olsson

May 7, 2024

City of Lee's Summit Development Services 220 SW Green Street Lee's Summit, MO, 64063

RE: Eagle Creek Villas First Plat Olsson Project #A20-24670

To Whom It May Concern:

Eagle Creek Villas First Plat is a proposed 15.53-acre residential development in Lee's Summit, Jackson County, Missouri. The residential development will include 1/5 acre lots, tracts for open space along with the public infrastructure to support these lots. The property is located within the Eagle Creek Development and bounded by Pryor Road to the east, Eagle View Drive to the north, SW Feather Ridge Road to the west, and Eagle Creek Fifth Plat to the south.

The impacts from the development of Eagle Creek Villas First Plat have been previously analyzed with the Eagle Creek Villas Stormwater Drainage Report. The only update to the approved stormwater report was an increase of the spillway length from 58' to 65'. This change is reflected in the plans and has also been revised in the Pond Report which is included in this letter. No other changes to the overall development have been made since the Eagle Creek Villas Stormwater Drainage Report was prepared and approved. The approved Eagle Creek Villas Stormwater Drainage Report has been included with this memo, for reference. Since the overall plan had only a minor change from the previously approved memo, we are requesting approval of this memo to satisfy stormwater drainage requirements for the Eagle Creek Villas First Plat development.

Should you have any questions regarding this submittal, or the plan moving forward, please do not hesitate to reach out to me for discussion at (816) 442-6061, or ssaylor@olsson.com.

Sincerely,

Stephen Saylor, P.E. Olsson Project Engineer



UPDATED POND REPORT

Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

Pond No. 1 - Water Quality Basin

Pond Data

Contours -User-defined contour areas. Average end area method used for volume calculation. Begining Elevation = 953.00 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	953.00	02	0	0
0.99	953.99	02	2	2
1.00	954.00	3,088	15	18
2.00	955.00	4,081	3,585	3,602
3.00	956.00	5,131	4,606	8,208
4.00	957.00	6,237	5,684	13,892
5.00	958.00	7,400	6,819	20,711
6.00	959.00	8,619	8,010	28,720

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 3.00	Inactive	Inactive	Inactive	Crest Len (ft)	= 65.00	Inactive	Inactive	Inactive
Span (in)	= 3.00	0.00	0.00	0.00	Crest El. (ft)	= 959.00	0.00	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 953.00	0.00	0.00	0.00	Weir Type	= Ciplti	Rect		
Length (ft)	= 0.25	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 2.00	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Wet area)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00			

Weir Structures

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Monday, 04 / 29 / 2024

APPROVED PRELIMINARY STORMWATER DRAINAGE REPORT

STORMWATER DRAINAGE REPORT

EAGLE CREEK VILLAS

Prepared for:

Hunt Midwest Real Estate Development, Inc. 8300 NE Underground Drive Kansas City, MO 64161

Prepared by:

Olsson 1301 Burlington Street, Suite 100 North Kansas City, MO 64116



April 2022 Revised May 23, 2022 Olsson Project No. 020-2467



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I. GENERAL INFORMATION

This report is being submitted as a summary of the stormwater drainage design for Eagle Creek Villas, located west of the intersection of SW Pryor Road and SW Eagle View Drive in the City of Lee's Summit, Jackson County, Missouri in the existing Eagle Creek subdivision. This area was previously included with the Eagle Creek Development Plan, prepared in 2001, and was planned for patio homes along with townhomes. With the development plan in 2001, a stormwater study was completed and approved by the city of Lee's Summit, Missouri encompassing this area. The previously approved study determined detention requirements for the development should be waived due to the proximity of the floodplain. This report will review the previously approved 2001 layout compared to the 2022 updated layout and potential impacts to the overall drainage areas. During the pre-applicants call, the city requested treatment for the local 90% mean annual event, or water quality storm event (1.37" for a 24-hour rainfall event), to be considered.

II. EAGLE CREEK VILLAS

A. Site Description

The Eagle Creek Villas project will be constructed on 29.04 acres of the existing Eagle Creek development and includes 96 villa style single family homes on 1/5 acre lots, tracts for open space along with the public infrastructure to support those lots. The Eagle Creek development plan assumed the impervious area of the Eagle Creek Villas area would be 40% and be used for 4-unit townhomes. This proposed layout will be single family homes on 1/5 acre lots with large open space tracts. The impervious area percentage of the proposed villas layout is 35%, a 5% reduction from the approved development plan, thus reducing the runoff for the site. Further analysis of the water quality treatment is detailed below.

B. Water Quality Treatment

Per the approved storm study, detention requirements were waived; however, since the development plan was approved, water quality requirements have been adopted by the city. The city has requested with this updated development plan, consideration of a water quality basin be reviewed. In review of the layout, a water quality basin is being proposed south of SW Eagle View Drive, on the western end of the site. This basin will treat stormwater prior to discharging into the existing stream. The water quality volume required is 27,028 cubic feet. The proposed water quality basin volume shown is 28,703 cubic feet and has 14.89 acres tributary to it. The water quality volume will be held in the pond for 40 hours past the peak time. The release rate from the pond will be controlled by a 3" x 3" square orifice cut into a steel plate on the outlet control structure.

For the area north of SW Eagle View Drive, no water quality basin is proposed. The impervious area has decreased for this area due to the planned use changing from townhomes to single family villa lots, which will improve stormwater runoff. Along with the decrease of impervious area, this area has limited space and is restricted by the existing development west of the site. Placement of a water quality basin would ideally be in the western open space tract, which is adjacent to existing townhomes. The drainage areas planned for the existing storm sewer would increase if we try to get the proposed roadway area to a water quality basin located in the open space tract. The basin would discharge to an existing field inlet west of the tract causing capacity concerns in the existing storm line the basin would drain to. Taking into account the existing storm sewer impact and the basin overflow from larger rain events being contracted at the existing townhome yards, placing a basin in the tract would have a negative impact on the existing development.

Since the impervious area has decreased from the previously approved layout, the proposed layout is in compliance with the previously approved storm drainage study. We are recommending no water quality treatment for the north area, to avoid a potential negative impact on existing homeowners.

III. CONCLUSIONS AND RECOMMENDATIONS

The proposed site layout reduces the impervious area when compared to the Eagle Creek development plan and will a positive impact on drainage in the area. Due to the approved storm study for the Eagle Creek development waiving detention requirements for the site, only the water quality storm event will be detained prior to releasing into the existing creek. Based on the information provided, Olsson requests approval of this stormwater drainage report for the proposed development of Eagle Creek Villas.

APPENDIX A Exhibits and Calculations

National Flood Hazard Layer FIRMette



Legend



250

1,000

1,500

Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020



Natural Resources Conservation Service

USDA





Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
10000	Arisburg silt loam, 1 to 5 percent slopes	С	21.9	10.6%
10024	Greenton-Urban land complex, 5 to 9 percent slopes	D	6.6	3.2%
10082	Arisburg-Urban land complex, 1 to 5 percent slopes	С	15.9	7.7%
10117	Sampsel silty clay loam, 5 to 9 percent slopes	C/D	34.9	16.8%
10120	Sharpsburg silt loam, 2 to 5 percent slopes	С	38.8	18.7%
10128	Sharpsburg-Urban land complex, 2 to 5 percent slopes	D	33.5	16.1%
10181	Udarents-Urban land- Sampsel complex, 5 to 9 percent slopes	с	33.6	16.2%
30080	Greenton silty clay loam, 5 to 9 percent slopes	C/D	22.3	10.8%
Totals for Area of Inter	rest	L	207.3	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

Worksheet: Water Quality Sizing

Eagle Creek Villas Project : Location: Lee's Summit, Missouri By: BMW Date: 4/22/2022 Checked: Date:

Water Quality Volume (WQv)

WQv= P x Rv x A

Water Quality Basin South Tributary Drainage Area to Pond (acres) = 14.89 Impervious Area (acres) = 5.21 Rv = 0.05 + 0.009(i)i = Percent Impervious P (in)= 1.37 i (%) = 35 0.365 Rv = WQv (ac-ft) = 0.62 WQv (cu-ft) = 27028

Basin Volume							
	ELEVATION (FT)	AREA (SF)	(A1+A2)/2	VOL SUM (CF)			
	954	3,088.00	0.00	0.00			
	955	4,081.00	3,584.50	3,584.50			
Basin	956	5,131.00	4,606.00	8,190.50			
Basili	957	6,237.00	5,684.00	13,874.50			
	958	7,400.00	6,818.50	20,693.00			
	959	8,619.00	8,009.50	28,702.50			

Multi-Hydrograph Plot

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 1

Water Quality Basin

Hydrograph type	= SCS Runoff
Peak discharge	= 9.291 cfs
Time to peak	= 11.97 hrs
Hyd. Volume	= 18,843 cuft

Hyd. No. 1

Water Quality Basin

Hydrograph type	= SCS Runoff
Peak discharge	= 9.29 cfs
Time to peak	= 11.97 hrs
Hyd. Volume	= 18,843 cuft



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 04 / 20 / 2022

Hyd. No. 2

Water Quality Basin

Reservoir	Peak discharge	= 0.341 cfs
= 1 yrs	Time to peak	= 14.53 hrs
2 min	Hyd. volume	= 18,800 cuft
1 - Water Quality Basin	Max. Elevation	= 956.20 ft
 Water Quality Basin 	Max. Storage	= 9,337 cuft
	1 yrs 2 min 1 - Water Quality Basin	1 yrsTime to peak2 minHyd. volume1 - Water Quality BasinMax. Elevation

Storage Indication method used.



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 04 / 20 / 2022

Hyd. No. 2

Water Quality Basin

Peak discharge = 0.341 cfs
Time to peak = 14.53 hrs
Hyd. volume = 18,800 cuft
in Max. Elevation = 956.20 ft
Max. Storage = 9,337 cuft

Storage Indication method used.



Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Pond No. 1 - Water Quality Basin

Pond Data

Contours -User-defined contour areas. Average end area method used for volume calculation. Begining Elevation = 954.00 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	954.00	3,088	0	0
1.00	955.00	4,081	3,585	3,585
2.00	956.00	5,131	4,606	8,191
3.00	957.00	6,237	5,684	13,875
4.00	958.00	7,400	6,819	20,693
5.00	959.00	8,619	8,010	28,703

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 3.00	Inactive	Inactive	Inactive	Crest Len (ft)	= 58.00	Inactive	Inactive	Inactive
Span (in)	= 3.00	0.00	0.00	0.00	Crest El. (ft)	= 959.00	0.00	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 954.00	0.00	0.00	0.00	Weir Type	= Rect	Rect		
Length (ft)	= 0.25	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 2.00	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Wet area)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Weir Structures

Stage /

EAGLE CREEK VILLAS

Lee's Summit, MO

April 2022

Olsson Project No. 020-2467