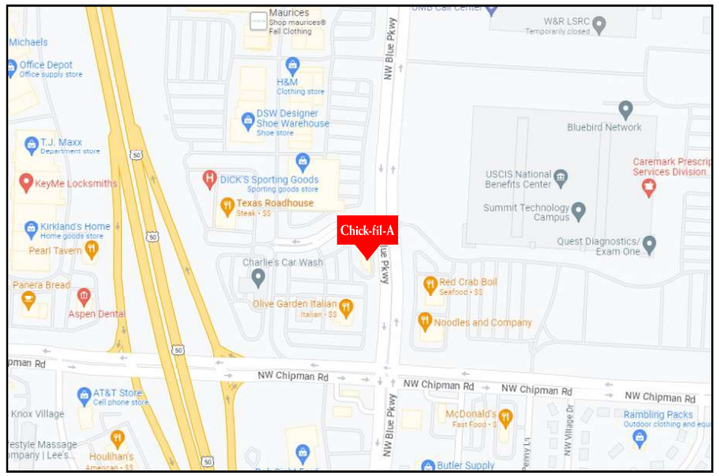

VICINITY MAP

N.T.S.

AERIAL VIEW

N.T.S.

NORTH

KRISTEN HAMILTON - 678-725-8852
KHamilton@chandlersigns.com
TERRI BROWN - 682-204-6687
TBrown@chandlersigns.com

DocuSigned by:
Michael Young
0DCEC3135B2844E...

Design #	**0637033Ar3**
Sheet	1 of 27
Client	#2859
Address	690 NW Blue Pkwy, Lees Summit, MO
Account Rep.	KRISTEN HAMILTON / TERRI BROWN
Designer	LEAH LANSFORD
Date	9/29/23

Approval / Date

Client	
Sales	
Estimating	
Art	
Engineering	
Landlord	

Revision/Date

R1(12-22-23)LL: CHANGED SIZE OF PANELS ON THE STOPDNE

R2(1/16/24)LL: ADDED STAMPED ENGINEERING

CORRECTION(01/17/2024)AM: REVISED DETAIL SECTION AND ADDED THE WORD G.C. IN FRONT OF REMOVALS FOR SIGNS B1-2, C, D, G1-2 AND H1-4

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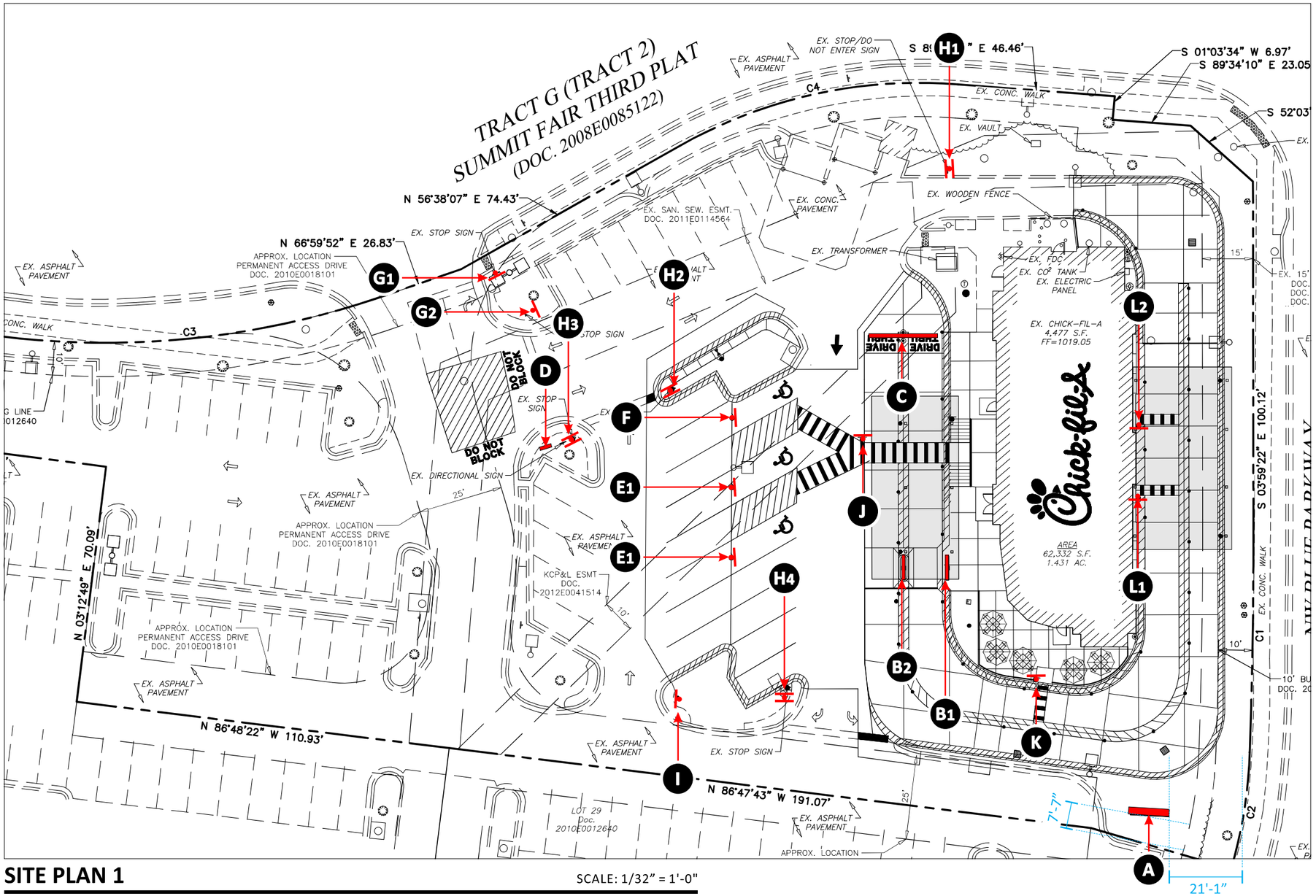
FINAL ELECTRICAL CONNECTION BY CUSTOMER

UL THIS SIGN IS INTENDED TO BE INSTALLED IN ACCORDANCE WITH ARTICLE 600 OF THE NATIONAL ELECTRICAL CODE AND/OR OTHER APPLICABLE LOCAL CODES. THIS INCLUDES PROPER GROUNDING & BONDING OF THE SIGN. SIGN WILL BEAR UL LABEL(S).

Chick-fil-A

AERIAL VIEW



SITE PLAN 1

SCALE: 1/32" = 1'-0"

SIGN I-D LETTER	FACE A	FACE B
A	3 LINES OF 4" ZIP CHANGE COPY Closed Sunday	3 LINES OF 4" ZIP CHANGE COPY Closed Sunday
B1 B2		N/A
C		
D	Enter	Enter
E1 E2	RESERVED PARKING VAN ACCESSIBLE WITH BOLLARD	N/A
F	RESERVED PARKING VAN ACCESSIBLE WITH BOLLARD	N/A
G1 G2	STOP	N/A
H1-4	STOP	DO NOT ENTER
I	One Way ←	N/A

PORTABLE BASES

SIGN I-D LETTER	FACE A	FACE B
J	PEDESTRIAN CROSSING	N/A
K	WATCH FOR VEHICLES	N/A
L1 L2	CAUTION TEAM MEMBER CROSSING	N/A

Design # 0637033Ar3

Sheet 2 of 27

Client #2859

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Account Rep. KRISTEN HAMILTON, TERRI BROWN

Designer LEAH LANSFORD

Date 9/29/23

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SITE PLAN/KEY

A D/F MONUMENT

SCALE: 1/2" = 1'-0"

ONE (1) REMOVAL REQUIRED - MANUFACTURE AND INSTALL — 60 SQ. FT.

OPPOSITE VIEW

SPECS:

FABRICATED ALUM. FRAME WITH ALUMINUM CLADDING AND ALUM. FIN / FILLER PAINTED MATTHEWS #74155 DARK BRONZE, SEMI-GLOSS FINISH.

MAIN ID LOGO FACES

CABINET AND RETAINERS PAINTED MATTHEWS #74155 DARK BRONZE, SEMI-GLOSS. WHITE PLEX FACES WITH 3M #3630-53 CARDINAL RED TRANSLUCENT VINYL APPLIED 1ST SURFACE. INTERNALLY ILLUMINATED WITH 7100K WHITE LEDS AS REQUIRED.

READER BOARD DISPLAY

.080" ALUM. FACE PAN PANEL WITH ROUTED OUT OPENING FOR READER BOARD AND R.O.S.T. COPY READING "CLOSED SUNDAY", WITH HINGED LOCKABLE F-TRACK VANDAL COVER PAINTED MATTHEWS #74155 DARK BRONZE, SEMI-GLOSS. CLEAR LEXAN FACE, SECURED TO FACE OF ALUM. PANEL. BACKED WITH 3/16" WHITE PLEX. TRACK TO ACCOMMODATE THREE (3) LINES OF CHANGEABLE 6" COPY BLACK COMMERCIAL SET OF 334 LETTERS. INCLUDE CHANGER ARM.

INTERNALLY ILLUMINATED SAME AS ABOVE.

FOUNDATION SYSTEM

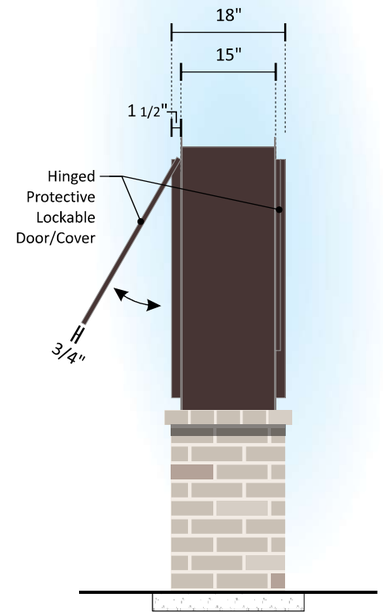
ONE (1) 4 ½" O.D. x .237" WALL STEEL SUPPORT ONE (1) 24" x 5'-0" DEEP CONCRETE PIER.

FOOTING AND 1" x 1" x .125" ALUM. FABRICATED BASE FRAME WITH 1/2" CEMENT BOARD.

MASONRY / BRICK: BY G.C.

SHALL MATCH THE BUILDING MATERIAL AND MORTAR FINISH.

PRIMARY ELECTRIC RUN THRU NEC APPROVED WEATHER-PROOF CONDUIT - COORDINATE PRIOR TO CONCRETE FOUNDATION / PAD POUR.



END VIEW

EXISTING CONDITIONS

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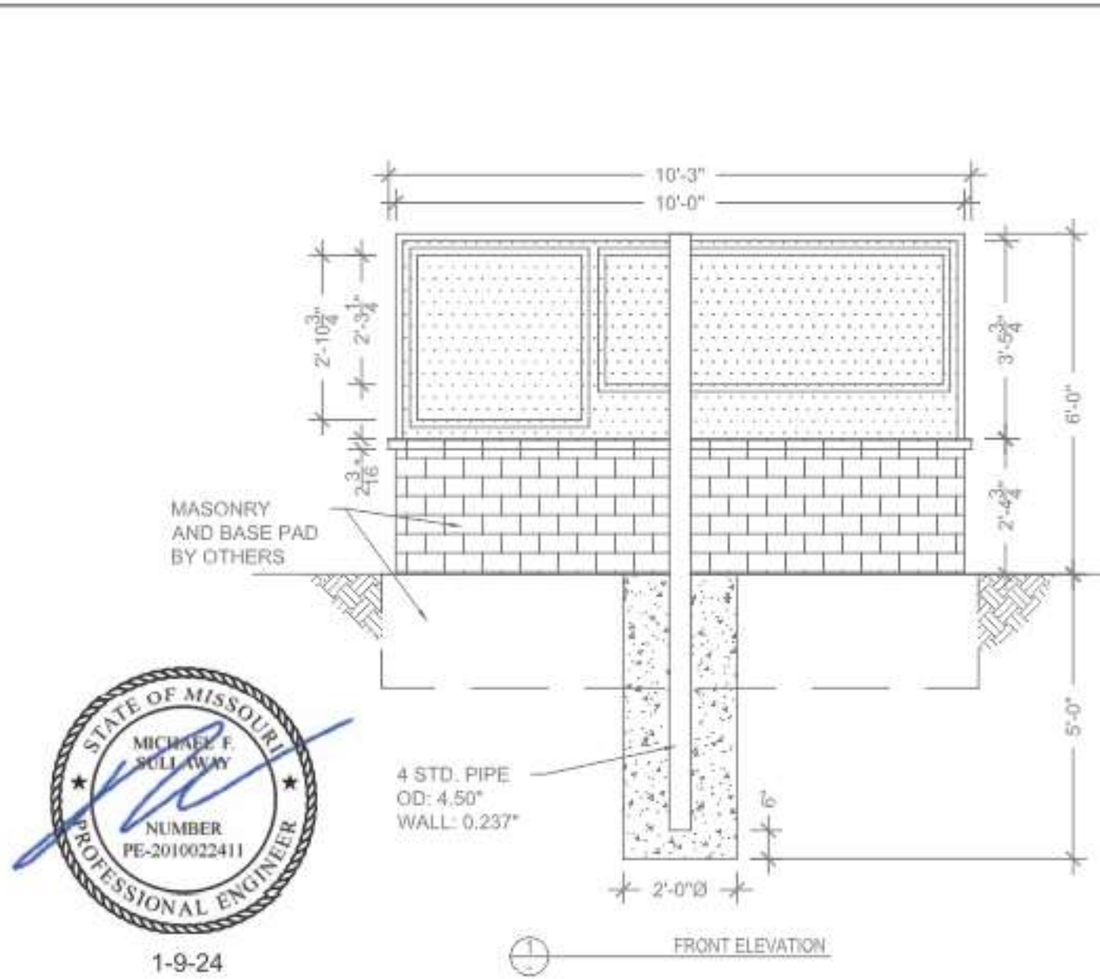
Chick-fil-A

A

SULLAWAY ENGINEERING
11545 W. BERNARDO COURT
SUITE 201, SAN DIEGO, CA 92127
PROJECTMANAGER@SULLAWAYENG.COM
PHONE: 1-858-312-5150 FAX: 1-858-777-3534

PROJECT: CHICK-FIL-A - MONUMENT SIGN - 690 NW BLUE PKWY, LEES SUMMIT, MO
PROJECT #: 43787A
CLIENT: CHANDLER SIGNS

DATE: 1-9-2024
ENGINEER: JD
LAST REVISED:



STATE OF MISSOURI
MICHAEL F. SULLAWAY
NUMBER PE-2010022411
PROFESSIONAL ENGINEER

1-9-24

GENERAL NOTES
1. DESIGN CODE: IBC 2018
2. DESIGN LOADS: ASCE 7-16
3. WIND VELOCITY: 110 MPH EXPOSURE C
4. CONCRETE 2500 PSI MIN.
5. PIPE STEEL ASTM A53, Fy= 35 KSI MIN.
6. PROVIDE PROTECTION AGAINST DISSIMILAR METALS USING ANTI-CORROSIVE PAINT OR NEOPRENE GASKETS.
7. LATERAL SOIL BEARING PER IBC CLASS 4 (150 PSF/FT)
8. ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION.

SULLAWAY ENGINEERING
11545 W. Bernardo Court, San Diego, CA 92127
projectmanager@sullawayeng.com
Phone: 858-312-5150 Fax: 858-777-3534

PROJECT: CHICK-FIL-A 2859
PROJ. NO.: 43787A
CLIENT: CHANDLER SIGNS

DATE: 1/9/24
ENGINEER: JD

units; pounds, feet unless noted otherwise

Applied Wind Loads; from ASCE 7-16

$F=q_z*G*C_f*A_f$ with $q_z = 0.00256K_zK_{zt}K_dV^2$ (29.3.2 & 29.4)

C_f= 1.417 (Fig. 29.3-1) max. height= 6.00
K_{zt}= 1.0 (26.8.2) (=1.0 unless unusual landscape) s= 6.00
K_z= from table 28.3-1 Exposure= c
K_d= 0.85 for signs (table 26.6-1)
V= 110 mph
G= 0.85 (26.9) weight= 0.600 kips
s/h= 1.000 M_{DL}= 0.00 k-ft
B/s= 1.67

Pole Loads	structure component	height at section c.g.	K_z	q_z	pressure q_z*G*C_f	A_f	shear	Wind Moment M_w	
	1	1.1	0.850	22.4	26.95	22.1	597	660	
	2	2.3046875	0.850	22.4	26.95	1.9	50	116	
	3	4.19791667	0.850	22.4	26.95	36.0	971	4077	
					sums:	60.0	1618	4.85	(M_w) k-ft amn= 3.0
	for s/h=1, add 10% (asce fig. 29.4-1):				x 1.10			5.34	

P_u= 0.72 kip M= 5.34 k-ft $M=sqrt(M_{DL}^2+M_w^2)$
$M_u=sqrt(1.2M_{DL}^2+1.0M_w^2)$ = 5.34 k-ft

Pole Design section; pipe

$M_u \le \phi M_n$ with $M_n = f_yZ$ f_y= 35 ksi = 0.9

H	M_u(k-ft)	Z req'd. (in)	Size(in)	t (in)	Z	USE
at grade	5.3	2.03	3	0.216	2.2	**4 STD. PIPE, Mn= 10.6 k-ft**

Footing Design footprint: round

ω= 1.3 IBC 1606.3.2 IBC Table 1806.2, sections 1806.3.4, 1807.3.2 S=(1.3x2x)
P= 1.26 kip S1 = S x d / 3 A = 2.34 x P / (S1 x b) S= 400
S1= 586 d =0.5xA (1+ (1+4.36x h/A) ^.5) IBC 1807.3.2.1
A= 2.52

footing: **2' - 0" dia.** **4' - 5" deep**
USE 5' DEEP

Design # 0637033Ar3
Sheet 4 of 27
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11545 W. Bernardo Court, San Diego, CA 92127
projectmanager@sullawayeng.com
Phone: 858-312-5150 Fax: 858-777-3534

PROJECT: CHICK-FIL-A 2859 DATE: 1/9/24
PROJ. NO.: 43787A ENGINEER: JD
CLIENT: CHANDLER SIGNS

V5.5 units; pounds, feet unless noted otherwise

Check Buckling for Round HSS Section

Pole Design-AISC section; pipe<=12"

weight= 0.6 kips

F_y= 35 ksi = 0.9 E= 29,000 ksi

H	M_u(k-ft)		Size(in)	t (in)	Z	S			
at grade	5.34	2.03	4.5	0.220	4	3	spec wt=	0.289	kcf
				r=	1.52		signage wt;	0.600	k
				A=	2.96		pipe weight	0.065	k
D/t=	20.4			h (L) =	6		P=	0.665	k
KL/r=	95.0						P_r=	0.80	k
K=	2		F_{cr}=	22.39	for KL/r<sqrt(E/f_y)				AISC Chap. E3
Fe=	32.8	ksi (E3-4)	F_{cr}=	28.74	for KL/r>sqrt(E/f_y)				
4.71sqrt(E/f_y)=	137.9		use F_{cr}=	22.39					
for D/t < 0.07 E/Fy section is compact			0.07 E/F_y=	58					
for D/t < 0.31 E/Fy section is non-compact			0.31 E/F_y=	257					
Section is **compact**									
P_n=$F_{cr}A_g$=	66.3	k	P_c=ϕPn=	59.7	k				
M_n=(0.021E/(D/t) + F_y)S=	16.6	k (non-compact)							(F8-2)
M_n=F_yZ=	11.8	k-ft (compact)							
M_n=F_{cr}S=	5.6	k-ft (slender - slender sections NOT USED)							
use M_n=	11.8	k-ft	$P_u/\phi P_n$ =	0.01337					
M_c=ϕM_n=	10.6	k-ft	$M_u/\phi M_n$ =	0.50341					
P_r/P_c=	0.0134								AISC Chap. H1
$P_r/2P_c$=	0.0067								
P_r/P_c + 8/9 * M_r/M_c =	0.461								
For P_r/P_c<0.2; $P_r/2P_c$ + M_r/M_c =	0.510								
use	**0.51**								

Round Pipe Torsion Capacity

Pipe	D=	4.5	in						
	t=	0.22041	in	E=	29	x10e6	psi		
	L=	40	ft						
	J=	14							
	C=	6	in3			C=2J/d		J=pie/32(d^4-(d-2t)^4)	
T_n=F_{cr}C				ϕ=	0.9			AISC H3	
F_{cr}=1.23E / (sqrt(L/D)(D/t)^1.25)				F_{cr} =	79.6	ksi	max;	F_{cr} =	188.6 ksi
F_{cr}=0.6E / ((D/t)^1.5)				F_{cr} =	188.6	ksi			
F_{cr} < 0.6F_y				0.6F_y =	21	ksi		F_{cr} =	21.0 ksi
Tu=	**3.2**	k-ft							
ϕT_n=	114	k-in	10	k-ft			($T_u/\phi T_n$)^2=	0.115	
$P_u/\phi P_n$ +$M_u/\phi M_n$ +($T_u/\phi T_n$)^2=	0.63								ok

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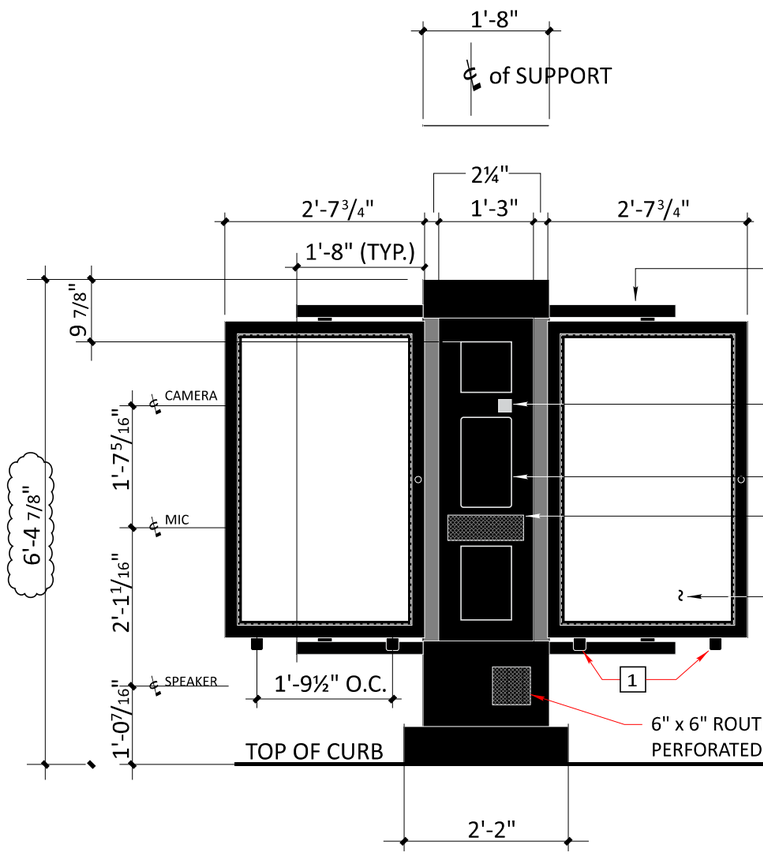
Chick-fil-A

A

DAL - MANUFACTURING
C7 GENERATION 2



ENTIRE DISPLAY TO BE POWDER COATED QPC P-820 MATTE BLACK FINISH.

PAINT ALL HARDWARE TO MATCH MATTE BLACK.

2" x 3" STEEL MENU BRACKET with PIVOT AT TOP and BOTTOM - SEE SECTIONS.

2" x 2" ROUTED CAMERA OPENING BACKED with .125" THK. MAKROLON AR BRONZE K09 ACRYLIC (CAMERA by OTHERS).

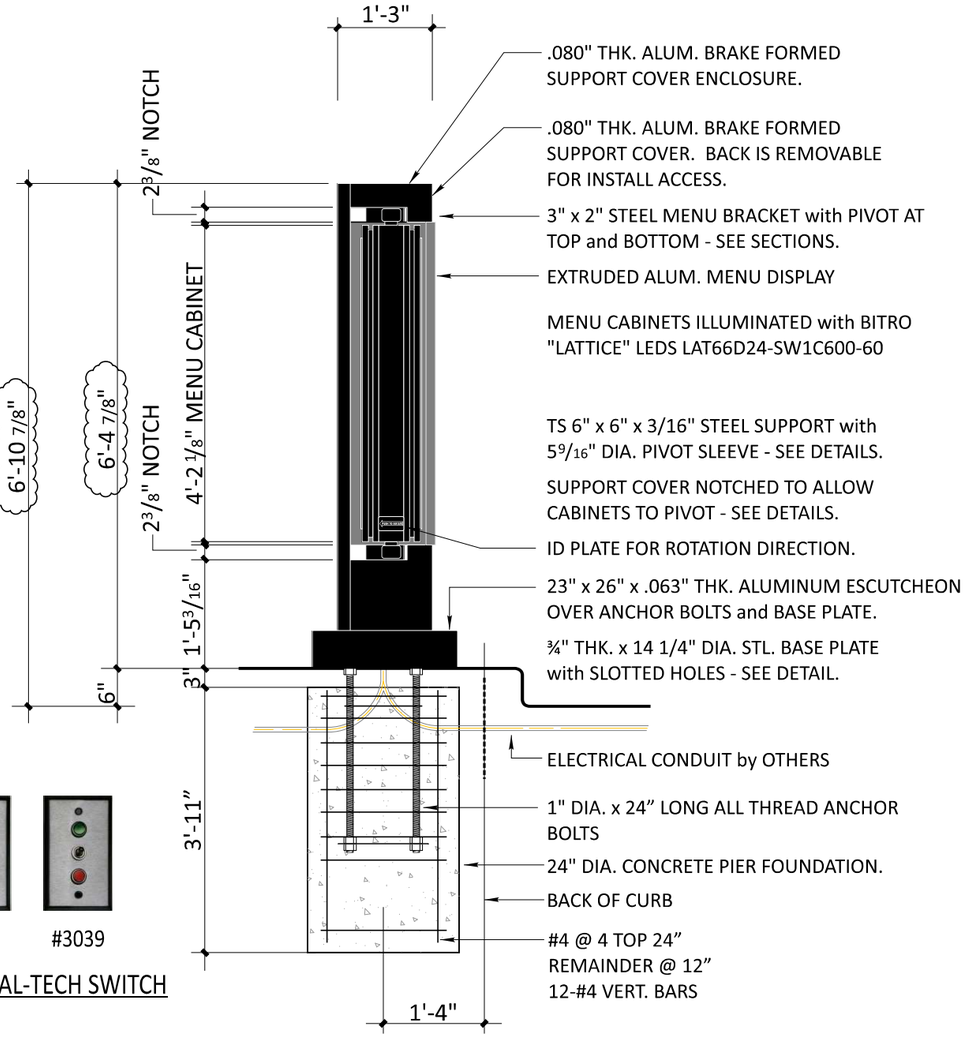
P.O.P. PANEL FRAME MOUNTS TO FACE OF SUPPORT COVER

4" x 12" ROUTED MIC OPENING BACKED with .063" THK. PERFORATED ALUMINUM PANEL WITH 1/4" HOLES ON 3/8" CENTERS. (MIC by OTHERS).

.187" THK. ACRYLITE P99 MATTE ACRYLIC COVER OVER MENU PANELS (27" x 45 ½" MENU INSERTS).

V.O.	MBT
25.5" x 44"	27" x 45.5"

6" x 6" ROUTED OPENING BACKED with .063" THK. PERFORATED ALUMINUM (SPEAKER by OTHERS).



.080" THK. ALUM. BRAKE FORMED SUPPORT COVER ENCLOSURE.

.080" THK. ALUM. BRAKE FORMED SUPPORT COVER. BACK IS REMOVABLE FOR INSTALL ACCESS.

3" x 2" STEEL MENU BRACKET with PIVOT AT TOP and BOTTOM - SEE SECTIONS.

EXTRUDED ALUM. MENU DISPLAY

MENU CABINETS ILLUMINATED with BITRO "LATTICE" LEDS LAT66D24-SW1C600-60

TS 6" x 6" x 3/16" STEEL SUPPORT with 5 9/16" DIA. PIVOT SLEEVE - SEE DETAILS.

SUPPORT COVER NOTCHED TO ALLOW CABINETS TO PIVOT - SEE DETAILS.

ID PLATE FOR ROTATION DIRECTION.

23" x 26" x .063" THK. ALUMINUM ESCUTCHEON OVER ANCHOR BOLTS and BASE PLATE.

¾" THK. x 14 1/4" DIA. STL. BASE PLATE with SLOTTED HOLES - SEE DETAIL.

ELECTRICAL CONDUIT by OTHERS

1" DIA. x 24" LONG ALL THREAD ANCHOR BOLTS

24" DIA. CONCRETE PIER FOUNDATION.

BACK OF CURB

#4 @ 4 TOP 24"
REMAINDER @ 12"
12-#4 VERT. BARS

END VIEW

SCALE: ½" = 1'-0"

B1 **B2**

CFA - C7 GEN 2 ORDERING STATION W/O CANOPY

SCALE: 1/2" = 1'-0"

TWO (2) REMOVALS REQUIRED - MANUFACTURE & INSTALL

G.C. TO REMOVE AND DISCARD EXISTING C7'S.

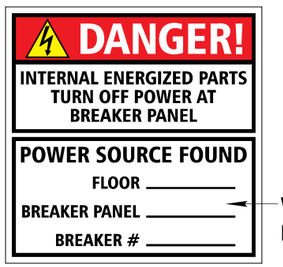
MANUFACTURE AND INSTALL TWO NEW C7'S IN NEW LOCATION.

G.C. TO CONFIRM ELECTRICAL.

#1992 #3039

INTERIOR SIGNAL-TECH SWITCH

2014/2017 NEC COMPLIANT LABEL



WARNING LABEL/ BREAKER LOCATION

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CORRECTION(01/17/2024)AM: REVISED DETAIL SECTION AND ADDED THE WORD G.C. IN FRONT OF REMOVALS FOR SIGNS B1-2, C, D, G1-2 AND H1-4

R3(02/26/2024)AM: UPDATED SP, ADDED NOTE FOR SIGN A FOR CSI TO PROVIDE NEW STEEL AND FOUNDATION AND ADDED NOT FOR SIGN D FOR CSI TO REMOVE AND DISCARD EXISTING D/F DIRECTIONAL

CHANDLER SIGNS

National Headquarters	14201 Sovereign Road #101 Fort Worth, TX 76155 (214)902-2000 Fax(214)902-2044
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South Texas	PO BOX 125 206 Doral Drive Portland, TX 78374 (361) 563-5599 Fax (361) 643-6533

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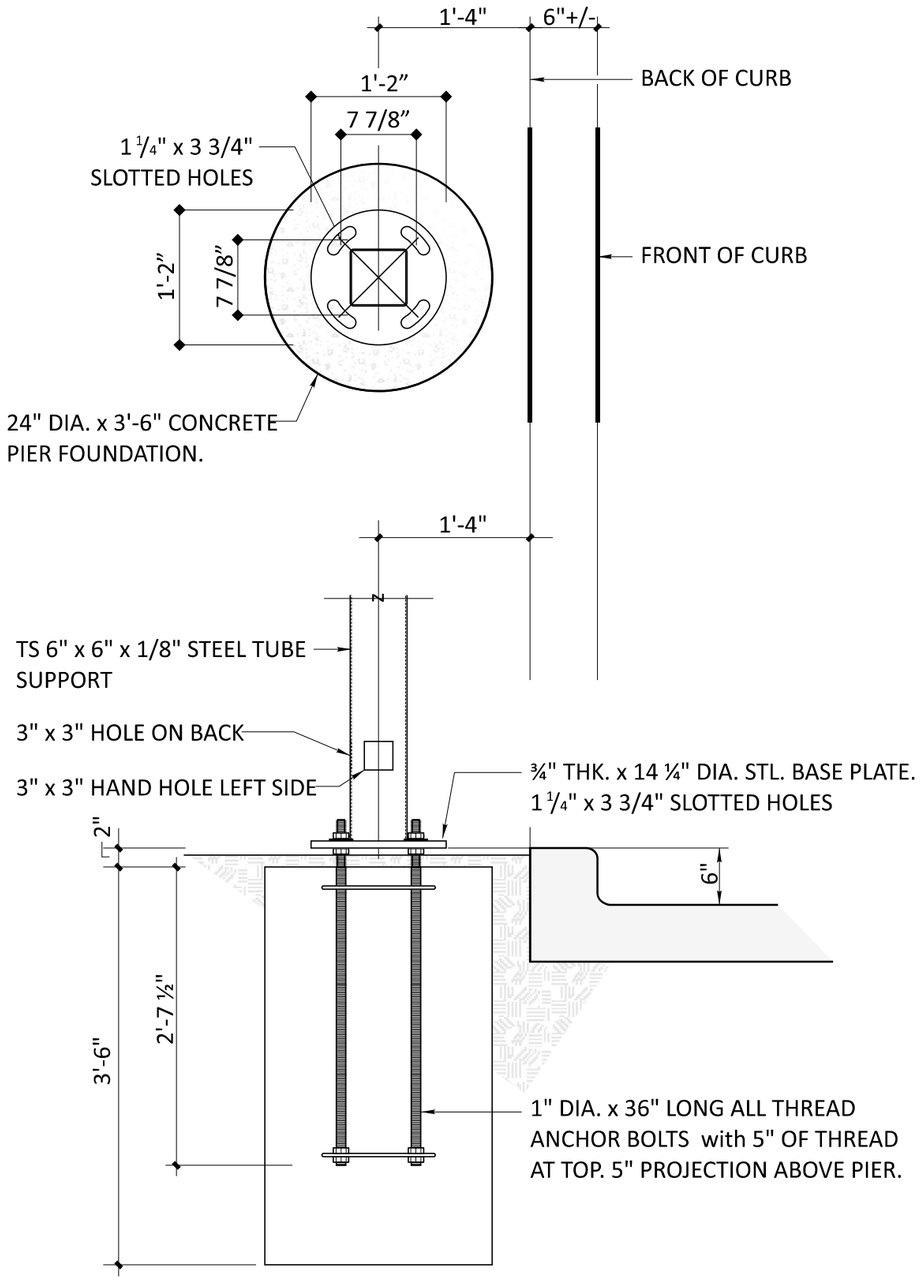
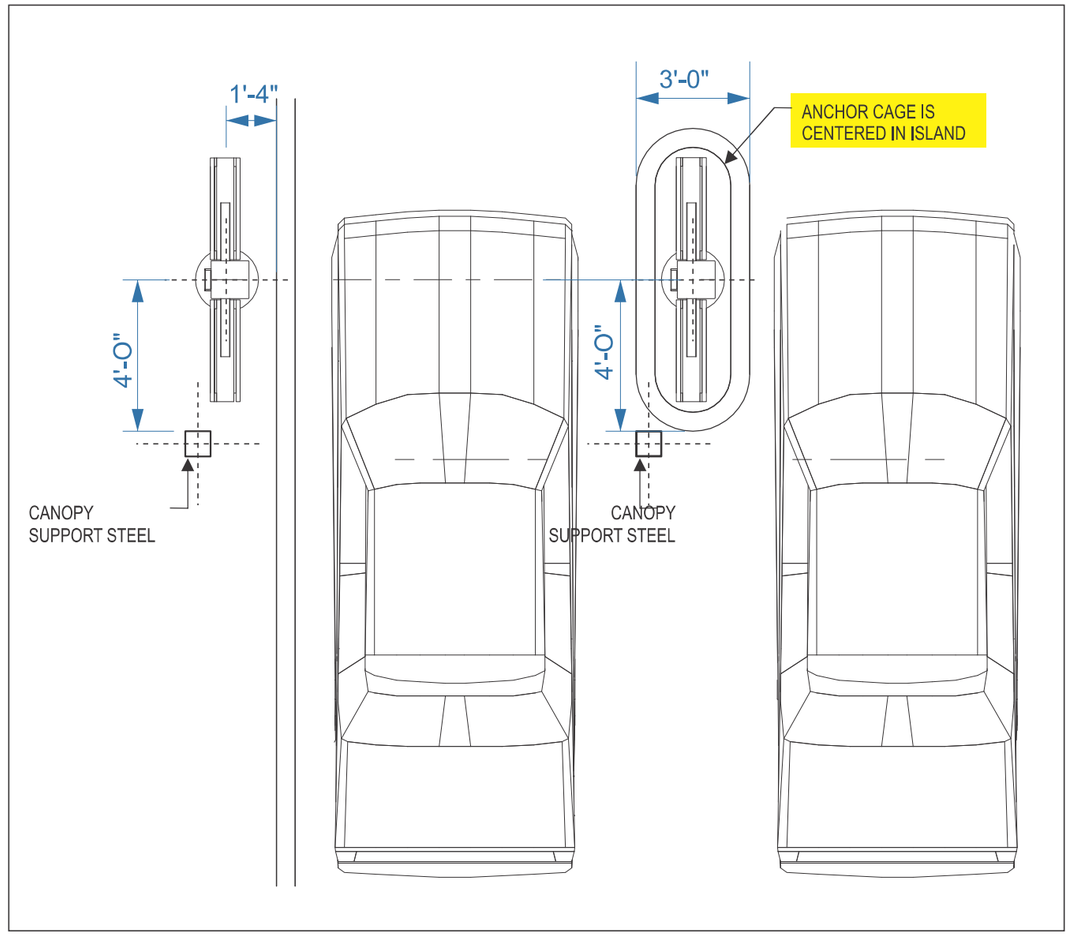
Chick-fil-A

B1-2



BASE PLATE AND ANCHOR DETAIL

¾" = 1'-0"



CHANDLER SIGNS TO PROVIDE ORDERING STATIONS.
CHANDLER SIGNS TO PROVIDE TEMPLATES AND ANCHOR BOLTS.
GEN. CONTRACTOR TO SET ANCHOR BOLTS AND POUR CONCRETE.
CHANDLER SIGNS TO INSTALL ORDERING STATIONS.

General Notes:
- MINIMUM CONCRETE RESISTANCE OF 3600 PSI AFTER 28 DAYS
- THE GROUND MUST NOT BE ALTERED AND MUST BE WELL DRAINED
- FOUNDATION IS BASE ON SAFE LATERAL SOIL BERING PRESSURE MINIMUM OF 150 PSF PER FOOT OF DEPTH. SOIL REPORT WAS NOT FURNISHED. ALLOWABLE BEARING PRESSURE SHOULD BE VERIFIED PRIOR TO PLACEMENT OF CONCRETE. DO NOT PLACE FOUNDATION FILL.
- ALL BACKFILLED TO BE PLACE IN COMPACTED LAYERS COMPACTED TO 95% MODIFIED PROCTOR DENSITY
- ELECTRICAL CONDUIT TO BE AS PER CITY CODE
- ALL VOIDS BETWEEN COLUMN BASE PLATE AND FOUNDATION SURFACE SHALL BE COMPLETELY FILLED WITH HIGH STRENGTH, NON-SHRINK GROUT

Design #	**0637033Ar3**
Sheet	7 of 27
Client	#2859
Address	690 NW Blue Pkwy, Lees Summit, MO
Account Rep.	KRISTEN HAMILTON / TERRI BROWN
Designer	LEAH LANSFORD
Date	9/29/23

Approval / Date

Client	
Sales	
Estimating	
Art	
Engineering	
Landlord	

Revision/Date

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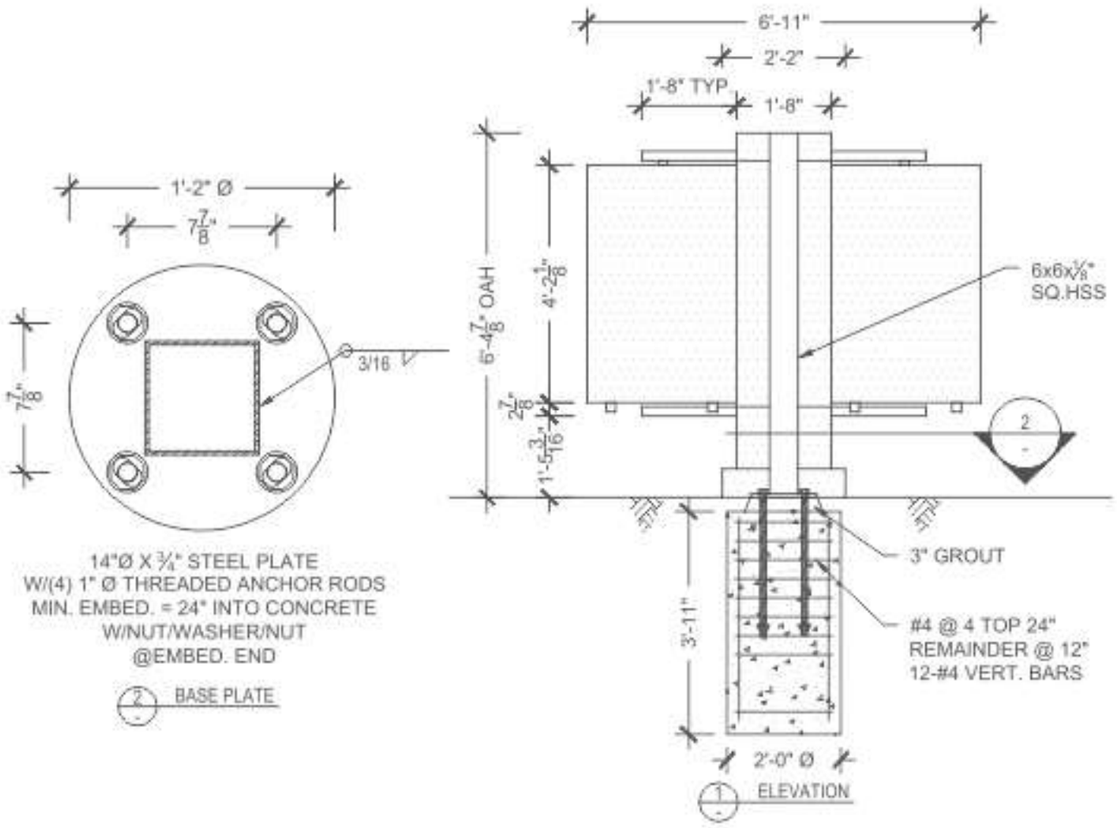
Chick-fil-A

B1-2

SULLAWAY ENGINEERING
11545 W BERNARDO CT., SUITE 201
SAN DIEGO, CA 92127
PROJECTMANAGER@SULLAWAYENG.COM
PHONE: 1-858-312-5150 FAX: 1-858-777-3534

PROJECT: CHICK-FIL-A, SIGN TYPE: CFA C7, 690 NW BLUE PKWY, LEE'S SUMMIT, MO
PROJECT #: 43787B
CLIENT: CHANDLER SIGNS

DATE: 01/09/2024
ENGINEER: SB/TH(IH)
LAST REVISED:



GENERAL NOTES

1. DESIGN CODE: IBC 2018
2. DESIGN LOADS: ASCE 7-16
3. WIND VELOCITY 110 MPH EXPOSURE C
4. CONCRETE 2500 PSI MINIMUM
5. SQ. HSS STEEL ASTM A500 GR. B, F_y = 46 KSI MIN.
6. PLATE STEEL ASTM A36
7. THREADED ANCHOR ROD STEEL ASTM F1554 GR. 36
8. STEEL REINFORCEMENT IN CONCRETE ASTM A615 GR 60
9. PROVIDE MIN. 3" CLEAR COVER ON ALL STEEL EMBEDDED IN CONCRETE WHEN CAST AGAINST SOIL
10. LATERAL SOIL BEARING PER IBC CLASS 4 (150 PSF/FT)
11. WELDING STRENGTH, Fexx = 70 KSI
12. PROVIDE PROTECTION AGAINST DISSIMILAR METALS
13. ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION

01-09-2024

SULLAWAY ENGINEERING
10815 Rancho Bernardo RD., SD, CA 92127
projectmanager@sullawayeng.com
Phone: 858-312-5150 Fax: 858-777-3534

PROJECT: CHICK-FIL-A
PROJ. NO.: 43787B
CLIENT: CHANDLER SIGNS

DATE: 1/9/24
ENGINEER: SB/TH(IH)

V5.5

units; pounds, feet unless noted otherwise

Applied Wind Loads; from ASCE 7-16

$F=q_z*G*C_f*A_f$ with $q_z = 0.00256K_zK_{zt}K_dV^2$ (29.3.2 & 29.4)

C_f= (Fig. 29.3-1) 1.00 max. height= 6.7

K_{zt}= 1.0 (26.8.2) (=1.0 unless unusual landscape)

K_z= from table 28.3-1 Exposure= c

K_d= 0.85 for signs (table 26.6-1)

V= 110 mph

G= 0.85 (26.9) weight= 0.341 kips

s/h= M_{DL}= 0.00 k-ft

B/s=

Pole Loads	structure component	height at section c.g.	K_z	q_z	pressure q_z*G*C_f	A_f	shear	Wind Moment M_w
	1	0.13	0.850	22.4	31.45	0.0	0	0
	2	0.50	0.850	22.4	31.45	1.1	34	17
	3	1.22	0.850	22.4	31.45	1.6	49	59
	4	1.77	0.850	22.4	31.45	0.8	26	46
	5	1.89	0.850	22.4	31.45	0.2	6	12
	6	4.01	0.850	22.4	31.45	28.9	909	3644
	7	6.14	0.850	22.4	31.45	0.2	5	30
	8	6.26	0.850	22.38	31.45	0.8	26	164
	9	6.50	0.850	22.38	31.45	0.5	17	108
					sums:	34.1	1072	4.08 (M_x) k-ft arm= 3.8

P_v= 0.41 kip M= 4.08 k-ft $M=sqrt(M_{DL}^2+M_w^2)$

$M_v=sqrt(1.2M_{DL}^2+1.0M_W^2)$ = 4.08 k-ft

Pole Design section; tube

$M_u \le \phi M_n$ with $M_n = f_yZ$ f_y= 46 ksi %√= 0.9

	H	M_u(k-ft)	Z req'd. (in)	Size(in)	t (in)	Z	USE
at	3" below grade	4.08	1.18	2.5	0.188	1.3	6x6x1/8" SQ.HSS, ϕMn=15.6k-ft

Footing Design footprint: round

ω= 1.3 IBC 1605.3.2 IBC Table 1806.2, sections 1806.3.4, 1807.3.2 S=(1.3x2x150 psf/ft)

P= 0.84 kip S1 = S x d / 3 A = 2.34 x P / (S1 x b) S= 400

S1= 519 d =0.5xA (1+ (1+4.36x h/A) ^.5) IBC 1807.3.2.1

A= 1.88

footing: 2' - 0" dia. 3' - 11" deep

Design #
0637033Ar3
Sheet 8 of 27
Client
#2859
Address
690 NW Blue Pkwy, Lees Summit, MO
Account Rep. KRISTEN HAMILTON, TERRI BROWN
Designer LEAH LANSFORD
Date 9/29/23
Approval / Date
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Chick-fil-A

B1-2

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10815 Rancho Bernardo RD., SD, CA 92127
projectmanager@sullawayeng.com
Phone: 858-312-5150 Fax: 858-777-3534

PROJECT: CHICK-FIL-A
PROJ. NO.: 43787B
CLIENT: CHANDLER SIGNS

DATE: 1/9/24
ENGINEER: SB/TH(IH)

v6.9

units; pounds, feet unless noted otherwise

Check 14" dia x 3/4" thk Round steel plate, A36

arm = 1.0875 in b = 10.05 in t = 0.75 in

Mplate = T * arm = 5.883 k-in (T= 5.410k per Simpson)

Z= bt^2/4= 1.413 in^3

ϕMn = %√*Fy*Z = 0.9*36ksi*Z = 45.790 k-in **OK**

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Version 3.2.2309.2

Company:		Date:	1/9/2024
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Address:			
Phone:			
E-mail:			

1.Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General
Design method:ACI 318-19
Units: Imperial units

Anchor Information:
Anchor type: Cast-in-place
Material: AB
Diameter (inch): 1.000
Effective Embedment depth, h_{ef} (inch): 24.000
Anchor category: -
Anchor ductility: Yes
h_{min} (inch): 26.63
C_{min} (inch): 6.00
S_{min} (inch): 6.00

Base Material
Concrete: Normal-weight
Concrete thickness, h (inch): 47.00
State: Cracked
Compressive strength, f'_c (psi): 2500
ψ_{cv}: 1.0
Reinforcement condition: Supplementary reinforcement not present
Supplemental edge reinforcement: Not applicable
Reinforcement provided at corners: No
Ignore concrete breakout in tension: No
Ignore concrete breakout in shear: No
Ignore 6do requirement: No
Build-up grout pad: No

Base Plate
Diameter x Thickness (inch): 14.00 x 0.75
Yield stress: 36000 psi

Profile type/size: HSS6X6X1/8

Recommended Anchor
Anchor Name: PAB Pre-Assembled Anchor Bolt - PAB8 (1"Ø)

Design #
0637033Ar3
Sheet 9 of 27
Client
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B1-2

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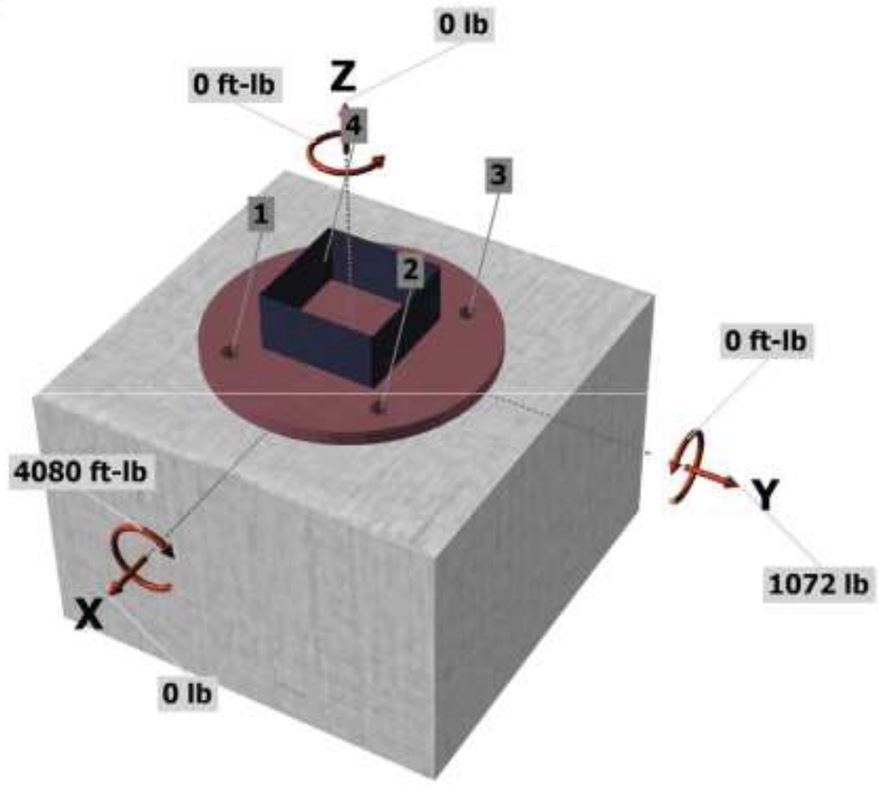
Company:		Date:	1/9/2024
Engineer:		Page:	2/6
Project:			
Address:			
Phone:			
E-mail:			

Load and Geometry
Load factor source: ACI 318 Section 5.3
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: Not applicable
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Strength level loads:

N_{ua} [lb]: 0
V_{uax} [lb]: 0
V_{uay} [lb]: 1072
M_{ux} [ft-lb]: -4080
M_{uy} [ft-lb]: 0
M_{uz} [ft-lb]: 0

<Figure 1>



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Company:		Date:	1/9/2024
Engineer:		Page:	3/6
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Address:			
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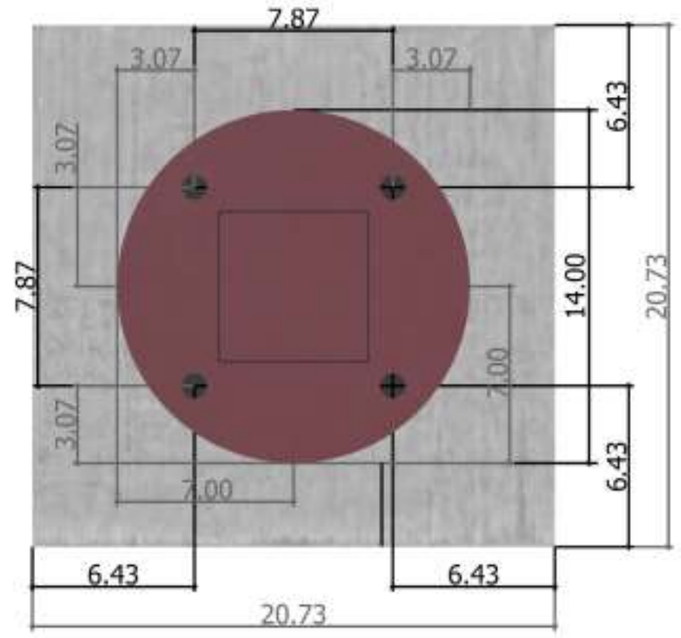
<Figure 2>



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B1-2

Anchor Designer™ Software
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Company:		Date:	1/9/2024
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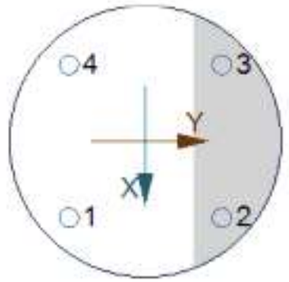
3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2+(V_{uay})^2}$ (lb)
1	2704.5	0.0	268.0	268.0
2	0.0	0.0	268.0	268.0
3	0.0	0.0	268.0	268.0
4	2704.5	0.0	268.0	268.0
Sum	5409.0	0.0	1072.0	1072.0

Maximum concrete compression strain (‰): 0.07
Maximum concrete compression stress (psi): 307
Resultant tension force (lb): 5409
Resultant compression force (lb): 5409
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. 17.6.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
35150	0.75	26363

5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.6.2)

$N_b = 16\lambda_a\sqrt{f'_c}h_{ef}^{5/3}$ (Eq. 17.6.2.2.1)

λ_a	f'_c (psi)	h_{ef} (in)	N_b (lb)
1.00	2500	9.533	34290

$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b$ (Sec. 17.5.1.2 & Eq. 17.6.2.1a)

A_{Nc} (in²)	A_{Nco} (in²)	$c_{a,min}$ (in)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
429.73	817.96	6.43	1.000	0.835	1.00	1.000	34290	0.70	10528

6. Pullout Strength of Anchor in Tension (Sec. 17.6.3)

$\phi N_{pn} = \phi \psi_{c,P} N_p = \phi \psi_{c,P} 6 A_{brg} f'_c$ (Sec. 17.5.1.2, Eq. 17.6.3.1 & 17.6.3.2.2a)

$\psi_{c,P}$	A_{brg} (in²)	f'_c (psi)	ϕ	ϕN_{pn} (lb)
1.0	5.15	2500	0.70	72156

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7. Side-Face Blowout Strength of Anchor in Tension (Sec. 17.6.4)

$\phi N_{sbg} = \phi (1+c_{a2}/c_{a1})/4)(1+s/6c_{a1})N_{sb} = \phi (1+c_{a2}/c_{a1})/4)(1+s/6c_{a1})(160c_a\sqrt{A_{brg}})\lambda_a\sqrt{f'_c}$ (Sec. 17.5.1.2, Eq. 17.6.4.1 & 17.6.4.2)

s (in)	c_{a1} (in)	c_{a2} (in)	A_{brg} (in²)	λ_a	f'_c (psi)	ϕ	ϕN_{sbg} (lb)
7.87	6.43	6.43	5.15	1.00	2500	0.70	49211

8. Steel Strength of Anchor in Shear (Sec. 17.7.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
21090	1.0	0.65	13709

9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.7.2)

Shear perpendicular to edge in y-direction:

$V_{cy} = \min[7(l_e / d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5}; 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}]$ (Eq. 17.7.2.2.1a & Eq. 17.7.2.2.1b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{cy} (lb)
8.00	1.000	1.00	2500	14.30	24334

$\phi V_{cbgy} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by}$ (Sec. 17.5.1.2 & Eq. 17.7.2.1b)

A_{Vc} (in²)	A_{Vco} (in²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgy} (lb)
444.66	920.21	1.000	0.790	1.000	1.000	24334	0.70	6502

Shear parallel to edge in x-direction:

$V_{by} = \min[7(l_e / d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5}; 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}]$ (Eq. 17.7.2.2.1a & Eq. 17.7.2.2.1b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
8.00	1.000	1.00	2500	6.43	7337

$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by}$ (Sec. 17.5.1.2, 17.7.2.1(c) & Eq. 17.7.2.1b)

A_{Vc} (in²)	A_{Vco} (in²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
199.94	186.05	1.000	1.000	1.000	1.000	7337	0.70	11039

10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.7.3)

$\phi V_{cpg} = \phi k_{cp} N_{cbg} = \phi k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b$ (Sec. 17.5.1.2 & Eq. 17.7.3.1b)

k_{cp}	A_{Nc} (in²)	A_{Nco} (in²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕV_{cpg} (lb)
2.0	429.73	165.38	1.000	1.000	1.000	1.000	9049	0.70	32921

11. Results

Interaction of Tensile and Shear Forces (Sec. 17.8)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	2704	26363	0.10	Pass
Concrete breakout	**5409**	**10528**	**0.51**	**Pass (Governs)**
Pullout	2704	72156	0.04	Pass
Side-face blowout	5409	49211	0.11	Pass

Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	268	13709	0.02	Pass
T Concrete breakout y+	**1072**	**6502**	**0.16**	**Pass (Governs)**
|| Concrete breakout x-	**536**	**11039**	**0.05**	**Pass (Governs)**

Design # 0637033Ar3
Sheet 11 of 27
Client #2859
Address 690 NW Blue Pkwy, Lees Summit, MO
Account Rep. KRISTEN HAMILTON / TERRI BROWN
Designer LEAH LANSFORD
Date 9/29/23

Approval / Date

Client	
Sales	
Estimating	
Art	
Engineering	
Landlord	

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B1-2

Anchor Designer™ Software
Version 3.2.2309.2

Company:		Date:	1/9/2024
Engineer:		Page:	6/6
Project:			
Address:			
Phone:			
E-mail:			

Pryout	1072	32921	0.03	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. 17.8.1	0.51	0.00	51.4%	1.0	Pass

PAB8 (1"Ø) with hef = 24.000 inch meets the selected design criteria.

12. Warnings

- Designer must exercise own judgement to determine if this design is suitable.

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0637033Ar3
Sheet 12 of 27
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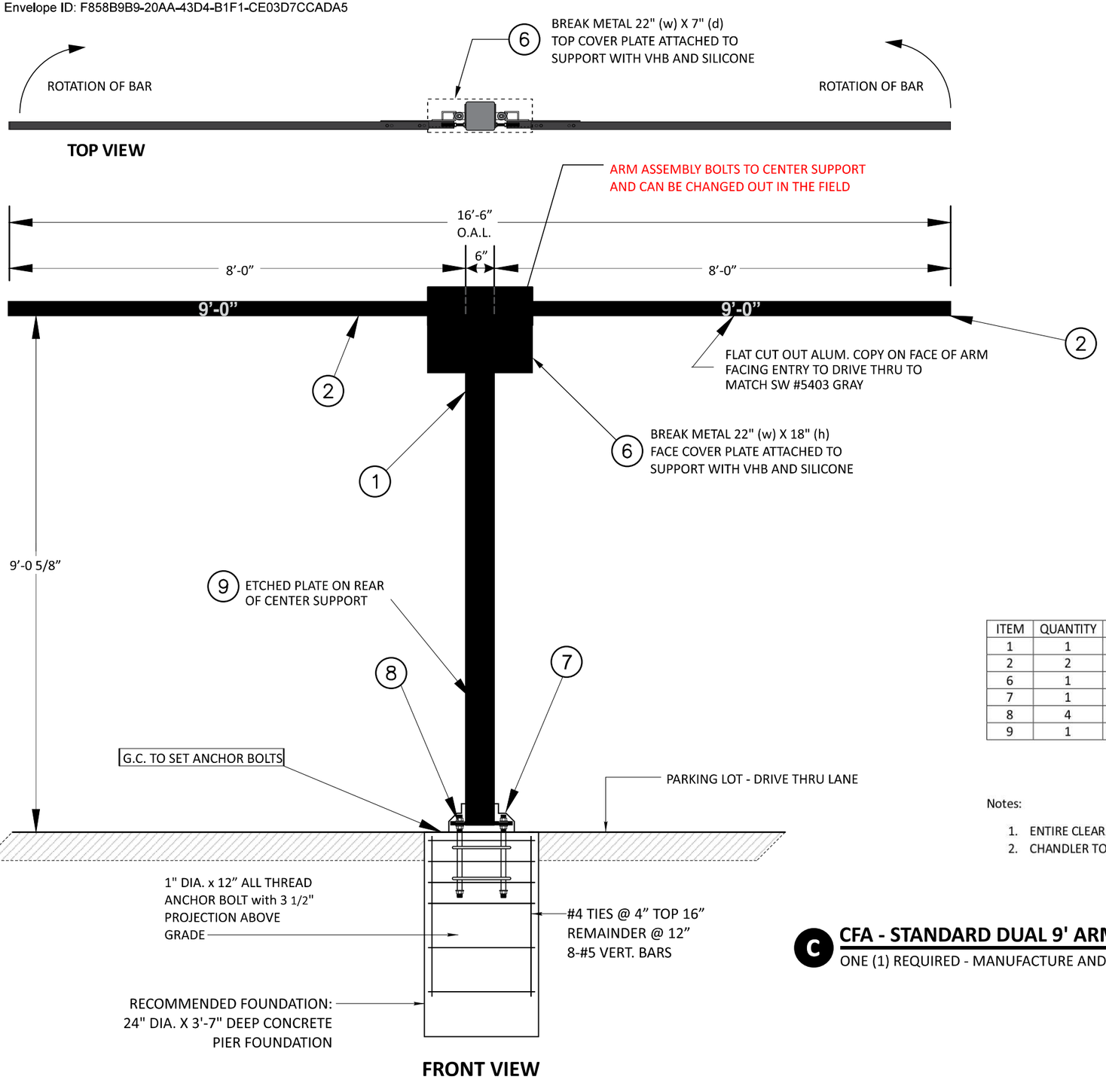
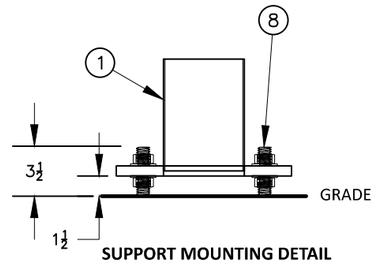
B1-2





SUPPORT MOUNTING DETAIL

ITEM	QUANTITY	DESCRIPTION
1	1	CENTER POST ASSEMBLY, 6"X 6" STEEL
2	2	CLEARANCE BAR ASSEMBLY, 2" X 2" ALUM ANGLE
6	1	TOP COVER, .125" ALUM, BREAK FORMED
7	1	BASE COVER, .08" ALUM, BREAK FORMED
8	4	1" X 36" LONG, ALL TREAD ANCHORS, INCLUDING NUTS AND WASHERS
9	1	CLEARANCE BAR ID PLATE

Notes:

1. ENTIRE CLEARANCE BAR AND HARDWARE TO BE POWDER-COATED P-820 MATTE BLACK FINISH
2. CHANDLER TO PROVIDE ALL-THREAD ANCHORS WITH FENDER WASHERS AND HEX BOLTS

C CFA - STANDARD DUAL 9' ARM CLEARANCE BAR - MATTE BLACK

SCALE: 1/2" = 1'-0"

ONE (1) REQUIRED - MANUFACTURE AND INSTALL

Design #
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Sheet 13 of 27

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#2859

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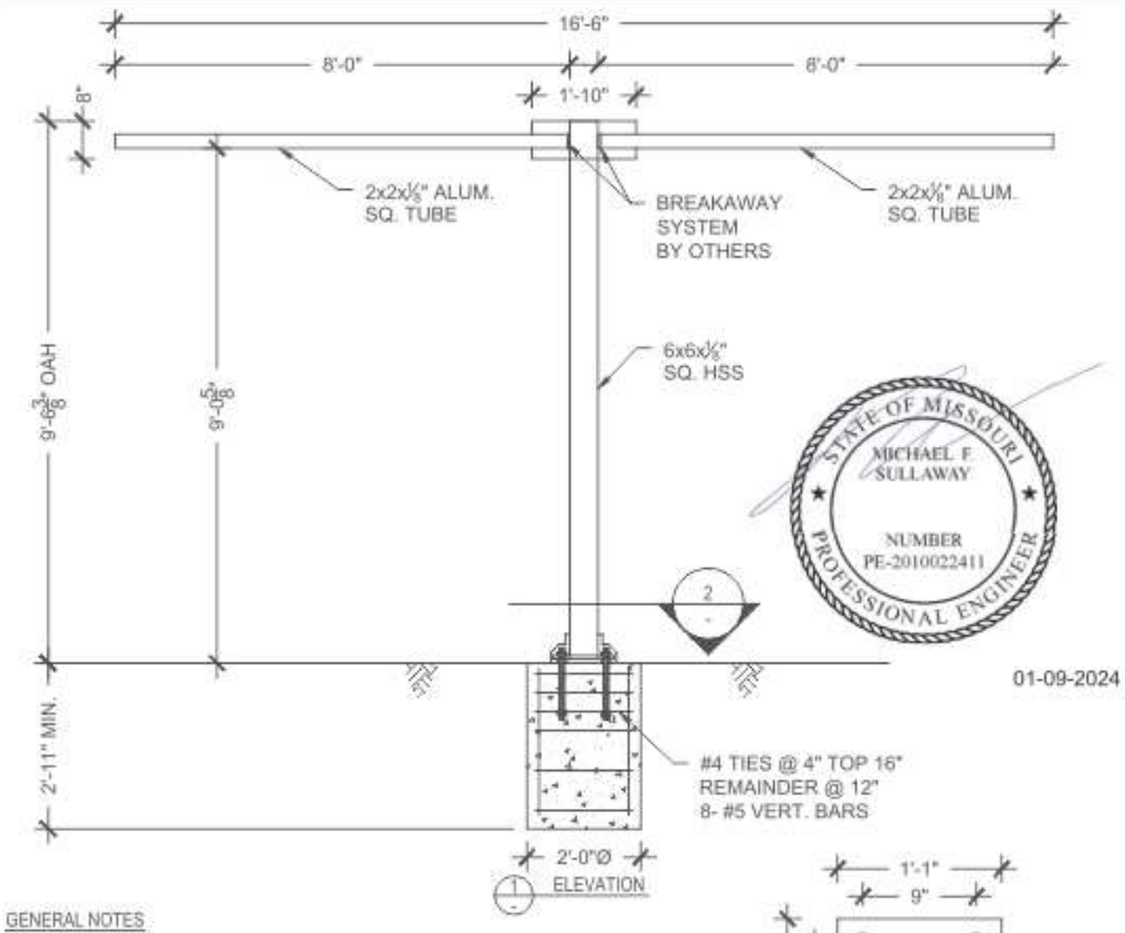
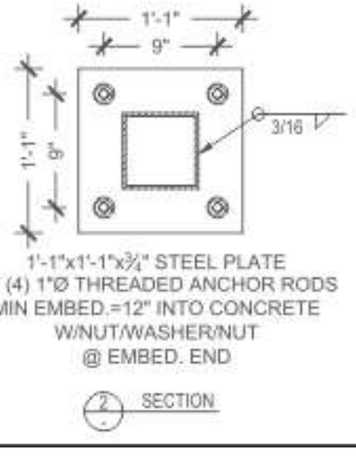
C

SULLAWAY ENGINEERING
11545 W. BERNARDO COURT, SUITE 201
SAN DIEGO, CA 92127
PROJECTMANAGER@SULLAWAYENG.COM
PHONE: 1-858-312-5150 FAX: 1-858-777-3534

PROJECT: CFA-2859, 690 NW BLUE PKWY, LEE'S SUMMIT, MO
PROJECT #: 43787C
CLIENT: CHANDLER SIGNS

DATE: 01/09/2024
ENGINEER: JC(IH)
LAST REVISED:





1'-1"x1'-1"x¾" STEEL PLATE
W/ (4) 1"Ø THREADED ANCHOR RODS
MIN EMBED.=12" INTO CONCRETE
W/NUT/WASHER/NUT
@ EMBED. END

GENERAL NOTES

1. DESIGN CODE: IBC 2018
2. DESIGN LOADS: ASCE 7-16
3. WIND VELOCITY 110 MPH EXPOSURE C
4. CONCRETE 2500 PSI MINIMUM
5. SQ. HSS STEEL ASTM A500 GR. B, F_y = 46 KSI MIN.
6. PLATE STEEL ASTM A36
7. ALUMINUM ELEMENTS 6061-T6
8. THREADED ANCHOR ROD STEEL ASTM F1554 GR. 36
9. STEEL REINFORCEMENT IN CONCRETE ASTM A615 GR 60
10. PROVIDE MIN. 3" CLEAR COVER ON ALL STEEL EMBEDDED IN CONCRETE WHEN CAST AGAINST SOIL
11. LATERAL SOIL BEARING PER IBC CLASS 4 (150 PSF/FT)
12. WELDING STRENGTH, Fexx = 70 KSI
13. PROVIDE PROTECTION AGAINST DISSIMILAR METALS
14. ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION

SULLAWAY ENGINEERING

PROJECT: CFA-2859
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DATE: 1/09/24
ENGINEER: JC(IH)

V5.5 units; pounds, feet unless noted otherwise

Applied Wind Loads; from ASCE 7-16

$F=q_z*G*C_f*A_f$ with $q_z = 0.00256K_zK_{zt}K_dV^2$ (29.3.2 & 29.4)

C_f= (Fig. 29.3-1) 1.00 max. height= 9.53

K_{zt}= 1.0 (26.8.2) (=1.0 unless unusual landscape)

K_z= from table 28.3-1 Exposure= c

K_d= 0.85 for signs (table 26.6-1)

V= 110 mph

G= 0.85 (26.9) weight= 0.083 kips

s/h= M_{DL}= 0.00 k-ft

B/s=

Pole Loads	structure component	height at section c.g.	K_z	q_z	pressure q_z*G*C_f	A_f	shear	Wind Moment M_w
	1	0.25	0.850	22.4	43.66	0.5	21	5
	2	4.68	0.850	22.4	43.66	4.2	183	855
	3	8.96	0.850	22.4	43.66	0.3	15	134
	4	9.14	0.850	22.4	43.66	2.7	120	1097
	5	9.38	0.850	22.4	43.66	0.6	25	234
					sums:	8.3	363	2.33 (M_w) k-ft am= 6.4

P_u= 0.10 kip M= 2.33 k-ft $M=sqrt(M_{DL}^2+M_w^2)$

$M_u=sqrt(1.2M_{DL}^2+1.0M_W^2)$ = 2.33 k-ft

Pole Design section; tube

$M_u \le \phi M_n$ with $M_n = f_yZ$ f_y= 46 ksi %√= 0.9

H	M_u(k-ft)	Z req'd. (in)	Size(in)	t (in)	Z	USE
at grade	2.33	0.67	2	0.25	1.0	6x6x1/8" SQ. HSS, ϕMn=15.6 k-ft

Footing Design footprint: round

ω= 1.3 IBC 1605.3.2 IBC Table 1806.2, sections 1806.3.4, 1807.3.2 S=(1.3x2x150 psf/ft)

P= 0.28 kip S1 = S x d / 3 A = 2.34 x P / (S1 x b) S= 400

S1= 388 d =0.5xA (1+ (1+4.36x h/A) ^.5) IBC 1807.3.2.1

A= 0.86

footing: 2' - 0" dia. 2' - 11" deep min.

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C

SULLAWAY ENGINEERING

PROJECT: CFA-2859 DATE: 1/09/24
PROJ. NO.: 43787C ENGINEER: JC(IH)
CLIENT: CHANDLER SIGNS

v5.0 units; pounds, feet unless noted otherwise

Check 2x2x1/8" Aluminum Sq. Tube (LRFD): ϕ= 0.75

Pnet=	(See Page#2)=	43.66 psf
Tributary Area=	ATrib=(7'-4")*(2")+(8")*(8")=	1.667 ft^2
Wind Load=	WL=Pnet*ATrib=	0.073 kips
Dead Load=	DL=1.2*10psf*ATrib=	0.020 kips
arm=	(8'-0")/2=	4 ft
MWL=	WL*arm=	0.291 k-ft
MDL=	DL*arm=	0.080 k-ft
Total Moment=	MDL + MWL=	4.453 k-in

Torsion on 6"x6"x1/8" SQ. HSS

Tu=	MWL*2 Clearance bar =	0.582 k-ft	6.99 k-in

Check 13x13x0.75" Steel Base Plate, A36

n =	2	arm=	1.650 in	b =	12.30 in	t =	0.75 in	T=	1.432
		Mplate =			T per bolt * n * arm =	4.726 k-in	(T= From Simpson's)
		Z=			bt^2/4=	1.730 in^3	
		ϕMn =			ϕ*Fy*Z = 0.9*36ksi*Z =	56.04 k-in	OK

Loads on 1" dia. Threaded Anchor Rods, F1554 Gr.36:

Mu =	=	2.33 k-ft	(See Page #2)
Vu =	=	0.363 kips	
T		k-ft	

SULLAWAY ENGINEERING

PROJECT: CFA-2859 **DATE:** 1/09/24
PROJ. NO.: 43787C **ENGINEER:** JC(IH)
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Check 2"x2"x0.125" Sq. Aluminum Tube (ADM 2015 - Ch F)

Mu = 4.453 k-in (See Page#3)

Yielding: (Governs)
(ADM 15, F.2) Mn = Z Fcy = 23.1 k-in
Mn = 1.5 St Fty = 29.0 k-in

PMn = 20.8 k-in **OK**

LB:
(ADM 15 F.3.1)
Bp = 45.0 ksi
Dp = 0.30 ksi
Cp = 61.4

91 = Bp-Fcy/1.6Dp = 20.8
92 = k1 Bp/1.6Dp = 15.2

since b/t< 91: Fc = Fcy = 35.00

Bbr = 66.8 ksi
Dbr = 0.67 ksi
Cbr = 67

91 = Bbr-1.5 Fcy/mDbr = 33.0
92 = k1 Bbr/mDbr = 77.2

since b/t< 91: Fc = 1.5 Fcy = 52.50

Mnlb = Fc If / ccf + Fb Iw / ccw = 49.57 k-in
PMnlb = 44.61 k-in **OK**

d	t
2	0.125
welded?	NO
Fcy = Fty =	35 ksi
Z =	0.660 in^3
St =	0.552 in^3
P =	0.9
8 =	1 ksi
E =	10100 ksi
k1c =	0.35
k2c =	2.27
k1f =	0.5
k2f =	2.04
b/t =	14.0
m =	0.65
I =	0.552 in^4
ccf =	0.938 in
ccw =	1.0 in

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C

SULLAWAY ENGINEERING

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PROJ. NO.: 43787C ENGINEER: JC(IH)
CLIENT: CHANDLER SIGNS

units; pounds, feet unless noted otherwise

Check HSS6X6X.125 for torsion and combined forces (AISC 14 H3)

Tr =	6.99 k-in	(See Page#3)	Fy =	46 ksi
			h =	5.65 in^3
h/t =	48.7		t =	0.116 in^3
$2.45 (E/Fy)^{1/2}$ =	61.52		E =	29000 ksi
$3.07 (E/Fy)^{1/2}$ =	77.08		L =	in
			C =	8.03 in^3
Fcr =	27.6 ksi	(eq'n. H3-3)	ϕ =	0.9
$Tc=\phi Tn = \phi$ Fcr C =	199.5 k-in	OK		
$Mr/Mc + (Tr/Tc)^2$ =	0.15 <1	OK	(eq'n. H3-6)	
			(See Page #2 for Mr & Mc)	

Anchor Designer™ Software
Version 3.2.2309.2

Company:		Date:	1/9/2024
Engineer:		Page:	1/6
Project:			
Address:			
Phone:			
E-mail:			

1.Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General
Design method:ACI 318-19
Units: Imperial units

Anchor Information:
Anchor type: Cast-in-place
Material: AB
Diameter (inch): 1.000
Effective Embedment depth, h_{ef} (inch): 12.000
Anchor category: -
Anchor ductility: Yes
h_{min} (inch): 14.63
C_{min} (inch): 6.00
S_{min} (inch): 6.00

Base Material
Concrete: Normal-weight
Concrete thickness, h (inch): 35.00
State: Cracked
Compressive strength, f'_c (psi): 2500
$\Psi_{c,V}$: 1.0
Reinforcement condition: Supplementary reinforcement not present
Supplemental edge reinforcement: Not applicable
Reinforcement provided at corners: No
Ignore concrete breakout in tension: No
Ignore concrete breakout in shear: No
Ignore 6do requirement: No
Build-up grout pad: No

Base Plate
Length x Width x Thickness (inch): 13.00 x 13.00 x 0.75

Recommended Anchor
Anchor Name: PAB Pre-Assembled Anchor Bolt - PAB8 (1"Ø)

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Georgia	111 Woodstone Place Dawsonville, GA 30534 (678) 725-8852 Fax (210) 349-8724
South Texas	PO BOX 125 206 Doral Drive Portland, TX 78374 (361) 563-5599 Fax (361) 643-6533

FINAL ELECTRICAL CONNECTION BY CUSTOMER

THIS SIGN IS INTENDED TO BE INSTALLED IN ACCORDANCE WITH ARTICLE 600 OF THE NATIONAL ELECTRICAL CODE AND/OR OTHER APPLICABLE LOCAL CODES. THIS INCLUDES PROPER GROUNDING & BONDING OF THE SIGN. SIGN WILL BEAR UL LABEL(S).

Chick-fil-A

C

Anchor Designer™ Software
Version 3.2.2309.2

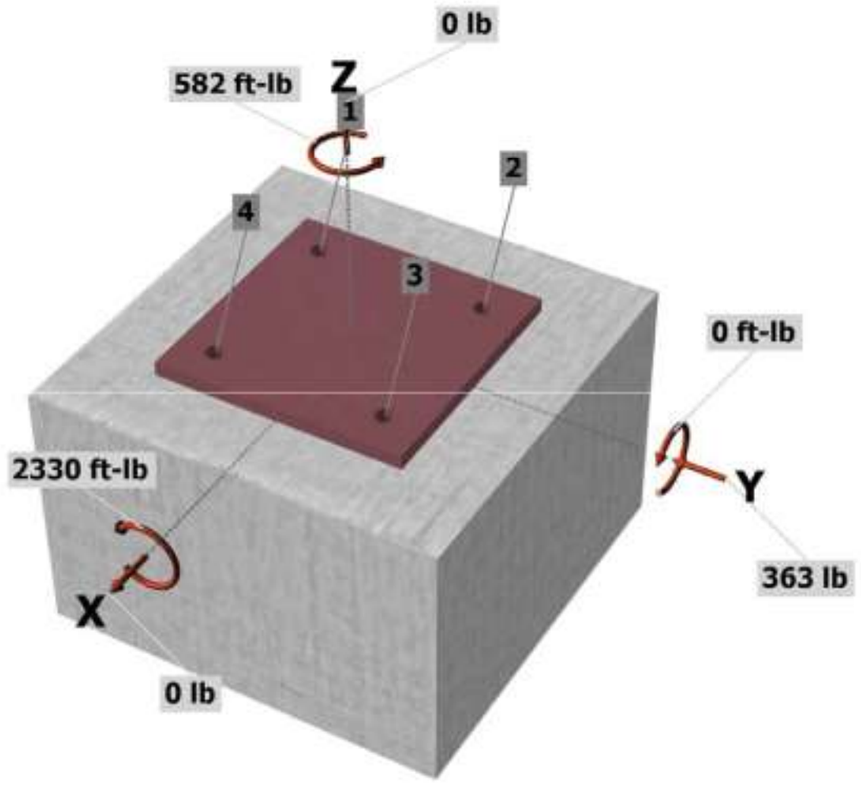
Company:		Date:	1/9/2024
Engineer:		Page:	2/6
Project:			
Address:			
Phone:			
E-mail:			

Load and Geometry
Load factor source: ACI 318 Section 5.3
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: Not applicable
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Strength level loads:

N_{ua} [lb]: 0
V_{uax} [lb]: 0
V_{uay} [lb]: -363
M_{ux} [ft-lb]: 2330
M_{uy} [ft-lb]: 0
M_{uz} [ft-lb]: 582

<Figure 1>



Anchor Designer™ Software
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Company:		Date:	1/9/2024
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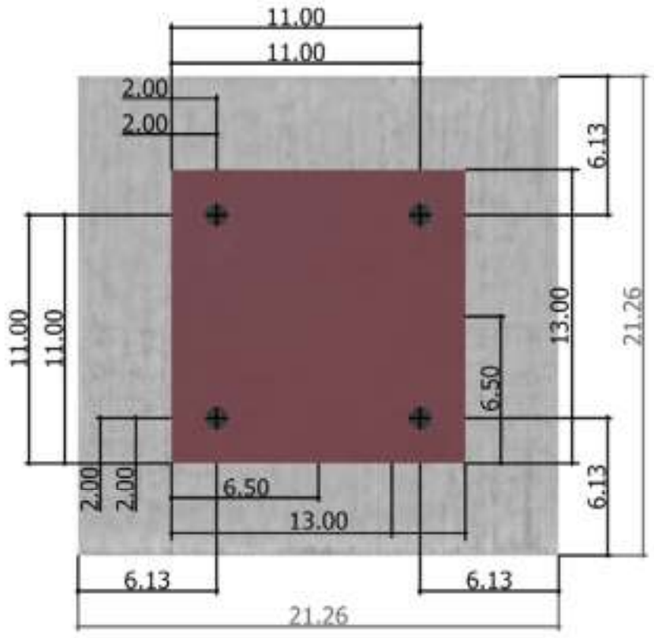
Phone:			
E-mail:			

<Figure 2>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com

Design #	**0637033Ar3**
Sheet	17 of 27
Client	#2859
Address	690 NW Blue Pkwy, Lees Summit, MO
Account Rep.	KRISTEN HAMILTON, TERRI BROWN
Designer	LEAH LANSFORD
Date	9/29/23

Approval / Date

Client	
Sales	
Estimating	
Art	
Engineering	
Landlord	

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R1(12-22-23)LL: CHANGED SIZE OF PANELS ON THE STOP/DNE

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R3(02/26/2024)AM: UPDATED SP, ADDED NOTE FOR SIGN A FOR CSI TO PROVIDE NEW STEEL AND FOUNDATION AND ADDED NOT FOR SIGN D FOR CSI TO REMOVE AND DISCARD EXISTING D/F DIRECTIONAL

CHANDLER SIGNS

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Chick-fil-A

C

Anchor Designer™ Software
Version 3.2.2309.2

Company:		Date:	1/9/2024
Engineer:		Page:	4/6
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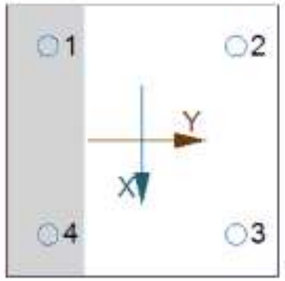
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2+(V_{uay})^2}$ (lb)
1	0.0	194.0	-284.7	344.5
2	1432.0	-194.0	-284.7	344.5
3	1432.0	-194.0	103.2	219.8
4	0.0	194.0	103.2	219.8
Sum	2864.0	0.0	-363.0	1128.6

Maximum concrete compression strain (‰): 0.03
Maximum concrete compression stress (psi): 119
Resultant tension force (lb): 2864
Resultant compression force (lb): 2864
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. 17.6.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
35150	0.75	26363

5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.6.2)

$N_b = 16\lambda_a\sqrt{f'_c}h_{ef}^{5/3}$ (Eq. 17.6.2.2.1)

λ_a	f'_c (psi)	h_{ef} (in)	N_b (lb)
1.00	2500	10.087	37671

$\phi N_{cbg} = \phi\,(A_{Nc}/A_{Nco})\,\psi_{ec,N}\,\psi_{ed,N}\,\psi_{c,N}\,\psi_{cp,N}N_b$ (Sec. 17.5.1.2 & Eq. 17.6.2.1a)

A_{Nc} (in²)	A_{Nco} (in²)	$c_{a,min}$ (in)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
451.99	915.67	6.13	1.000	0.822	1.00	1.000	37671	0.70	10694

6. Pullout Strength of Anchor in Tension (Sec. 17.6.3)

$\phi N_{pn} = \phi\psi_{c,P}N_p = \phi\psi_{c,P}8A_{brg}f'_c$ (Sec. 17.5.1.2, Eq. 17.6.3.1 & 17.6.3.2.2a)

$\psi_{c,P}$	A_{brg} (in²)	f'_c (psi)	ϕ	ϕN_{pn} (lb)
1.0	5.15	2500	0.70	72156

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8. Steel Strength of Anchor in Shear (Sec. 17.7.1)

V_{sa} (lb)	$\psi_{g,ear}$	ϕ	$\phi_{g,ear}\phi V_{sa}$ (lb)
21090	1.0	0.65	13709

9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.7.2)

Shear perpendicular to edge in x-direction:

$V_{cb} = \min[7(l_e/d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5};\ 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}]$ (Eq. 17.7.2.2.1a & Eq. 17.7.2.2.1b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{cb} (lb)
8.00	1.000	1.00	2500	15.13	26483

$\phi V_{cbgx} = \phi\,(A_{Vc}/A_{Vco})\,\psi_{ec,V}\psi_{ed,V}\psi_{c,V}\psi_{h,V}V_{ba}$ (Sec. 17.5.1.2 & Eq. 17.7.2.1b)

A_{Vc} (in²)	A_{Vco} (in²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{ba} (lb)	ϕ	ϕV_{cbgx} (lb)
482.50	1030.13	1.000	0.781	1.000	1.000	26483	0.70	6782

Shear perpendicular to edge in y-direction:

$V_{by} = \min[7(l_e/d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5};\ 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}]$ (Eq. 17.7.2.2.1a & Eq. 17.7.2.2.1b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
8.00	1.000	1.00	2500	15.13	26483

$\phi V_{cbgy} = \phi\,(A_{Vc}/A_{Vco})\,\psi_{ec,V}\psi_{ed,V}\psi_{c,V}\psi_{h,V}V_{by}$ (Sec. 17.5.1.2 & Eq. 17.7.2.1b)

A_{Vc} (in²)	A_{Vco} (in²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgy} (lb)
482.50	1030.13	1.000	0.781	1.000	1.000	26483	0.70	6782

Shear parallel to edge in x-direction:

$V_{by} = \min[7(l_e/d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5};\ 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}]$ (Eq. 17.7.2.2.1a & Eq. 17.7.2.2.1b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
8.00	1.000	1.00	2500	6.13	6830

$\phi V_{cbgx} = \phi\,(2)(A_{Vc}/A_{Vco})\,\psi_{ec,V}\psi_{ed,V}\psi_{c,V}\psi_{h,V}V_{by}$ (Sec. 17.5.1.2, 17.7.2.1(c) & Eq. 17.7.2.1b)

A_{Vc} (in²)	A_{Vco} (in²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
195.49	169.10	1.000	1.000	1.000	1.000	6830	0.70	11054

Shear parallel to edge in y-direction:

$V_{ba} = \min[7(l_e/d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5};\ 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}]$ (Eq. 17.7.2.2.1a & Eq. 17.7.2.2.1b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{ba} (lb)
8.00	1.000	1.00	2500	6.13	6830

$\phi V_{cbgy} = \phi\,(2)(A_{Vc}/A_{Vco})\,\psi_{ec,V}\psi_{ed,V}\psi_{c,V}\psi_{h,V}V_{ba}$ (Sec. 17.5.1.2, 17.7.2.1(c) & Eq. 17.7.2.1b)

A_{Vc} (in²)	A_{Vco} (in²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{ba} (lb)	ϕ	ϕV_{cbgy} (lb)
195.49	169.10	1.000	1.000	1.000	1.000	6830	0.70	11054

10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.7.3)

$\phi V_{cp} = \phi k_{cp}N_{ba} = \phi k_{cp}(A_{Nc}/A_{Nco})\,\psi_{ed,N}\,\psi_{c,N}\,\psi_{cp,N}N_b$ (Sec. 17.5.1.2 & Eq. 17.7.3.1a)

k_{cp}	A_{Nc} (in²)	A_{Nco} (in²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕV_{cp} (lb)
2.0	113.00	915.67	0.822	1.000	1.000	37671	0.70	5347

11. Results

Interaction of Tensile and Shear Forces (Sec. 17.8)

Design #
0637033Ar3
Sheet 18 of 27
Client
#2859
Address
690 NW Blue Pkwy, Lees Summit, MO
Account Rep. KRISTEN HAMILTON / TERRI BROWN
Designer LEAH LANSFORD
Date 9/29/23
Approval / Date
Client
Sales
Estimating
Art
Engineering
Landlord

Revision/Date

R1(12-22-23)LL: CHANGED SIZE OF PANELS ON THE STOPDINE

R2(1/16/24)LL: ADDED STAMPED ENGINEERING

CORRECTION(01/17/2024)AM: REVISED DETAIL SECTION AND ADDED THE WORD G.C. IN FRONT OF REMOVALS FOR SIGNS B1-2, C, D, G1-2 AND H1-4

R3(02/26/2024)AM: UPDATED SP, ADDED NOTE FOR SIGN A FOR CSI TO PROVIDE NEW STEEL AND FOUNDATION AND ADDED NOT FOR SIGN D FOR CSI TO REMOVE AND DISCARD EXISTING D/F DIRECTIONAL

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Chick-fil-A

C

Anchor Designer™ Software
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Company:		Date:	1/9/2024
Engineer:		Page:	6/6
Project:			
Address:			
Phone:			
E-mail:			

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	1432	26363	0.05	Pass
Concrete breakout	**2864**	**10694**	**0.27**	**Pass (Governs)**
Pullout	1432	72156	0.02	Pass

Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	345	13709	0.03	Pass
T Concrete breakout x+	388	6782	0.06	Pass
T Concrete breakout y-	569	6782	0.08	Pass
|| Concrete breakout x-	569	11054	0.05	Pass
|| Concrete breakout y+	388	11054	0.04	Pass
Concrete breakout, combined	-	-	**0.10**	**Pass (Governs)**
Pryout	345	5347	0.06	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. 17.8.1	0.27	0.00	26.8%	1.0	Pass

PAB8 (1"Ø) with hef = 12.000 inch meets the selected design criteria.

12. Warnings

- Designer must exercise own judgement to determine if this design is suitable.

Design #
0637033Ar3
Sheet 19 of 27
Client
#2859
Address
690 NW Blue Pkwy,
Lees Summit, MO
Account Rep. KRISTEN HAMILTON
TERRI BROWN
Designer LEAH LANSFORD
Date 9/29/23
Approval / Date

Client	
Sales	
Estimating	
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Engineering	
Landlord	

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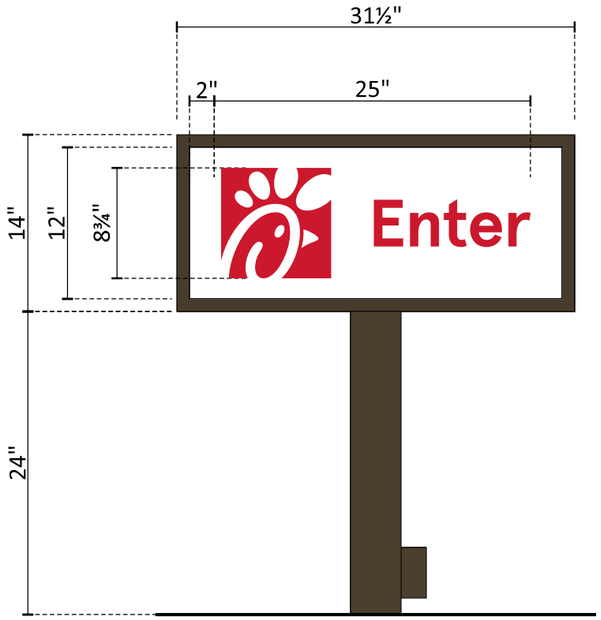
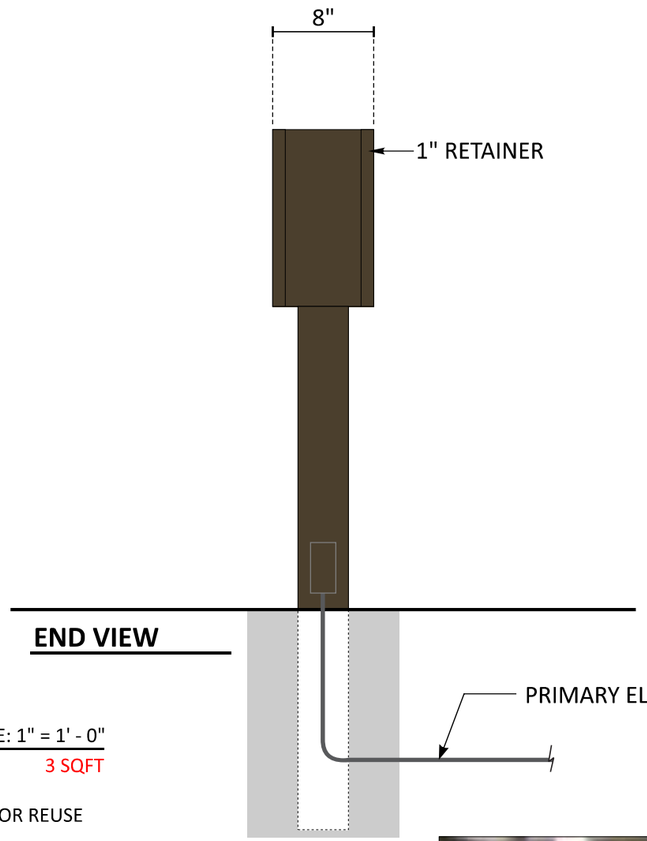
Chick-fil-A

C



SIDE 1



END VIEW

SIDE 2

2014/2017 NEC
COMPLIANT LABEL

WARNING LABEL/
BREAKER LOCATION

D CFA - D/F DIRECTIONAL

SCALE: 1" = 1' - 0"

ONE (1) REMOVAL REQUIRED - MANUFACTURE & INSTALL

3 SQFT

CSI TO REMOVE AND DISCARD EXISTING D/F DIRECTIONAL. CAP ELECTRICAL FOR REUSE

MANUFACTURE AND INSTALL ONE NEW D/F VIS 22 DIRECTIONAL.
REUSE EXISTING ELECTRICAL.

GENERAL SPECIFICATIONS

D/F ALUMINUM SIGN CABINET INTERNALLY ILLUMINATED WITH 6500K WHITE LEDS.

FILLER, RETAINER AND SUPPORT PAINTED MATTHEWS #20181 DK BRONZE, SATIN.

ACRYLIC FACES

#7328 WHITE ACRYLIC DECORATED WITH AVERY UC900-440-T RED TRANSLUCENT VINYL (BRAND REQ.) APPLIED 1ST SURFACE.

TEXT TO BE APERCU SENTENCE CASE BOLD

STEEL POLE

4" x 4" STEEL SQUARE TUBE SUPPORT

EXISTING CONDITIONS

Design #	0637033Ar3
Sheet	20 of 27
Client	#2859
Address	690 NW Blue Pkwy, Lees Summit, MO
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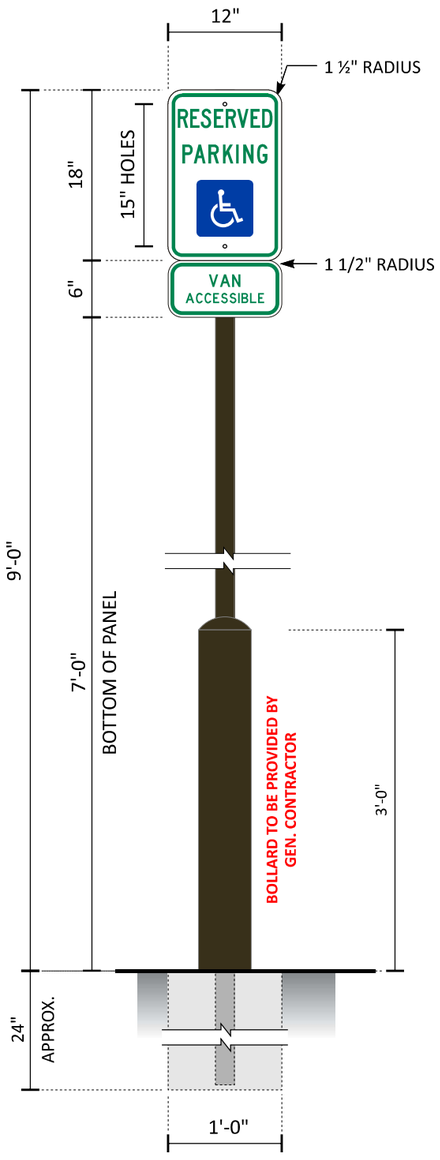
D

BOLLARD MOUNTED HANDICAP SIGNS



SPECIFICATIONS

SIGN PANELS

.080 ALUMINUM WITH BACKS PAINTED MATTHEWS #20181 DARK BRONZE, SATIN AND REFLECTIVE VINYL GRAPHICS APPLIED 1ST SURFACE.

SIGN PANELS ARE TO BE SECURED TO SIGN POST WITH NUTS AND BOLTS PAINTED TO MATCH VINYL BACKGROUND

SIGN POST

2" x 2" x .125"(WALL) ALUMINUM SQ. TUBE, CAPPED ON TOP WITH PUSH IN PLASTIC CAP PAINTED MATTHEWS #20181 DARK BRONZE, SATIN.
CAP IS A BUYOUT FROM MCMASTER CARR #9565K31
INVENTORY #02948

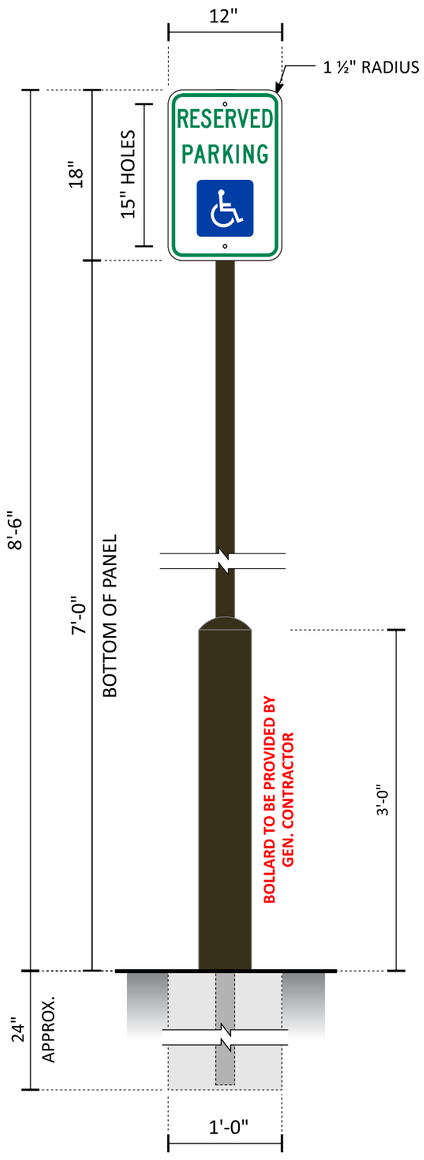
BOLLARD - TO BE PROVIDED BY GEN.CONTRACTOR

5 9/16" O.D. x .280 WALL THICKNESS STEEL PIPE.

BOLLARD AND CONCRETE TOP TO BE PAINTED MATTHEWS #20181 DARK BRONZE, SATIN.
INSTALLER TO PROVIDE PAINT.

DAL - MANUFACTURING

MUTCD PANEL - UPPERCASE



E1 E2 **CFA - S/F HCVA SIGN** SCALE: 3/4" = 1'-0"

TWO (2) REQUIRED - MANUFACTURE AND INSTALL — 2 SQ. FT.

F **CFA - S/F HC SIGN** SCALE: 3/4" = 1'-0"

ONE (1) REQUIRED - MANUFACTURE AND INSTALL — 1.5 SQ. FT.

DFW MANUFACTURING / NEW VIS 2022 MUTCD PANEL

Design #	**0637033Ar3**
Sheet	21 of 27
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Account Rep.	KRISTEN HAMILTON, TERRI BROWN
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South Texas	PO BOX 125 206 Doral Drive Portland, TX 78374 (361) 563-5599 Fax (361) 643-6533

FINAL ELECTRICAL CONNECTION BY CUSTOMER

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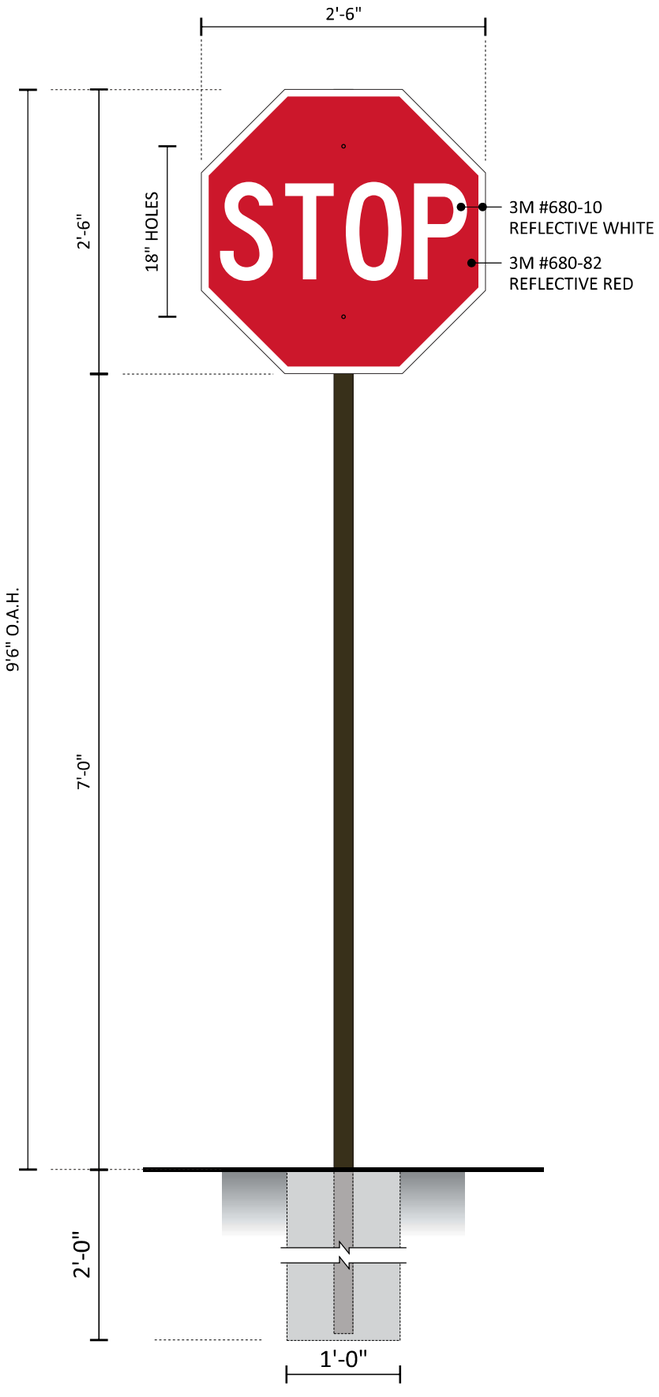
Chick-fil-A

E1-2, F



G1 G2 CFA - DOT S/F STOP SIGN

SCALE: 3/4" = 1'-0"

TWO (2) REMOVALS REQUIRED - MANUFACTURE AND INSTALL — 5 SQ. FT.

G.C. TO REMOVE AND DISCARD EXISTING STOP SIGNS IN PARKING LOT LANDSCAPING.

MANUFACTURE AND INSTALL TWO NEW S/F VIS 22 STOP SIGNS.

SPECIFICATIONS

SIGN PANELS
.080 ALUMINUM PANELS WITH BACKS PAINTED MATTHEWS
#20181 DK. BRONZE, SATIN AND REFLECTIVE VINYL
GRAPHICS APPLIED 1ST SURFACE.

SIGN PANEL IS TO BE SECURED TO SIGN POST WITH NUTS AND BOLTS PAINTED TO MATCH VINYL BACKGROUND

SIGN POST
2" x 2" x .125"(WALL) ALUMINUM SQ. TUBE, CAPPED ON TOP WITH
PUSH IN PLASTIC CAP PAINTED MATTHEWS #20181
DARK BRONZE, SATIN.
CAP IS A BUYOUT FROM MCMASTER CARR #9565K31
INVENTORY #02948

NOTE:
IF SIGN IS LOCATED ON SIDEWALK, THEN IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO INSTALL A PVC SLEEVE INTO THE CONCRETE WHEN THE SIDEWALK IS POURED PRIOR TO INSTALLATION OF THE SIGN.

DAL - MANUFACTURING

MUTCD PANEL - UPPERCASE

EXISTING CONDITIONS

EXISTING CONDITIONS

DFW MANUFACTURING / NEW VIS 2022 MUTCD PANEL

Design #	0637033Ar3
Sheet	22 of 27
Client	#2859
Address	690 NW Blue Pkwy, Lees Summit, MO
Account Rep.	KRISTEN HAMILTON, TERRI BROWN
Designer	LEAH LANSFORD
Date	9/29/23

Approval / Date

Client	
Sales	
Estimating	
Art	
Engineering	
Landlord	

Revision/Date

R1(12-22-23)LL: CHANGED SIZE OF PANELS ON THE STOPDINE

R2(1/16/24)LL: ADDED STAMPED ENGINEERING

CORRECTION(01/17/2024)AM: REVISED DETAIL SECTION AND ADDED THE WORD G.C. IN FRONT OF REMOVALS FOR SIGNS B1-2, C, D, G1-2 AND H1-4

R3(02/26/2024)AM: UPDATED SP, ADDED NOTE FOR SIGN A FOR CSI TO PROVIDE NEW STEEL AND FOUNDATION AND ADDED NOT FOR SIGN D FOR CSI TO REMOVE AND DISCARD EXISTING D/F DIRECTIONAL

CHANDLER SIGNS

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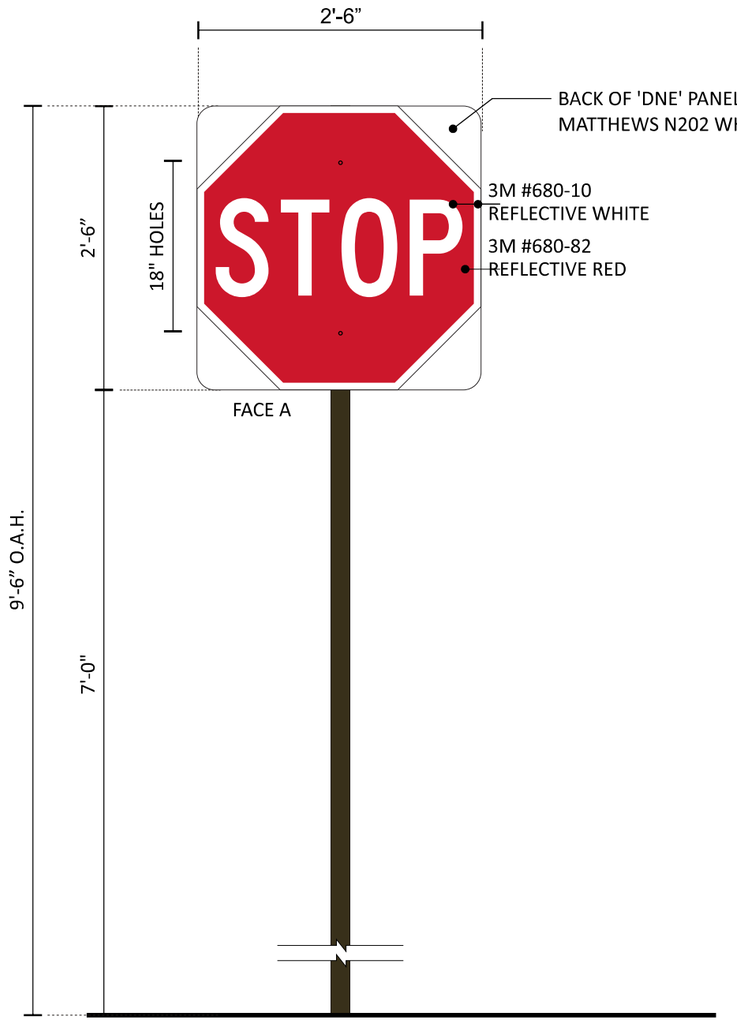
Chick-fil-A

G1-2



SPECIFICATIONS

SIGN PANELS
.080 ALUMINUM PANELS WITH BACKS PAINTED **MATTHEWS N202 WHITE**, SATIN AND REFLECTIVE VINYL GRAPHICS APPLIED 1ST SURFACE.

SIGN PANEL IS TO BE SECURED TO SIGN POST WITH NUTS AND BOLTS PAINTED TO MATCH VINYL BACKGROUND

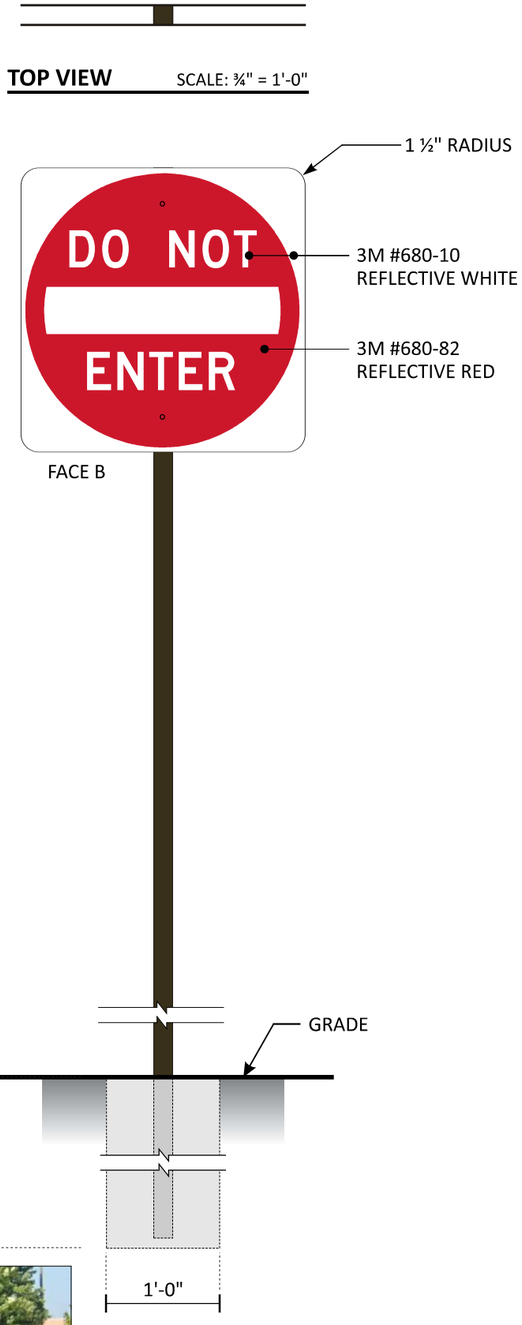
SIGN POST
2" x 2" x .125"(WALL) ALUMINUM SQ. TUBE, CAPPED ON TOP WITH PUSH IN PLASTIC CAP.
PAINTED MATTHEWS #20181 DARK BRONZE, SATIN.
CAP IS A BUYOUT FROM MCMASTER CARR #9565K31
INVENTORY #02948

NOTE:
IF SIGN IS LOCATED ON SIDEWALK, THEN IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO INSTALL A PVC SLEEVE INTO THE CONCRETE WHEN THE SIDEWALK IS POURED PRIOR TO INSTALLATION OF THE SIGN.

DAL - MANUFACTURING

MUTCD PANEL - UPPERCASE

TOP VIEW SCALE: ¾" = 1'-0"



H1-4 D/F STOP/DNE POST AND PANEL

SCALE: 3/4" = 1'-0"

FOUR (4) REMOVALS REQUIRED - MANUFACTURE AND INSTALL 4 SQ. FT.

G.C. TO REMOVE TWO (2) D/F STOP/DNE, AND TWO (2) S/F STOP SIGNS FROM PARKING LOT LANDSCAPING.

MANUFACTURE AND INSTALL FOUR (4) NEW STOP/DNE SIGNS.

EXISTING CONDITIONS

DFW MANUFACTURING / NEW VIS 2022 MUTCD PANEL

Design # 0637033Ar3
Sheet 23 of 27
Client #2859
Address 690 NW Blue Pkwy, Lees Summit, MO
Account Rep. KRISTEN HAMILTON, TERRI BROWN
Designer LEAH LANSFORD
Date 9/29/23

Approval / Date

Client	
Sales	
Estimating	
Art	
Engineering	
Landlord	

Revision/Date

R1(12-22-23)LL: CHANGED SIZE OF PANELS ON THE STOP/DNE

R2(1/16/24)LL: ADDED STAMPED ENGINEERING

CORRECTION(01/17/2024)AM: REVISED DETAIL SECTION AND ADDED THE WORD G.C. IN FRONT OF REMOVALS FOR SIGNS B1-2, C, D, G1-2 AND H1-4

R3(02/26/2024)AM: UPDATED SP, ADDED NOTE FOR SIGN A FOR CSI TO PROVIDE NEW STEEL AND FOUNDATION AND ADDED NOT FOR SIGN D FOR CSI TO REMOVE AND DISCARD EXISTING D/F DIRECTIONAL

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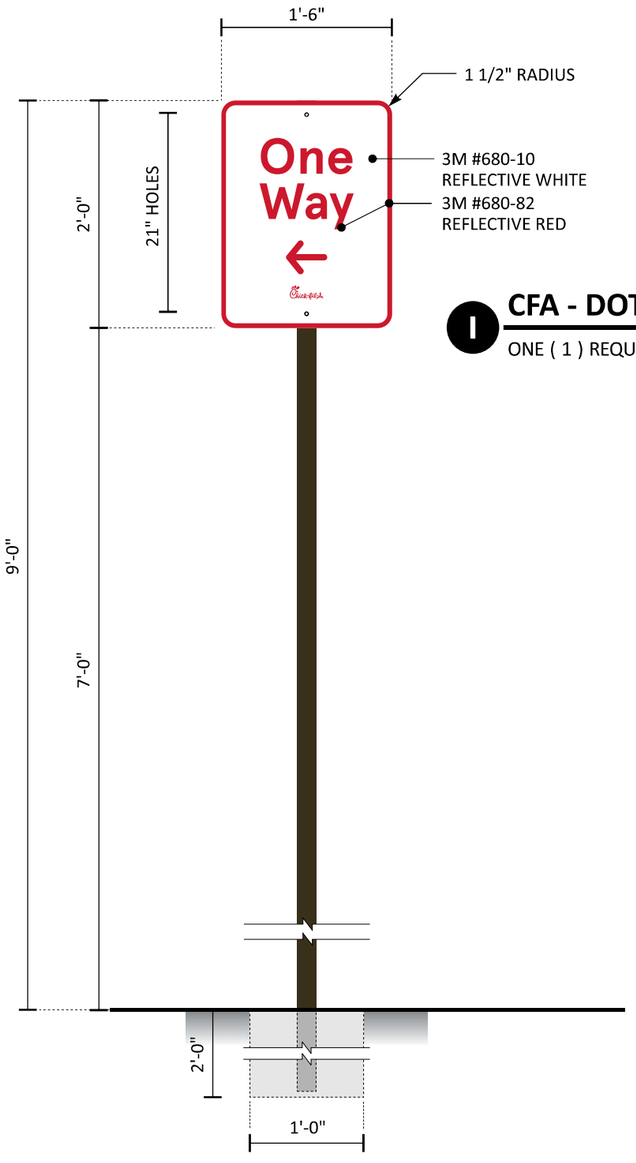
Chick-fil-A

H1-4

DAL - MANUFACTURING

NEW CFA WAYFINDING PANELS



I CFA - DOT S/F DRIVE THRU WITH ARROW POST AND PANEL SCALE: 3/4" = 1'-0"

ONE (1) REQUIRED - MANUFACTURE AND INSTALL 7 SQ. FT.

SPECIFICATIONS

SIGN PANEL
.080 ALUMINUM PANEL WITH BACK PAINTED MATTHEWS
#20181 DK. BRONZE, SATIN AND REFLECTIVE VINYL
GRAPHICS APPLIED 1ST SURFACE.

SIGN PANEL IS TO BE SECURED TO SIGN POST WITH NUTS AND BOLTS PAINTED TO MATCH VINYL BACKGROUND

SIGN POST
2" x 2" x .125"(WALL) ALUMINUM SQ. TUBE, CAPPED ON TOP WITH
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DARK BRONZE, SATIN.
CAP IS A BUYOUT FROM MCMASTER CARR #9565K31
INVENTORY #02948

NOTE:
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DAL - MANUFACTURING

MUTCD PANEL - UPPERCASE

DFW MANUFACTURING / NEW VIS 2022 MUTCD PANEL

Design #	**0637033Ar3**
Sheet	24 of 27
Client	#2859
Address	690 NW Blue Pkwy, Lees Summit, MO
Account Rep.	KRISTEN HAMILTON, TERRI BROWN
Designer	LEAH LANSFORD
Date	9/29/23

Approval / Date

Client	
Sales	
Estimating	
Art	
Engineering	
Landlord	

Revision/Date

R1(12-22-23)LL: CHANGED SIZE OF PANELS ON THE STOP/DNE

R2(1/16/24)LL: ADDED STAMPED ENGINEERING

CORRECTION(01/17/2024)AM: REVISED DETAIL SECTION AND ADDED THE WORD G.C. IN FRONT OF REMOVALS FOR SIGNS B1-2, C, D, G1-2 AND H1-4

R3(02/26/2024)AM: UPDATED SP, ADDED NOTE FOR SIGN A FOR CSI TO PROVIDE NEW STEEL AND FOUNDATION AND ADDED NOT FOR SIGN D FOR CSI TO REMOVE AND DISCARD EXISTING D/F DIRECTIONAL

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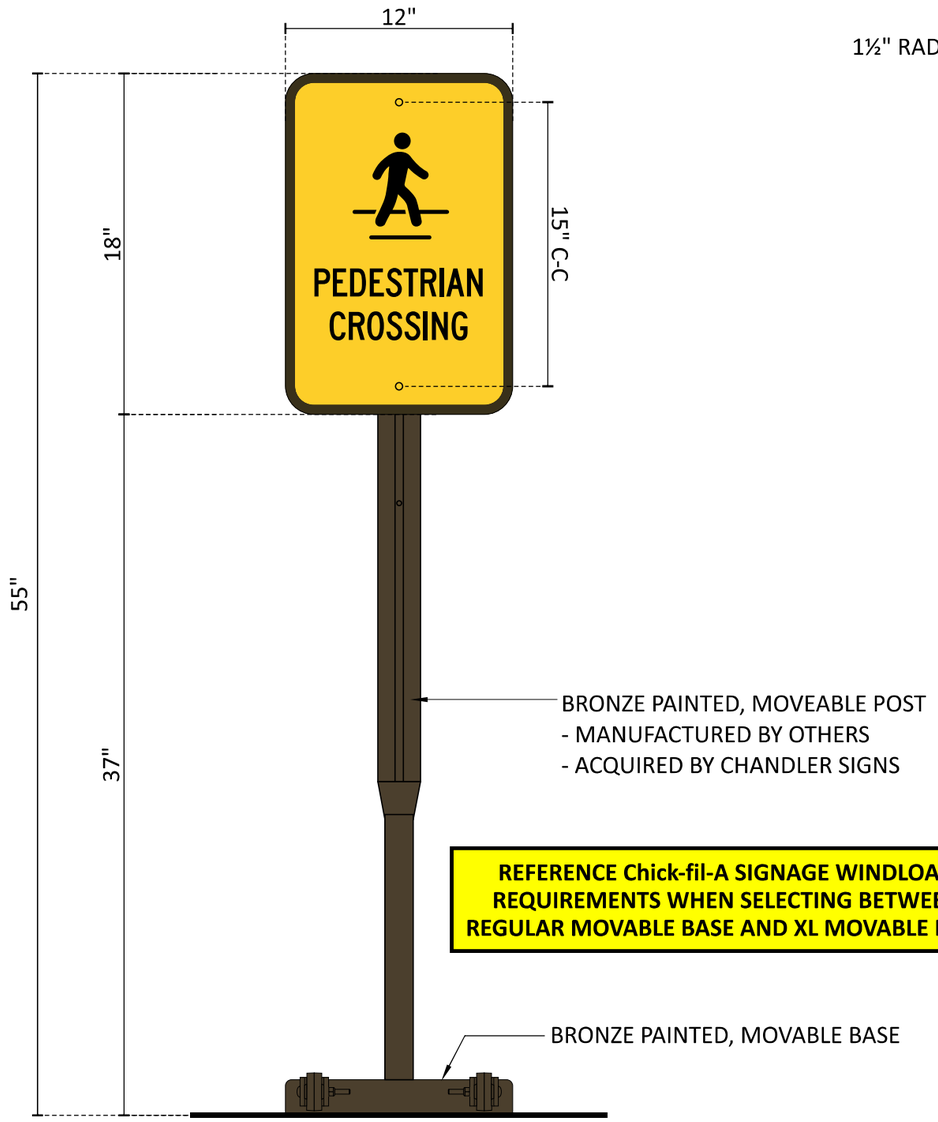
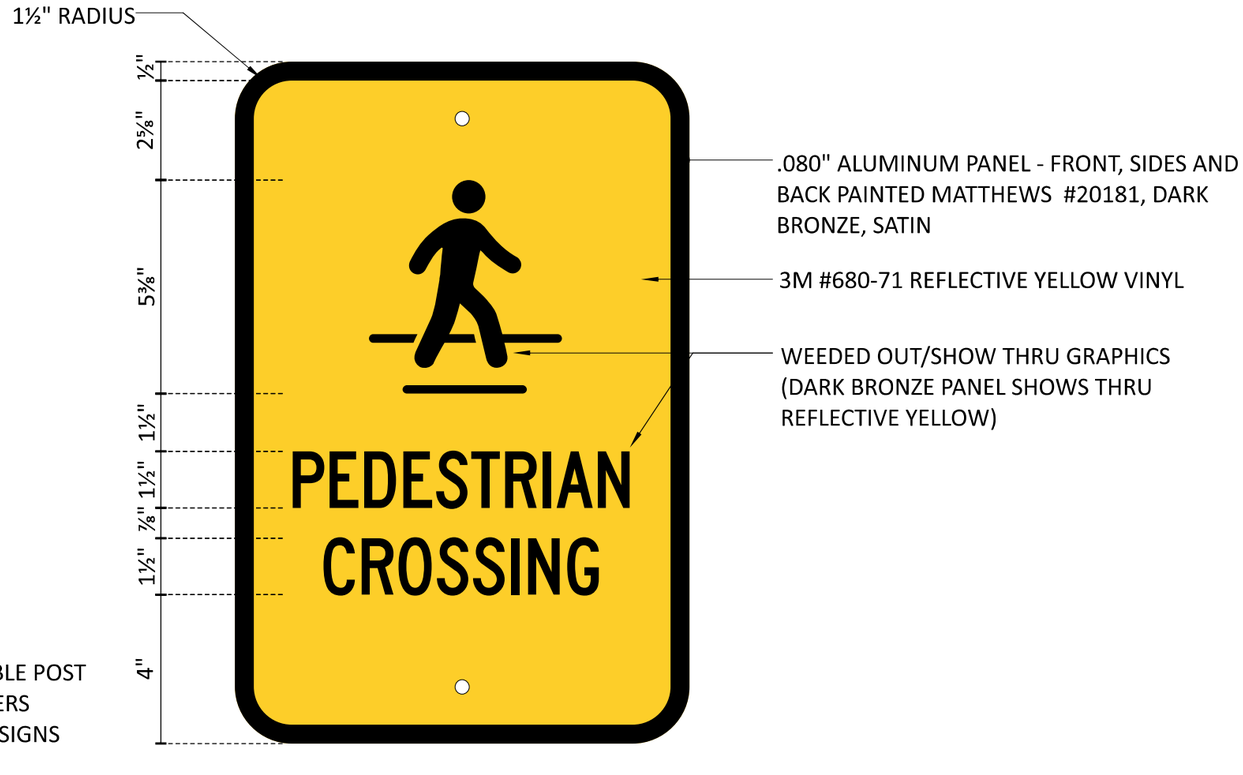
Chick-fil-A

I





PANEL DETAIL - PEDESTRIAN CROSSING
SCALE: 3" = 1'-0"

J **SAFETY SIGN - PEDESTRIAN CROSSING** SCALE: 1-1/2" = 1'-0"
ONE (1) REQUIRED - MANUFACTURE & INSTALL

PLEASE NOTE: PORTABLE BASE AND POST PROVIDED BY PATTISON.

DFW MANUFACTURING / NEW VIS 2022 MUTCD PANEL

Design #	**0637033Ar3**
Sheet	25 of 27
Client	#2859
Address	690 NW Blue Pkwy, Lees Summit, MO
Account Rep.	KRISTEN HAMILTON / TERRI BROWN
Designer	LEAH LANSFORD
Date	9/29/23

Approval / Date

Client	
Sales	
Estimating	
Art	
Engineering	
Landlord	

Revision/Date

R1(12-22-23)LL: CHANGED SIZE OF PANELS ON THE STOPDINE

R2(1/16/24)LL: ADDED STAMPED ENGINEERING

CORRECTION(01/17/2024)AM: REVISED DETAIL SECTION AND ADDED THE WORD G.C. IN FRONT OF REMOVALS FOR SIGNS B1-2, C, D, G1-2 AND H1-4

R3(02/26/2024)AM: UPDATED SP, ADDED NOTE FOR SIGN A FOR CSI TO PROVIDE NEW STEEL AND FOUNDATION AND ADDED NOT FOR SIGN D FOR CSI TO REMOVE AND DISCARD EXISTING D/F DIRECTIONAL

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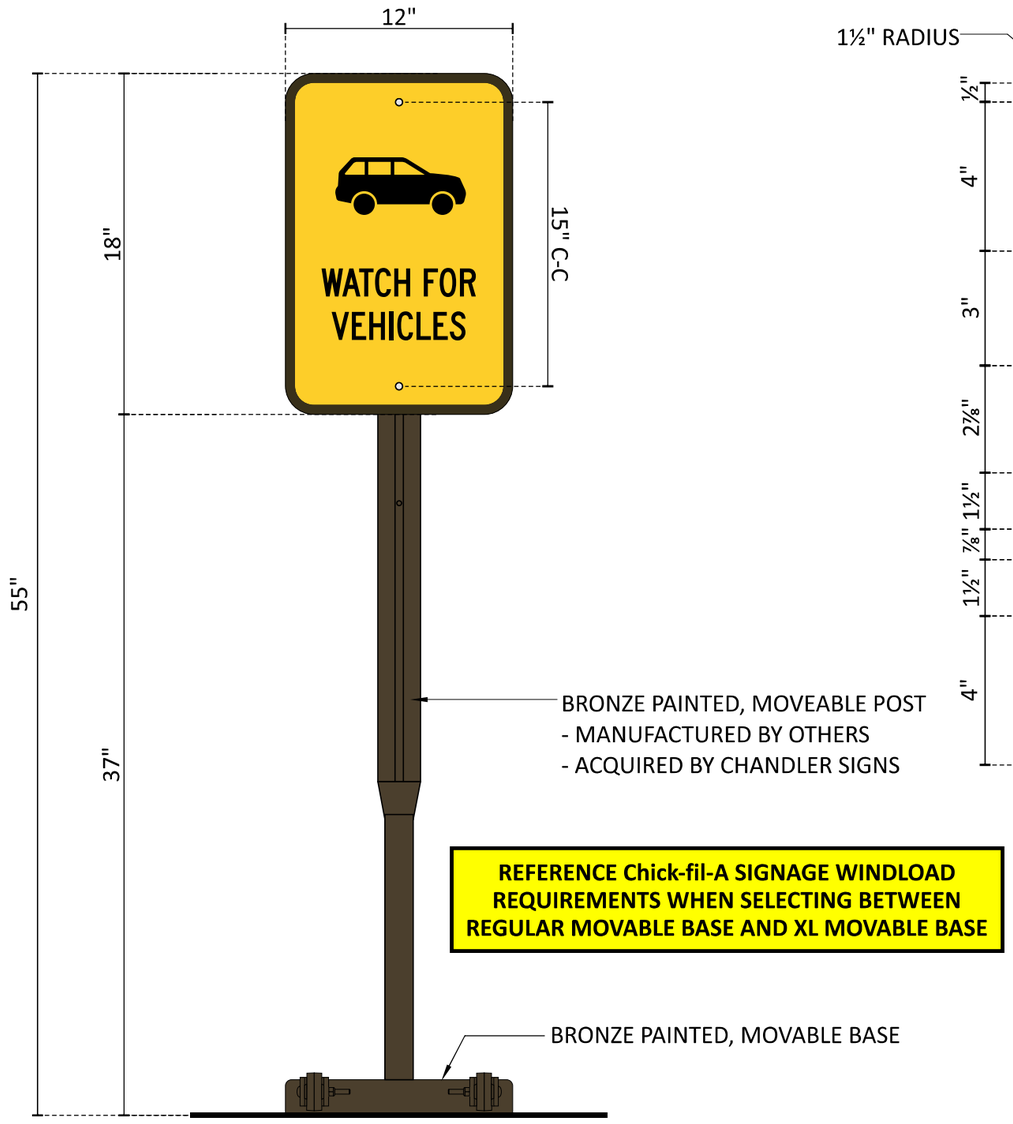
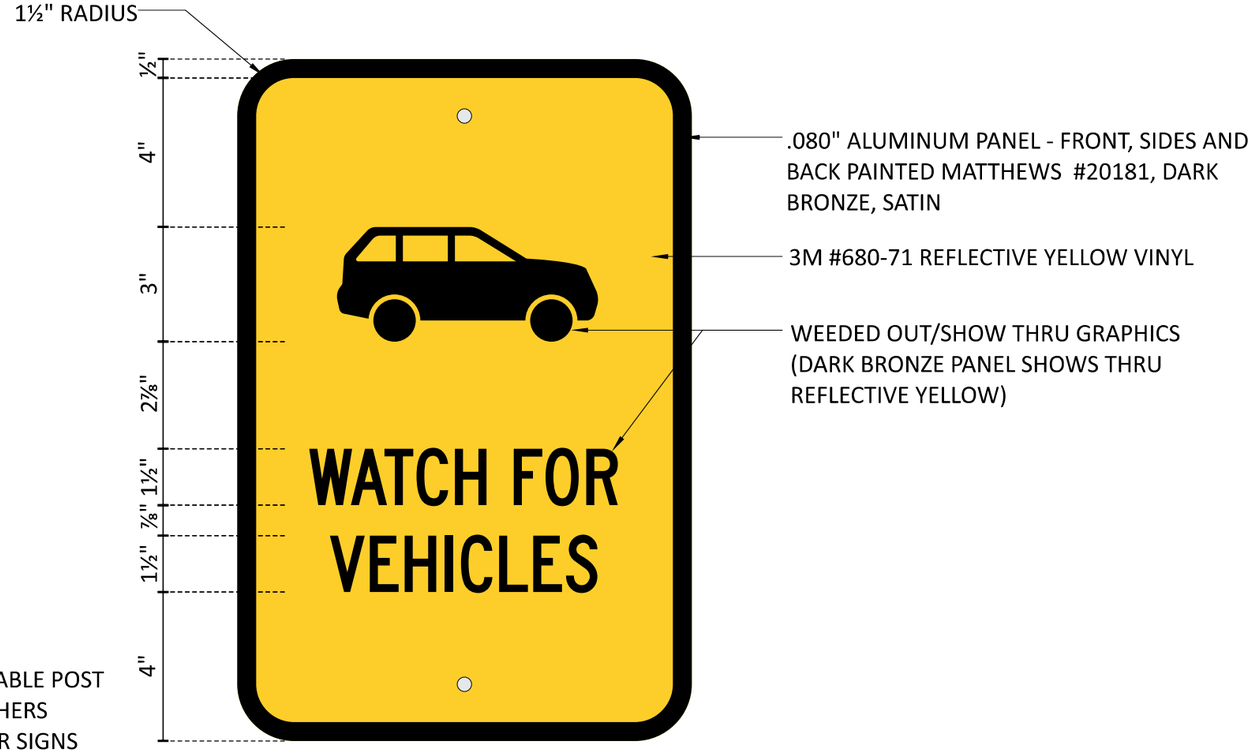
Chick-fil-A

J





PANEL DETAIL - WATCH FOR VEHICLES
SCALE: 3" = 1'-0"

K **SAFETY SIGN - WATCH FOR VEHICLES** SCALE: 1-1/2" = 1'-0"
ONE (1) REQUIRED - MANUFACTURE & INSTALL

PLEASE NOTE: PORTABLE BASE AND POST PROVIDED BY PATTISON.

DFW MANUFACTURING / NEW VIS 2022 MUTCD PANEL

Design #	**0637033Ar3**
Sheet	26 of 27
Client	#2859
Address	690 NW Blue Pkwy, Lees Summit, MO
Account Rep.	KRISTEN HAMILTON, TERRI BROWN
Designer	LEAH LANSFORD
Date	9/29/23

Approval / Date

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Landlord	

Revision/Date

R1(12-22-23)LL: CHANGED SIZE OF PANELS ON THE STOPDINE

R2(1/16/24)LL: ADDED STAMPED ENGINEERING

CORRECTION(01/17/2024)AM: REVISED DETAIL SECTION AND ADDED THE WORD G.C. IN FRONT OF REMOVALS FOR SIGNS B1-2, C, D, G1-2 AND H1-4

R3(02/26/2024)AM: UPDATED SP, ADDED NOTE FOR SIGN A FOR CSI TO PROVIDE NEW STEEL AND FOUNDATION AND ADDED NOT FOR SIGN D FOR CSI TO REMOVE AND DISCARD EXISTING D/F DIRECTIONAL

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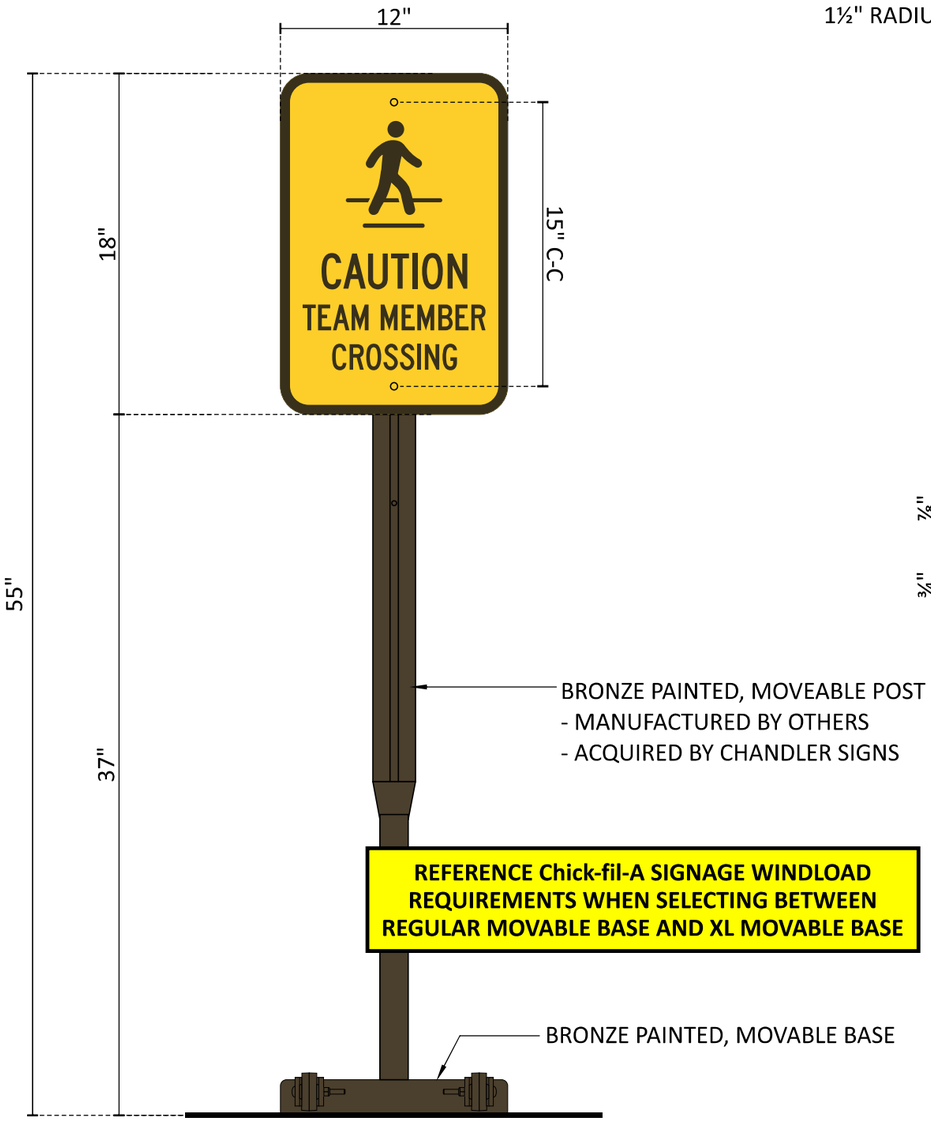
Chick-fil-A

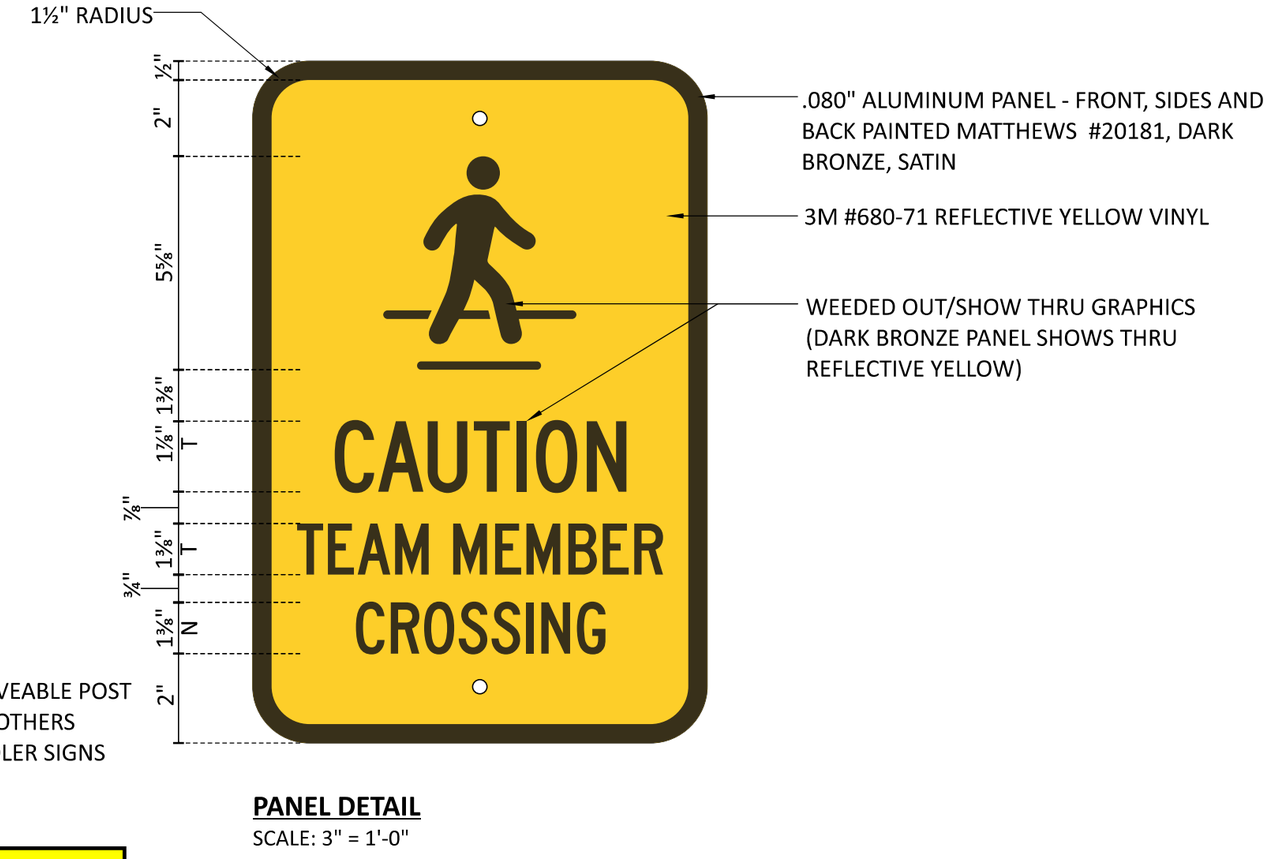
K



L1 **L2** **SAFETY SIGN - TEAM MEMBER CROSSING** SCALE: 1-1/2" = 1'-0"

TWO (2) REQUIRED - MANUFACTURE & INSTALL

PLEASE NOTE: PORTABLE BASE AND POST PROVIDED BY PATTISON.



PANEL DETAIL

SCALE: 3" = 1'-0"

DocuSigned by:
Michael Young
0DCEC3135B2844E...

DFW MANUFACTURING / NEW VIS 2022 MUTCD PANEL

Design #	**0637033Ar3**
Sheet	27 of 27
Client	#2859
Address	690 NW Blue Pkwy, Lees Summit, MO
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Designer	LEAH LANSFORD
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CORRECTION(01/17/2024)AM: REVISED DETAIL SECTION AND ADDED THE WORD G.C. IN FRONT OF REMOVALS FOR SIGNS B1-2, C, D, G1-2 AND H1-4

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L1-2