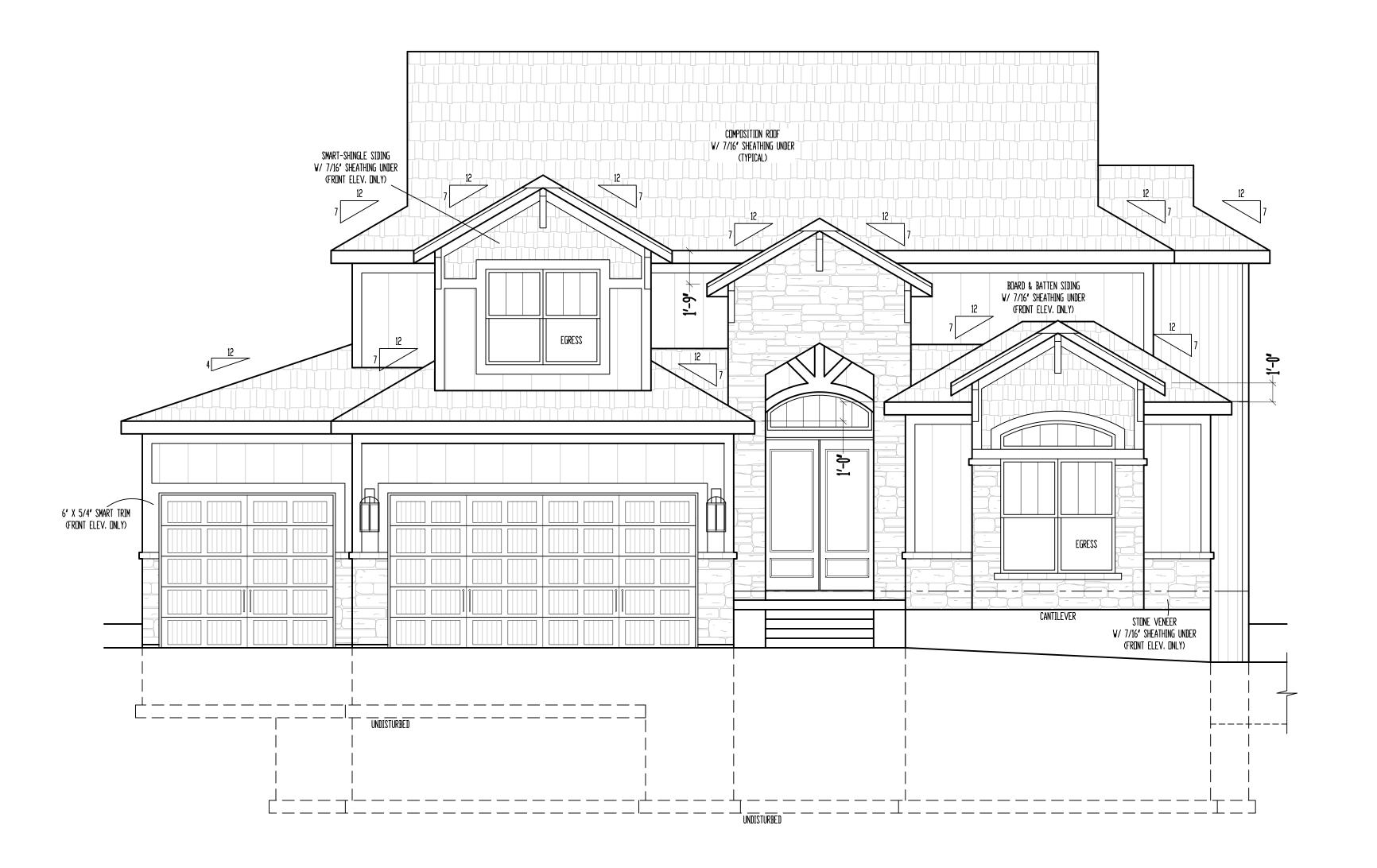
NOTE: GOVERNING CODES & GENERAL CONTRACTOR'S WRITTEN SPECIFICATIONS TAKE PRECEDENCE OVER THESE PLANS.



FRONT ELEVATION

SCALE: 1/4" = 1'-0"

ian. However, the lese plans should not be I, architect or engineer. ervision, Viewpoint

rt have gone into the creation and design of this plan. However an architect or engineer and construction from these plans ithout the assistance of a construction professional, architect is impossibility of any on site consultation and supervision, Vi

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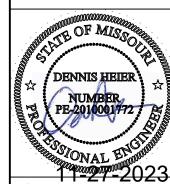
that whosoever believeth in him should not perish, but have everlasting life"



The
DURANGO
Site Description:
Lot 83,
Summit View
Farms 4th Plat
Street Address:
2307 SW Serena Pl.,

Design Title:

Lee's Summit,
Missouri
General Contractor:
Wood Brothers, Inc.

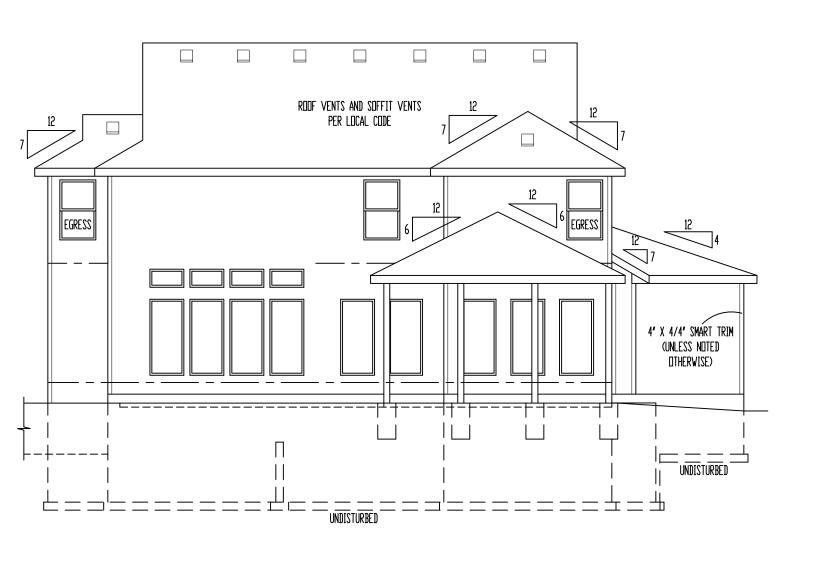


Date: 11 -Rev. 1: Rev. 2: Rev. 3:

> Sheet Title: FRONT ELEVATION



REAR ELEVATION SCALE: 1/8" = 1'-0"



LEFT ELEVATION SCALE: 1/8" = 1'-0"

ELEVATIONS:

ADJUST FOUNDATION TO GRADE

2" X 6" CEDAR DECKING

6' X 6' CEDAR/TTD. POSTS
2' X 2' CEDAR SPINDLES

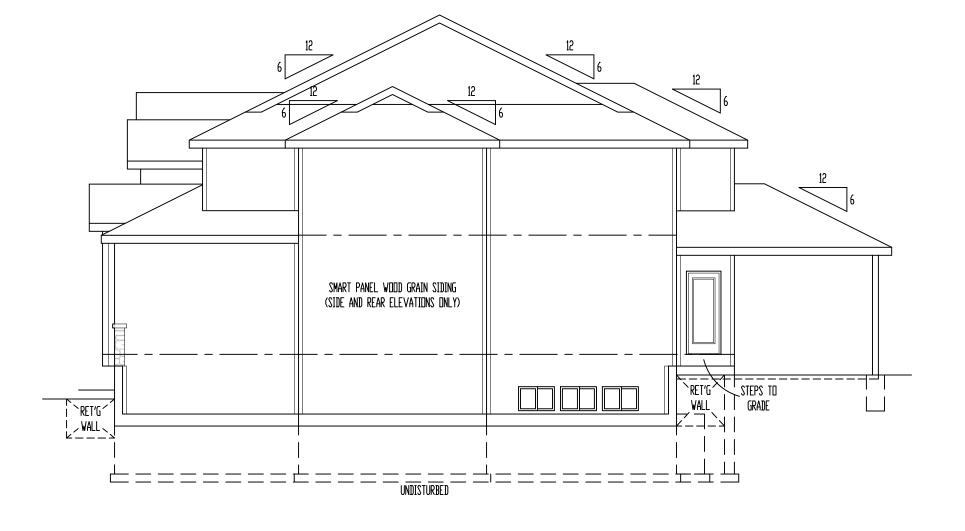
2" X 6" CEDAR TOP RAIL STAIRS PER PLAN

OPTIONAL DECK:

DECK CONSTRUCTION TO COMPLY WITH MUNICIPALITY'S

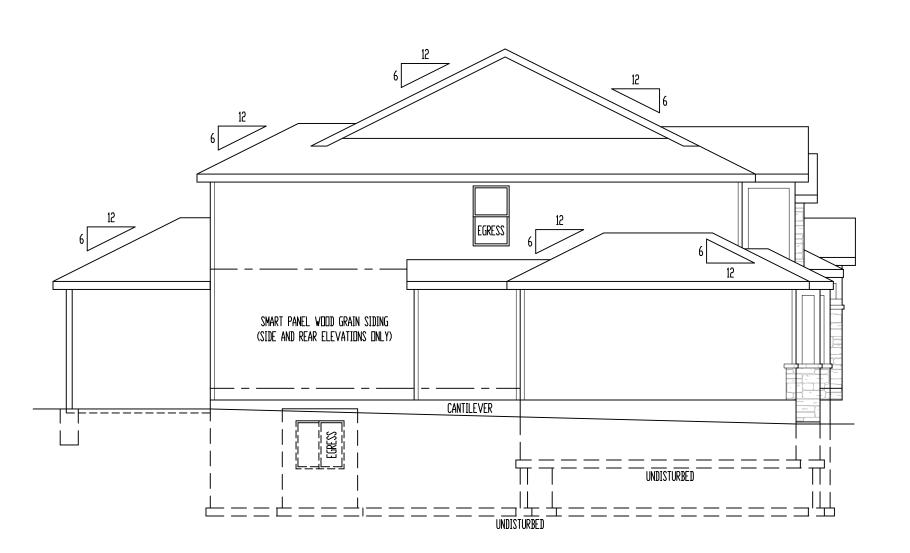
SMART PANEL WOOD GRAIN SIDING ON SIDE AND REAR ELEVATIONS COMPOSITION ROOF SHINGLES
LOCATE ROOF AND SOFFIT VENTS PER CODE

RESIDENTIAL DECK STANDARDS 2' X 10' #2 TTD. @ 16' D.C. FLOOR JOISTS (MAX. SPAN: 14'-0')



RIGHT ELEVATION

SCALE: 1/8" = 1'-0"



General Contractor: Wood Brothers, Inc.

Design Title: **The**

DURANGO

Site Description: **Lot 83**,

Summit View

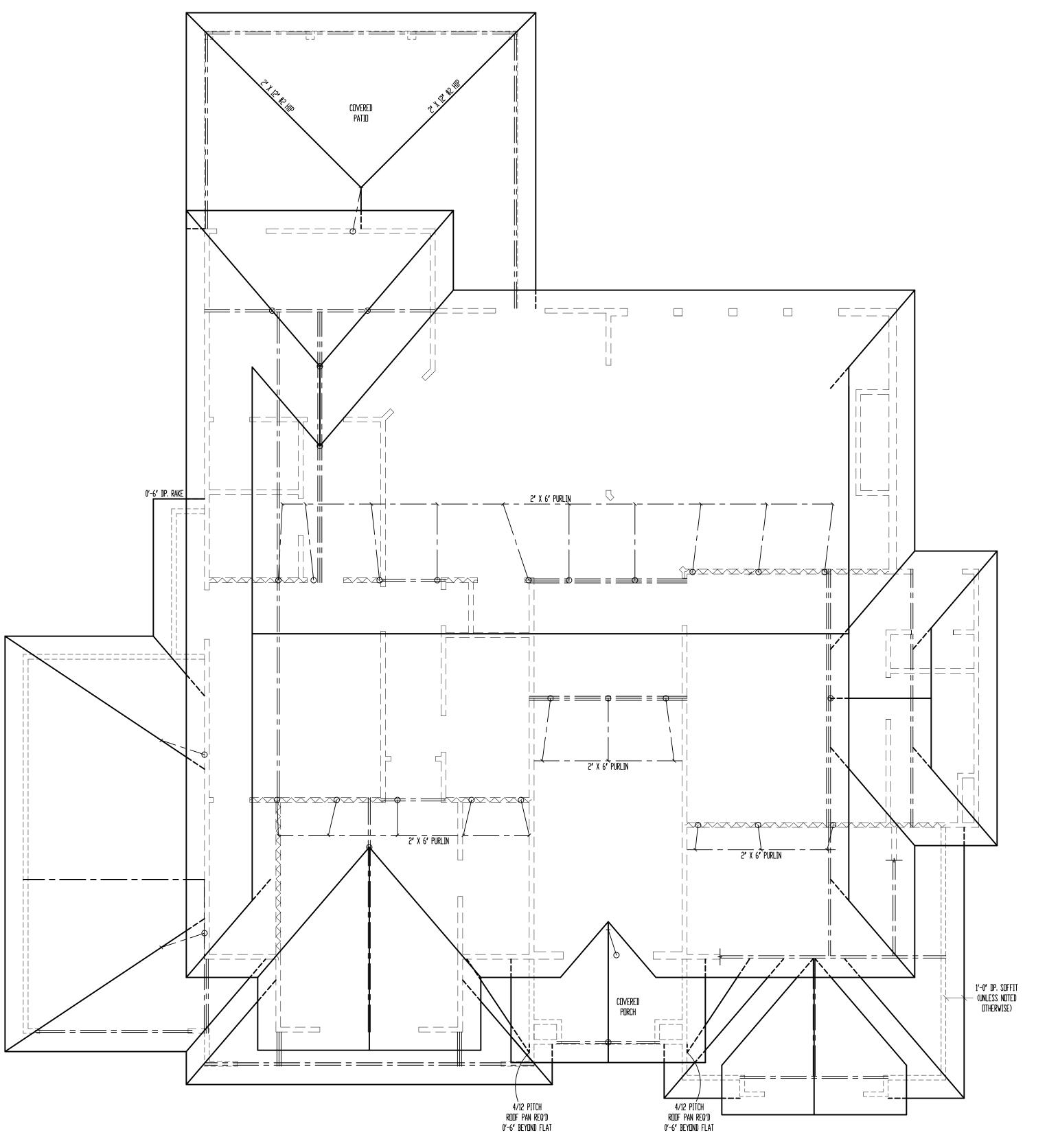
Farms 4th Plat
Street Address:
2307 SW Serena Pl.,

Lee's Summit,

Missouri

Rev. 1: Rev. 2: Rev. 3:

Sheet Title: SIDES & REAR **ELEVATIONS**



$R\Box\Box$

SCALE: 1/4" = 1'-0"

*ALL RAFTERS SHALL BE 2" X 6" #2 @ 16" D.C., UNLESS NOTED OTHERWISE.

SEE DETAIL 7/S3.2 FOR ALTERNATE RAFTER BEARING DETAIL WHEN RAFTERS ARE REQUIRED TO BEAR HIGHER THAN THE WALL DOUBLE TOP PLATE.

FLASHING NOTE: DRIP EDGE, VALLEYS AND FLASHINGS TO BE METAL CLAD.

ROOF NOTES: ROOF DESIGNED FOR LIGHT ROOF COVERING 30psf TOTAL LOAD (10psf DL, 20psf LL (SL))

* RAFTERS (HEM-FIR, DDUG-FIR, DR EQUAL): SEE SPAN CHARTS BELDW

| | CODE MINI | MUM | | |
|---------------------|-----------|-------------------|-----------------------------|---------------|
| | RAFTERS | SPACING | MAX HORIZONTAL CLEARSPAN | |
| | #2-2x6 | @24" D.C. | 11'-7 ' | |
| >>> | #2-2x6 | 016 ′ □.C. | 14'-2 ' | {{{ |
| | #2-2x8 | @24" D.C. | 14'-8 ' | |
| | #2-2x8 | 016 ′ □.C. | 17'-11 " | |
| | #2-2x10 | @24" D.C. | 17'-10 ' | |
| | #2-2x10 | 016 ′ □.C. | 21'-11 ' | |
| | HOTE ACR | | DUA FOR A RAFTER REFLEATION | - |

NOTE: CODE MINIMUM ALLOWS FOR A RAFTER DEFLECTION OF L/180 TOTAL LOAD

HIGHER PERFORMANCE (RECOMMENDED)

RAFTERS SPACING MAX HORIZONTAL CLEARSPAN
#2-2x6 @24" D.C. 8'-6"

* VAULTS TO BE 2x10 DEPTH
* RIDGE BOARDS ARE: (UNLESS OTHERWISE NOTED)

- #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 OVER 10/12 PITCH

* ALL HIPS & VALLEYS ARE: (UNLESS OTHERWISE NOTED)
- #2- 2X8 UP TO 10/12 PITCH

- #2- 2X10 OVER 10/12 PITCH
* PURLINS ARE 2X6 MIN.

- PURLIN STRUTS ARE AT 4'-0' D.C.

- PURLIN STRUTS ARE AT 4-0 LIC.
- PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A
45 DEGREE ANGLE WITH THE HORIZONTAL

- ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED LENGTH DF 8'-0'
- PURLINS STRUTS SHALL BE CONSTRUCTED IN A
'T' CONFIGURATION AND PER THE FOLLOWING CHART:

| PURLIN STRUT | MAX PURLIN STRUT LENGTH |
|-----------------------|-------------------------|
| (2) 2x4 | 8′-0 ′ |
| (1) 2x4 & (1) 2x6 | 12'-0 ' |
| (1) 2x6 & (1) 2x8 | 20'-0 " |
| (2) 2x6 & (1) 2x8 | 30'-0 " |
| CONSULT ARCH./ENGR. > | 30′-0 ″ |

* RIDGE BRACES ARE SAME AS PURLIN BRACES-SPACING, SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE) * HIP & VALLEY BRACES ARE SAME AS PURLIN SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE)

* VERTICAL BRACE IF DOT IS UNDER HIP OR VALLEY

* SLASH IS TOP END OF BRACE (/),

DOT IS POTTOM OF PRACE (a)

DOT IS BOTTOM OF BRACE (o).

* DENOTES BEARING WALL

* DENOTES ROOF BRACE

* — DENOTES PURLIN

* — DENOTES BEARING STRUCTURE

Viewpoint Residential Design, LLC.

gone into the creation and design of this plan. Howeve chitect or engineer and construction from these plans so a easistance of a construction professional, architect assibility of any on site consultation and supervision, Vie L.C, and Designer assume no responsibility for any defliciencies, due to any defliciencies, omissions or error in the

egotten Son, Caes agotten Son, design transcription to the side will be seen to the seed of the seed to the seen the seed to the seen the seen the seed to the seen t

ENTIAL DESIGN LLC

Design Title:
The
DURANGO

Site Description:

Lot 83,

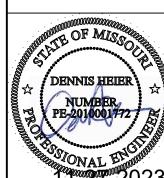
Summit View

Farms 4th Plat

Street Address:

2307 SW Serena Pl.,
Lee's Summit,
Missouri
General Contractor:

Wood Brothers, Inc.



Date: 11 - 25 - AD 20 Rev. 1: Rev. 2:

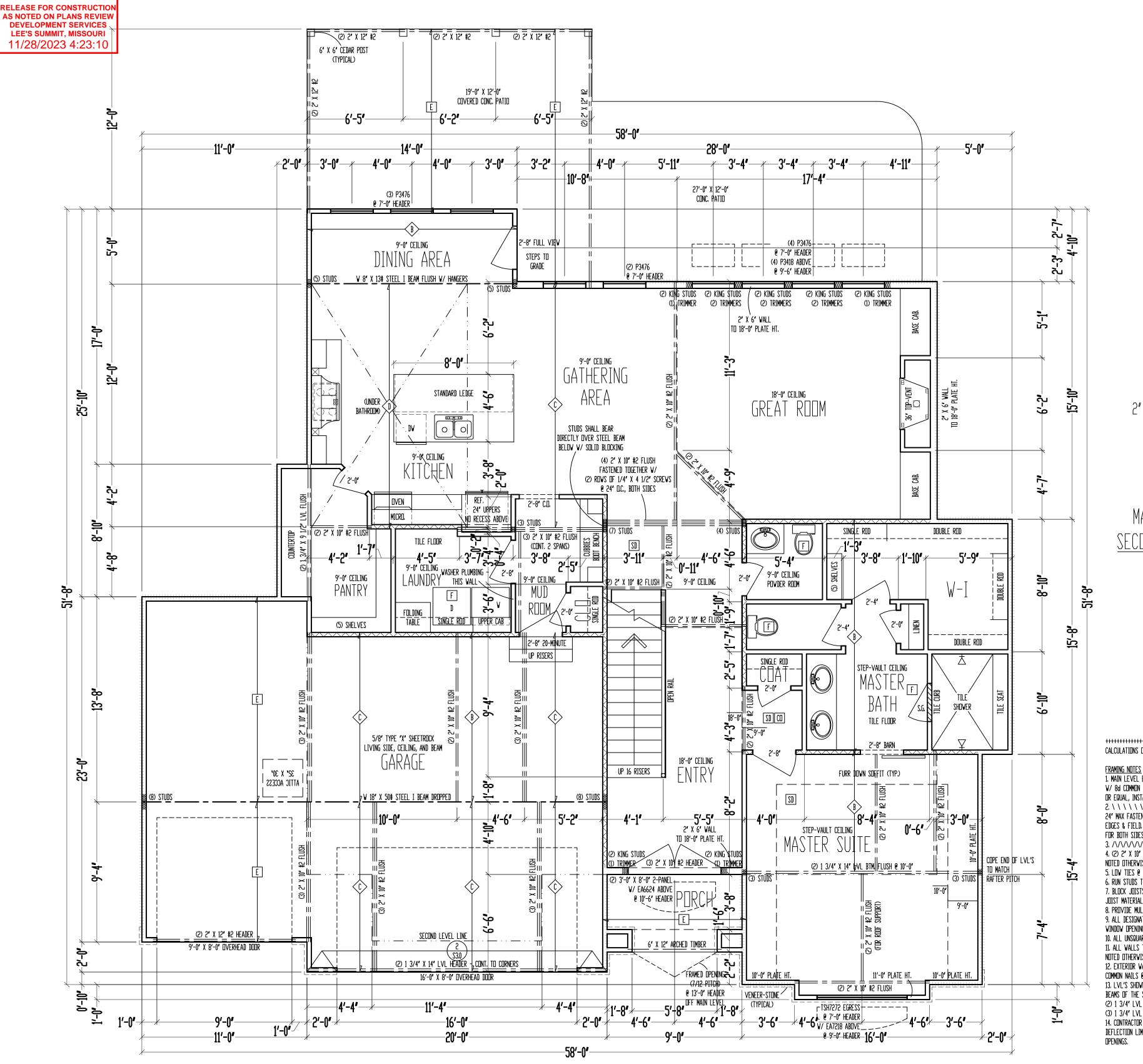
Rev. 3:

Sheet Title:

ROOF PLAN

Sheet No.:

A-3
of 6



JOIST SCHEDULE 2' X 10" #2 TTD. FLOOR JOIST € 16° D.C. 2" X 10" #2 FLOOR JOIST e 16″ □.C. 2" X 10" #2 FLOOR JOIST € 16' D.C. - DOUBLE EVERY OTHER 2" X 10" #2 FLOOR JOIST € 16° D.C. – DOUBLED 2' X 6' #2 CEILING JOIST @ 16″ □.C.

9'-0" CEILING 2" X 10" FLOOR SYSTEM ABOVE MAIN LEVEL SCALE: 1/4'' = 1'-0''

MAIN LEVEL: 1734 SQ. FT SECOND LEVEL: 1330 SQ. FT TDTAL: 3064 SQ. FT

> GARAGE: 695 SQ. FT. COV. DUT/LIV: 253 SQ. FT. UNFIN. BASEMENT: 1448 SQ. FT.

CALCULATIONS ON SHEET S1.1.

1. MAIN LEVEL EXTERIOR WALLS SHALL BE SHEATHED W/ 7/16" D.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS.

2. \ \ \ \ \ \ \ \ = G.B.: 1/2" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX FASTENED W/ NO. 6 - 1 1/4" TYPE W OR S DRYWALL SCREWS @ 7" D.C. EDGES & FIELD. (MIN. 8'-0' SECTIONS ONE SIDE OF WALL (OR) MIN. 4'-0' SECTION

3. /\/\/\/\/\/\\ = LOAD BEARING INTERIOR WALL. 4. (2) 2" X 10" #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.

5. LOW TIES @ 4'-0" D.C. (TYPICAL) 6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH

JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS). 8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. ALL DESIGNATED 2" X 6" WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND

WINDOW OPENINGS. 10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE.

11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2' X 4'S @ 16' D.C., UNLESS NOTED OTHERWISE.

12. EXTERIOR WALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW WITH 16d COMMON NAILS @ 8" D.C. MAX. (WHERE APPLICABLE.)

13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS: (2) 1 3/4" LVL PLIES = 3 1/2" GLULAM

(3) 1 3/4" LVL PLIES = 5 1/2" GLULAM 14. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY

Design Title: The **DURANGO**

Site Description: Lot 83, **Summit View** Farms 4th Plat

Street Address: 2307 SW Serena Pl. Lee's Summit, Missouri General Contractor:

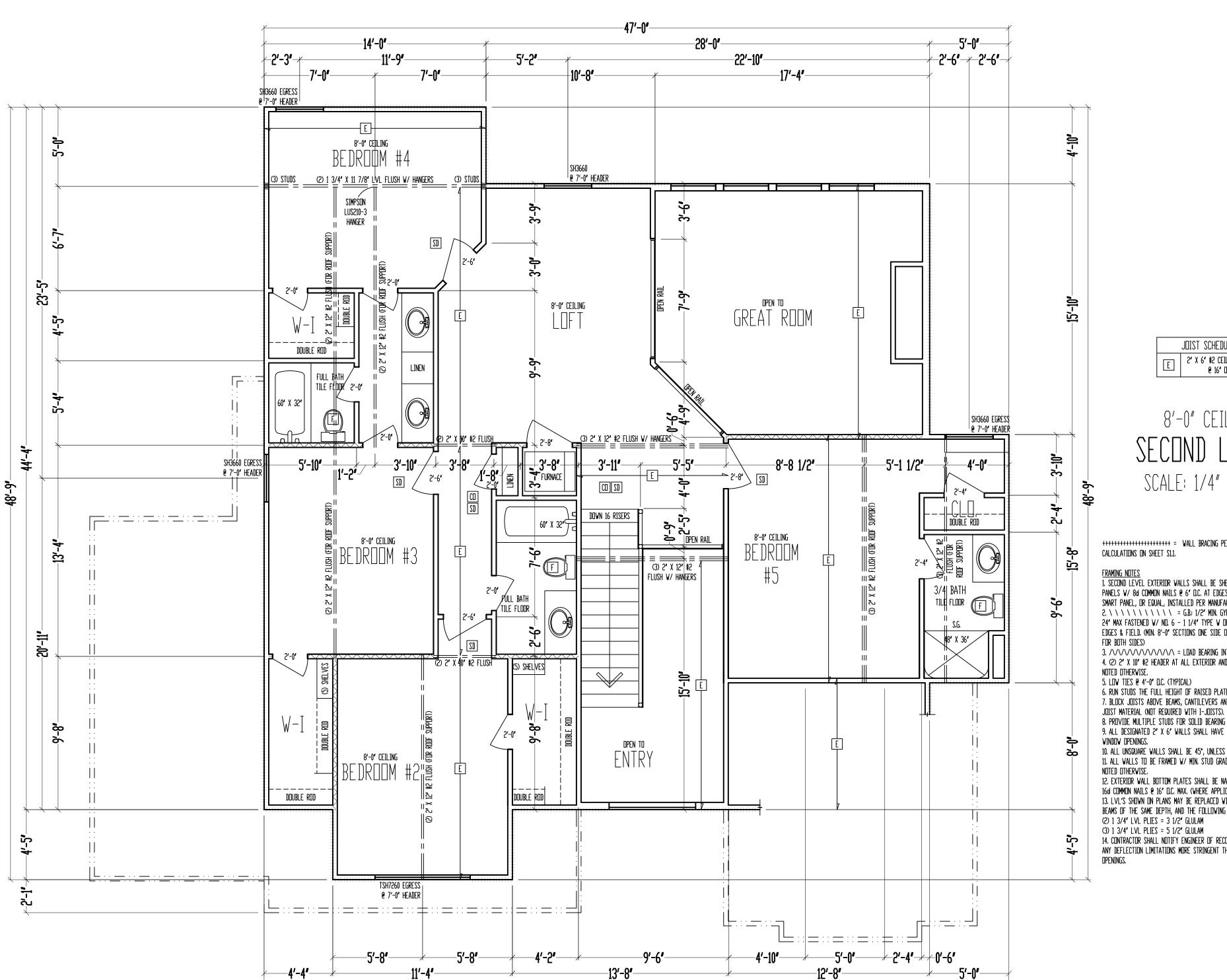
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Rev. 1:

Rev. 2: Rev. 3:

Sheet Title: MAIN LEVEL PLAN



-47**′-0″**-

JOIST SCHEDULE 2" X 6" #2 CEILING JOIST **e** 16**°** □.C.

8'-0" CEILING SECOND LEVEL SCALE: 1/4" = 1'-0"

CALCULATIONS ON SHEET S1.1.

1. SECOND LEVEL EXTERIOR WALLS SHALL BE SHEATHED W/ 7/16' D.S.B. A.P.A. PANELS W/ 8d CDMMDN NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, OR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS. 2. \ \ \ \ \ \ \ \ = G.B.: 1/2" MIN. GYPSUM BOARD OVER STUDS SPACED 24' MAX FASTENED W/ ND. 6 - 1 1/4' TYPE W DR S DRYWALL SCREWS @ 7' D.C. EDGES & FIELD. (MIN. 8'-0" SECTIONS ONE SIDE OF WALL (OR) MIN. 4'-0" SECTION FOR BOTH SIDES)

3. $\/\/\/\/\/\/\$ = LOAD BEARING INTERIOR WALL. 4. (2) 2' X 10' #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.

5. LOV TIES @ 4'-0" D.C. (TYPICAL)

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. ALL DESIGNATED 2" X 6" WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS.

10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2' X 4'S @ 16' D.C., UNLESS

NOTED OTHERWISE. 12. EXTERIOR WALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW WITH

16d COMMON NAILS @ 16" D.C. MAX. (WHERE APPLICABLE.) 13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS:

(2) 1 3/4" LVL PLIES = 3 1/2" GLULAM (3) 1 3/4" LVL PLIES = 5 1/2" GLULAM

14. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY

Design Title: The **DURANGO**

Site Description: Lot 83, **Summit View** Farms 4th Plat Street Address:

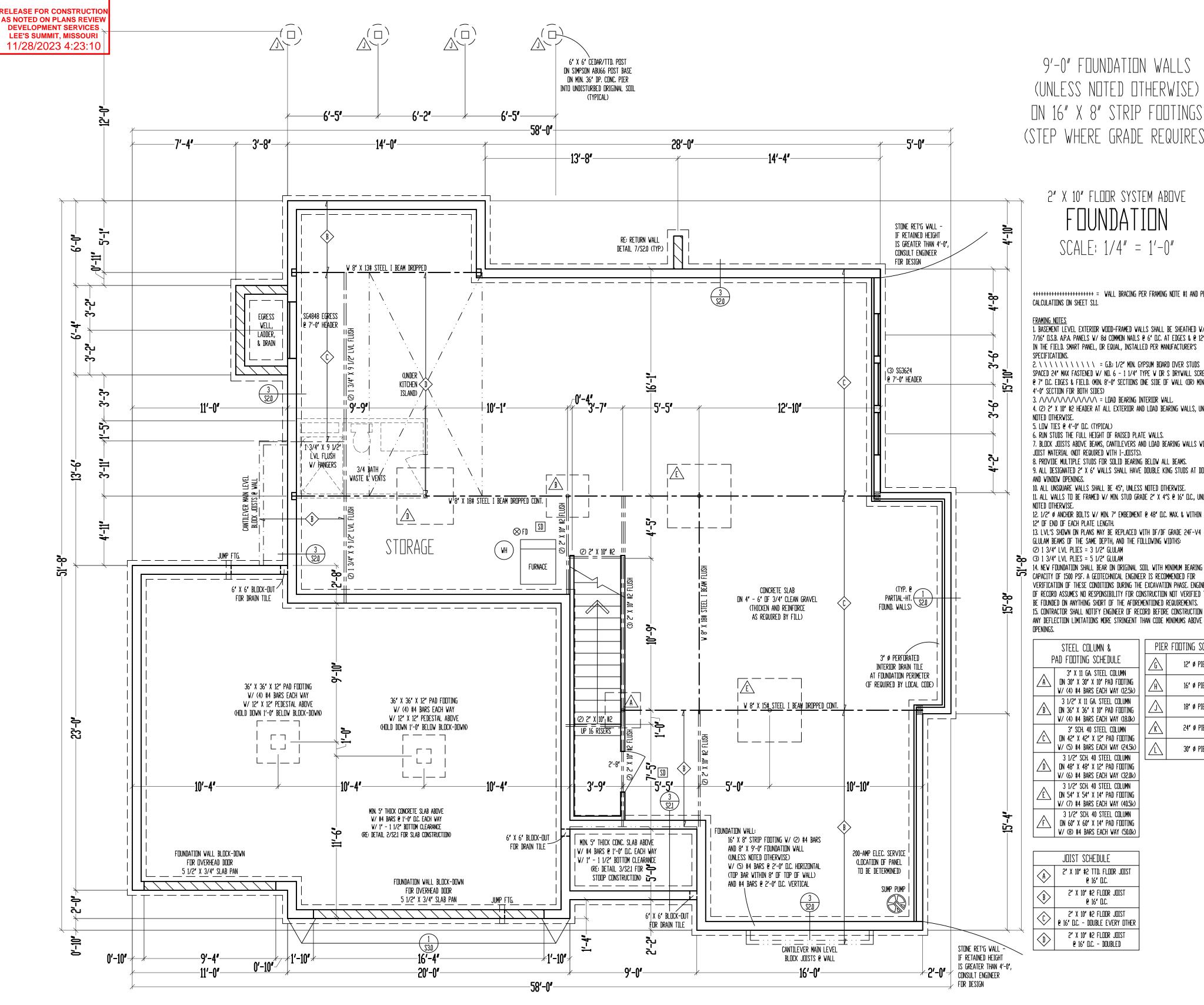
2307 SW Serena Pl., Lee's Summit, Missouri General Contractor: Wood Brothers, Inc.



Rev. 1: Rev. 2: Rev. 3:

Sheet Title: **SECOND LEVEL**

PLAN



9'-0" FOUNDATION WALLS (UNLESS NOTED OTHERWISE) ON 16" X 8" STRIP FOOTINGS (STEP WHERE GRADE REQUIRES)

> 2" X 10" FLOOR SYSTEM ABOVE FOUNDATION

SCALE: 1/4" = 1'-0"

1. BASEMENT LEVEL EXTERIOR WOOD-FRAMED VALLS SHALL BE SHEATHED $\ensuremath{\mathsf{W}}/$ 7/16' D.S.B. A.P.A. PANELS W/ 8d CDMMDN NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD, SMART PANEL, OR EQUAL, INSTALLED PER MANUFACTURER'S

2. \ \ \ \ \ \ \ \ = G.B.; 1/2" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX FASTENED W/ NO. 6 - 1 1/4" TYPE W OR S DRYWALL SCREWS € 7" D.C. EDGES & FIELD. (MIN. 8'-0" SECTIONS DNE SIDE DF WALL (DR) MIN.

3. $\/\/\/\/\/\$ = LOAD BEARING INTERIOR WALL. 4. (2) 2' X 10' #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. ALL DESIGNATED 2" X 6" WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR

10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2' X 4'S @ 16' D.C., UNLESS

12. 1/2" Ø ANCHOR BOLTS W/ MIN. 7" EMBEDMENT @ 48" D.C. MAX. & WITHIN 6"

13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS:

CAPACITY OF 1500 PSF. A GEOTECHNICAL ENGINEER IS RECOMMENDED FOR VERIFICATION OF THESE CONDITIONS DURING THE EXCAVATION PHASE, ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANYTHING SHORT OF THE AFOREMENTIONED REQUIREMENTS. 15. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY

| ٥, | | | | | | |
|----------------|---|--|-----------------------|-----------------|--|--|
| STEEL COLUMN & | | | PIER FOOTING SCHEDULE | | | |
| F | PAD FOOTING SCHEDULE | | (| 12' Ø PIER FTG. | | |
| | 3″ X 11 GA. STEEL COLUMN | | | | | |
| | ON 30" X 30" X 10" PAD FOOTING W/ (4) #4 BARS EACH WAY (12.5k) | | <u>₩</u> | 16" Ø PIER FTG. | | |
| | 3 1/2' X 11 GA. STEEL COLUMN DN 36' X 36' X 10' PAD FOOTING | | | 18' Ø PIER FTG. | | |
| | W/ (4) #4 BARS EACH WAY (18.0k) | | \wedge | OM & DIED ETC | | |
| | 3' SCH. 40 STEEL COLUMN | | $\langle K \rangle$ | 24" Ø PIER FTG. | | |
| | DN 42" X 42" X 12" PAD FOOTING W/ (5) #4 BARS EACH WAY (24.5k) | | Λ | 30° Ø PIER FTG. | | |
| | | | | | | |

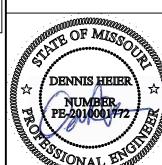
Design Title: The **DURANGO**

Site Description:

Lot 83, **Summit View** Farms 4th Plat Street Address:

2307 SW Serena Pl., Lee's Summit, Missouri General Contractor:

Wood Brothers, Inc.



Rev. 1: Rev. 2:

Rev. 3: Sheet Title:

FOUNDATION PLAN

RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSO O A DESCRIPTION OF BUILDING ELEMENTS

11/28/2023 4:2

| 3.1 OPES CRIPTION OF BUILDING ELEMENTS | NUMBER AND TYPE OF FASTENER | SPACING AND LOCATION |
|---|--|--|
| | ROOF 1 | • |
| BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOE NAIL | 4-8d (2 ½ " x 0.113") | TOENAIL |
| CEILING JOISTS TO PLATE, TOE NAIL | 4-8d (2 ½ " x 0.113") | PER JOIST, TOENAIL |
| CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS, FACE NAIL | 4-10d (3" x 0.128") | FACE NAIL |
| CEILING JOIST TO PARALLEL RAFTER (HEEL JOINT) | TBLE R802.5.2 | FACE NAIL |
| COLLAR TIE TO RAFTER, FACE NAIL OR 1 ½" x 20 GA. RIDGE STRAP TO RAFTER | 4-10d (3" x 0.128") | FACE NAIL, EACH RAFTER |
| RAFTER OR ROOF TRUSS TO PLATE | 3-16d BOX NAILS (3½" x 0.135") OR 3-10d COMMON NAILS (3" x 0.148") | 2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS |
| ROOF RAFTERS TO RIDGE, VALLEY, OR HIP RAFTERS OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM | 4-16d (3 ½" x 0.135") - TOENAIL; 3-16d BOX (3 ½" x 0.135") - END NAIL | TOENAIL, END NAIL |
| | WALL | |
| STUD TO STUD (NOT AT BRACED WALL PANELS) | 10d (3" x 0.128") | 16" O.C. FACE NAIL |
| STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS) | 16d (3 ½ " x 0.135") | 12" O.C. FACE NAIL |
| BUILT-UP HEADER, TWO PIECES WITH 1/2" SPACER | 16d (3 ½ " x 0.135") | 12" O.C. EACH EDGE FACE NAIL |
| CONTINUOUS HEADER TO STUD | 4-8d (2 ½ " x 0.131") | TOENAIL |
| TOP PLATE TO TOP PLATE | 10d (3" x 0.128") | 12" O.C. FACE NAIL |
| DOUBLE TOP PLATE SPLICE | 8-16d COMMON (3 ∮ " x 0.162") | FACE NAIL ON EACH SIDE OF END JOINT (MIN. 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT) |
| BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (NOT AT BRACED WALL PANELS) | 16d COMMON (3 ½" x 0.162") | 16" O.C. FACE NAIL |
| BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (AT BRACED WALL PANEL) | 3-16d BOX (3 ½" x 0.135") | 3 EACH 16" O.C. FACE NAIL |
| TOP OR SOLE PLATE TO STUD, END NAIL | 4-8d BOX (2½" x 0.113") - TOENAIL; 3-16d BOX (3½" x 0.135") - END NAIL | TOENAIL, END NAIL (SEE LEFT) |
| TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS | 3-10d BOX (3" x 0.128") | FACE NAIL |
| 1" BRACE TO EACH STUD AND PLATE | 3-8d BOX (2 ½" x 0.113") | FACE NAIL |
| 1"x6" SHEATHING TO EACH BEARING | 3-8d BOX (2 ½" x 0.113") | FACE NAIL |
| 1"x8" SHEATHING TO EACH BEARING | 3-8d BOX (2 ½" x 0.113") - FACE NAIL; WIDER THAN 1"x8" - 4-8d BOX (2 ½" x 0.113") | FACE NAIL |
| | FLOOR | |
| JOIST TO SILL, TOP PLATE, OR GIRDER | 4-8d BOX (2 ½" x 0.113") | TOE NAIL |
| RIM JOIST, BAND JOIST, OR BLOCKING TO SILL OR TOP PLATE (ROOF APPLICATIONS ALSO) | 8d BOX (2 ½" x 0.113") | 4" O.C. TOE NAIL |
| 1" x 6" SUBFLOOR OR LESS TO EACH JOIST | 3-8d BOX (2 ½" x 0.113") | FACE NAIL |
| 2" SUBFLOOR TO JOIST OR GIRDER | 3-16d BOX (3 ½" x 0.135") | BLIND AND FACE NAIL |
| 2" PLANKS (PLAN & BEAM - FLOOR AND ROOF) | 3-16d BOX (3 ½" x 0.135") | AT EACH BEARING, FACE NAIL |
| BAND OR RIM JOIST TO JOIST | 3-16d COMMON (3 ½" x 0.162") | END NAIL |
| BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS | 10d BOX (3" x 0.128") | 24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES |
| LEDGER STRIP SUPPORTING JOISTS OR RAFTERS | 4-16d BOX (3 ½" x 0.135") | AT EACH JOIST OR RAFTER, FACE NAIL |
| BRIDGING OR BLOCKING TO JOIST | 2-10d BOX (3" x 0.128") | EACH END, TOENAIL |

FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

| RIPTION OF BUILDING MATERIAL VOOD STRUCTURAL PANELS, SU | SI DESCRIPTION OF FASTENER IBFLOOR, ROOF AND INTERIOR WALL SHEA | EDGE SPACING (INCHES) ATHING TO FRAMING AND PARTICLEBOA | INTERMEDIATE SUPPORTS (INCHE RD WALL SHEATHING TO FRAMING |
|--|--|--|---|
| %" - <u>V</u> " | 6d COMMON (2" x 0.113") NAIL (SUBFLOOR, WALL) 8d COMMON NAIL (ROOF) | 6 | 12 |
| ¹ % ₂ " - 1" | 8d COMMON NAIL (21/2" x 0.131") | 6 | 12 |
| 1 ½ "- 1 ½ " | 10d COMMON (3" x 0.148") NAIL OR 8d (2½" x 0.131") DEFORMED NAIL | 6 | 12 |
| | OTHER WALL | SHEATHING ¹ | |
| " STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING | 1½" GALVANIZED ROOFING NAIL, 76" HEAD DIAMETER, OR 1½" LONG 16 GA. STAPLE WITH 76" OR 1" CROWN | 3 | 6 |
| STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING | 1 3 GALVANIZED ROOFING NAIL, 7 HEAD DIAMETER, OR 1 1 CROWN | 3 | 6 |
| ½ " GYPSUM SHEATHING | 1½" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1½" SCREWS, TYPE W OR S | 7 | 7 |
| % " GYPSUM SHEATHING | 1 % " GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1 % " LONG; 1 % " SCREWS, TYPE W OR S | 7 | 7 |
| W | OOD STRUCTURAL PANELS, COMBINATION | N SUBFLOOR UNDERLAYMENT TO FRAM | ING |
| ¾ " AND LESS | 6d DEFORMED (2" x 0.120") NAIL OR 8d COMMON (2½" x 0.131") NAIL | 6 | 12 |
| % " - 1" | 8d COMMON (2½" x 0.131") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL | 6 | 12 |
| 11/8" - 11/4" | 10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL | 6 | 12 |

LISTED IN THIS TABLE

- CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLABS
- THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION
- PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT.
- FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.
- FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE
- CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A
- MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS
- AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0 $\,$
- REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE) INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND, GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES
- IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB SHALL BE DESIGNED BY A LICENSED ENGINEER
- SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH ½" Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS) AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH. PER IRC SECTION R403.1.6
- FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET
- THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

MINIMUM OF 1/2

- 15. ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 - 2x10's, UNLESS NOTED OTHERWISE
- BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS
- INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A
- ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED **OTHERWISE**
- WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS.
- ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT
- JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE
- COLD-FORMED STEEL JOIST HANGERS JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 1%" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT
- ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3

INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

- ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3. ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER ½ OF VERTICAL DISTANCE BETWEEN CEILING AND
- BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED
- PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH
- A %" GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION) ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi
- ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi
- COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. ½" x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE
- WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE
- ALL ROOF SHEATHING SHALL BE $\frac{7}{16}$ OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

- GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE
- BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0" ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH 1/4" TO 1/4" OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN χ_{50} OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

- 38. PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP
- PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR. INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE

MASONRY VENEER

- MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1½", WITH NOT LESS THAN %" MORTAR OR GROUT COVER TO OUTSIDE FACE.
- VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 🔏 CORRUGATED.
- EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.
- VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

GARAGE NOTES

- DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1%" SOLID CORE OR HONEY-COMBED STEEL
- DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE
- VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC 2018

GARAGE NOTES (CONTINUED)

THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMI IM 5/1 GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM 5/4" TYPE X GYP, BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS

SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH 5/8" GYP. BOARD. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING AND SHALL BE FASTENED WITH 2½"" x 0.120" NAILS AT 7" O.C. STAGGERED WITH (7) 31/4" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER. MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

DESIGN LOADING (PER TABLE R301.5)

| DESIGN LOADING (FER TABLE R. | <u> </u> | | | | | | |
|--|------------------|-------------------------------------|--|--|--|--|--|
| MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS (PSF) | | | | | | | |
| USE | LIVE LOAD | DEAD LOAD | | | | | |
| UNINHABITABLE ATTICS WITHOUT STORAGE | 10 | 10 | | | | | |
| UNINHABITABLE ATTICS WITH LIMITED STORAGE | 20 | 10 | | | | | |
| HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS | 30 | 10 | | | | | |
| BALCONIES (EXTERIOR) AND DECKS | 40 | 10 ^d | | | | | |
| FIRE ESCAPES | 40 | 10 | | | | | |
| GUARDRAILS AND HANDRAILS ^a | 200 ^c | • | | | | | |
| GUARDRAIL IN-FILL COMPONENTS ^b | 50 ^c | • | | | | | |
| PASSENGER VEHICLE GARAGES | 50 | DEPENDENT UPON SLAB CONSTRUCTION | | | | | |
| ROOMS OTHER THAN SLEEPING ROOM | 40 | 10 ^d | | | | | |
| SLEEPING ROOM | 30 | 10 ^d | | | | | |
| STAIRS | 40 | 10 ^d | | | | | |

a. A single concentrated load applied in any direction at any point along the top. b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to

withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load.

d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed

INSULATION/EFFICIENCY

- BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102 1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON

| INCHESTION AND FEMALESTON DESCRIPTION | NAL NITC DV COMPONENT /TABLE NIA400 4 4 |
|--|---|
| INSULATION AND FENESTRATION REQUIRE CLIMATE ZONE | 4-A |
| FENESTRATION U-FACTOR | 0.35 |
| SKYLIGHT U-FACTOR | 0.55 |
| GLAZED FENSTRATION SHGC | 0.40 |
| CEILING R-VALUE | 49 |
| WOOD FRAME WALL R-VALUE | 15 |
| MASS WALL R-VALUE | 8 / 13 |
| FLOOR R-VALUE | 19 |
| BASEMENT WALL R-VALUE | 10-CONTINUOUS OR 13-CAVITY |
| SLAB R-VALUE AND DEPTH | 10 AT 2'-0" |
| CRAWL SPACE WALL R-VALUE | 10-CONTINUOUS OR 13-CAVITY |
| DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE | 8 |
| DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE | 6 |
| CATHEDRAL VAULTED CEILING R-VALUE | 38 |

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2018 IRC. **EXCEPTIONS:**

- AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT SEALS.
- WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE. THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN

DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING: POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM

- PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST.
- ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 $\,$ SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA.

EXCEPTION: THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

| ME | CHANICAL VENTILATIO | N SYSTEM FAN EFFICA | CY |
|---------------------------|--------------------------------|--------------------------------|--------------------------------|
| FAN LOCATION | AIR FLOW RATE MINIMUM (CFM) | MINIMUM EFFICACY (CFM/WATT) | AIR FLOW RATE MAXIMUM (CFM) |
| RANGE HOODS | ANY | 2.8 | ANY |
| IN-LINE FAN | ANY | 2.8 | ANY |
| BATHROOM, UTILITY ROOM | 10 | 1.4 | 90 |
| BATHROOM, UTILITY ROOM | 90 | 2.8 | ANY |

| 20,12,110 21, 2, 10,111 100,1112 101,111 | ESTABLE TERMINATION AND THE SOURCE TERMINATION A | | | | | | | | |
|--|--|----------------------------|--|----------------------------|--|--|--|--|--|
| DIMENSIONAL LUMBER REAM SIZE (TVRE | MULTIPLE-PLY WOOD BEAM FASTENING SCHEDULE | | | | | | | | |
| DIMENSIONAL LUMBER BEAM SIZE/TYPE | FASTENERS | LVL BEAM SIZE/TYPE | FASTENERS | LVL BEAM SIZE/TYPE | FASTENERS | | | | |
| (2) 2x | (2) ROWS 10d @ 12" O.C. ONE SIDE | (2) 1 ¾" UP TO 11 ⅓" DEPTH | (2) ROWS 16d @ 12" O.C. ONE SIDE | (3) 1 ¾" x 14"+ DEPTH | (3) ROWS 16d @ 12" O.C. BOTH SIDES | | | | |
| (3) 2x | (2) ROWS 10d @ 12" O.C. BOTH SIDES | (2) 1 ¾" 14"+ DEPTH | (3) ROWS 16d @ 12" O.C. ONE SIDE | (4) 1 ¾" UP TO 11 ⅓" DEPTH | (2) ROWS ½" x 5" SIMPSON SDS OR SDWS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM BOTH SIDES | | | | |
| (4) 2x | (2) ROWS ¼" x 5" SIMPSON SDS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM, BOTH SIDES | (3) 1 ¾" UP TO 11 ½" DEPTH | (2) ROWS OF 16d @ 12" O.C. BOTH SIDES | (4) 1 ¾" x 14"+ DEPTH | (3) ROWS ¼" x 5" SIMPSON SDS OR SDWS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM BOTH SIDES | | | | |



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RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW **DEVELOPMENT SERVICES** LEE'S SUMMIT, MISSOURI 11/28/2023 4:23:10

RESIDENTIAL SEISMIC & WIND ANALYSIS

| DETERMINE WEIGHT OF HOUSE: | | | | INPUT CALCULATED VALUE |
|-------------------------------------|------------------|------------------|-------------------------|---------------------------|
| LOCATION | | DEAD LOAD (psf) | AREA (ft ²) | WEIGHT (lbs.) |
| ROOF | | 10 | 2714 | 27140 |
| CEILING | | 10 | 2714 | 27140 |
| SECOND FLOOR | | 10 | 1330 | 13300 |
| FIRST FLOOR | | 10 | 2714 | 27140 |
| | WALL LENGTH (ft) | WALL HEIGHT (ft) | WALL UNIT WT. (psf) | WEIGHT (lbs) |
| SECOND FLOOR EXT. WALL DL | 191.5 | 9 | 9 | 15511.5 |
| FIRST FLOOR EXT. WALL DL | 219.34 | 10 | 10 | 21934 |
| | | DEAD LOAD (psf) | AREA (ft2) | WEIGHT (lbs) |
| SECOND FLOOR INT. PARTITION WALL DL | | 6 | 1330 | 7980 |
| FIRST FLOOR INT. PARTITION WALL DL | | 6 | 2714 | 16284 |

| | PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED) | | | | | | | | |
|-------------|---|--------|------------|---------------------|-----------|------|---------------------------------------|--|--|
| | FRONT-TO-BACK | | | | SIDE-TO-S | IDE | | | |
| | AREA | LOAD | | | AREA | LOAD | | | |
| SLOPED ROOF | 403 | 1732 | | SLOPED ROOF | 128 | 564 | | | |
| VERT. ROOF | 32 | 438 | CUMULATIVE | VERT. ROOF | 216 | 3012 | CUMULATIVE | | |
| 2ND | 470 | 6661 | 8831 | 2ND | 487.5 | 6865 | 10441 | | |
| 1ST | 638 | 8736 | 17567 | 1ST | 568.37 | 7925 | 18366 | | |
| | | | | F) - PER ASCE CH. 6 | | | | | |
| | SLOPED ROOF | ZONE B | 5.9 | | ZONE C | 11.6 | 2a (FIG. 28.6-1, ASCE7) | | |
| | WALL/VERT. ROOF | ZONE A | | 17.4 | ZONE D | 3.4 | 10.334 | | |
| 1 | MEAN ROOF HT., h | | 26 | | · · | | · · · · · · · · · · · · · · · · · · · | | |

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area

 q_{z10_ASD} =0.6 q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012) q_{z10} =0.00256 $K_zK_{zt}K_dV^2$ (ASCE7-10 Velocity Pressure)

2ND FLOOR TRIBUTARY WEIGHT 1ST FLOOR TRIBUTARY WEIGHT $\mathrm{S}_{\mathrm{S}}\left(\mathrm{SITE}\;\mathrm{GROUND}\;\mathrm{MOTION}$ - $\%\mathrm{g}$ - FROM ASCE7 SEISMIC MAP) 12.0% F_a (from ASCE7 Table 11.4-1) S_{DS} (= 2/3 * S_S * F_a) 0.128 R (from ASCE7 Table 12.2-1)

| | SEISMIC SHE | <u>EAR</u> | |
|-------------|-------------|---------------------------------------|--|
| LOCATION | | From ASCE7 (Eq. 12.8-1): | V (= 1.2 * S _{DS} * W / R) (lbs.) |
| 2ND FLOOR | | | 1466 |
| 1ST FLOOR | | | 2411 |
| | + | · · · · · · · · · · · · · · · · · · · | |

| Sheathing Location | Min. Sheathing Schedule | Fastening Schedule | Allowable Shear (#/LF) | Code Reference |
|----------------------|---|--|------------------------|-----------------------------|
| Exterior (Option #1) | 7/16" APA Rated Plywcod/OSB | 1-1/2" 16gs. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" atud spacing, 12" OC Field For 16" atud spacing | 155 | per IBC, Table 2306.3(1) |
| Exterior (Option #2) | 7/16" APA Rated Plywood/OSB | 1-1/2" 16gs. Staples w/ 1" penstration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud apacing | 230 | per IBC, Table 2306.3(1) |
| Exterior (Option #3) | 7/16" APA Rated Plywood/OSB | 1-1/2" 16gs. Steples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud specing, 12" OC Field For 16" stud specing | 310 | per IBC, Table 2306.3(1) |
| Exterior (Option #4) | 7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing | 8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing | 220 | AF&PA SDPW Table 4.3A |
| Exterior (Option #5) | 7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing | 8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing | 320 | AF&PA SDPW: Table 4.3A |
| Exterior (Option #6) | 7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge | 8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field | 410 | AF&PA SDPW: Table 4.3A |
| Interior | 1/2" Gypsum Board | No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field | 60 | per IBC, Table 2306.4.4 |
| Interior | 16 Ga. Simpson/USP Type WB Steel X-Brace (or equal) | (3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3) | 325 | |

| EXTERIOR SHEATHING OPTION FOR SECOND FLOOR | 4 |
|--|---|
| EXTERIOR SHEATHING OPTION FOR FIRST FLOOR | 4 |

| WIDTH OF 1ST STORY (FT.) | 58 | WIDTH OF 2ND STORY (FT.) |
|---------------------------|-------|--------------------------|
| DEPTH OF 1ST STORY (FT.) | 51.67 | DEPTH OF 2ND STORY (FT.) |
| BACK WALL OF GARAGE (FT.) | 0 | . |
| GAR. WALL: 1=F-B, 2=S-S | 2 | |

| | | | EXTER | RIOR STRUCTURAL WALL I | LENGTHS (ft.) & RESISTANCES | - | . | |
|--------------------|---------------|-------------------|----------------|------------------------|-----------------------------|-------------------|-----------------------------|--------------------|
| | | SE | ISMIC | | | WIND | | |
| | FRONT-TO-BACK | RESISTANCE (lbs.) | SIDE-TO-SIDE | RESISTANCE (lbs.) | FRONT-TO-BACK | RESISTANCE (lbs.) | SIDE-TO-SIDE | RESISTANCE (lbs.) |
| 2ND FLOOR | 49 | 13720 | 58 | 16240 | 49 | 19208 | 58 | 22736 |
| 1ST FLOOR | 87 | 24360 | 60 | 16800 | 87 | 34104 | 60 | 23520 |
| | | | | | | | | |
| | | ADDITIONAL RESIS | TANCE REQUIRED | | Anchor Bolt Spacing | (in.) | 16d Nail Spacing req'd at I | oottom plate (in.) |
| | | SEISMIC | WIND | | diameter (in.) | 0.5 | 2nd Floor F-B | 32 |
| 2ND FLOOR FRONT-T | O-BACK | 0 | 0 | | Shear value (per NDS) | 944 | 2nd Floor S-S | 30 |
| 2ND FLOOR SIDE-TO- | SIDE | 0 | 0 | | Spacing F-B (inches) | 106.6 | 1st Floor F-B | 16 |
| 1ST FLOOR FRONT-T | O-BACK | 0 | 0 | | spacing S-S (inches) | 114.5 | 1st Floor S-S | 17 |
| 1ST FLOOR SIDE-TO- | SIDE | 0 | 0 | ' | | | | |

| | | RESISTANCE REQUI | RED IN ADDITION TO RES | SISTANCE PROVIDED BY EXTERIOR W | /ALLS** | | |
|-------------------------|---|--|-----------------------------------|--|---|--|-----|
| | ADDITIONAL RESISTANCE REQUIRED (POUNDS) | PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE | INTERIOR X-BRACES (325#/BRACE) | INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) | INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.) | RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS) | OK? |
| 2ND FLOOR FRONT-TO-BACK | 0 | | | | | 0 | YES |
| 2ND FLOOR SIDE-TO-SIDE | 0 | | | | | 0 | YES |
| 1ST FLOOR FRONT-TO-BACK | 0 | | | | | 0 | YES |
| 1ST FLOOR SIDE-TO-SIDE | 0 | | | | | 0 | YES |

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE),

2) SEE SHEET ST FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING
PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2-8" OR LONGER
ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRE

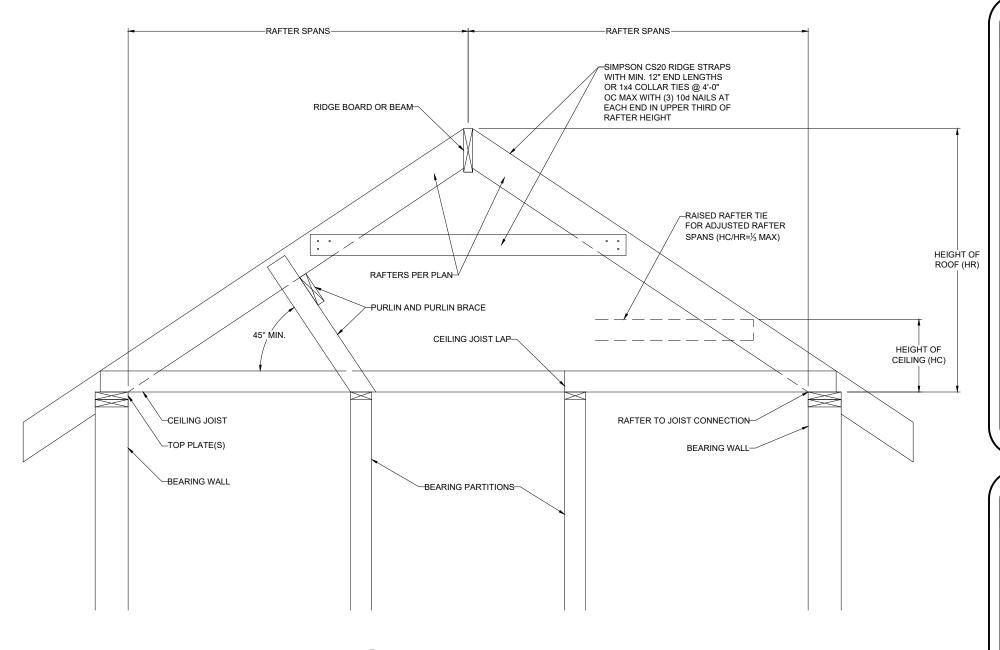
| ALL LATERAL BRACE | NO ACHIEVED AT EXT | ENION WALLO AND WA | LEG DIRECTET ON TO | MDATIONS, THERE ORE, | NO INTERIOR BRACING PER 2012 IF | CO OLOTION ROUZ.Z.T IOT | (E&OII)E |
|---------------------|-------------------------------|--------------------------------|--------------------------------|---------------------------|---------------------------------|-------------------------|---------------------------------------|
| | | | | WIND UPLIFT | ANALYSIS | | |
| | X/12 | DEGREES | | | | | |
| ROOF PITCH (MAX) | 6 | 26.6 | PITCH OF 6 OR LESS: I | EOH -13.3, E -7.2, G -5.2 | | | |
| | | ASCE 7 | | | | | |
| | LENGTH (FT.) | PRESSURE (PSF) | LINEAL FT. OF OH | UPLIFT PER FT* (LBS) | | | |
| OVERHANG | 1 | 16.56 | 221.34 | 16.56 | | | |
| | TOTAL AREA (FT ²) | ZONE E AREA (FT ²) | ZONE G AREA (FT ²) | PRESSURE ZN. E (PSF) | PRESSURE ZN. G (PSF) | TOTAL FORCE (LBS) | FORCE PER LINEAL FT @ PERIMETER (LBS) |
| MAIN ROOF** | 2996.86 | 1551.794776 | 1445.065224 | 15.12 | 10.5 | 38636 | 176.1 |
| | | | | | | | |
| *ALONG PERIMETER | | TOTAL UPLIFT PER LINEAL | FOOT ALONG EXTERIOR (PO | UNDS) | 192.7 | UPLIFT OK | |
| **INSIDE EXTERIOR W | VALLS | RESISTANCE DUE TO DEAD | WEIGHT & (3) 10d TOENAILS | 1 | 251.6 | | |

NOTE FOR CONSTRUCTION

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARI DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING



BRACED RAFTER CONSTRUCTION S1.1 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

Combustion Air Calculation

Per 2018 IRC Section G2407.5

Required combined area:

100000 BTU/h Appliance #1 Furnace Appliance #2 BTU/h 50000 BTU/h Appliance #3 Water Heater Total BTU/hr 150000 BTU/h

1162 ft² Area of Combined Space (floor where appliances are located) Ceiling Height in Usable Space 8.5 ft

Note: Per 2018 IRC Section G2407.5.3.2, The volumes of spaces in different stories shall be considered as communicating spaces where such spaces are connected by one or more openings in doors or floors having a total minimum free area of 2 square inches per 1,000 BTU/h of total input rating of all appliances

Yes Is floor where appliances are located open to adjacent level? If Yes, what is the area of open space adjacent to appliance area?

Per 2018 IRC Section G2407.5.1 (Standard Method), the minimum required volume shall be 50 cubic feet per 1,000 BTU/hr (Total BTU/hr / 1,000 BTU/hr x 50 ft³)

Area of Combined Space > Required combined area?

Required air space in combined areas: 7500 ft³

882 ft²

OK

Per Section G2407.5.3.1, each opening shall have a minimum free area of 1 square inch per 1,000 BTU/hr of the total input rating of all appliances in the space, but not less than 100 square inches. One opening shall commence within 12 inches of the top and one opening shall commence within 12 inches of the bottom of the enclosure. The minimum dimension of air openings shall be not less than 3 inches.

150 in² Minmum required opening area: Minimum grill size: 14 x 11 (inches) Note: two grills required - one within 12" of floor, one within 12" of clg.

SPEC SUMMIT VIEW FARMS SVF083 :

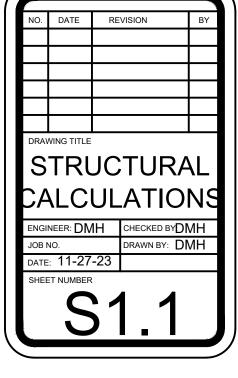
2307 SW SERENA LEE'S SUMMIT, MI

WOOD BROTHERS, INC

CLIENT:

JOB

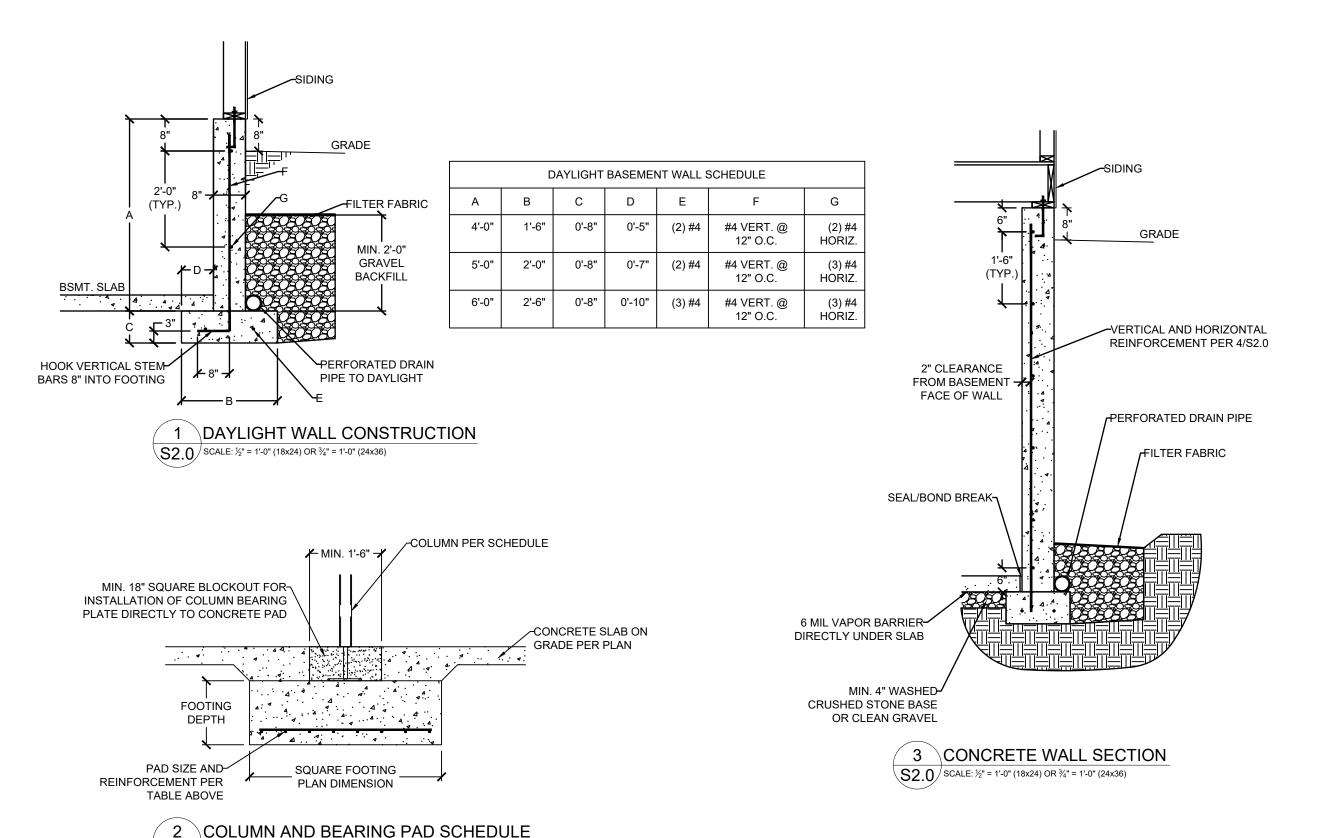




\$2.0 SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)

MIN. (2) #4 BARS EXTENDING 24"

PAST OVER-EXCAVATION AND INTO INTERSECTING WALL



VERTICAL REINFORCEMENT SPACING 10" THICK WALL CONCRETE STRENGTH/GRADE 8" THICK WALL REINFORCEMENT (#4 BARS) 9' 9' 10' 8' 10' 8' 3.000 PSI/ GRADE 40 24 24 16 24 24 18 3,500 PSI/ GRADE 40 24 24 18 16 24 24 3,000 PSI/ GRADE 60 24 24 24 18 16 24 3,500 PSI/ GRADE 60 24 24 24 18 24 16 HORIZONTAL REINFORCEMENT - MINIMUM GRADE 40 STEEL ONE BAR 12" FROM TOP OF WALL; 6-#4 7-#4 7-#4 6-#4 7-#4 7-#4 MAX. SPACING 24" OC

FOOTNOTES:

1) WALL HEIGHT IS MEASURED FROM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB 2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE WALL. OTHER WALLS SHALL HAVE VERTICAL REINFORCEMENT AS FOLLOWS:

A) 8" WALL - MINIMUM 5" FROM THE OUTSIDE FACE

B) 10" WALL - MINIMUM 63/4" FROM THE OUTSIDE FACE C) EXTEND BARS TO WITHIN 8" OF THE TOP OF THE WALL

3) REINFORCEMENT CLEARANCES:

A) CONCRETE EXPOSED TO EARTH - MINIMUM 11/2"

B) NOT EXPOSED TO WEATHER (INTERIOR SIDE OF WALLS) -3/4" C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY

SLABS) - 1½" 4) HORIZONTAL RÉINFORCEMENT:

A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL

B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE

D) SUPPLEMENTAL REINFORCEMENT AT CORNERS - PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF

THE EDGE OF INSIDE CORNERS. 5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND CORNERS.

6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE 31/2". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP

7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS

8) WALL SHALL NOT BE BACKFILLED UNTIL FLOOR SYSTEM AND DIAPHRAGM ARE IN PLACE

4 \FOUNDATION WALL REINFORCEMENT TABLE S2.0/NO SCALE



-SLAB PER PLAN, IF APPLICABLE

PER PLAN

-REBAR PER

5" INTO WALL

PLAN, DRILLED

PER PLAN

8 \CONCRETE GRADE BEAM

S2.0 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

3" CLEAR (TYP.)

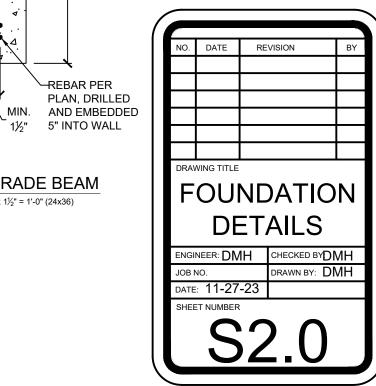
CLEAR-

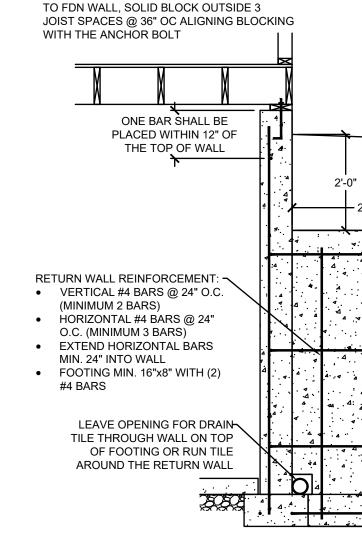
(TYP.)



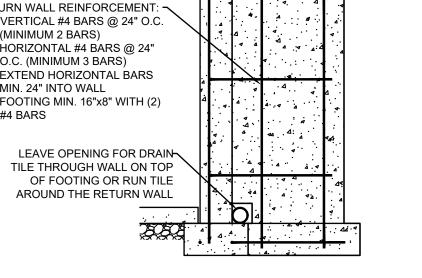
FARMS 7 SW SERENA I S'S SUMMIT, MIS SPEC SUMMIT SVF083 LOT 83, 2307 LEE'8 JOB

DENNIS HEIER PE-2010001772 11-27-2023

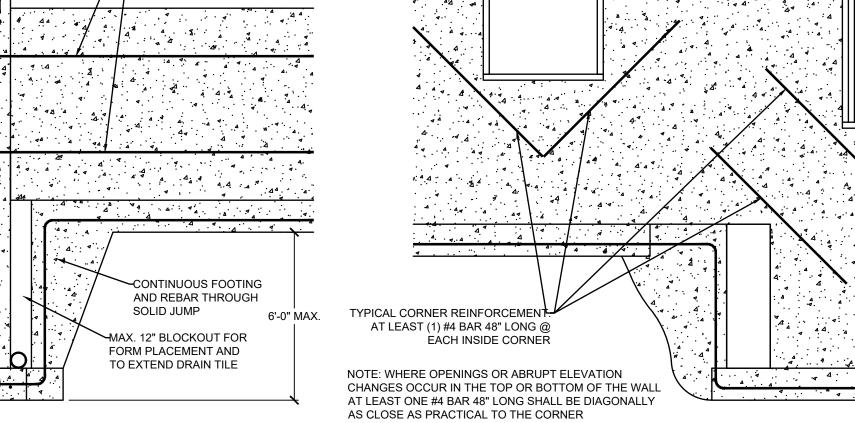




NOTE: WHERE FLOOR JOIST RUNS PARALLEL

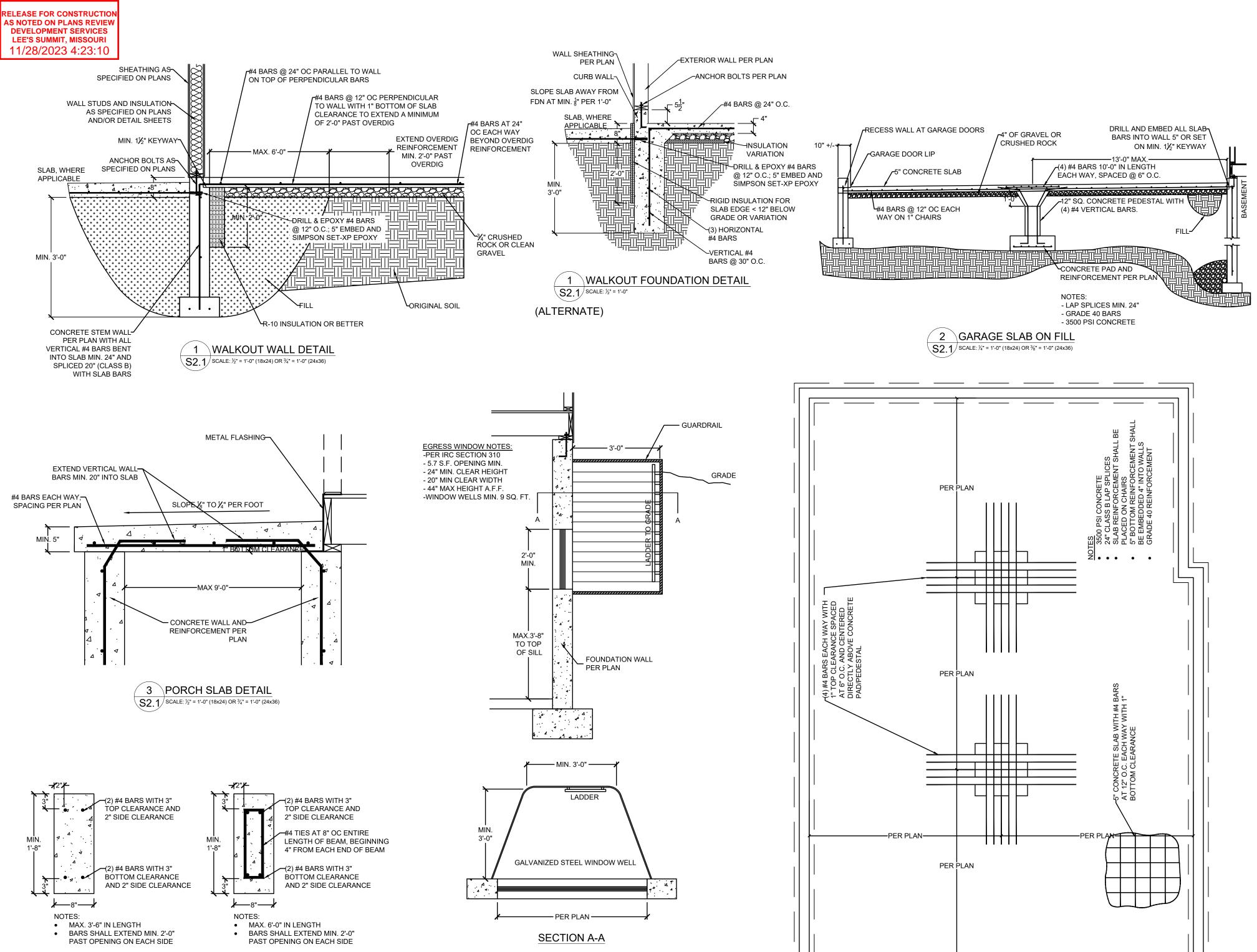


7 RETURN WALL DETAIL S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)



6 REINFORCEMENT AT OPENING CORNERS 5 \SOLID JUMP S2.0/AND STEP CORNERS @ INSIDE CORNERS \$2.0\scale: \frac{10}{2} = 1'-0" (18x24) OR \frac{3}{4}" = 1'-0" (24x36)

SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)



5 EGRESS WINDOW WELL ELEVATION AND PLAN DETAILS

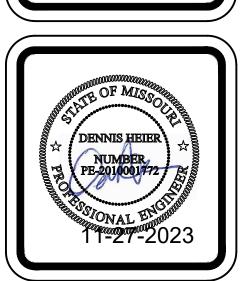
S2.1 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

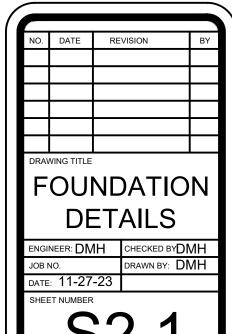
4 \CONCRETE HEADER DETAILS

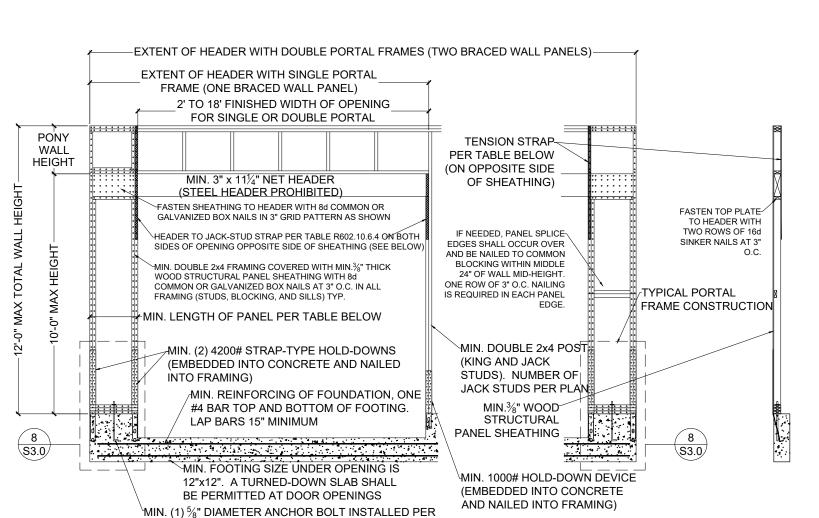
S2.1 SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)

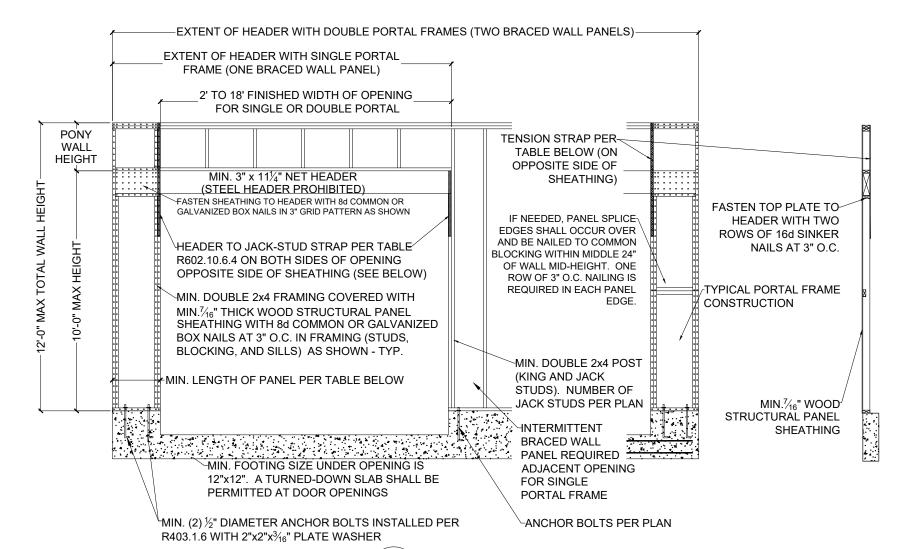
ENGINEERING, LLC
14718 NW DELIA STREET * PORTLAND, OREGON 97229
OFFICE; 971,255,6099 * MOBILE; 971,255,6099 *
EMAIL; DENNIS@VISTASTRUCTURAL,COM

SLIENT: WOOD BROTHERS, INC
JOB TITLE: SVF083 SPEC
LOT 83, SUMMIT VIEW FARMS - 4TH PLAT
LOCATION: 2307 SW SERENA PL.
LEE'S SUMMIT, MISSOURI







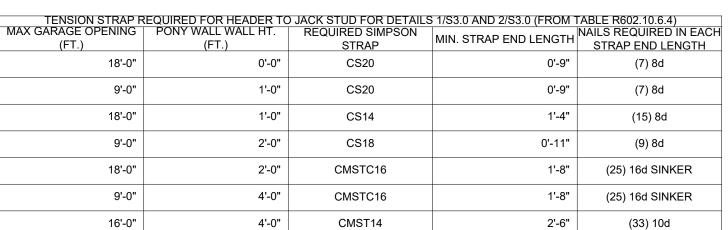


R403.1.6 - WITH 2"x2"x3/16" PLATE WASHER 1 \METHOD PFH (PORTAL FRAME WITH

S3.0/HOLD-DOWNS) - PER FIGURE IRC R602.10.6.2

SCALE: ½" = 1'-0" (18x24) OR 3/8" = 1'-0" (24x36)

| | MINIMUN | | ENGTH F (INCHES) | | AIL 1/S3.0 |
|-------------------------------|---------|--------|---------------------|------|------------|
| | | W | ALL HEIGI | HT | |
| | 8 FEET | 9 FEET | 10 | 11 | 12 |
| | OILLI | SILLI | FEET | FEET | FEET |
| SUPPORTING ROOF ONLY | 16 | 16 | 16 | 18 | 20 |
| SUPPORTING ONE STORY AND ROOF | 24 | 24 | 24 | 27 | 29 |

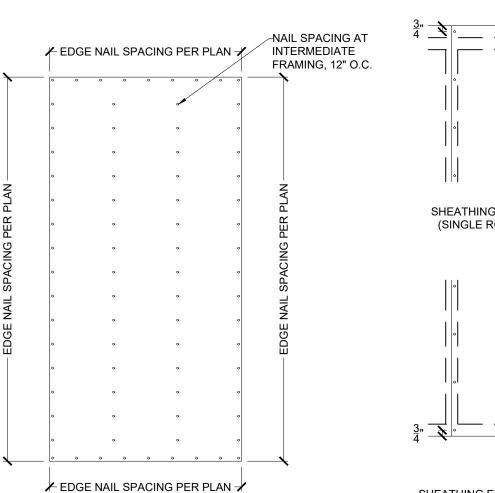




SCALE: 1/4" = 1'-0" (18x24) OR 3/8" = 1'-0" (24x36)

| MINIMUM F | PANEL LENG | GTH FOR D | | 0 (INCHES) |
|-----------|------------|-----------|-----------------|-----------------|
| 8 FEET | 9 FEET | | 11 FEET | 12 FEET |
| 24 | 27 | 30 | 33 ^a | 36 ^a |

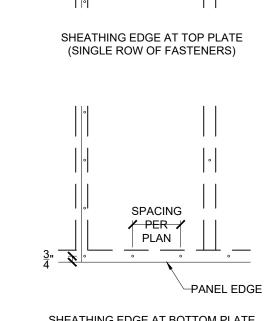
a. Maximum opening height for PFG is 10 feet in accordance with Figure R602.10.6.3, but wall height may be increased to 12 feet with pony wall

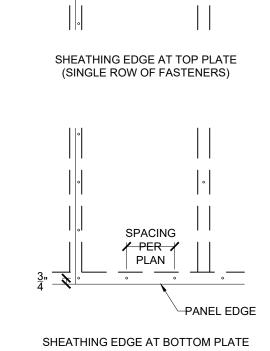


3 EXTERIOR WALL SHEATHING

SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)

S3.0/PANEL ATTACHMENT

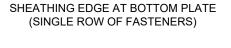




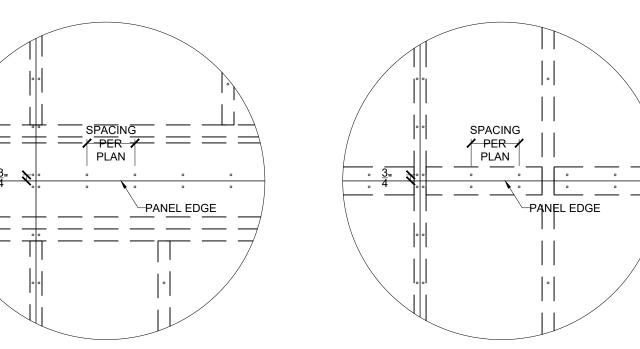
SPACING

/ PER /

PLAN



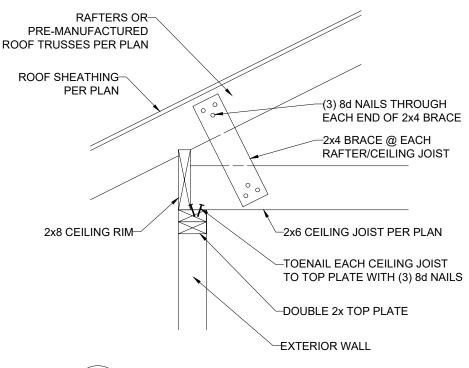
SHEATHING EDGE AT TOP S3.0/AND BOTTOM PLATES SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



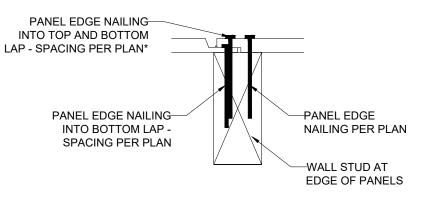


S3.0/FRAMING MEMBER SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



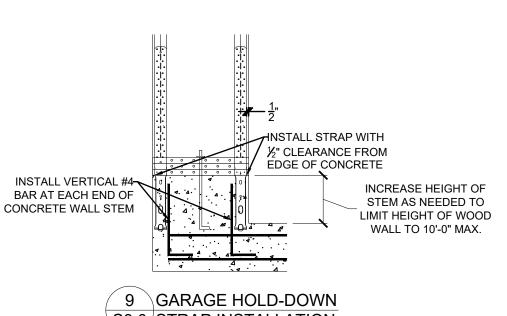


7 RAFTER BEARING OPTION DETAIL $\sqrt{3.0}$ SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

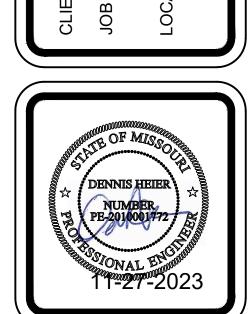


*NOTE: NAILING INTO TOP AND BOTTOM LAP IS IN ADDITION TO NAILING REQUIRED INTO BOTTOM LAP. FOR EXAMPLE, IF PLAN CALLS FOR NAILS @ 6" O.C. AT EDGES, BOTTOM LAP SHALL BE FASTENED AT 6" O.C AND, IN ADDITION, NAILING SHALL ALSO BE INSTALLED THROUGH TOP AND BOTTOM LAP @ 6" O.C. STAGGERED 3" FROM BOTTOM LAP NAILING

8 FASTENING INSTRUCTIONS FOR S3.0/SHIPLAP PANEL SHEATHING SCALE: 4" = 1'-0" (18x24) OR 6" = 1'-0" (24x36)



S3.0/STRAP INSTALLATION SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)



FARMS

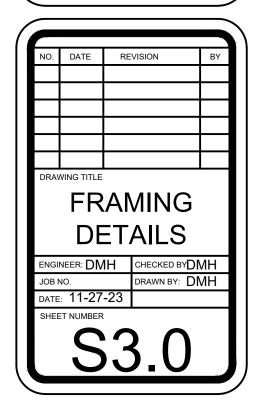
SPEC SUMMIT

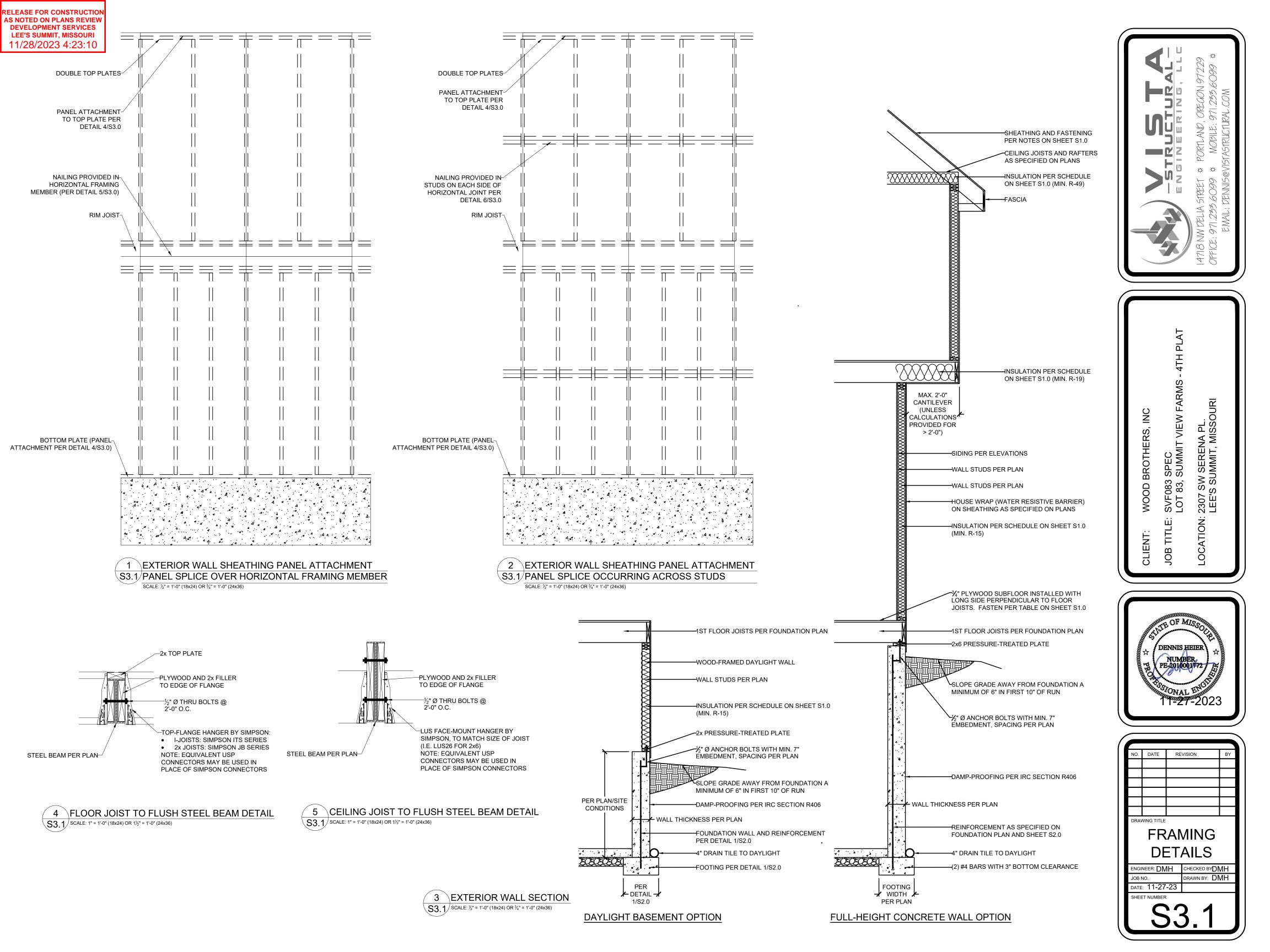
SVF083 LOT 83,

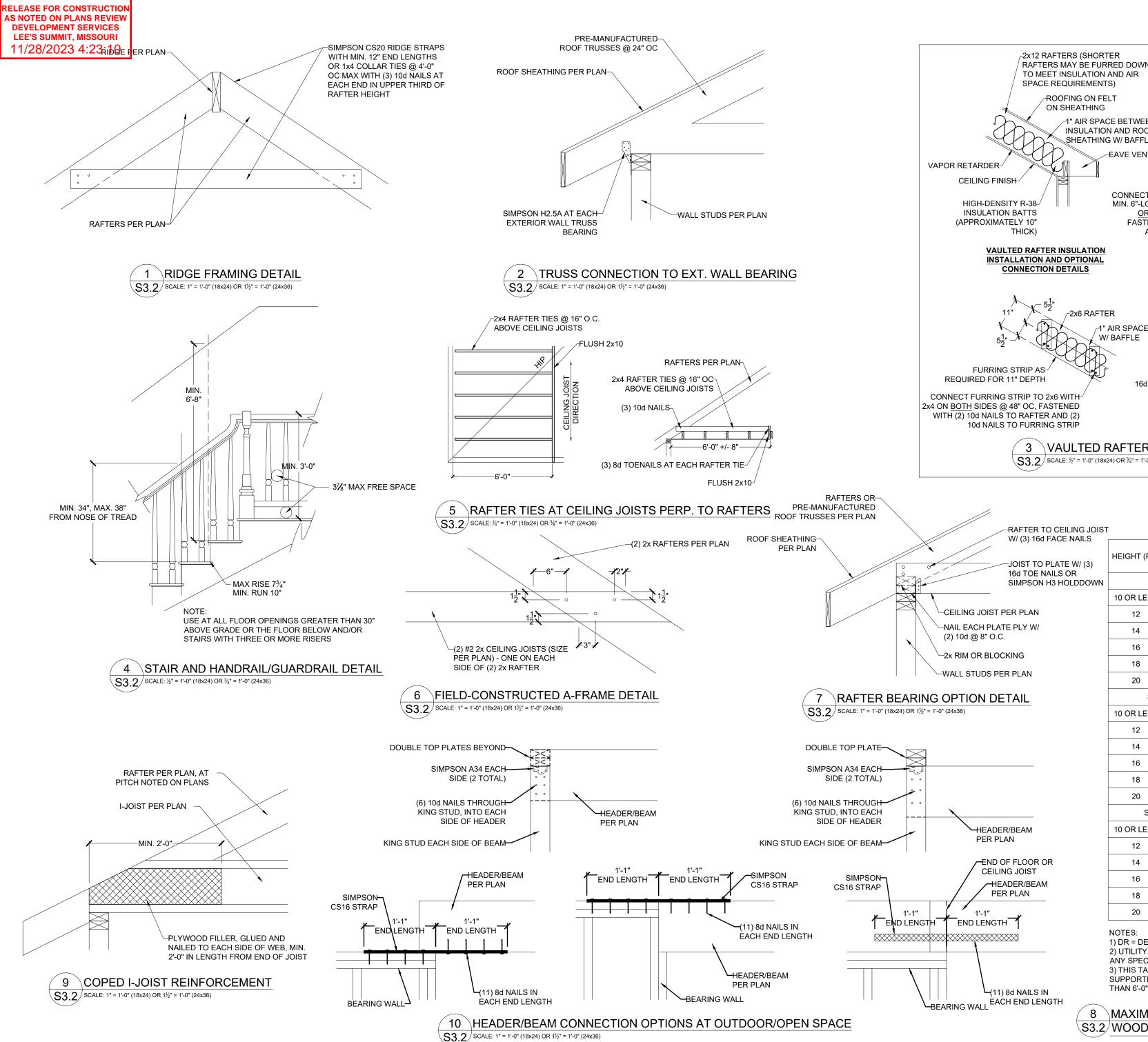
SW 'S SL

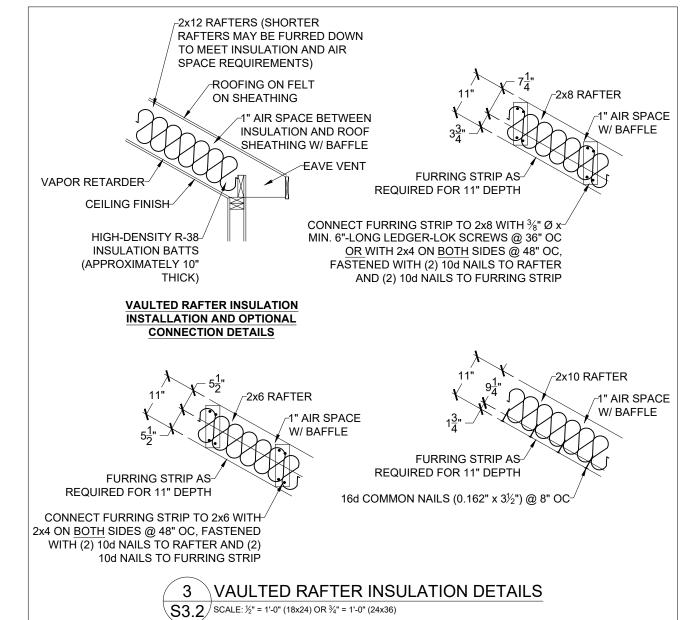
2307 LEE'

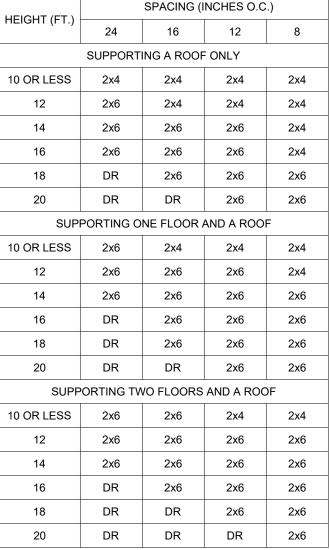
₩.











1) DR = DESIGN REQUIRED 2) UTILITY, STANDARD, STUD AND #3 GRADE LUMBER OF ANY SPECIES ARE NOT PERMITTED 3) THIS TABLE DOES NOT APPLY FOR STUDS SUPPORTING MEMBERS WITH A TRIB. LENGTH GREATER THAN 6'-0"

8 MAXIMUM ALLOWABLE LENGTH OF S3.2/WOOD WALL STUDS (IRC TABLE 602.3.1)

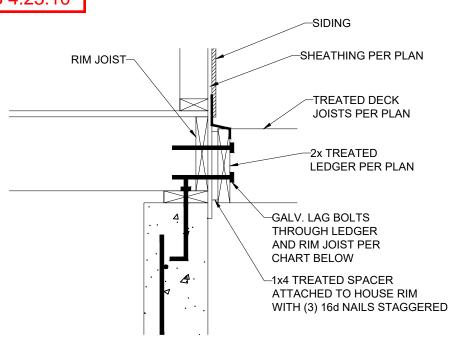


SPEC SUMMIT VIEW FARMS SW SERENA PL. S SUMMIT, MISSOU SVF083 :

WOOD BROTHERS, INC

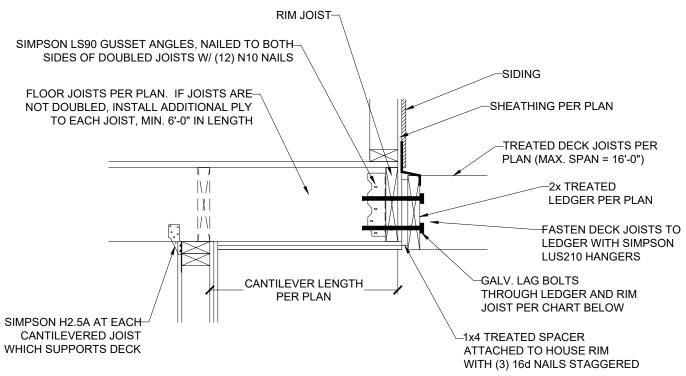


| DRAWING TITLE FRAMING DETAILS ENGINEER: DMH CHECKED BYDM | | | | 100 |
|---|---------------------|--------|----------|---------------|
| FRAMING DETAILS | NO. D | ATE | REVISION | BY |
| FRAMING DETAILS | | | | |
| DETAILS | DRAWING | TITLE | | • |
| | F | FRA | MIN | G |
| ENGINEER: DMH CHECKED BYDM | | DE | TAIL: | S |
| | | | CHECKE | ву DMH |
| JOB NO. DRAWN BY: DM | | R: DMH | | |
| DATE: 11-27-23 | ENGINEEI JOB NO. | | DRAWN E | Y: DMH |
| SHEET NUMBER | ENGINEEI JOB NO. | | DRAWN E | sy: DMH |

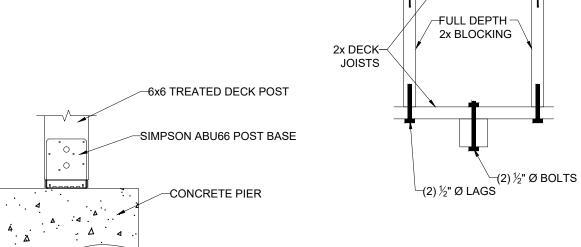


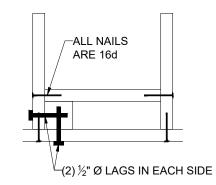
DECK LEDGER ATTACHMENT GUIDE

| DECK JOIST SPAN | 1/2" Ø GALV. LAG OR $3/8$ " Ø LEDGER-LOK SPACING |
|--------------------|--|
| 10'-0" OR LESS | 16" OC |
| 10'-0" - 13'-11" | 12" OC OR @ 16" OC DOUBLED EVERY OTHER |
| 14'-0" - 18'-0" | 8" OC OR @ 16" OC DOUBLED |
| • | |



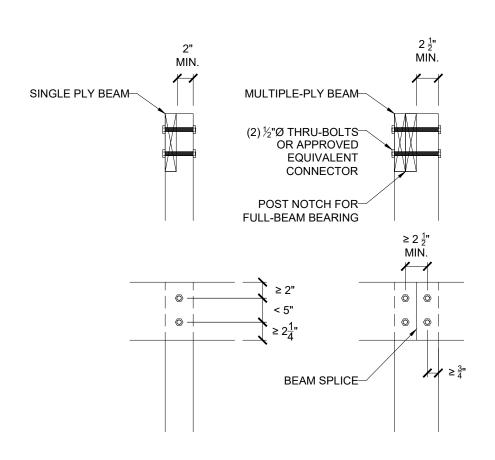
2 CANTILEVER WITH DECK ATTACHMENT



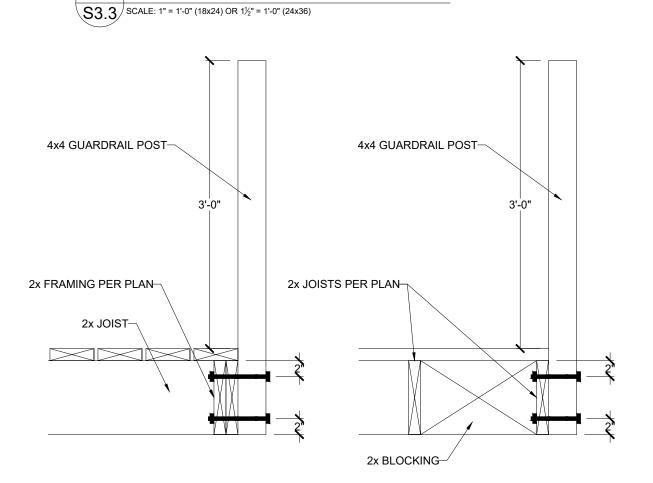


4 REINF. POST CONNECTIONS S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

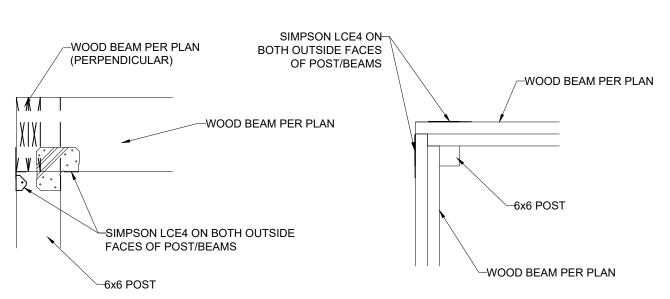
1 LEDGER ATTACHMENT S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



5 LET-IN (COVERED) DECK BEAM CONNECTION S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



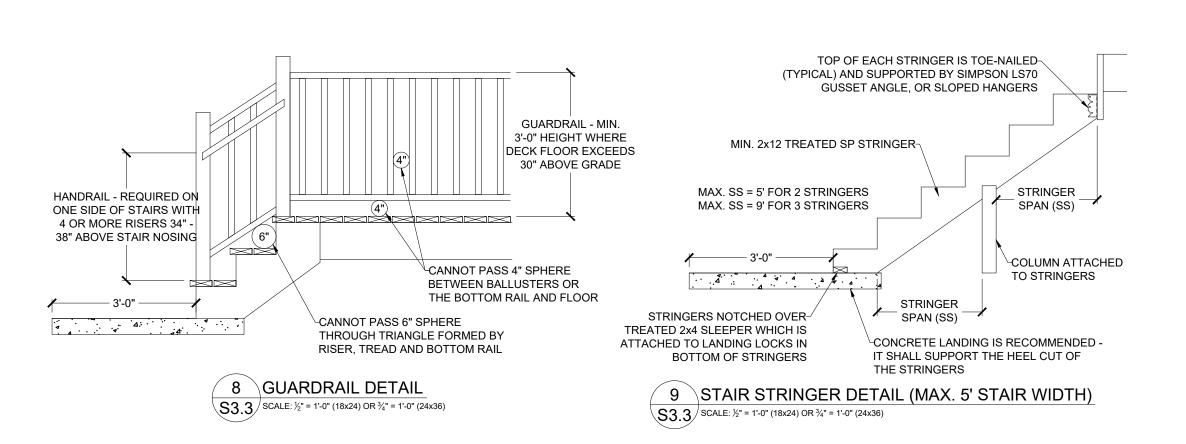
6 GUARDRAIL CONNECTION S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



7 ALTERNATE COVERED DECK/PORCH INTERSECTION S3.3 CORNER BEAM CONNECTION

SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

3 DECK POST BASE \$3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

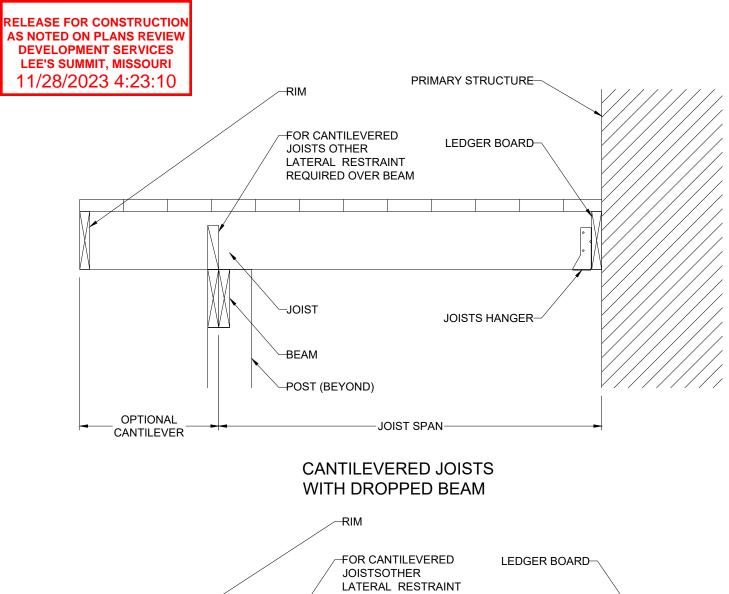


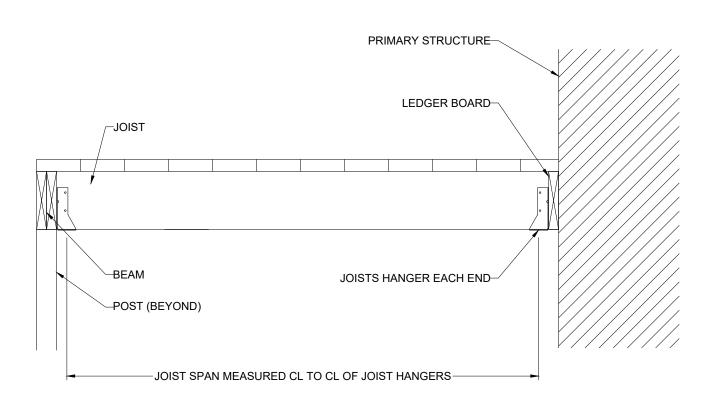


CLIENT: WOOD BROTHERS, INC
JOB TITLE: SVF083 SPEC
LOT 83, SUMMIT VIEW FARMS - 4TH
LOCATION: 2307 SW SERENA PL.
LEE'S SUMMIT, MISSOURI

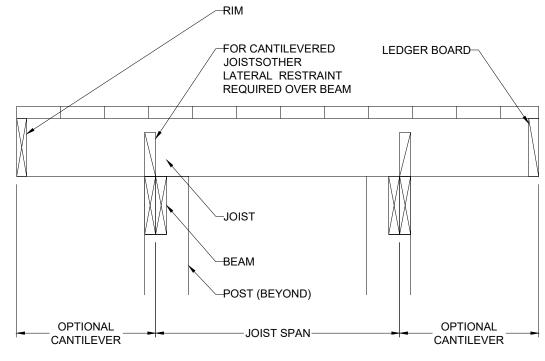


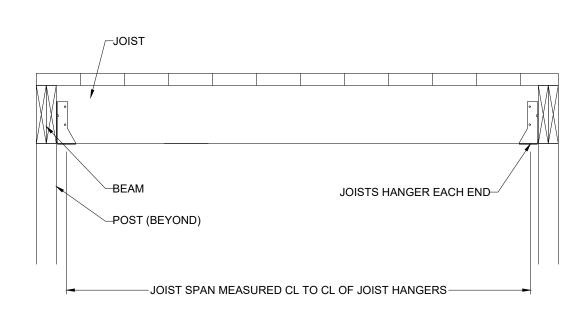
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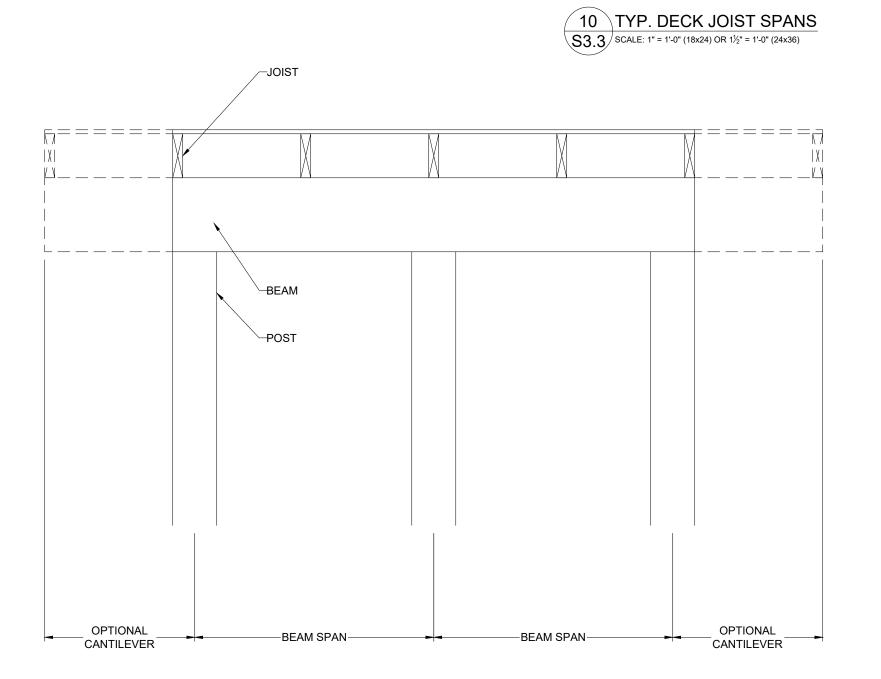
JOISTS WITH FLUSH BEAM

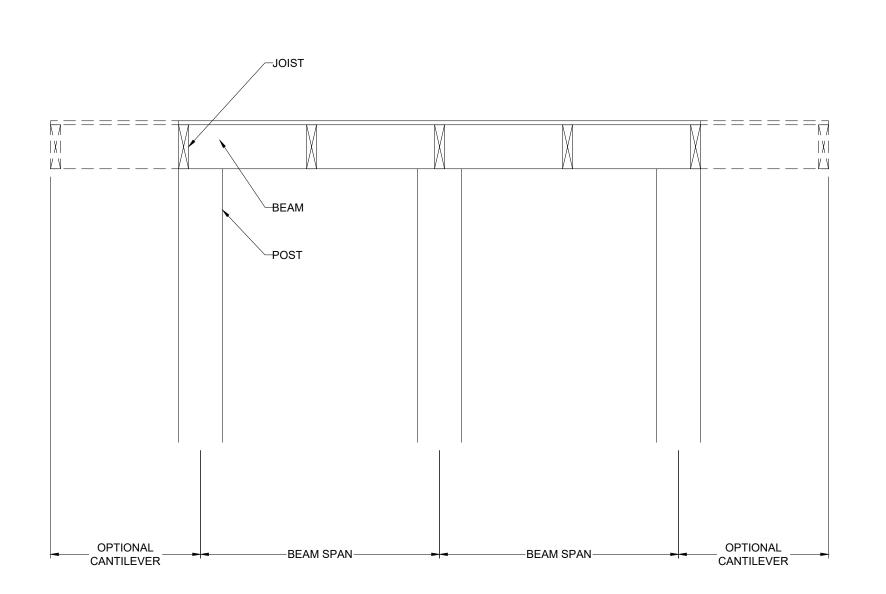




JOISTS ON FREE-STANDING DECK WITH DROPPED BEAM

JOISTS WITH FLUSH BEAM



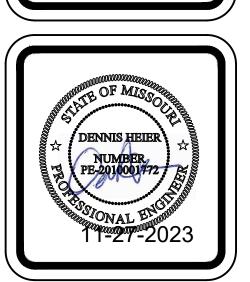


ENGINEERING, LLC

14718 NW PELIA STREET & PORTLAND, OREGON 97229

OFFICE, 971,255,6099 & MOBILE, 971,255,6099 & EMAIL; DENNIS@VISTASTRUCTURAL,COM

CLIENT: WOOD BROTHERS, INC
JOB TITLE: SVF083 SPEC
LOT 83, SUMMIT VIEW FARMS - 4TH
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LEE'S SUMMIT, MISSOURI



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DROPPED BEAM FLUSH BEAM