
314.434.1200 / MiTek-US.com

Job  
AS NOTED FOR PLAN REVIEW  
DEVELOPMENT SERVICES  
P240893 SUMMIT, MISSOURI 8

Premier Building Supply (Spring Hill) KS, Spring Hills, KS - 66083,

Truss Type

Valley

Qty

1

Ply

1

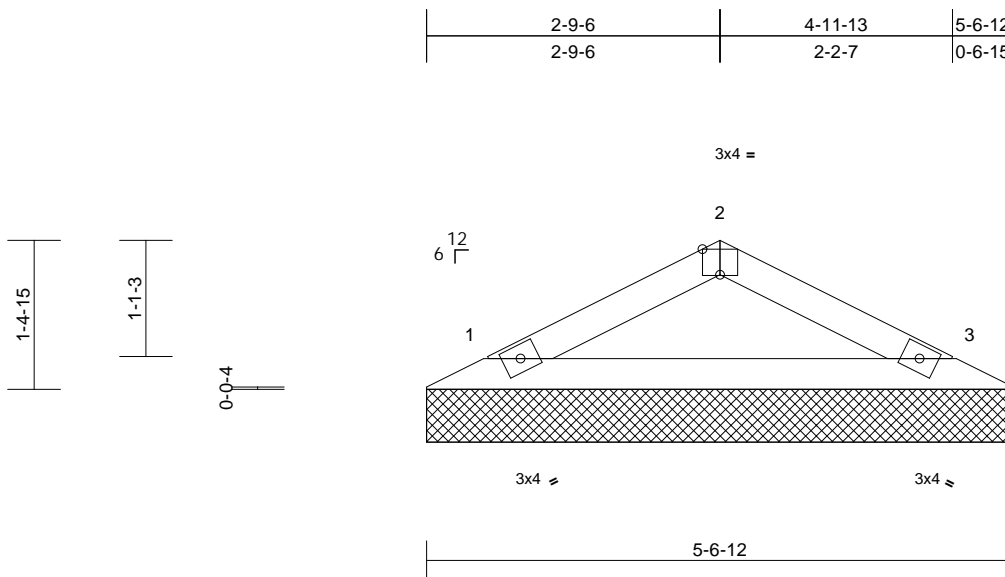
Job Reference (optional)

I65080540

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:36

Page: 1

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Scale = 1:21.8

Plate Offsets (X, Y): [2:0-2-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.13	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.24	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-P							
BCDL	10.0										
Weight: 16 lb FT = 20%											

#### LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 5-7-12 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size) 1=5-6-12, 3=5-6-12

Max Horiz 1=21 (LC 16)

Max Uplift 1=30 (LC 16), 3=30 (LC 17)

Max Grav 1=223 (LC 22), 3=223 (LC 23)

#### FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-200/174, 2-3=-200/184

BOT CHORD 1-3=-119/153

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 1 and 30 lb uplift at joint 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

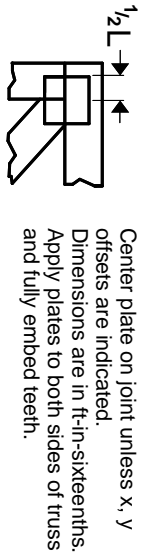
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcsccomponents.com](http://www.sbcsccomponents.com))

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# Symbols

## PLATE LOCATION AND ORIENTATION



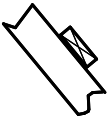
For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

## PLATE SIZE

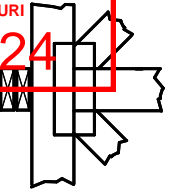
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



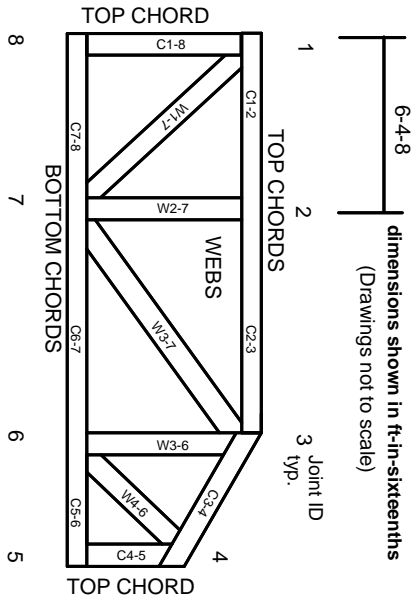
Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

## BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

# Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## Product Code Approvals

ICC-ES Reports:  
ESR-1988, ESR-2362, ESR-2685, ESR-3282  
ESR-4722, ESL-1388

## Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.  
Lumber design values are in accordance with ANSI/TPI 1 section 6.3. These truss designs rely on lumber values established by others.

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# General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.