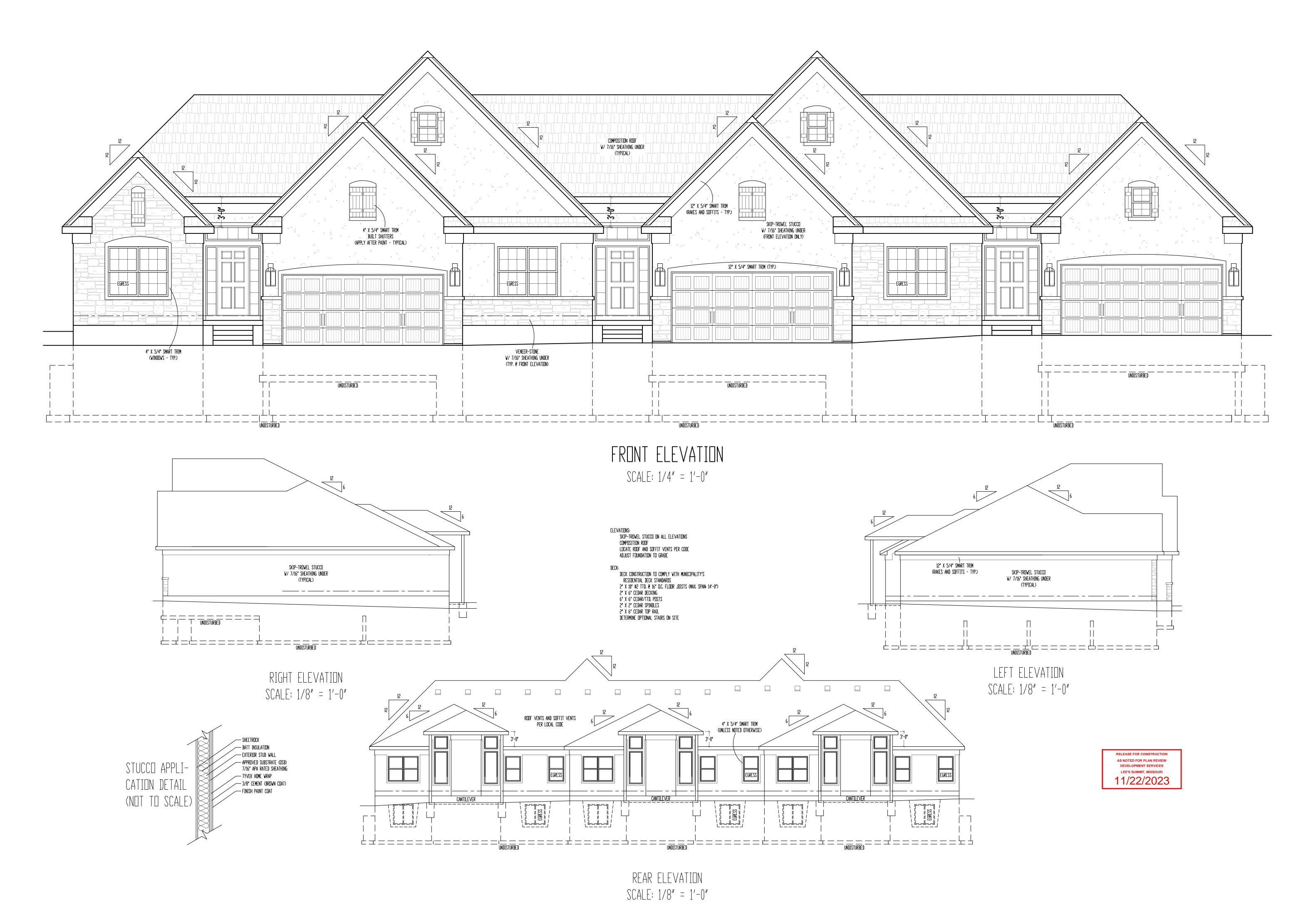
Sheet Title: **ELEVATIONS**

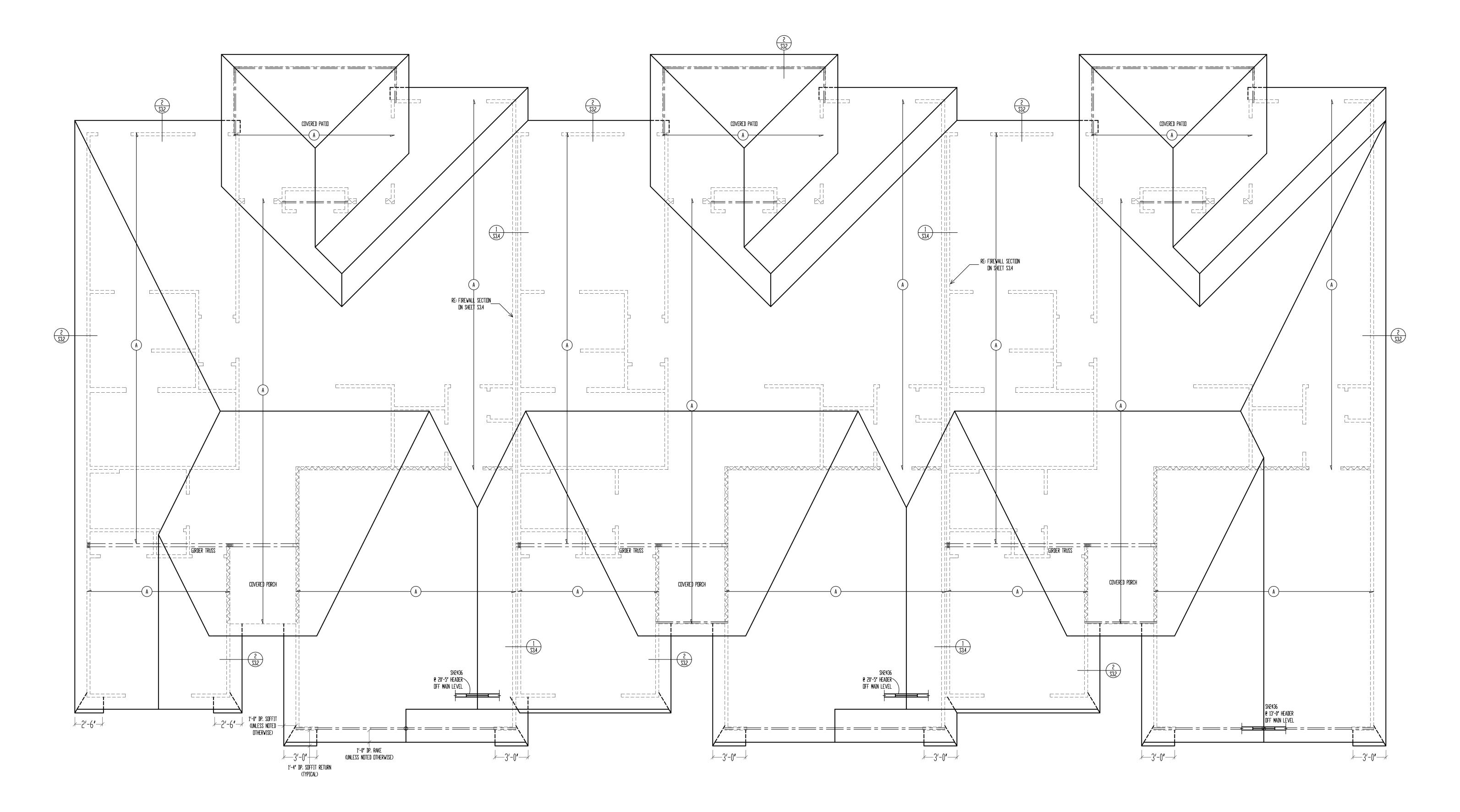
Sheet No.:

A-1
of 4



Sheet Title:
ROOF PLAN





ROOF

SCALE: 1/4" = 1'-0"

TRUSS SCHEDULE

PREMANUFACTURED ROOF TRUSSES @ 24" D.C (SEE SEPARATE LAYOUT BY MANUFACTURER)

ROOF TRUSSES
- ROOF TRUSSES PROPOSED TO BE USED.
- TRUSSES SHALL BE DESIGNED FOR 20 PSF SNOW LOAD, 10 PSF ROOF DEAD LOAD, 10 PSF CEILING LIVE LOAD, AND 5 PSF CEILING DEAD LOAD.
- THE ENGINEER RESPONSIBLE FOR THE STRUCTURAL DESIGN OF THE HOUSE SHALL REVIEW THE TRUSS DRAWINGS FOR GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING, PRIOR TO SUBMITTING THE TRUSS DRAWINGS TO THE CODES ADMINISTRATION OFFICE FOR APPROVAL.
- FAILURE OF THE RESPONSIBLE PARTIES TO SUBMIT THE TRUSS DRAWINGS TO THE RESPONSIBLE ENGINEER SHALL RELIEVE THE ENGINEER OF ALL LIABILITY FOR THE ENTIRE PLAN. TRUSS LOADS AND TRANSFER PATHS ON THIS PLAN ARE ASSUMED LOADS ONLY AND CAN ONLY BE VERIFIED AFTER TRUSS LAYOUTS AND DESIGNS ARE COMPLETED.
- ATTACH EACH END OF EACH TRUSS TO TOP PLATE WITH CONNECTOR RATED FOR MANUFACTURER'S DESIGN UPLIFT LOAD (SEE SEPARATE DESIGN BY MANUF.)
- 2-PLY GIRDER: LGT2

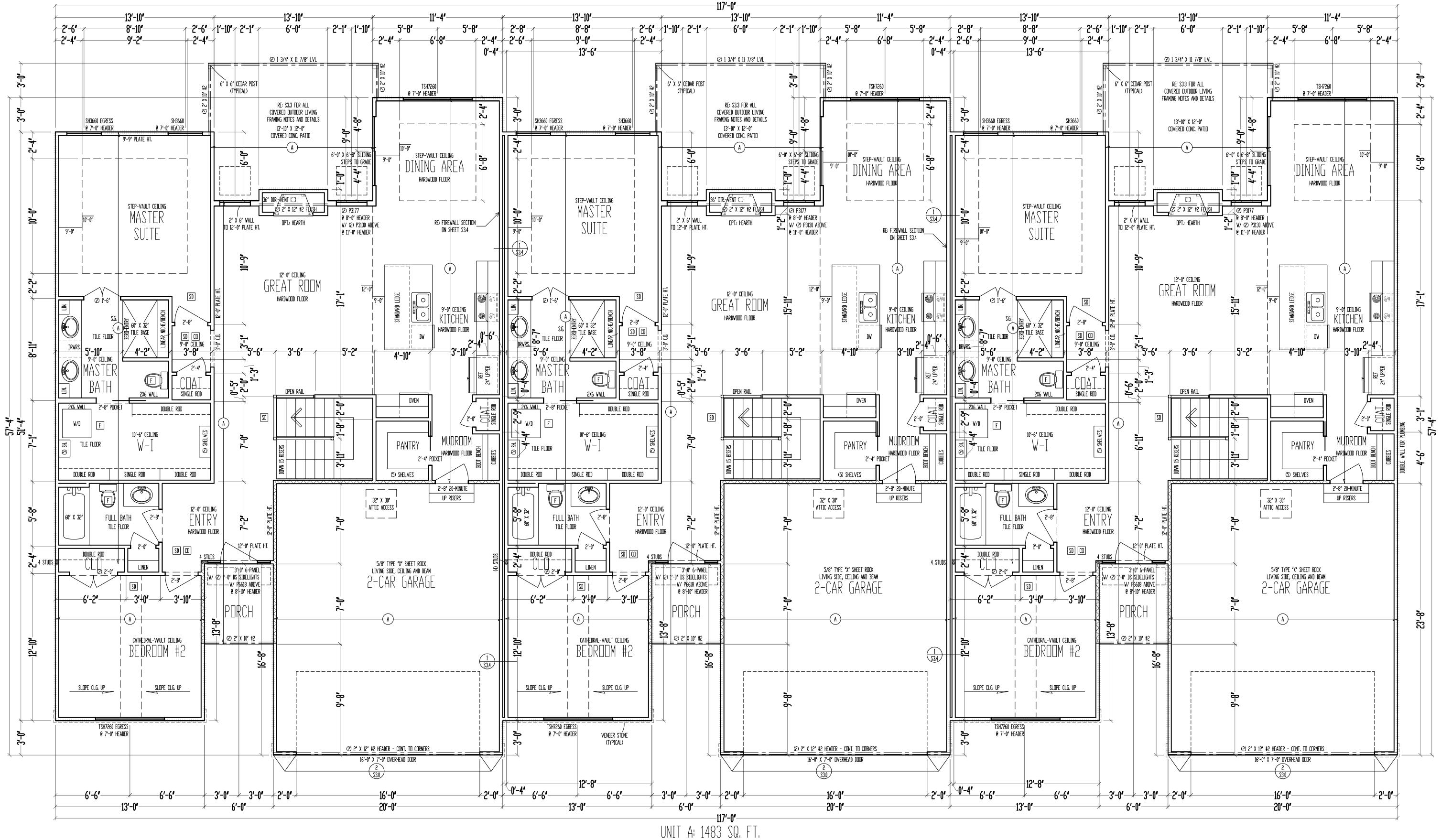
- 3-PLY GIRDER: LGT3-SDS2.5

- 4-PLY GIRDER: LGT4-SDS3

Sheet Title: **MAIN LEVEL**

PLAN Sheet No.:

11/22/2023



9'-0" CEILING SCALE: 1/4" = 1'-0"

(SEE SEPARATE LAYDUT BY MANUFACTURER)

GARAGE A: 472 SQ. FT. TRUSS SCHEDULE PREMANUFACTURED ROOF TRUSSES @ 24" O.C

GARAGE B: 472 SQ. FT. GARAGE C: 472 SQ. FT. COV. DUT/LIV A: 171 SQ. FT. COV. DUT/LIV B: 171 SQ. FT. CDV. DUT/LIV C: 171 SQ. FT.

UNIT C: 1483 SQ. FT.

TOTAL: 4449 SQ. FT.

<u>roof trusses</u> - roof trusses proposed to be used. - TRUSSES SHALL BE DESIGNED FOR 20 PSF SNOW LOAD, 10 PSF ROOF DEAD LOAD, 10 PSF CEILING LIVE LOAD, AND 5 PSF

CEILING DEAD LOAD. - THE ENGINEER RESPONSIBLE FOR THE STRUCTURAL DESIGN OF THE HOUSE SHALL REVIEW THE TRUSS DRAWINGS FOR GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING, PRIOR TO SUBMITTING THE TRUSS DRAWINGS TO THE CODES ADMINISTRATION OFFICE FOR APPROVAL. - FAILURE OF THE RESPONSIBLE PARTIES TO SUBMIT THE TRUSS DRAWINGS TO THE RESPONSIBLE ENGINEER SHALL RELIEVE THE ENGINEER OF ALL LIABILITY FOR THE ENTIRE PLAN. TRUSS LOADS AND TRANSFER PATHS ON THIS PLAN ARE ASSUMED LOADS ONLY AND CAN ONLY BE VERIFIED AFTER TRUSS LAYOUTS AND DESIGNS ARE COMPLETED. - ATTACH EACH END OF EACH TRUSS TO TOP PLATE WITH SIMPSON H2.5. - ATTACH GIRDER TRUSSES TO TOP PLATE WITH CONNECTOR RATED FOR MANUFACTURER'S DESIGN UPLIFT LOAD (SEE

SEPARATE DESIGN BY MANUF.) - 2-PLY GIRDER: LGT2 - 3-PLY GIRDER: LGT3-SDS2.5

- 4-PLY GIRDER: LGT4-SDS3

1. MAIN LEVEL EXTERIOR WALLS SHALL BE SHEATHED W/ 7/16' D.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS. 2. \ \ \ \ \ \ \ \ \ = G.B.: 1/2" MIN. GYPSUM BDARD OVER STUDS SPACED 24" MAX FASTENED W/ NO. 6 - 1 1/4" TYPE W OR S DRYWALL SCREWS @ 7" D.C. EDGES & FIELD.

(MIN. 8'-0' SECTIONS ONE SIDE OF WALL (OR) MIN. 4'-0' SECTION FOR BOTH SIDES) 3. / / / / / / / / / / / / = LOAD BEARING INTERIOR WALL.

4. (2) 2' X 10' #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE. 5. LOW TIES @ 4'-0' D.C. (TYPICAL)

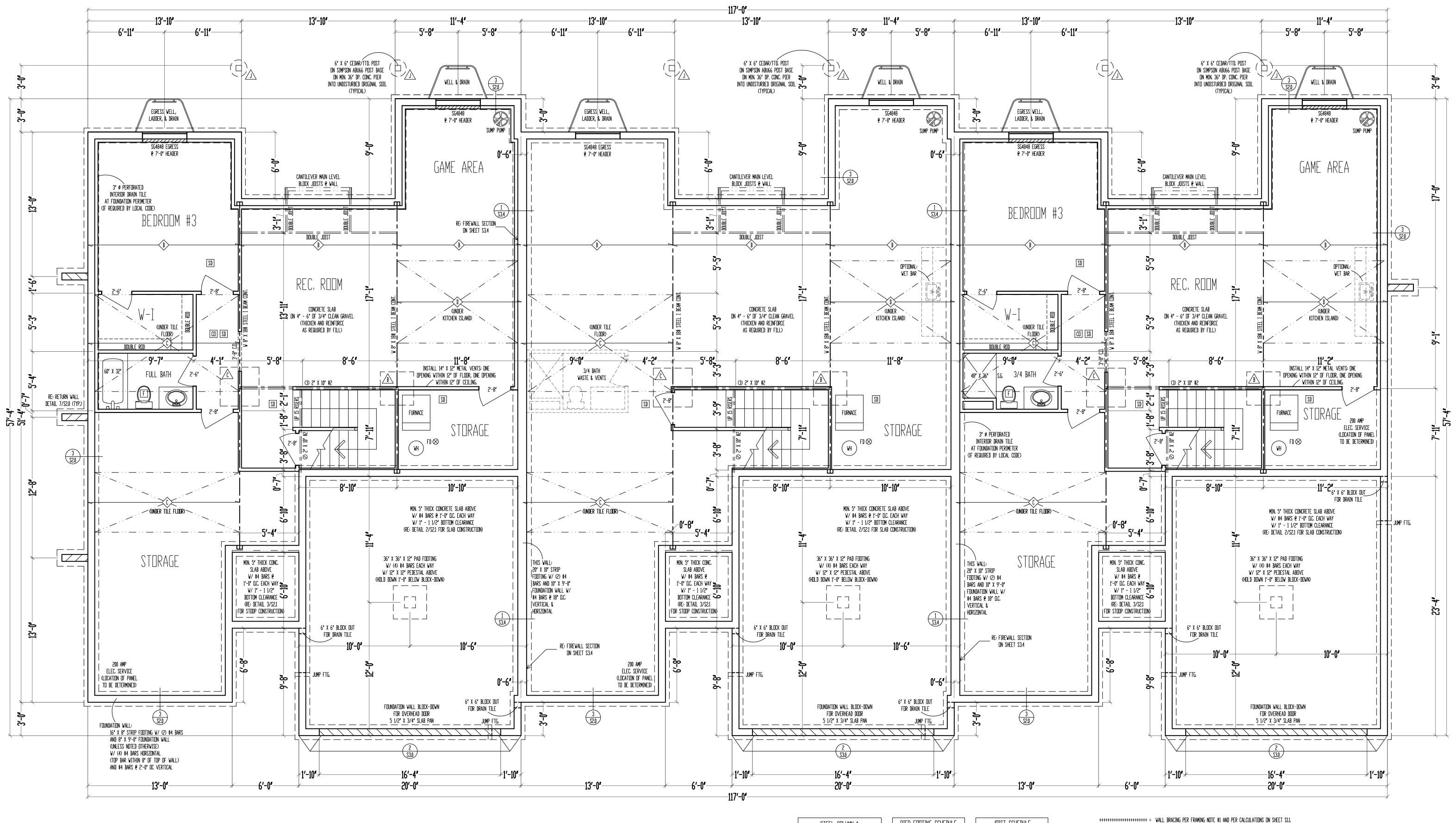
6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS). 8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. ALL DESIGNATED 2" X 6" WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS.

10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2' X 4'S @ 16' D.C., UNLESS NOTED OTHERWISE. 12. EXTERIOR WALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW WITH 16d COMMON NAILS @ 8' D.C. MAX. (WHERE APPLICABLE.) 13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS:

(2) 1 3/4" LVL PLIES = 3 1/2" GLULAM (3) 1 3/4" LVL PLIES = 5 1/2" GLULAM 14. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY OPENINGS.

Sheet Title: **FOUNDATION PLAN**

Sheet No.: 11/22/2023



LOWER LEVEL A: 845 SQ. FT. LOWER LEVEL B: --- SQ. FT. LOWER LEVEL C: 840 SQ. FT.

UNFINISHED A: 426 SQ. FT. UNFINISHED B: 1275 SQ. FT. UNFINISHED C: 428 SQ. FT.

ON 16" X 8" STRIP FOOTINGS (STEP WHERE GRADE REQUIRES)

2" X 10" FLOOR SYSTEM FOUNDATION SCALE: 1/4" = 1'-0"

	STEEL COLUMN &	PIER	R FO
P	AD FOOTING SCHEDULE	⊘	
Â	3' X 11 GA. STEEL COLUMN ON 30' X 30' X 10' PAD FOOTING W/ (4) #4 BARS EACH WAY (12.5k)		
B	3 1/2" X 11 GA. STEEL COLUMN DN 36" X 36" X 10" PAD FODTING W/ (4) #4 BARS EACH WAY (18.0k)	\triangle	
<u>Ĉ</u>	3' SCH. 40 STEEL COLUMN ON 42' X 42' X 12' PAD FOOTING W/ (5) #4 BARS EACH WAY (24.5k)	K	
	3 1/2" SCH. 40 STEEL COLUMN ON 48" X 48" X 12" PAD FOOTING W/ (6) #4 BARS EACH WAY (32.0k)		
Æ	3 1/2" SCH. 40 STEEL COLUMN ON 54" X 54" X 14" PAD FOOTING W/ (7) #4 BARS EACH WAY (40.5k)		

W/ (8) #4 BARS EACH WAY (50.0k)

JOIST SCHEDULE FOOTING SCHEDULE 2" X 10" #3 FLOOR JOIST 12" Ø PIER FTG. **€** 16″ □.C. 2" X 10" #2 FLOOR JOIST 16" Ø PIER FTG. e 16° □.C. 2" X 10" #2 FLOOR JOIST 18" Ø PIER FTG. € 16° 🛛.C. DOUBLE EVERY OTHER 24" Ø PIER FTG. 2" X 10" #2 FLOOR JOIST € 16° D.C. DOUBLED

1. BASEMENT LEVEL EXTERIOR VOIDD-FRAMED WALLS SHALL BE SHEATHED W/ 7/16' D.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR

3. $\/\/\/\/\/\/\$ = LOAD BEARING INTERIOR WALL. 4. (2) 2' X 10' #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.

5. LOW TIES @ 4'-0' D.C. (TYPICAL) 6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLDCK JUISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JUIST MATERIAL (NOT REQUIRED WITH I-JUISTS).

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. ALL DESIGNATED 2' X 6' WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS. 10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE.

11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2' X 4'S @ 16' O.C., UNLESS NOTED OTHERWISE. 12. 1/2' Ø ANCHOR BOLTS W/ MIN. 7' EMBEDMENT @ 48' D.C. MAX. & VITHIN 6' - 12' OF END OF EACH PLATE LENGTH. 13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS:

(2) 1 3/4" LVL PLIES = 3 1/2" GLULAM (3) 1 3/4" LVL PLIES = 5 1/2" GLULAM

14. NEW FOUNDATION SHALL BEAR ON ORIGINAL SOIL WITH MINIMUM BEARING CAPACITY OF 1500 PSF. A GEOTECHNICAL ENGINEER IS RECOMMENDED FOR VERIFICATION OF THESE CONDITIONS DURING THE EXCAVATION PHASE, ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANYTHING SHORT OF THE AFDREMENTIONED REQUIREMENTS. 15. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY OPENINGS.

9'-0" FOUNDATION WALLS (UNLESS NOTED OTHERWISE)

3 1/2" SCH. 40 STEEL COLUMN DN 60' X 60' X 14' PAD FOOTING

DESCRIPTION OF BUILDING ELEM	MENTS	T	R STRUCTURAL MEMBERS PE OF FASTENER		SPACING OF FASTENERS
		RC	DOF ¹		
BLOCKING BETWEEN JOISTS OR RAFT PLATE, TOE NAIL	ERS TO TOP	3-8d (2 ½)	' x 0.113")		-
CEILING JOISTS TO PLATE, TOE	NAIL	3-8d (2 ½)	" x 0.113")		-
CEILING JOISTS NOT ATTACHED TO RAFTER, LAPS OVER PARTITIONS, F		3-	10d	-	
COLLAR TIE TO RAFTER, FACE NAIL (GAGE RIDGE STRAP		3-10d (3"	' x 0.128")	-	
RAFTER OR ROOF TRUSS TO PLATE	, TOE NAIL		0.135") OR 3-10d COMMON	2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL O	
ROOF RAFTERS TO RIDGE, VALLEY	, OR HIP	`	" x 0.148") , 3-16d (3½" x 0.135")	OPPOSITE SIDE OF EACH RAFTER OR TRUSS	
RAFTERS: TOE NAIL FACE NA	AIL	,	ALL ¹		-
BUILT-UP STUDS - FACE NA	IL	Τ	x 0.128")		24" O.C.
ABUTTING STUDS AT INTERSECTIN CORNERS, FACE NAIL	NG WALL	16d (3½"	x 0.135")		12" O.C.
BUILT-UP HEADER, TWO PIECES WITH	½" SPACER	16d (3 ½ 2"	x 0.135")		16" O.C. ALONG EACH EDGE
CONTINUED HEADER, TWO PIE		16d (3 ½ "	x 0.135")		16" O.C. ALONG EACH EDGE
		· ·	" x 0.113")		
CONTINUOUS HEADER TO STUD, T	OE NAIL	,	x 0.128")		24" O.C.
DOUBLE STUDS, FACE NAII	_				
DOUBLE TOP PLATES, FACE N	IAIL	10d (3")	x 0.128")		24" O.C.
DOUBLE TOP PLATES, MINIMUM 24-IN OF END JOINTS, FACE NAIL IN LAPP		8-16d (3 ½	<u>(</u> " x 0.135")		<u>-</u>
SOLE PLATE TO JOIST OR BLOCKING,	FACE NAIL	16d (3 ½ "	x 0.135")		16" O.C.
SOLE PLATE TO JOIST OR BLOCKING WALL PANELS	AT BRACED	3-16d (3 ½	<u>(</u> " x 0.135")		16" O.C.
STUD TO SOLE PLATE, TOE N	AIL	3-8d (2 ½ " x 0.113") O	R 2-16d (3½" x 0.135")		-
TOP OR SOLE PLATE TO STUD, EN		2-16d (3 ½	<u>(</u> " × 0.135")		<u>-</u>
TOP PLATES, LAPS AT CORNERS		2-10d (3"	' x 0.128")	-	
INTERSECTIONS, FACE NAI	L	·	· 		
1" BRACE TO EACH STUD AND PLATE, FACE NAIL		·	" x 0.113")		<u>-</u>
1"x6" SHEATHING TO EACH BEARING, FACE NAIL		2-8d (2½'	" x 0.113")		-
1"x8" SHEATHING TO EACH BEARING,	FACE NAIL	2-8d (2 ½ / ₂	" x 0.113")		-
WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL		3-8d (2½'	" x 0.113")		-
		FLC	DOR ¹		
JOIST TO SILL OR GIRDER, TOE	NAIL	3-8d (2 ½ 2	" x 0.113")		-
RIM JOIST TO TOP PLATE, TOE NAI APPLICATIONS ALSO)	L (ROOF	8d (2 ½ " x 0.113"		6" O.C.	
RIM JOIST OR BLOCKING TO SILL PLAT	E TOE NAII	8d (2 ½ " x 0.113")			6" O.C.
1"x6" SUBFLOOR OR LESS TO EACH J	·	2-8d (2 %	" x 0.113")		<u>-</u>
NAIL		·			
2" SUBFLOOR TO JOIST OR GIRDER, FACE NAIL	BLIND AND	2-16d (3 / 2	<u>(</u> " x 0.135")		-
2" PLANKS (PLANK AND BEAM - FLOOR	AND ROOF)	2-16d (3 ½	<u>(</u> " x 0.135")	AT EACH BEARING	
BUILT-UP GIRDERS AND BEAMS, 2-INC LAYERS	CH LUMBER	10d (3"	x 0.128")	NAIL EACH LAYER AS FOLLOWS: 32" O.C. AT TO AND BOTTOM AND STAGGERED. TWO NAILS A	
		3-16d (3 ½	<u>(</u> " x 0.135")		ENDS AND AT EACH SPLICE AT EACH JOIST OR RAFTER
LEDGER STRIP SUPPORTING JOISTS C	OR RAFTERS	,	·		
DESCRIPTION OF BUILDING MATERIALS WOOD STRUCTURAL PANELS, SUE		IPTION OF FASTENER	R STRUCTURAL MEMBERS EDGE SPACING (INC ATHING TO FRAMING AND P		INTERMEDIATE SUPPORTS (INCH IRD WALL SHEATHING TO FRAMING
	6d COM	MON (2" x 0.113") NAIL			
¾" - ½" 		, WALL) 8d COMMON NAIL (ROOF)	6		12
¹⁹ ⁄ ₃₂ " - 1"		MON NAIL (2½" x 0.131") DN (3" x 0.148") NAIL OR 8d	6		12
11/8" - 11/4"		131") DEFORMED NAIL	6		12
	41/11 (2.41)		L SHEATHING ¹		
½ " GYPSUM SHEATHING	STAPLE GA	ANIZED ROOFING NAIL; LVANIZED, 1½" LONG; 1¼" EWS, TYPE W OR S	7		7
%" GYPSUM SHEATHING	STAPLE GA	'ANIZED ROOFING NAIL; LVANIZED, 1⅓" LONG; 1⅓" EWS, TYPE W OR S	7		7
wo	OOD STRUCTU	RAL PANELS, COMBINATIO	N SUBFLOOR UNDERLAYM	ENT TO FRAM	ING ¹
¾" AND LESS		ED (2" x 0.120") NAIL OR 8d ON (2½" x 0.131") NAIL	6		12
½ " - 1"		N (2½" × 0.131") NAIL OR 8d MED (2½" × 0.120") NAIL	6		12
11/8" - 11/4"		DN (3" x 0.148") NAIL OR 8d MED (2½" x 0.120") NAIL	6		12

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION LISTED IN THIS TABLE

FOUNDATION NOTES

CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR

PORCHES AND GARAGE FLOOR SLABS
THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION

STANDARDS
3. PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING
LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM
OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A

MINIMUM 20 GALLON SUMP PIT.

4. FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY

SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.

5. FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE

#4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRAFOR FROST PROTECTION.

6. CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE

FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0
 REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE)

INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB

10. BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND, GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES

11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB SHALL BE DESIGNED BY A LICENSED ENGINEER

12. SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH ½" Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS)
AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6

13. FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET

14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

15. ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS

ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 - 2x10's, UNLESS NOTED OTHERWISE ON PLANS

17. BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS

18. INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A

19. ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED OTHERWISE

20. WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS.

21. ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT

22. JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.

23. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS

24. JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID

BLOCKING MIN. 11/8" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT

25. ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.326. ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.

0. ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R002.3.

27. ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER ½ OF VERTICAL DISTANCE BETWEEN CEILING AND ROOF

28. BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED

29. PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A ½" GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION)

30. ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi

31. ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi
32. COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND
THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM
FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. ½" x 2" BOLTS SHALL THEN BE
INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE
WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE
INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

33. WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING.

34. ALL ROOF SHEATHING SHALL BE $\frac{7}{16}$ " OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0"

36. ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

37. ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH $\frac{1}{2}$ " OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN $\frac{1}{150}$ OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

38. PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP.

PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

MASONRY VENEER

40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1½", WITH NOT LESS THAN 5%" MORTAR OR GROUT COVER TO OUTSIDE FACE.

VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 7/8" CORRUGATED.

42. EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.

THAN 32 INCHES ON CENTER HORIZON TALLY AND 24 INCHES ON CENTER VERTICALLY.

43. VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL

OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS

SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

GARAGE NOTES

44. DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1%" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE

45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 90-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC SECTION R301.2.1

GARAGE NOTES (CONTINUED)

44. THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 5%" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM 5%" TYPE X GYP. BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH 5%" GYP. BOARD.

45. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER

BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING AND SHALL BE FASTENED WITH 2½"" x 0.120" NAILS AT 7" O.C. STAGGERED WITH (7) 3½" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER. MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

DESIGN LOADING (PER TABLE R301.5)

MINIMUM UNIFORMLY DISTRIB		
USE	LIVE LOAD	DEAD LOAD
UNINHABITABLE ATTICS WITHOUT STORAGE	10	10
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20	10
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30	10
BALCONIES (EXTERIOR) AND DECKS	40	10 ^d
FIRE ESCAPES	40	10
GUARDRAILS AND HANDRAILS ^a	200 ^c	-
GUARDRAIL IN-FILL COMPONENTS ^b	50 ^c	-
PASSENGER VEHICLE GARAGES	50	DEPENDENT UPON SLAB CONSTRUCTION
ROOMS OTHER THAN SLEEPING ROOM	40	10 ^d
SLEEPING ROOM	30	10 ^d
STAIRS	40	10 ^d

a. A single concentrated load applied in any direction at any point along the top.

b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement.
c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the

infill components. These loads shall be determined independently of one another, and loads are assumed

not to occur with any other live load.

d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed.

INSULATION/EFFICIENCY

BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)

CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1.

INSULATION AND FENESTRATION REQUIRE	MENTS BY COMPONENT (TABLE N1102.1.1)
CLIMATE ZONE	4-A
FENESTRATION U-FACTOR	0.35
SKYLIGHT U-FACTOR	0.55
GLAZED FENSTRATION SHGC	0.40
CEILING R-VALUE	49
WOOD FRAME WALL R-VALUE	13
MASS WALL R-VALUE	8 / 13
FLOOR R-VALUE	19
BASEMENT WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
SLAB R-VALUE AND DEPTH	10 AT 2'-0"
CRAWL SPACE WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	8
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	6
CATHEDRAL VAULTED CEILING R-VALUE	38

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2012 IRC.

1. AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED

WITHOUT ADDITIONAL JOINT SEALS.
WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE
SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE
JOINT SO AS TO PREVENT A HINGE EFFECT.

CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.

DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:

POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST.

ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA.

EXCEPTION: THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

ME	CHANICAL VENTILATIO	N SYSTEM FAN EFFICA	CY
FAN LOCATION	AIR FLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY (CFM/WATT)	AIR FLOW RATE MAXIMUM (CFM)
RANGE HOODS	ANY	2.8	ANY
IN-LINE FAN	ANY	2.8	ANY
BATHROOM, UTILITY ROOM	10	1.4	90
BATHROOM, UTILITY ROOM	90	2.8	ANY



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SERVICES
SESUMMIT MISSOURI

122/2023

RESIDENTIAL SEISMIC & WIND ANALYSIS

				INPUT
DETERMINE WEIGHT OF HOUSE:				CALCULATED VALUE
LOCATION		DEAD LOAD (psf)	AREA (ft ²)	WEIGHT (lbs.)
ROOF		10	6349	63490
CEILING		10	6349	63490
FIRST FLOOR		10	6349	63490
	WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WEIGHT (lbs)
FIRST FLOOR EXT. WALL DL	354.66	10	10	35466
		DEAD LOAD (psf)	AREA (ft2)	WEIGHT (lbs)
FIRST FLOOR INT. PARTITION WALL DL		6	6349	38094

	PRO	DJECTED AREAS (WIND	DESIGN PER 115 MPH	3-SECOND GUST, EXPOS	SURE C AND MEAN ROOF HEIGHT <= 3	30 FT ASSUMED)	
		-TO-BACK			SIDE-TO-S	,	
	AREA	LOAD			AREA	LOAD	
SLOPED ROOF	555	4515		SLOPED ROOF	708	6024	
VERT. ROOF	853	10129	CUMULATIVE	VERT. ROOF	30	373	CUMULATIVE
1ST	1287	15282	30007	1ST	663.63	8250	14728
	<u> </u>		PRESSURE (PSI	F) - PER ASCE CH. 6			
	SLOPED ROOF	ZONE B		9.7	ZONE C	11.3	2a (FIG. 28.6-1, ASCE7)
	WALL/VERT. ROOF	ZONE A		14.2	ZONE D	7.7	12.066
	MEAN ROOF HT., h		24		4444444		

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area. q_{z10} =0.00256 $K_z K_{zt} K_d V^2$ (ASCE7-10 Velocity Pressure)

q_{z10_ASD}=0.6q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)

1ST FLOOR TRIBUTARY WEIGHT S_S (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP) 12.0% F_a (from ASCE7 Table 11.4-1) 1.6 S_{DS} (= 2/3 * S_{S} * F_{a}) 0.128 R (from ASCE7 Table 12.2-1)

SEISMIC SHEAR V (= 1.2 * S_{DS} * W / R) (lbs.) LOCATION From ASCE7 (Eq. 12.8-1): 1ST FLOOR

Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable Shear (#/LF)	Code Reference
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	155	per IBC, Table 2306.3(1)
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	230	per IBC, Table 2306.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	310	per IBC, Table 2306.3(1)
Exterior <u>(Option #4)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220	AF&PA SDPWS Table 4.3A
Exterior <u>(Option #5)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320	AF&PA SDPWS Table 4.3A
Exterior <u>(Option #6)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410	AF&PA SDPW Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60	per IBC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)	325	

		· · · · · · · · · · · · · · · · · · ·		<u> </u>				
			EXTER	RIOR STRUCTURAL WALL	LENGTHS (ft.) & RESISTANCES			
		SE	ISMIC			WIND		
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)
1ST FLOOR	114	31920	49.5	13860	114	44688	49.5	19404
				-				
		ADDITIONAL RESIS	STANCE REQUIRED		Anchor Bolt Spacing	g (in.)	16d Nail Spacing req'd at	bottom plate (in.)
		SEISMIC	WIND		diameter (in.)	0.5	1st Floor F-B	11
1ST FLOOR FRONT-	TO-BACK	0	0		Shear value (per NDS)	944	1st Floor S-S	43
1ST FLOOR SIDE-TO	-SIDE	0	0		Spacing F-B (inches)	72.9		
BASEMENT FRONT-I	TÓ-BACK	0	Ō	-	spacing S-S (inches)	288.0		

WIDTH OF 1ST STORY (FT.)

DEPTH OF 1ST STORY (FT.)

BACK WALL OF GARAGE (FT.)

GAR. WALL: 1=F-B, 2=S-S

117

60.33

0

RESISTANCE REQUIRED IN ADDITION TO RESISTANCE PROVIDED BY EXTERIOR WALLS** INT. WALL LENGTH ADDITIONAL PORTAL FRAMES OR RESISTANCE PROVIDED BY INTERIOR WALL LENGTH W/ 1/2" SHEATHED W/ OSB INTERIOR X-BRACES RESISTANCE PERF. SHEAR WALL ADDITIONAL METHODS OK? (325#/BRACE) GYPSUM BOARD PER TABLE (FT.) (TOTAL LENGTH, ONE REQUIRED (POUNDS) RESISTANCE (POUNDS) SIDE, FT.) 1ST FLOOR FRONT-TO-BACK 1ST FLOOR SIDE-TO-SIDE

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE),

2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING

PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRED

				WIND UPLIFT	T ANALYSIS		
	X/12	DEGREES			_		
ROOF PITCH (MAX)	12	45.0	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2			
		ASCE 7			_		
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)			
OVERHANG	1	-1.08	356.66	-1.08			
	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)
MAIN ROOF**	7058.61	-534.089424	7592.699424	-1.08	-0.36	-2157	-6.1
*ALONG PERIMETER		TOTAL UPLIFT PER LINEAL	FOOT ALONG EXTERIOR (PO	DUNDS)	-7.2	UPLIFT OK	
**INSIDE EXTERIOR V	VALLS	RESISTANCE DUE TO DEAD	WEIGHT & (3) 10d TOENAILS	S	251.6		

NOTE FOR CONSTRUCTION:

EXTERIOR SHEATHING OPTION FOR FIRST FLOOR

EXTERIOR SHEATHING OPTION FOR BASEMENT WALLS

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION

Combustion Air Calculation Per 2018 IRC Section G2407.5

100000 BTU/h Appliance #1 Furnace Appliance #2 BTU/h Appliance #3 50000 BTU/h Water Heater

150000 BTU/h Total BTU/hr

Area of Combined Space (floor where appliances are located) Ceiling Height in Usable Space

728 ft² 8.75 ft

Note: Per 2018 IRC Section G2407.5.3.2, The volumes of spaces in different stories shall be considered as communicating spaces where such spaces are connected by one or more openings in doors or floors having a total minimum free area of 2 square inches per 1,000 BTU/h of total input rating of all appliances

Is floor where appliances are located open to adjacent level? If Yes, what is the area of open space adjacent to appliance area?

Yes

7500 ft³

OK

Per 2018 IRC Section G2407.5.1 (Standard Method), the minimum required volume shall be 50 cubic feet per 1,000 BTU/hr

(Total BTU/hr / 1,000 BTU/hr x 50 ft 3)

Required air space in combined areas:

857 ft² Required combined area:

Area of Combined Space > Required combined area?

Per Section G2407.5.3.1, each opening shall have a minimum free area of 1 square inch per 1,000 BTU/hr of the total input rating of all appliances in the space, but not less than 100 square inches. One opening shall commence within 12 inches of the top and one opening shall commence within 12 inches of the bottom of the enclosure. The minimum dimension of air openings shall be not less than 3 inches.

Note: two grills required - one within 12" of floor, one within 12" of clg.

150 in² Minmum required opening area: Minimum grill size: 14 x 11





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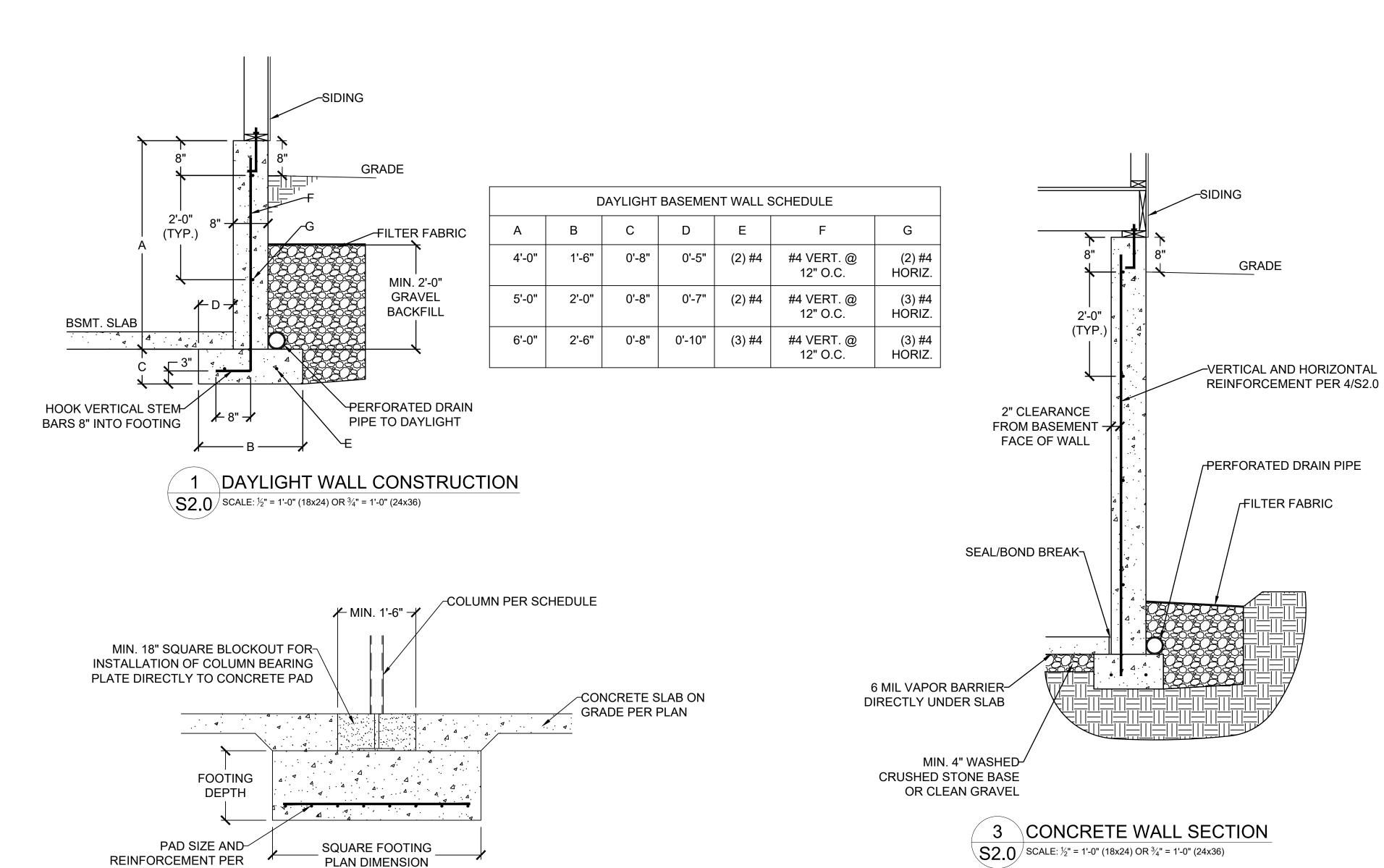


TABLE ABOVE

2 COLUMN AND BEARING PAD SCHEDULE

MIN. (2) #4 BARS EXTENDING 24"

PAST OVER-EXCAVATION AND

INTO INTERSECTING WALL

-CONTINUOUS FOOTING

AND REBAR THROUGH

6'-0" MAX.

SOLID JUMP

MAX. 12" BLOCKOUT FOR

FORM PLACEMENT AND

TO EXTEND DRAIN TILE

5 SOLID JUMP

S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

VERTICAL REINFORCEMENT SPACING CONCRETE STRENGTH/GRADE 8" THICK WALL 10" THICK WALL REINFORCEMENT (#4 BARS) 10' 10' 3,000 PSI/ GRADE 40 24 24 16 24 24 18 3,500 PSI/ GRADE 40 24 24 24 18 3,000 PSI/ GRADE 60 24 16 24 24 18 3,500 PSI/ GRADE 60 24 24 24 18 HORIZONTAL REINFORCEMENT - MINIMUM GRADE 40 STEEL ONE BAR 12" FROM TOP OF WALL; 6-#4 4-#4 5-#4 6-#4 MAX. SPACING 24" OC

FOOTNOTES:

1) WALL HEIGHT IS MEASURED FROM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB 2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE WALL. OTHER WALLS SHALL HAVE VERTICAL REINFORCEMENT AS FOLLOWS:

- A) 8" WALL MINIMUM 5" FROM THE OUTSIDE FACE
- B) 10" WALL MINIMUM 63/4" FROM THE OUTSIDE FACE
- C) EXTEND BARS TO WITHIN 8" OF THE TOP OF THE WALL

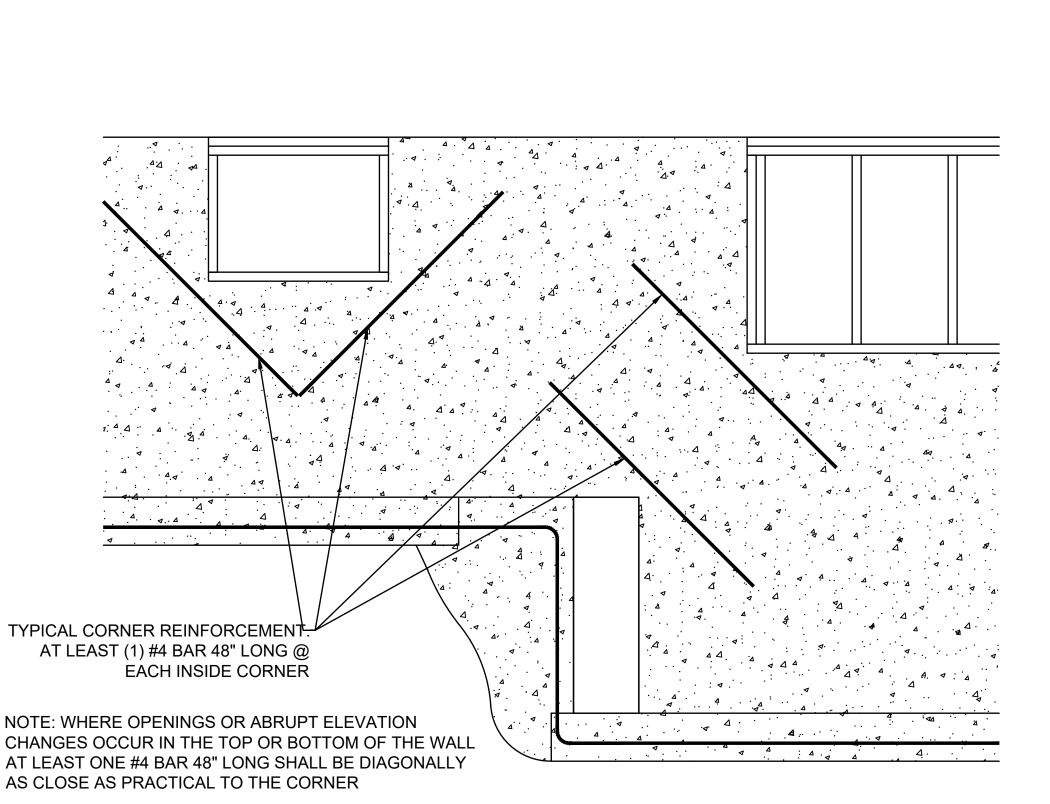
3) REINFORCEMENT CLEARANCES:

- A) CONCRETE EXPOSED TO EARTH MINIMUM 11/2"
- B) NOT EXPOSED TO WEATHER (INTERIOR SIDE OF WALLS) -3/4" C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY
- SLABS) 1½"

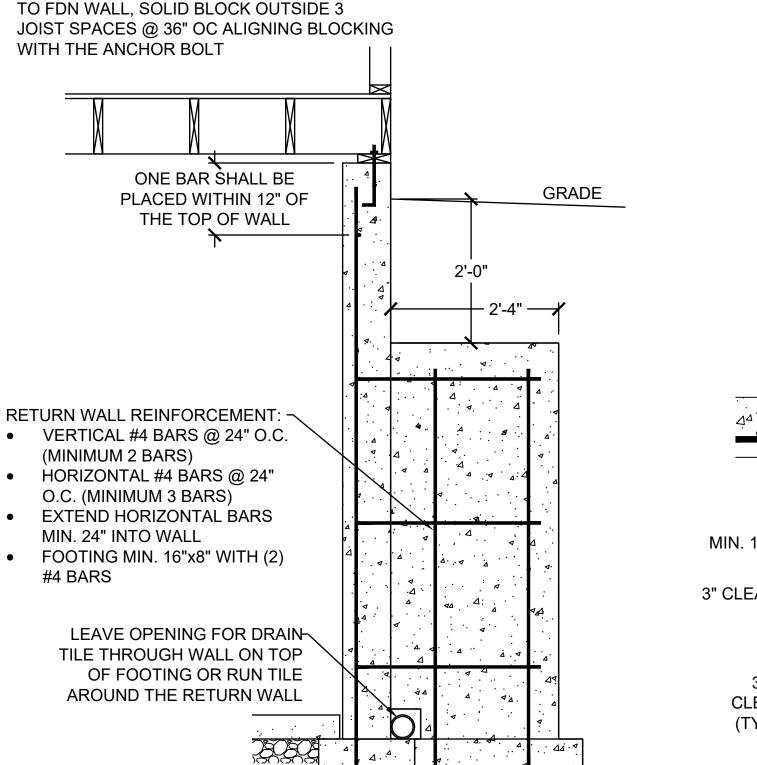
4) HORIZONTAL REINFORCEMENT:

- A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL
 - B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE
 - (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE
- D) SUPPLEMENTAL REINFORCEMENT AT CORNERS PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF THE EDGE OF INSIDE CORNERS.
- 5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND CORNERS.
- 6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE 3½". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP OF THE WALL
- 7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS

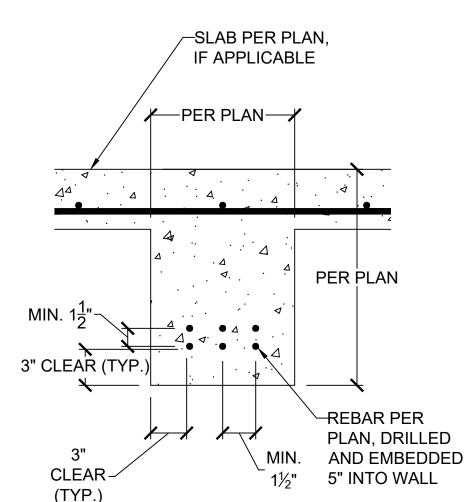
4 FOUNDATION WALL REINFORCEMENT TABLE S2.0 NO SCALE



6 REINFORCEMENT AT OPENING CORNERS S2.0 AND STEP CORNERS @ INSIDE CORNERS SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)



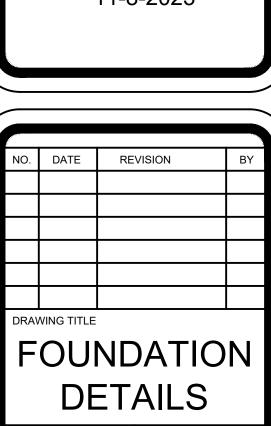
NOTE: WHERE FLOOR JOIST RUNS PARALLEL



8 CONCRETE GRADE BEAM S2.0 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

RETURN WALL DETAIL $\frac{1}{52.0}$ SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

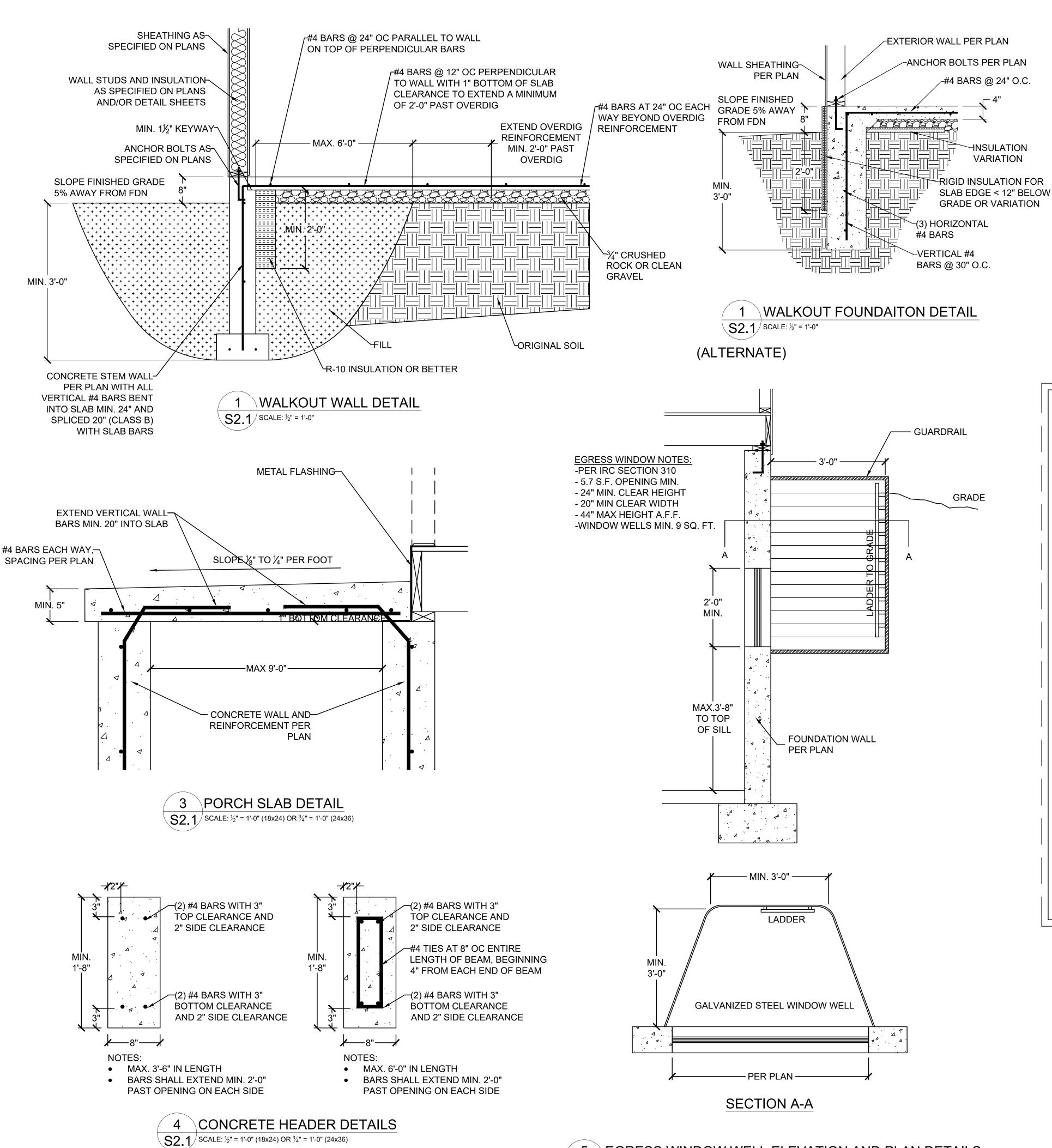


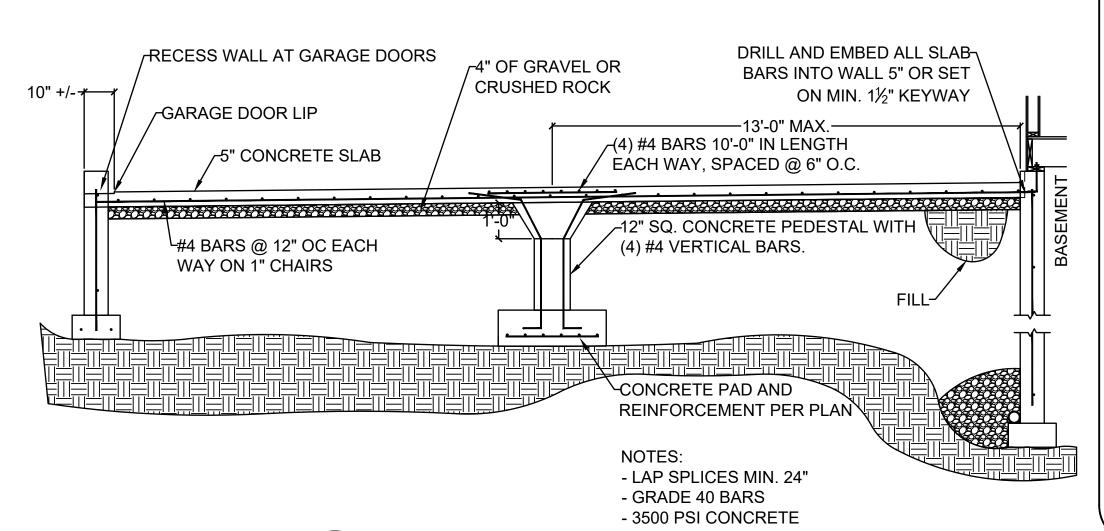


ENGINEER: DMH CHECKED BY:DMH DRAWN BY: DMH

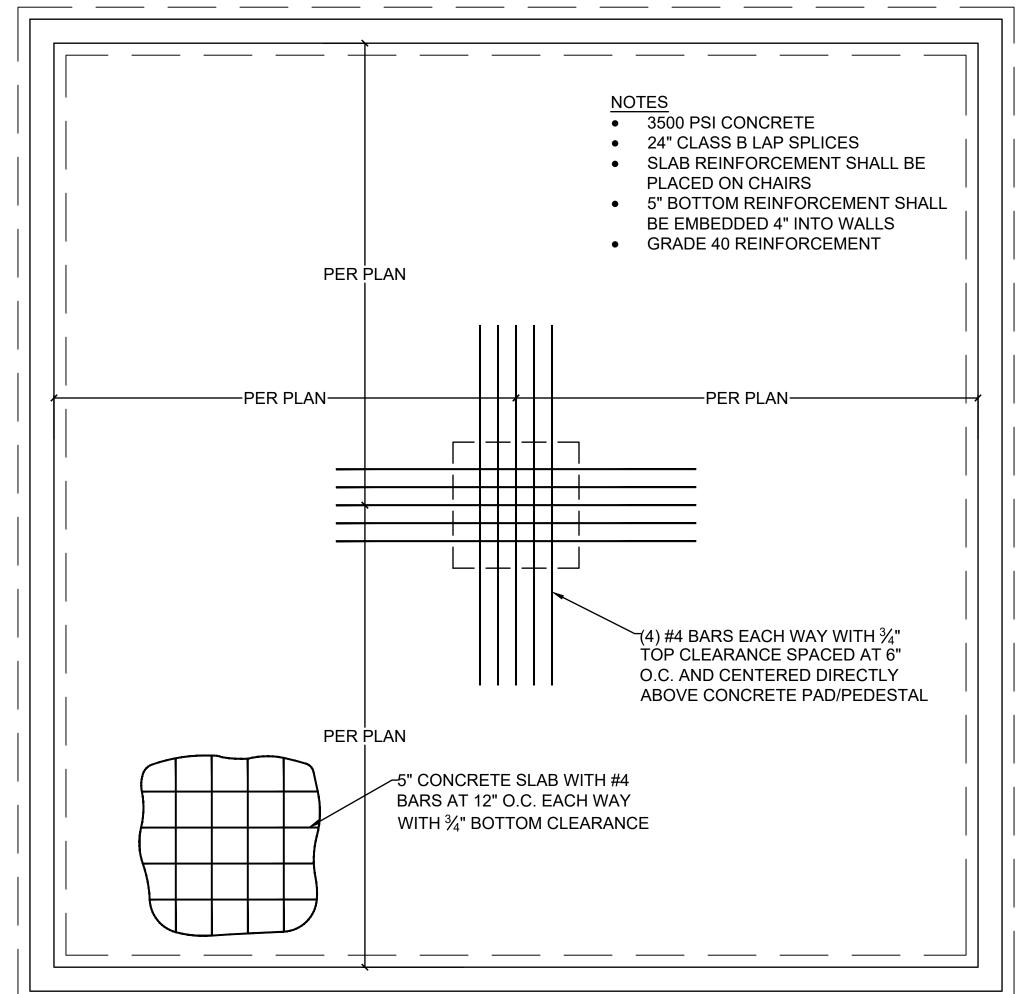
DATE: 11-08-23 SHEET NUMBER

11/22/2023



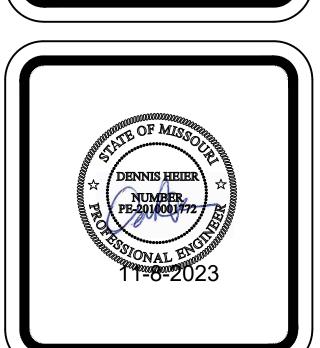


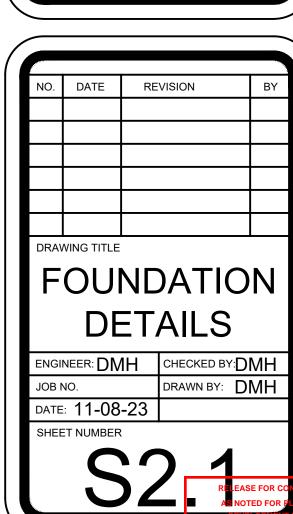
2 GARAGE SLAB ON FILL S2.1 SCALE: 1/4" = 1'-0" (18x24) OR 3/8" = 1'-0" (24x36)





CLIENT: KEVIN HIGDON CONSTRUCTION
JOB TITLE: TCR013 TRIPLEX

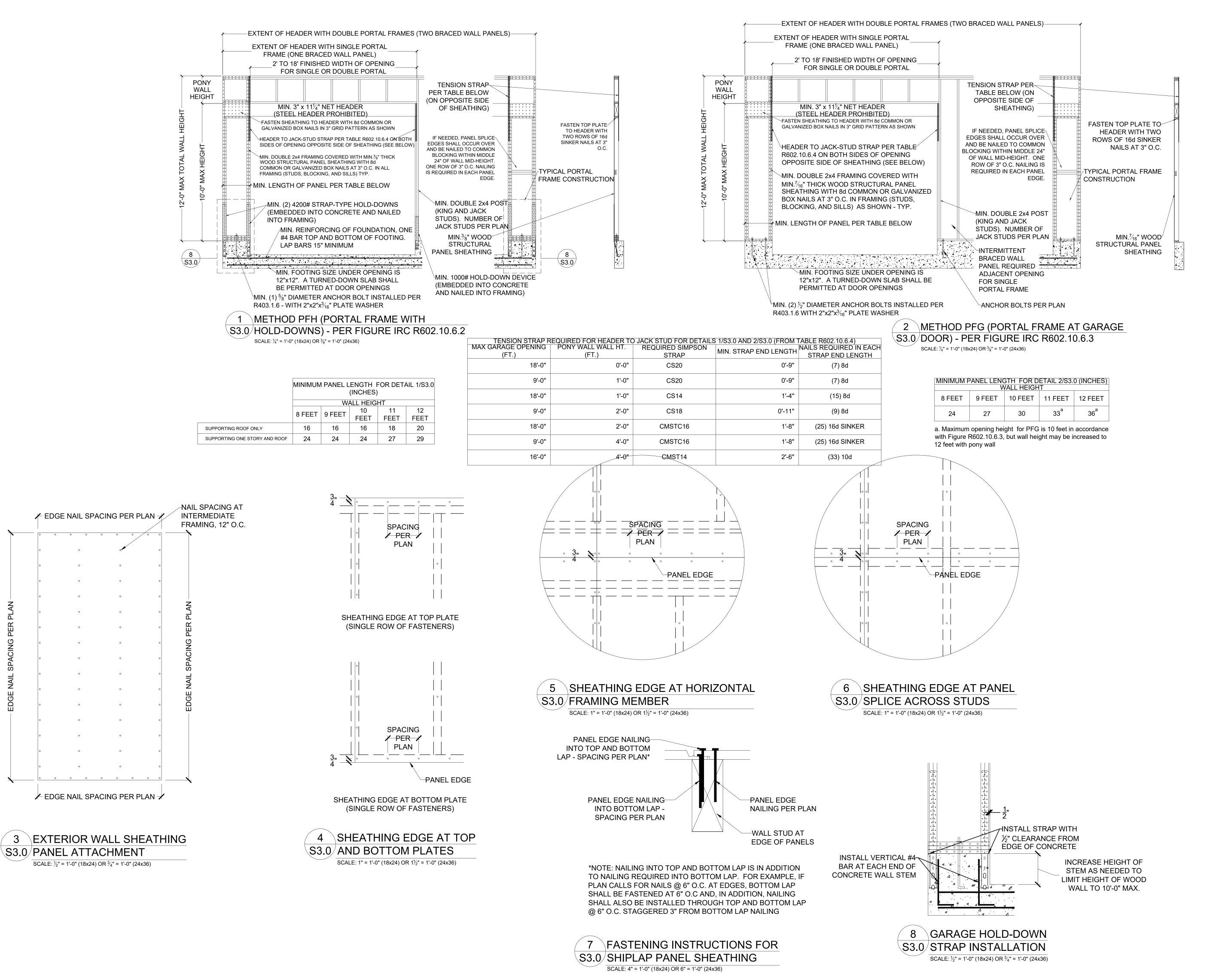




5 EGRESS WINDOW WELL ELEVATION AND PLAN DETAILS SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)

DEVELOPMENT SERVICES
LEE'S SUMMIT MISSOURI

11/22/2023





CLIENT: KEVIN HIGDON CONSTRUCTION
JOB TITLE: TCR013 TRIPLEX



DRAWING TITLE

FRAMING

DETAILS

ENGINEER: DMH

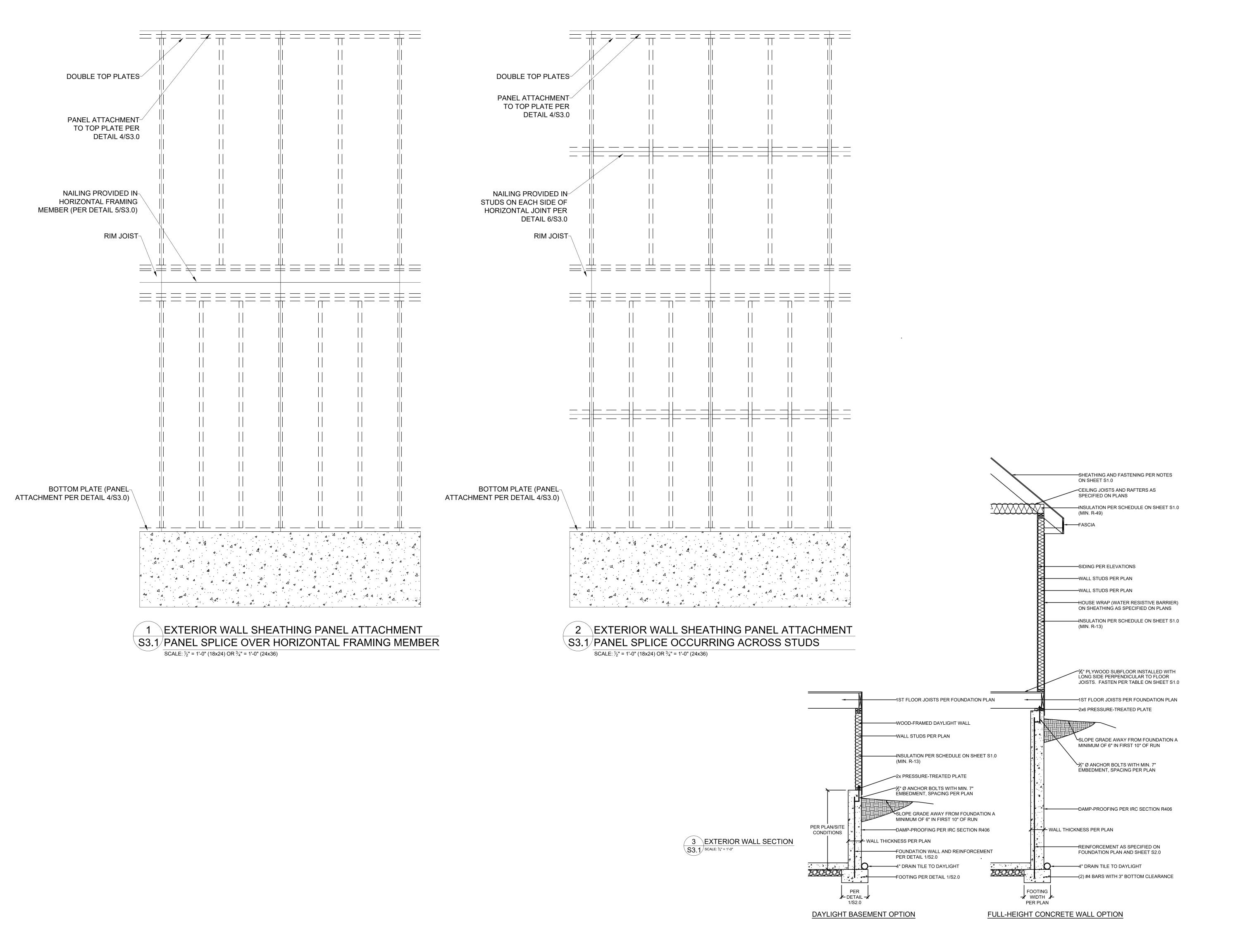
JOB NO.

DATE: 11-08-23

SHEET NUMBER

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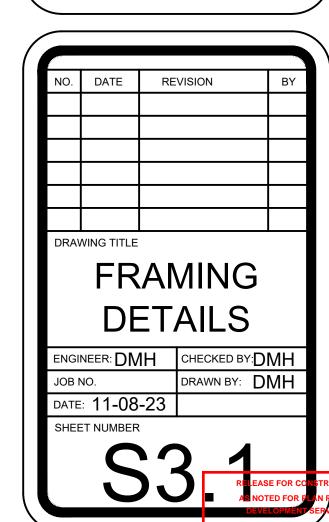
11/22/2023

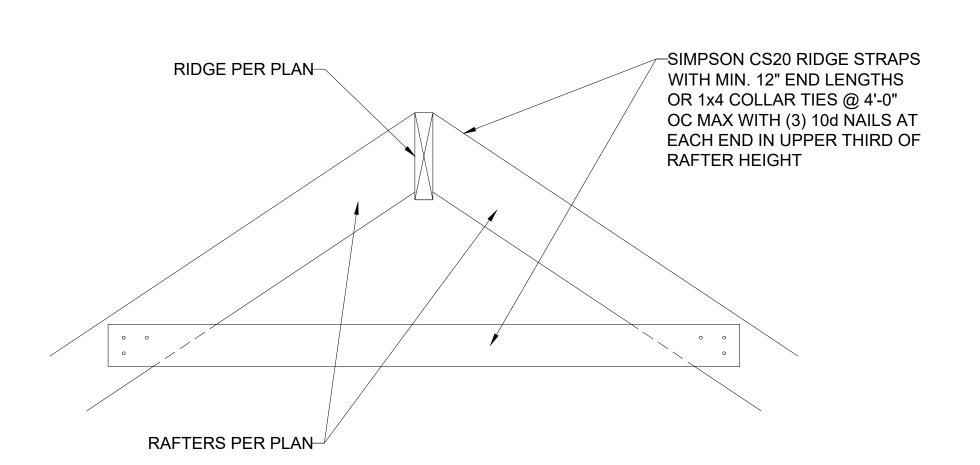




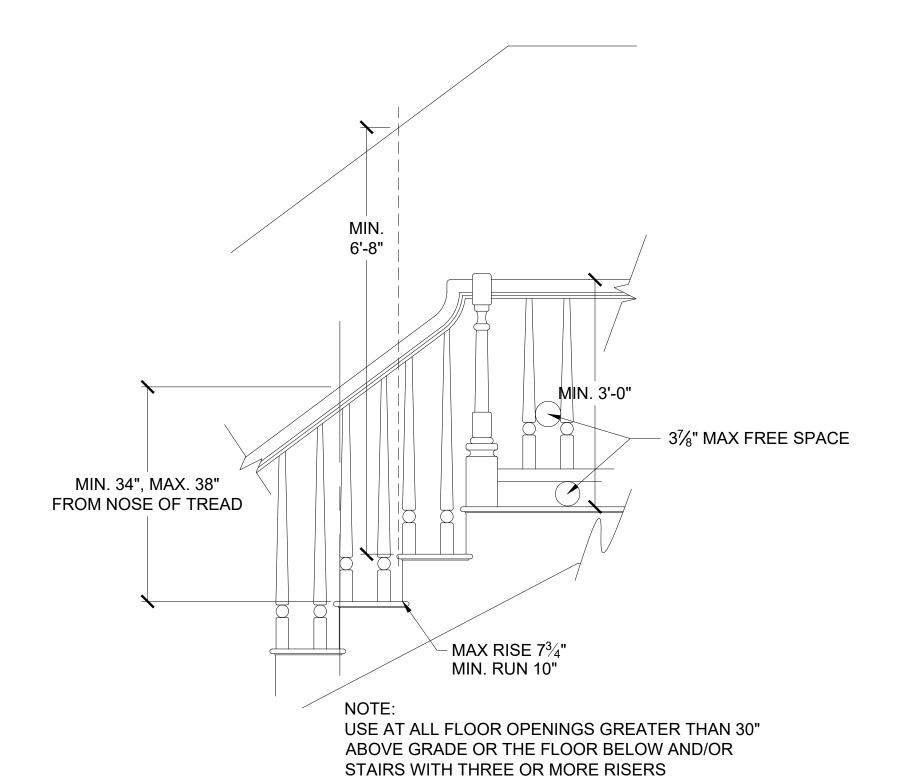
CLIENT: KEVIN HIGDON CONSTRUCTION



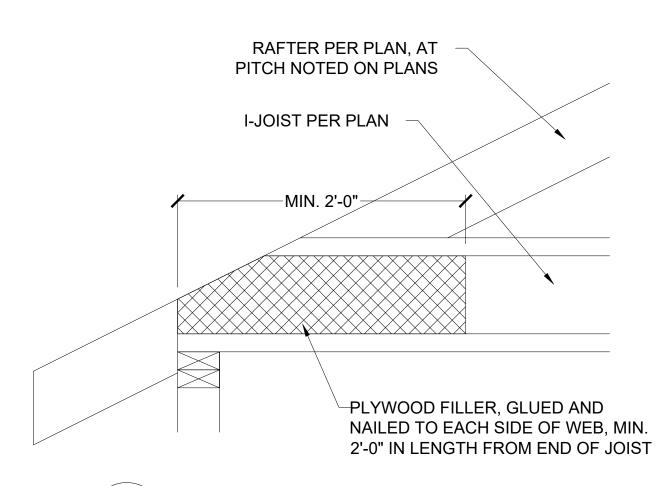




RIDGE FRAMING DETAIL S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



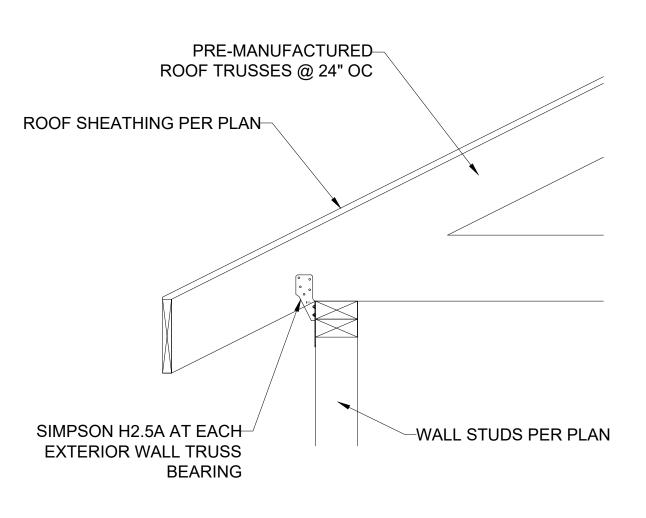
4 STAIR AND HANDRAIL/GUARDRAIL DETAIL



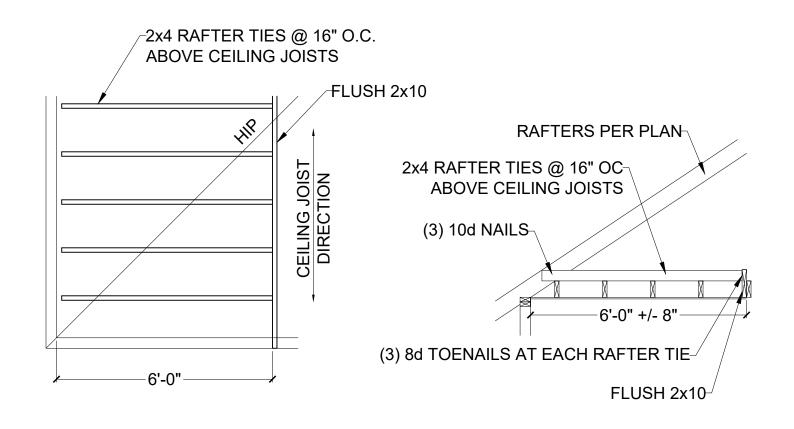
SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

9 COPED I-JOIST REINFORCEMENT

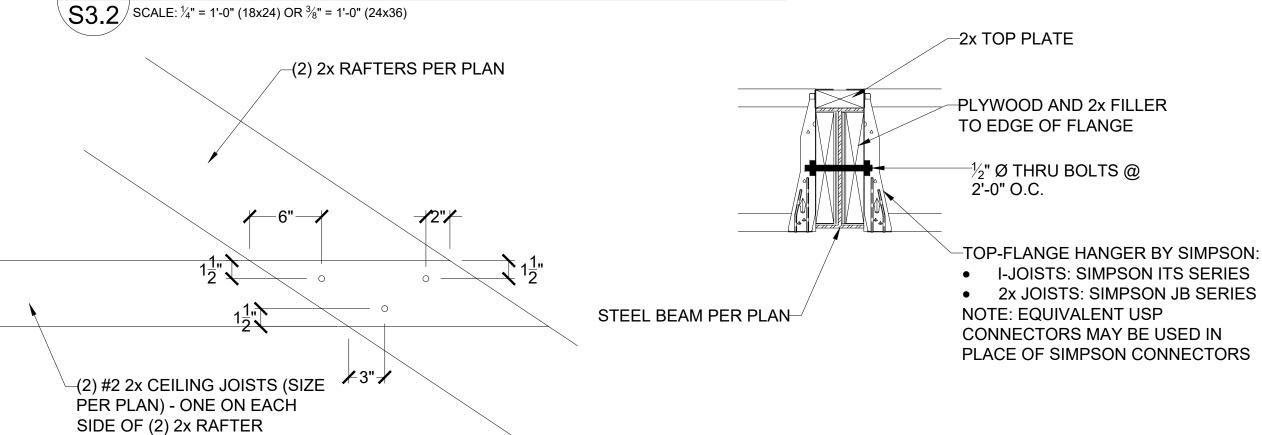
S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



TRUSS CONNECTION TO EXT. WALL BEARING SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



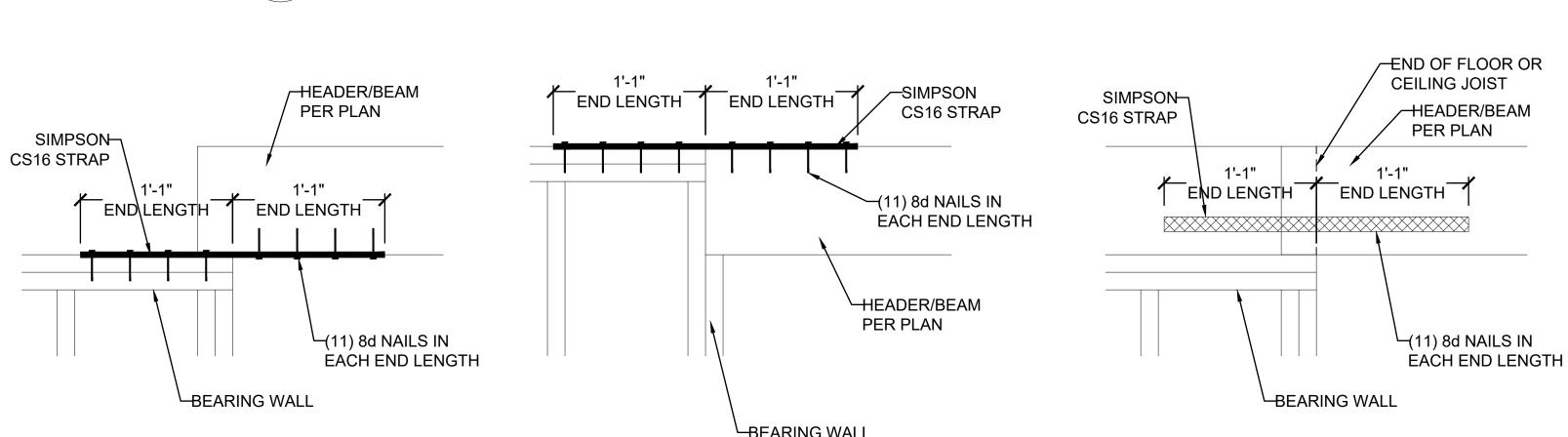
5 RAFTER TIES AT CEILING JOISTS PERP. TO RAFTERS



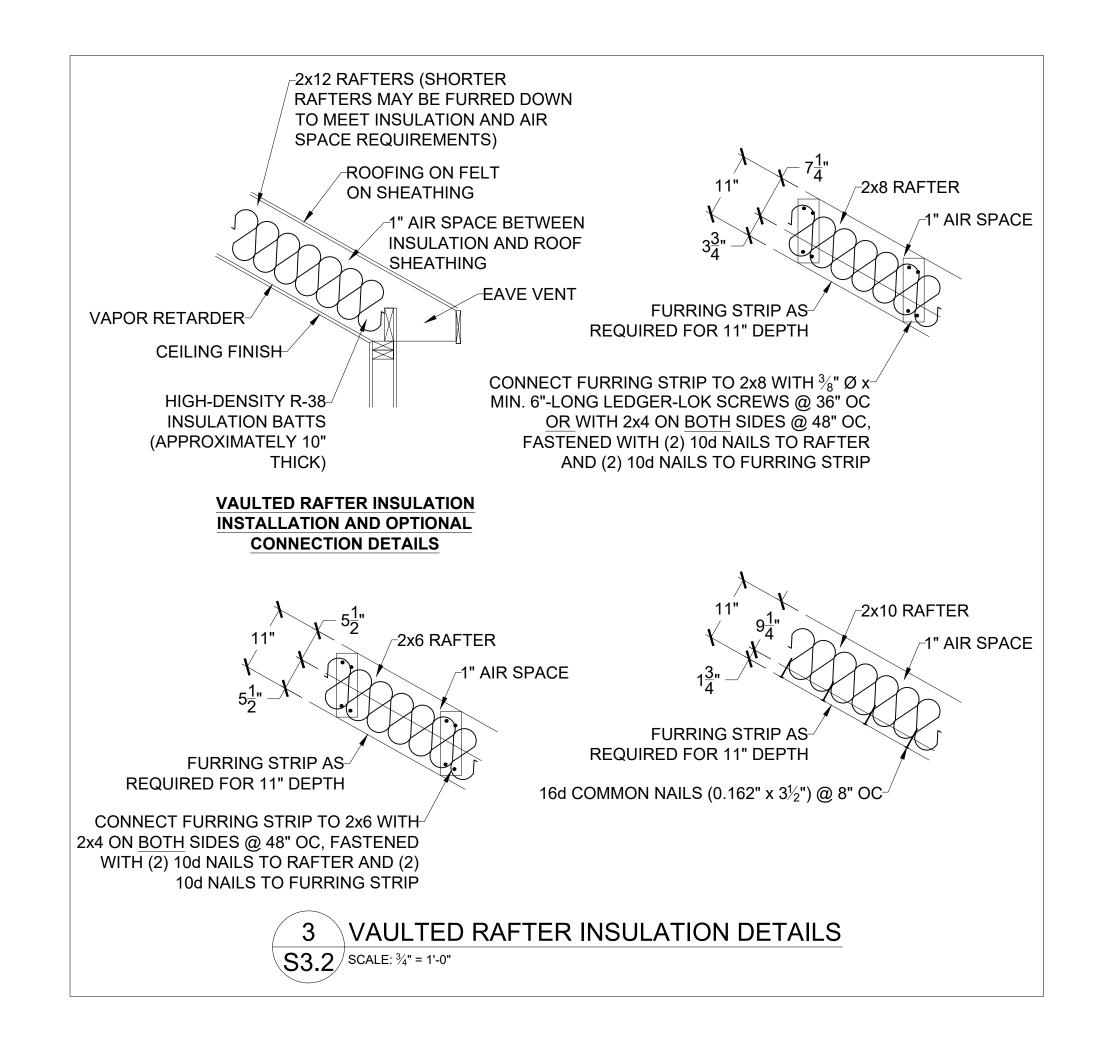
6 FIELD-CONSTRUCTED A-FRAME DETAIL S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

7 \FLOOR JOIST TO FLUSH STEEL BEAM DETAIL S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



(11) 8d NAILS IN EACH END LENGTH	HEADE PER PL	ER/BEAM LAN	
ARING WALL	BEARING WALL		BEARING WAL
10 HEADER/BEAM CONNE	ECTION OPTIONS AT OUTDO	OR/OPEN SPACE	

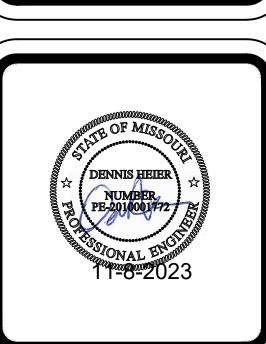


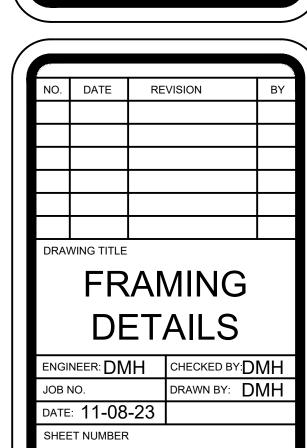
	SPACING (INCHES O.C.)				
HEIGHT (FT.)	24	16	12	8	
	SUPPORT	ING A ROOF	ONLY		
10 OR LESS	2x4	2x4	2x4	2x4	
12	2x6	2x4	2x4	2x4	
14	2x6	2x6	2x6	2x4	
16	2x6	2x6	2x6	2x4	
18	DR	2x6	2x6	2x6	
20	DR	DR	2x6	2x6	
SUPP	ORTING O	NE FLOOR	AND A ROOF	=	
10 OR LESS	2x6	2x4	2x4	2x4	
12	2x6	2x6	2x6	2x4	
14	2x6	2x6	2x6	2x6	
16	DR	2x6	2x6	2x6	
18	DR	2x6	2x6	2x6	
20	DR	DR	2x6	2x6	
SUPPO	ORTING TW	O FLOORS	AND A ROC	F	
10 OR LESS	2x6	2x6	2x4	2x4	
12	2x6	2x6	2x6	2x6	
14	2x6	2x6	2x6	2x6	
16	DR	2x6	2x6	2x6	
18	DR	DR	2x6	2x6	
20	DR	DR	DR	2x6	

NOTES: 1) DR = DESIGN REQUIRED 2) UTILITY, STANDARD, STUD AND #3 GRADE LUMBER OF ANY SPECIES ARE NOT PERMITTED 3) THIS TABLE DOES NOT APPLY FOR STUDS SUPPORTING MEMBERS WITH A TRIB. LENGTH GREATER THAN 6'-0"

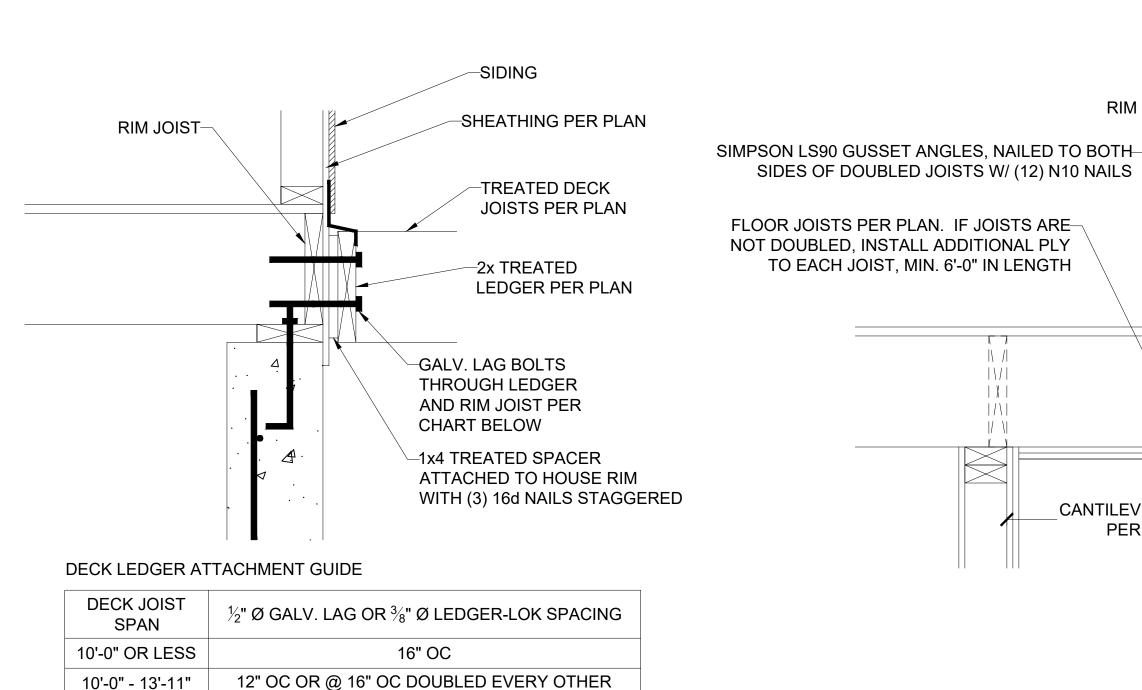
8 MAXIMUM ALLOWABLE LENGTH OF S3.2/WOOD WALL STUDS (IRC TABLE 602.3.1)







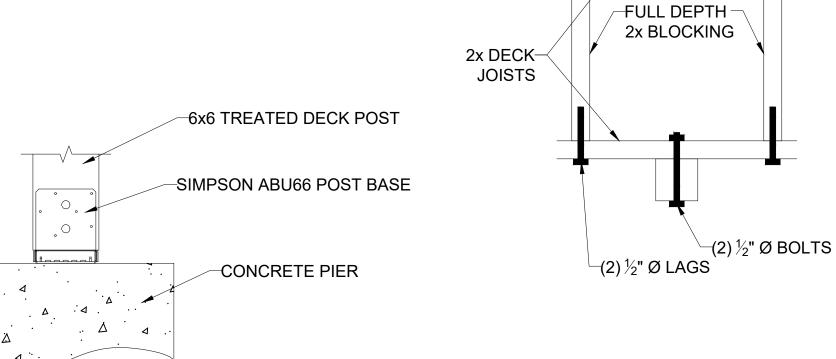
11/22/2023

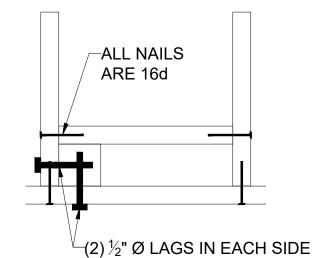


FLOOR JOISTS PER PLAN. IF JOISTS ARE-NOT DOUBLED, INSTALL ADDITIONAL PLY SHEATHING PER PLAN TO EACH JOIST, MIN. 6'-0" IN LENGTH -2x TREATED LEDGER PER PLAN GALV. LAG BOLTS THROUGH LEDGER CANTILEVER LENGTH AND RIM JOIST PER PER PLAN **CHART BELOW**

SIDES OF DOUBLED JOISTS W/ (12) N10 NAILS

RIM JOIST-





WITH (3) 16d NAILS STAGGERED

-SIDING

-1x4 TREATED SPACER

ATTACHED TO HOUSE RIM

TREATED DECK JOISTS PER

PLAN (MAX. SPAN = 16'-0")

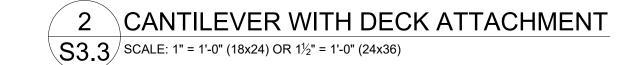
3 DECK POST BASE 3.3 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

LEDGER ATTACHMENT S3.3 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

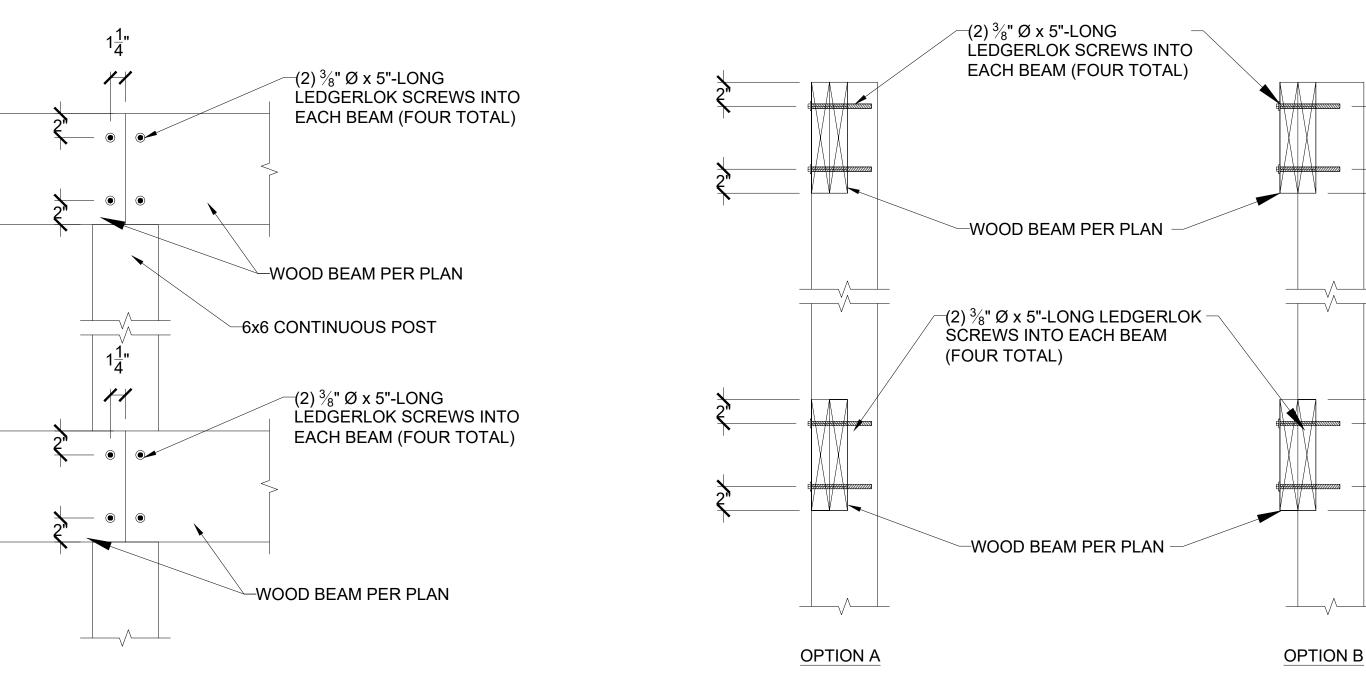
8" OC OR @ 16" OC DOUBLED

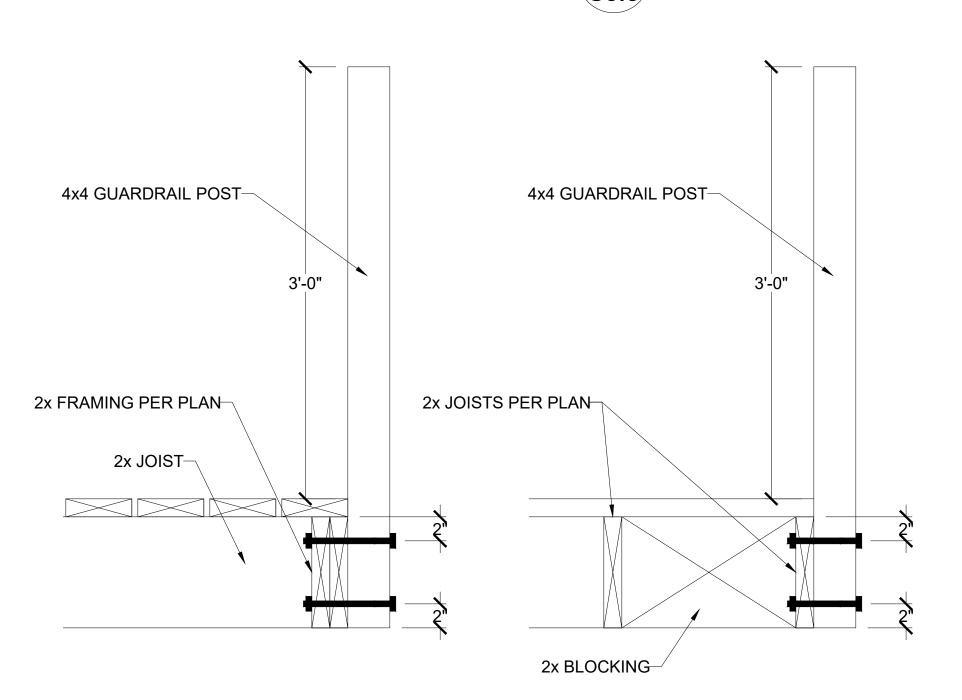
10'-0" - 13'-11"

14'-0" - 18'-0"



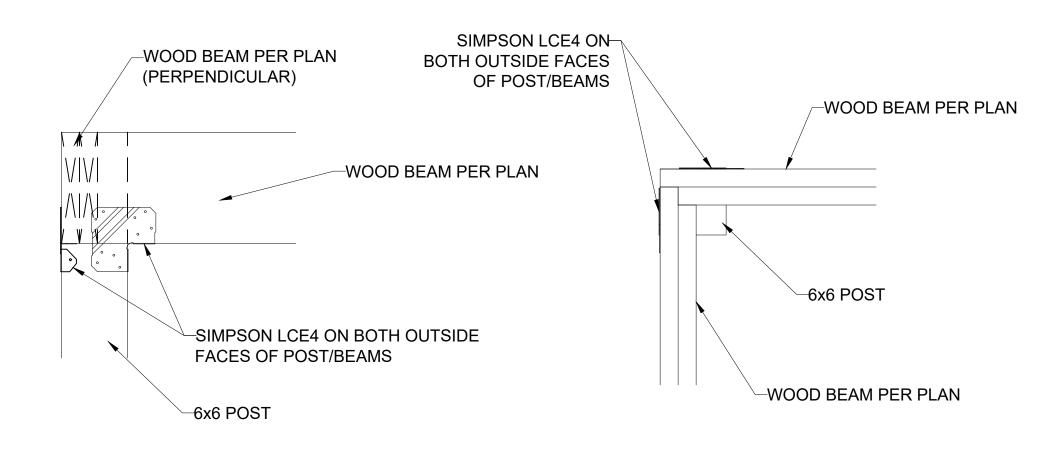


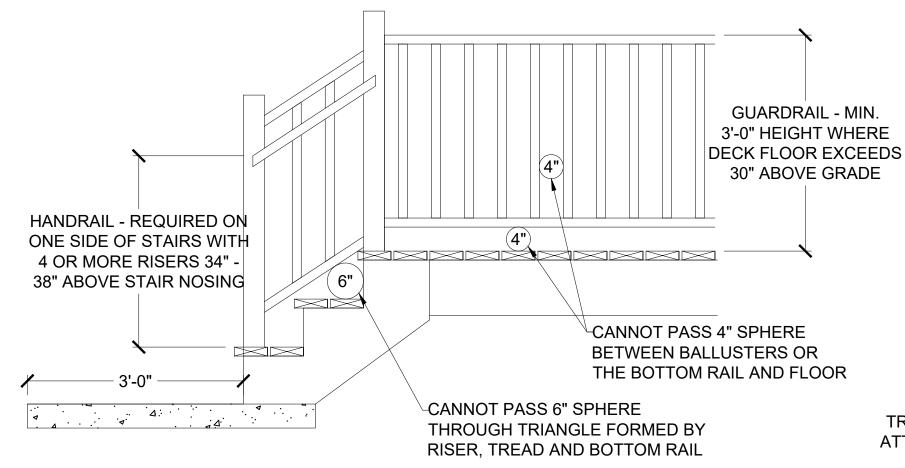


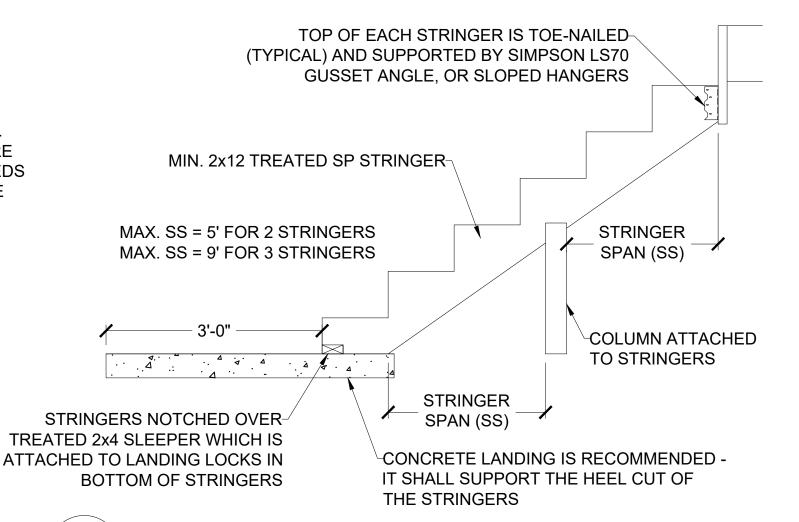


5 LET-IN (COVERED) DECK BEAM CONNECTION









7 ALTERNATE COVERED DECK/PORCH INTERSECTION S3.3 CORNER BEAM CONNECTION

8 GUARDRAIL DETAIL SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

9 STAIR STRINGER DETAIL (MAX. 5' STAIR WIDTH) S3.3 SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)



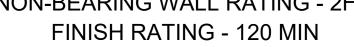
NO. DATE REVISION DRAWING TITLE FRAMING DETAILS ENGINEER: DMH | CHECKED BY: DMH DRAWN BY: DMH DATE: 11-08-23 SHEET NUMBER

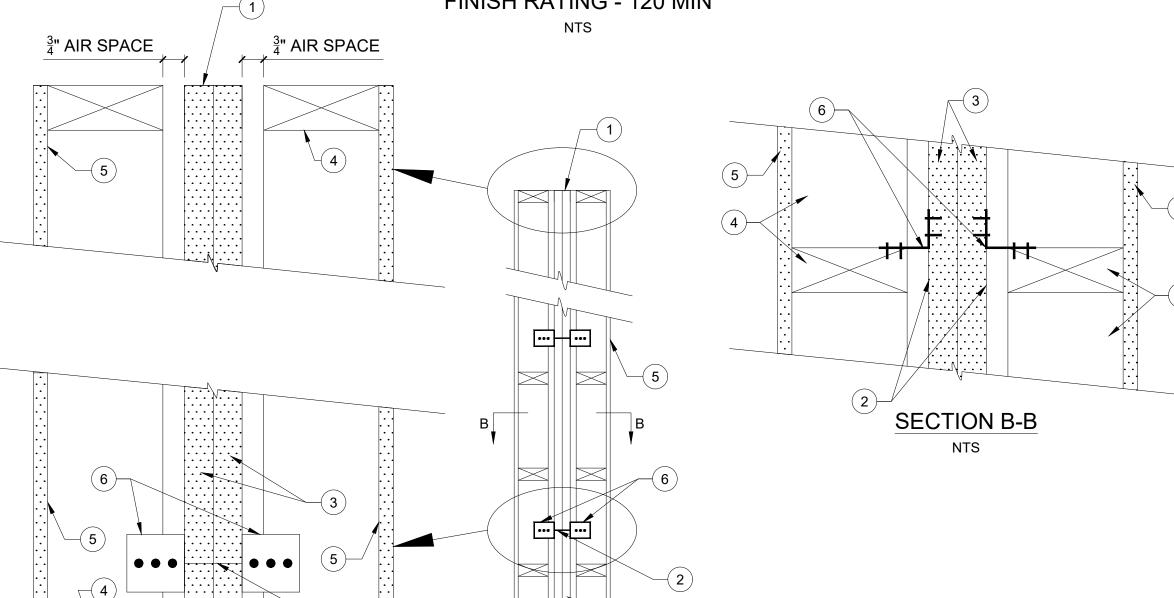
11/22/2023

SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

DESIGN NO. U366

NON-BEARING WALL RATING - 2HR





#	COMPONENT
1	2" WIDE CHANNEL AT FLOOR, INTERMEDIATE OR OF TOP WALL
2	2" DEEP x 1 ³ ₈ " H-SHAPED STEEL STUDS @ 24" OC
3	(2) LAYERS OF 1" THICK GYPSUM BOARD LINER PANELS IN 24" WIDTHS
4	2x4 WOOD STUDS @24" OC MAX, MIN $\frac{3}{4}$ " SEPARATION BETWEEN WOOD FRAMING & AREA SEPARATION WALL
5	MIN_{2}^{1} " THICK x 4' WIDE GYPSUM BOARD APPLIED HORIZONTAL OR VERTICAL
6	ALUMINUM ANGLE ATTACHMENT CLIPS- MIN 2" WIDE WITH MIN 2" AND 2^1_2 " LEGS

AREA SEPARATION WALL: (MAX HEIGHT - 44 FT)

4

CONFIGURATION B

EXPOSED TO FIRE FROM EITHER SIDE

NTS

- 1. FLOOR, INTERMEDIATE OR TOP OF WALL 2 IN. WIDE CHANNEL SHAPED WITH 1-IN LONG LEGS FORMED FROM NO. 25 MSG GALV STEEL, SECURED WITH SUITABLE FASTENERS SPACED @ 24 IN OC
- 2. STEEL STUDS STEEL MEMBERS FORMED FROM NO. 25 MSG GALV STEEL HAVING "H" SHAPED FLANGE SPACED @ 24 IN OC; OVERALL DEPTH 2 IN AND FLANGE WIDTH 1-3/8 IN.
- 3. GYPSUM BOARD* 2 LAYERS OF 1 IN THICK GYPSUM WALLBOARD LINER PANELS, SUPPLIED IN NOM 24 IN WIDTHS. VERTICAL EDGES OF PANELS FRICTION FITTED INTO "H" SHAPED STUDS.

 (JAMES HARDIE GYPSUM INC-TYPE HARDILINER)

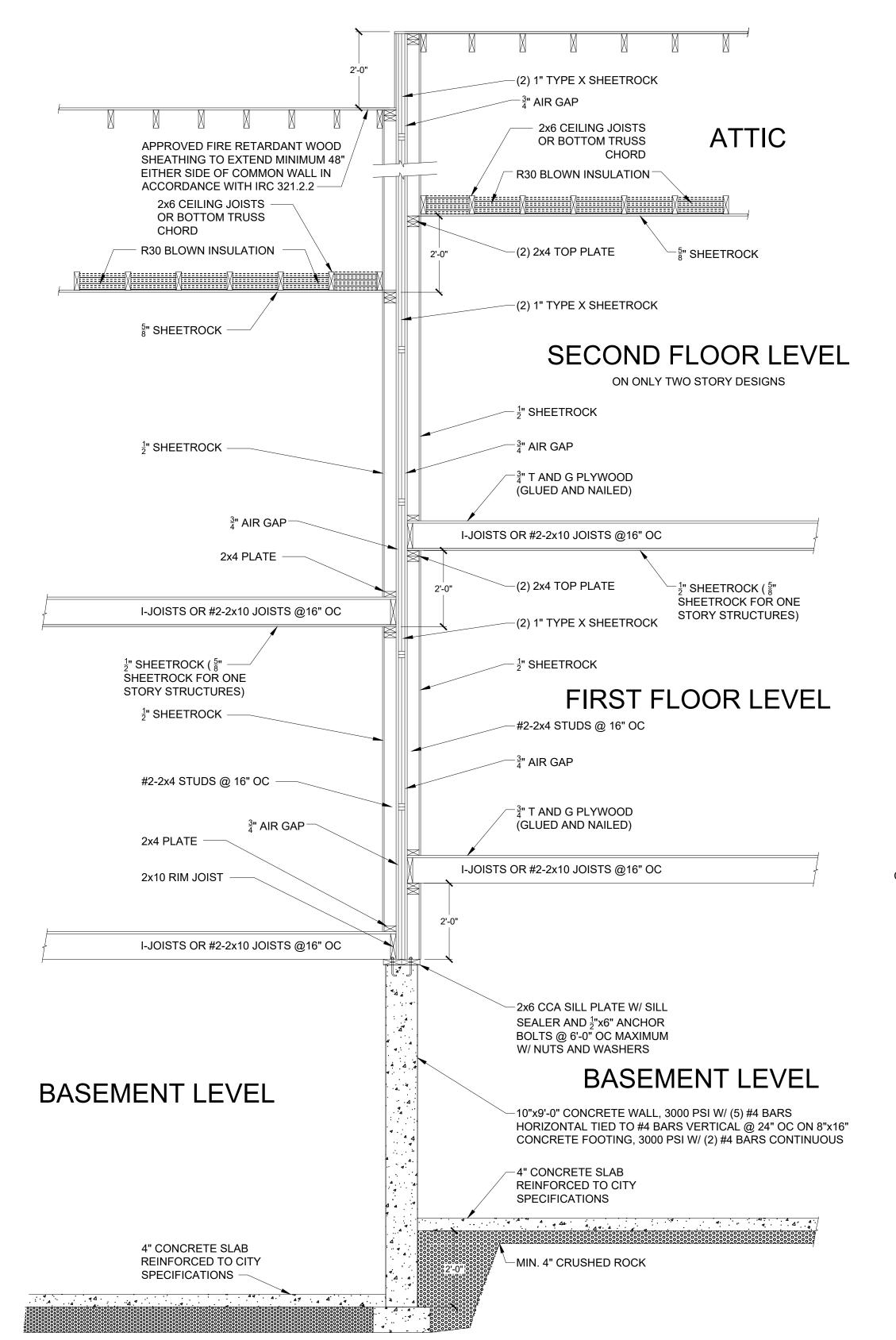
PROTECTED WALL: (BEARING OR NON-BEARING WALL)

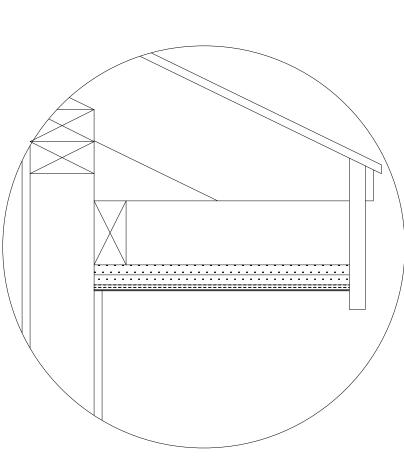
- 4. WOOD STUDS NOM 2 BY 4 IN. MAX SPACING @ 24 IN. OC. STUDS CROSS-BRACED AT MIDHEIGHT WHERE NECESSARY FOR CLIP ATTACHMENT. MIN. $\frac{3}{4}$ " SEPARATION BETWEEN WOOD FRAMING AND AREA SEPARATION WALL.
- 5. GYPSUM BOARD CLASSIFIED OR UNCLASSIFIED MIN. $\frac{1}{2}$ IN. THICK, 4FT WIDE, APPLIED EITHER HORIZONTALLY OR VERTICALLY. WALLBOARD ATTACHED TO STUDS WITH $1\frac{1}{4}$ IN. LONG STEEL DRYWALL NAILS SPACED @ 8 IN. OC. VERTICAL JOINTS LOCATED OVER STUDS. (OPTIONAL) JOINTS COVERED WITH PAPER TAPE AND JOINT COMPOUND. NAIL HEADS COVERED WITH JOINT COMPOUND.
- 6. ATTACHMENT CLIPS ALUMINUM ANGLE, 0.063 IN. THICK, MIN 2 IN. WIDE WITH MIN 2 IN. AND $2\frac{1}{4}$ IN. LEGS. CLIPS SECURED WITH TYPE S SCREWS $\frac{3}{8}$ IN. LONG TO "H" STUDS AND WITH TYPE W SCREWS $1\frac{1}{4}$ IN. LONG TO WOOD FRAMING THROUGH HOLES PROVIDED IN CLIP. CLIPS SPACED A MAX OF 10 FT OC VERTICALLY BETWEEN WOOD FRAMING AND "H" STUDS FOR SEPARATION WALLS UP TO 23 FT HIGH. FOR SEPARATION WALLS UP TO 44FT HIGH, CLIPS SPACED AS DESCRIBED ABOVE FOR THE UPPER 24 FT AND THE REMAINING WALL AREA BELOW REQUIRES CLIPS A MAX 5 FT OC VERTICALLY BETWEEN WOOD FRAMING AND "H" STUDS.

*BEARING THE UL CLASSIFICATION MARK

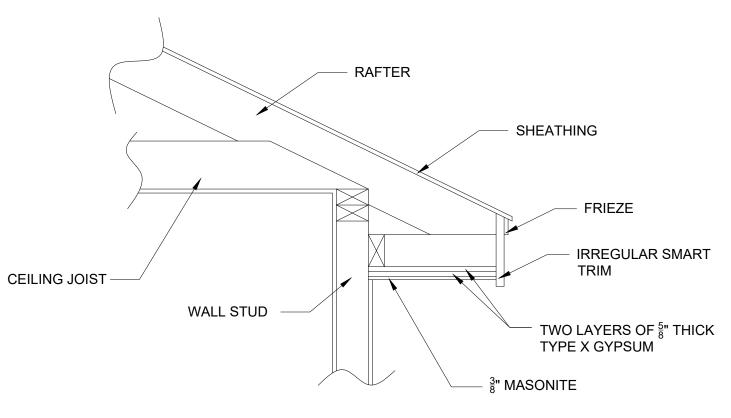
SEPARATION WALL AND AJOINING WALL NOTES:

- TWO HOUR FIRE WALL PER UL DESIGN # U366 SHOWN IN THE UL FIRE RESISTANCE DIRECTORY.
- B. INSULATE STUD CAVITIES WITH $3\frac{1}{2}$ " BATT INSULATION
- C. PLUMBING OR ELECTRICAL ALLOWED IN AJOINING WALLS
- D. A SEPARATE FIRE SEPARATION WALL INSPECTION WILL BE REQUIRED
 E. ANY SHAFT WALL PENETRATIONS IN EXCESS OF ¹/₈" BUT LESS THAN ³/₈" TO BE FILLED WITH APPROVED
- FIRE CAULK OR FIRE FOAM. PENETRATIONS IN EXCESS OF $\frac{1}{2}$ " TO BE FIRE PROOFED WITH OVERLAPPING LAYER OF $\frac{5}{8}$ " TYPE X SHEET ROCK, PROPERLY NAILED AND GLUED. SEAL ADDITIONAL DRYWALL PATCH COMPLETELY WITH FIRE CAULK
- ATTIC FIRE SEPARATION WALL: (1)-2 HOUR SHAFT WALL FIRE TEST U366





TWO LAYERS OF $\frac{5}{8}$ " THICK TYPE X GYPSUM BOARD COVERED BY $\frac{3}{8}$ " MASONITE APPLIED AT RIGHT ANGLES W/ 3" LONG TYPE W SCREWS @ 8" OC

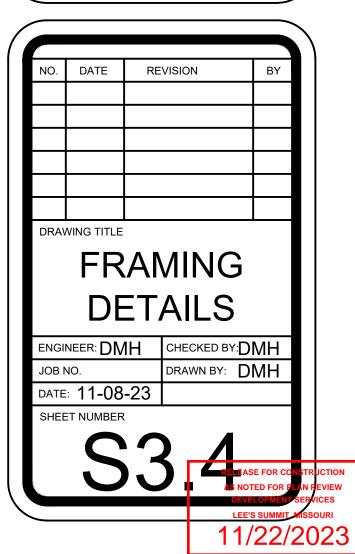


1 HOUR SOFFIT DETAIL



13 TRIPLEX 13, THE TOWNHOMES OF CHAPEL RIDGE PLAT 327, and 829 NE ALGONQUIN ST.





FIRE WALL SECTION

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