



	FASTENER SCHEDULE FOR STRUCTURAL MEMBERS		
DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION	
	ROOF ¹		
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOE NAIL	4-8d (2 ½ " x 0.113")	TOENAIL	
CEILING JOISTS TO PLATE, TOE NAIL	4-8d (2½" x 0.113")	PER JOIST, TOENAIL	
CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS, FACE NAIL	4-10d (3" x 0.128")	FACE NAIL	
CEILING JOIST TO PARALLEL RAFTER (HEEL JOINT)	TBLE R802.5.2	FACE NAIL	
COLLAR TIE TO RAFTER, FACE NAIL OR 1 ½ × 20 GA. RIDGE STRAP TO RAFTER	4-10d (3" x 0.128")	FACE NAIL, EACH RAFTER	
RAFTER OR ROOF TRUSS TO PLATE	3-16d BOX NAILS (3½" x 0.135") OR 3-10d COMMON NAILS (3" x 0.148")	2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS	
ROOF RAFTERS TO RIDGE, VALLEY, OR HIP RAFTERS OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM	4-16d (3 ½" x 0.135") - TOENAIL; 3-16d BOX (3 ½" x 0.135") - END NAIL	TOENAIL, END NAIL	
	WALL		
STUD TO STUD (NOT AT BRACED WALL PANELS)	10d (3" x 0.128")	16" O.C. FACE NAIL	
STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS)	16d (3 ½ " x 0.135")	12" O.C. FACE NAIL	
BUILT-UP HEADER, TWO PIECES WITH ½" SPACER	16d (3 ½ " x 0.135")	12" O.C. EACH EDGE FACE NAIL	
CONTINUOUS HEADER TO STUD	4-8d (2 ½ " x 0.131")	TOENAIL	
TOP PLATE TO TOP PLATE	10d (3" x 0.128")	12" O.C. FACE NAIL	
DOUBLE TOP PLATE SPLICE	8-16d COMMON (3 $\frac{1}{2}$ " x 0.162")	FACE NAIL ON EACH SIDE OF END JOINT (MIN. 24 LAP SPLICE LENGTH EACH SIDE OF END JOINT)	
BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (NOT AT BRACED WALL PANELS)	16d COMMON (3 ½" x 0.162")	16" O.C. FACE NAIL	
BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (AT BRACED WALL PANEL)	3-16d BOX (3 $\frac{1}{2}$ " x 0.135")	3 EACH 16" O.C. FACE NAIL	
TOP OR SOLE PLATE TO STUD, END NAIL	4-8d BOX (2 $\frac{1}{2}$ " x 0.113") - TOENAIL; 3-16d BOX (3 $\frac{1}{2}$ " x 0.135") - END NAIL	TOENAIL, END NAIL (SEE LEFT)	
TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	3-10d BOX (3" x 0.128")	FACE NAIL	
1" BRACE TO EACH STUD AND PLATE	3-8d BOX (2 ½" x 0.113")	FACE NAIL	
1"x6" SHEATHING TO EACH BEARING	3-8d BOX (2 ½" x 0.113")	FACE NAIL	
1"x8" SHEATHING TO EACH BEARING	3-8d BOX (2 $\frac{1}{2}$ " x 0.113") - FACE NAIL; WIDER THAN 1"x8" - 4-8d BOX (2 $\frac{1}{2}$ " x 0.113")	FACE NAIL	
	FLOOR		
JOIST TO SILL, TOP PLATE, OR GIRDER	4-8d BOX (2 ½" x 0.113")	TOE NAIL	
RIM JOIST, BAND JOIST, OR BLOCKING TO SILL OR TOP PLATE (ROOF APPLICATIONS ALSO)	8d BOX (2 ½" x 0.113")	4" O.C. TOE NAIL	
1" x 6" SUBFLOOR OR LESS TO EACH JOIST	3-8d BOX (2 ½" x 0.113")	FACE NAIL	
2" SUBFLOOR TO JOIST OR GIRDER	3-16d BOX (3 ½" x 0.135")	BLIND AND FACE NAIL	
2" PLANKS (PLAN & BEAM - FLOOR AND ROOF)	3-16d BOX (3 ½" x 0.135")	AT EACH BEARING, FACE NAIL	
BAND OR RIM JOIST TO JOIST	3-16d COMMON (3 ½" x 0.162")	END NAIL	
BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS	10d BOX (3" x 0.128")	24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES	
LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	4-16d BOX (3 ½" x 0.135")	AT EACH JOIST OR RAFTER, FACE NAIL	
BRIDGING OR BLOCKING TO JOIST	2-10d BOX (3" x 0.128")	EACH END, TOENAIL	
		<u>. </u>	

CRIPTION OF BUILDING MATERIALS	FASTNER SCHEDULE FOR DESCRIPTION OF FASTENER	EDGE SPACING (INCHES)	INTERMEDIATE SUPPORTS (INCHE
WOOD STRUCTURAL PANELS, SUE	BFLOOR, ROOF AND INTERIOR WALL SHEA	ATHING TO FRAMING AND PARTICLEBO	ARD WALL SHEATHING TO FRAMING
¾" - ½"	6d COMMON (2" x 0.113") NAIL (SUBFLOOR, WALL) 8d COMMON NAIL (ROOF)	6	12
¹⁹ % 32" - 1"	8d COMMON NAIL (2½" x 0.131")	6	12
11/8" - 11/4"	10d COMMON (3" x 0.148") NAIL OR 8d (2 ½ " x 0.131") DEFORMED NAIL	6	12
	OTHER WALL	SHEATHING 1	
½" STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	$1\frac{1}{2}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR $1\frac{1}{4}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6
25" STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	$1\frac{3}{4}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR $1\frac{1}{2}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6
½" GYPSUM SHEATHING	1½" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1½" SCREWS, TYPE W OR S	7	7
%" GYPSUM SHEATHING	1¾" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1¾" LONG; 1¾" SCREWS, TYPE W OR S	7	7
wo	OOD STRUCTURAL PANELS, COMBINATION	N SUBFLOOR UNDERLAYMENT TO FRAM	I ING
¾ " AND LESS	6d DEFORMED (2" x 0.120") NAIL OR 8d COMMON (21/2" x 0.131") NAIL	6	12
7∕8" - 1"	8d COMMON (2½" x 0.131") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12
11/8" - 11/4"	10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (2 ½ " x 0.120") NAIL	6	12

 IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION LISTED IN THIS TABLE

FOUNDATION NOTES

- 1. CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLABS
- . THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION STANDARDS
- 3. PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A
- MINIMUM 20 GALLON SUMP PIT.

 4. FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY
- SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.

 5. FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40

 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE
- 6. CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE
- 7. FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0

 8. REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE)
- INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND,
- BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND, GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY
- VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES

 11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB
- SHALL BE DESIGNED BY A LICENSED ENGINEER

 12. SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH ½" Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS)

AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6

14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

13. FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET

FRAMING NOTES

- 15. ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS
 16. ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 2x10's, UNLESS NOTED OTHERWISE
- 17. BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS
- 18. INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A
- 19. ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED
- 20. WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS.
- SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS.

 21. ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT
- 22. JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.
- 23. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS
- 24. JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 1/8" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT
- 25. ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.326. ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.
- 27. ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER 1/3 OF VERTICAL DISTANCE BETWEEN CEILING AND
- ROOF
 28. BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED
- 29. PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A ½" GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION)
- 30. ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi
- 31. ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi
 32. COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND
 THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM
 FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. ½" x 2" BOLTS SHALL THEN BE
 INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE
 WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE
 INSPECTED BY AN AWS-CERTIFIED INSPECTOR.
- 33. WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING.
- 34. ALL ROOF SHEATHING SHALL BE $\frac{7}{16}$ " OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

- 35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0"
- 36. ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

37. ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH ½" TO ½" OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN ½0 OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

DETECTORS OUTSIDE EACH SLEEPING AREA.

LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC 2018

38. PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP.

39. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF

MASONRY VENEER

40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1½", WITH NOT LESS

ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE

- THAN 5/8" MORTAR OR GROUT COVER TO OUTSIDE FACE.

 41. VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 7/8"
- CORRUGATED.

 42. EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE
- THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.

 43. VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

GARAGE NOTES

DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1¾" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE
 VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST

GARAGE NOTES (CONTINUED)

- THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 5/8" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM 5/8" TYPE X GYP. BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS
- SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH %" GYP. BOARD.

 45. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER
 BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM
 FLOOR TO CEILING AND SHALL BE FASTENED WITH 2½"" x 0.120" NAILS AT 7" O.C.

 STAGGERED WITH (7) 3¼" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER.
 MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

DESIGN LOADING (PER TABLE R301.5)

MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS (PSF)								
USE	LIVE LOAD	DEAD LOAD						
UNINHABITABLE ATTICS WITHOUT STORAGE	10	10						
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20	10						
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30	10						
BALCONIES (EXTERIOR) AND DECKS	40	10 ^d						
FIRE ESCAPES	40	10						
GUARDRAILS AND HANDRAILS ^a	200°	-						
GUARDRAIL IN-FILL COMPONENTS ^b	50 ^c	-						
PASSENGER VEHICLE GARAGES	50	DEPENDENT UPON SLAB CONSTRUCTION						
ROOMS OTHER THAN SLEEPING ROOM	40	10 ^d						
SLEEPING ROOM	30	10 ^d						
STAIRS	40	10 ^d						

a. A single concentrated load applied in any direction at any point along the top.
b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement.
c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load.
d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed.

INSULATION/EFFICIENCY

- 1. BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1.

INSULATION AND FENESTRATION REQUIRE	
CLIMATE ZONE	4-A
FENESTRATION U-FACTOR	0.35
SKYLIGHT U-FACTOR	0.55
GLAZED FENSTRATION SHGC	0.40
CEILING R-VALUE	49
WOOD FRAME WALL R-VALUE	15
MASS WALL R-VALUE	8 / 13
FLOOR R-VALUE	19
BASEMENT WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
SLAB R-VALUE AND DEPTH	10 AT 2'-0"
CRAWL SPACE WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	8
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	6
CATHEDRAL VAULTED CEILING R-VALUE	38

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2018 IRC. EXCEPTIONS:

- 1. AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT SEALS.
- 2. WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.

DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:

- POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED
- OR OTHERWISE SEALED DURING THE TEST.
 ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100
 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE
 DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE
 MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR
 OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE
 TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100
 SQUARE FEET OF CONDITIONED FLOOR AREA.

EXCEPTION: THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

MECHANICAL VENTILATION SYSTEM FAN EFFICACY							
FAN LOCATION	AIR FLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY (CFM/WATT)	AIR FLOW RATE MAXIMUM (CFM)				
RANGE HOODS	ANY	2.8	ANY				
IN-LINE FAN	ANY	2.8	ANY				
BATHROOM, UTILITY ROOM	10	1.4	90				
BATHROOM, UTILITY ROOM	90	2.8	ANY				

MULTIPLE-PLY WOOD BEAM FASTENING SCHEDULE									
DIMENSIONAL LUMBER BEAM SIZE/TYPE	FASTENERS	LVL BEAM SIZE/TYPE	FASTENERS	LVL BEAM SIZE/TYPE	FASTENERS				
(2) 2x	(2) ROWS 10d @ 12" O.C. ONE SIDE	(2) 1 $\frac{3}{4}$ " UP TO 11 $\frac{7}{8}$ " DEPTH	(2) ROWS 16d @ 12" O.C. ONE SIDE	(3) 1 ¾" x 14"+ DEPTH	(3) ROWS 16d @ 12" O.C. BOTH SIDES				
(3) 2x	(2) ROWS 10d @ 12" O.C. BOTH SIDES	(2) 1 ¾" 14"+ DEPTH	(3) ROWS 16d @ 12" O.C. ONE SIDE	(4) 1 $rac{3}{4}$ " UP TO 11 $rac{7}{8}$ " DEPTH	(2) ROWS ¼" x 5" SIMPSON SDS OR SDWS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM BOTH SIDES				
(4) 2x	(2) ROWS ¼" x 5" SIMPSON SDS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM, BOTH SIDES	(3) 1 $\frac{3}{4}$ " UP TO 11 $\frac{7}{8}$ " DEPTH	(2) ROWS OF 16d @ 12" O.C. BOTH SIDES	(4) 1 ¾" x 14"+ DEPTH	(3) ROWS ¼" x 5" SIMPSON SDS OR SDWS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM BOTH SIDES				



RBERG AT AT HOOK FARMS

DILLE. KALTIOU SPELLER LOT 186, RETREA

DENNIS HEIER

NUMBER

PE-2010001772

NO.	DATE	RE	VISION		BY
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JOB N	10.		DRAWN E	BY: D	МН
DATE	: 12-11	-23			



RESIDENTIAL SEISMIC & WIND ANALYSIS

DETERMINE WEIGHT OF HOUSE:				INPUT CALCULATED VALUE
LOCATION		DEAD LOAD (psf)	AREA (ft ²)	WEIGHT (lbs.)
ROOF		10	3238	32380
CEILING		10	3238	32380
SECOND FLOOR		10	794	7940
FIRST FLOOR		10	3238	32380
	WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WEIGHT (lbs)
SECOND FLOOR EXT. WALL DL	166	8	8	10624
FIRST FLOOR EXT. WALL DL	241	10	10	24100
		DEAD LOAD (psf)	AREA (ft2)	WEIGHT (lbs)
SECOND FLOOR INT. PARTITION WALL DL		6	794	4764
FIRST FLOOR INT. PARTITION WALL DL		6	3238	19428

PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED)								
FRONT-TO-BACK				SIDE-TO-SIDE				
	AREA	LOAD			AREA	LOAD		
SLOPED ROOF	262	1134		SLOPED ROOF	300	1321		
VERT. ROOF	0	0	CUMULATIVE	VERT. ROOF	0	0	CUMULATIVE	
2ND	411.03	5984	7117	2ND	335.97	5110	6431	
1ST	689.37	9490	16607	1ST	636.13	8870	15301	
BSMT ^a	0	0	0	BSMT ^a	132	2297	9947	
			PRESSURE (PSF) - PER ASCE CH. 6				
	SLOPED ROOF	ZONE B		5.9	ZONE C	11.6	2a (FIG. 28.6-1, ASCE7)	
	WALL/VERT. ROOF	ZONE A		17.4	ZONE D	3.4	11.566	
	MEAN ROOF HT., h		30					

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area. q_{z10} =0.00256 $K_z K_{zt} K_d V^2$ (ASCE7-10 Velocity Pressure) q_{z10_ASD}=0.6q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)

2ND FLOOR TRIBUTARY WEIGHT 1ST FLOOR TRIBUTARY WEIGHT BASEM S_S (SITE F_a (from

EXTERIOR SHEATHING OPTION FOR SECOND FLOOR

 S_{DS} (= 2 R (from

2ND FLOOR

TION	From ASCE7 (Eq. 12.8-1):	V (= 1.2 * S _{DS} * W / F	R) (lbs.)
SEISMIC SHEAR			
m ASCE7 Table 12.2-1)		6.5	
= 2/3 * S _S * F _a)		0.128	
om ASCE7 Table 11.4-1)		1.6	
TE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP)		12.0%	
MENT TRIBUTARY WEIGHT		100138	
LOOK TRIBUTARY WEIGHT		100138	

FLOOR EMENT				2366 2366
Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable Shear (#/LF)	Code Reference
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	155	per IBC, Table 2305.3(1)
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	230	per IBC, Table 2305.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	310	per IBC, Table 2305.3(1)
Exterior (<i>Option #4</i>)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220	AF&PA SDPWS Table 4.3A
Exterior (Option #5)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320	AF&PA SDPWS Table 4.3A
Exterior (<i>Option #6</i>)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410	AF&PA SDPWS Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60	per IBC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)	325	

EXTERIOR SHEATHII	NG OPTION FOR FIRST	T FLOOR	4		WIDTH OF 1ST STORY (FT.)	62.67	WIDTH OF 2ND STORY (FT.)	45.67
EXTERIOR SHEATHII	NG OPTION FOR BASE	MENT WALLS	5		DEPTH OF 1ST STORY (FT.)	57.83	DEPTH OF 2ND STORY (FT.)	37.33
		•			BACK WALL OF GARAGE (FT.)	0		
					GAR. WALL: 1=F-B, 2=S-S	2		
			EVED	IOD OTRUGTURAL MALL	ENOTING (6) A DEGLOTANOES			
				IOR STRUCTURAL WALL I	LENGTHS (ft.) & RESISTANCES			
		SE	ISMIC			WIND		
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)
2ND ELOOP	22	9060	60	16900	22	12544	60	22520

2ND FLOOR 32 8960 60 16800 32 12544 60 23520 ST FLOOR 77 21560 41 11480 77 30184 41 16072 BASEMENT 0 0 24 9120 0 0 24 12768 ADDITIONAL RESISTANCE REQUIRED									
ADDITIONAL RESISTANCE REQUIRED Anchor Bolt Spacing (in.) 16d Nail Spacing req'd at bottom plate (in.) 2nd Floor F-B 4 2nd Floor F-B 5 3nd Floor F-B	2ND FLOOR	32	8960	60	16800	32	12544	60	23520
ADDITIONAL RESISTANCE REQUIRED SEISMIC WIND 2ND FLOOR FRONT-TO-BACK 2ND FLOOR SIDE-TO-SIDE 1ST FLOOR FRONT-TO-BACK 1ST FLOOR SIDE-TO-SIDE BASEMENT FRONT-TO-BACK 0 0 0 16d Nail Spacing req'd at bottom plate (in.) 2nd Floor F-B 2nd Floor F-B 4 2nd Floor F-B 4 2nd Floor S-S 5 2nd Floor F-B 4 2nd Floor S-S 5 2nd Floor F-B 4 2nd Floor S-S 5 2nd Floor S-S 5 2nd Floor S-S 5 2nd Floor S-B 4 2nd Floor S-S 5 2nd Floor S-S 5 2nd Floor S-S 5 2nd Floor S-B 4 2nd Floor S-S 5 2nd Floor	1ST FLOOR	77	21560	41	11480	77	30184	41	16072
SEISMIC WIND diameter (in.) 0.5 2nd Floor F-B 4	BASEMENT	0	0	24	9120	0	0	24	12768
SEISMIC WIND diameter (in.) 0.5 2nd Floor F-B 4	<u> </u>	· · · · · · · · · · · · · · · · · · ·			_		· · · · · · · · · · · · · · · · · · ·		
2ND FLOOR FRONT-TO-BACK 0 0 5 2ND FLOOR SIDE-TO-SIDE 0 0 0 126.2 1st Floor F-B 1st Floor F-B 1 1ST FLOOR FRONT-TO-BACK 0 0 0 148.5 1st Floor S-S 2 1ST FLOOR SIDE-TO-SIDE 0 0 0 0 0 0 BASEMENT FRONT-TO-BACK 0 0 0 0 0 0 0			ADDITIONAL RESIS	TANCE REQUIRED		Anchor Bolt Spacing	(in.)	16d Nail Spacing req'd at	bottom plate (in.)
2ND FLOOR SIDE-TO-SIDE 0 0 Spacing F-B (inches) 126.2 1st Floor F-B 1 1ST FLOOR FRONT-TO-BACK 0 0 spacing S-S (inches) 148.5 1st Floor S-S 2 1ST FLOOR SIDE-TO-SIDE 0 0 0 0 0 0 0 BASEMENT FRONT-TO-BACK 0			SEISMIC	WIND		diameter (in.)	0.5	2nd Floor F-B	4
1ST FLOOR FRONT-TO-BACK 0 0 spacing S-S (inches) 148.5 1st Floor S-S 2 1ST FLOOR SIDE-TO-SIDE 0	2ND FLOOR FRONT-	TO-BACK	0	0		Shear value (per NDS)	944	2nd Floor S-S	5
1ST FLOOR SIDE-TO-SIDE 0 0 BASEMENT FRONT-TO-BACK 0 0	2ND FLOOR SIDE-TO	-SIDE	0	0		Spacing F-B (inches)	126.2	1st Floor F-B	1
BASEMENT FRONT-TO-BACK 0 0	1ST FLOOR FRONT-1	TO-BACK	0	0		spacing S-S (inches	148.5	1st Floor S-S	2
	1ST FLOOR SIDE-TO	-SIDE	0	0					
BASEMENT SIDE-TO-SIDE 0 0	BASEMENT FRONT-T	O-BACK	0	0					
	BASEMENT SIDE-TO-	-SIDE	0	0					

		RESISTANCE REQUIF	RED IN ADDITION TO RES	SISTANCE PROVIDED BY EXTERIOR V	VALLS**		
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)	OK?
2ND FLOOR FRONT-TO-BACK	0				l .	0	YES
2ND FLOOR SIDE-TO-SIDE	0					0	YES
1ST FLOOR FRONT-TO-BACK	0					0	YES
1ST FLOOR SIDE-TO-SIDE	0		#		l .	0	YES
BASEMENT FRONT-TO-BACK	0					0	YES
BASEMENT SIDE-TO-SIDE	0					Ó	YES

2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING

PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER
ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRE

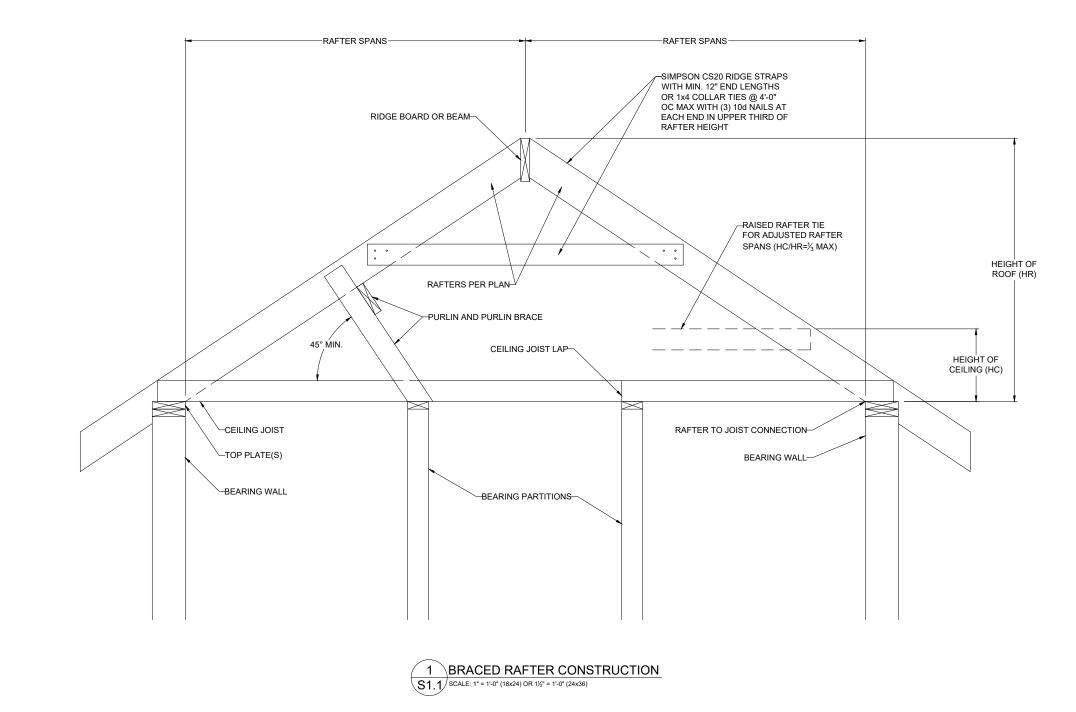
X/12	DECREE		WIND UPLIFT	ANALYSIS		
X/12	DECDEEC					
	DEGREES					
5	22.6	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2			
	ASCE 7					
LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)			
1	16.56	243	16.56			
TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)
3624.2061	1384.866576	2239.339524	15.12	10.5	44452	184.4
*ALONG PERIMETER TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS)				201.0	UPLIFT OK	
**INSIDE EXTERIOR WALLS RESISTANCE DUE TO DEAD WEIGHT & (3) 10d TOENAILS				251.6		
	1 OTAL AREA (FT ²) 3624.2061	ASCE 7 LENGTH (FT.) PRESSURE (PSF) 1 16.56 OTAL AREA (FT²) ZONE E AREA (FT²) 3624.2061 1384.866576 TOTAL UPLIFT PER LINEAL	ASCE 7 LENGTH (FT.) PRESSURE (PSF) LINEAL FT. OF OH 1 16.56 243 OTAL AREA (FT²) ZONE E AREA (FT²) ZONE G AREA (FT²) 3624.2061 1384.866576 2239.339524 TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (PA	ASCE 7 LENGTH (FT.) PRESSURE (PSF) LINEAL FT. OF OH UPLIFT PER FT* (LBS) 1 16.56 243 16.56 OTAL AREA (FT²) ZONE E AREA (FT²) ZONE G AREA (FT²) PRESSURE ZN. E (PSF) 3624.2061 1384.866576 2239.339524 15.12 TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS)	ASCE 7 LENGTH (FT.) PRESSURE (PSF) LINEAL FT. OF OH UPLIFT PER FT* (LBS) 1 16.56 243 16.56 OTAL AREA (FT²) ZONE E AREA (FT²) ZONE G AREA (FT²) PRESSURE ZN. E (PSF) PRESSURE ZN. G (PSF) 3624.2061 1384.866576 2239.339524 15.12 10.5 TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS) 201.0	ASCE 7 LENGTH (FT.) PRESSURE (PSF) LINEAL FT. OF OH UPLIFT PER FT* (LBS) 1 16.56 243 16.56 OTAL AREA (FT²) ZONE E AREA (FT²) ZONE G AREA (FT²) PRESSURE ZN. E (PSF) PRESSURE ZN. G (PSF) TOTAL FORCE (LBS) 3624.2061 1384.866576 2239.339524 15.12 10.5 44452 TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS) 201.0 UPLIFT OK

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING

WITH CONSTRUCTION



8.5 ft

OK

Combustion Air Calculation Per 2018 IRC Section G2407.5 100000 BTU/h Appliance #1 Furnace Appliance #2 50000 BTU/h Appliance #3 Water Heater Total BTU/hr 150000 BTU/h Area of Combined Space (floor where appliances are located)

Note: Per 2018 IRC Section G2407.5.3.2, The volumes of spaces in different stories shall be considered as communicating spaces where such spaces are connected by one or more openings in doors or floors having a total minimum free area of 2 square

inches per 1,000 BTU/h of total input rating of all appliances

Ceiling Height in Usable Space

Is floor where appliances are located open to adjacent level? If Yes, what is the area of open space adjacent to appliance area?

Per 2018 IRC Section G2407.5.1 (Standard Method), the minimum required volume shall be 50 cubic feet per 1,000 BTU/hr

(Total BTU/hr / 1,000 BTU/hr x 50 ft 3)

Area of Combined Space > Required combined area?

7500 ft³ Required air space in combined areas:

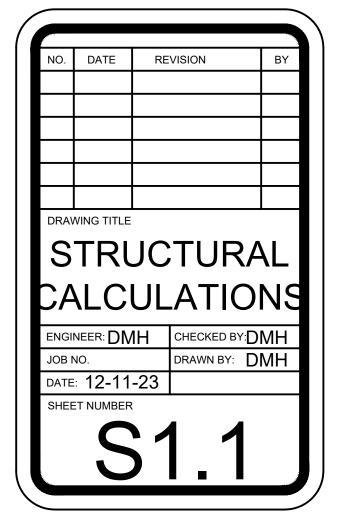
882 ft² Required combined area:

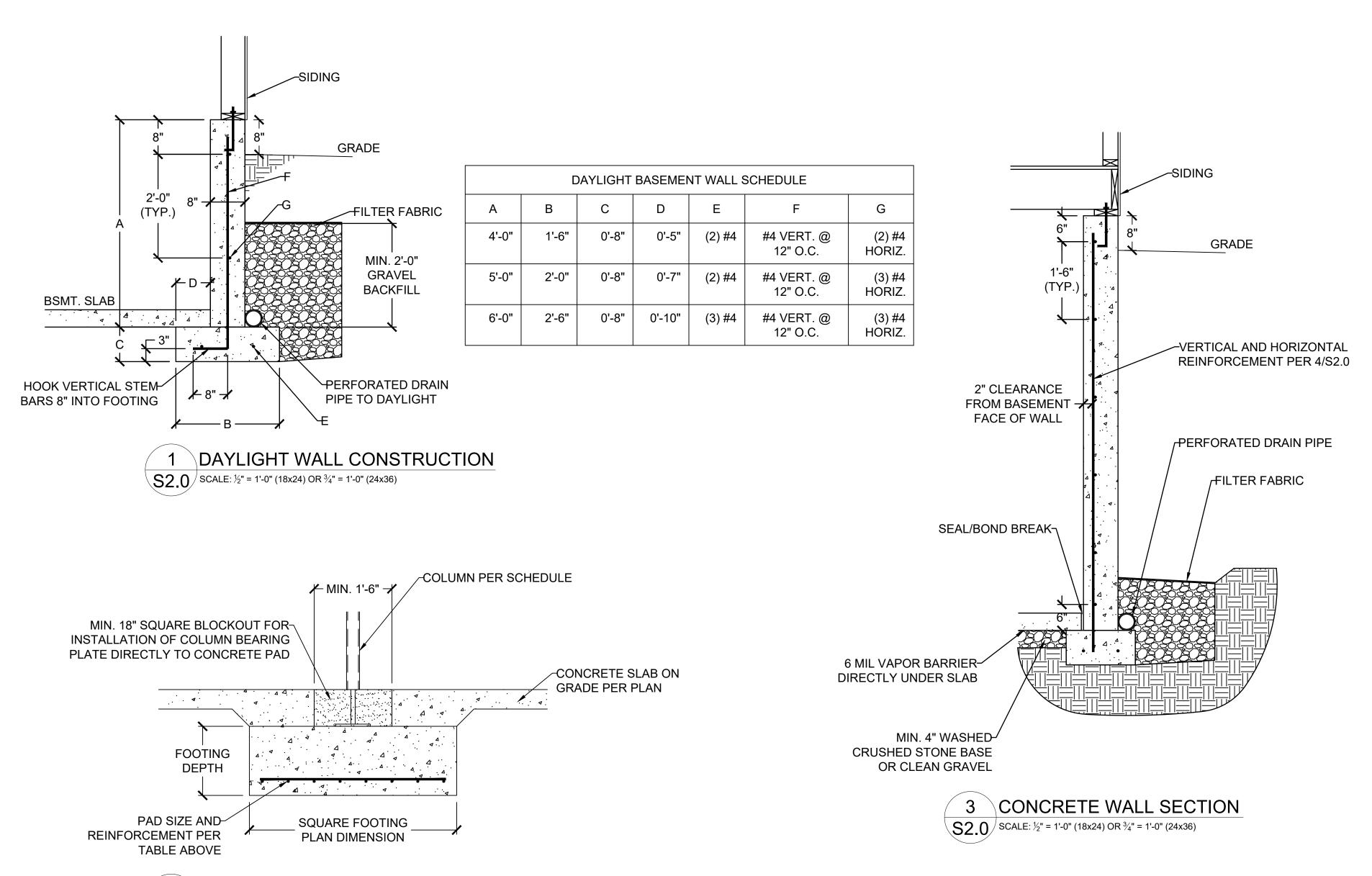
Per Section G2407.5.3.1, each opening shall have a minimum free area of 1 square inch per 1,000 BTU/hr of the total input rating of all appliances in the space, but not less than 100 square inches. One opening shall commence within 12 inches of the top and one opening shall commence within 12 inches of the bottom of the enclosure. The minimum dimension of air openings shall be not less than 3 inches.

150 in² Minmum required opening area: Minimum grill size: 14 x 11 Note: two grills required - one within 12" of floor, one within 12" of clg.









TYPICAL CORNER REINFORCEMENT! - L

AT LEAST (1) #4 BAR 48" LONG @

AS CLOSE AS PRACTICAL TO THE CORNER

EACH INSIDE CORNER

NOTE: WHERE OPENINGS OR ABRUPT ELEVATION

VERTICAL REINFORCEMENT SPACING CONCRETE STRENGTH/GRADE 8" THICK WALL 10" THICK WALL REINFORCEMENT (#4 BARS) 9' 10' 3,000 PSI/ GRADE 40 24 24 24 24 24 3,500 PSI/ GRADE 40 24 3,000 PSI/ GRADE 60 24 24 24 24 18 3,500 PSI/ GRADE 60 24 24 16 24 24 18 HORIZONTAL REINFORCEMENT - MINIMUM GRADE 40 STEEL ONE BAR 12" FROM TOP OF WALL; 6-#4 7-#4 6-#4 7-#4 7-#4 7-#4 MAX. SPACING 24" OC

FOOTNOTES:

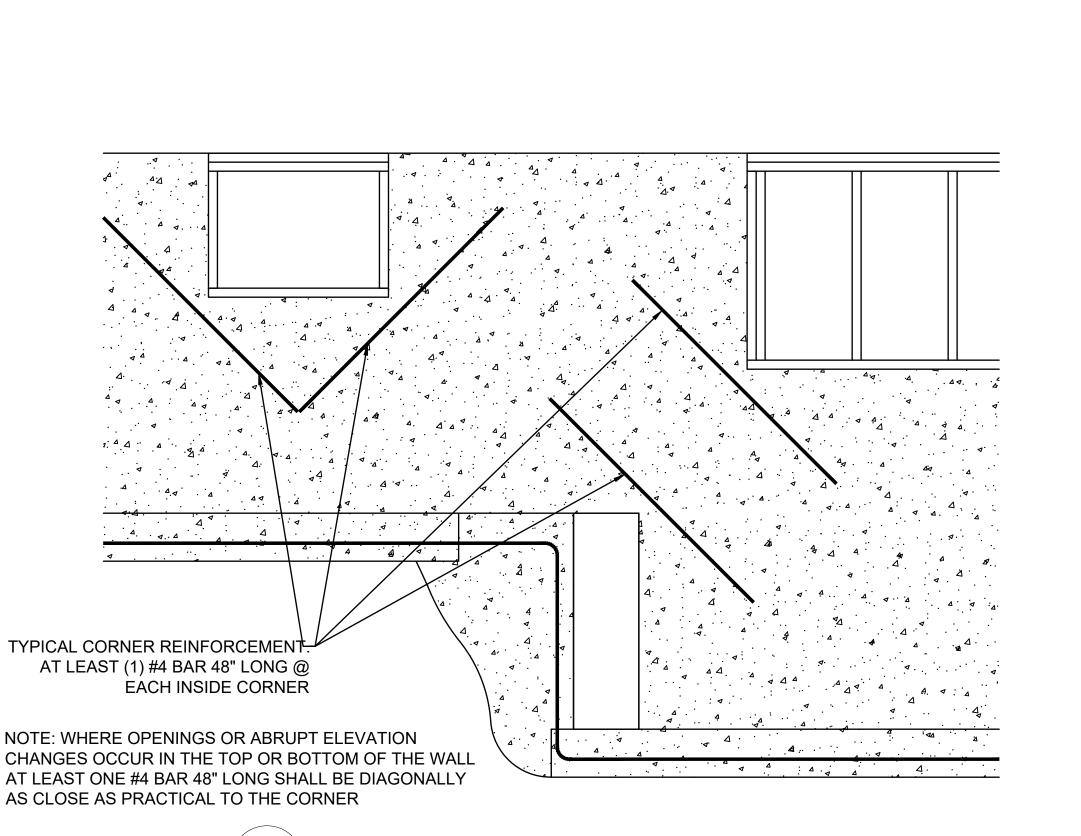
1) WALL HEIGHT IS MEASURED FROM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB 2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE WALL. OTHER WALLS SHALL HAVE VERTICAL REINFORCEMENT AS FOLLOWS:

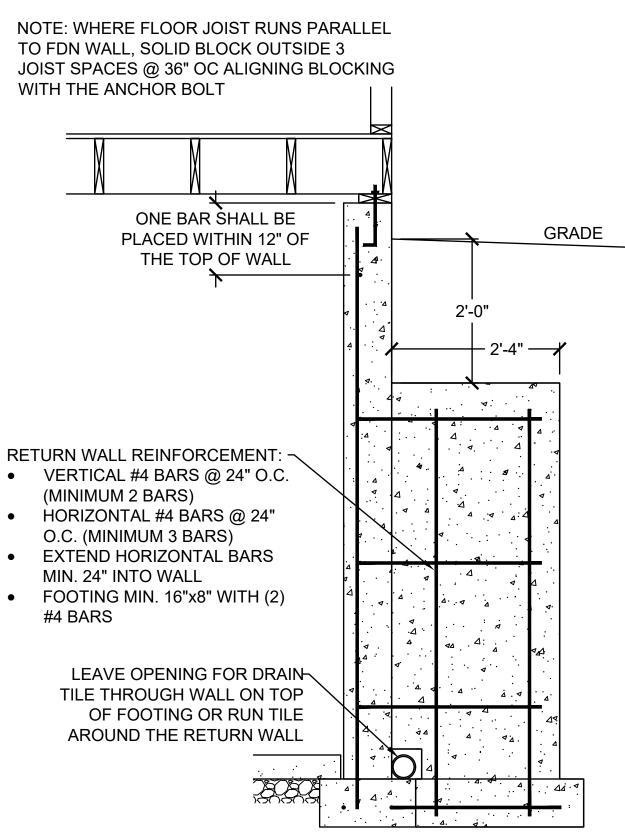
- A) 8" WALL MINIMUM 5" FROM THE OUTSIDE FACE
- B) 10" WALL MINIMUM 63/4" FROM THE OUTSIDE FACE
- C) EXTEND BARS TO WITHIN 8" OF THE TOP OF THE WALL

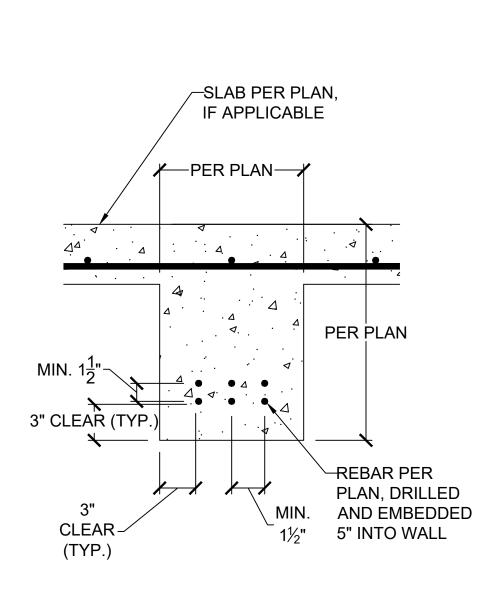
3) REINFORCEMENT CLEARANCES:

- A) CONCRETE EXPOSED TO EARTH MINIMUM 11/2"
- B) NOT EXPOSED TO WEATHER (INTERIOR SIDE OF WALLS) -3/4" C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY
- SLABS) 1½"
- 4) HORIZONTAL REINFORCEMENT:
 - A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC
 - C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE
 - D) SUPPLEMENTAL REINFORCEMENT AT CORNERS PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF THE EDGE OF INSIDE CORNERS.
- 5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND
- 6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE 3½". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP
- 7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS
- WALL SHALL NOT BE BACKFILLED UNTIL FLOOR SYSTEM AND DIAPHRAGM ARE IN PLACE

4 FOUNDATION WALL REINFORCEMENT TABLE S2.0 NO SCALE



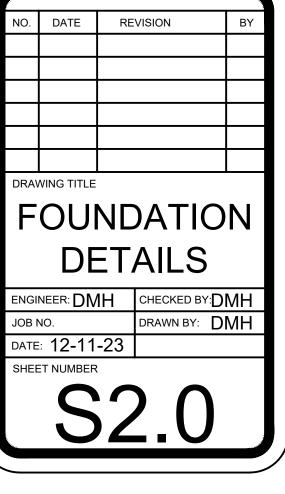




8 CONCRETE GRADE BEAM S2.0 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

RETURN WALL DETAIL $\frac{1}{52.0}$ SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)





5 SOLID JUMP S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

2 COLUMN AND BEARING PAD SCHEDULE

MIN. (2) #4 BARS EXTENDING 24"

PAST OVER-EXCAVATION AND

INTO INTERSECTING WALL

-CONTINUOUS FOOTING

AND REBAR THROUGH

6'-0" MAX.

SOLID JUMP

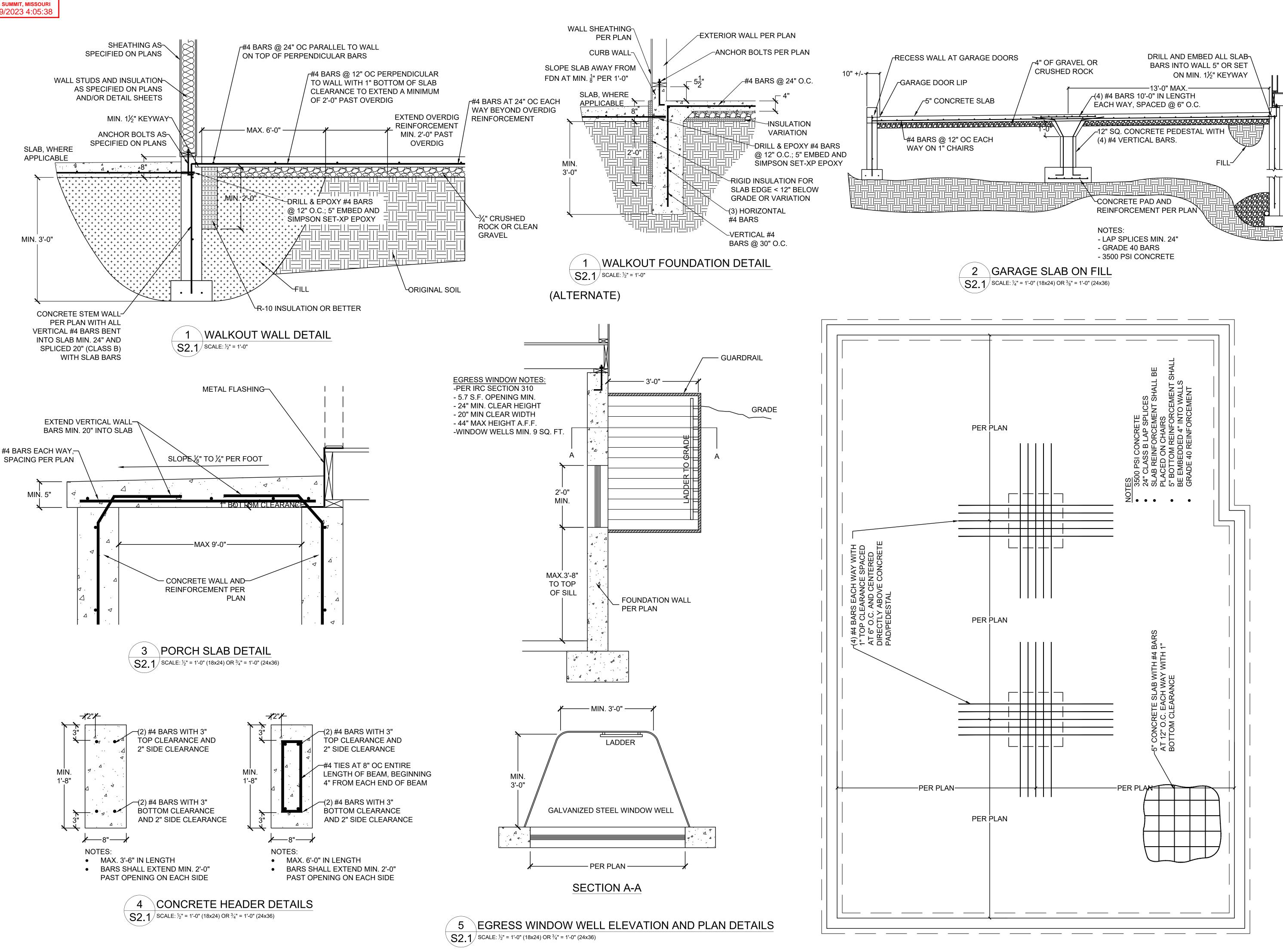
MAX. 12" BLOCKOUT FOR

FORM PLACEMENT AND

TO EXTEND DRAIN TILE

S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

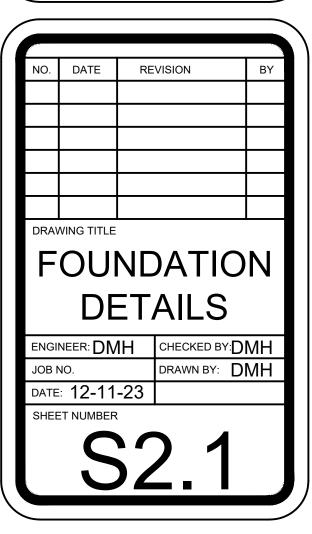
6 REINFORCEMENT AT OPENING CORNERS S2.0 AND STEP CORNERS @ INSIDE CORNERS SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

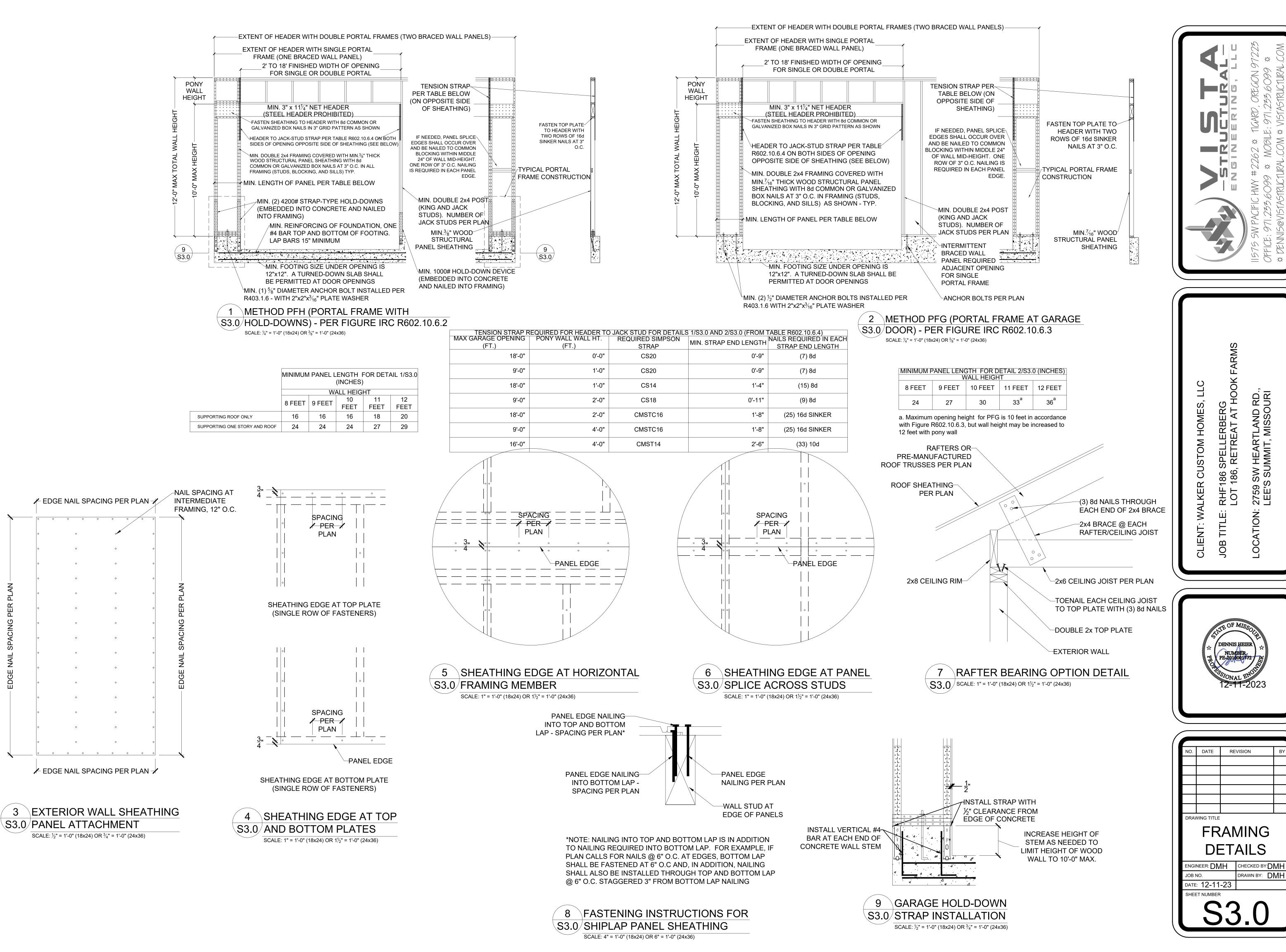


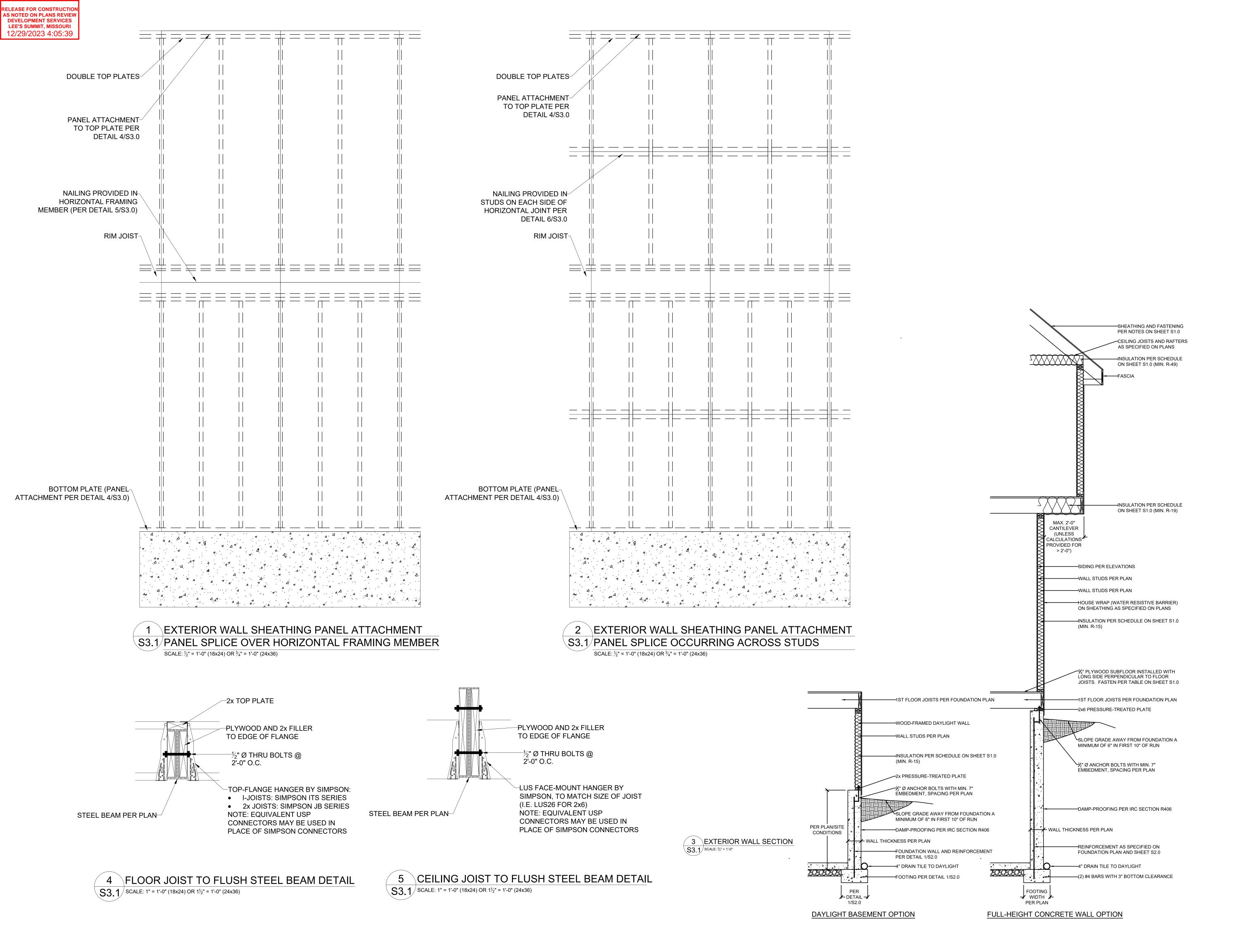


CLIENT: WALKER CUSTOM HOMES, LLC
JOB TITLE: RHF186 SPELLERBERG





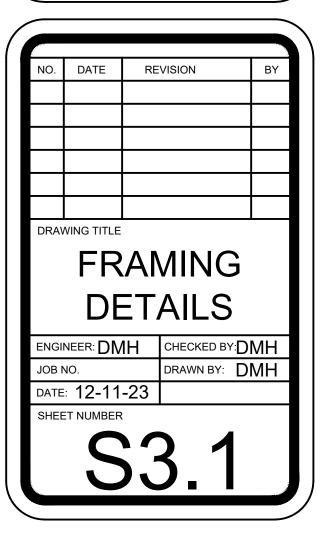






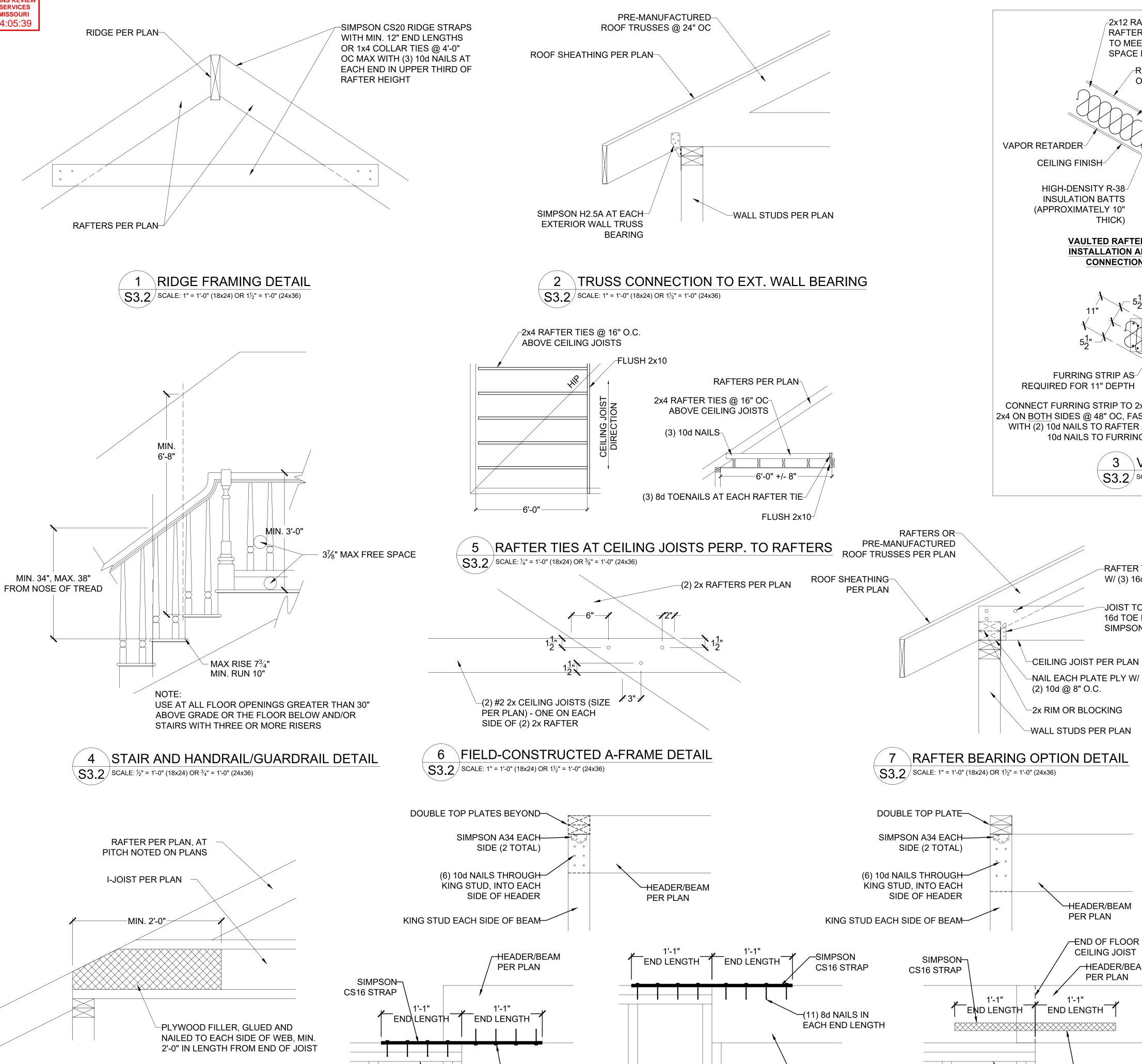
CLIENT: WALKER CUSTOM HOMES, LLC





9 COPED I-JOIST REINFORCEMENT

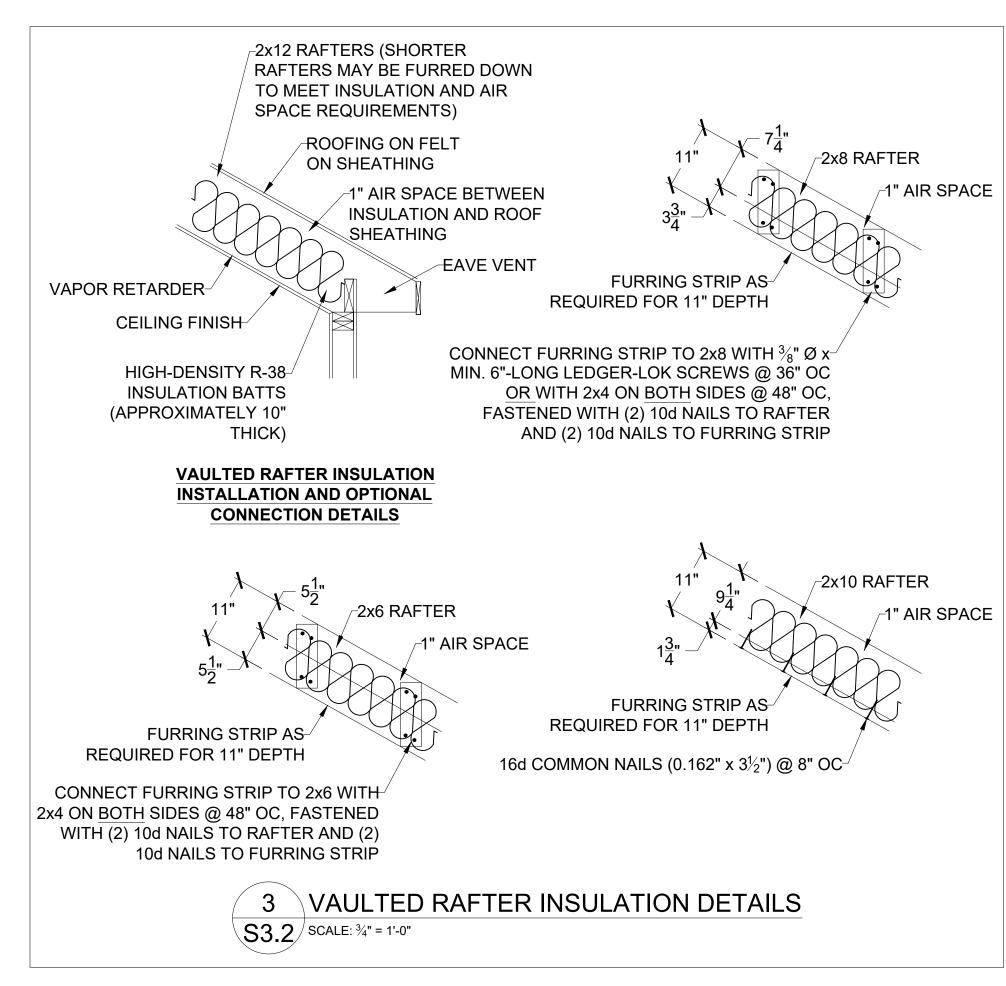
S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



└─(11) 8d NAILS IN

BEARING WALL-

EACH END LENGTH



W/ (3) 16d FACE NAILS

JOIST TO PLATE W/ (3)

16d TOE NAILS OR

PER PLAN

-END OF FLOOR OR

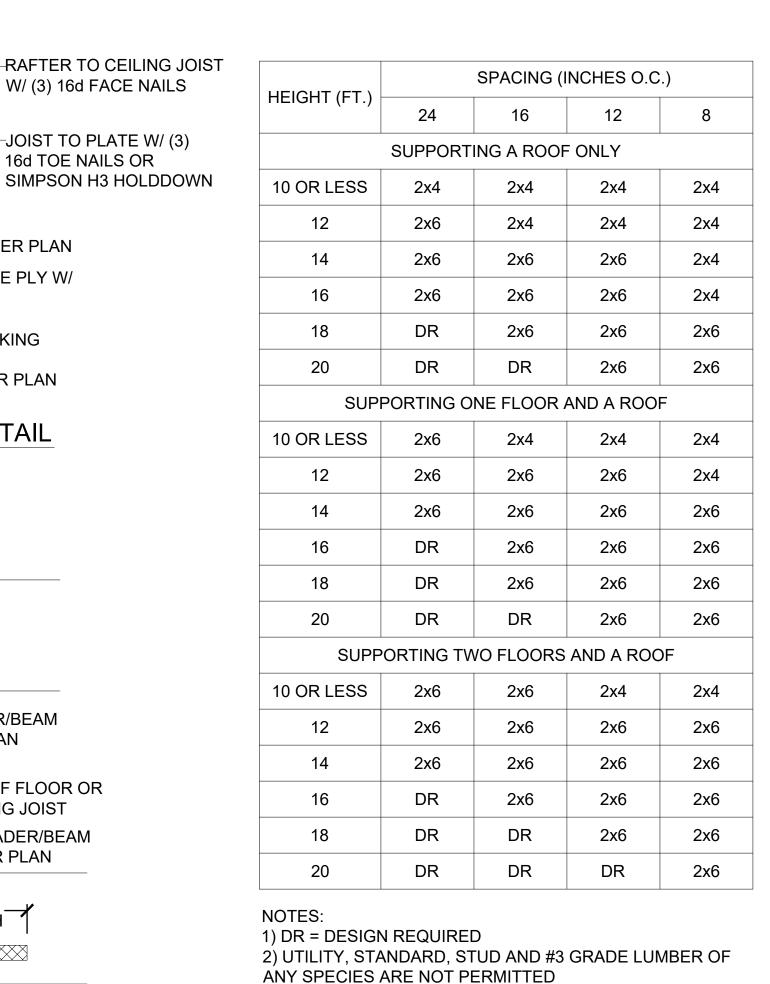
-HEADER/BEAM

└─(11) 8d NAILS IN

BEARING WALL EACH END LENGTH

CEILING JOIST

PER PLAN



8 MAXIMUM ALLOWABLE LENGTH OF S3.2/WOOD WALL STUDS (IRC TABLE 602.3.1)

SUPPORTING MEMBERS WITH A TRIB. LENGTH GREATER

3) THIS TABLE DOES NOT APPLY FOR STUDS

THAN 6'-0"



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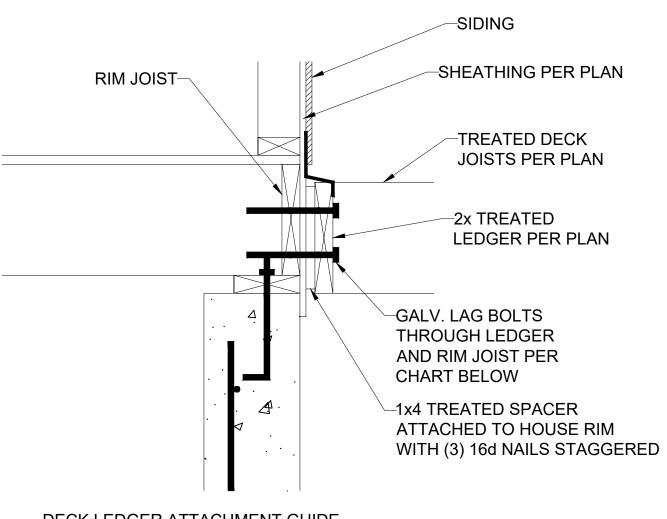
	DATE	REVISION	BY			
DRA	WING TITLE					
	FRAMING DETAILS					
ENGI			MH			
ENGI JOB I	DET	AILS				
JOB I	DET	CHECKED BY:D				
JOB I	DET	CHECKED BY:D				

10 HEADER/BEAM CONNECTION OPTIONS AT OUTDOOR/OPEN SPACE S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

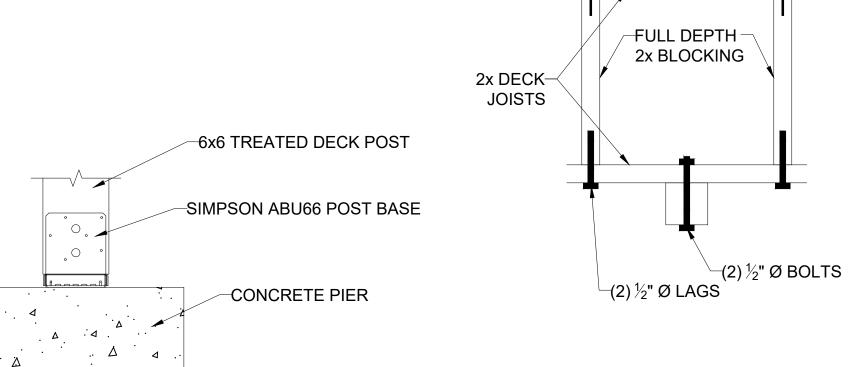
HEADER/BEAM

PER PLAN

─BEARING WALL



RIM JOIST-SIMPSON LS90 GUSSET ANGLES, NAILED TO BOTH-SIDES OF DOUBLED JOISTS W/ (12) N10 NAILS -SIDING FLOOR JOISTS PER PLAN. IF JOISTS ARE-NOT DOUBLED, INSTALL ADDITIONAL PLY SHEATHING PER PLAN TO EACH JOIST, MIN. 6'-0" IN LENGTH TREATED DECK JOISTS PER PLAN (MAX. SPAN = 16'-0") -2x TREATED LEDGER PER PLAN FASTEN DECK JOISTS TO LEDGER WITH SIMPSON LUS210 HANGERS GALV. LAG BOLTS CANTILEVER LENGTH_ THROUGH LEDGER AND RIM PER PLAN JOIST PER CHART BELOW SIMPSON H2.5A AT EACH **CANTILEVERED JOIST** -1x4 TREATED SPACER WHICH SUPPORTS DECK ATTACHED TO HOUSE RIM



ALL NAILS ARE 16d (2) ½" Ø LAGS IN EACH SIDE

4 REINF. POST CONNECTIONS

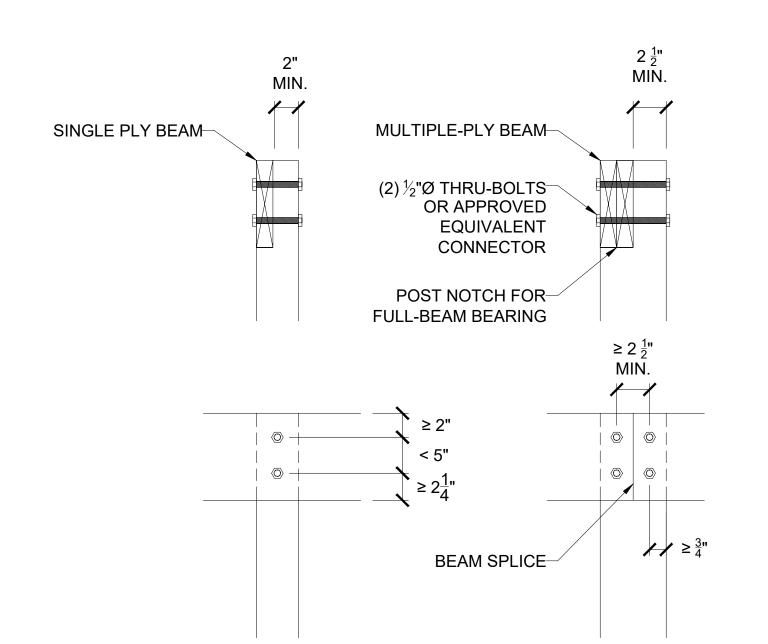
-WOOD BEAM PER PLAN

S3.3 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

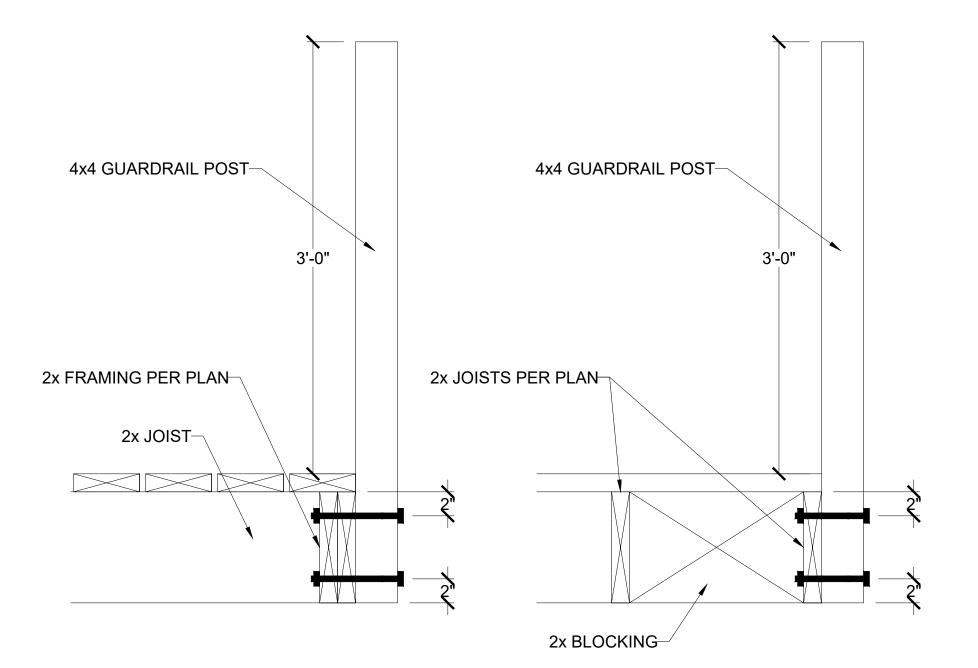
DECK LEDGER ATTACHMENT GUIDE

DECK JOIST SPAN	$^{1\!\!/_{\!\!2}}$ " Ø GALV. LAG OR $^{3\!\!/_{\!\!8}}$ " Ø LEDGER-LOK SPACING
10'-0" OR LESS	16" OC
10'-0" - 13'-11"	12" OC OR @ 16" OC DOUBLED EVERY OTHER
14'-0" - 18'-0"	8" OC OR @ 16" OC DOUBLED

1 LEDGER ATTACHMENT S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

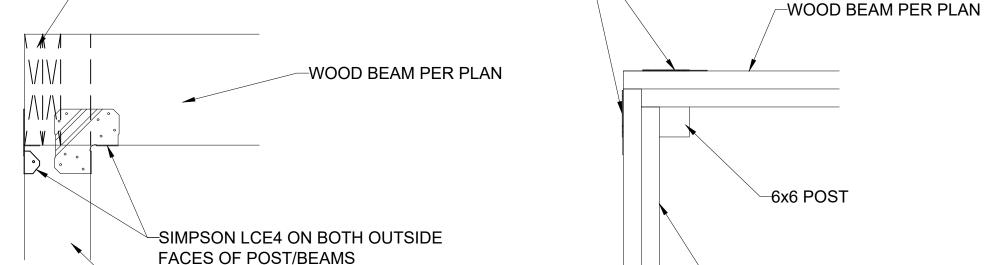


2 CANTILEVER WITH DECK ATTACHMENT S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



6 GUARDRAIL CONNECTION

S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



SIMPSON LCE4 ON-

OF POST/BEAMS

BOTH OUTSIDE FACES

7 ALTERNATE COVERED DECK/PORCH INTERSECTION S3.3 CORNER BEAM CONNECTION

SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

-6x6 POST

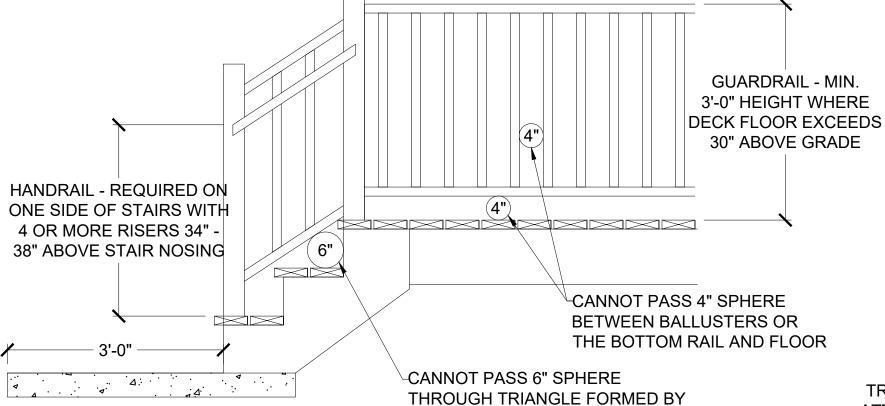
3 DECK POST BASE

—WOOD BEAM PER PLAN

(PERPENDICULAR)

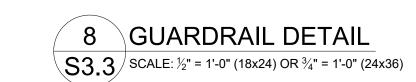
S3.3 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

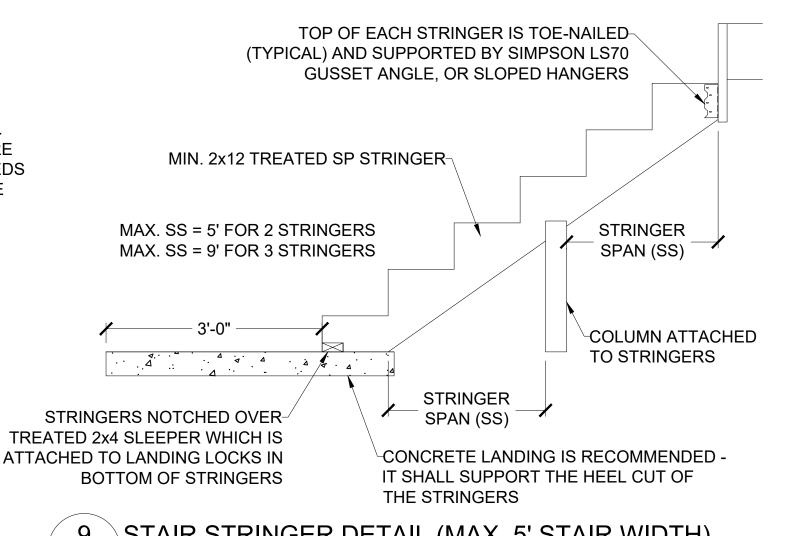
5 LET-IN (COVERED) DECK BEAM CONNECTION SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



RISER, TREAD AND BOTTOM RAIL

WITH (3) 16d NAILS STAGGERED



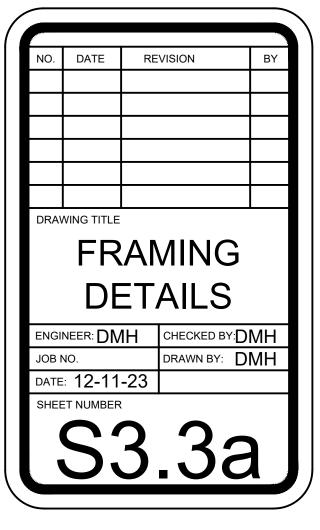


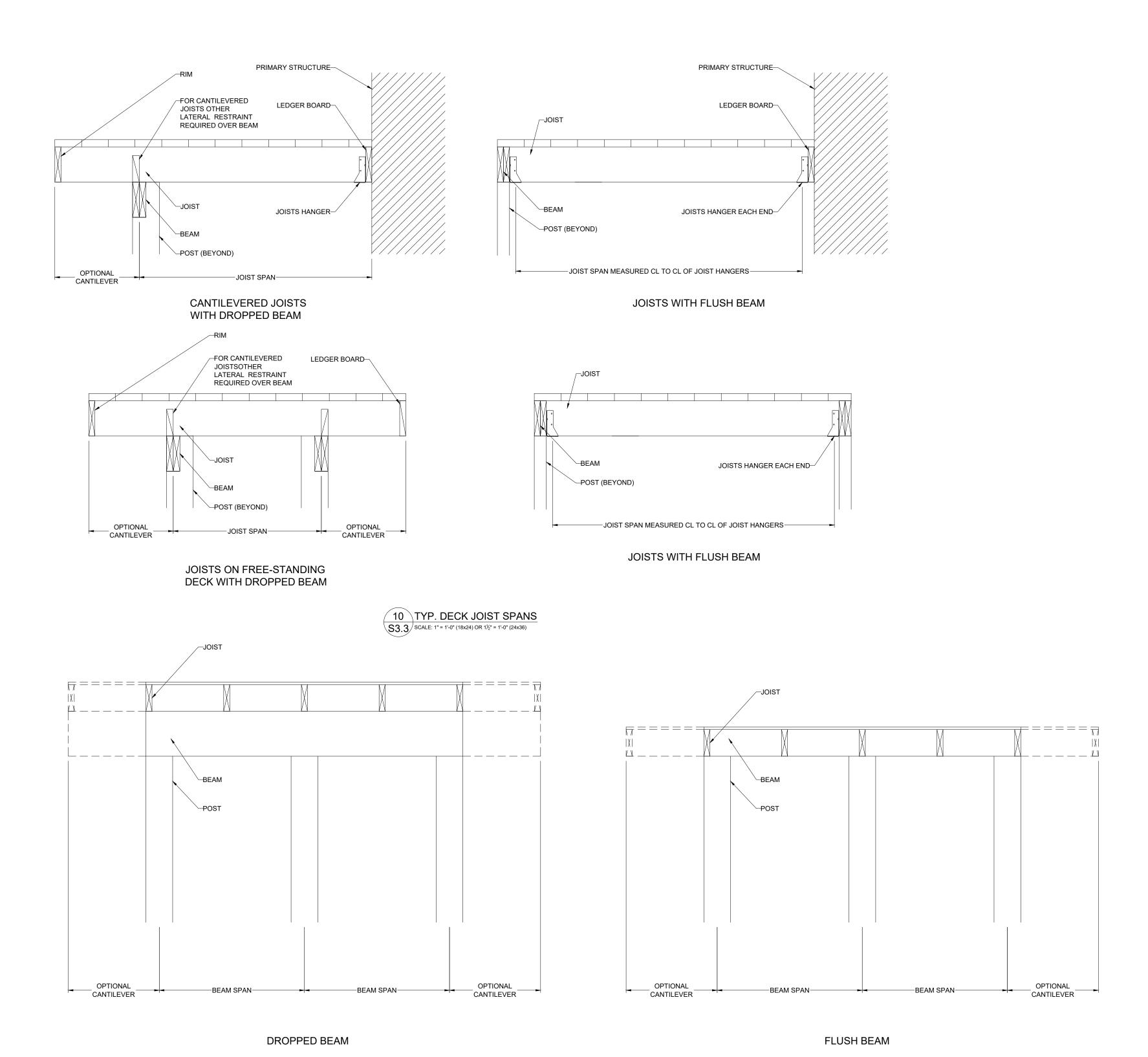
9 STAIR STRINGER DETAIL (MAX. 5' STAIR WIDTH)
S3.3 SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)



JOB TITLE: RHF186 SPELLERBERG
LOT 186, RETREAT AT HOOK FARM
LOCATION: 2759 SW HEARTLAND RD.,
LEE'S SUMMIT, MISSOURI







11 TYP. DECK JOIST SPANS SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



CLIENT: WALKER CUSTOM HOMES, LLC

DENNIS HEIER

NUMBER

PE-2010001772

SHAME

PE-2010001772

PE-2010

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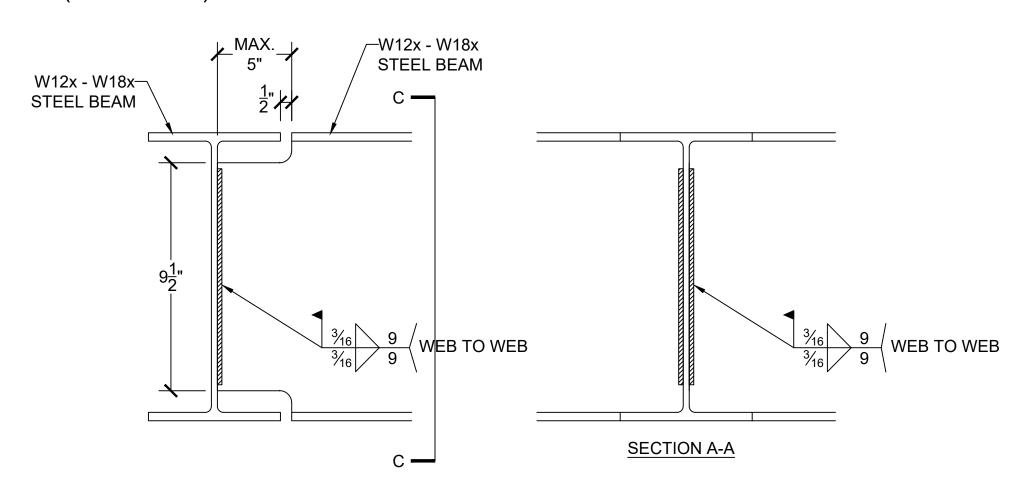
DRAWING TITLE

FRAMING
DETAILS

ENGINEER: DMH
JOB NO.
DATE: 12-11-23
SHEET NUMBER

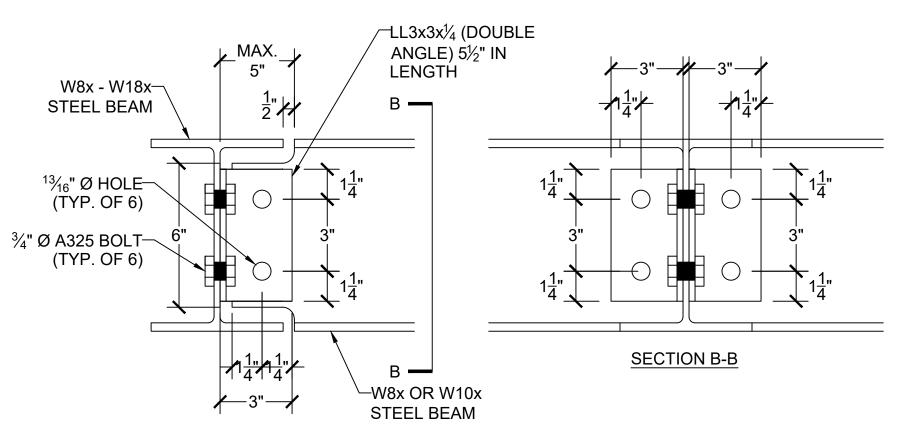
WELDED T-BEAM CONNECTION FOR W8x AND W10x BEAMS $\sqrt{S3.4}$ SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

(OPTION #1)



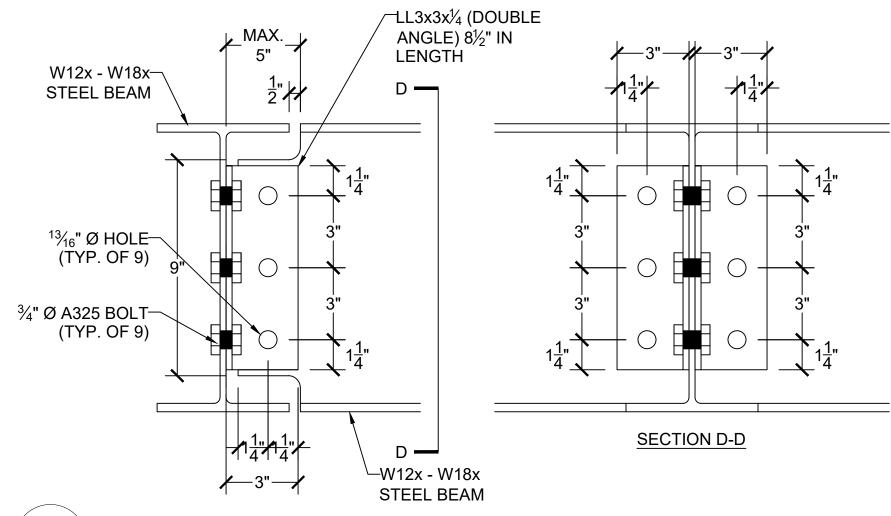
2 WELDED T-BEAM CONNECTION FOR W12x, W14x, W16x & W18x BEAMS SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

(OPTION #1)



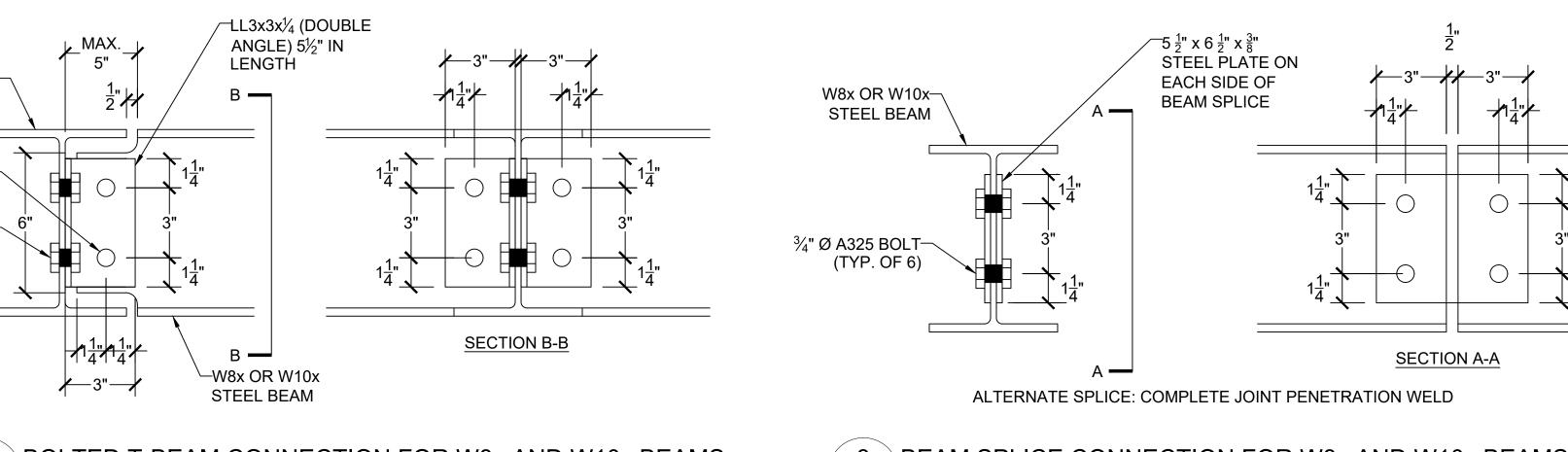
BOLTED T-BEAM CONNECTION FOR W8x AND W10x BEAMS $\sqrt{S3.4}$ SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

(OPTION #2)

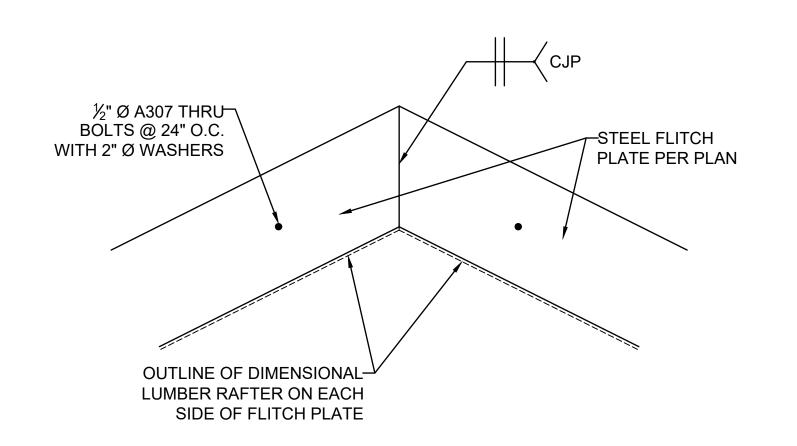


BOLTED T-BEAM CONNECTION FOR W12x, W14x, W16x & W18x BEAMS S3.4 SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

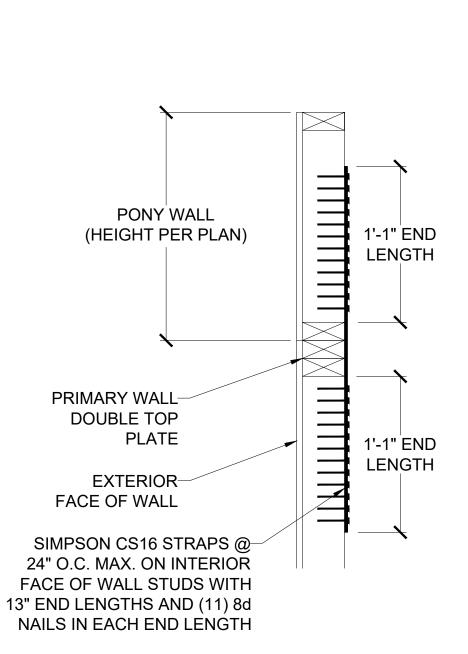
(OPTION #2)



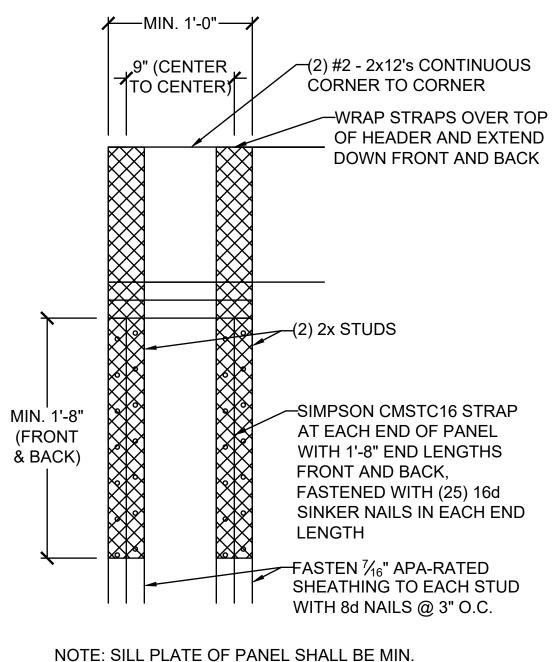
BEAM SPLICE CONNECTION FOR W8x AND W10x BEAMS S3.4 SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)



RAFTER FLITCH PLATE DETAIL S3.4 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

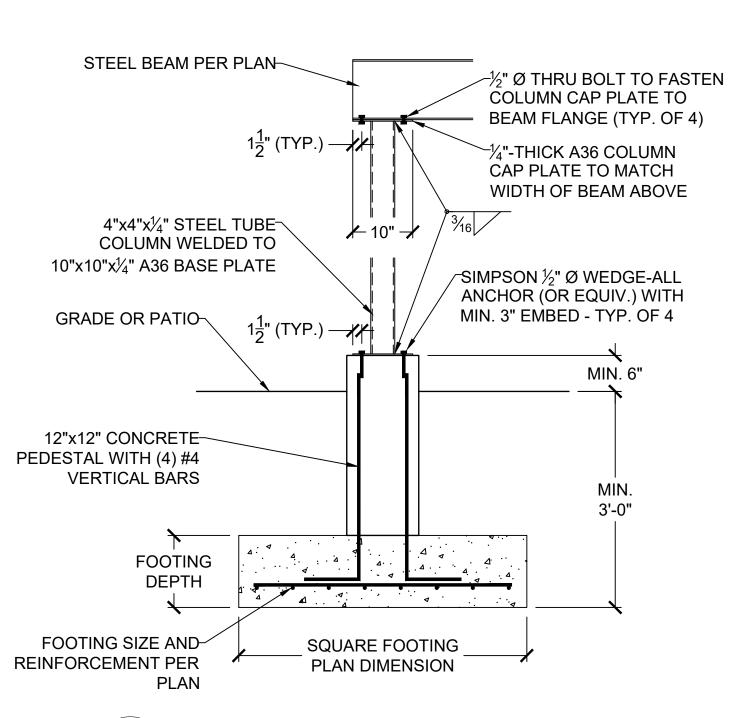






NOTE: SILL PLATE OF PANEL SHALL BE MIN. (1) 2x AND FASTENED WITH ½" Ø ANCHOR BOLT AND 2" Ø WASHER PLATE





EXTERIOR STEEL COLUMN CONNECTIONS S3.4 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)



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