

RE: P240393

MiTek, Inc.
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200

Site Information:

Customer: Clayton Properties Project Name: P240393
Lot/Block: 192 Model:
Address: 3212 SW Arboridge Cir Subdivision: Hawthorne Ridge
City: Lee's Summit State: MO

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014 Design Program: MiTek 20/20 8.6
Wind Code: ASCE 7-16 Wind Speed: 115 mph
Roof Load: 45.0 psf Floor Load: N/A psf

This package includes 24 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I65080517	A1	4/23/2024	21	I65080537	V6	4/23/2024
2	I65080518	A2	4/23/2024	22	I65080538	V07	4/23/2024
3	I65080519	A3	4/23/2024	23	I65080539	V7	4/23/2024
4	I65080520	A4	4/23/2024	24	I65080540	V8	4/23/2024
5	I65080521	A6	4/23/2024				
6	I65080522	A7	4/23/2024				
7	I65080523	B01	4/23/2024				
8	I65080524	B02	4/23/2024				
9	I65080525	C1	4/23/2024				
10	I65080526	C2	4/23/2024				
11	I65080527	C3	4/23/2024				
12	I65080528	C4	4/23/2024				
13	I65080529	C5	4/23/2024				
14	I65080530	PB1	4/23/2024				
15	I65080531	PB2	4/23/2024				
16	I65080532	V1	4/23/2024				
17	I65080533	V2	4/23/2024				
18	I65080534	V3	4/23/2024				
19	I65080535	V4	4/23/2024				
20	I65080536	V5	4/23/2024				

The truss drawing(s) referenced above have been prepared by
MiTek USA, Inc. under my direct supervision
based on the parameters provided by .

Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of Missouri is December 31, 2025.

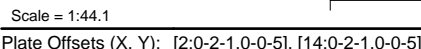
Missouri COA: 001193

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek. Any project specific information included is for MiTek customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



April 23, 2024

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:30 Page: 1
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LUMBER		BOT CHORD	2-25=23/121, 24-25=23/121, 23-24=23/121, 22-23=23/121, 20-22=23/121, 19-20=23/121, 18-19=23/121, 17-18=23/121, 16-17=23/121, 14-16=23/121	10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
TOP CHORD	2x4 SP No.2			
BOT CHORD	2x4 SP No.2			11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
OTHERS	2x3 SPF No.2			
SLIDER	Left 2x4 SP No.2 -- 1-6-7, Right 2x4 SP No.2 -- 1-6-7	WEBS	8-20=120/7, 7-22=236/96, 6-23=224/102, 5-24=180/117, 4-25=149/185, 9-19=236/96, 10-18=224/102, 11-17=180/118, 12-16=149/181	12) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
BRACING		NOTES		13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 2, 62 lb uplift at joint 22, 63 lb uplift at joint 23, 55 lb uplift at joint 24, 93 lb uplift at joint 25, 61 lb uplift at joint 19, 64 lb uplift at joint 18, 56 lb uplift at joint 17, 85 lb uplift at joint 16 and 4 lb uplift at joint 14.
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.		1) Unbalanced roof live loads have been considered for this design.	
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.		2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TC DL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) 0-11-0 to 4-4-0, Exterior(2N) 4-4-0 to 10-4-0, Corner(3R) 10-4-0 to 15-4-0, Exterior(2N) 15-4-0 to 21-7-0 zone; cantilever	14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
REACTIONS (size)		LOAD CASE(S) Standard		
	2=20-8-0, 14=20-8-0, 16=20-8-0, 17=20-8-0, 18=20-8-0, 19=20-8-0, 20=20-8-0, 22=20-8-0, 23=20-8-0, 24=20-8-0, 25=20-8-0			
Max Horiz	2=106 (LC 20)			
Max Uplift	2=27 (LC 17), 14=-4 (LC 13), 16=27 (LC 17), 17=27 (LC 17), 18=27 (LC 17), 19=27 (LC 17), 20=27 (LC 17), 22=27 (LC 17), 23=27 (LC 17), 24=27 (LC 17), 25=27 (LC 17)			

	16=85 (LC 17), 17=56 (LC 17), 18=64 (LC 17), 19=61 (LC 17), 22=62 (LC 16), 23=63 (LC 16), 24=55 (LC 16), 25=93 (LC 16) 2=183 (LC 1), 14=183 (LC 1), 16=198 (LC 37), 17=218 (LC 24), 18=265 (LC 24), 19=276 (LC 24), 20=161 (LC 29), 22=276 (LC 23), 23=265 (LC 23), 24=218 (LC 23), 25=198 (LC 36)	left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60	
Max Grav			
		3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10	
FORCES	(lb) - Maximum Compression/Maximum Tension	5) Unbalanced snow loads have been considered for this design.	
TOP CHORD	1-2=0/12, 2-4=-137/57, 4-5=-85/73, 5-6=-75/101, 6-7=-76/158, 7-8=-83/211, 8-9=-83/211, 9-10=-76/158, 10-11=-75/102, 11-12=-60/39, 12-14=-104/19, 14-15=0/12	6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.	
		7) All plates are 1.5x4 MT20 unless otherwise indicated.	
		8) Gable requires continuous bottom chord bearing.	
		9) Gable studs spaced at 2'-0" oc.	



April 23, 2024

 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE

WARNING – verify design parameters and noted notes on this and included MiTek Reference Tag M7473 Rev. 1/2/2023 before use. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)

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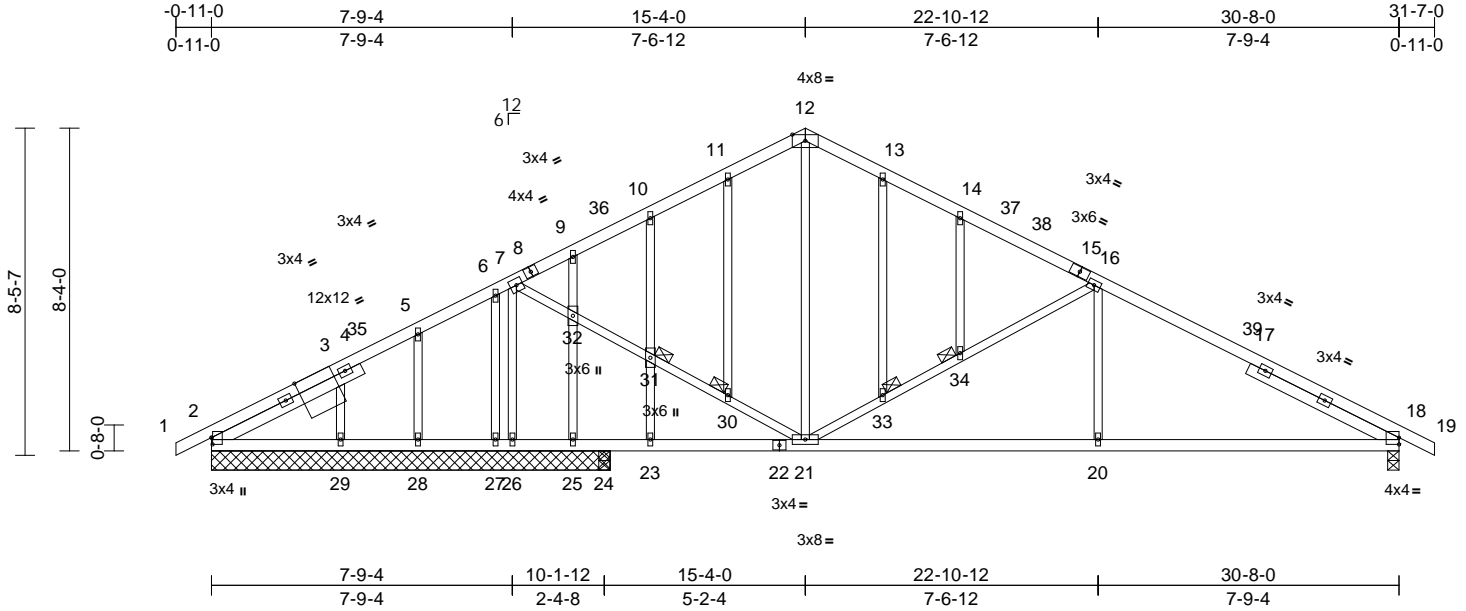
Job	Truss	Truss Type	Qty	Ply	
P240393	A2	Common Structural Gable	1	1	I65080518
Job Reference (optional)					

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:31

Page: 1

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Scale = 1:59.5

Plate Offsets (X, Y): [2:0-2-1,0-0-5], [3:2-6-7,Edge], [18:Edge,0-2-1]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.85	Vert(LL)	-0.09	18-20	>999	240	244/190
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.63	Vert(CT)	-0.19	18-20	>999	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(CT)	0.03	18	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
Weight: 166 lb FT = 20%											

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2
SLIDER	Left 2x4 SP No.2 -- 4-3-11, Right 2x4 SP No.2 -- 4-3-11

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-1-9 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 20-21,18-20.
JOINTS	1 Brace at Jt(s): 30, 31, 33, 34

REACTIONS	(size)	2=10-3-8, 18=0-3-8, 24=0-3-8, 25=10-3-8, 26=10-3-8, 27=10-3-8, 28=10-3-8, 29=10-3-8
	Max Horiz	2=153 (LC 20)
	Max Uplift	2=42 (LC 17), 18=213 (LC 17), 24=161 (LC 16), 25=107 (LC 23), 26=22 (LC 17), 27=11 (LC 17), 28=62 (LC 16), 29=101 (LC 16)
	Max Grav	2=166 (LC 36), 18=1104 (LC 24), 24=686 (LC 23), 25=75 (LC 24), 26=607 (LC 24), 27=331 (LC 24), 28=153 (LC 36), 29=281 (LC 1)

FORCES

TOP CHORD	(lb) - Maximum Compression/Maximum Tension
	1-2=0/12, 2-3=-132/254, 3-5=-42/232, 5-6=0/240, 6-7=-13/159, 7-9=-669/193, 9-10=-721/221, 10-11=-704/248, 11-12=-646/255, 12-13=-639/230, 13-14=-707/213, 14-16=-845/201, 16-18=-1653/298, 18-19=0/12

BOT CHORD	2-29=-192/119, 28-29=-194/119, 27-28=-194/119, 26-27=-194/119, 25-26=-194/119, 24-25=-194/119, 23-24=-194/119, 21-23=-194/119, 20-21=-148/1376, 18-20=-148/1376
WEBS	12-21=-65/241, 21-33=-895/294, 33-34=-871/283, 16-34=-853/280, 16-20=0/335, 7-32=-42/935, 31-32=-44/930, 30-31=-41/918, 21-30=-44/947, 7-26=-570/45, 11-30=-23/60, 10-31=-283/116, 23-31=-301/120, 9-32=-274/85, 25-32=-276/91, 6-27=-314/52, 5-28=-122/83, 3-29=-216/135, 13-33=-47/22, 14-34=-38/12

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 15-4-0, Exterior(2R) 15-4-0 to 20-4-0, Interior (1) 20-4-0 to 31-7-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 2, 22 lb uplift at joint 26, 213 lb uplift at joint 18, 107 lb uplift at joint 25, 11 lb uplift at joint 27, 62 lb uplift at joint 28, 101 lb uplift at joint 29 and 161 lb uplift at joint 24.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23,2024

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of the design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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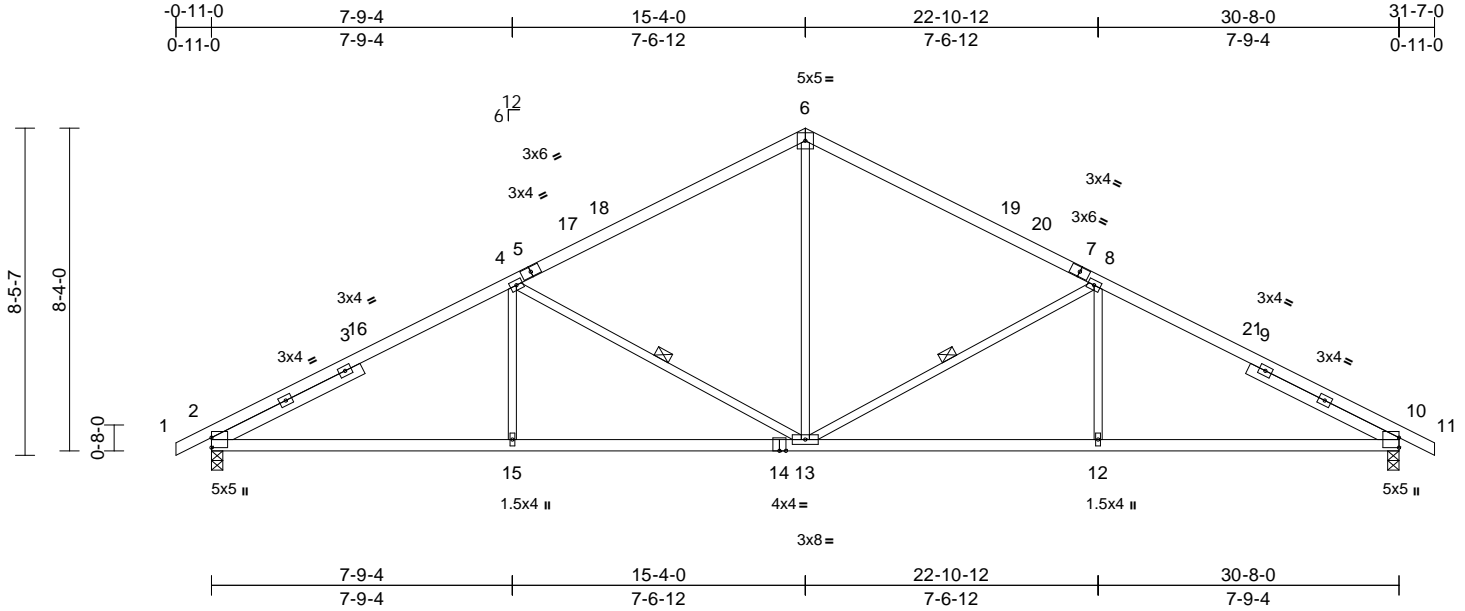
Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
P240393	A3	Common	6	1	165080519

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:32

Page: 1

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	-0.10	12-13	>999	240	MT20	244/190
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.71	Vert(CT)	-0.22	2-15	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(CT)	0.10	10	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 139 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP 2400F 2.0E *Except* 1-5,7-11:2x4 SP 1650F 1.5E
BOT CHORD 2x4 SP No.2
WEBS 2x3 SPF No.2
SLIDER Left 2x4 SP No.2 -- 4-3-11, Right 2x4 SP No.2 -- 4-3-11

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 8-13, 4-13

REACTIONS (size) 2=0-3-8, 10=0-3-8
Max Horiz 2=-153 (LC 17)
Max Uplift 2=-233 (LC 16), 10=-233 (LC 17)
Max Grav 2=1451 (LC 23), 10=1451 (LC 24)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/12, 2-4=-2366/375, 4-6=-1634/361, 6-8=-1634/361, 8-10=-2365/375, 10-11=0/12
BOT CHORD 2-15=-339/2003, 13-15=-339/2003, 12-13=-230/2003, 10-12=-230/2003
WEBS 6-13=-95/841, 8-13=-826/288, 8-12=0/322, 4-13=-826/287, 4-15=0/322

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 15-4-0, Exterior(2R) 15-4-0 to 20-4-0, Interior (1) 20-4-0 to 31-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 2 and 233 lb uplift at joint 10.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23,2024

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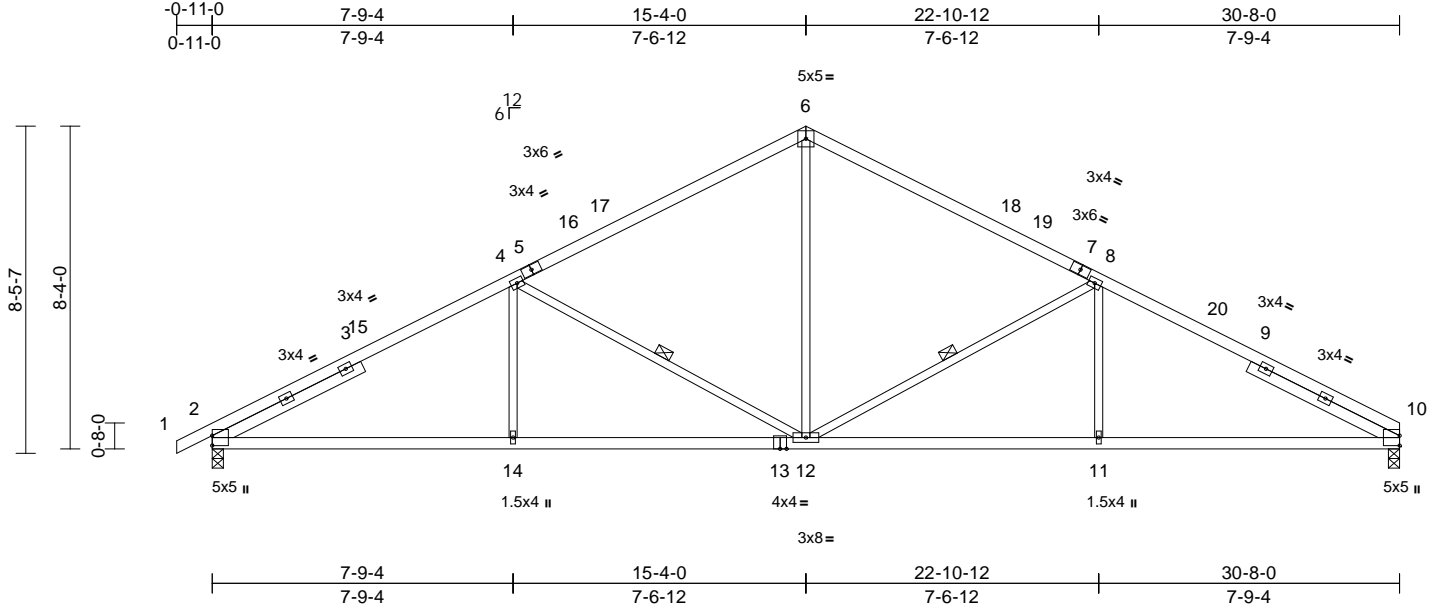
Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
P240393	A4	Common	3	1	165080520

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Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:32

Page: 1

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Scale = 1:59.5

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.76	-0.10	10-11	>999	240	MT20	244/190
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.72	-0.23	10-11	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.58	0.10	10	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
										Weight: 138 lb FT = 20%	

LUMBER

TOP CHORD	2x4 SP 2400F 2.0E *Except* 1-5,7-10:2x4 SP 1650F 1.5E
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
SLIDER	Left 2x4 SP No.2 -- 4-3-11, Right 2x4 SP No.2 -- 4-3-11

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-10-14 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 8-12, 4-12

REACTIONS

(size)	2=0-3-8, 10=0-3-8
Max Horiz	2=155 (LC 16)
Max Uplift	2=233 (LC 16), 10=209 (LC 17)
Max Grav	2=1452 (LC 23), 10=1386 (LC 24)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/12, 2-4=-2367/376, 4-6=-1636/362, 6-8=-1637/366, 8-10=-2371/386
BOT CHORD	2-14=-341/2004, 12-14=-341/2004, 11-12=-235/2009, 10-11=-235/2009
WEBS	6-12=-100/843, 8-12=-831/290, 8-11=0/325, 4-12=-826/287, 4-14=0/323

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 15-4-0, Exterior(2R) 15-4-0 to 20-4-0, Interior (1) 20-4-0 to 30-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 2 and 209 lb uplift at joint 10.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Job	Truss	Truss Type	Qty	Ply	
P240393	A6	Monopitch	3	1	
					Job Reference (optional)

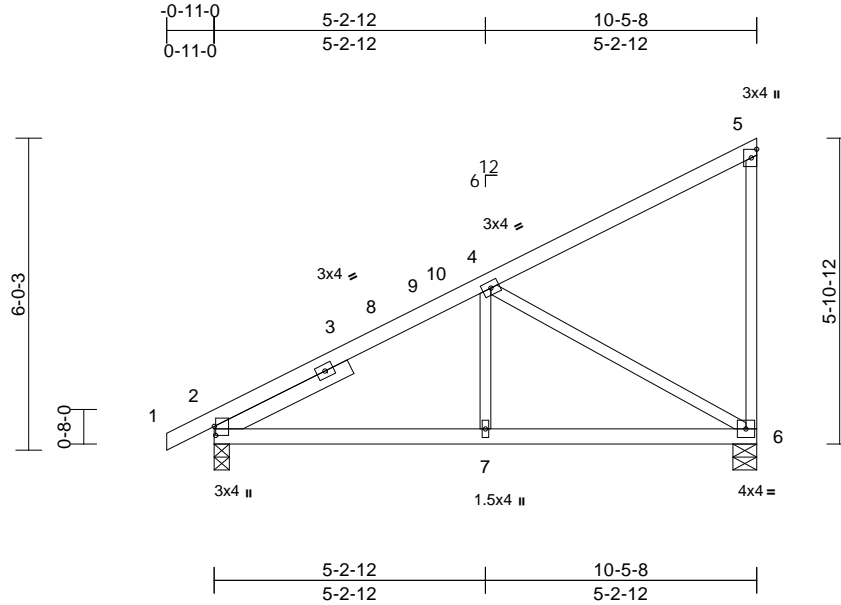
I65080521

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:32

Page: 1

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Scale = 1:44.4

Plate Offsets (X, Y): [2:0-2-1,0-0-5]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	-0.02	6-7	>999	240	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.29	Vert(CT)	-0.04	6-7	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
											Weight: 50 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x3 SPF No.2
 SLIDER Left 2x4 SP No.2 -- 2-10-10

BRACING

TOP CHORD Structural wood sheathing directly applied or
 6'-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 9'-7-1 oc
 bracing.

REACTIONS (size) 2=0-3-8, 6=0-5-8
 Max Horiz 2=248 (LC 13)
 Max Uplift 2=-93 (LC 16), 6=-134 (LC 16)
 Max Grav 2=599 (LC 23), 6=633 (LC 23)

FORCES (lb) - Maximum Compression/Maximum
 Tension
 TOP CHORD 1-2=0/12, 2-4=-756/178, 4-5=-167/129,
 5-6=-241/163

BOT CHORD 2-7=-360/609, 6-7=-360/609
 WEBS 4-7=0/241, 4-6=-691/321

NOTES

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust)
 Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft;
 Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope)
 exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0,
 Interior (1) 4-1-0 to 10-4-4 zone; cantilever left and right
 exposed; end vertical left and right exposed; C-C for
 members and forces & MWFRS for reactions shown;
 Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15
 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate
 DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9;
 Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this
 design.

- 4) This truss has been designed for greater of min roof live
 load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on
 overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom
 chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf
 on the bottom chord in all areas where a rectangle
 3'-06-00 tall by 2'-00-00 wide will fit between the bottom
 chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing
 capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to
 bearing plate capable of withstanding 134 lb uplift at
 joint 6 and 93 lb uplift at joint 2.
- 9) This truss is designed in accordance with the 2018
 International Residential Code sections R502.11.1 and
 R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

April 23, 2024

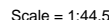
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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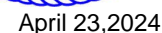
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[illegible]

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust)
Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft;
Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope)
exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0,
Interior (1) 4-1-0 to 10-4-4 zone; cantilever left and right
exposed ; end vertical left and right exposed; C-C for
members and forces & MWFRS for reactions shown;
Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss
only. For studs exposed to wind (normal to the face),
see Standard Industry Gable End Details as applicable,
or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15
Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate
DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9;
Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this
design.
- 5) This truss has been designed for greater of min roof live
load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on
overhangs non-concurrent with other live loads.
- 6) All plates are 1.5x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom
chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members.
- 11) All bearings are assumed to be SP No.2 crushing
capacity of 565 psi.
- 12) Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 33 lb uplift at joint
9, 7 lb uplift at joint 2, 65 lb uplift at joint 10, 62 lb uplift
at joint 11, 52 lb uplift at joint 12 and 106 lb uplift at joint
13.

LOAD CASE(S) Standard



WARNING – verify design parameters and noted notes on this and included MiTek Reference Tag M7473 Rev. 1/2/2023 before use. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)

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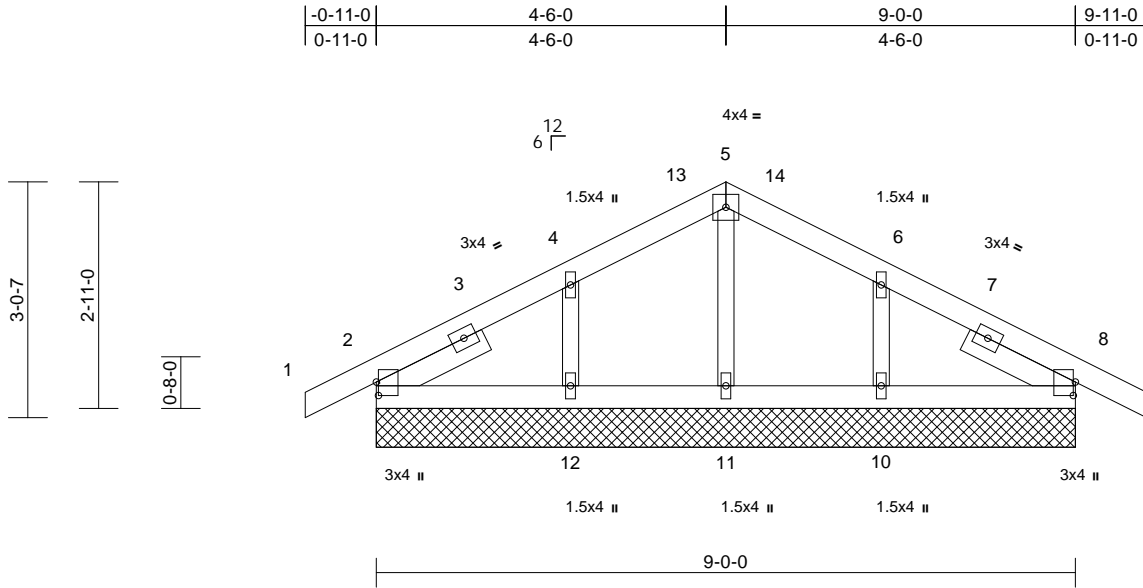
Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
P240393	B01	Common	1	1	I65080523

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Page: 1

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Scale = 1:29.7

Plate Offsets (X, Y): [2:0-2-1,0-0-5], [8:0-2-1,0-0-5]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	999	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	8	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
										Weight: 40 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x3 SPF No.2
SLIDER	Left 2x4 SP No.2 -- 1-6-7, Right 2x4 SP No.2 -- 1-6-7

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	2=9-0-0, 8=9-0-0, 10=9-0-0, 11=9-0-0, 12=9-0-0
Max Horiz	2=-51 (LC 21)
Max Uplift	2=-38 (LC 16), 8=-49 (LC 17), 10=-80 (LC 17), 12=-84 (LC 16)
Max Grav	2=287 (LC 23), 8=287 (LC 24), 10=316 (LC 24), 11=116 (LC 24), 12=316 (LC 23)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/12, 2-4=-136/77, 4-5=-107/177, 5-6=-107/175, 6-8=-136/68, 8-9=0/12
BOT CHORD	2-12=-2/54, 11-12=-2/54, 10-11=-2/54, 8-10=-2/54
WEBS	5-11=-83/0, 4-12=-262/223, 6-10=-262/221

NOTES

- 1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-11-0 to 4-1-0, Exterior(2N) 4-1-0 to 4-6-0, Corner(3R) 4-6-0 to 9-6-0, Exterior(2N) 9-6-0 to 9-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 2, 49 lb uplift at joint 8, 84 lb uplift at joint 12 and 80 lb uplift at joint 10.
- 13) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 8.

- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024

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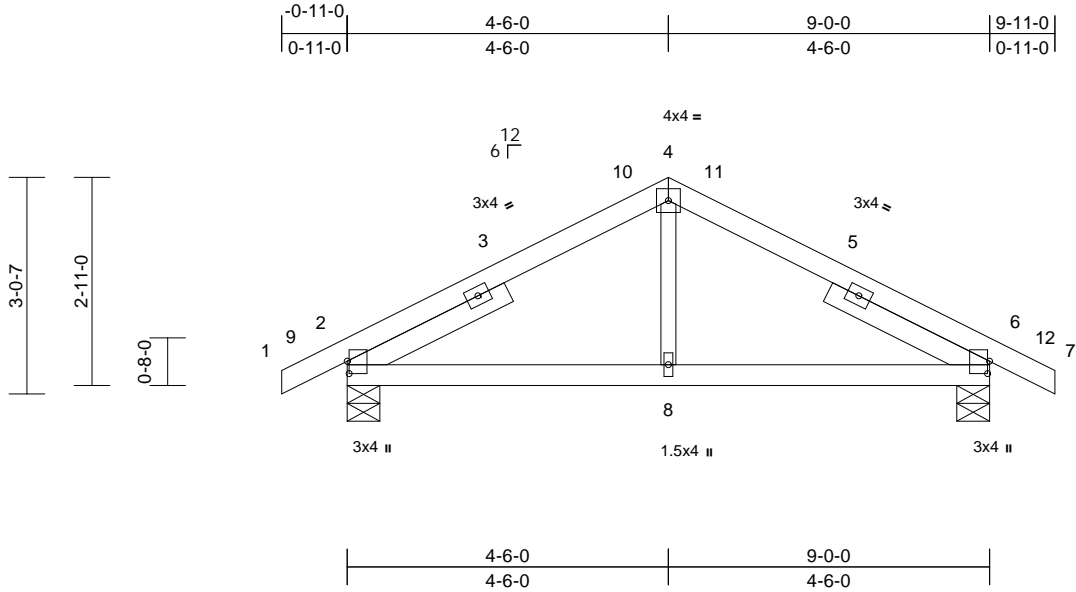
Job	Truss	Truss Type	Qty	Ply	
P240393	B02	Common	2	1	
Job Reference (optional)					I65080524

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Page: 1

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Scale = 1:32.3

Plate Offsets (X, Y): [2:0-2-1,0-0-5], [6:0-2-1,0-0-5]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.41	Vert(LL)	-0.01	2-8	>999	240	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.22	Vert(CT)	-0.02	2-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
											Weight: 41 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SP No.2
SLIDER	Left 2x4 SP No.2 -- 2-5-12, Right 2x4 SP No.2 -- 2-5-12

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6'-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS

(size)	2=0-5-8, 6=0-5-8
Max Horiz	2=-51 (LC 17)
Max Uplift	2=-84 (LC 16), 6=-84 (LC 17)
Max Grav	2=608 (LC 23), 6=608 (LC 24)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/12, 2-4=-590/257, 4-6=-590/257, 6-7=0/12
BOT CHORD	2-8=-111/389, 6-8=-111/389
WEBS	4-8=0/204

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 4-6-0, Exterior(2R) 4-6-0 to 9-6-0, Interior (1) 9-6-0 to 9-11-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-06-00 tall by 2'-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 2 and 84 lb uplift at joint 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024

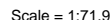
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	
P240393	C1	Piggyback Base Supported Gable	1	1	I65080525
Job Reference (optional)					

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Page: 2

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- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-11-0 to 4-1-0, Exterior(2N) 4-1-0 to 18-8-0, Corner(3R) 18-8-0 to 23-7-8, Exterior(2N) 23-7-8 to 28-7-6, Corner(3R) 28-7-6 to 33-7-8, Exterior(2N) 33-7-8 to 39-1-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 1.5x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 13) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 27, 64 lb uplift at joint 2, 28 lb uplift at joint 38, 13 lb uplift at joint 39, 73 lb uplift at joint 41, 60 lb uplift at joint 42, 61 lb uplift at joint 43, 61 lb uplift at joint 44, 61 lb uplift at joint 45, 61 lb uplift at joint 46, 60 lb uplift at joint 47, 109 lb uplift at joint 48, 51 lb uplift at joint 37, 39 lb uplift at joint 35, 50 lb uplift at joint 34, 24 lb uplift at joint 33, 74 lb uplift at joint 31, 59 lb uplift at joint 30, 63 lb uplift at joint 29 and 59 lb uplift at joint 28.
- 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

⚠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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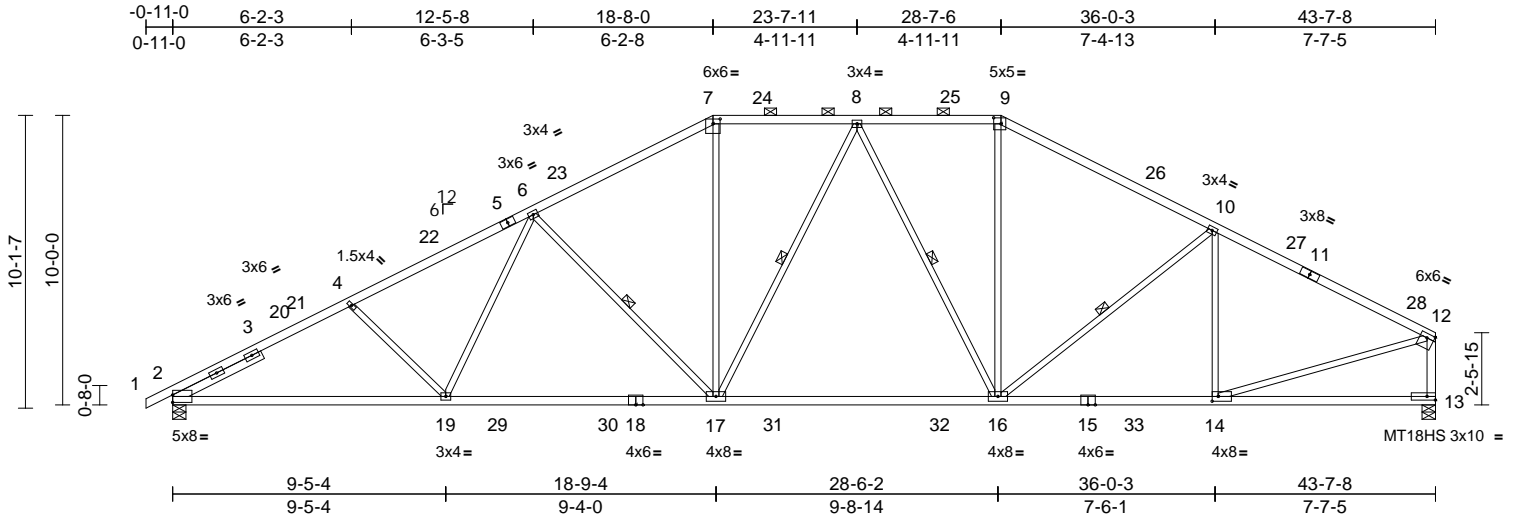
Job	Truss	Truss Type	Qty	Ply	
P240393	C2	Piggyback Base	10	1	I65080526
Job Reference (optional)					

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:33

Page: 1

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Scale = 1:79.6

Plate Offsets (X, Y): [2:Edge,0-3-1], [7:0-3-0,0-2-0], [9:0-3-0,0-2-8], [12:Edge,0-1-12], [14:0-2-8,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.81	Vert(LL)	-0.39	16-17	>999	240	MT20	244/190
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.99	Vert(CT)	-0.62	16-17	>841	180	MT18HS	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(CT)	0.16	13	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
Weight: 213 lb FT = 20%												

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E *Except* 5-7:2x4 SP 1650F 1.5E, 7-9:2x4 SP No.2
BOT CHORD 2x4 SP 1650F 1.5E *Except* 15-13:2x4 SP No.2, 18-15:2x4 SP 2400F 2.0E
WEBS 2x3 SPF No.2 *Except* 13-12:2x4 SP No.2
SLIDER Left 2x4 SP No.2 -- 3-5-1

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-1 oc purlins, except end verticals, and 2-0-0 oc purlins (3-3-12 max.): 7-9.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 10-16, 6-17, 8-17, 8-16

REACTIONS

(size) 2=0-5-8, 13=0-5-8
Max Horiz 2=205 (LC 13)
Max Uplift 2=-296 (LC 17), 13=-232 (LC 17)
Max Grav 2=2504 (LC 47), 13=2527 (LC 47)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/12, 2-4=-4488/592, 4-6=-4176/575, 6-7=-3158/553, 7-8=-2698/542, 8-9=-2470/524, 9-10=-2934/518, 10-12=-3141/452, 12-13=-2439/370
BOT CHORD 2-19=-587/3840, 17-19=-494/3412, 16-17=-350/2610, 14-16=-365/2699, 13-14=-55/130
WEBS 7-17=-90/1000, 9-16=-66/832, 10-16=-407/222, 10-14=-626/188, 12-14=-324/2689, 4-19=-374/230, 6-19=-50/612, 6-17=-1142/309, 8-17=-245/310, 8-16=-583/171

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 18-8-0, Exterior(2R) 18-8-0 to 23-7-11, Interior (1) 23-7-11 to 28-7-6, Exterior(2R) 28-7-6 to 33-7-6, Interior (1) 33-7-6 to 43-5-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 2 SP 1650F 1.5E crushing capacity of 565 psi, Joint 13 SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2 and 232 lb uplift at joint 13.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 23, 2024

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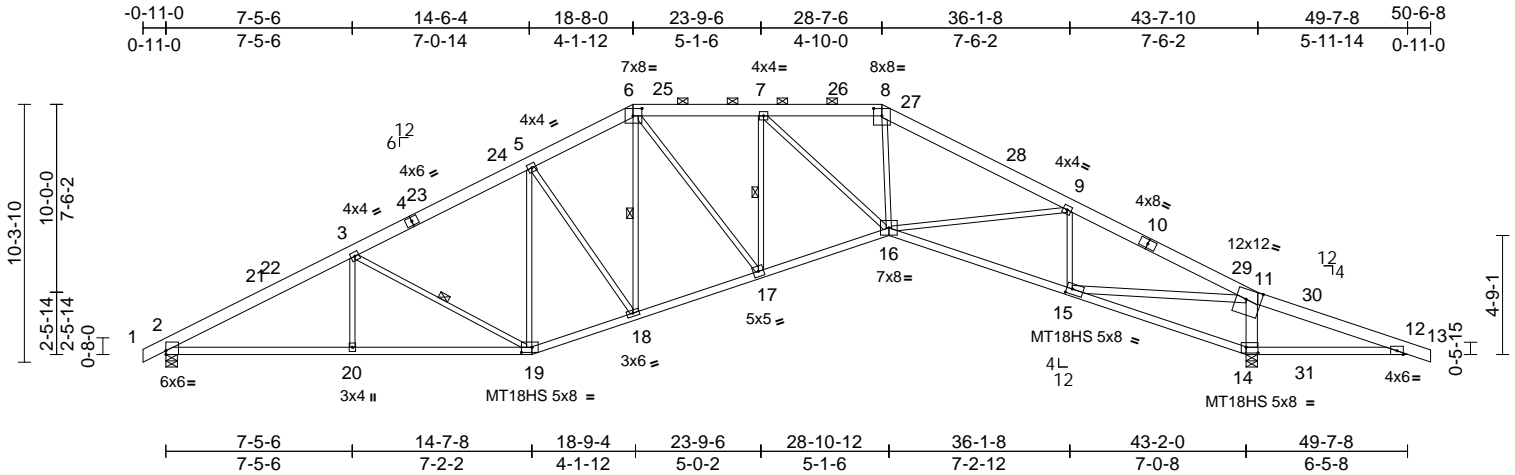
Job	Truss	Truss Type	Qty	Ply	
P240393	C3	Piggyback Base	3	1	Job Reference (optional)
					I65080527

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:33

Page: 1

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Scale = 1:92.1

Plate Offsets (X, Y): [2:Edge,0-1-14], [6:0-4-4,0-3-8], [8:0-4-0,0-3-8], [11:0-7-4,0-4-12], [14:0-5-12,0-2-12], [15:0-2-12,0-2-8], [19:0-5-0,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.41	16	>999	240	MT20 197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.78	Vert(CT)	-0.66	15-16	>782	180	MT18HS 197/144
TCDL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(CT)	0.45	14	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
Weight: 248 lb FT = 20%											

LUMBER
TOP CHORD 2x6 SPF No.2
BOT CHORD 2x4 SP 1650F 1.5E *Except* 14-12:2x4 SP No.2
WEBS 2x3 SPF No.2 *Except* 11-14:2x6 SPF No.2, 15-11:2x4 SP 1650F 1.5E

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (3-4-6 max.): 6-8.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
5-7-10 oc bracing: 14-15
5-1-12 oc bracing: 12-14.
WEBS 1 Row at midpt 3-19, 6-18, 7-17

REACTIONS (size) 2=0-5-8, 14=0-5-8
Max Horiz 2=-185 (LC 21)
Max Uplift 2=-298 (LC 16), 14=-388 (LC 17)
Max Grav 2=2449 (LC 43), 14=2927 (LC 43)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/23, 2-3=-4395/707, 3-5=-3551/665, 5-6=-3341/687, 6-7=-3490/684, 7-8=-4355/647, 8-9=-5206/651, 9-11=-4577/322, 11-12=-1178/1323, 12-13=0/23
BOT CHORD 2-20=-500/3774, 19-20=-500/3774, 18-19=-323/3172, 17-18=-233/3032, 16-17=-278/3693, 15-16=-127/4186, 14-15=-1571/1393, 12-14=-1149/1162
WEBS 3-20=0/326, 3-19=-882/246, 5-19=-463/54, 5-18=-313/294, 6-18=-205/404, 6-17=-82/1199, 8-16=-76/1700, 9-16=-221/988, 9-15=-1322/463, 7-17=-1085/145, 7-16=0/1193, 11-14=-2523/635, 11-15=-1143/4925

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCCL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 18-8-0, Exterior(2R) 18-8-0 to 23-9-6, Interior (1) 23-9-6 to 28-7-6, Exterior(2R) 28-7-6 to 33-7-6, Interior (1) 33-7-6 to 50-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP 1650F 1.5E crushing capacity of 565 psi, Joint 14 SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 298 lb uplift at joint 2 and 388 lb uplift at joint 14.

- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 23, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of the design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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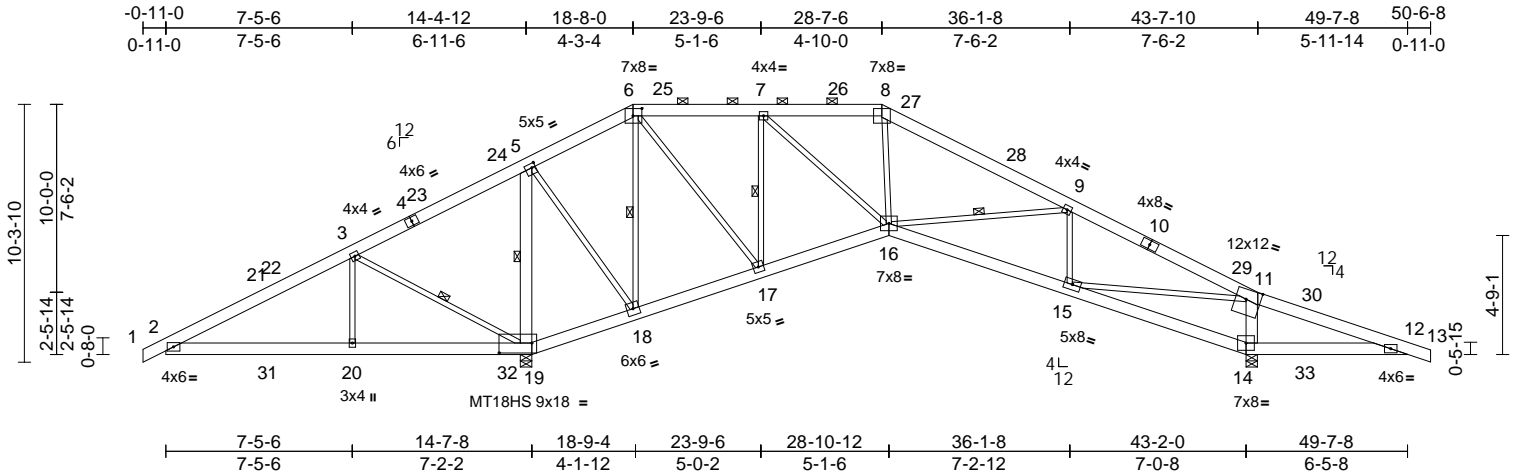
Job	Truss	Truss Type	Qty	Ply	
P240393	C4	Piggyback Base	6	1	I65080528
Job Reference (optional)					

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Page: 1

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Scale = 1:92.1

Plate Offsets (X, Y): [5:0-1-12,0-2-0], [6:0-4-4,0-3-8], [11:0-7-0,0-4-12], [19:1-3-12,0-4-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.70	Vert(LL)	-0.13	15-16	>999	240	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.40	Vert(CT)	-0.21	15-16	>999	180	MT18HS	197/144
TCDL	10.0	Rep Stress Incr	YES	WB	0.89	Horz(CT)	0.12	14	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
Weight: 264 lb FT = 20%												

LUMBER

TOP CHORD 2x6 SPF No.2
 BOT CHORD 2x6 SPF No.2
 WEBS 2x3 SPF No.2 *Except* 19-5,11-14:2x6 SPF No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-11-13 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-8.
 BOT CHORD Rigid ceiling directly applied or 5-4-14 oc bracing.
 WEBS 1 Row at midpt 5-19, 6-18, 7-17, 9-16, 3-19

REACTIONS (size) 14=0-5-8, 19=0-5-8, (req. 0-5-13)
 Max Horiz 19=185 (LC 17)
 Max Uplift 14=350 (LC 17), 19=446 (LC 16)
 Max Grav 14=1846 (LC 24), 19=3691 (LC 43)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/32, 2-3=-874/932, 3-5=-1633/2034, 5-6=-1124/972, 6-7=-915/449, 7-8=-1231/94, 8-9=-1533/173, 9-11=-1942/124, 11-12=-1225/1352, 12-13=0/32
 BOT CHORD 2-20=-649/871, 19-20=-649/871, 18-19=-1845/1860, 17-18=-898/1483, 16-17=-489/1423, 15-16=0/1753, 14-15=-1488/1350, 12-14=-1191/1222
 WEBS 3-20=-529/342, 5-19=-2450/884, 6-18=-1492/695, 5-18=-607/1575, 7-17=-1128/153, 8-16=-252/738, 9-15=-565/176, 9-16=-785/442, 3-19=-1149/985, 6-17=-88/1232, 7-16=0/1208, 11-14=-1318/313, 11-15=-248/2408

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior (1) 4-1-0 to 18-8-0, Exterior(2R) 18-8-0 to 23-9-6, Interior (1) 23-9-6 to 28-7-6, Exterior(2R) 28-7-6 to 33-7-6, Interior (1) 33-7-6 to 50-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- WARNING: Required bearing size at joint(s) 19 greater than input bearing size.
- All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 446 lb uplift at joint 19 and 350 lb uplift at joint 14.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 23, 2024

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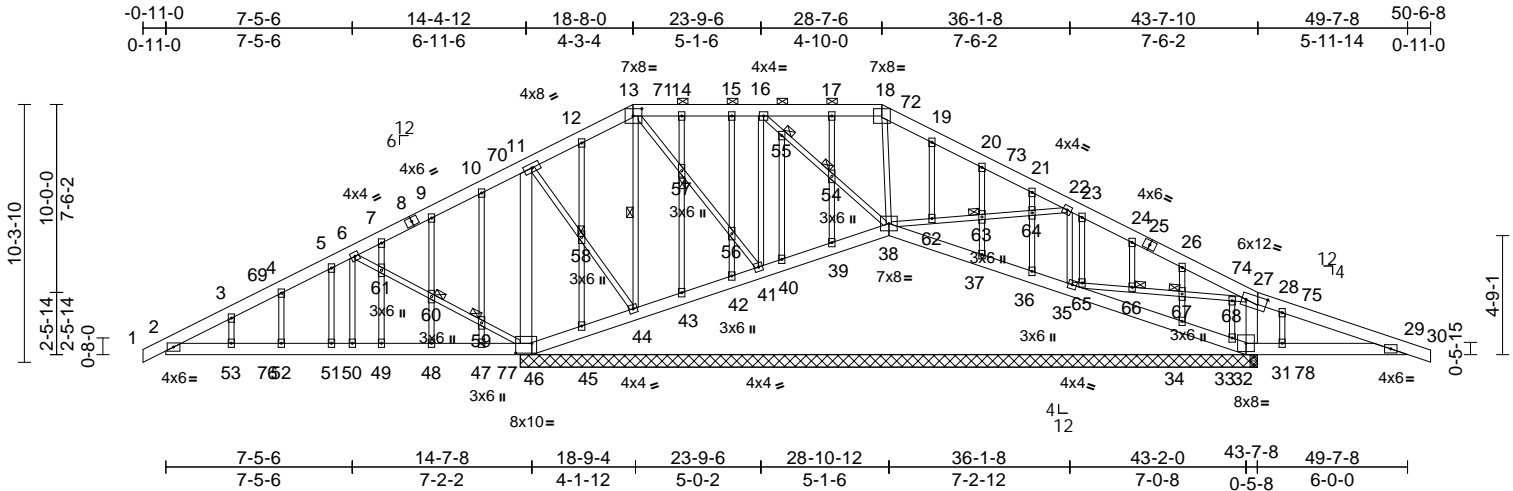
Job	Truss	Truss Type	Qty	Ply	
P240393	C5	Piggyback Base Structural Gable	1	1	Job Reference (optional)
I65080529					

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Scale = 1:92.1

Plate Offsets (X, Y): [13:0-4-4,0-3-8], [27:0-10-0,0-3-0], [46:0-7-12,0-4-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.64	Vert(LL)	0.00	45-46	>999	240	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.36	Vert(CT)	0.00	34-35	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(CT)	0.03	32	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0											
											Weight: 321 lb	FT = 20%

LUMBER

TOP CHORD	2x6 SPF No.2
BOT CHORD	2x6 SPF No.2
WEBS	2x3 SPF No.2 *Except* 46-11,27-32:2x6 SPF No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-6-6 oc purlins, except 2-0-0 oc purlins (9-1-1 max.): 13-18.
BOT CHORD	Rigid ceiling directly applied or 5-2-6 oc bracing.
WEBS	1 Row at midpt 13-44
JOINTS	1 Brace at Jt(s): 54, 55, 57, 58, 59, 60, 63, 66, 67

REACTIONS

(size)	32=29-5-8, 33=29-5-8, 34=29-5-8, 36=29-5-8, 37=29-5-8, 38=29-5-8, 39=29-5-8, 40=29-5-8, 41=29-5-8, 42=29-5-8, 43=29-5-8, 44=29-5-8, 45=29-5-8, 46=29-5-8, 65=29-5-8
Max Horiz	46=185 (LC 17)
Max Uplift	32=653 (LC 13), 33=740 (LC 49), 34=102 (LC 17), 36=99 (LC 49), 37=115 (LC 17), 38=294 (LC 46), 39=54 (LC 12), 40=11 (LC 43), 41=46 (LC 13), 42=34 (LC 12), 43=45 (LC 12), 44=101 (LC 60), 45=24 (LC 16), 46=496 (LC 16), 65=83 (LC 17)
Max Grav	32=1704 (LC 49), 33=287 (LC 13), 34=507 (LC 43), 36=110 (LC 55), 37=515 (LC 43), 38=142 (LC 17), 39=379 (LC 42), 40=46 (LC 7), 41=201 (LC 42), 42=244 (LC 42), 43=324 (LC 42), 44=72 (LC 12), 45=196 (LC 43), 46=2904 (LC 43), 65=376 (LC 48)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/32, 2-3=-858/720, 3-4=-836/742, 4-5=-823/809, 5-6=-875/888, 6-7=-1549/1729, 7-9=-1617/1876, 9-10=-1607/1925, 10-11=-1603/1994, 11-12=-993/1337, 12-13=-926/1318, 13-14=-652/997, 14-15=-651/996, 15-16=-651/996, 16-17=-368/619, 17-18=-369/619, 18-19=-476/811, 19-20=-504/727, 20-21=-595/808, 21-22=-587/727, 22-23=-610/709, 23-24=-599/713, 24-26=-610/680, 26-27=-662/672, 27-28=-1082/1225, 28-29=-1083/1255, 29-30=0/32
BOT CHORD	2-53=-626/852, 52-53=-626/852, 51-52=-626/852, 50-51=-626/852, 49-50=-626/852, 48-49=-626/852, 47-48=-626/852, 46-47=-625/852, 45-46=-1857/1875, 44-45=-1804/1834, 43-44=-1257/1340, 42-43=-1252/1337, 41-42=-1252/1334, 40-41=-1051/1143, 39-40=-1056/1144, 38-39=-1056/1144, 37-38=-655/813, 36-37=-637/804, 35-36=-666/822, 34-35=-1267/1227, 33-34=-1253/1229, 32-33=-1489/1427, 31-32=-1115/1089, 29-31=-1116/1089



April 23, 2024

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Job	Truss	Truss Type	Qty	Ply	
P240393	C5	Piggyback Base Structural Gable	1	1	I65080529
Job Reference (optional)					

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Page: 2

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- WEBS 6-50=-346/357, 11-58=-802/894,
44-58=-846/943, 13-44=-790/576,
22-35=-257/242, 11-46=-1454/1076,
16-41=-383/261, 18-38=-527/279,
38-62=-299/201, 62-63=-291/196,
63-64=-292/197, 22-64=-297/200,
6-61=-1250/1051, 60-61=-1174/985,
59-60=-1182/990, 46-59=-1176/987,
13-57=-291/295, 56-57=-297/303,
41-56=-305/310, 16-55=-403/538,
54-55=-382/508, 38-54=-386/515,
27-32=-327/315, 35-65=-416/614,
65-66=-398/584, 66-67=-398/584,
67-68=-398/584, 27-68=-397/582,
17-54=-339/137, 39-54=-336/140,
40-55=-31/43, 15-56=-202/108,
42-56=-207/117, 14-57=-289/172,
43-57=-292/179, 12-58=-263/122,
45-58=-324/177, 10-59=-80/43,
47-59=-92/46, 9-60=-128/65, 48-60=-112/61,
7-61=-166/102, 49-61=-52/36, 5-51=-140/92,
4-52=-103/60, 3-53=-83/73, 19-62=-62/36,
20-63=-410/182, 37-63=-437/194,
21-64=-81/19, 36-64=-98/36, 23-65=-228/55,
24-66=-30/9, 26-67=-405/146,
34-67=-422/152, 33-68=-16/26,
28-31=-185/109
- 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust)
Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft;
Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope)
exterior zone and C-C Exterior(2E) -0-11-0 to 4-1-0,
Interior (1) 4-1-0 to 18-8-0, Exterior(2R) 18-8-0 to
23-9-6, Interior (1) 23-9-6 to 28-7-6, Exterior(2R) 28-7-6
to 33-7-6, Interior (1) 33-7-6 to 50-6-8 zone; cantilever
left and right exposed ; end vertical left and right
exposed;C-C for members and forces & MWFRS for
reactions shown; Lumber DOL=1.60 plate grip
DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss
only. For studs exposed to wind (normal to the face),
see Standard Industry Gable End Details as applicable,
or consult qualified building designer as per ANSI/TPI 1.
- 4) TCELL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15
Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate
DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9;
Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this
design.
- 6) This truss has been designed for greater of min roof live
load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on
overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 3x4 MT20 unless otherwise indicated.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom
chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members.
- 12) Bearings are assumed to be: Joint 33 SPF No.2
crushing capacity of 425 psi, Joint 32 SPF No.2
crushing capacity of 425 psi.
- 13) Bearing at joint(s) 65 considers parallel to grain value
using ANSI/TPI 1 angle to grain formula. Building
designer should verify capacity of bearing surface.
- 14) Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 496 lb uplift at
joint 46, 294 lb uplift at joint 38, 653 lb uplift at joint 32,
101 lb uplift at joint 44, 46 lb uplift at joint 41, 54 lb uplift
at joint 39, 11 lb uplift at joint 40, 34 lb uplift at joint 42,
45 lb uplift at joint 43, 24 lb uplift at joint 45, 115 lb uplift
at joint 37, 99 lb uplift at joint 36, 83 lb uplift at joint 65,
102 lb uplift at joint 34 and 740 lb uplift at joint 33.

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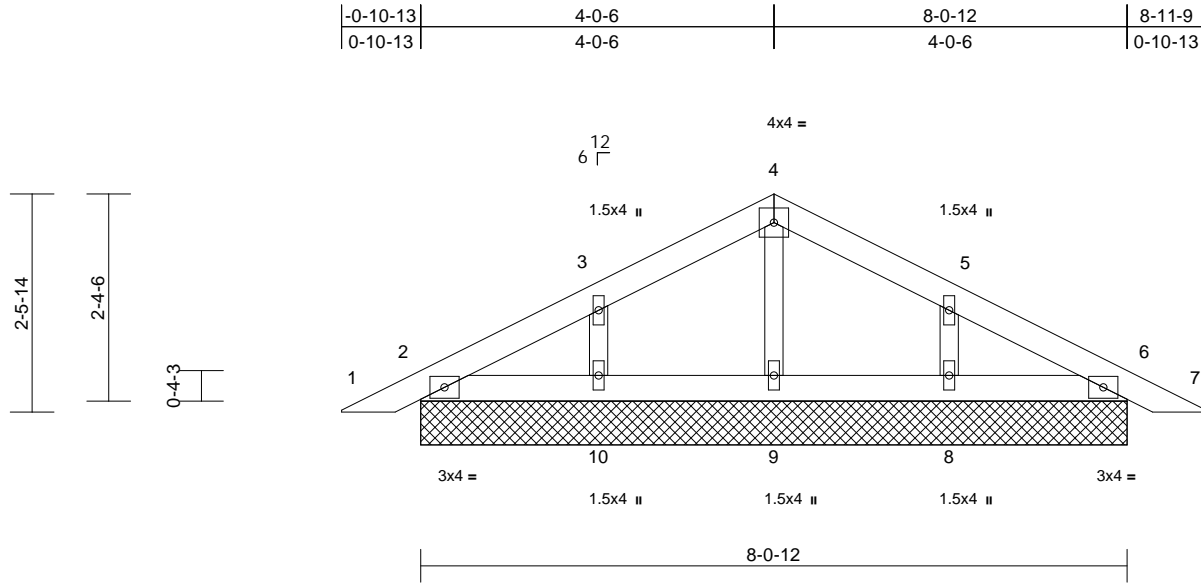
Job	Truss	Truss Type	Qty	Ply	
P240393	PB1	Piggyback	2	1	Job Reference (optional)
I65080530					

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Page: 1

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Scale = 1:26.3

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.10	n/a	-	n/a	999	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.04	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	6	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-P							
BCDL	10.0										
										Weight: 31 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS	(size)	2=8-0-12, 6=8-0-12, 8=8-0-12, 9=8-0-12, 10=8-0-12
	Max Horiz	2=-44 (LC 17)
	Max Uplift	2=-19 (LC 16), 6=-27 (LC 17), 8=-75 (LC 17), 10=-75 (LC 16)
	Max Grav	2=185 (LC 23), 6=185 (LC 24), 8=306 (LC 24), 9=134 (LC 23), 10=306 (LC 23)

FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/29, 2-3=-60/42, 3-4=-76/93, 4-5=-76/99, 5-6=-60/42, 6-7=0/29
BOT CHORD	2-10=-13/49, 9-10=-13/49, 8-9=-13/49, 6-8=-13/49
WEBS	4-9=-97/13, 3-10=-260/213, 5-8=-260/173

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 2, 27 lb uplift at joint 6, 75 lb uplift at joint 10 and 75 lb uplift at joint 8.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard



April 23, 2024

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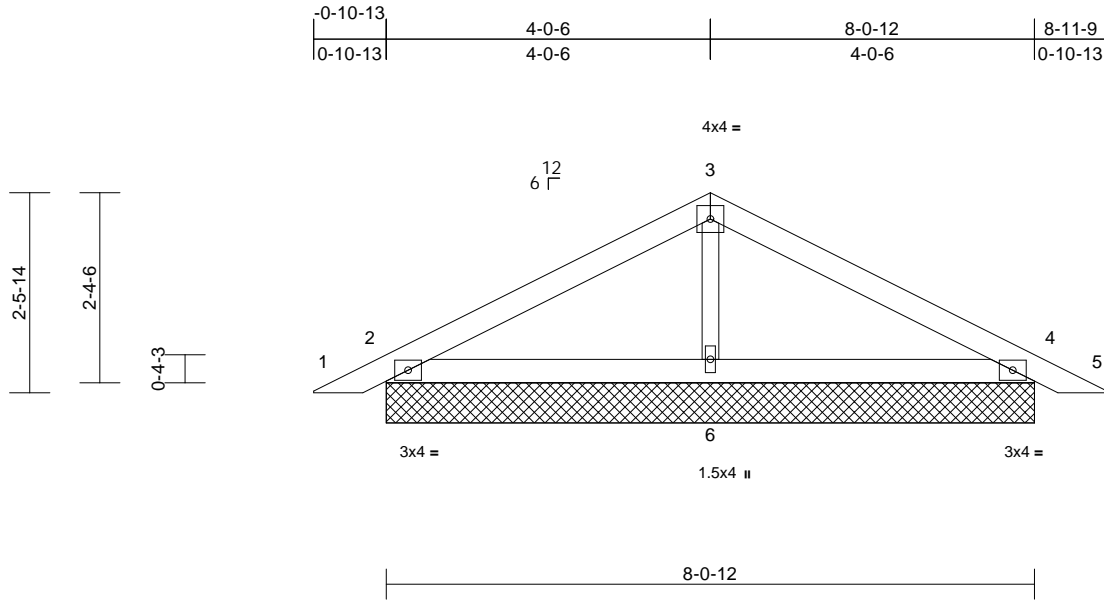
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Job	Truss	Truss Type	Qty	Ply	
P240393	PB2	Piggyback	19	1	Job Reference (optional)
					I65080531

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.54	Vert(LL)	n/a	-	n/a	999	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.16	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-P								
BCDL	10.0										Weight: 30 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x3 SPF No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=8-0-12, 4=8-0-12, 6=8-0-12
Max Horiz 2=44 (LC 20)
Max Uplift 2=-67 (LC 16), 4=-75 (LC 17), 6=-8 (LC 16)
Max Grav 2=340 (LC 23), 4=340 (LC 24), 6=348 (LC 24)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/29, 2-3=-123/76, 3-4=-123/87, 4-5=0/29
BOT CHORD 2-6=0/46, 4-6=0/46
WEBS 3-6=-248/180

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 2, 75 lb uplift at joint 4 and 8 lb uplift at joint 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard



April 23, 2024

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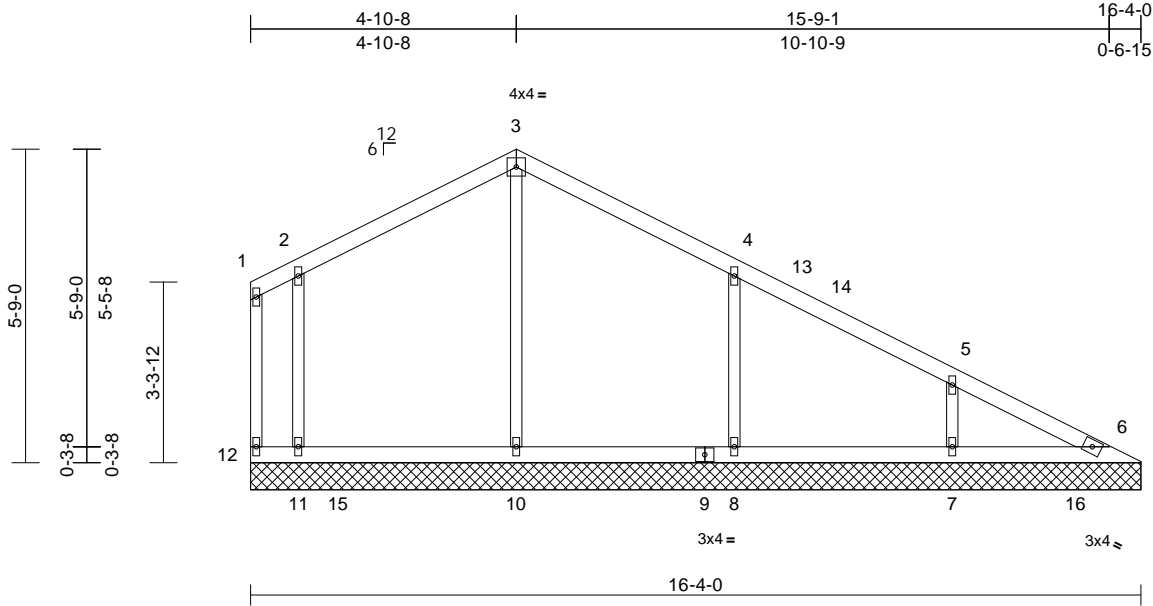
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Chesterfield, MO 63017
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Job	Truss	Truss Type	Qty	Ply	
P240393	V1	Valley	1	1	
Job Reference (optional)					I65080532

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Page: 1



Scale = 1:42.3

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.36	Vert(LL)	n/a	-	n/a	999	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.17	Vert(TL)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.20	Horiz(TL)	0.00	6	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
										Weight: 63 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS	(size)	6=16-4-0, 7=16-4-0, 8=16-4-0, 10=16-4-0, 11=16-4-0, 12=16-4-0
	Max Horiz	12=169 (LC 12)
	Max Uplift	6=6 (LC 13), 7=113 (LC 17), 8=137 (LC 17), 11=131 (LC 16), 12=168 (LC 5)
	Max Grav	6=120 (LC 25), 7=344 (LC 6), 8=543 (LC 23), 10=429 (LC 26), 11=549 (LC 22), 12=49 (LC 16)

FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-12=-93/145, 1-2=-78/102, 2-3=-117/179, 3-4=-118/191, 4-5=-123/96, 5-6=-196/118
BOT CHORD	11-12=-101/196, 10-11=-101/196, 8-10=-101/196, 7-8=-101/196, 6-7=-101/196
WEBS	3-10=-269/126, 2-11=-476/220, 4-8=-463/239, 5-7=-258/188

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-4 to 4-10-8, Exterior(2R) 4-10-8 to 9-10-8, Interior (1) 9-10-8 to 15-8-15 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 12, 6 lb uplift at joint 6, 131 lb uplift at joint 11, 137 lb uplift at joint 8 and 113 lb uplift at joint 7.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024

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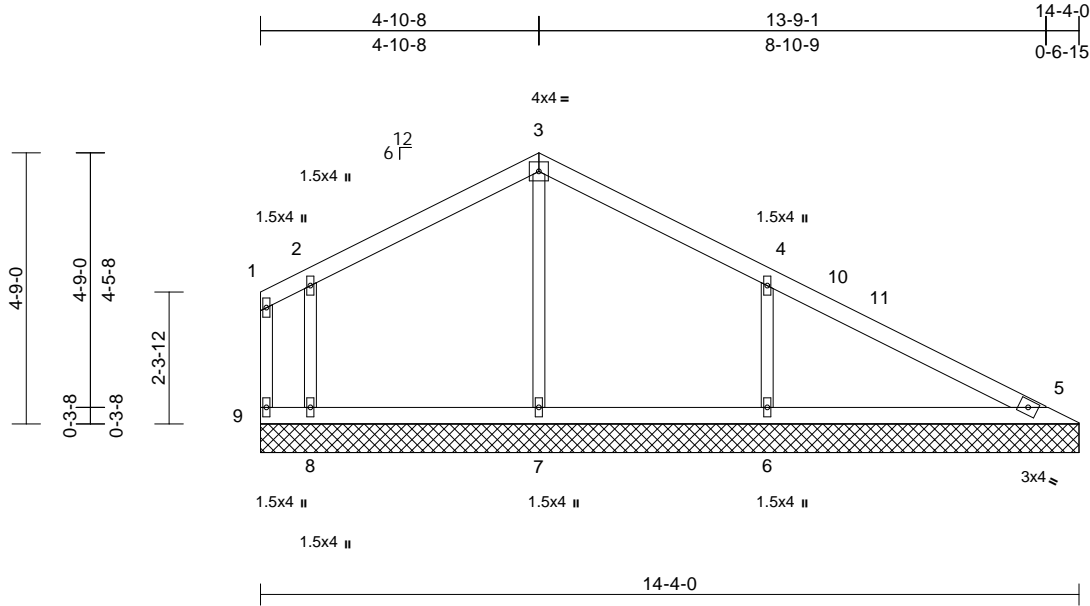
16023 Swingley Ridge Rd.
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314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	
P240393	V2	Valley	1	1	
					I65080533
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:35
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Page: 1



Scale = 1:40.3

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.47	n/a	-	n/a	999	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.19	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.13	Horiz(TL)	0.00	5	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
										Weight: 53 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS	(size)	5=14-4-0, 6=14-4-0, 7=14-4-0, 8=14-4-0, 9=14-4-0
	Max Horiz	9=124 (LC 12)
	Max Uplift	5=-3 (LC 17), 6=-170 (LC 17), 8=-138 (LC 16), 9=-177 (LC 22)
	Max Grav	5=177 (LC 23), 6=643 (LC 23), 7=330 (LC 22), 8=567 (LC 22), 9=52 (LC 16)

FORCES	(lb) - Maximum Compression/Maximum Tension	
	TOP CHORD	1-9=-85/154, 1-2=-61/106, 2-3=-113/161, 3-4=-119/179, 4-5=-115/117
	BOT CHORD	8-9=-70/143, 7-8=-70/143, 6-7=-70/143, 5-6=-70/143
	WEBS	3-7=-258/110, 2-8=-488/269, 4-6=-528/298

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-4 to 4-10-8, Exterior(2R) 4-10-8 to 9-10-8, Interior (1) 9-10-8 to 13-8-15 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 177 lb uplift at joint 9, 3 lb uplift at joint 5, 138 lb uplift at joint 8 and 170 lb uplift at joint 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

MiTek®

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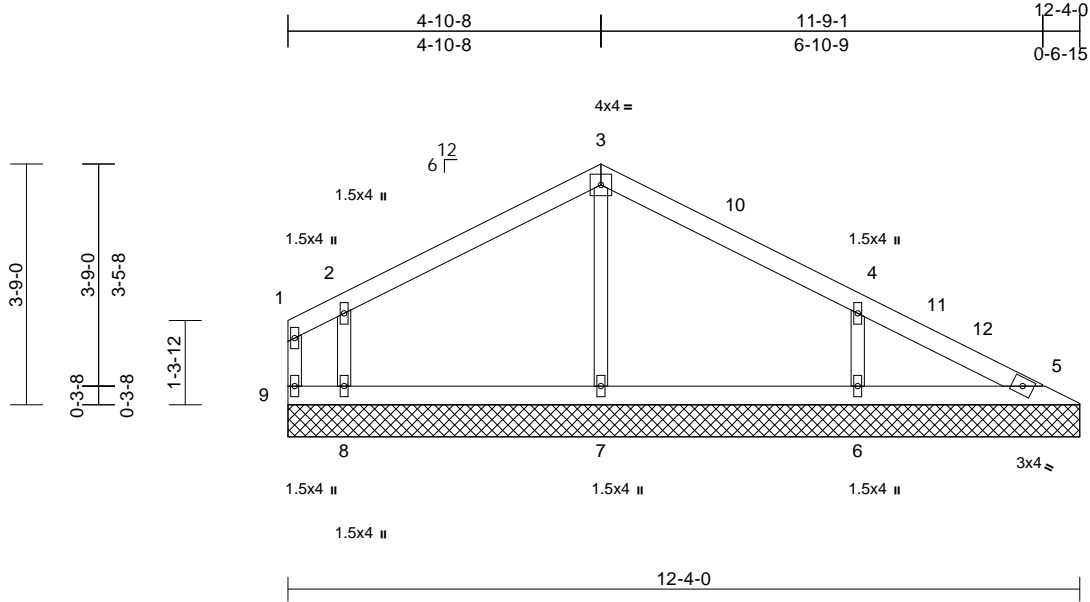
Job	Truss	Truss Type	Qty	Ply	
P240393	V3	Valley	1	1	Job Reference (optional)
					I65080534

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:35

Page: 1

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Scale = 1:35.9

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.35	n/a	-	n/a	999	MT20	197/144
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.11	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horiz(TL)	0.00	5	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0									Weight: 43 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6'-0" oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6'-0" oc bracing.

REACTIONS	(size)	5=12'-4", 6=12'-4", 7=12'-4", 8=12'-4", 9=12'-4"
	Max Horiz	9=-80 (LC 12)
	Max Uplift	5=-10 (LC 16), 6=-131 (LC 17), 8=-150 (LC 16), 9=-175 (LC 22)
	Max Grav	5=91 (LC 23), 6=523 (LC 23), 7=348 (LC 22), 8=563 (LC 22), 9=62 (LC 16)

FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-9=-93/159, 1-2=-44/106, 2-3=-115/136, 3-4=-117/149, 4-5=-72/83
BOT CHORD	8-9=-34/85, 7-8=-34/85, 6-7=-34/85, 5-6=-34/85
WEBS	3-7=-265/116, 2-8=-493/309, 4-6=-445/271

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-4 to 4-10-8, Exterior(2R) 4-10-8 to 9-10-8, Interior (1) 9-10-8 to 11-8-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4'-0" oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0" tall by 2'-0" wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 175 lb uplift at joint 9, 10 lb uplift at joint 5, 150 lb uplift at joint 8 and 131 lb uplift at joint 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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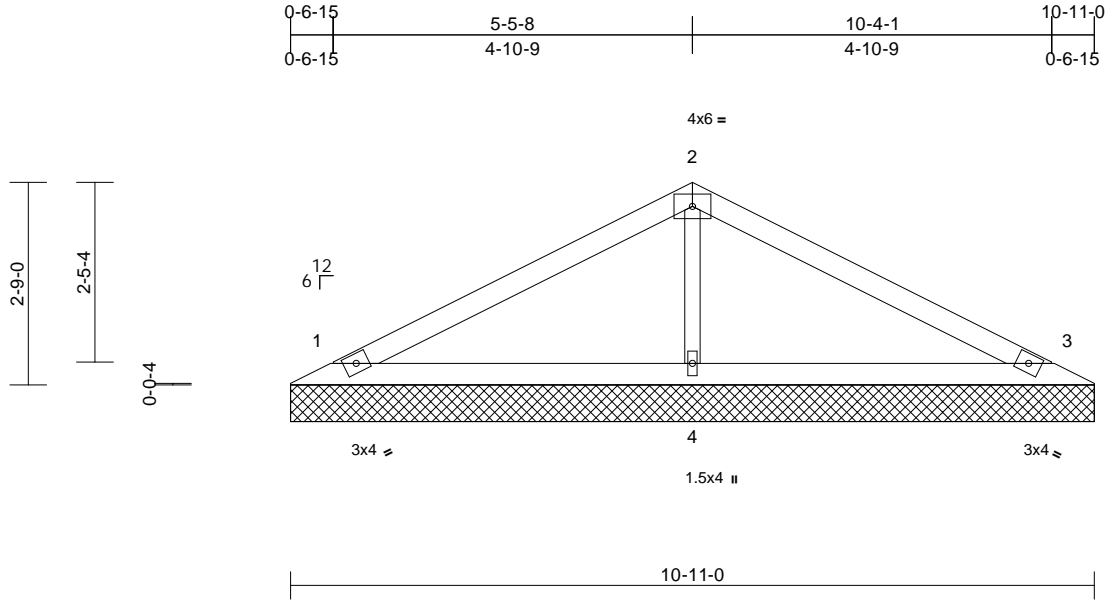
Job	Truss	Truss Type	Qty	Ply	
P240393	V4	Valley	1	1	Job Reference (optional)
					I65080535

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:35

Page: 1

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Scale = 1:31.3

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.62	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.24	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horiz(TL)	0.00	3	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
										Weight: 34 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	1=10-11-0, 3=10-11-0, 4=10-11-0
Max Horiz	1=-46 (LC 21)
Max Uplift	1=-49 (LC 16), 3=-57 (LC 17), 4=-37 (LC 16)
Max Grav	1=304 (LC 22), 3=304 (LC 23), 4=479 (LC 22)

FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=-175/78, 2-3=-175/85
BOT CHORD	1-4=-3/53, 3-4=-3/53
WEBS	2-4=-330/219

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 57 lb uplift at joint 3 and 37 lb uplift at joint 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024

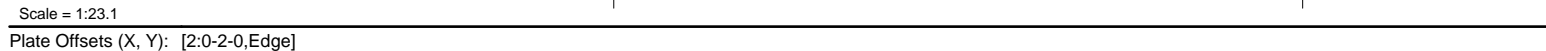
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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LUMBER		6) Gable requires continuous bottom chord bearing.
TOP CHORD	2x4 SP No.2	7) Gable studs spaced at 4-0-0 oc.
BOT CHORD	2x4 SP No.2	8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
BRACING		9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.	10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 1 and 39 lb uplift at joint 3.
REACTIONS		12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
	(size) 1=6-11-0, 3=6-11-0	
	Max Horiz 1=27 (LC 16)	
	Max Uplift 1=-39 (LC 16), 3=-39 (LC 17)	
	Max Grav 1=300 (LC 22), 3=300 (LC 23)	
FORCES		
	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-270/212, 2-3=-270/224	
BOT CHORD	1-3=-144/200	
LOAD CASE(S)		Standard

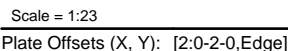
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 1 and 39 lb uplift at joint 3.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

April 23, 2024

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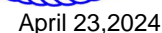


LUMBER		6) Gable requires continuous bottom chord bearing.
TOP CHORD	2x4 SP No.2	7) Gable studs spaced at 4-0-0 oc.
BOT CHORD	2x4 SP No.2	8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
BRACING		9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
TOP CHORD	Structural wood sheathing directly applied or 3-0-0 oc purlins.	10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 1 and 12 lb uplift at joint 3.
REACTIONS	(size) 1=2-11-0, 3=2-11-0	12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
	Max Horiz 1=8 (LC 16)	
	Max Uplift 1=-12 (LC 16), 3=-12 (LC 17)	
	Max Grav 1=82 (LC 22), 3=82 (LC 23)	
FORCES	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-78/70, 2-3=-78/74	
BOT CHORD	1-3=-48/60	
	LOAD CASE(S)	Standard

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust)
Vasd=91mph; TCFL=6.0psf; BCDL=6.0psf; h=35ft;
Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope)
exterior zone and C-C Exterior(2E) zone; cantilever left
and right exposed ; end vertical left and right
exposed;C-C for members and forces & MWFRS for
reactions shown; Lumber DOL=1.60 plate grip
DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss
only. For studs exposed to wind (normal to the face),
see Standard Industry Cable End Details as applicable,
or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15
Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate
DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9;
Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this
design.

- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4'-0" oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0" tall by 2'-0" wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 1 and 12 lb uplift at joint 3.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

WARNING – verify design parameters and noted notes on this and included MiTek Reference Tag M-7473 Rev. 1/2/2023 before use. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria, and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)

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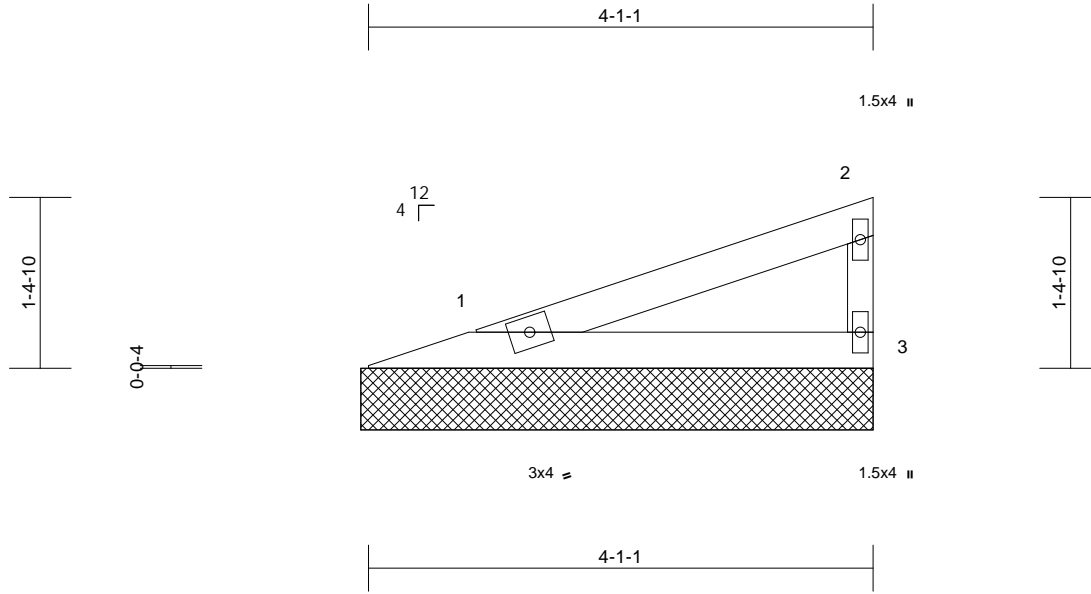
Job	Truss	Truss Type	Qty	Ply	
P240393	V07	Valley	1	1	Job Reference (optional)
					I65080538

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Run: 8.63 S Apr 5 2024 Print: 8.630 S Apr 5 2024 MiTek Industries, Inc. Mon Apr 22 14:48:36

Page: 1

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Scale = 1:18.7

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.28	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.11	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-P							
BCDL	10.0										
										Weight: 12 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-1-13 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size)

1=4-1-13, 3=4-1-13
Max Horiz 1=50 (LC 15)
Max Uplift 1=-27 (LC 12), 3=-34 (LC 16)
Max Grav 1=181 (LC 22), 3=181 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-71/43, 2-3=-150/142
BOT CHORD	1-3=-22/23

NOTES

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 4-0-0 oc.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 1 and 34 lb uplift at joint 3.
 - 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- LOAD CASE(S)** Standard



April 23, 2024

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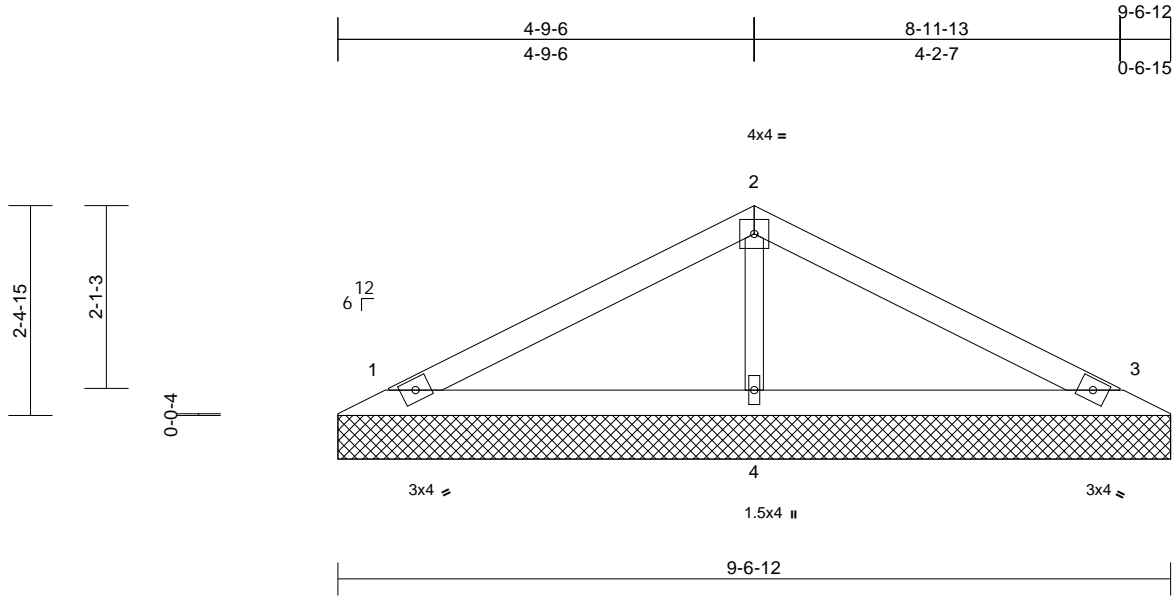
Job	Truss	Truss Type	Qty	Ply	
P240393	V7	Valley	1	1	Job Reference (optional)
I65080539					

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Scale = 1:26.5

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.43	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.18	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horiz(TL)	0.00	3	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-S							
BCDL	10.0										
Weight: 30 lb FT = 20%											

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	1=9-6-12, 3=9-6-12, 4=9-6-12
Max Horiz	1=40 (LC 16)
Max Uplift	1=-42 (LC 16), 3=-49 (LC 17), 4=-32 (LC 16)
Max Grav	1=256 (LC 22), 3=256 (LC 23), 4=406 (LC 23)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-147/69, 2-3=-147/77
BOT CHORD	1-4=-3/46, 3-4=-3/46
WEBS	2-4=-278/200

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1, 49 lb uplift at joint 3 and 32 lb uplift at joint 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



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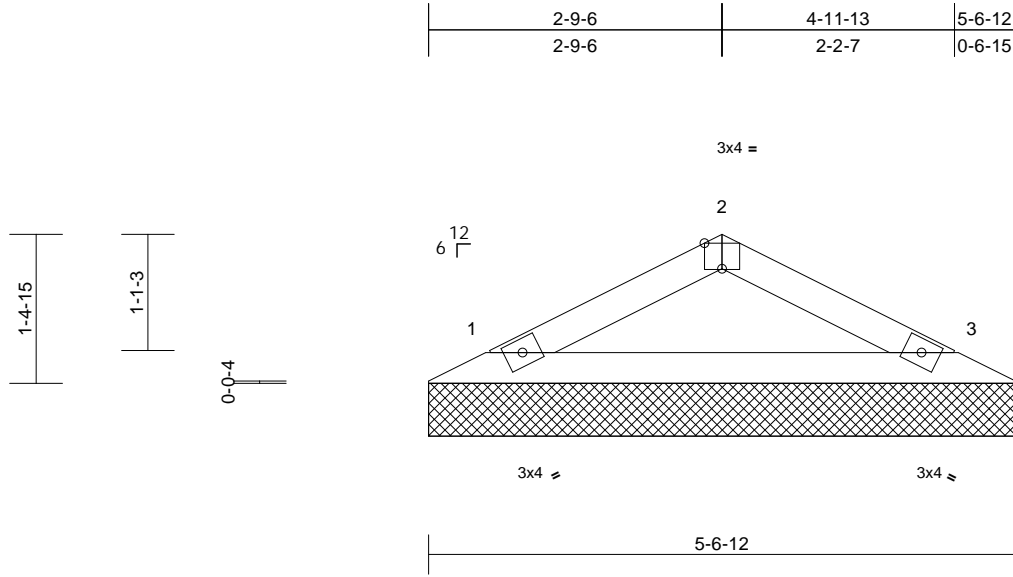
Job	Truss	Truss Type	Qty	Ply	
P240393	V8	Valley	1	1	
					I65080540
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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Scale = 1:21.8

Plate Offsets (X, Y): [2:0-2:0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL	1.15	TC	0.13	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf)	25.0	Lumber DOL	1.15	BC	0.24	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-P							
BCDL	10.0										
										Weight: 16 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-7-12 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 1=5-6-12, 3=5-6-12
Max Horiz 1=21 (LC 16)
Max Uplift 1=30 (LC 16), 3=30 (LC 17)
Max Grav 1=223 (LC 22), 3=223 (LC 23)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-200/174, 2-3=-200/184

BOT CHORD 1-3=-119/153

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 1 and 30 lb uplift at joint 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 23, 2024

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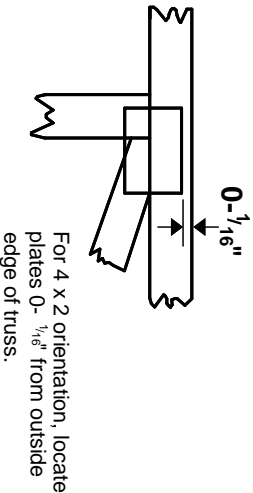
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Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

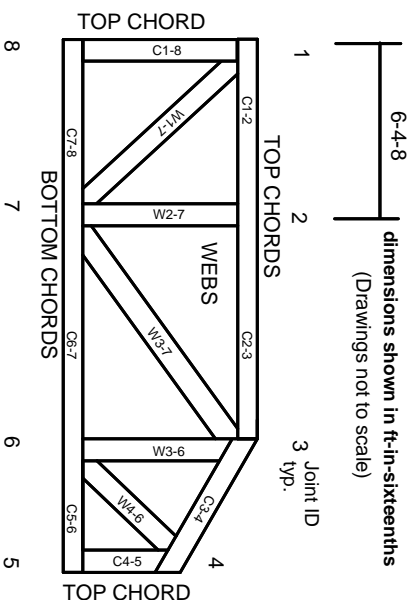


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3. These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.