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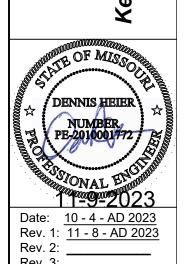
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"For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16).

RESIDENTIAL DESIGN LLC
sile/Text: (816) 547-4437 Email: jpfeifer@viewpointdesign.

Chapel Ridge - 2nd Plat
Street Address:
817, and 815 NE Algonquin St.,
Lee's Summit. Missouri

TCR012 Triplex
General Contractor:
in Higdon Construction, LLC

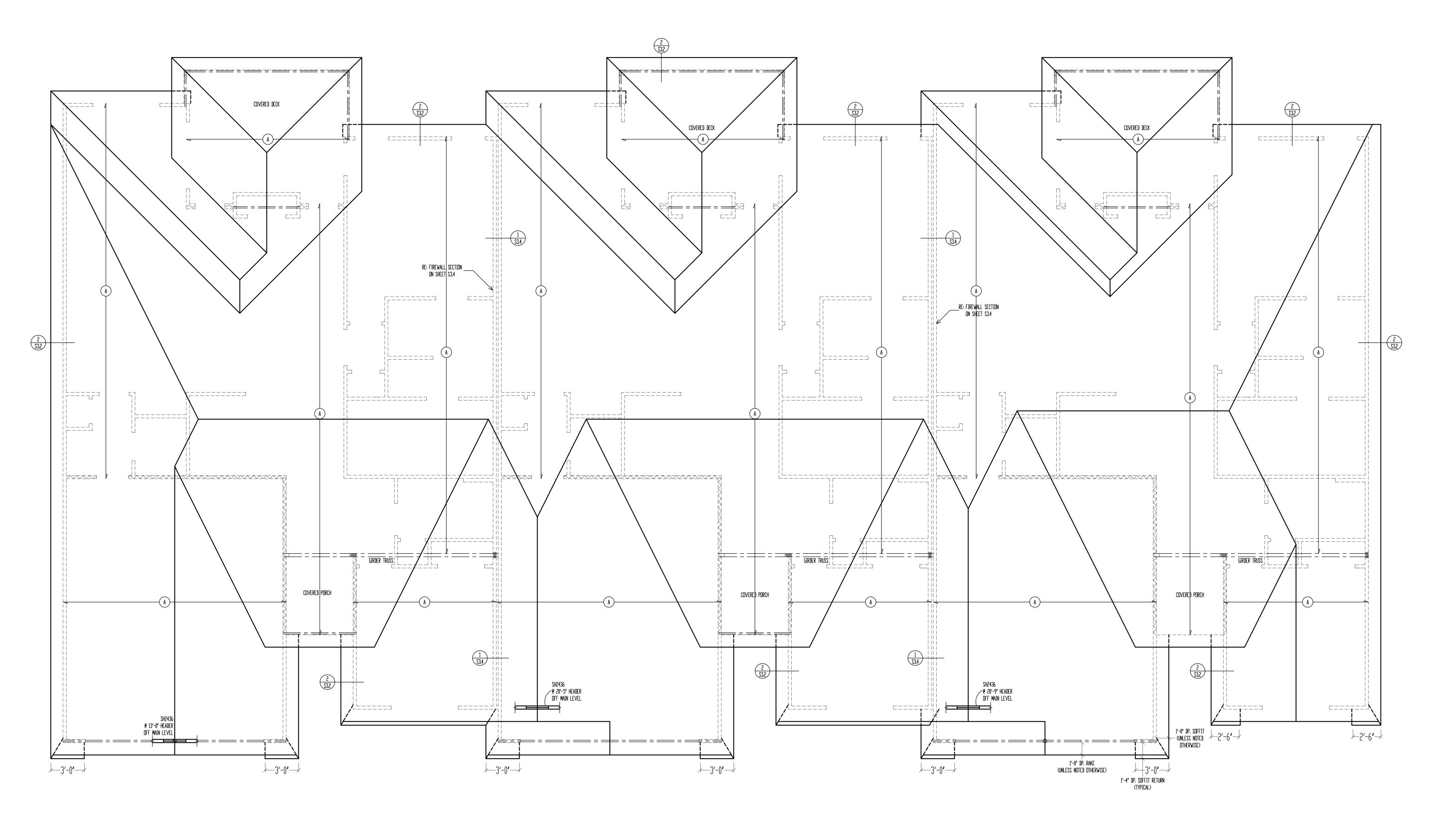


Sheet Title: **ELEVATIONS**

Sheet No.:

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of 4



ROOF

SCALE: 1/4'' = 1'-0''

TRUSS SCHEDULE

PREMANUFACTURED ROOF TRUSSES @ 24' D.C (SEE SEPARATE LAYOUT BY MANUFACTURER)

ROOF TRUSSES

ROOF TRUSSES PROPOSED TO BE USED.

TRUSSES SHALL BE DESIGNED FOR 20 PSF SNOW LOAD, 10 PSF ROOF DEAD LOAD, 10 PSF CEILING LIVE LOAD, AND 5 PSF CEILING DEAD LOAD.

THE ENGINEER RESPONSIBLE FOR THE STRUCTURAL DESIGN OF THE HOUSE SHALL REVIEW THE TRUSS DRAWINGS FOR GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING, PRIOR TO SUBMITTING THE TRUSS DRAWINGS TO THE CODES ADMINISTRATION OFFICE FOR APPROVAL.

FAILURE OF THE RESPONSIBLE PARTIES TO SUBMIT THE TRUSS DRAWINGS TO THE RESPONSIBLE ENGINEER SHALL RELIEVE THE ENGINEER OF ALL LIABILITY FOR THE ENTIRE PLAN. TRUSS LOADS AND TRANSFER PATHS ON THIS PLAN ARE ASSUMED LOADS ONLY AND CAN ONLY BE VERIFIED AFTER TRUSS LAYOUTS AND DESIGNS ARE COMPLETED.

ATTACH EACH END OF EACH TRUSS TO TOP PLATE WITH SIMPSON H2.5.

ATTACH GIRDER TRUSSES TO TOP PLATE WITH CONNECTOR RATED FOR MANUFACTURER'S DESIGN UPLIFT LOAD (SEE SEPARATE DESIGN BY MANUF.)

- 2-PLY GIRDER: LGT2

- 3-PLY GIRDER: LGT3-SDS2.5

- 4-PLY GIRDER: LGT4-SDS3

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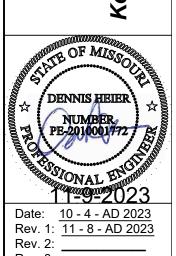
Street Address:

Street Address:

17, and 815 NE Algonquin St.,

Lee's Summit, Missouri

TCR012 Triplex
General Contractor:
In Higdon Construction, LLC



Sheet Title:
ROOF PLAN

Sheet No.:
A-2
of 4

1. MAIN LEVEL EXTERIOR WALLS SHALL BE SHEATHED W/ 7/16' D.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR EQUAL, 2. \ \ \ \ \ \ \ \ \ = G.B.: 1/2" MIN. GYPSUM BDARD OVER STUDS SPACED 24" MAX FASTENED W/ NO. 6 - 1 1/4" TYPE W OR S DRYWALL SCREWS @ 7" D.C. EDGES & FIELD.

(MIN. 8'-0' SECTIONS ONE SIDE OF WALL (OR) MIN. 4'-0' SECTION FOR BOTH SIDES) 3. / / / / / / / / / / / / = LOAD BEARING INTERIOR WALL.4. (2) 2" X 10" #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.

5. LOW TIES @ 4'-0' D.C. (TYPICAL) 6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLDCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS).

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. ALL DESIGNATED 2' X 6' WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS. 10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2' X 4'S @ 16' D.C., UNLESS NOTED OTHERWISE.

12. EXTERIOR WALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW WITH 16d COMMON NAILS @ 8' D.C. MAX. (WHERE APPLICABLE.) 13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS: (2) 1 3/4" LVL PLIES = 3 1/2" GLULAM (3) 1 3/4" LVL PLIES = 5 1/2" GLULAM

14. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY OPENINGS.

TRUSS SCHEDULE PREMANUFACTURED ROOF TRUSSES @ 24" D.C (SEE SEPARATE LAYDUT BY MANUFACTURER)

9'-0" CEILING

SCALE: 1/4" = 1'-0"

UNIT C: 1483 SQ. FT. TOTAL: 4449 SQ. FT.

GARAGE A: 472 SQ. FT. GARAGE B: 472 SQ. FT. GARAGE C: 472 SQ. FT. COV. DUT/LIV A: 171 SQ. FT. COV. DUT/LIV B: 171 SQ. FT.

CDV. DUT/LIV C: 171 SQ. FT.

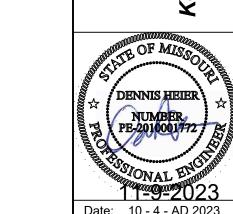
- THE ENGINEER RESPONSIBLE FOR THE STRUCTURAL DESIGN OF THE HOUSE SHALL REVIEW THE TRUSS DRAWINGS FOR GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING, PRIOR TO SUBMITTING THE TRUSS DRAWINGS TO THE CODES ADMINISTRATION OFFICE FOR APPROVAL. - FAILURE OF THE RESPONSIBLE PARTIES TO SUBMIT THE TRUSS DRAWINGS TO THE RESPONSIBLE ENGINEER SHALL RELIEVE THE ENGINEER OF ALL LIABILITY FOR THE ENTIRE PLAN. TRUSS LOADS AND TRANSFER PATHS ON THIS PLAN ARE ASSUMED LOADS ONLY AND CAN ONLY BE VERIFIED AFTER TRUSS LAYOUTS AND DESIGNS ARE COMPLETED. - ATTACH EACH END OF EACH TRUSS TO TOP PLATE WITH SIMPSON H2.5. - ATTACH GIRDER TRUSSES TO TOP PLATE WITH CONNECTOR RATED FOR MANUFACTURER'S DESIGN UPLIFT LOAD (SEE SEPARATE DESIGN BY MANUF.) - 2-PLY GIRDER: LGT2 - 3-PLY GIRDER: LGT3-SDS2.5 - 4-PLY GIRDER: LGT4-SDS3

CEILING DEAD LOAD.

<u>roof trusses</u> - roof trusses proposed to be used.

- TRUSSES SHALL BE DESIGNED FOR 20 PSF SNOW LOAD, 10 PSF ROOF DEAD LOAD, 10 PSF CEILING LIVE LOAD, AND 5 PSF

INSTALLED PER MANUFACTURER'S SPECIFICATIONS.



Trible: Triplex

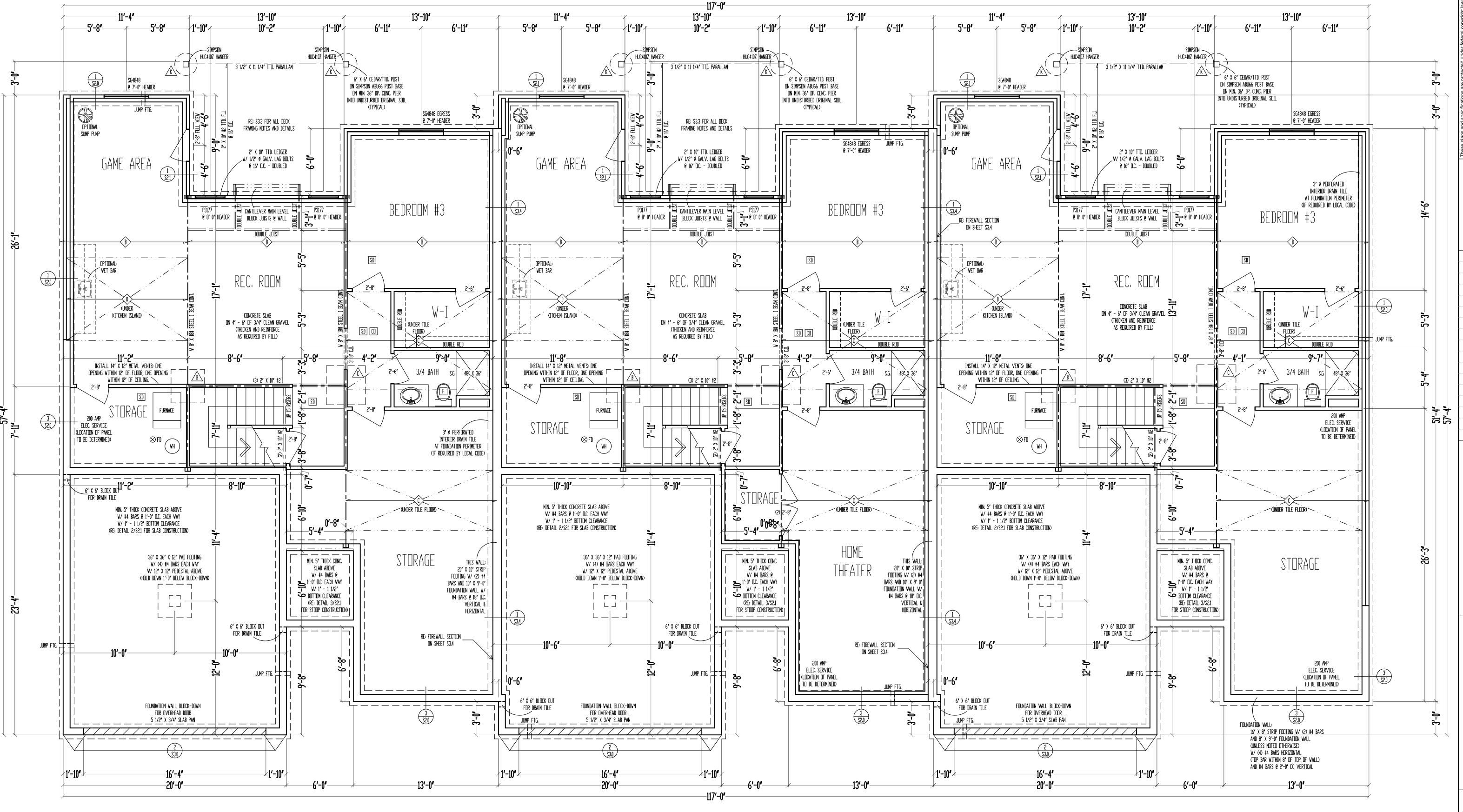
TCR0

Signal Color

Date: 10 - 4 - AD 2023 Rev. 1: 11 - 8 - AD 2023

Sheet Title: **MAIN LEVEL** PLAN

Sheet No.:



LOWER LEVEL A: 860 SQ. FT.
LOWER LEVEL B: 1226 SQ. FT.
LOWER LEVEL C: 860 SQ. FT.

UNFINISHED A: 428 SQ. FT. UNFINISHED B: 75 SQ. FT. UNFINISHED C: 428 SQ. FT.

9'-0" FOUNDATION WALLS
(UNLESS NOTED OTHERWISE)
ON 16" X 8" STRIP FOOTINGS
(STEP WHERE GRADE REQUIRES)

2" X 10" FLOOR SYSTEM
FOUNDATION
SCALE: 1/4" = 1'-0"

P	١,	
Â	3' X 11 GA. STEEL COLUMN ON 30' X 30' X 10' PAD FOOTING W/ (4) #4 BARS EACH WAY (12.5k)	
B	3 1/2' X 11 GA. STEEL COLUMN DN 36' X 36' X 10' PAD FOOTING W/ (4) #4 BARS EACH WAY (18.0k)	
<u>⟨</u>	3' SCH. 40 STEEL COLUMN ON 42' X 42' X 12' PAD FOOTING W/ (5) #4 BARS EACH WAY (24.5k)	
	3 1/2' SCH. 40 STEEL COLUMN DN 48' X 48' X 12' PAD FOOTING W/ (6) #4 BARS EACH WAY (32.0k)	
Œ	3 1/2' SCH. 40 STEEL COLUMN DN 54' X 54' X 14' PAD FOOTING W/ (7) #4 BARS EACH WAY (40.5k)	
F	3 1/2" SCH. 40 STEEL COLUMN DN 60" X 60" X 14" PAD FOOTING W/ (8) #4 BARS EACH WAY (50.0k)	

LIFK FURITING 2CHFNOFF				JUI21 2CHFDOFF
<u>~</u>	12" Ø PIER FTG.		(4)	2" X 10" #3 FLOOR JOIST @ 16" D.C.
$ \overline{z}\rangle$	16" Ø PIER FTG.		⟨B⟩	2" X 10" #2 FLOOR JOIST @ 16" D.C.
> >>	18' Ø PIER FTG.		\Leftrightarrow	2' X 10' #2 FLOOR JOIST @ 16' D.C. DOUBLE EVERY OTHER
<u>ƙ</u>	24' Ø PIER FTG.			2" X 10" #2 FLOOR JOIST @ 16" O.C. DOUBLED

3. /\/\/\/\/\/\\ = LOAD BEARING INTERIOR WALL.
4. (2) 2' X 10' #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.

5. LOW TIES @ 4'-0' D.C. (TYPICAL)
6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS.
7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS).

BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS).
 PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS.
 ALL DESIGNATED 2' X 6' WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS.
 ALL UNSQUARE WALLS SHALL BE 45', UNLESS NOTED OTHERWISE.

11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2' X 4'S @ 16' D.C., UNLESS NOTED OTHERWISE.

12. 1/2' Ø ANCHOR BOLTS W/ MIN. 7' EMBEDMENT @ 48' D.C. MAX. & WITHIN 6' - 12' OF END OF EACH PLATE LENGTH.

13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS:

(2) 1 3/4' LVL PLIES = 3 1/2' GLULAM

(3) 1 3/4' LVL PLIES = 5 1/2' GLULAM

14. NEW FDUNDATION SHALL BEAR ON ORIGINAL SDIL WITH MINIMUM BEARING CAPACITY OF 1500 PSF. A GEDTECHNICAL ENGINEER IS RECOMMENDED FOR VERIFICATION OF THESE CONDITIONS DURING THE EXCAVATION PHASE. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANYTHING SHORT OF THE AFOREMENTIONED REQUIREMENTS.

15. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY OPENINGS.

DENNIS HEIER

NUMBER
PE-2010001772

Date: 10 - 4 - AD 2023
Rev. 1: 11 - 8 - AD 2023
Rev. 2: Rev. 3: Sheet Title:

FOUNDATION
PLAN

Trible:

TCR0

Project Title:

12 Trip

n Construe

Sheet No.:

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of 4

RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI

AN	TRUCTION S REVIEW RVICES					
MI	SSOURI		FASTENER SCHEDULE FO	R STRUCTURAL MEMBERS		
10	DESCRIPTION OF BUILDING ELEM	MENTS	NUMBER AND TY	PE OF FASTENER		SPACING OF FASTENERS
			RC	DOF ¹		
	BLOCKING BETWEEN JOISTS OR RAFTI	ERS TO TOP	3-84 (21/-)	' x 0.113")		
PLATE, TOE NAIL			,	,		-
	CEILING JOISTS TO PLATE, TOE	NAIL	3-8d (2½'	' x 0.113")		-
	CEILING JOISTS NOT ATTACHED TO F RAFTER, LAPS OVER PARTITIONS, F.		3-	10d		-
	COLLAR TIE TO RAFTER, FACE NAIL O	OR 1¼" x 20	3-10d (3"	' x 0.128")		
	GAGE RIDGE STRAP			·		-
	RAFTER OR ROOF TRUSS TO PLATE,	TOE NAIL		0.135") OR 3-10d COMMON " x 0.148")		ILS ON ONE SIDE AND 1 TOE NAIL ON E SIDE OF EACH RAFTER OR TRUSS
	ROOF RAFTERS TO RIDGE, VALLEY		4-16d (3½" x 0.135").	, 3-16d (3½" x 0.135")		-
	RAFTERS: TOE NAIL FACE NA	AIL	, , , , , , , , , , , , , , , , , , ,			
	BUILT-UP STUDS - FACE NAI	1	T	x 0.128")		24" O.C.
	ABUTTING STUDS AT INTERSECTIN			x 0.135")		12" O.C.
	CORNERS, FACE NAIL		100 (0/2	X 0.100)		
	BUILT-UP HEADER, TWO PIECES WITH	½" SPACER	16d (3½"	x 0.135")	•	16" O.C. ALONG EACH EDGE
	CONTINUED HEADER, TWO PIE	CES	16d (3½"	x 0.135")		16" O.C. ALONG EACH EDGE
	CONTINUED HEADER, TWO FIE			,		
	CONTINUOUS HEADER TO STUD, T	OE NAIL	4-8d (2½'	' x 0.113")		-
	DOUBLE STUDS, FACE NAIL		10d (3" :	x 0.128")		24" O.C.
			404 (21)	0. 4.20!!\		2411.0.0
	DOUBLE TOP PLATES, FACE N	AIL	10d (3)	x 0.128")		24" O.C.
	DOUBLE TOP PLATES, MINIMUM 24-ING OF END JOINTS, FACE NAIL IN LAPP		8-16d (3½	" x 0.135")		-
	<u> </u>		16d (3½"	x 0.135")		16" O.C.
	SOLE PLATE TO JOIST OR BLOCKING,	FACE NAIL	100 (3/2	X 0.133)		
	SOLE PLATE TO JOIST OR BLOCKING A	AT BRACED	3-16d (3½	" x 0.135")		16" O.C.
	STUD TO SOLE PLATE, TOE N	ΛΙΙ	3-8d (2½" x 0.113") O	R 2-16d (3½" x 0.135")		-
	3100 TO SOLE PLATE, TOE NA	AIL	, ,	, ,		
	TOP OR SOLE PLATE TO STUD, EN	ID NAIL	2-16d (3½	" x 0.135")		-
	TOP PLATES, LAPS AT CORNERS		2-10d (3"	' x 0.128")		-
	INTERSECTIONS, FACE NAIL	-	2-8d (2½" x 0.113")		<u> </u>	
	1" BRACE TO EACH STUD AND PLATE,	FACE NAIL	2-80 (2/2	*X U.113*)		-
	1"x6" SHEATHING TO EACH BEARING,	FACE NAIL	2-8d (2½'	' x 0.113")		-
	411.011 CLICATURIO TO EACU DE ADINO	EACE NAIL	2-8d (2½'	' x 0.113")		-
	1"x8" SHEATHING TO EACH BEARING,	FACE NAIL	(-,2	,		
	WIDER THAN 1"x8" SHEATHING TO EAC FACE NAIL	H BEARING,	3-8d (2½'	' x 0.113")		-
			FI (DOR ¹		
			T			
	JOIST TO SILL OR GIRDER, TOE	NAIL	3-8d (2½'	' x 0.113")		-
	RIM JOIST TO TOP PLATE, TOE NAII APPLICATIONS ALSO)	L (ROOF	8d (2½"	x 0.113"		6" O.C.
	<u>_</u>		94 (2½"	v 0 112")		6" O.C.
	RIM JOIST OR BLOCKING TO SILL PLAT	E, TOE NAIL	8d (2½" x 0.113")			0 0.0.
	1"x6" SUBFLOOR OR LESS TO EACH JO NAIL	OIST, FACE	2-8d (2½" x 0.113")		-	
	2" SUBFLOOR TO JOIST OR GIRDER, I	BLIND AND	2-16d (3½" x 0.135")		_	
	FACE NAIL		2 100 (0,2 × 0.100)			
	2" PLANKS (PLANK AND BEAM - FLOOR	AND ROOF)	2-16d (3½" x 0.135")		AT EACH BEARING	
	BUILT-UP GIRDERS AND BEAMS, 2-INC	`H I I IMRED	10d (3" :	x 0.128")	NAIL EACH LAYER AS FOLLOWS: 32" O.C. AT TOP	
	LAYERS	T LOWIDLIN				OM AND STAGGERED. TWO NAILS AT ENDS AND AT EACH SPLICE
	LEDGER STRIP SUPPORTING JOISTS O		3-16d (3½	" x 0.135")	,	AT EACH JOIST OR RAFTER
	LEDGER STRIF SUFFORTING JOISTS O	IN NAFTENS				
	DESCRIPTION OF BUILDING MATERIALS	•	IPTION OF FASTENER	R STRUCTURAL MEMBERS EDGE SPACING (INC	•	INTERMEDIATE SUPPORTS (INCHES)
	WOOD STRUCTURAL PANELS, SUB	I		ATHING TO FRAMING AND PA	ARTICLEBOA	RD WALL SHEATHING TO FRAMING
	3/8" - 1/2"		MON (2" x 0.113") NAIL , WALL) 8d COMMON NAIL	6		12
	107 11 411	04 0004	(ROOF)			40
	¹⁹ / ₃₂ " - 1"		MON NAIL (2½" x 0.131")	6		12
	11/8" - 11/4"		ON (3" x 0.148") NAIL OR 8d 131") DEFORMED NAIL	6		12
			OTHER WALI	L SHEATHING ¹		
	1/11 0 1 7 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		ANIZED ROOFING NAIL;	_		_
	½" GYPSUM SHEATHING	STAPLE GA SCRI	LVANIZED, 1½" LONG; 1¼" EWS, TYPE W OR S	7		7
	5/II OVEDOLINA OLIFATILINO	1¾" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1½"		_		_
	%" GYPSUM SHEATHING	STAPLE GA SCRI	EWS, TYPE W OR S	7		7
	WC	OD STRUCTU	DAL DANELS COMBINATIO	N SUBFLOOR UNDERLAYME	NT TO EDAM	ING ¹
	WC			332. LOOK ONDERLATME	IO I RAIVI	
	¾" AND LESS		ED (2" x 0.120") NAIL OR 8d	6		12
	/4 ANU LESS	СОММ	ON (2½" x 0.131") NAIL	o		12
	7⁄8" - 1"		N (2½" x 0.131") NAIL OR 8d	6		12
	/8 - 1	DEFORM	MED (2½" x 0.120") NAIL	ō		12
	1½" - 1½"			6		12
	1/8 - 1/4	10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL		6		12

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION LISTED IN THIS TABLE

FOUNDATION NOTES

- CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLABS
- 2. THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION STANDARDS
- 3. PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A
- MINIMUM 20 GALLON SUMP PIT.

 4. FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST.
- ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.

 5. FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40
- #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE FOR FROST PROTECTION.
- 6. CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE
- FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0
 REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE)
- INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- 10. BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND, GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES
- 11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB SHALL BE DESIGNED BY A LICENSED ENGINEER
- 12. SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH ½" Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS)
 AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6
 FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET
- 14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

- 5. ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS

 ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 2x10's LINI ESS NOTED OTHERWI
- 6. ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 2x10's, UNLESS NOTED OTHERWISE ON PLANS
- 17. BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS
- 18. INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A
- 19. ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED
- 20. WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS.
- 21. ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT
- 22. JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.
- 23. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS
- 24. JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 1%" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT
- 25. ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3
- 26. ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.
- 27. ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER 1/3 OF VERTICAL DISTANCE BETWEEN CEILING AND
- 28. BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED
- 29. PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A ½" GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR
- GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION)

 30. ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi
- 31. ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi
 32. COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND
 THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM
 FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. ½" x 2" BOLTS SHALL THEN BE
 INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE
 WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE. AND WOULD NEED TO BE
- INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

 33. WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE
- VENT BEGINS 12" FROM THE CEILING.

 34. ALL ROOF SHEATHING SHALL BE $\frac{7}{16}$ " OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

- 35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0"
- 36. ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

37. ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH $\frac{1}{8}$ " TO $\frac{1}{4}$ " OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN $\frac{1}{150}$ OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

- 38. PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP.
- PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

MASONRY VENEER

- 40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1½", WITH NOT LESS THAN 5%" MORTAR OR GROUT COVER TO OUTSIDE FACE.
- 41. VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 7/8" CORRUGATED.
- 42. EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.
- 43. VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

GARAGE NOTES

- 44. DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1%" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE
- 45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 90-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC SECTION R301.2.1

GARAGE NOTES (CONTINUED)

44. THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM %" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM %" TYPE X GYP. BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH %" GYP. BOARD.

45. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER

45. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING AND SHALL BE FASTENED WITH 2½"" x 0.120" NAILS AT 7" O.C. STAGGERED WITH (7) 3¼" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER. MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

DESIGN LOADING (PER TABLE R301.5)

MINIMUM UNIFORMLY DISTRIB		ADS (PSF)
USE	LIVE LOAD	ĎEAD LOAD
UNINHABITABLE ATTICS WITHOUT STORAGE	10	10
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20	10
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30	10
BALCONIES (EXTERIOR) AND DECKS	40	10 ^d
FIRE ESCAPES	40	10
GUARDRAILS AND HANDRAILS ^a	200 ^c	-
GUARDRAIL IN-FILL COMPONENTS ^b	50 ^c	ı
PASSENGER VEHICLE GARAGES	50	DEPENDENT UPON SLAB CONSTRUCTION
ROOMS OTHER THAN SLEEPING ROOM	40	10 ^d
SLEEPING ROOM	30	10 ^d
STAIRS	40	10 ^d

a. A single concentrated load applied in any direction at any point along the top.

b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement.

c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed

not to occur with any other live load.

d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed.

INSULATION/EFFICIENCY

- 1. BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1.

INSULATION AND FENESTRATION REQUIRE	1
CLIMATE ZONE	4-A
FENESTRATION U-FACTOR	0.35
SKYLIGHT U-FACTOR	0.55
GLAZED FENSTRATION SHGC	0.40
CEILING R-VALUE	49
WOOD FRAME WALL R-VALUE	13
MASS WALL R-VALUE	8 / 13
FLOOR R-VALUE	19
BASEMENT WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
SLAB R-VALUE AND DEPTH	10 AT 2'-0"
CRAWL SPACE WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	8
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	6
CATHEDRAL VAULTED CEILING R-VALUE	38

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2012 IRC.

- 1. AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED
- WITHOUT ADDITIONAL JOINT SEALS.

 2. WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE
- JOINT SO AS TO PREVENT A HINGE EFFECT.

 CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.
- DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:
- POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST.
- ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA.

EXCEPTION: THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

CHANICAL VENTILATIO	N SYSTEM FAN EFFICA	CY
AIR FLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY (CFM/WATT)	AIR FLOW RATE MAXIMUM (CFM)
ANY	2.8	ANY
ANY	2.8	ANY
10	1.4	90
90	2.8	ANY
	AIR FLOW RATE MINIMUM (CFM) ANY ANY	AIR FLOW RATE MINIMUM EFFICACY (CFM/WATT) ANY 2.8 ANY 2.8 10 1.4



ACAIG IDANACE

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SHEET NUMBER



RESIDENTIAL SEISMIC & WIND ANALYSIS

				INPUT
DETERMINE WEIGHT OF HOUSE:				CALCULATED VALUE
LOCATION		DEAD LOAD (psf)	AREA (ft ²)	WEIGHT (lbs.)
ROOF		10	6349	63490
CEILING		10	6349	63490
FIRST FLOOR		10	6349	63490
	WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WEIGHT (lbs)
FIRST FLOOR EXT. WALL DL	354.66	10	10	35466
		DEAD LOAD (psf)	AREA (ft2)	WEIGHT (lbs)
FIRST FLOOR INT. PARTITION WALL DL		6	6349	38094

	PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED)						
	FRONT	-TO-BACK			SIDE-TO-S	IDE	
	AREA	LOAD			AREA	LOAD	
SLOPED ROOF	555	4515		SLOPED ROOF	708	6024	
VERT. ROOF	853	10129	CUMULATIVE	VERT. ROOF	30	373	CUMULATIVE
1ST	1287	15282	30007	1ST	663.63	8250	14728
			PRESSURE (PSI	F) - PER ASCE CH. 6			
	SLOPED ROOF	ZONE B		9.7	ZONE C	11.3	2a (FIG. 28.6-1, ASCE7)
	WALL/VERT. ROOF	ZONE A		14.2	ZONE D	7.7	12.066
	MEAN ROOF HT., h		24				

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area.

 q_{z10} =0.00256 $K_zK_{zt}K_dV^2$ (ASCE7-10 Velocity Pressure) q_{z10_ASD} =0.6 q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)

1ST FLOOR TRIBUTARY WEIGHT $S_{S} \text{ (SITE GROUND MOTION - } \%g - \text{FROM ASCE7 SEISMIC MAP)}$ $F_{a} \text{ (from ASCE7 Table 11.4-1)}$ $S_{DS} (= 2/3 * S_{S} * F_{a})$ R (from ASCE7 Table 12.2-1)

 SEISMIC SHEAR

 LOCATION
 From ASCE7 (Eq. 12.8-1):
 V (= 1.2 * S_{DS} * W / R) (lbs.)

 1ST FLOOR
 3420

Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable Shear (#/LF)	Code Reference
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	155	per IBC, Table 2306.3(1)
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	230	per IBC, Table 2306.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	310	per IBC, Table 2306.3(1)
Exterior (Option #4)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220	AF&PA SDPWS Table 4.3A
Exterior (Option #5)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320	AF&PA SDPWS Table 4.3A
Exterior <u>(Option #6)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410	AF&PA SDPWS Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60	per IBC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)	325	

EXTERIOR SHEATHING OPTION FOR FIRST FLOOR	4
EXTERIOR SHEATHING OPTION FOR BASEMENT WALLS	4

WIDTH OF 1ST STORY (FT.)	117	WIDTH OF 2ND STORY (FT.)
DEPTH OF 1ST STORY (FT.)	60.33	DEPTH OF 2ND STORY (FT.)
BACK WALL OF GARAGE (FT.)	0	
GAR WALL 1=F-B 2=S-S	2	

1st Floor S-S

EXTERIOR STRUCTURAL WALL LENGTHS (ft.) & RESISTANCES								
		SE	ISMIC		WIND			
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)
1ST FLOOR	114	31920	49.5	13860	114	44688	49.5	19404
ADDITIONAL RESISTANCE REQUIRED Anchor Bolt Spacing (in.) 16d Nail Spacing req						16d Nail Spacing reg'd at	bottom plate (in)	
SEISMIC WIND					diameter (in.)	0.5	1st Floor F-B	11

	SEISMIC	WINE
1ST FLOOR FRONT-TO-BACK	0	0
1ST FLOOR SIDE-TO-SIDE	0	0
BASEMENT FRONT-TO-BACK	0	0

BASEMENT SIDE-TO-SIDE 0 0

RESISTANCE REQUIRED IN ADDITION TO RESISTANCE PROVIDED BY EXTERIOR WALLS**							
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	ADDITIONAL METHODS	OK?
1ST FLOOR FRONT-TO-BACK	0					0	YES
1ST FLOOR SIDE-TO-SIDE	0					0	YES

Spacing F-B (inches) spacing S-S (inches)

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE),

2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER

ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRED

	WIND UPLIFT ANALYSIS						
	X/12	DEGREES					
ROOF PITCH (MAX)	12	45.0	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2			
_		ASCE 7					
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)			
OVERHANG	1	-1.08	356.66	-1.08			
	TOTAL AREA (FT²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)
MAIN ROOF**	7058.61	-534.089424	7592.699424	-1.08	-0.36	-2157	-6.1
*ALONG PERIMETER	*ALONG PERIMETER TOTAL UPLIFT PER LINEAL			DUNDS)	-7.2	UPLIFT OK	
**INSIDE EXTERIOR W	**INSIDE EXTERIOR WALLS RESISTANCE DUE TO DEAD W			S	251.6		

NOTE FOR CONSTRUCTION:

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION

Combustion Air Calculation
Per 2018 IRC Section G2407.5

Appliance #1 Furnace 100000 BTU/h
Appliance #2 BTU/h
Appliance #3 Water Heater 50000 BTU/h

Total BTU/hr 150000 BTU/h

Area of Combined Space (floor where appliances are located)
Ceiling Height in Usable Space

728 ft² 8.75 ft

Note: Per 2018 IRC Section G2407.5.3.2, The volumes of spaces in different stories shall be considered as communicating spaces where such spaces are connected by one or more openings in doors or floors having a total minimum free area of 2 square inches per 1,000 BTU/h of total input rating of all appliances

Is floor where appliances are located open to adjacent level?

If Yes, what is the area of open space adjacent to appliance area?

Yes 610

OK

Per 2018 IRC Section G2407.5.1 (Standard Method), the minimum required volume shall be 50 cubic feet per 1,000 BTU/hr

(Total BTU/hr / 1,000 BTU/hr x 50 ft^3)

Required air space in combined areas: 7500 ft³

Required combined area: 857 ft²

Area of Combined Space > Required combined area?

Per Section G2407.5.3.1, each opening shall have a minimum free area of 1 square inch per 1,000 BTU/hr of the total input rating of all appliances in the space, but not less than 100 square inches. One opening shall commence within 12 inches of the top and one opening shall commence within 12 inches of the bottom of the enclosure. The minimum dimension of air openings shall be not less than 3 inches.

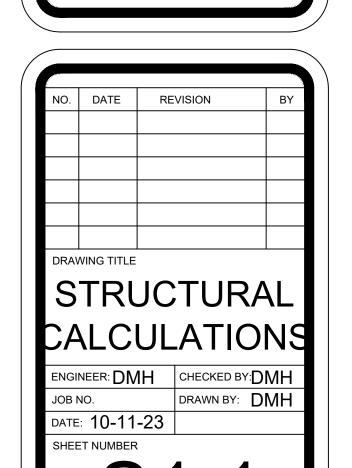
Minmum required opening area: 150 in²
Minimum grill size: 14 x 11 (in

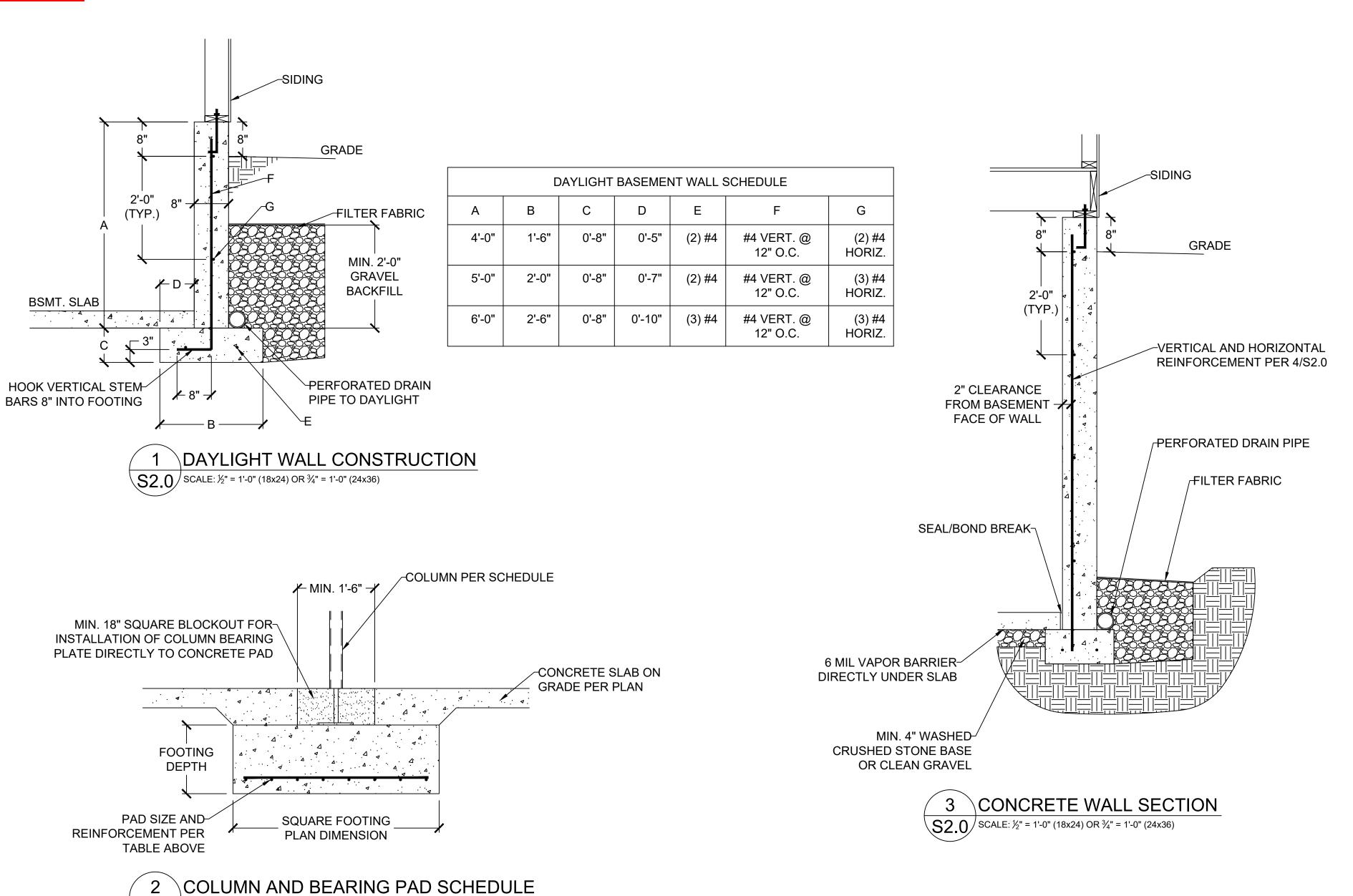
Note: two grills required - one within 12" of floor, one within 12" of clg.



RIPLEX HE TOWNHOMES OF CHAPEL R

JOB TITLE: TC LOSSINATION S19





TYPICAL CORNER REINFORCEMENT!

AT LEAST (1) #4 BAR 48" LONG @

AS CLOSE AS PRACTICAL TO THE CORNER

EACH INSIDE CORNER

NOTE: WHERE OPENINGS OR ABRUPT ELEVATION

CHANGES OCCUR IN THE TOP OR BOTTOM OF THE WALL AT LEAST ONE #4 BAR 48" LONG SHALL BE DIAGONALLY

VERTICAL REINFORCEMENT SPACING CONCRETE STRENGTH/GRADE 8" THICK WALL 10" THICK WALL REINFORCEMENT (#4 BARS) 10' 10' 3,000 PSI/ GRADE 40 24 24 16 24 24 18 3,500 PSI/ GRADE 40 24 24 24 18 3,000 PSI/ GRADE 60 24 16 24 24 18 3,500 PSI/ GRADE 60 24 24 24 18 HORIZONTAL REINFORCEMENT - MINIMUM GRADE 40 STEEL ONE BAR 12" FROM TOP OF WALL; 6-#4 4-#4 5-#4 6-#4 MAX. SPACING 24" OC

FOOTNOTES:

1) WALL HEIGHT IS MEASURED FROM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB 2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE WALL. OTHER WALLS SHALL HAVE VERTICAL REINFORCEMENT AS FOLLOWS:

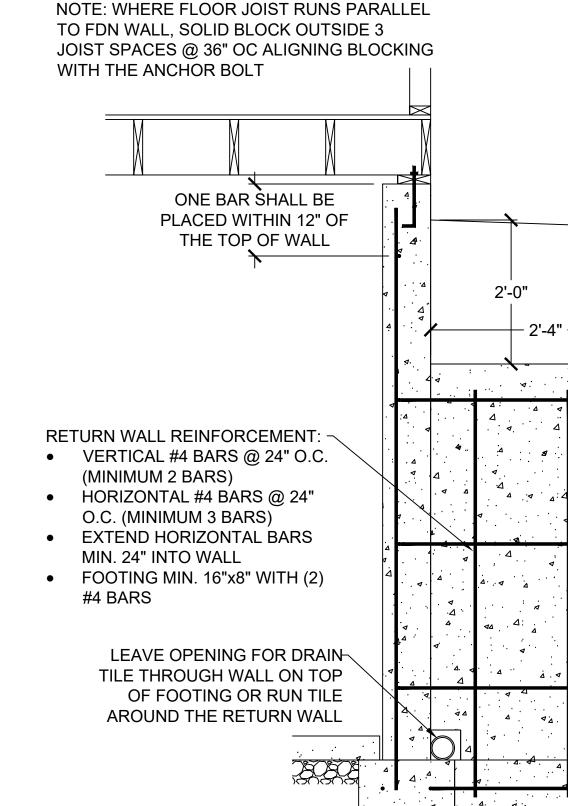
- A) 8" WALL MINIMUM 5" FROM THE OUTSIDE FACE
- B) 10" WALL MINIMUM 63/4" FROM THE OUTSIDE FACE
- C) EXTEND BARS TO WITHIN 8" OF THE TOP OF THE WALL

3) REINFORCEMENT CLEARANCES:

- A) CONCRETE EXPOSED TO EARTH MINIMUM 11/2"
- B) NOT EXPOSED TO WEATHER (INTERIOR SIDE OF WALLS) -3/4" C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY
- SLABS) 1½"
- 4) HORIZONTAL REINFORCEMENT:
- A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL
- B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE
- D) SUPPLEMENTAL REINFORCEMENT AT CORNERS PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF
- THE EDGE OF INSIDE CORNERS. 5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND
- CORNERS. 6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE 3½". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP OF THE WALL
- 7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS

4 \FOUNDATION WALL REINFORCEMENT TABLE S2.0 NO SCALE

GRADE



S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

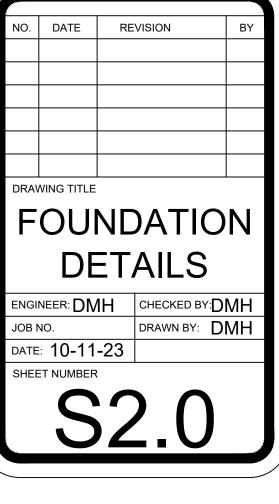
CONCRETE GRADE BEAM SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

-SLAB PER PLAN, IF APPLICABLE —PER PLAN—→ PER PLAN 3" CLEAR (TYP. REBAR PER PLAN, DRILLED AND EMBEDDED 5" INTO WALL (TYP.)

RETURN WALL DETAIL







5 SOLID JUMP S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

/MIN. (2) #4 BARS EXTENDING 24"

PAST OVER-EXCAVATION AND INTO INTERSECTING WALL

-CONTINUOUS FOOTING

AND REBAR THROUGH

6'-0" MAX.

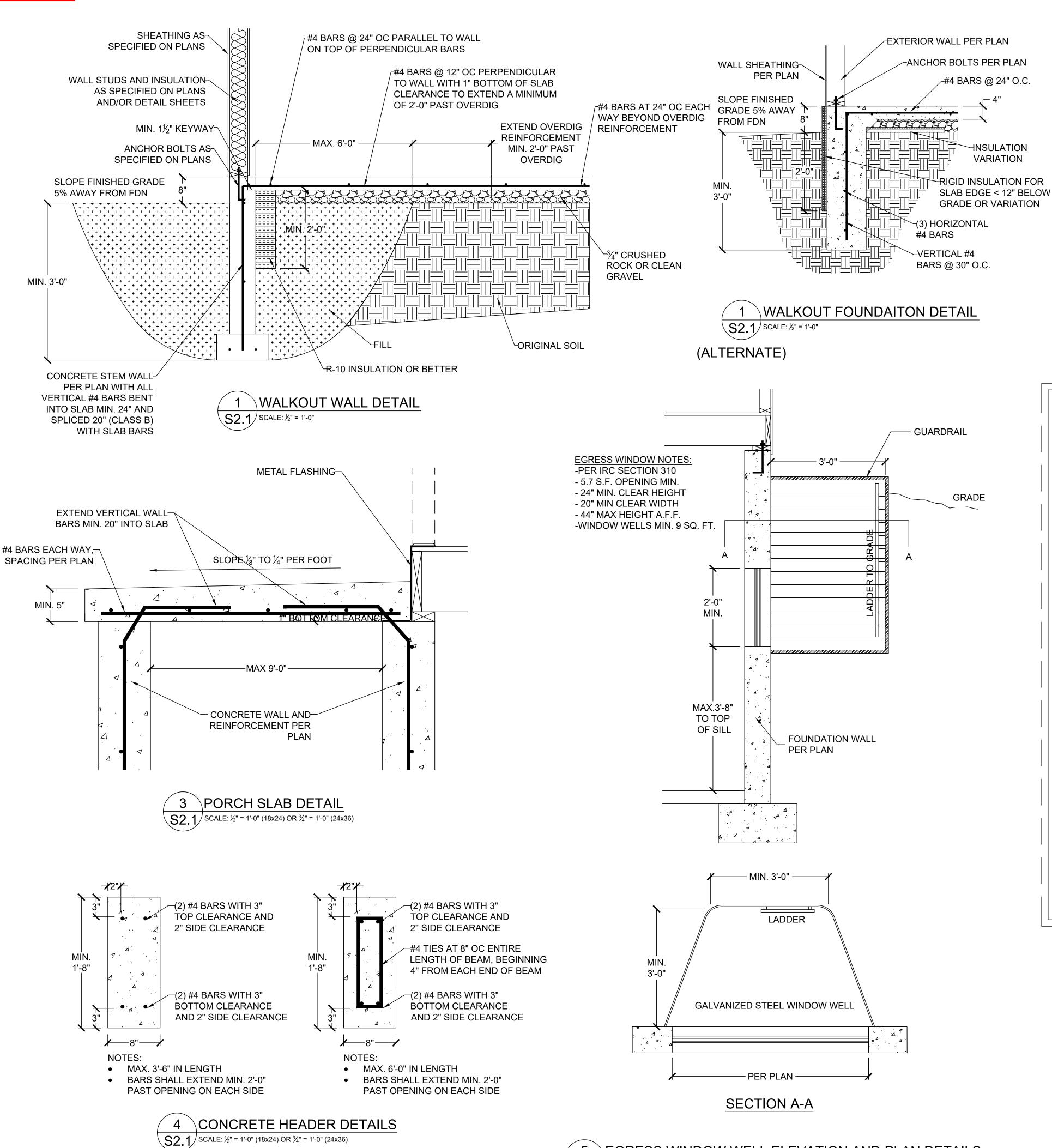
SOLID JUMP

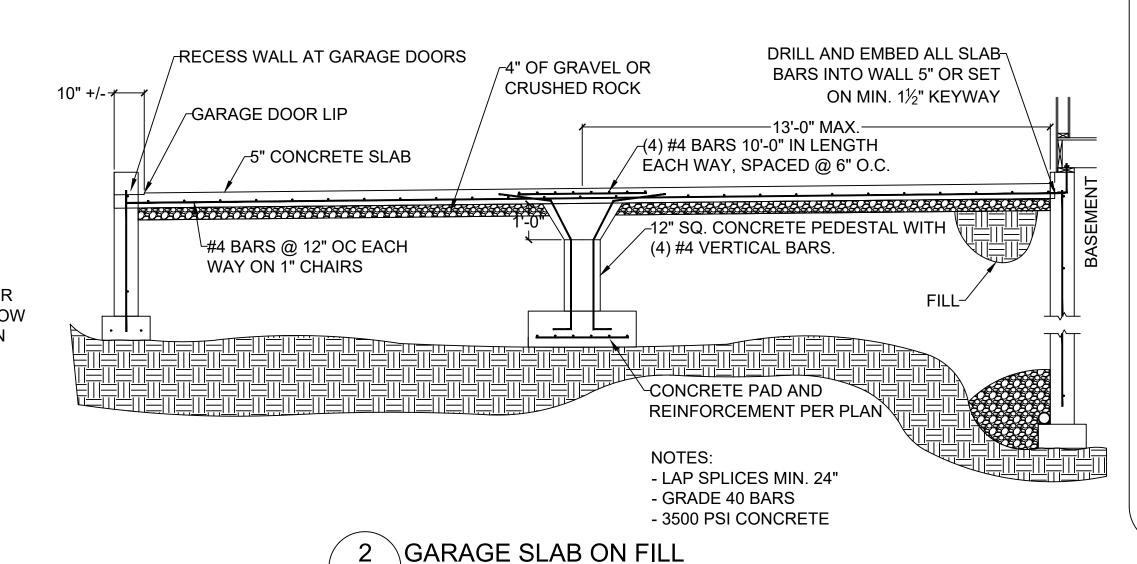
MAX. 12" BLOCKOUT FOR

FORM PLACEMENT AND

TO EXTEND DRAIN TILE

6 REINFORCEMENT AT OPENING CORNERS S2.0/AND STEP CORNERS @ INSIDE CORNERS SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)





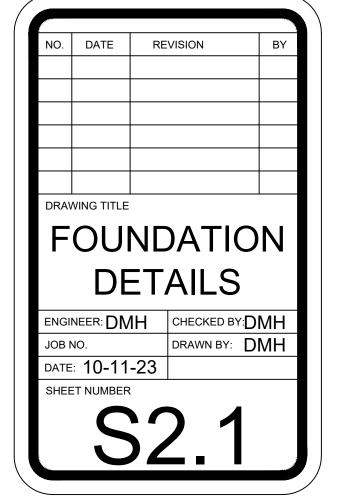
• 3500 PSI CONCRETE • 24" CLASS B LAP SPLICES SLAB REINFORCEMENT SHALL BE PLACED ON CHAIRS 5" BOTTOM REINFORCEMENT SHALL BE EMBEDDED 4" INTO WALLS GRADE 40 REINFORCEMENT PER PLAN -PER PLAN- \neg (4) #4 BARS EACH WAY WITH $\frac{3}{4}$ " TOP CLEARANCE SPACED AT 6" O.C. AND CENTERED DIRECTLY ABOVE CONCRETE PAD/PEDESTAL PER PLAN ~5" CONCRETE SLAB WITH #4 BARS AT 12" O.C. EACH WAY WITH 3/4" BOTTOM CLEARANCE

S2.1 SCALE: $\frac{1}{4}$ " = 1'-0" (18x24) OR $\frac{3}{8}$ " = 1'-0" (24x36)

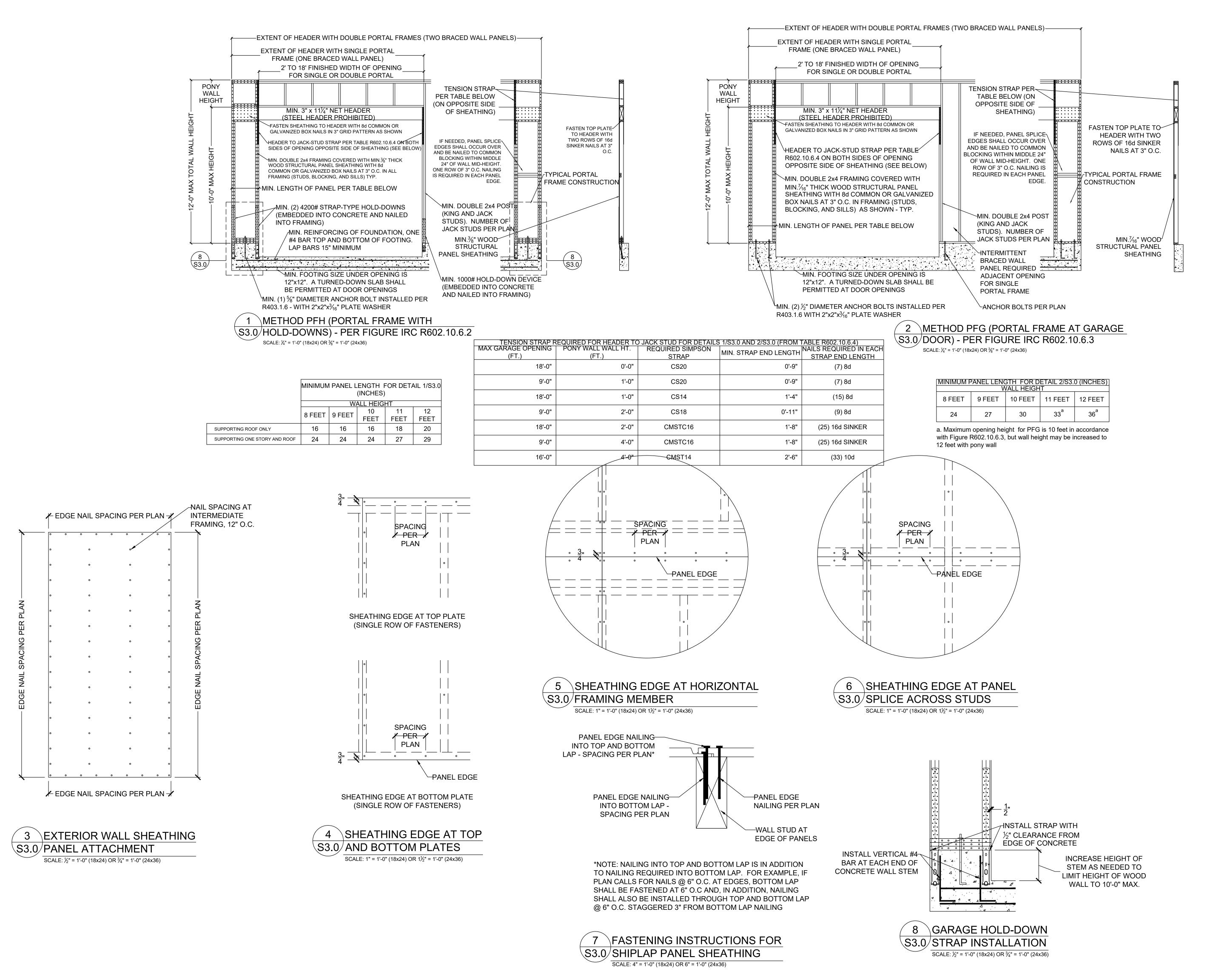


CLIENT: KEVIN HIGDON CONSTRUCTION
JOB TITLE: TCR012 TRIPLEX





5 EGRESS WINDOW WELL ELEVATION AND PLAN DETAILS SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)





IIGDOIN COINS I RUCTION 012 TRIPLEX 12, THE TOWNHOMES OF CHAPEL RIDGE

5 2

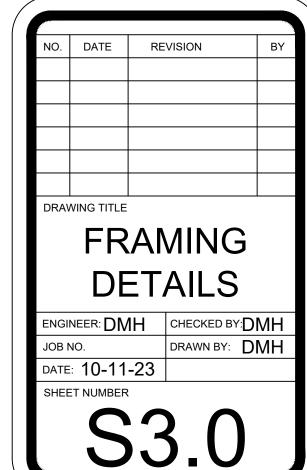
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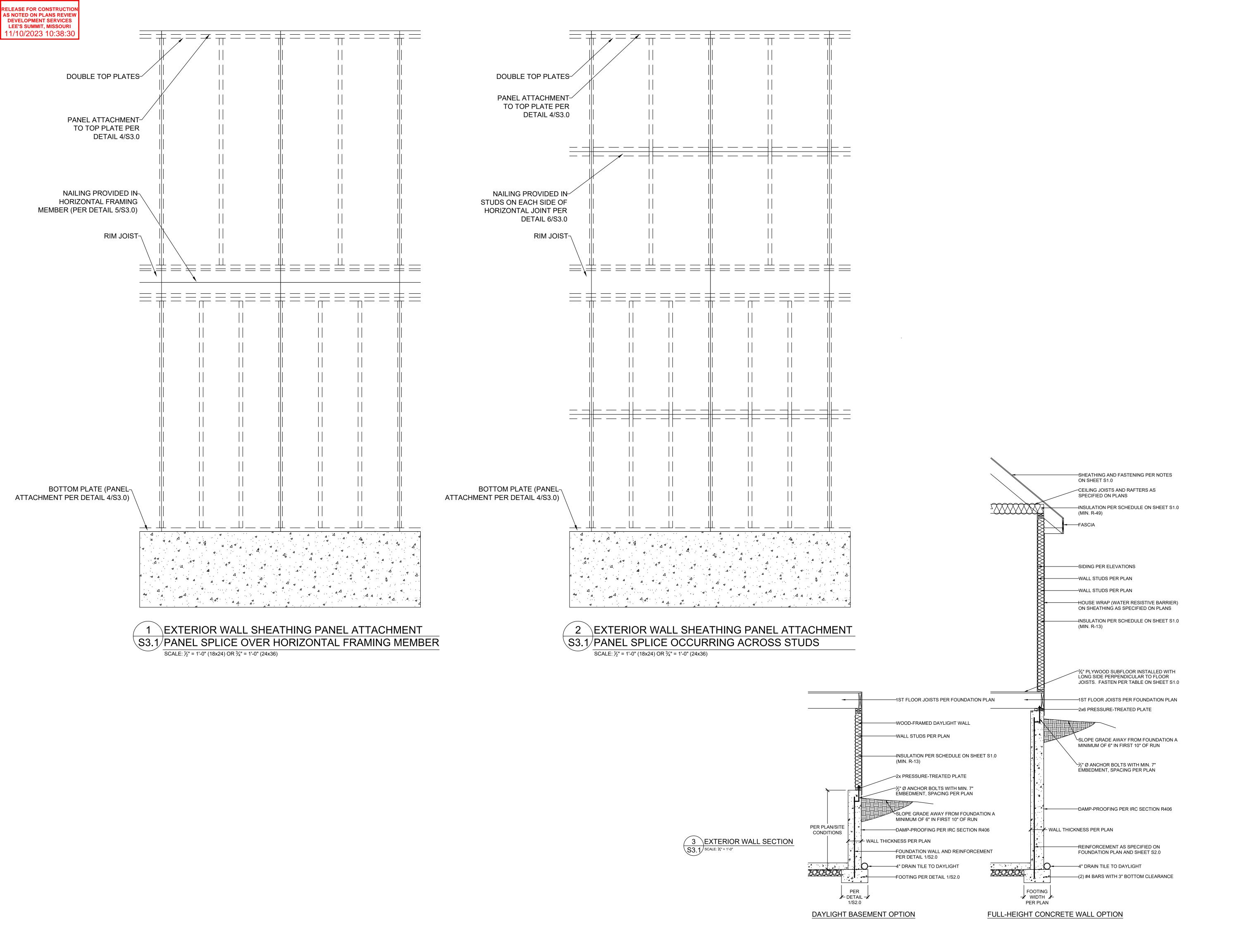
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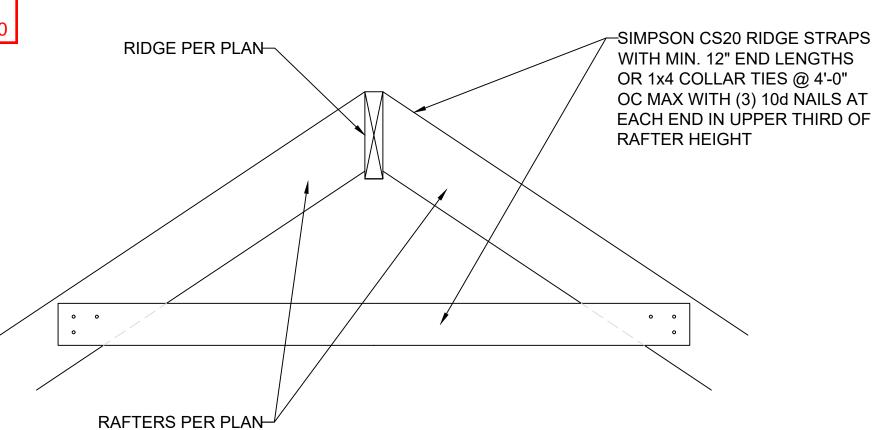


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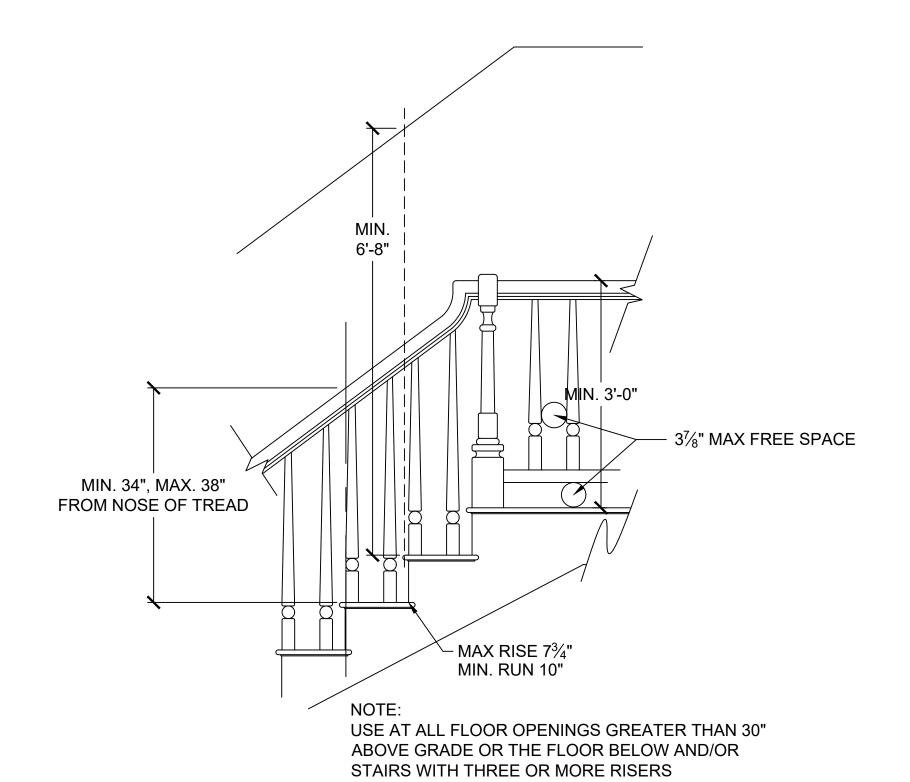
JOB TITLE: TCR012 TRIPLEX



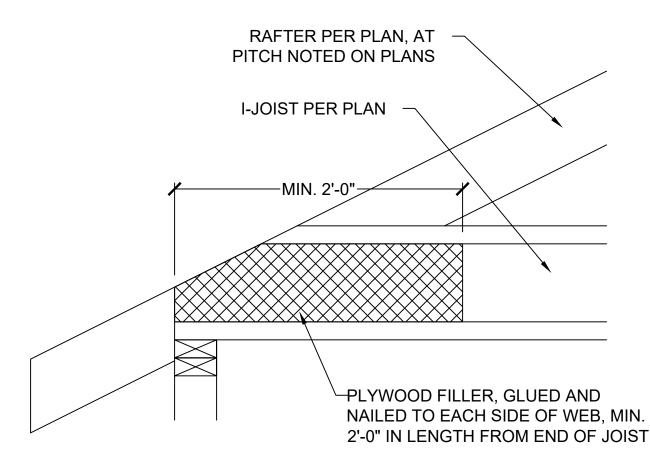
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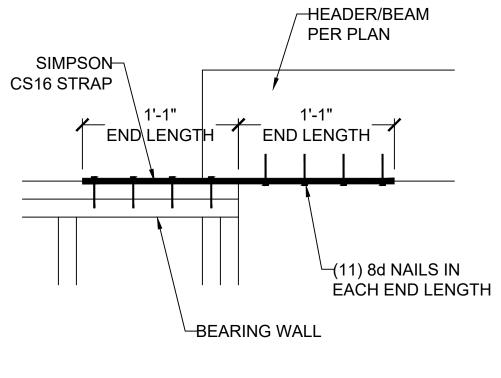
RIDGE FRAMING DETAIL S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

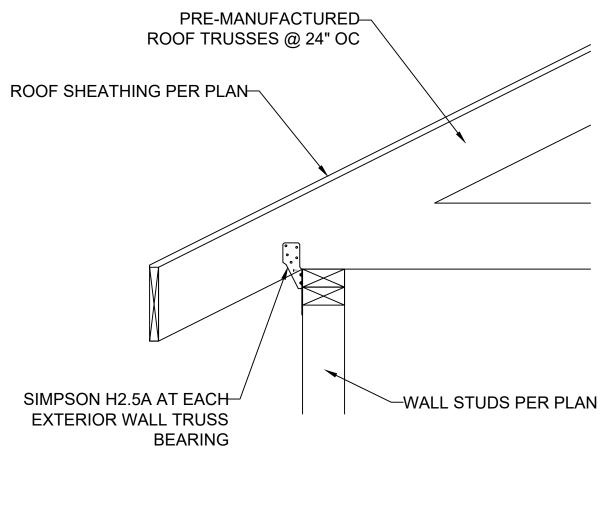


4 \STAIR AND HANDRAIL/GUARDRAIL DETAIL SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

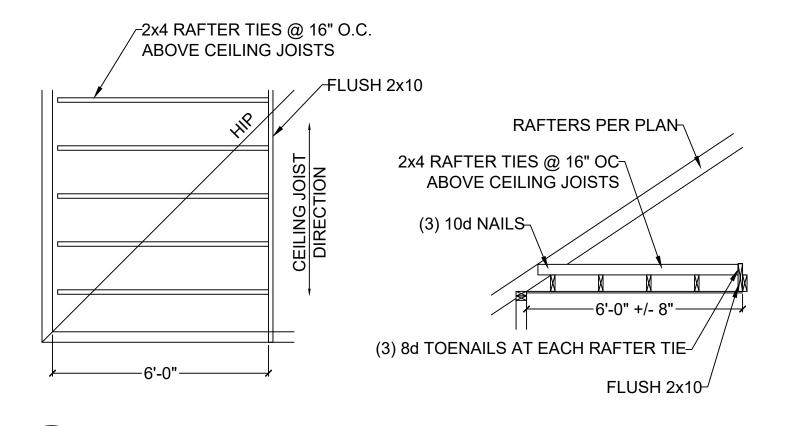


9 \COPED I-JOIST REINFORCEMENT $\sqrt{S3.2}$ SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

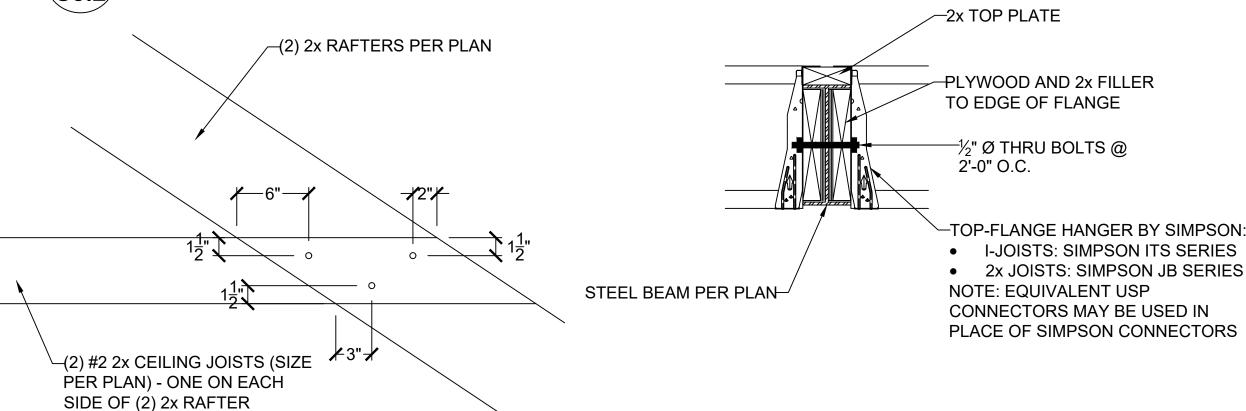




TRUSS CONNECTION TO EXT. WALL BEARING SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

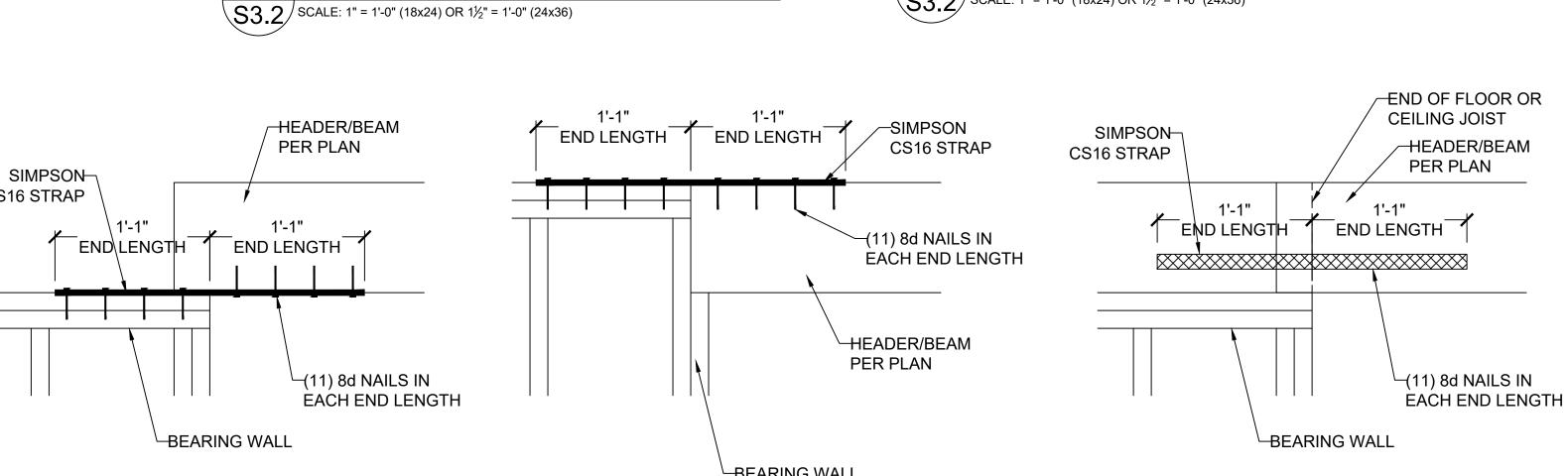


5 RAFTER TIES AT CEILING JOISTS PERP. TO RAFTERS SCALE: $\frac{1}{4}$ " = 1'-0" (18x24) OR $\frac{3}{6}$ " = 1'-0" (24x36)

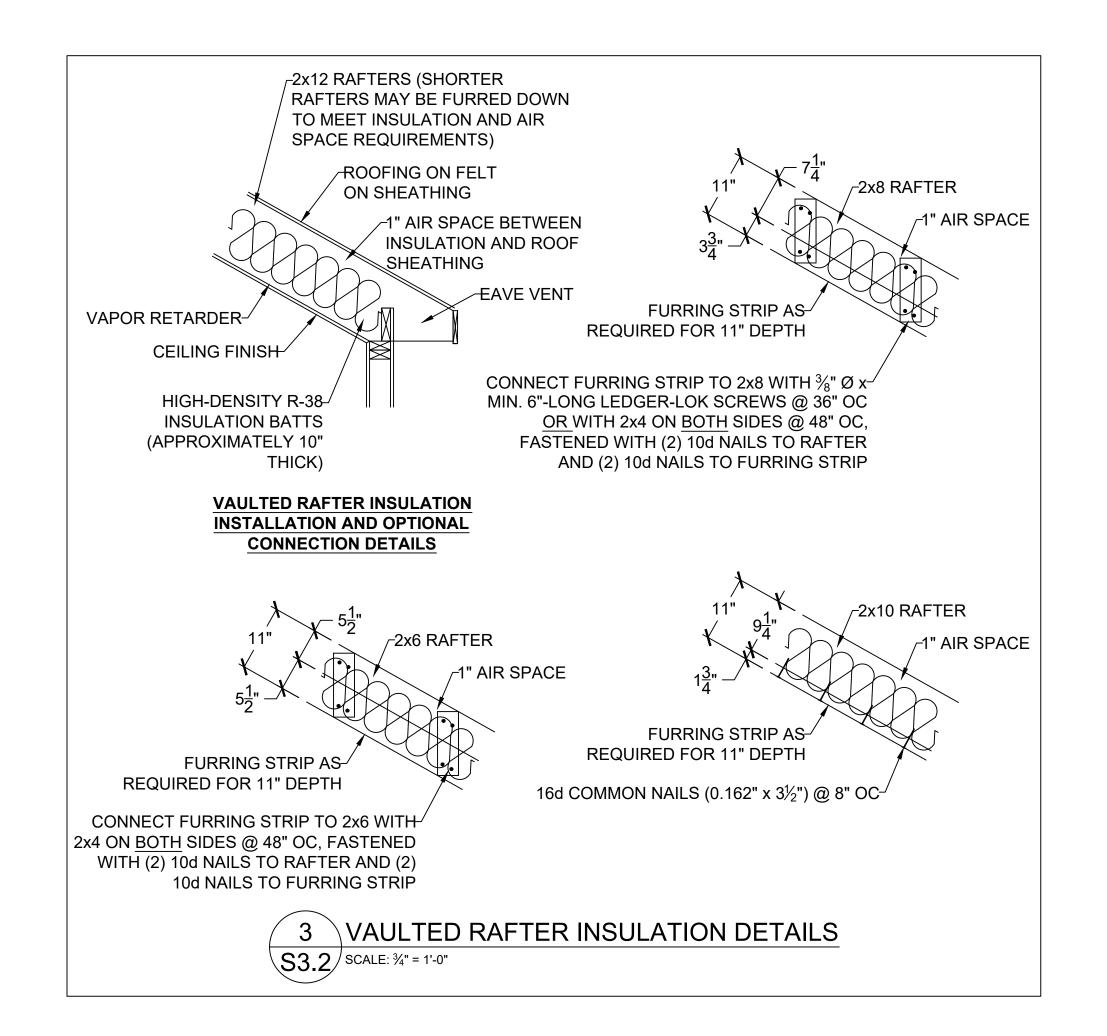


6 \FIELD-CONSTRUCTED A-FRAME DETAIL

7 \FLOOR JOIST TO FLUSH STEEL BEAM DETAIL $\frac{\text{S3.2}}{\text{SCALE: 1"}} = 1'-0" (18x24) \text{ OR } 1\frac{1}{2}" = 1'-0" (24x36)$



BEARING WALL 10 \HEADER/BEAM CONNECTION OPTIONS AT OUTDOOR/OPEN SPACE S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



HEICHT /ET \		SPACING (I	NCHES O.C	.)
HEIGHT (FT.)	24	16	12	8
	SUPPORT	ING A ROOF	ONLY	
10 OR LESS	2x4	2x4	2x4	2x4
12	2x6	2x4	2x4	2x4
14	2x6	2x6	2x6	2x4
16	2x6	2x6	2x6	2x4
18	DR	2x6	2x6	2x6
20	DR	DR	2x6	2x6
SUPP	ORTING O	NE FLOOR	AND A ROOF	F
10 OR LESS	2x6	2x4	2x4	2x4
12	2x6	2x6	2x6	2x4
14	2x6	2x6	2x6	2x6
16	DR	2x6	2x6	2x6
18	DR	2x6	2x6	2x6
20	DR	DR	2x6	2x6
SUPPO	ORTING TV	VO FLOORS	AND A ROC)F
10 OR LESS	2x6	2x6	2x4	2x4
12	2x6	2x6	2x6	2x6
14	2x6	2x6	2x6	2x6
16	DR	2x6	2x6	2x6
18	DR	DR	2x6	2x6
20	DR	DR	DR	2x6

1) DR = DESIGN REQUIRED 2) UTILITY, STANDARD, STUD AND #3 GRADE LUMBER OF ANY SPECIES ARE NOT PERMITTED 3) THIS TABLE DOES NOT APPLY FOR STUDS SUPPORTING MEMBERS WITH A TRIB. LENGTH GREATER THAN 6'-0"

8 MAXIMUM ALLOWABLE LENGTH OF S3.2/WOOD WALL STUDS (IRC TABLE 602.3.1)

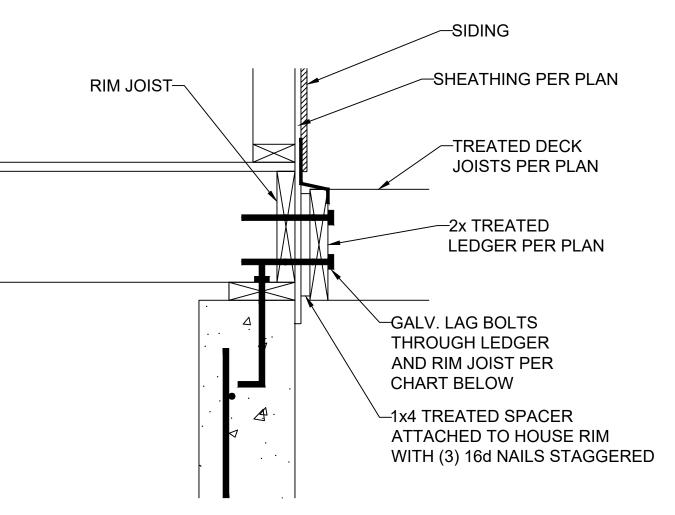


0 7 TCR LOT 2ND

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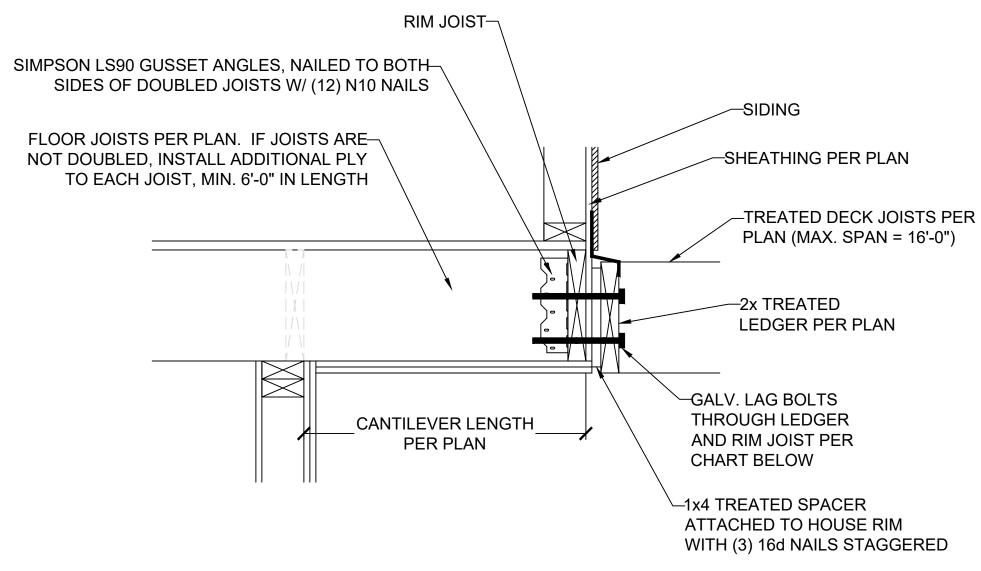
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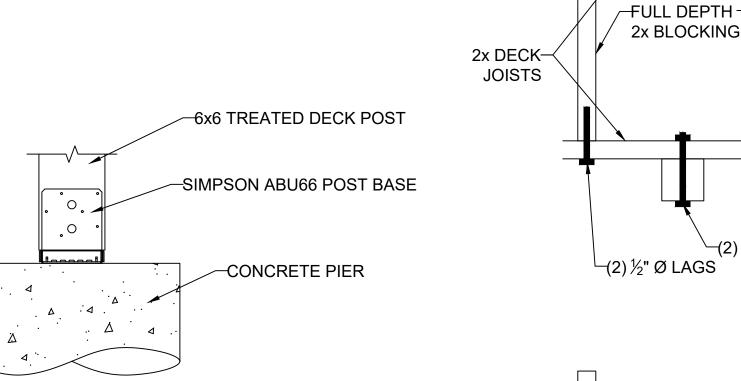
DECK LEDGER ATTACHMENT GUIDE

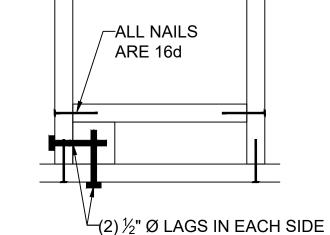
DECK JOIST SPAN	$\frac{1}{2}$ " Ø GALV. LAG OR $\frac{3}{8}$ " Ø LEDGER-LOK SPACING
10'-0" OR LESS	16" OC
10'-0" - 13'-11"	12" OC OR @ 16" OC DOUBLED EVERY OTHER
14'-0" - 18'-0"	8" OC OR @ 16" OC DOUBLED

LEDGER ATTACHMENT



2 \CANTILEVER WITH DECK ATTACHMENT

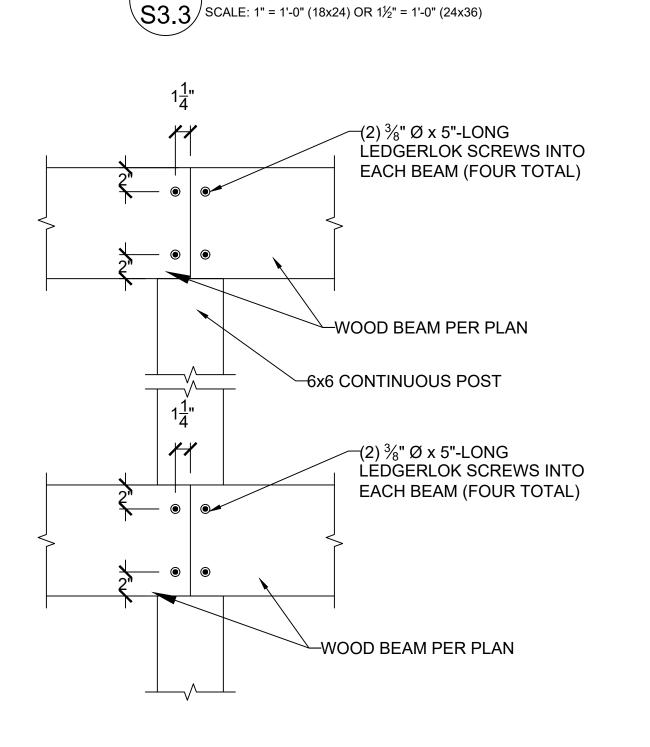


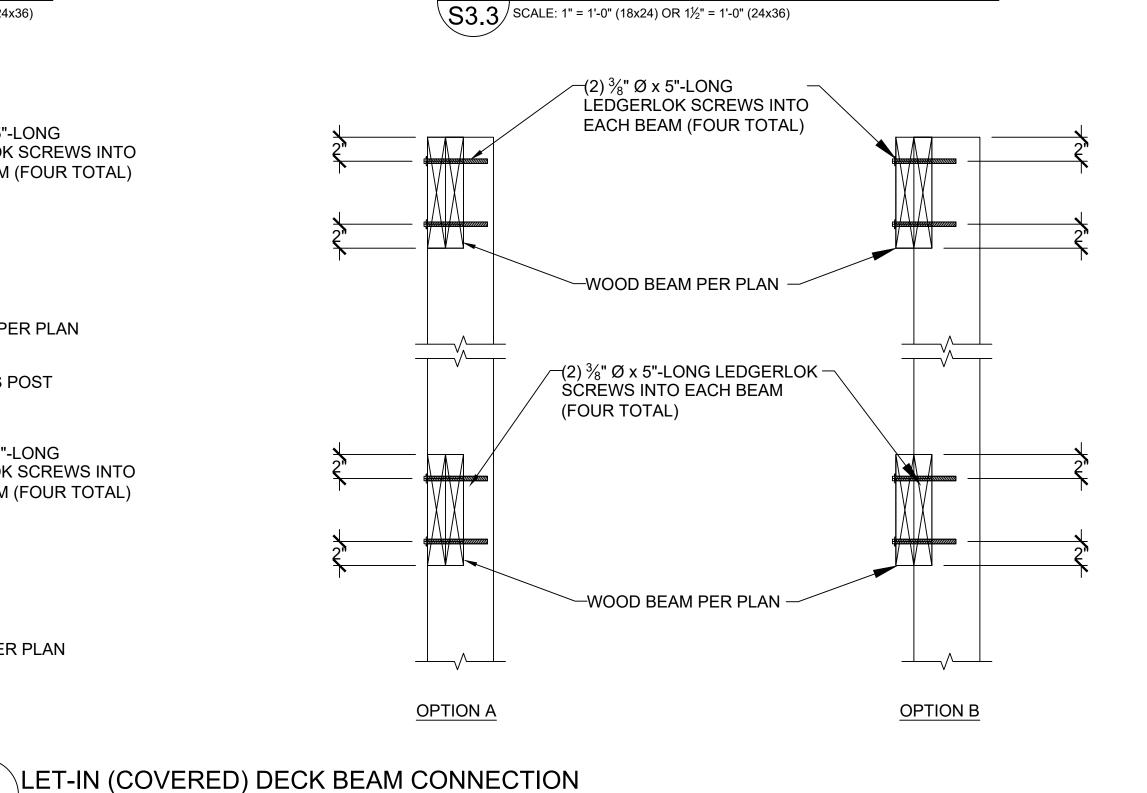


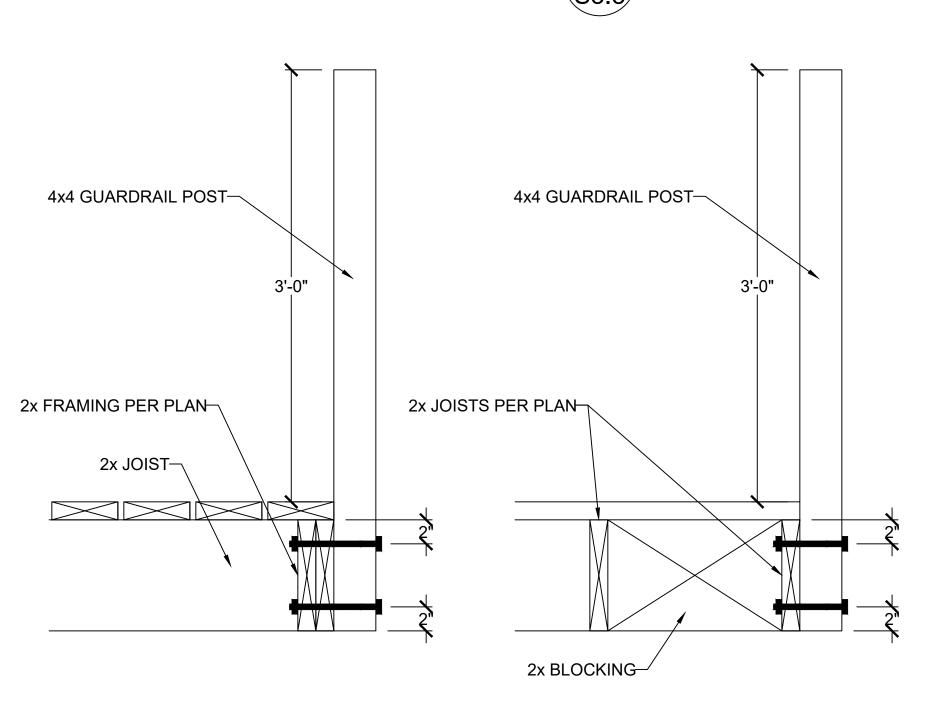
–(2) ½" Ø BOLTS

3 DECK POST BASE \$3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

REINF. POST CONNECTIONS S3.3 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



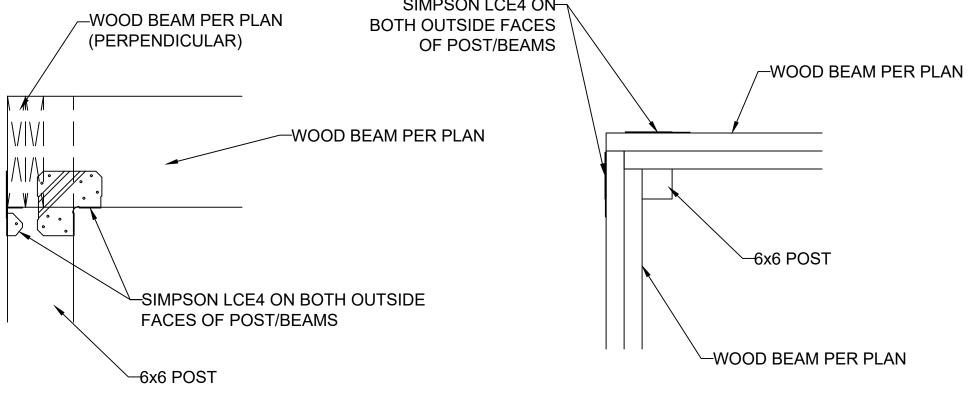




6 \GUARDRAIL CONNECTION S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

SIMPSON LCE4 ON-BOTH OUTSIDE FACES OF POST/BEAMS -WOOD BEAM PER PLAN

S3.3 | SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

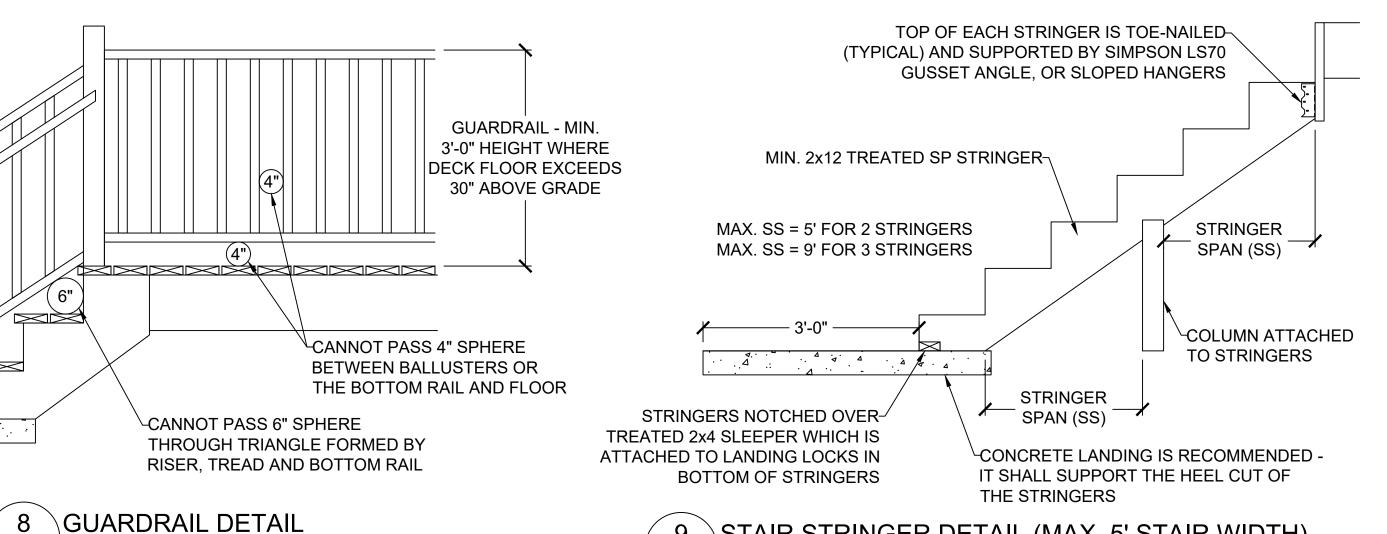




HANDRAIL - REQUIRED ON

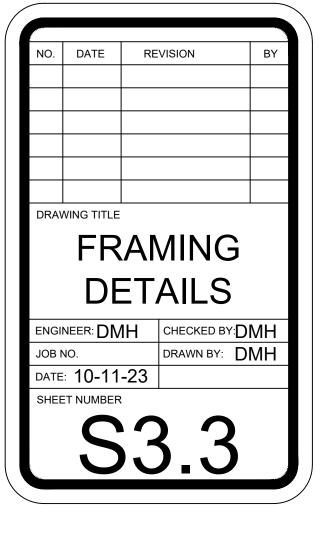
ONE SIDE OF STAIRS WITH

4 OR MORE RISERS 34" -38" ABOVE STAIR NOSING



9 \STAIR STRINGER DETAIL (MAX. 5' STAIR WIDTH) SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)





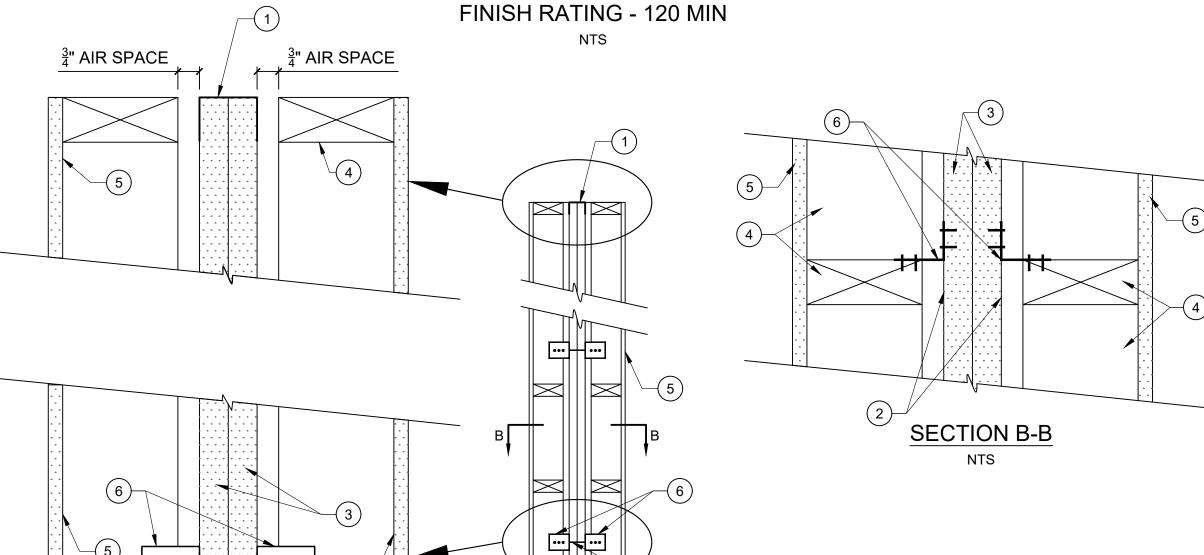
7 \ALTERNATE COVERED DECK/PORCH INTERSECTION S3.3/CORNER BEAM CONNECTION

SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



DESIGN NO. U366

NON-BEARING WALL RATING - 2HR



#	COMPONENT
1	2" WIDE CHANNEL AT FLOOR, INTERMEDIATE OR OF TOP WALL
2	2" DEEP x $1\frac{3}{8}$ " H-SHAPED STEEL STUDS @ 24" OC
3	(2) LAYERS OF 1" THICK GYPSUM BOARD LINER PANELS IN 24" WIDTHS
4	$2x4$ WOOD STUDS @24" OC MAX, MIN $\frac{3}{4}$ " SEPARATION BETWEEN WOOD FRAMING & AREA SEPARATION WALL
5	MIN ½" THICK x 4' WIDE GYPSUM BOARD APPLIED HORIZONTAL OR VERTICAL
6	ALUMINUM ANGLE ATTACHMENT CLIPS- MIN 2" WIDE WITH MIN 2" AND $2\frac{1}{2}$ " LEGS

AREA SEPARATION WALL: (MAX HEIGHT - 44 FT)

-(4)

CONFIGURATION B

EXPOSED TO FIRE FROM EITHER SIDE

NTS

- 1. FLOOR, INTERMEDIATE OR TOP OF WALL 2 IN. WIDE CHANNEL SHAPED WITH 1-IN LONG LEGS FORMED FROM NO. 25 MSG GALV STEEL, SECURED WITH SUITABLE FASTENERS SPACED @ 24 IN OC
- 2. STEEL STUDS STEEL MEMBERS FORMED FROM NO. 25 MSG GALV STEEL HAVING "H" SHAPED FLANGE SPACED @ 24 IN OC; OVERALL DEPTH 2 IN AND FLANGE WIDTH 1-3/8 IN.
- 3. GYPSUM BOARD* 2 LAYERS OF 1 IN THICK GYPSUM WALLBOARD LINER PANELS, SUPPLIED IN NOM 24 IN WIDTHS. VERTICAL EDGES OF PANELS FRICTION FITTED INTO "H" SHAPED STUDS.

 (JAMES HARDIE GYPSUM INC-TYPE HARDILINER)

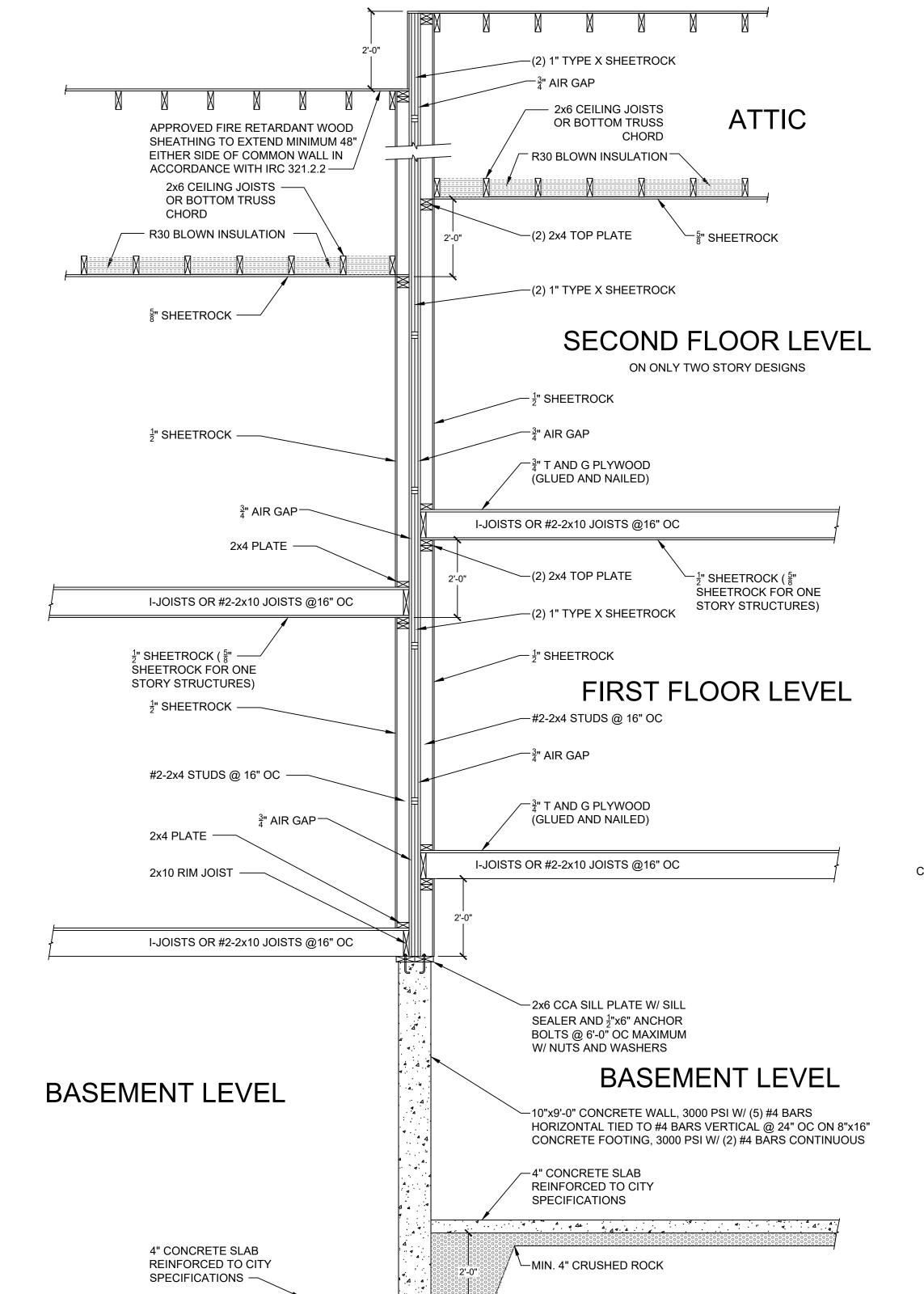
PROTECTED WALL: (BEARING OR NON-BEARING WALL)

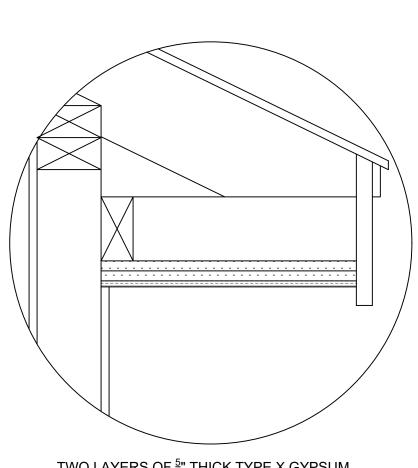
- 4. WOOD STUDS NOM 2 BY 4 IN. MAX SPACING @ 24 IN. OC. STUDS CROSS-BRACED AT MIDHEIGHT WHERE NECESSARY FOR CLIP ATTACHMENT. MIN. $\frac{3}{4}$ " SEPARATION BETWEEN WOOD FRAMING AND AREA SEPARATION WALL.
- 5. GYPSUM BOARD CLASSIFIED OR UNCLASSIFIED MIN. $\frac{1}{2}$ IN. THICK, 4FT WIDE, APPLIED EITHER HORIZONTALLY OR VERTICALLY. WALLBOARD ATTACHED TO STUDS WITH $1\frac{1}{4}$ IN. LONG STEEL DRYWALL NAILS SPACED @ 8 IN. OC. VERTICAL JOINTS LOCATED OVER STUDS. (OPTIONAL) JOINTS COVERED WITH PAPER TAPE AND JOINT COMPOUND. NAIL HEADS COVERED WITH JOINT COMPOUND.
- 6. ATTACHMENT CLIPS ALUMINUM ANGLE, 0.063 IN. THICK, MIN 2 IN. WIDE WITH MIN 2 IN. AND $2\frac{1}{4}$ IN. LEGS. CLIPS SECURED WITH TYPE S SCREWS $\frac{3}{8}$ IN. LONG TO "H" STUDS AND WITH TYPE W SCREWS $1\frac{1}{4}$ IN. LONG TO WOOD FRAMING THROUGH HOLES PROVIDED IN CLIP. CLIPS SPACED A MAX OF 10 FT OC VERTICALLY BETWEEN WOOD FRAMING AND "H" STUDS FOR SEPARATION WALLS UP TO 23 FT HIGH. FOR SEPARATION WALLS UP TO 44FT HIGH, CLIPS SPACED AS DESCRIBED ABOVE FOR THE UPPER 24 FT AND THE REMAINING WALL AREA BELOW REQUIRES CLIPS A MAX 5 FT OC VERTICALLY BETWEEN WOOD FRAMING AND "H" STUDS.

*BEARING THE UL CLASSIFICATION MARK

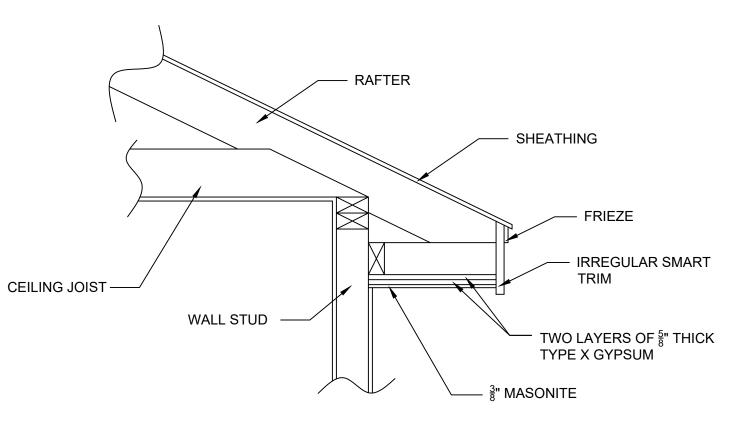
SEPARATION WALL AND AJOINING WALL NOTES:

- TWO HOUR FIRE WALL PER UL DESIGN # U366 SHOWN IN THE UL FIRE RESISTANCE DIRECTORY.
- B. INSULATE STUD CAVITIES WITH 3½" BATT INSULATION
- C. PLUMBING OR ELECTRICAL ALLOWED IN AJOINING WALLS
- D. A SEPARATE FIRE SEPARATION WALL INSPECTION WILL BE REQUIRED
- E. ANY SHAFT WALL PENETRATIONS IN EXCESS OF $\frac{1}{8}$ " BUT LESS THAN $\frac{3}{8}$ " TO BE FILLED WITH APPROVED FIRE CAULK OR FIRE FOAM. PENETRATIONS IN EXCESS OF $\frac{1}{2}$ " TO BE FIRE PROOFED WITH OVERLAPPING
 - LAYER OF 5" TYPE X SHEET ROCK, PROPERLY NAILED AND GLUED. SEAL ADDITIONAL DRYWALL PATCH COMPLETELY WITH FIRE CAULK
 - ATTIC FIRE SEPARATION WALL: (1)-2 HOUR SHAFT WALL FIRE TEST U366





TWO LAYERS OF $\frac{5}{8}$ " THICK TYPE X GYPSUM BOARD COVERED BY $\frac{3}{8}$ " MASONITE APPLIED AT RIGHT ANGLES W/ 3" LONG TYPE W SCREWS @ 8" OC



1 HOUR SOFFIT DETAIL



EVIN HIGDON CONSTRUCTION

TCR012 TRIPLEX

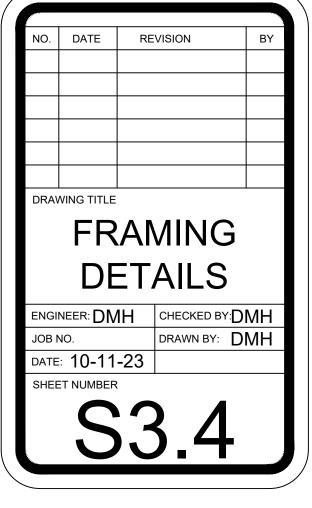
LOT 12, THE TOWNHOMES OF CHAPEL RIDG

2ND PLAT

1: 819, 817, 815 NE ALGONQUIN ST.

LEE'S SUMMIT, MISSOURI





FIRE WALL SECTION

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