



FRONT ELEVATION
 SCALE 1/4"=1'-0"



REAR ELEVATION
 SCALE 1/4"=1'-0"

08/11/2022 PLAN UPDATE

STATE OF MISSOURI
 PROFESSIONAL ENGINEER
 CHRIS JACKSON
 LICENSE NO. 2008001865
 EXPIRES 12/31/2025

STRUCTURAL REVIEW
 H.D.F. 42639

HD ENGINEERING & DESIGN, INC.
 1433 W. 171st STREET
 OVERLAND PARK, KS 66209
 WWW.HDENGINEERINGDESIGN.COM
 (816) 234-2333

08/14/2022 CITY COMMENTS

Square Footage	Finished
First Floor	5763
Basement	6724
Total	12487

CDG
 Castrop Design Group
 Christopher C. Castrop
 Architectural Design and Consulting
 4338 West 16th Street
 Overland Park, Kansas 66205
 913.515.7664
 castrop-designgroup@live.com

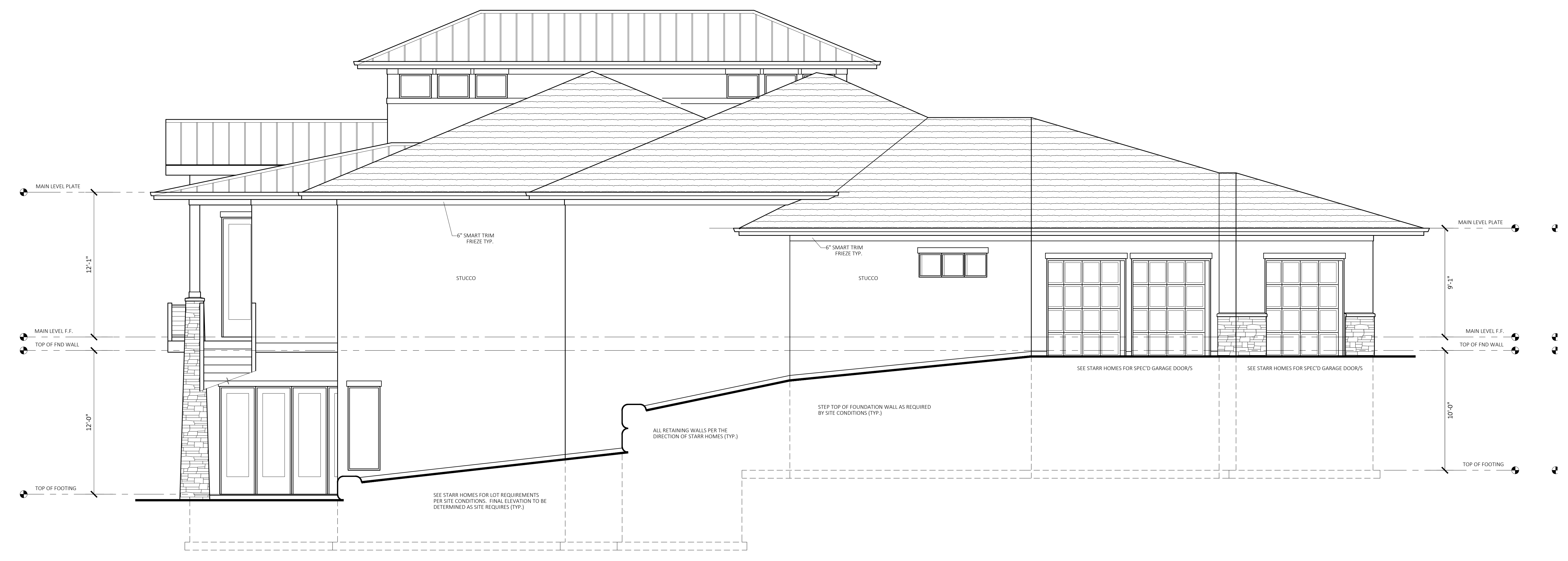
No.	Description	Date

CONSTRUCTION DOCUMENTS

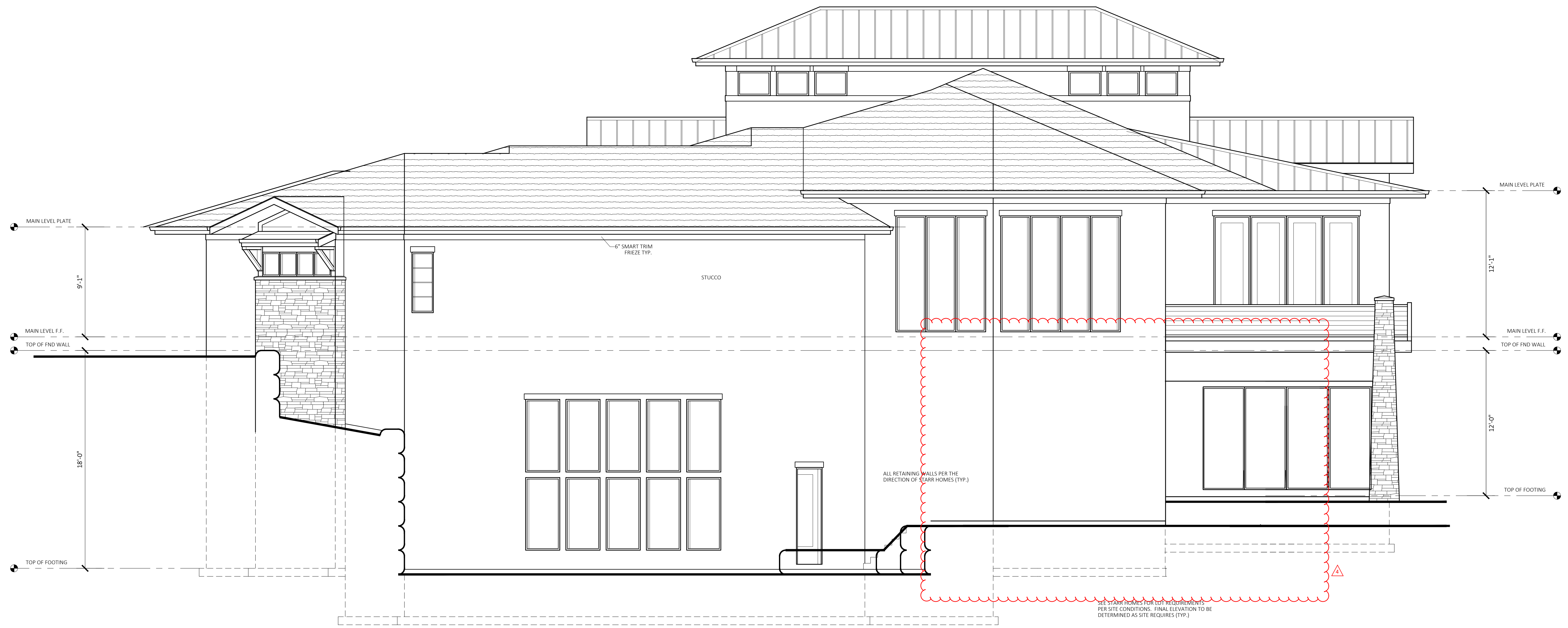
Project Number: Milligan
 Date: 2022 AUG 05
 Drawn By: MGS
 Checked By: CDG

A 101
 Scale: 1/4" = 1'-0"

RELEASE FOR CONSTRUCTION
 AS NOTED FOR PLAN REVIEW
 DEVELOPMENT SERVICES
 LOT 3 AND 4A
 08/24/2023

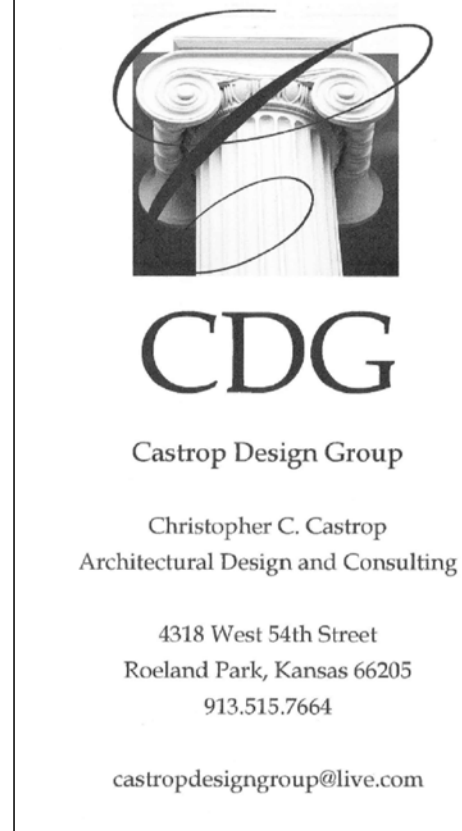


LEFT ELEVATION
 SCALE 1/4"=1'-0"



RIGHT ELEVATION
 SCALE 1/4"=1'-0"

The Tyler and Erin Milligan Residence
 Lot 3A and 4A NE Promised View Dr
 Lee's Summit, Missouri



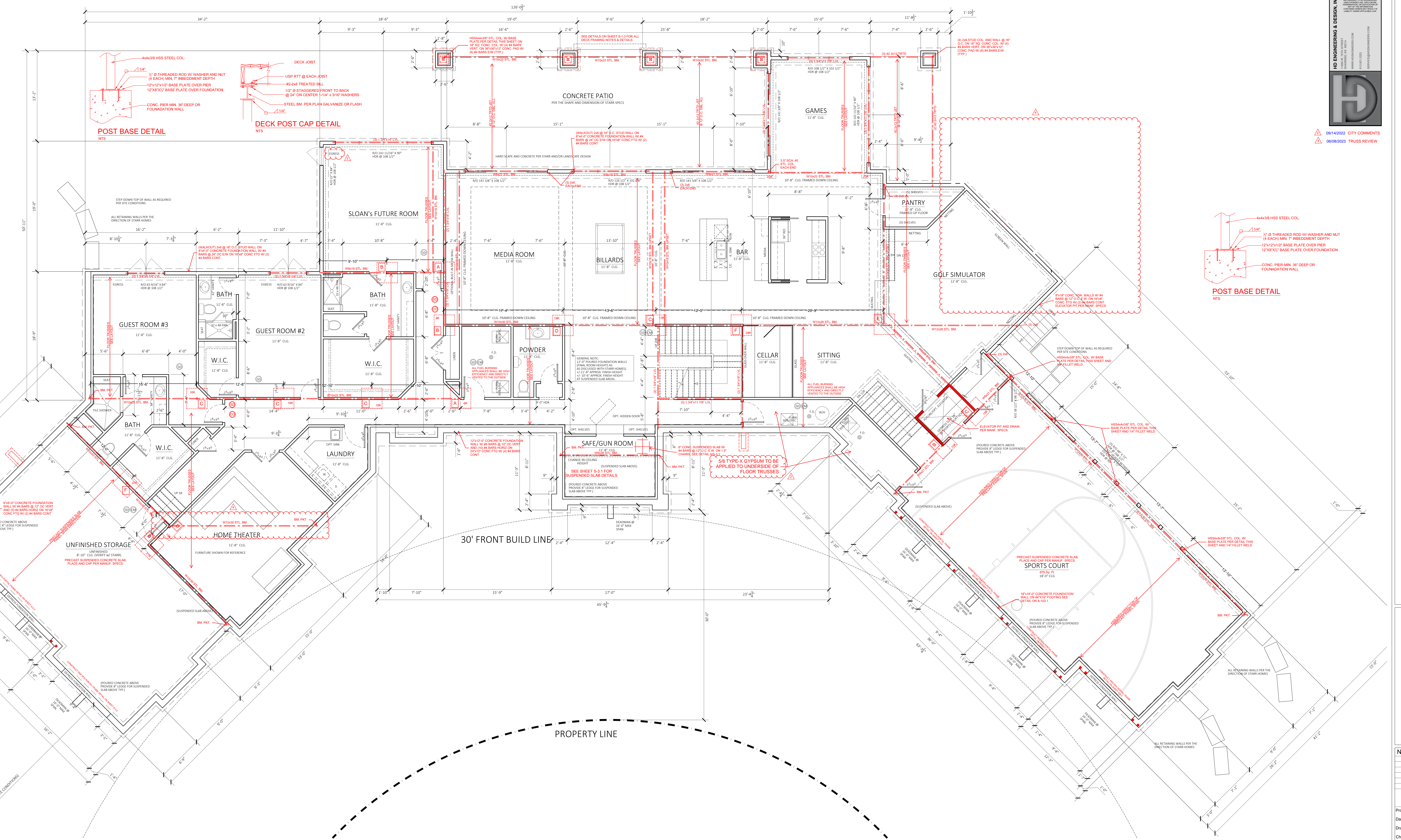
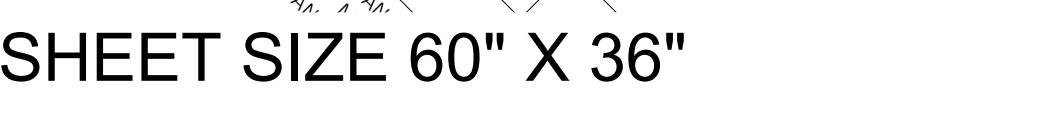
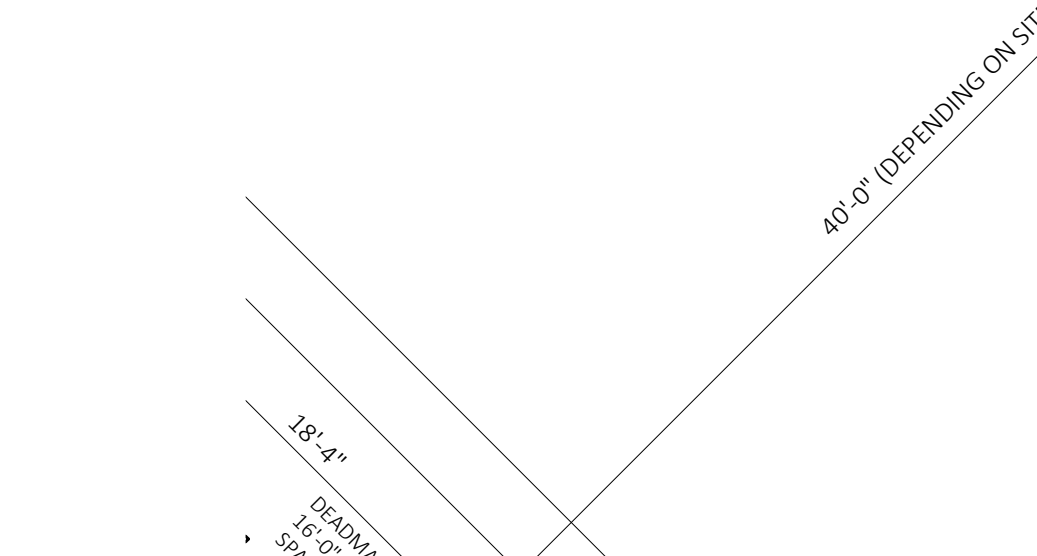
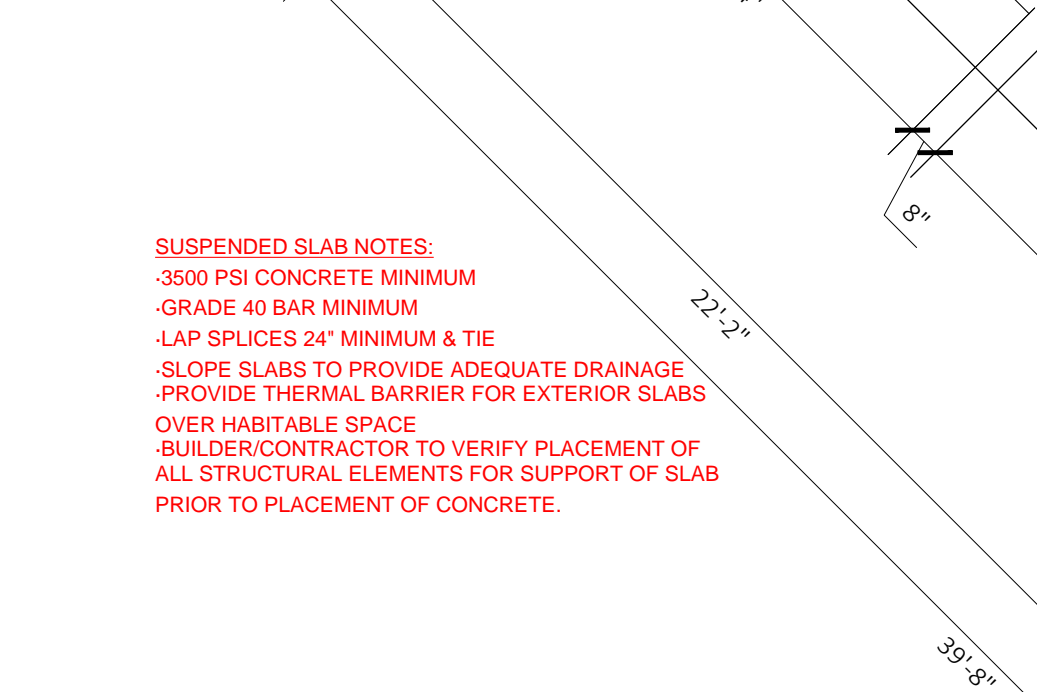
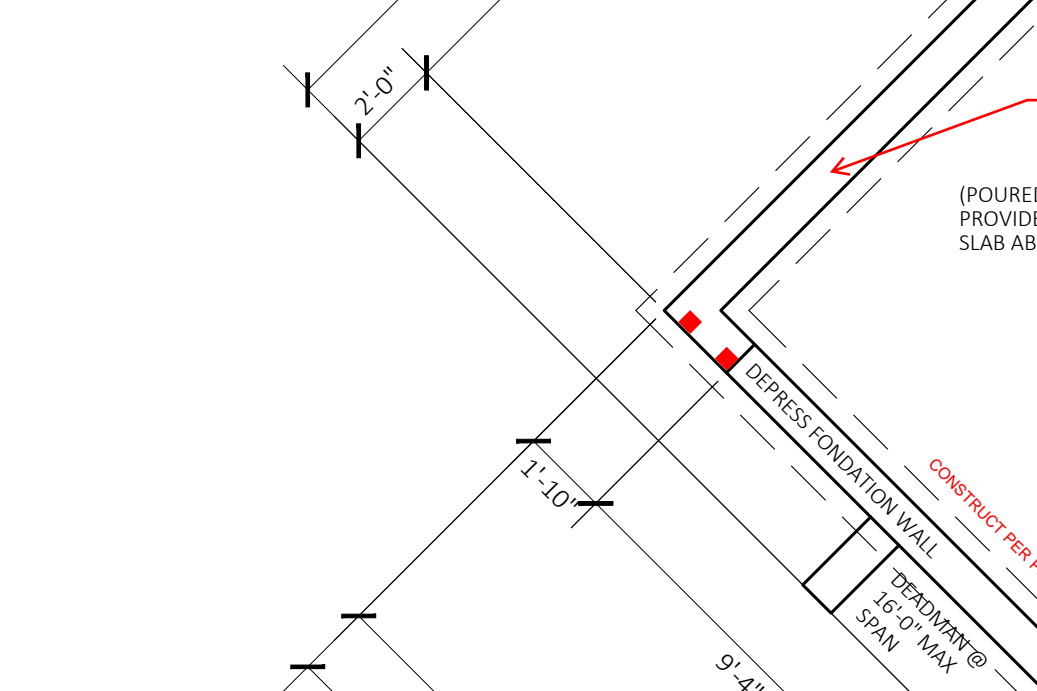
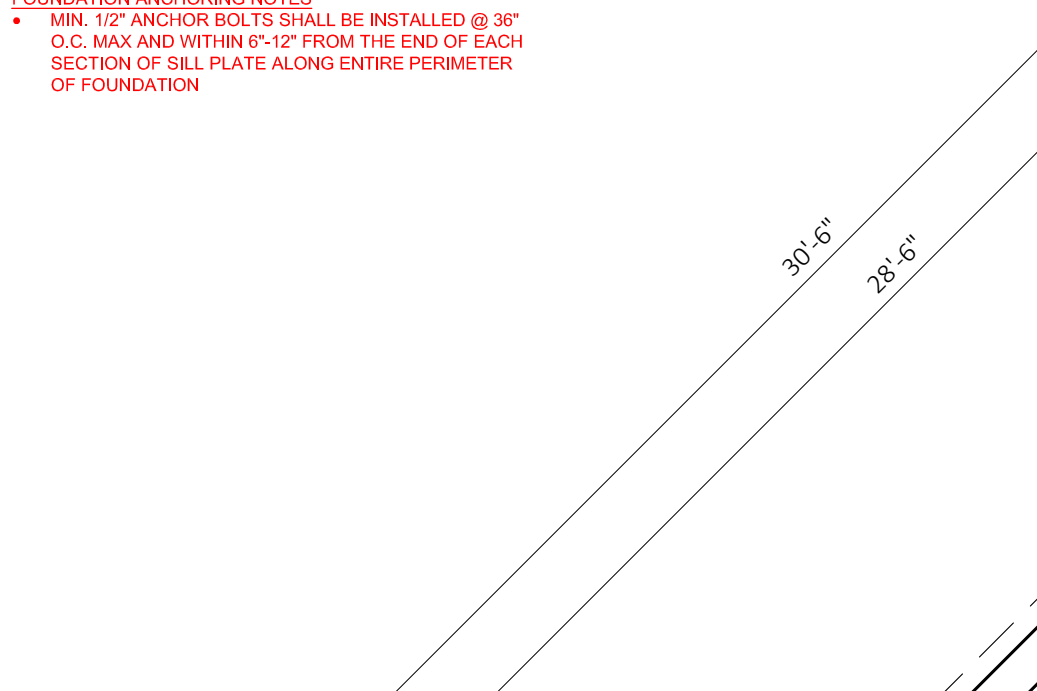
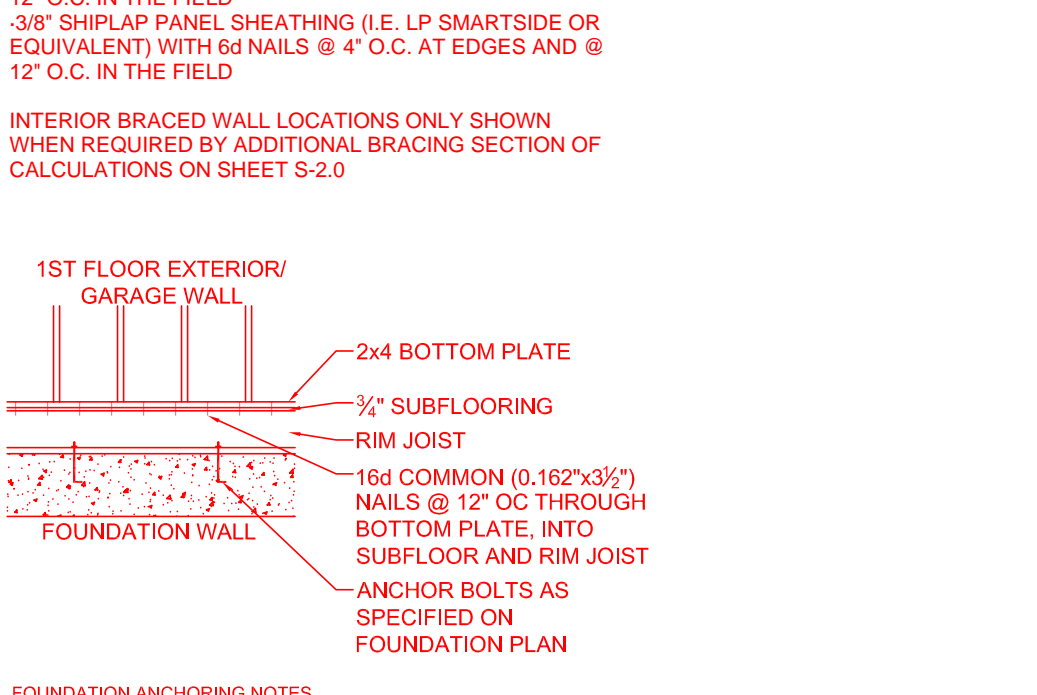
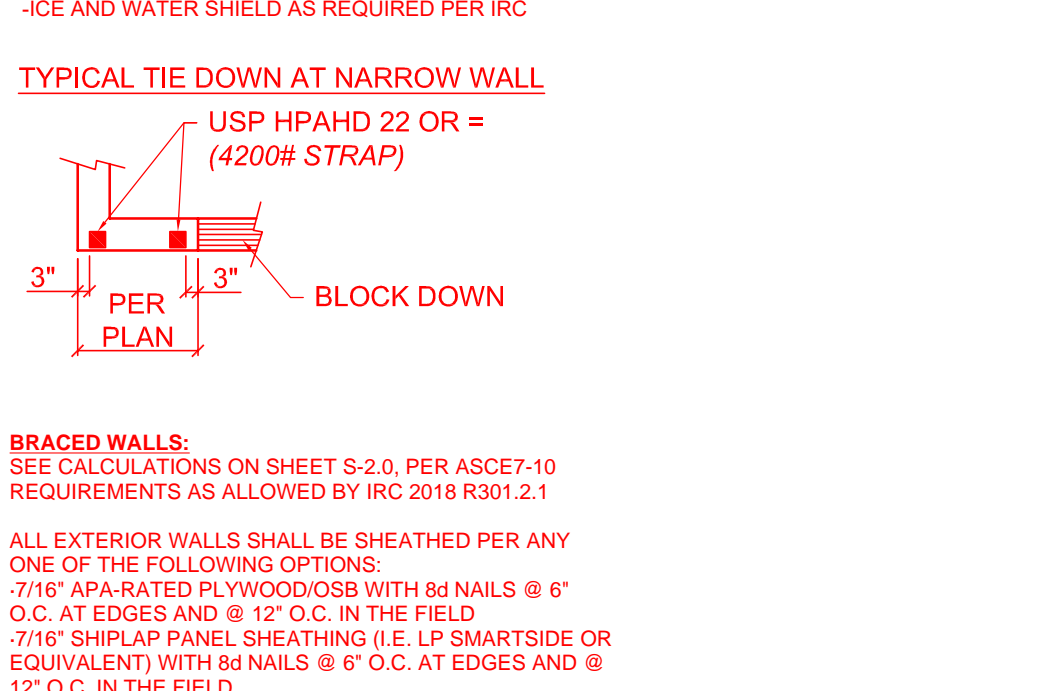
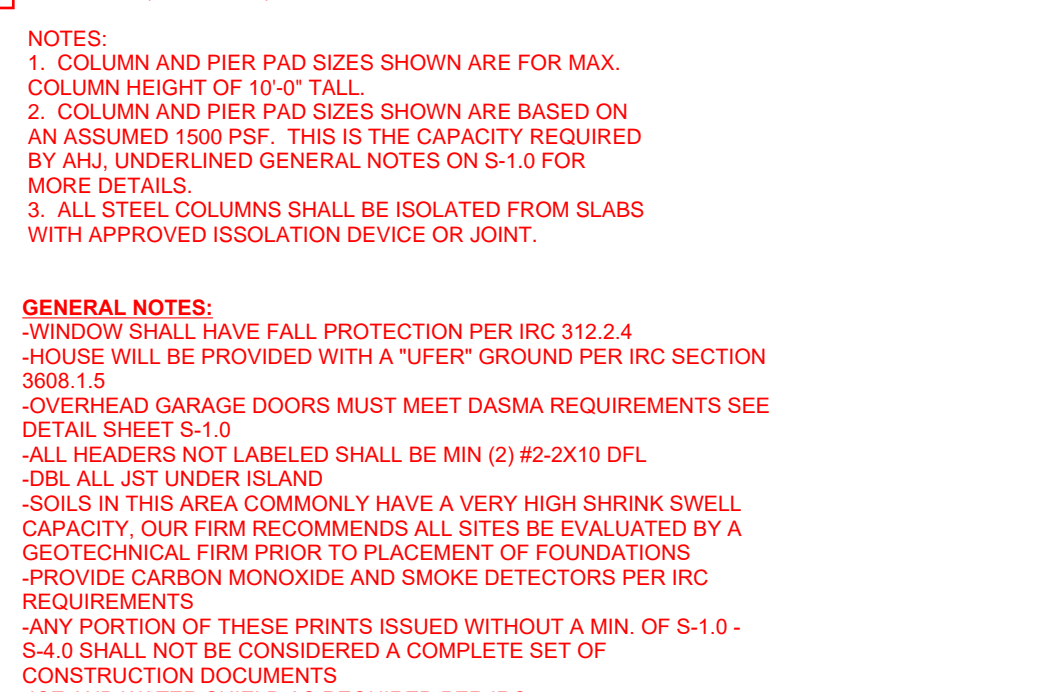
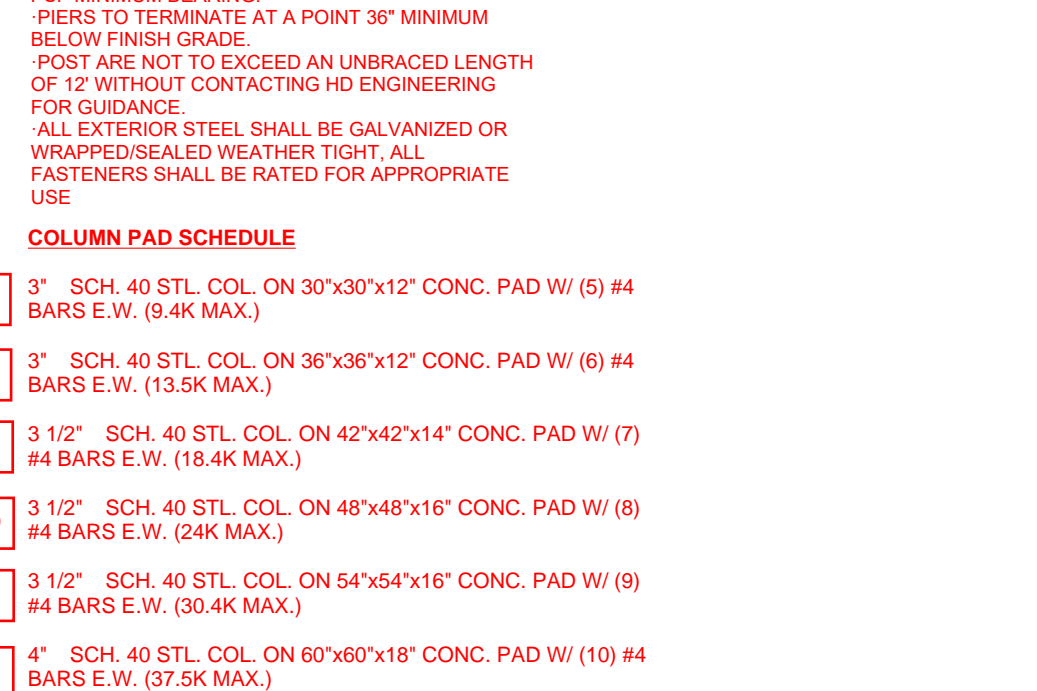
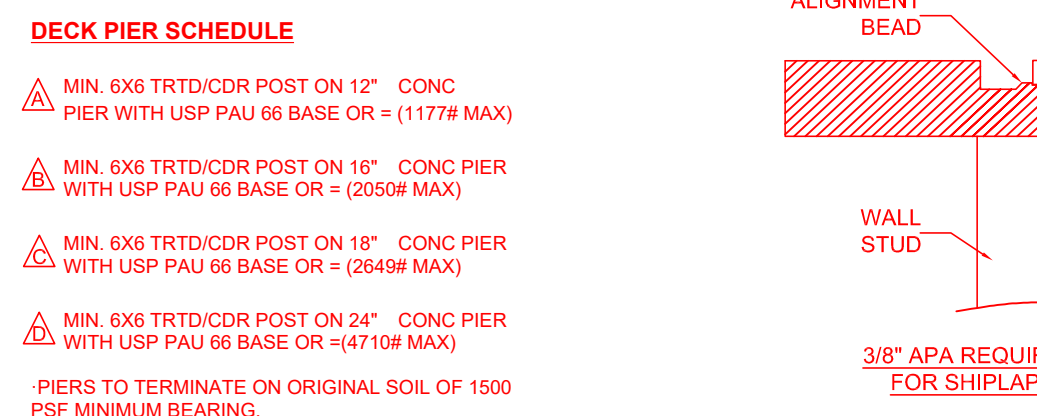
No.	Description	Date

CONSTRUCTION DOCUMENTS
 Project Number: Milligan
 Date: 2022 AUG 05
 Drawn By: MGS
 Checked By: CDG
A 102
 Scale: 1/4"=1'-0"

06/08/2023 TRUSS REVIEW
 08/11/2022 PLAN UPDATE

STRUCTURAL REVIEW
 HCH 42023

HD ENGINEERING & DESIGN, INC.
 10300 W. 171st STREET
 BLUE VALLEY BUSINESS PARK
 OVERLAND PARK, KS 66203
 913.223.2222
 HDENGINEERINGDESIGN.COM



08/11/2022 PLAN UPDATE

08/14/2022 CITY COMMENTS

06/08/2023 TRUSS REVISED

STATE OF MISSOURI
Professional Engineer
CHRIS ZACHARY
000001849
00112002

STRUCTURAL REVIEW
2024 42639

HD ENGINEERING & DESIGN, INC.
1100 W. 17TH STREET
OVERTON, MO 64653
WWW.HDENGINEERING.COM
913.831.2222

STARR HOMES LLC
PO BOX 22866
Overland Park, KS 66283

7229 W. 161st St.
Shilwell, KS 66483
Blue Valley Business Park

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The Tyler and Erin Milligan Residence
Lot 3A and 4A NE Promised View Dr
Lee's Summit, Missouri

CDG
Castrop Design Group
Christopher C. Castrop
Architectural Design and Consulting
4338 West 26th Street
Overland Park, Kansas 66205
913.515.7664
castropdesigngroup@gmail.com

No.	Description	Date

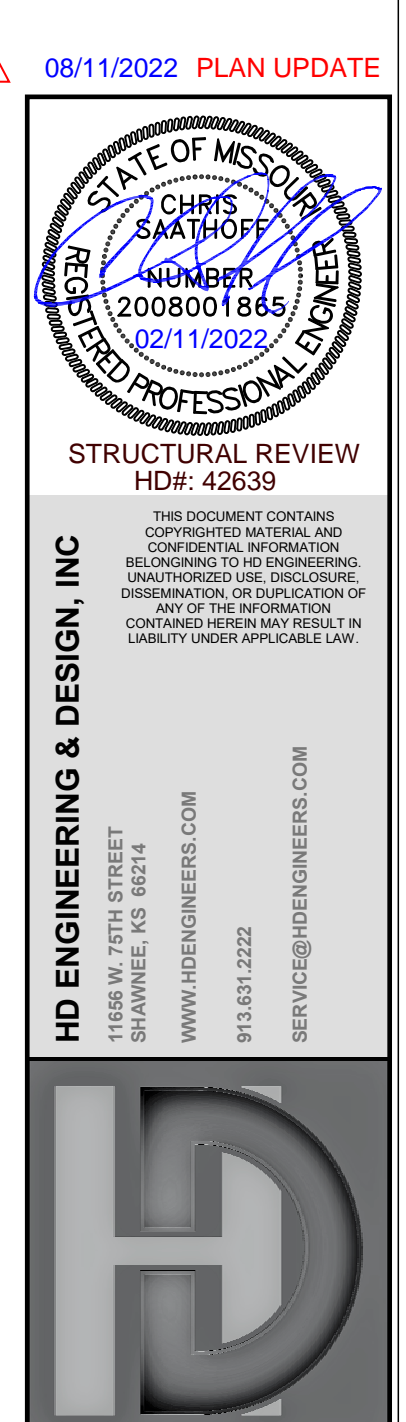
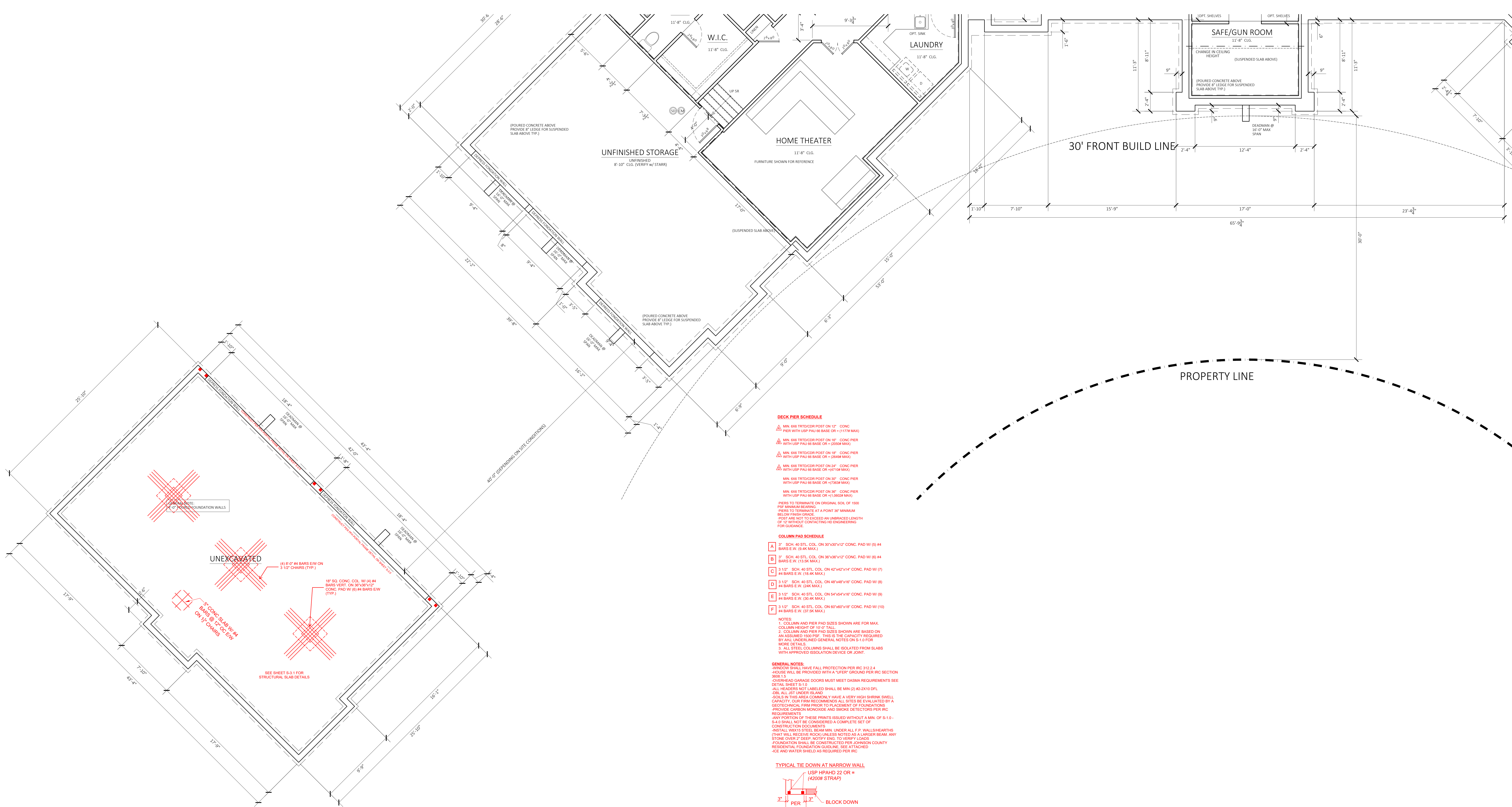
CONSTRUCTION DOCUMENTS

Project Number: Milligan
Date: 2022 AUG 05
Drawn By: MGS
Checked By: CDG

A 103

Scale: 1/4"=1'-0"

08/24/2023



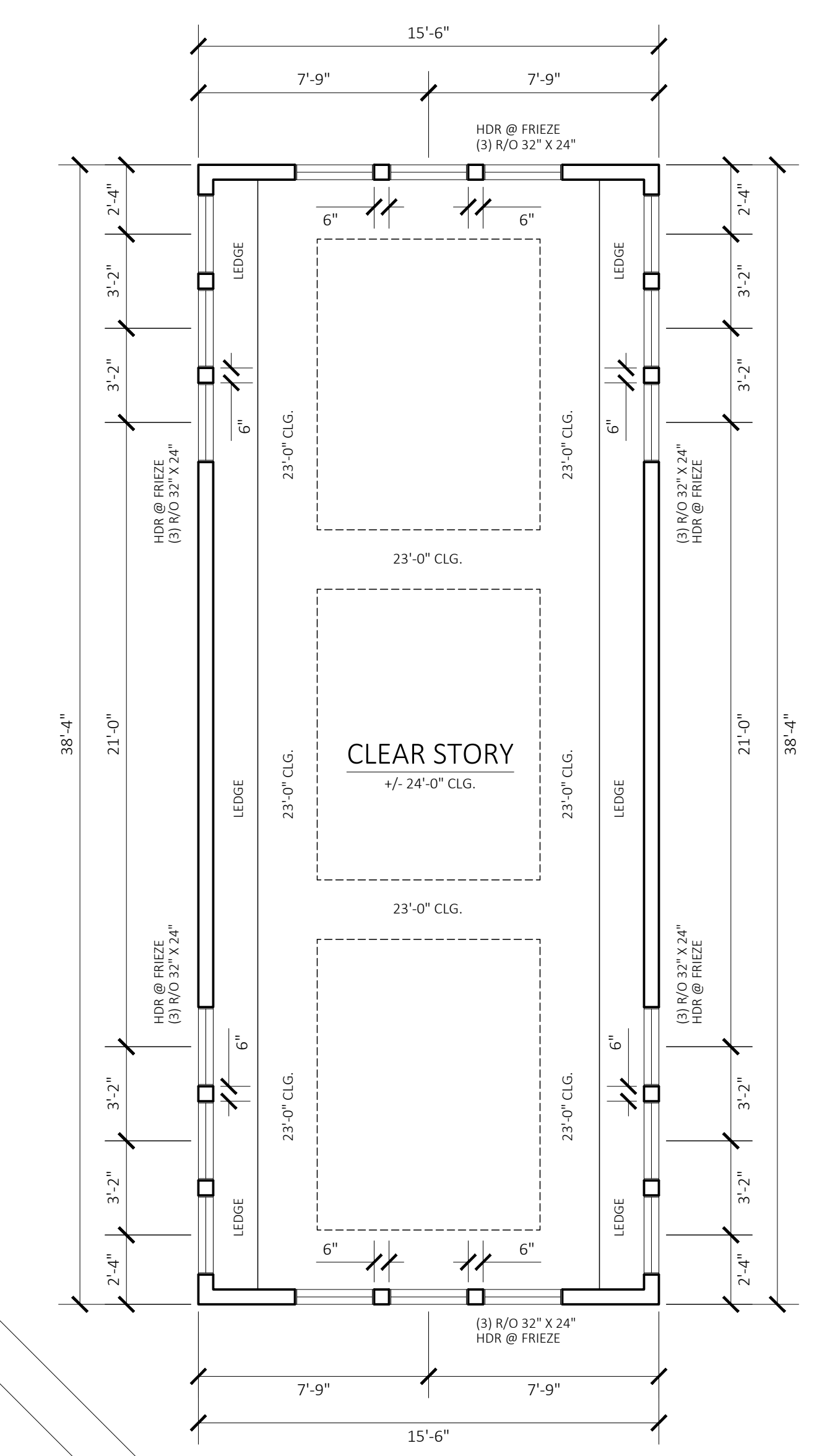
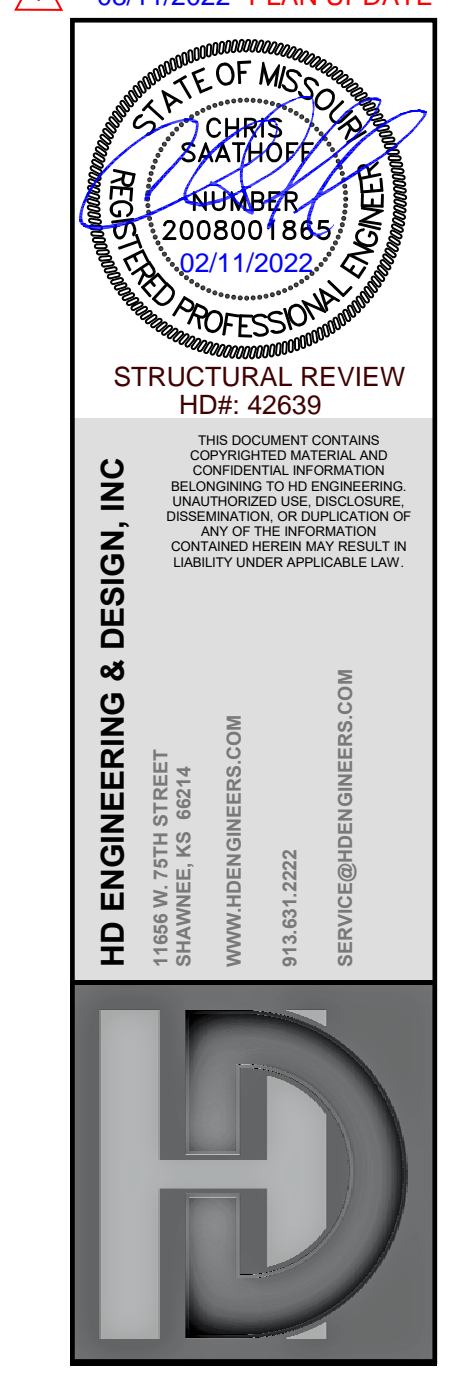
No.	Description	Date

CONSTRUCTION DOCUMENTS

Project Number: Milligan
Date: 2022 AUG 05
Drawn By: MGS
Checked By: CDG

A 104

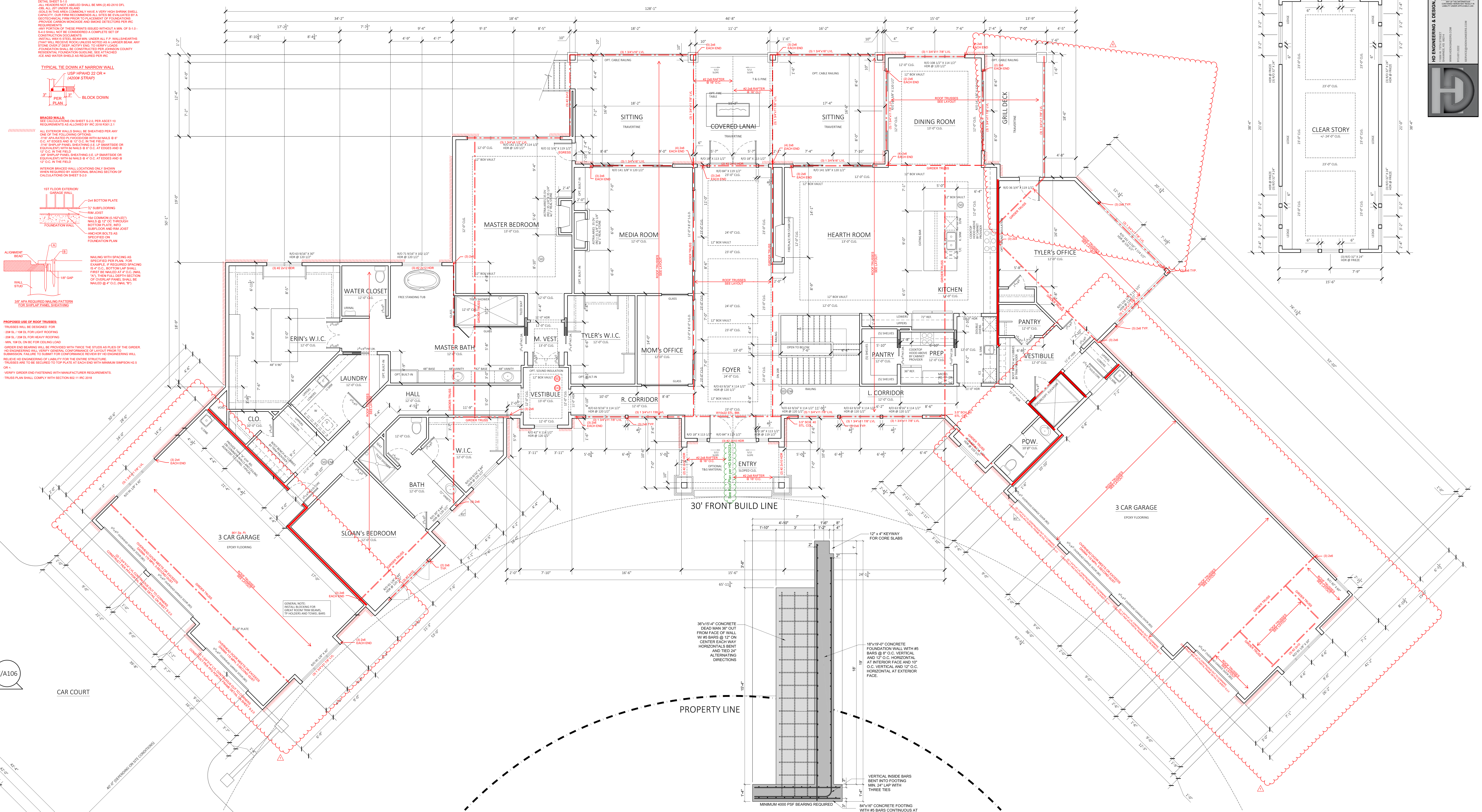
Scale: 1/4" = 1'-0"
08/24/2023



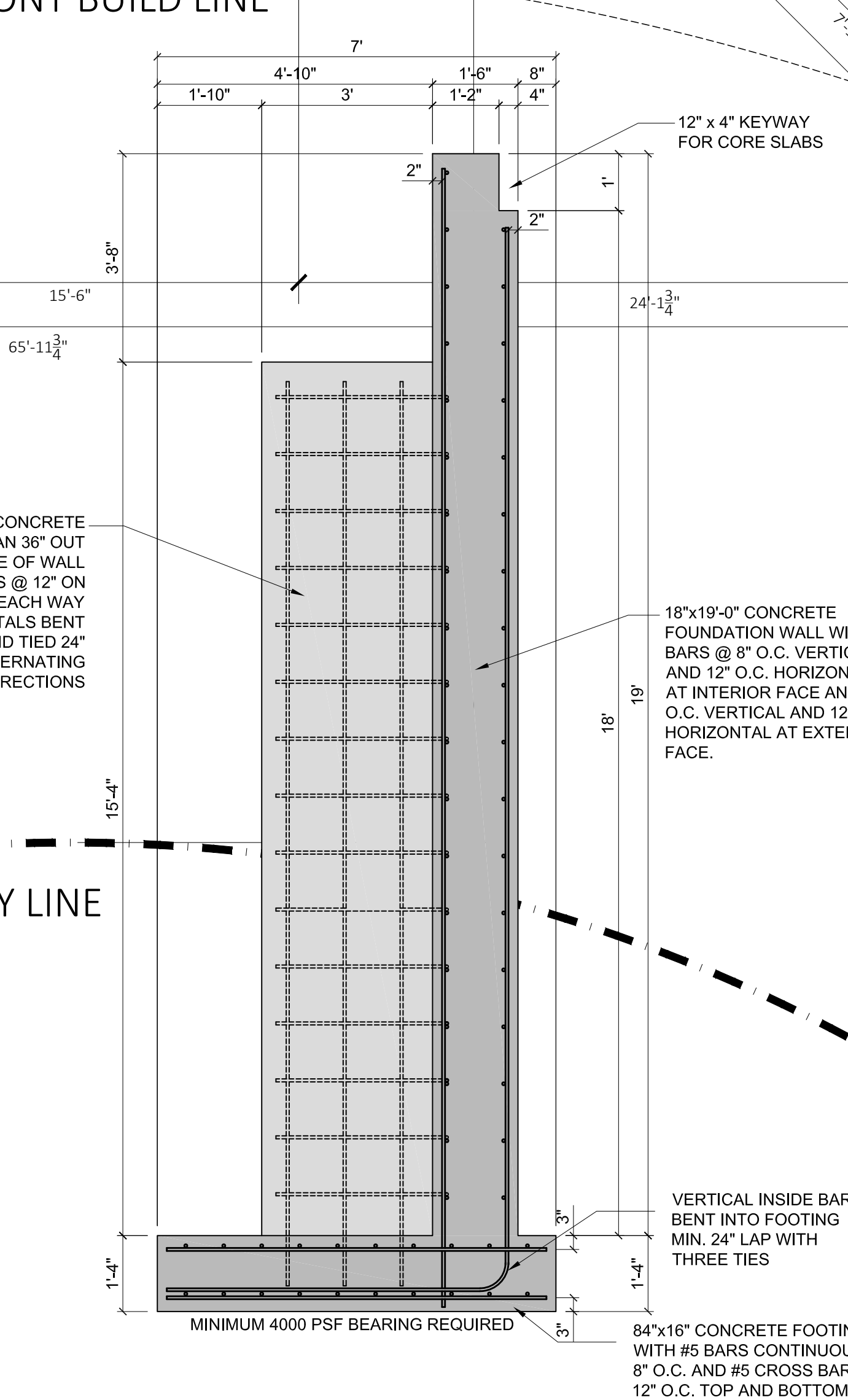
- LOAD BEARING WALL
 - LOAD BEARING BEAM
 - SMOKE DETECTOR
 - CARBON MONOXIDE SENSOR
- GENERAL NOTES:**
 WINDOW SHALL HAVE FALL PROTECTION PER IRC 310.2.4
 ROOFING SHALL BE PROVIDED WITH A VENTED DRAINAGE PER IRC SECTION 308.1
 COVERED GARAGE DOORS MUST MEET DASHA REQUIREMENTS SEE SECTION 310.1
 ALL HEADERS NOT LABELED SHALL BE MIN (2) K20X10 DFL
 SOLE IN THIS AREA COMMONLY HAVE A VERY HIGH SHEAR SWELL CAPACITY. DUE TO THIS CAPACITY, ALL STUDS SHALL BE EVALUATED BY A GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF FOUNDATIONS. PROVIDE CORROSION MONITORING AND SMOKE DETECTORS PER IRC REQUIREMENTS.
 ANY PORTION OF THESE PRINTS ISSUED WITHOUT A MIN. OF 5-10-24 CONTRACT DOCUMENTS SHALL BE VOID.
 METAL STUDS SHALL BE INSTALLED UNDER ALL F.P. WALLS/SHEATHS THAT WILL RECEIVE LOADS UNLESS NOTED AS A LARGER BEAM OR STONE/CONCRETE. SEE SECTION 310.1 FOR DETAILS.
 FOUNDATION SHALL BE CONSTRUCTED PER LOCAL COUNTY RESIDENTIAL FOUNDATION OUTLINE. SEE ATTACHED GC AND WATER SHEED AS REQUIRED PER IRC
- TYPICAL TIE DOWN AT NARROW WALL**
 USP HP#4D 22 OR #4000B STRAP
 BLOCK DOWN
 PER PLAN
- BRACED WALLS**
 SEE CALCULATIONS ON SHEET S-10 FOR ADDITIONAL REQUIREMENTS AS ALLOWED BY IRC 2018.10.1.2.1
 ALL EXTERIOR WALLS SHALL BE SHEATHED PER ANY ONE OF THE FOLLOWING OPTIONS:
 1) APPLICABLE PLYWOOD/OSB WITH MIN. 5/8" @ 12" O.C. AT EDGES AND 12" O.C. IN THE FIELD.
 2) 2" SIP LAP PANEL SHEATHING (E.L.P. SHEATHING OR EQUIVALENT) WITH MIN. 5/8" @ 12" O.C. AT EDGES AND 12" O.C. IN THE FIELD.
 3) SIP LAP PANEL SHEATHING (E.L.P. SHEATHING OR EQUIVALENT) WITH MIN. 5/8" @ 12" O.C. AT EDGES AND 12" O.C. IN THE FIELD.
 INTERIOR BRACED WALL LOCATIONS ONLY SHOWN WHEN REQUIRED BY ADDITIONAL BRACING SECTION OF CALCULATIONS ON SHEET S-10
- 1ST FLOOR EXTERIOR GARAGE WALL**
 2x4 BOTTOM PLATE
 2x4 SUBSOILING
 RM JOIST
 16x COMMON (16"x16") NAILS @ 12" O.C. THROUGH BOTTOM PLATE INTO SUBSOILING AND RM JOIST
 ANCHOR BOLTS AS SPECIFIED ON FOUNDATION PLAN
- ALIGNMENT**
 WALL STUD
 1/8" GAP
 3/8" APA REQUIRED NAILING PATTERN FOR SHEATHING
- PROPOSED USE OF ROOF TRUSSES:**
 TRUSSES WILL BE DESIGNED FOR:
 20# SL 110# CL FOR LIGHT ROOFING
 20# SL 20# CL FOR HEAVY ROOFING
 16# WALK ON ROOF FOR CEILING LOAD
- GRINDER BEARINGS WILL BE PROVIDED WITH TWICE THE STUDS AS PLUS OF THE GRINDER HD ENGINEERING WILL VERIFY COMPLIANCE OF LAYOUT PRIOR TO SUBMISSION. FAILURE TO SUBMIT FOR COMPLIANCE REVIEW BY HD ENGINEERING WILL RELIEVE HD ENGINEERING OF LIABILITY FOR THE ENTIRE STRUCTURE. TRUSSES ARE TO BE SECURED TO TOP PLATE AT EACH END WITH MINIMUM SIMPSON H2.5 OR #4.**
 VERIFY GRINDER END FASTENING WITH MANUFACTURER REQUIREMENTS.
 TRUSS PLAN SHALL COMPLY WITH SECTION 802.1 IRC 2018



SHEET SIZE 60" X 36"



18' FOUNDATION WALL



MAIN LEVEL FLOOR PLAN
SCALE 1/4"=1'-0"



MAIN LEVEL DETACHED GARAGE
 SCALE 1/4"=1'-0"

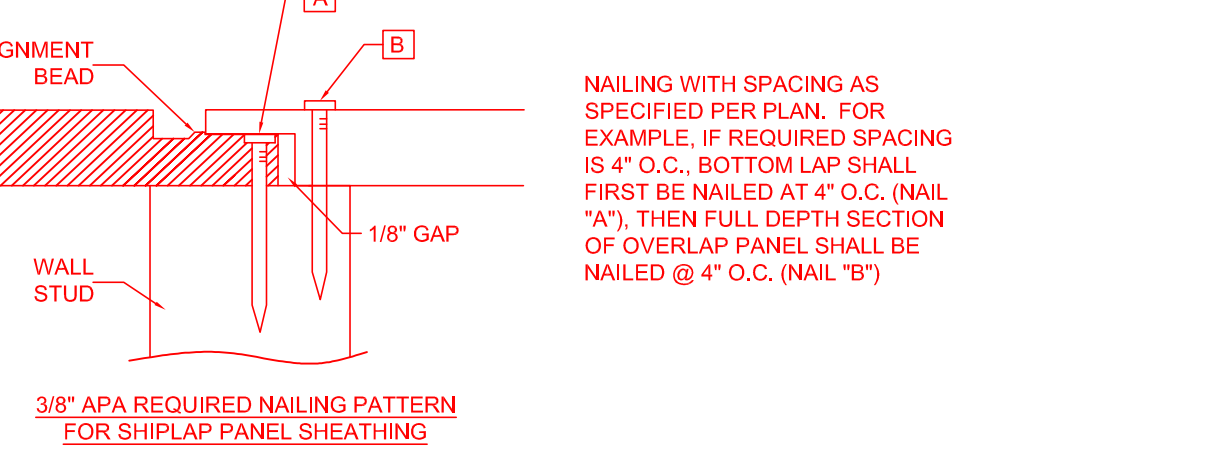
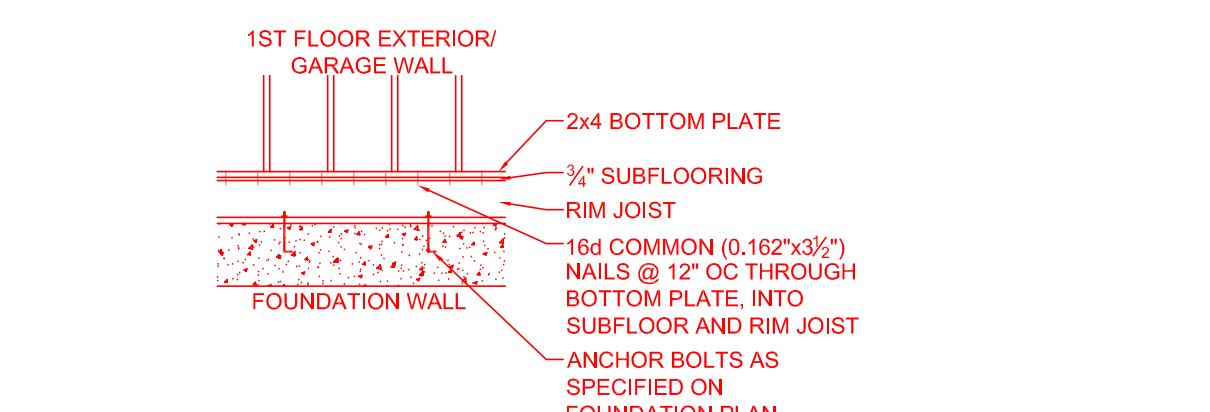
PROPOSED USE OF ROOF TRUSSES:
 TRUSSES WILL BE DESIGNED FOR:
 -20# SL 180 DL FOR LIGHT ROOFING
 -20# SL 120 DL FOR HEAVY ROOFING
 -MIN. 10# DL ON RC FOR CEILING LOAD
 -CHORD END BEAMS WILL BE PROVIDED WITH TWICE THE STUDS AS PILES OF THE ORDER
 -NO ENGINEERING WILL VERIFY GENERAL PERFORMANCE OF LAYOUT PRIOR TO SUBMISSION. FAILURE TO SUBMIT FOR CONFORMANCE REVIEW BY AN ENGINEER WILL RELIEVE THE ENGINEERING OF LIABILITY FOR THE ENTIRE STRUCTURE.
 -TRUSSES ARE TO BE SECURED TO PLATE AT EACH END WITH MINIMUM SIMPSON HD-5 OR 4.
 -VERIFY CHORD END FASTENINGS WITH MANUFACTURER REQUIREMENTS.
 -TRUSS PLAN SHALL COMPLY WITH SECTION RC 11 IRC 2018.

— LOAD BEARING WALL
 - - - - - LOAD BEARING BEAM
 (C) SMOKE DETECTOR
 (C) CARBON MONOXIDE SENSOR

GENERAL NOTES:
 -WINDSHIELD SHALL HAVE FALL PROTECTION PER IRC 312.2.4
 -HOUSE WILL BE PROVIDED WITH A "PERF" GROUND PER IRC SECTION 250.5.1
 -GARAGE DOORS MUST MEET DOWM REQUIREMENTS SEE DETAIL SHEET S-1.0
 -ALL HANGERS NOT LABELED SHALL BE MIN (2) 2X10 DRL OR ALL 2X12 UNDER JOIST.
 -SOLES IN THIS AREA GENERALLY HAVE A VERY HIGH SHRINK SWELL CAPACITY. CUR FROM PROFORMERALS. SITE BE EVALUATED BY A GEOTECHNICAL ENGINEER TO PLACEMENT OF FOUNDATION REQUIREMENTS.
 -ANY PORTION OF THESE PRINTS ISSUED WITHOUT A MIN. OF 5-10-24 SHALL NOT BE CONSIDERED A COMPLETE SET OF CONTRACT.
 -STEEL BEAMS SHALL BE 18# MIN UNDER ALL 2" WALLS AND 20# MIN UNDER ALL 4" WALLS AND 24# MIN UNDER ALL 6" WALLS AND 28# MIN UNDER ALL 8" WALLS AND 32# MIN UNDER ALL 10" WALLS. (THIS WILL BE NOTED AS A LARGER BEAM. ANY OTHER OVER 4" DEEP BEAMS TO BE USED TO VERIFY FOUNDATION SHALL BE CONSTRUCTED PER JOHNSON COUNTY RESIDENTIAL FOUNDATION GUIDELINE. SEE ATTACHED ICE AND WATER SHIELD AS REQUIRED PER IRC.

TYPICAL TIE DOWN AT NARROW WALL
 USP HPAHD 22 OR = (4200# STRAP)
 3" PER 3" BLOCK DOWN

BRACED WALLS:
 SEE CALCULATIONS ON SHEET S-2.0. PER ACET-10 REQUIREMENTS AS ALLOWED BY IRC 610.3.1.1
 ALL EXTERIOR WALLS SHALL BE SHEATHED PER ANY ONE OF THE FOLLOWING OPTIONS:
 1) 1/2" SHEATHING OVER 2X4 WALLS WITH 6# NAILS @ 6" O.C. @ EDGES AND @ 12" O.C. IN THE FIELD.
 2) 1/2" SHIP-LAP PANEL SHEATHING (E.L.P. SMARTSIDE OR EQUIVALENT) WITH 6# NAILS @ 4" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD.
 3) 5/8" SHIP-LAP PANEL SHEATHING (E.L.P. SMARTSIDE OR EQUIVALENT) WITH 6# NAILS @ 4" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD.
 INTERIOR BRACED WALL LOCATIONS ONLY SHOWN WHEN REQUIRED BY ADDITIONAL BRACING SECTION OF CALCULATIONS ON SHEET S-2.0.



3/8" APA REQUIRED NAILING PATTERN FOR SHIP-LAP SHEATHING

STARR HOMES LLC
 PO BOX 2366
 Overland Park, KS 66283

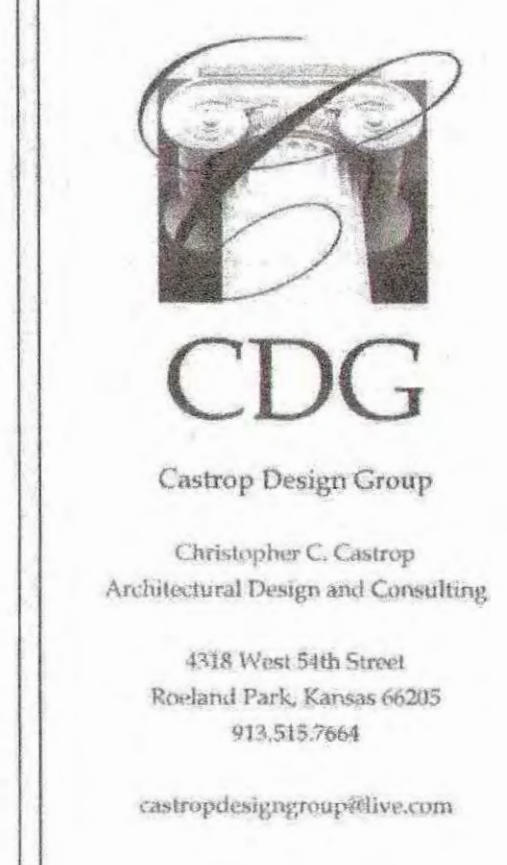
7228 W. 181st St.
 Stillwell, KS 66063
 Blue Valley Business Park

STARR HOMES LLC
 COPYRIGHT: STARR HOMES LLC

HO ENGINEERING & DESIGN, INC.
 STRUCTURAL REVIEW
 PER 4203

06/08/2023 TRUSS REVIEW

The Tyler and Erin Milligan Residence
 Lot 3A and 4A NE Promised View Dr
 Lee's Summit, Missouri



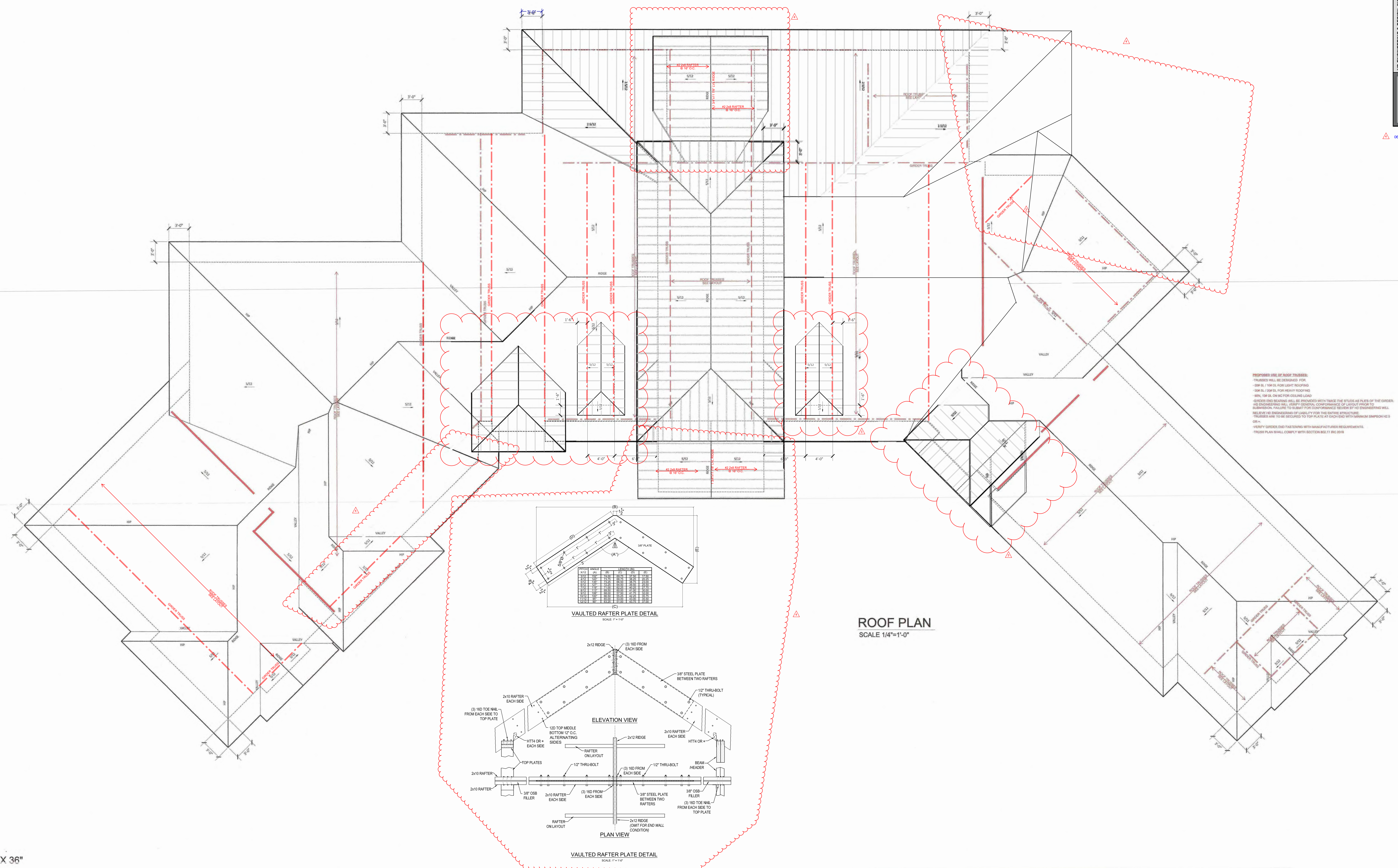
No.	Description	Date
Revised 1-13-2023	MB	
Added Standing Seam Metal Roof & 2 Chimneys		

CONSTRUCTION DOCUMENTS

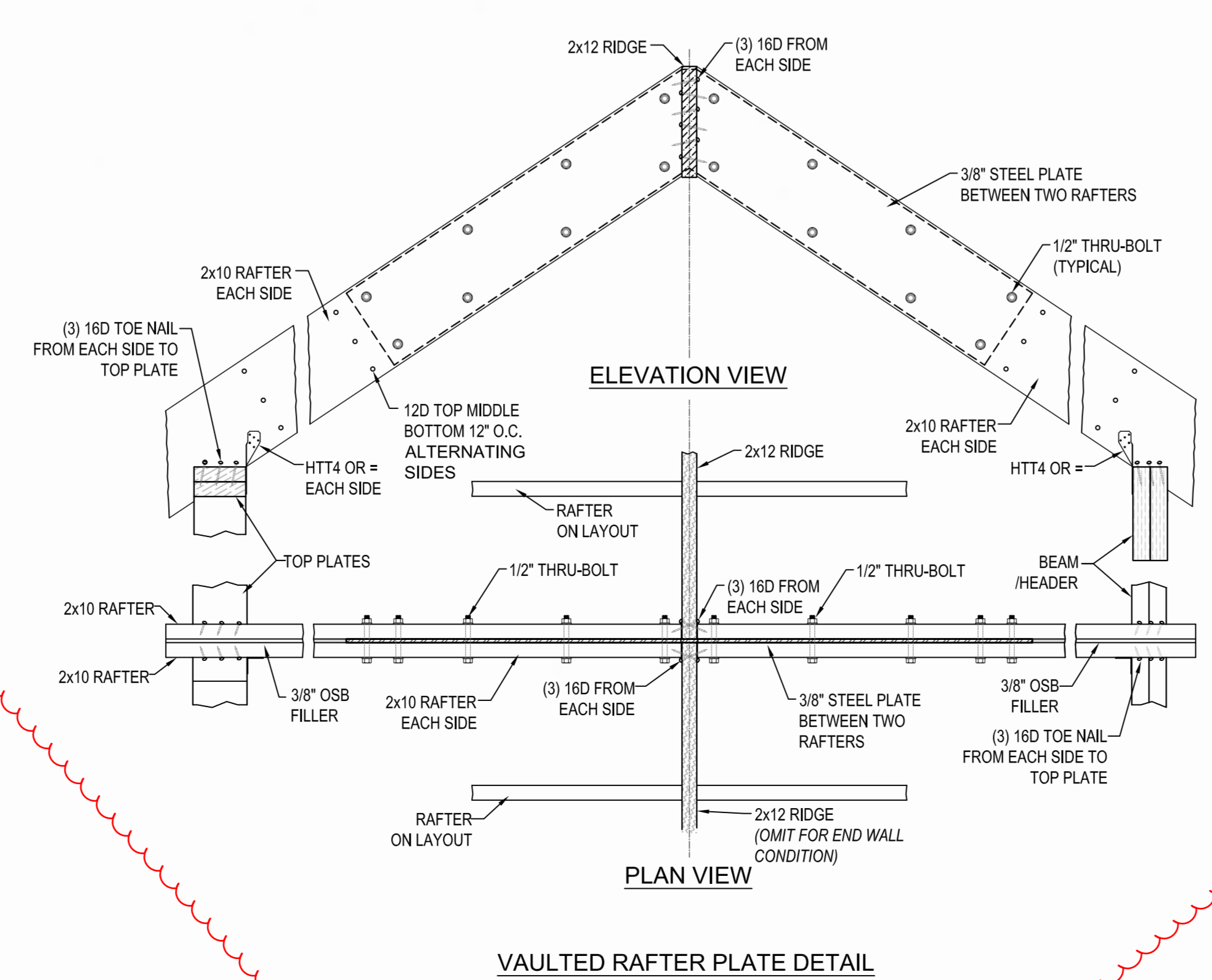
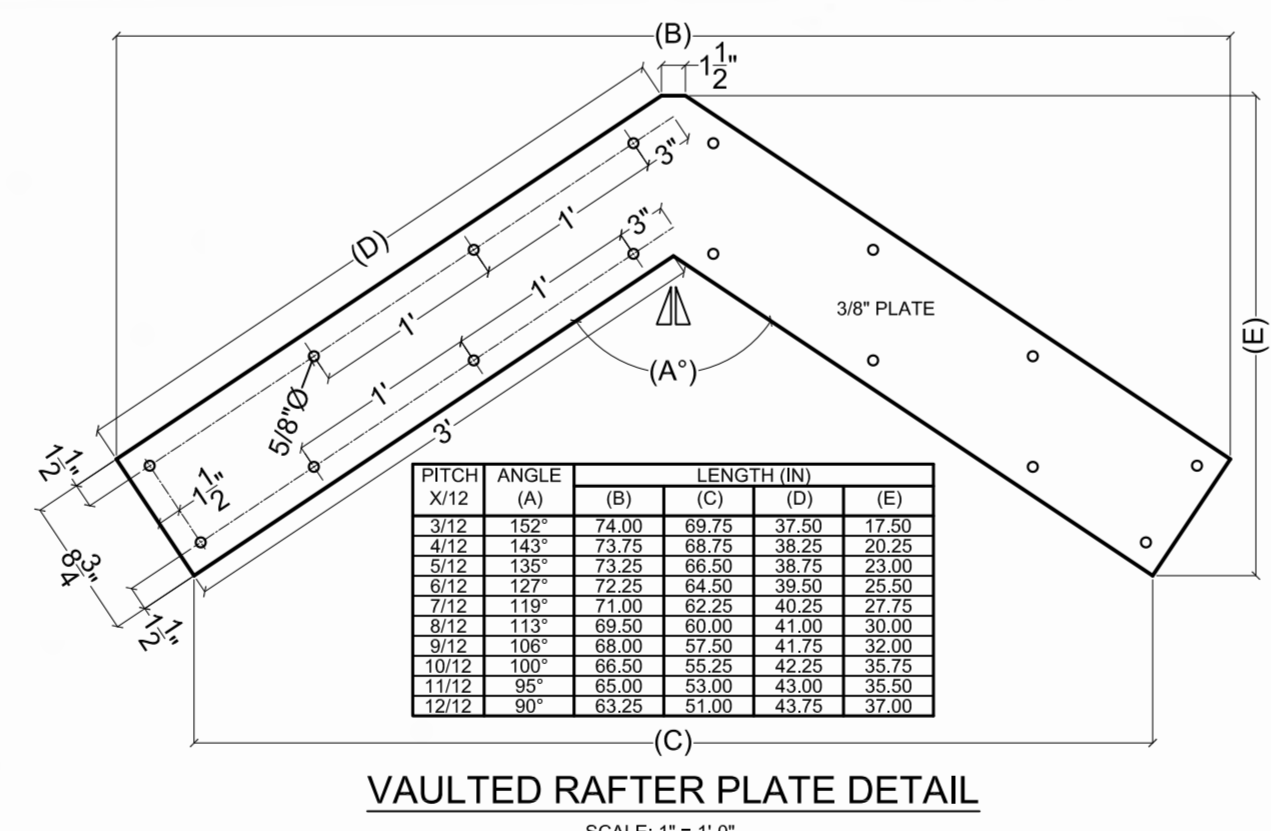
Project Number: Milligan
 Date: 2022 AUG 05
 Drawn By: MGS
 Checked By: CDG

A 105
 Scale: 1/4" = 1'-0"

RELIEF FOR CONSTRUCTION
 ANY OTHER CLARIFICATION
 CONTACT: MGS
 DATE: 08/24/2023

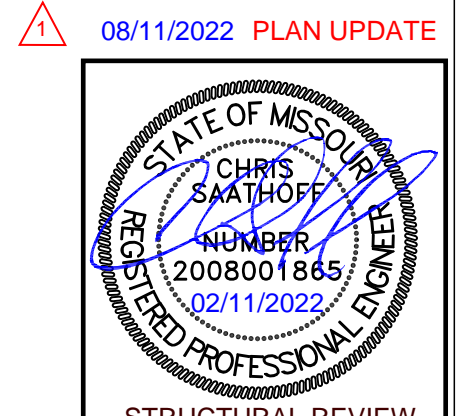


PROPOSED USE OF ROOF TRUSSES:
 TRUSSES WILL BE DESIGNED FOR:
 - 20 LB/DFL FOR LIGHT ROOFING
 - 30 LB/DFL FOR HEAVY ROOFING
 - 100 LB/DFL FOR CEILING LOADS
 - WIND RESISTANCE WILL BE PROVIDED WITH TWICE THE STUDS AS PER THE ORDER.
 HO ENGINEERING WILL VERIFY GENERAL PERFORMANCE OF LAYOUT PRIOR TO SUBMISSION. FAILURE TO SUBMIT FOR PERFORMANCE REVIEW BY HO ENGINEERING WILL RELIEVE HO ENGINEERING OF LIABILITY FOR THE ENTIRE STRUCTURE.
 TRUSSES ARE TO BE SECURED TO TOP PLATE AT EACH END WITH BRASS NAIL EMPLOYMENT'S OR
 HEAVY GIRDERS AND FASTENING WITH MANUFACTURER'S REQUIREMENTS.
 TRUSS PLAN SHALL COMPLY WITH SECTION 802.11 IRC 2018.



ROOF PLAN
 SCALE 1/4" = 1'-0"

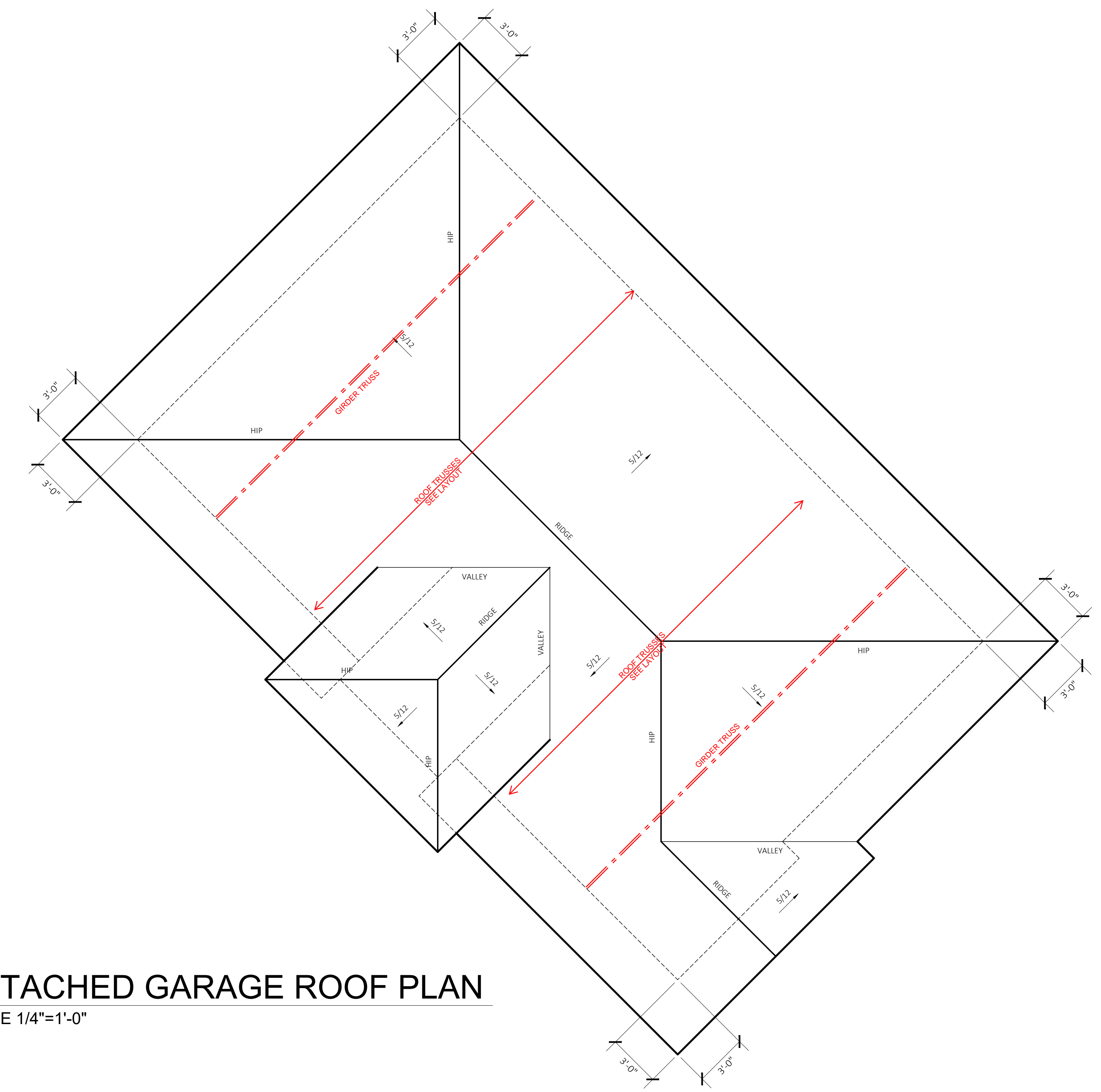
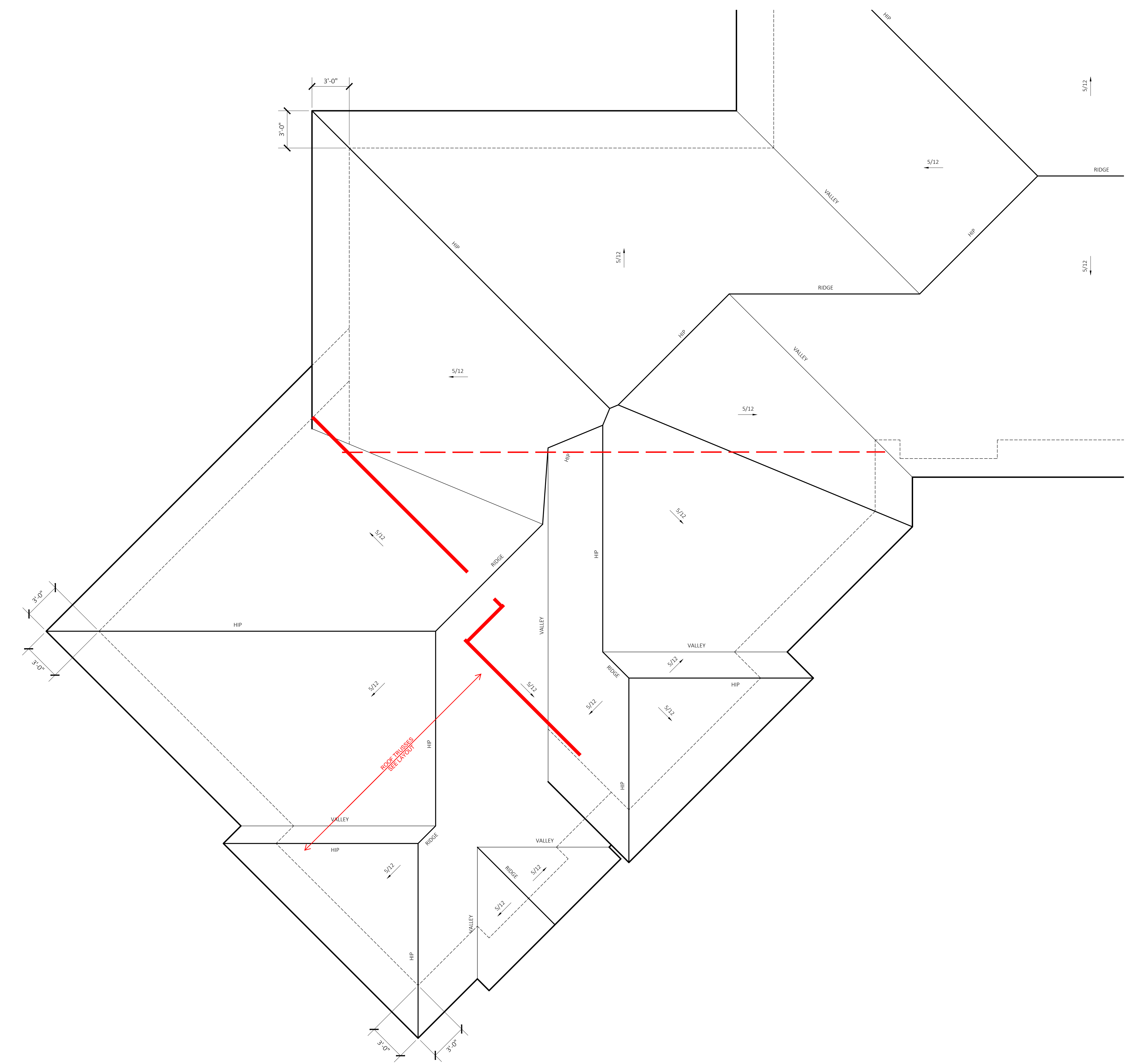
The Tyler and Erin Milligan Residence
 Lot 3A and 4A NE Promised View Dr
 Lee's Summit, Missouri



08/11/2022 PLAN UPDATE
 STRUCTURAL REVIEW
 HCH 42021

No.	Description	Date

CONSTRUCTION DOCUMENTS
 Project Number Milligan
 Date 2022 AUG 05
 Drawn By MGS
 Checked By CDG
A 105.1
 Scale 1/4" = 1'-0"
 08/24/2023



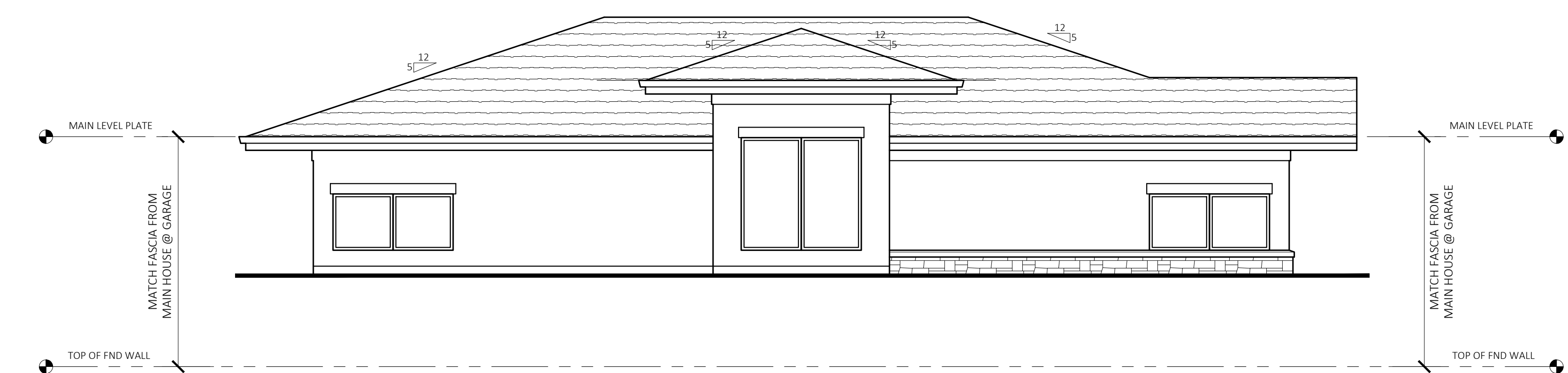
DETACHED GARAGE ROOF PLAN
 SCALE 1/4"=1'-0"

DETACHED GARAGE ELEVATIONS SHEET

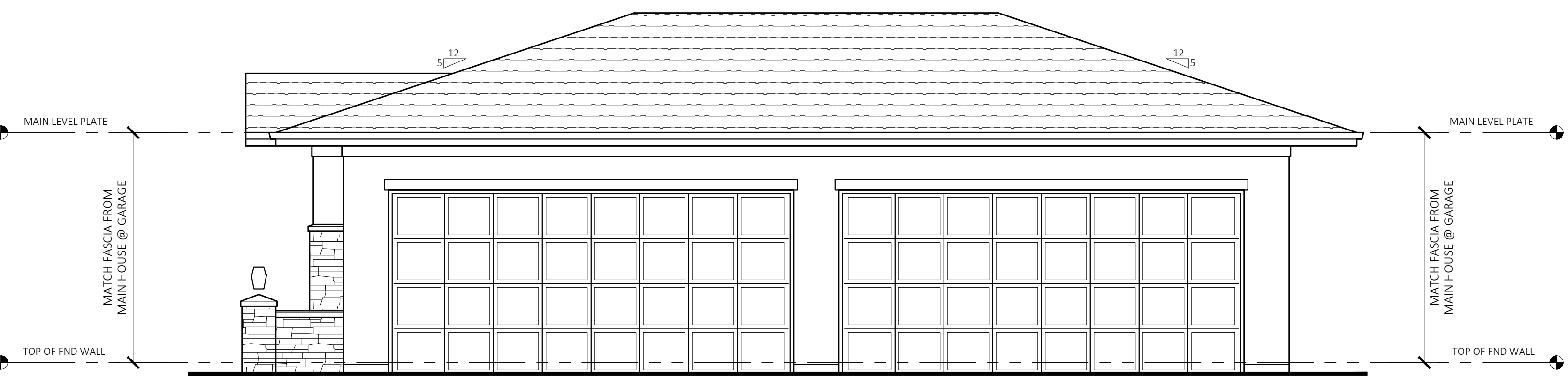
STARR HOMES LLC
PO BOX 22666
Overland Park, KS 66283
7229 W. 161st St.
Shilwell, KS 66203
Blue Valley Business Park

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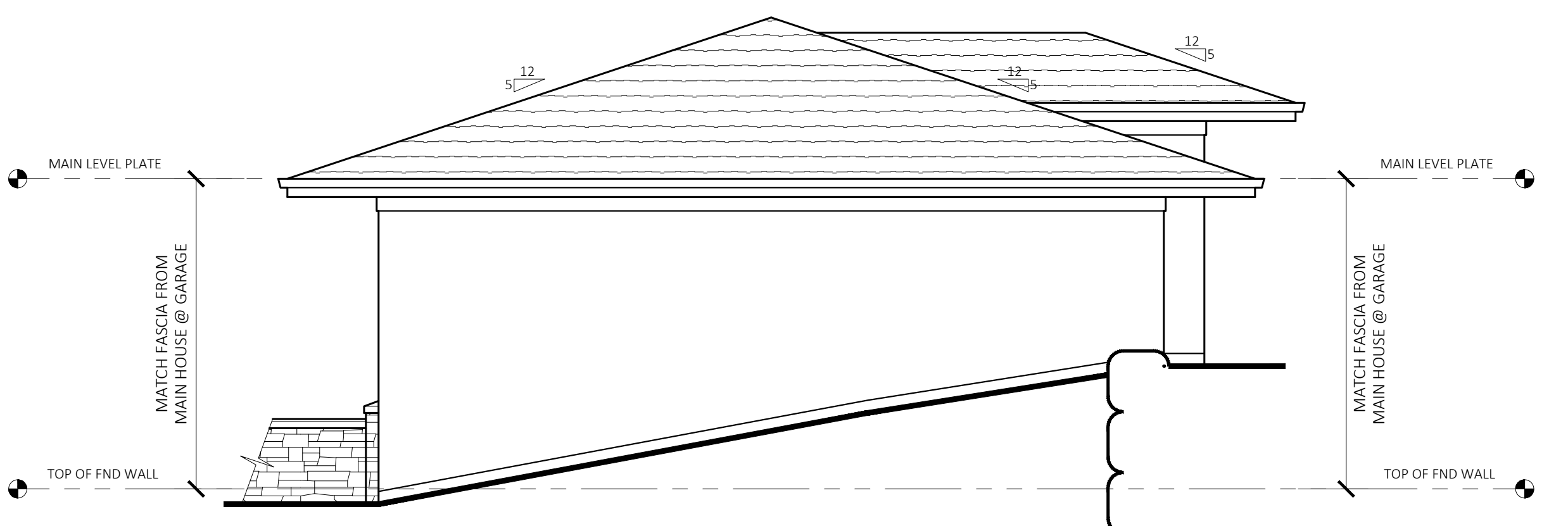
The Tyler and Erin Milligan Residence
Lot 3A and 4A NE Promised View Dr
Lee's Summit, Missouri



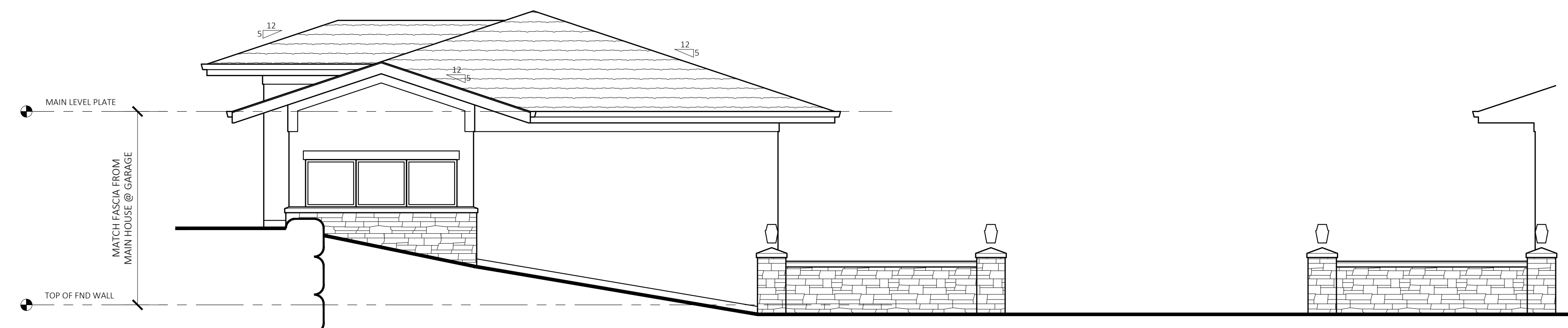
A BACK ELEVATION
SCALE 1/4"=1'-0"



C CAR COURT ELEVATION
SCALE 1/4"=1'-0"



D REAR ELEVATION
SCALE 1/4"=1'-0"



B FRONT ENTRANCE ELEVATION
SCALE 1/4"=1'-0"

08/11/2022 PLAN UPDATE

CHADWICK C. CASTRUP
REGISTERED PROFESSIONAL ENGINEER
STRUCTURAL REVIEW
NO. 42021

HD ENGINEERING & DESIGN, INC.
1020 W. 37th Street
Overland Park, KS 66204
913.242.2222
www.hd-engineering.com

CDG
Castrup Design Group
Chadwick C. Castrup
Architectural Design and Consulting
4338 West 56th Street
Overland Park, Kansas 66205
913.515.7664
castrup-designgroup@live.com

No.	Description	Date

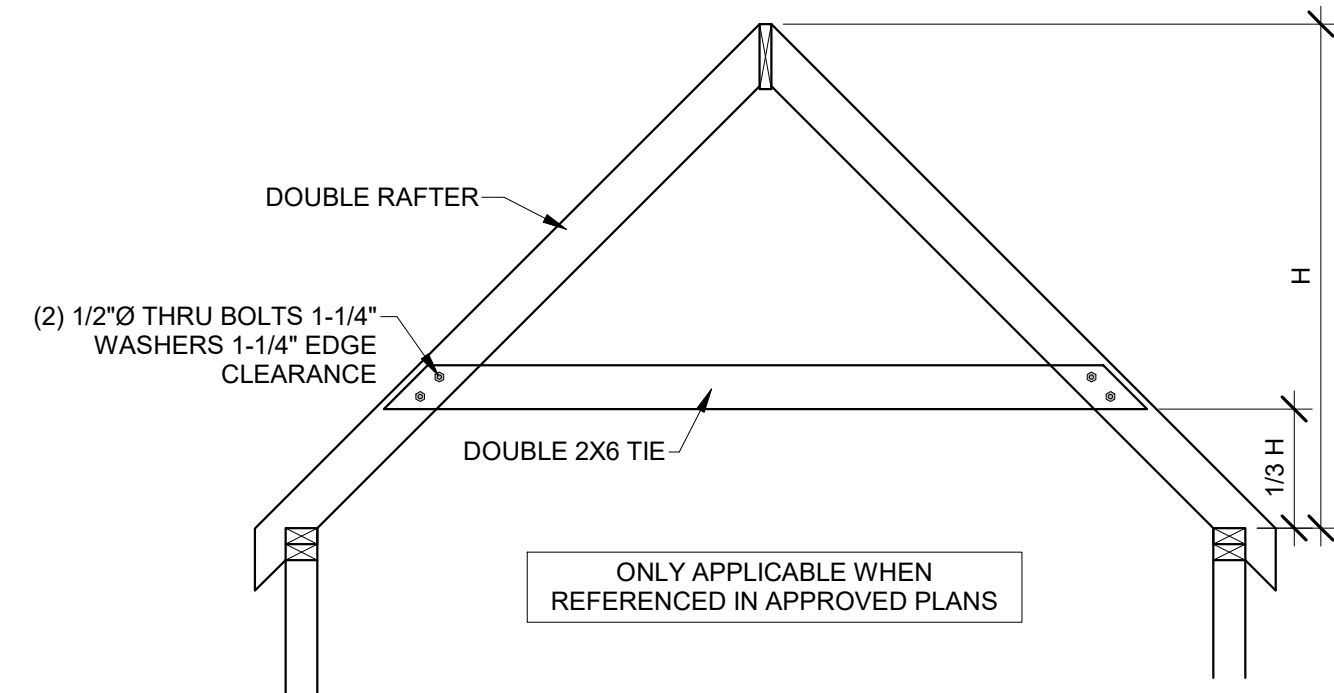
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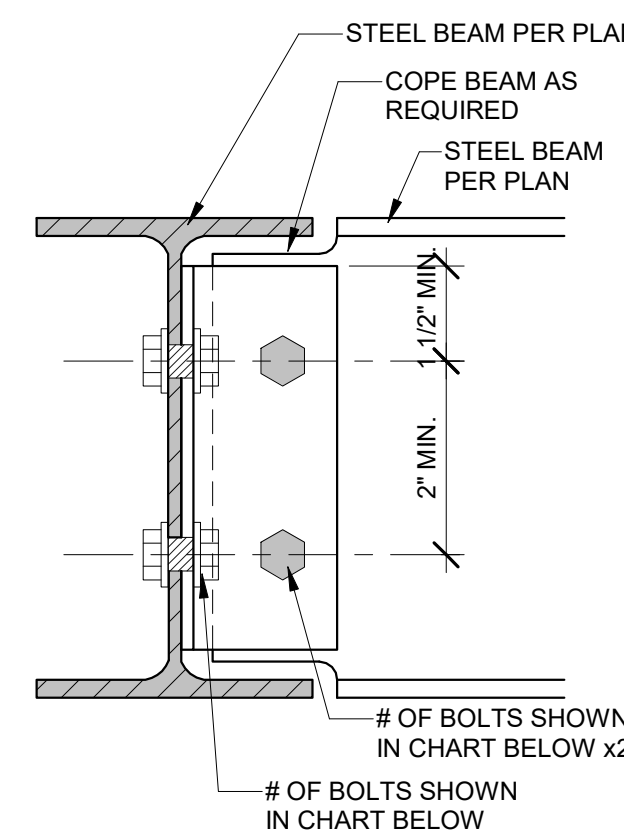
A 106

Scale: 1/4" = 1'-0"

08/24/2023



11 HIP SUPPORT FRAME
3/8" = 1'-0"

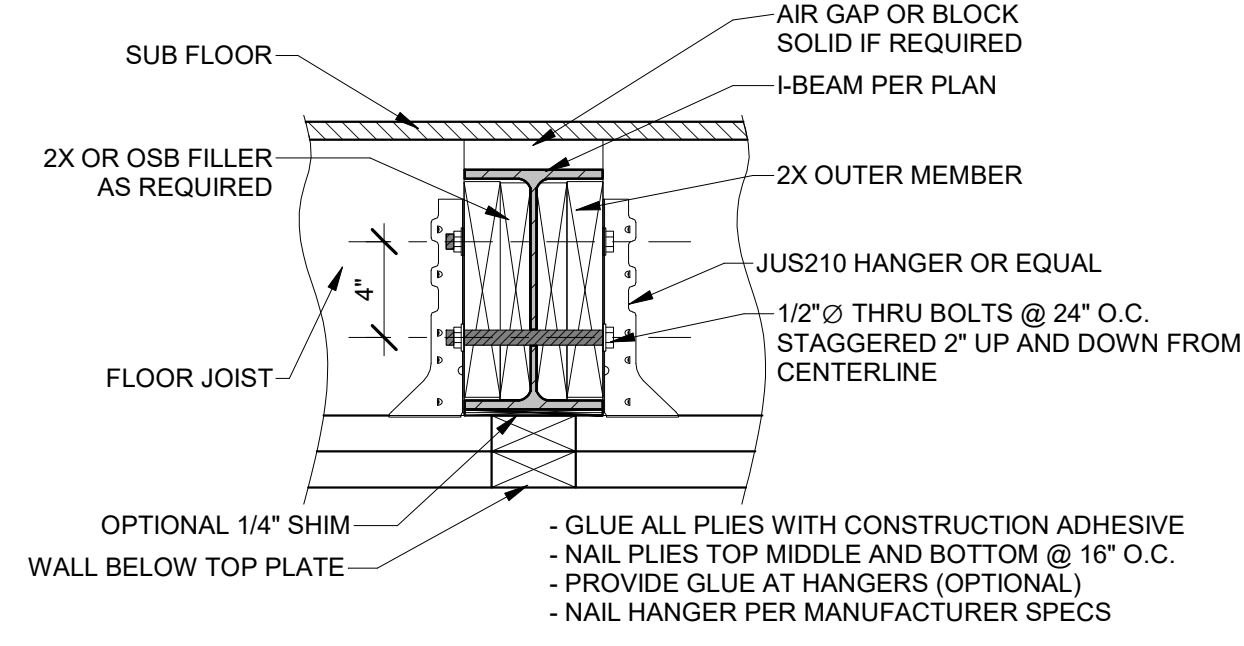


10 BEAM TO GIRDER CONNECTION
3" = 1'-0"

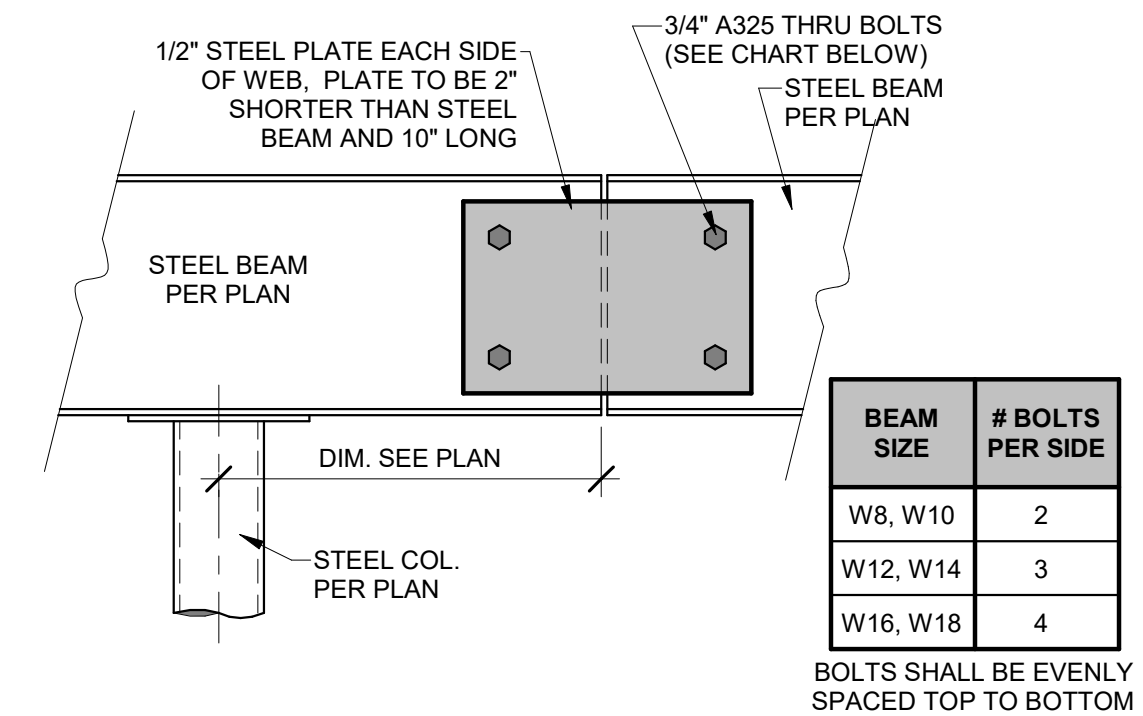
BEAM CONNECTION SCHEDULE

BEAM SIZE	# OF BOLT IN CONNECTION
W8, W10	2
W12, W14	3
W16, W18	4

NOTES:
1. NUMBER OF BOLTS DETERMINED BY SMALLER OF TWO BEAMS BEING CONNECTED
2. ALL BOLTS, 3/4" DIAMETER A325-N, UNO
3. FULL PERIMETER 1/4" FILLET WELD MAY BE SUBSTITUTED FOR EITHER OR BOTH BOLTED CONNECTIONS



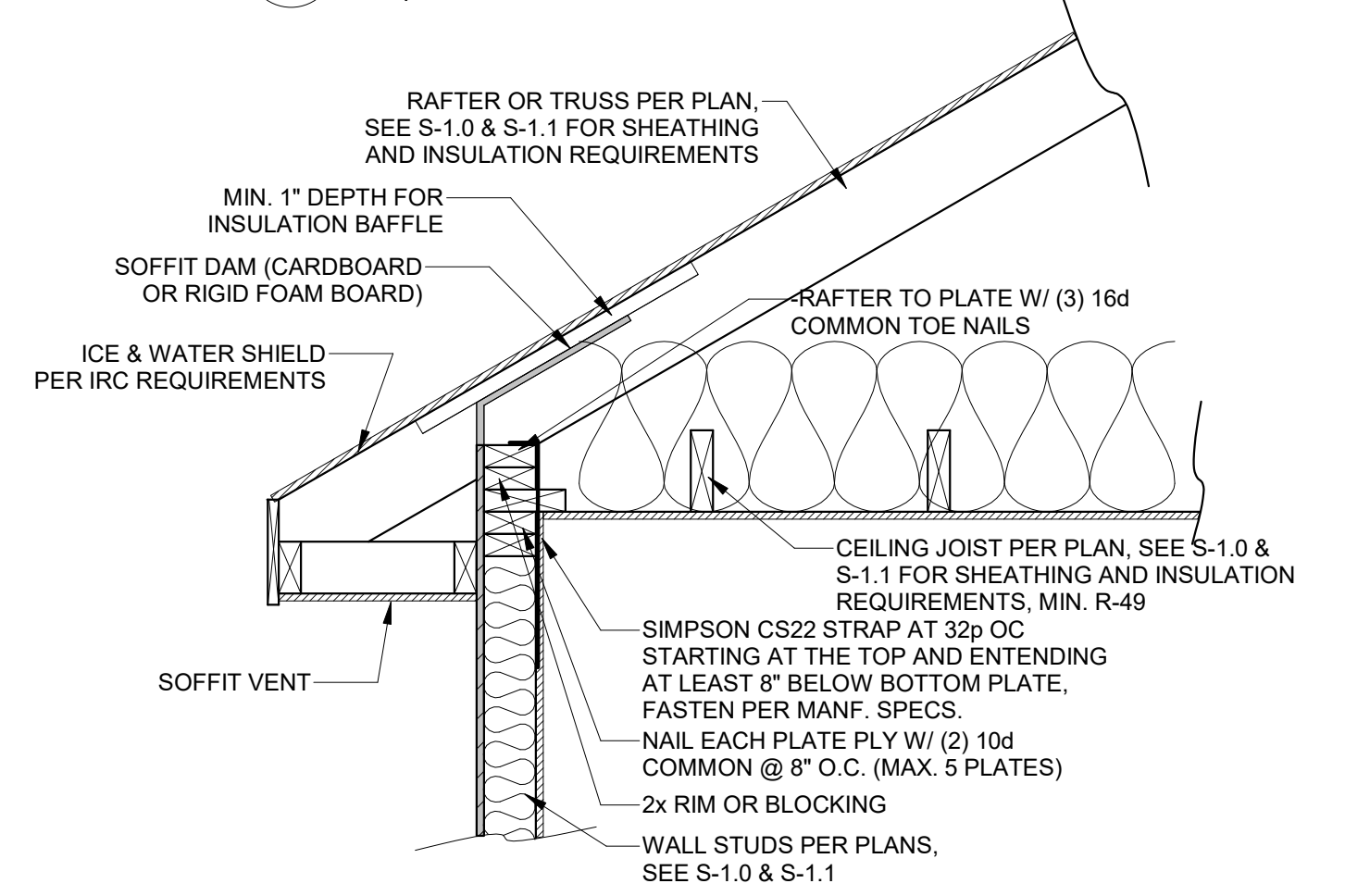
8 UPSET STEEL BEAM DETAIL
1 1/2" = 1'-0"



9 STEEL BEAM SPLICE DETAIL
1 1/2" = 1'-0"

BEAM SIZE	# BOLTS PER SIDE
W8, W10	2
W12, W14	3
W16, W18	4

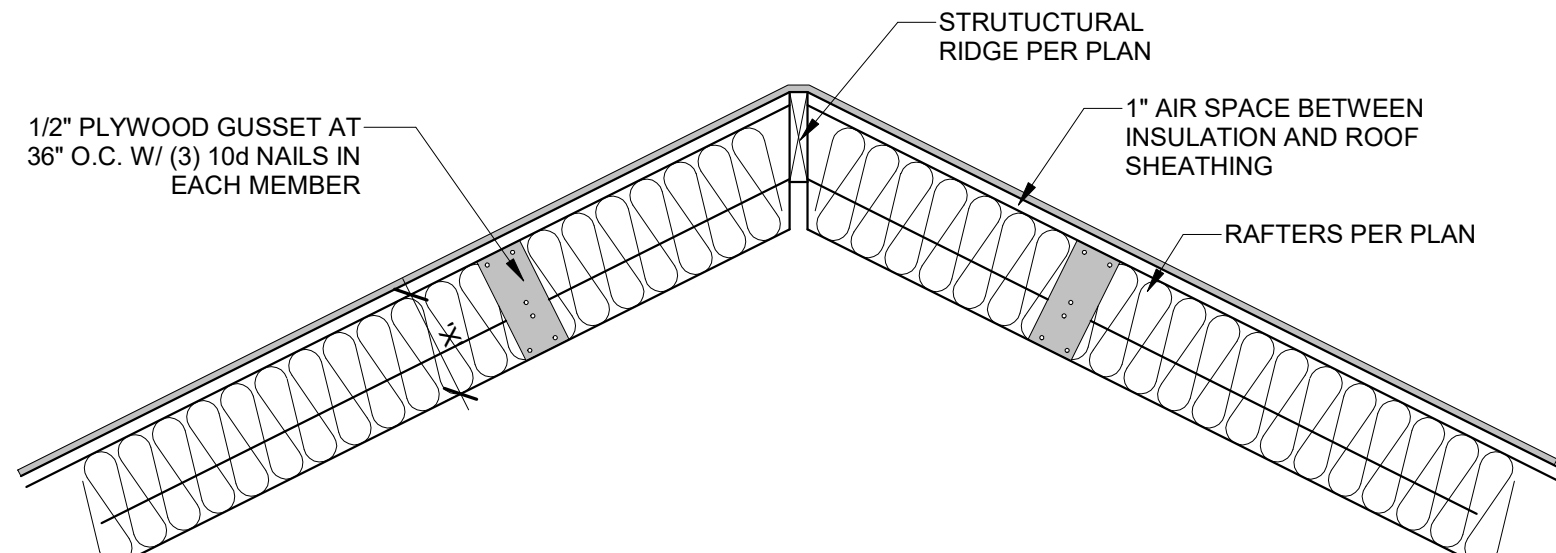
BOLTS SHALL BE EVENLY SPACED TOP TO BOTTOM



7 OPTION 4 RAFTER BEARING
1" = 1'-0"

HIP/VALLEY ALLOWABLE SPAN TABLE

TYPE	MAX. UNSUPPORTED SPAN				
	2x8	2x10	2x12	1 3/4"x9 1/2" LVL	1 3/4"x11 7/8" LVL
HIP RAFTER	11'-3"	13'-3"	15'-2"	15'-8"	18'-2"
VALLEY RAFTER	8'-11"	10'-6"	12'-0"	13'-2"	15'-3"

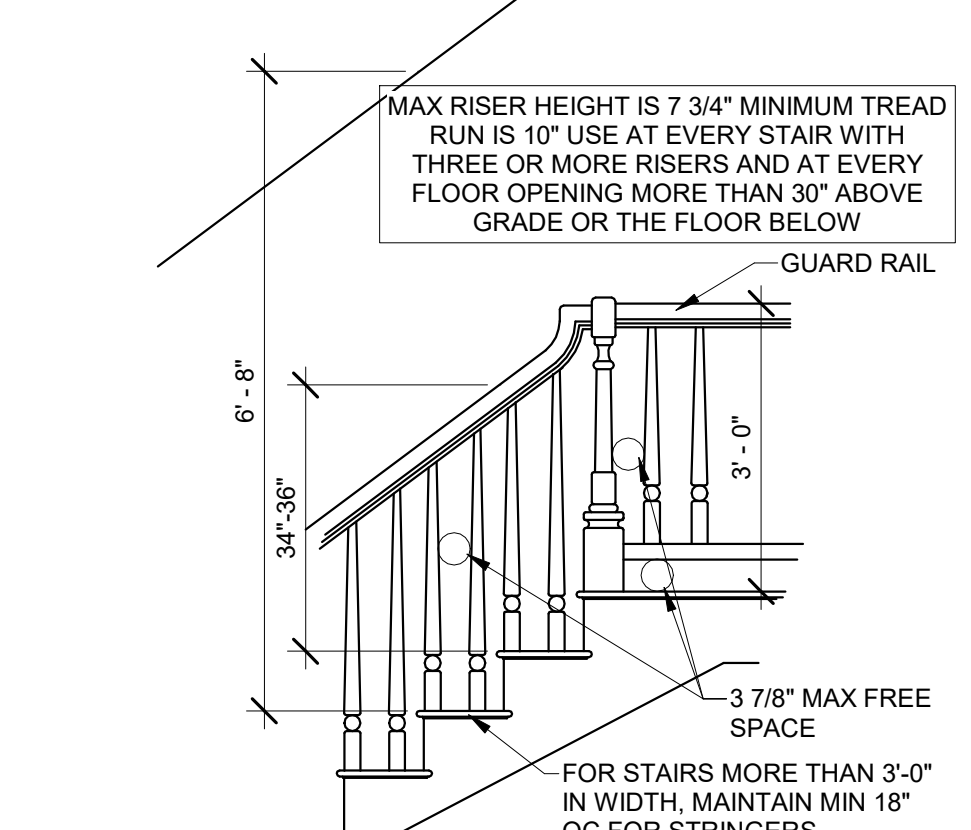


14 VAULTED RAFTER INSULATION
3/4" = 1'-0"

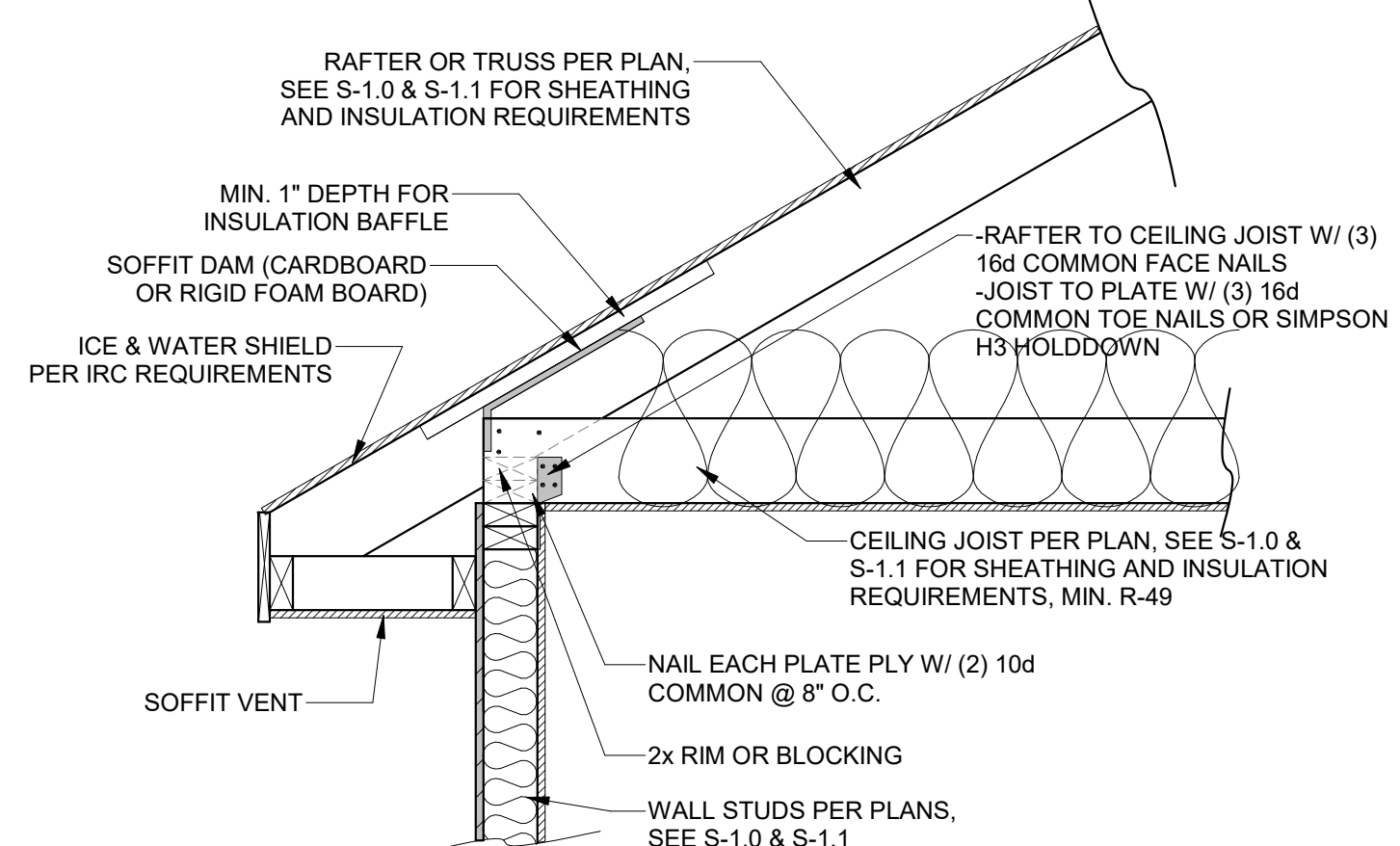
VAULT FURR DOWN SCHEDULE

RAFTER SIZE	R-30C INSULATION (X = 9 1/4")	R-38C INSULATION (X = 11 1/4")
2x6	2x6	2x8
2x8	2x4	2x6
2x10	NOT REQUIRED	2x4
2x12	NOT REQUIRED	2x2

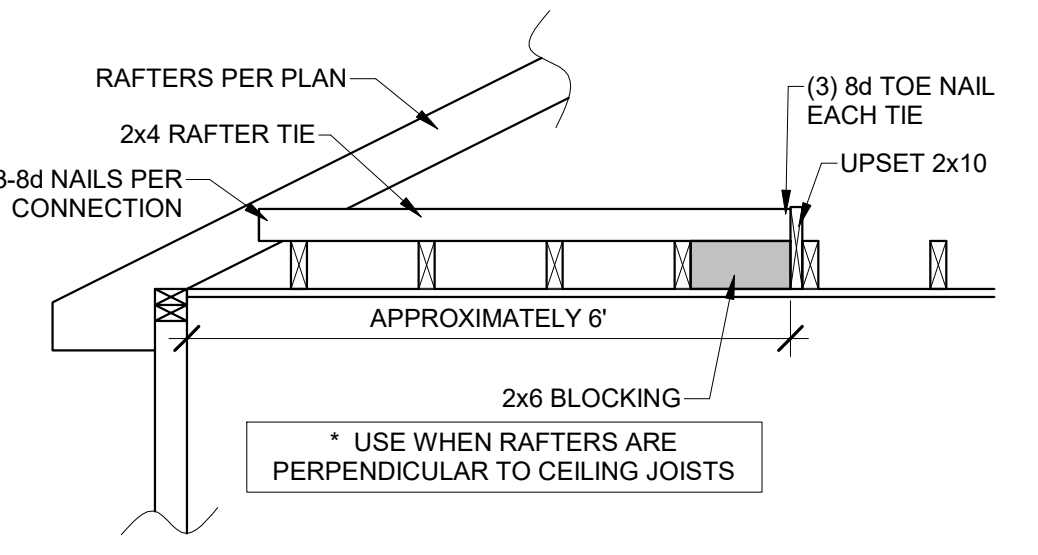
NOTES:
1. ALL VAULTS SHALL BE FURRED DOWN WITH 2x FRAMING TO THE REQUIRED DEPTH OF INSULATION, PLUS 1" AIR SPACE.
2. R-38C REQUIRED = 11" WITH AIR SPACE.
3. ALL VAULTED RAFTERS SHALL BE MIN. #2 2x6 DFL @ 16" O.C. OR PER ROOF PLAN.



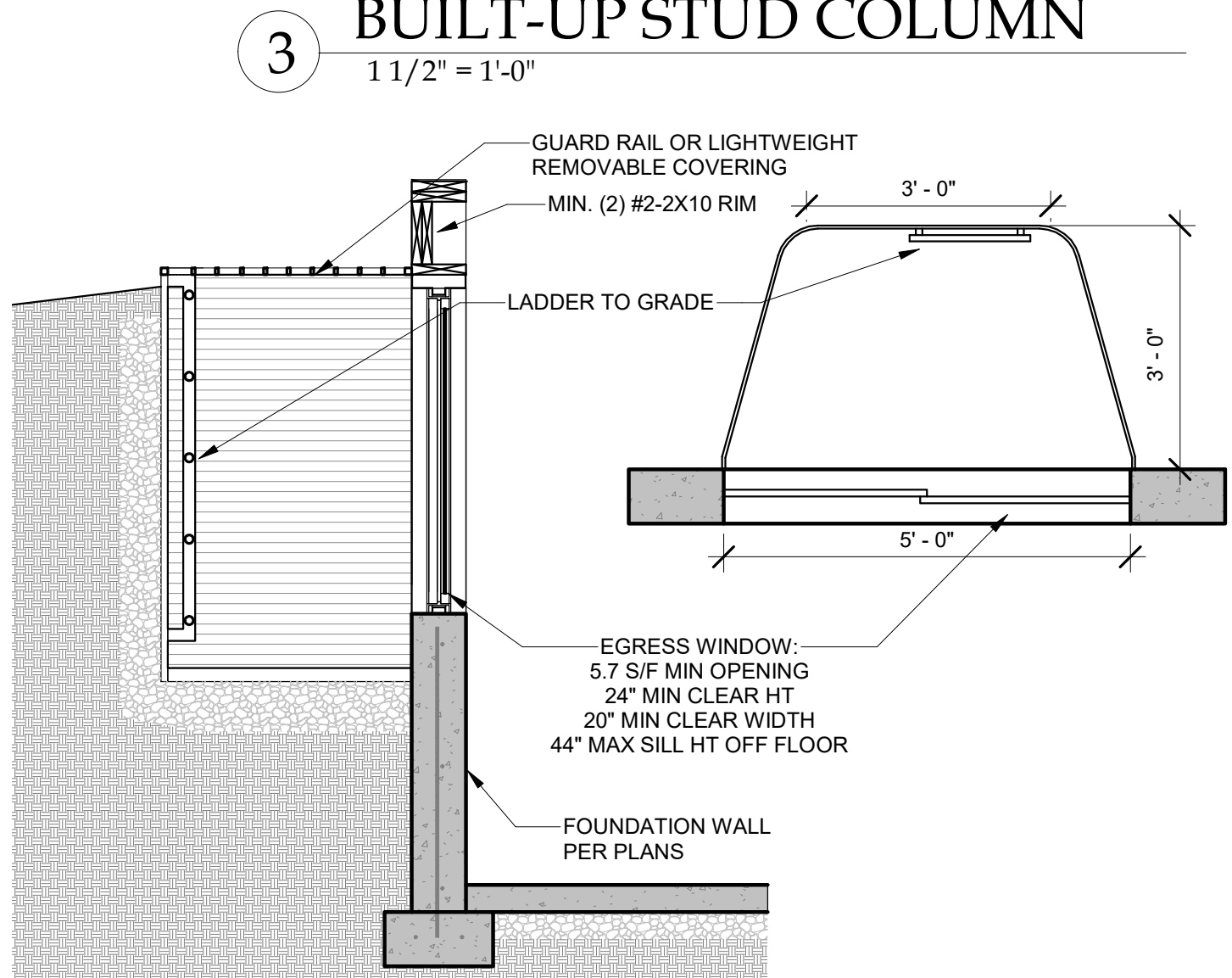
4 STAIR/RAIL DETAIL
1/2" = 1'-0"



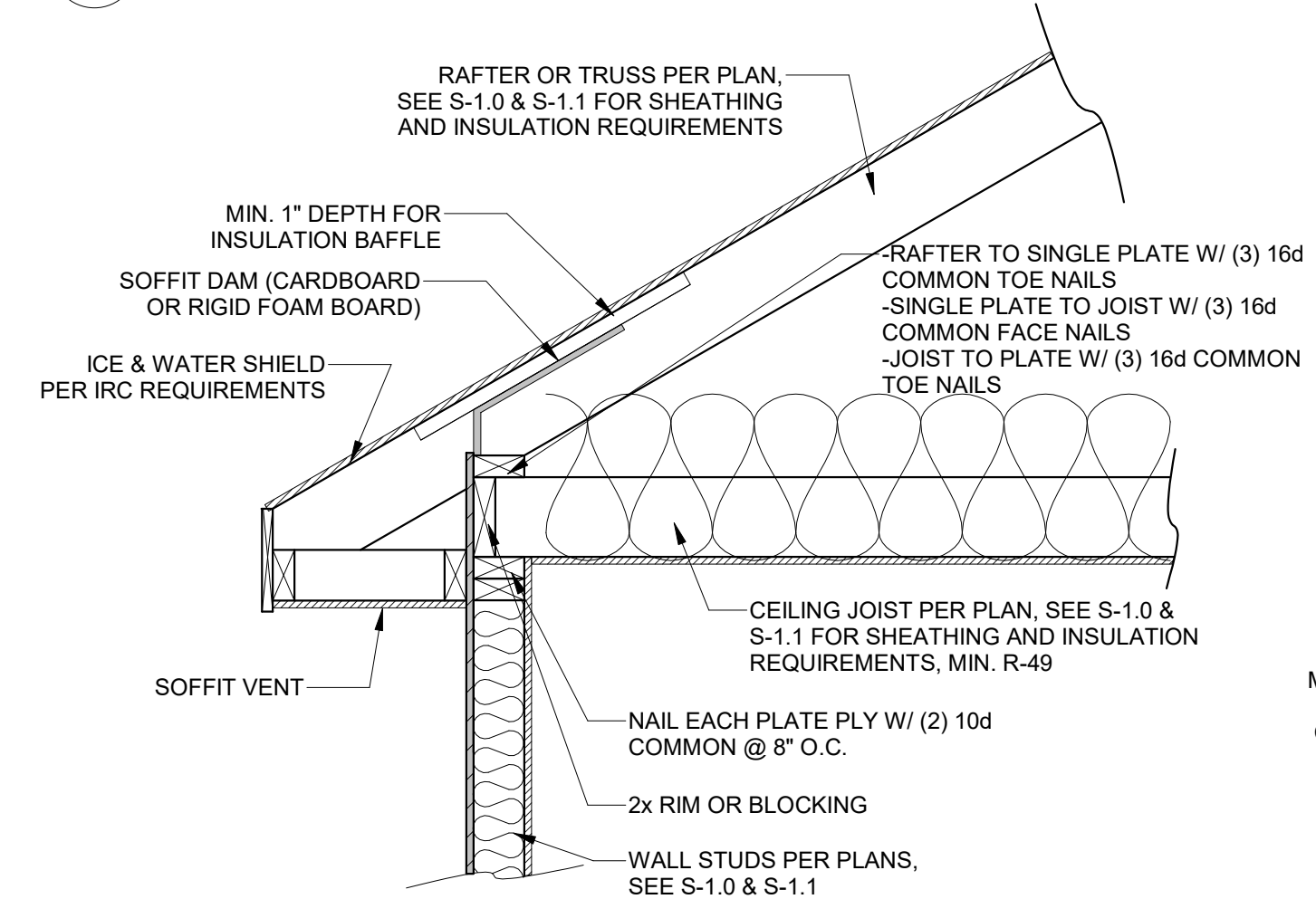
6 OPTION 3 RAFTER BEARING
1" = 1'-0"



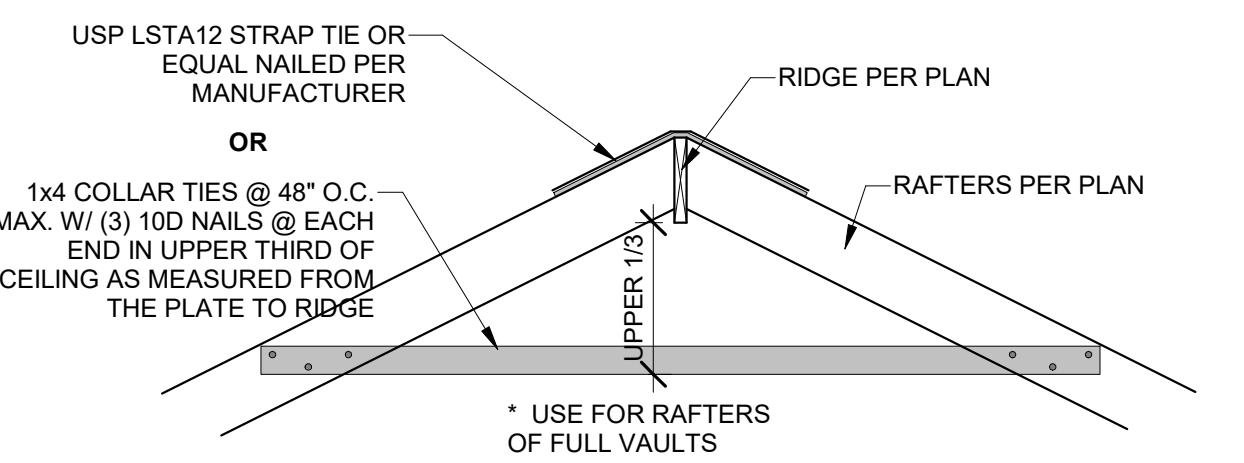
12 RAFTER TIE CONNECTION
1/2" = 1'-0"



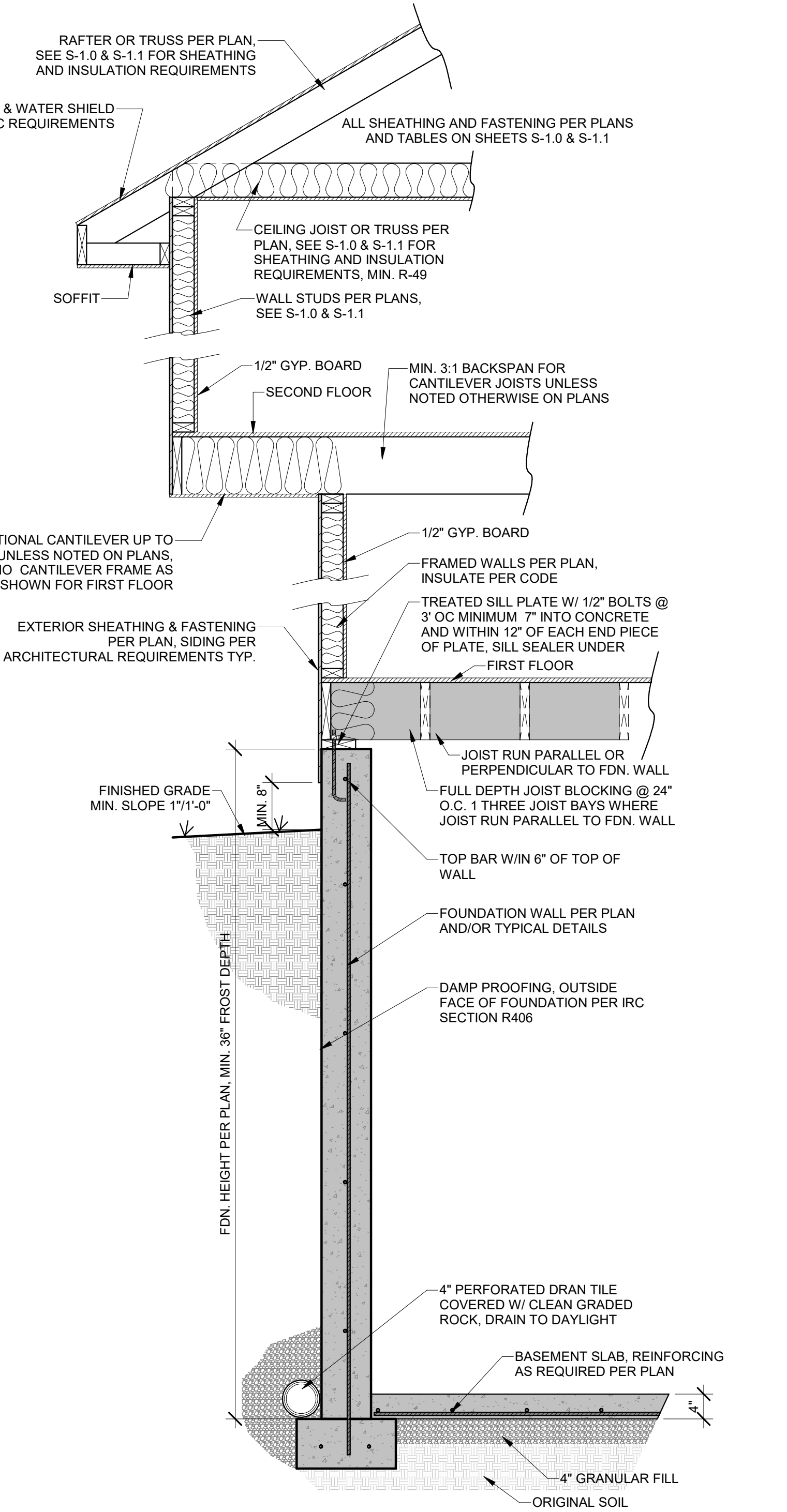
3 BUILT-UP STUD COLUMN
1 1/2" = 1'-0"



5 OPTION 2 RAFTER BEARING
1" = 1'-0"
THIS OPTION NOT AVAILABLE IN KC, MO



13 RIDGE SUPPORT
1/2" = 1'-0"

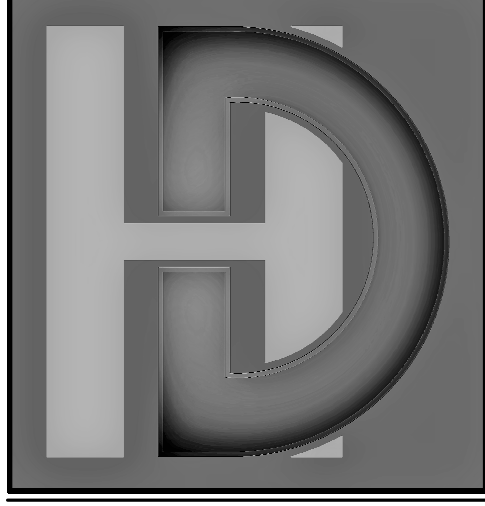


1 TYPICAL WALL SECTION
3/4" = 1'-0"

2 EGRESS WINDOW SECTION
1/2" = 1'-0"

DUE TO THE WIDE VARIETY OF SOIL CONDITIONS IN OUR AREA AND THE WIDE VARIETY OF PLASTICITY INDEX AND SOIL BEARING CAPACITIES OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY HD ENGINEERING OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF ANY "STANDARD" FOUNDATIONS.

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STARR HOMES, LLC
TYLER & ERIN RESIDENCE - LOT 3A & 4E
PROMISED VIEW DR, LEE'S SUMMIT, MO

STRUCTURAL DETAILS & NOTES

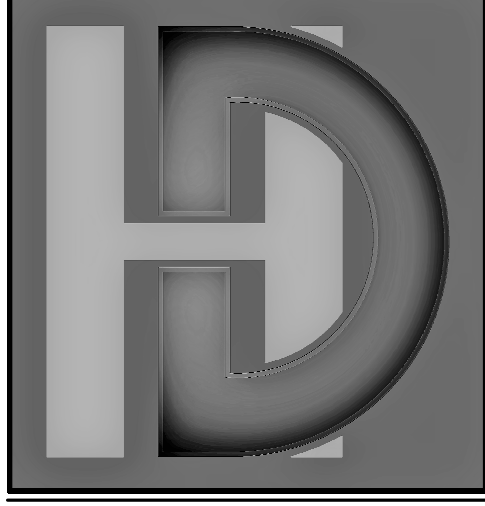
HD#: 42639

DATE: 10/11/2021
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FRAMING SECTIONS

S-1.2



STARR HOMES, LLC
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 PROMISED VIEW DR, LEE'S SUMMIT, MO

STRUCTURAL DETAILS & NOTES

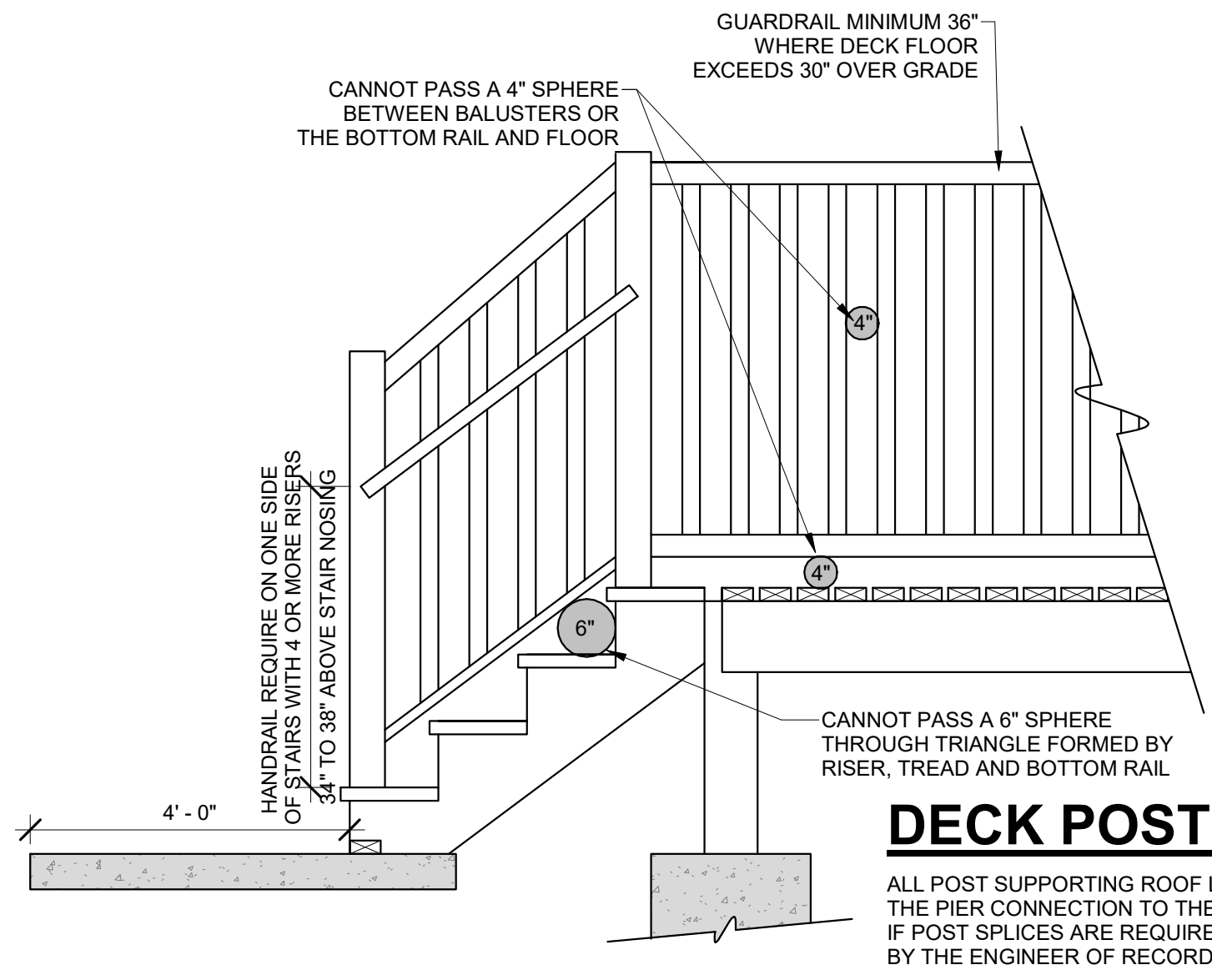
HD#: 42639

DATE: 10/11/2021
 CHECKED BY: CLS

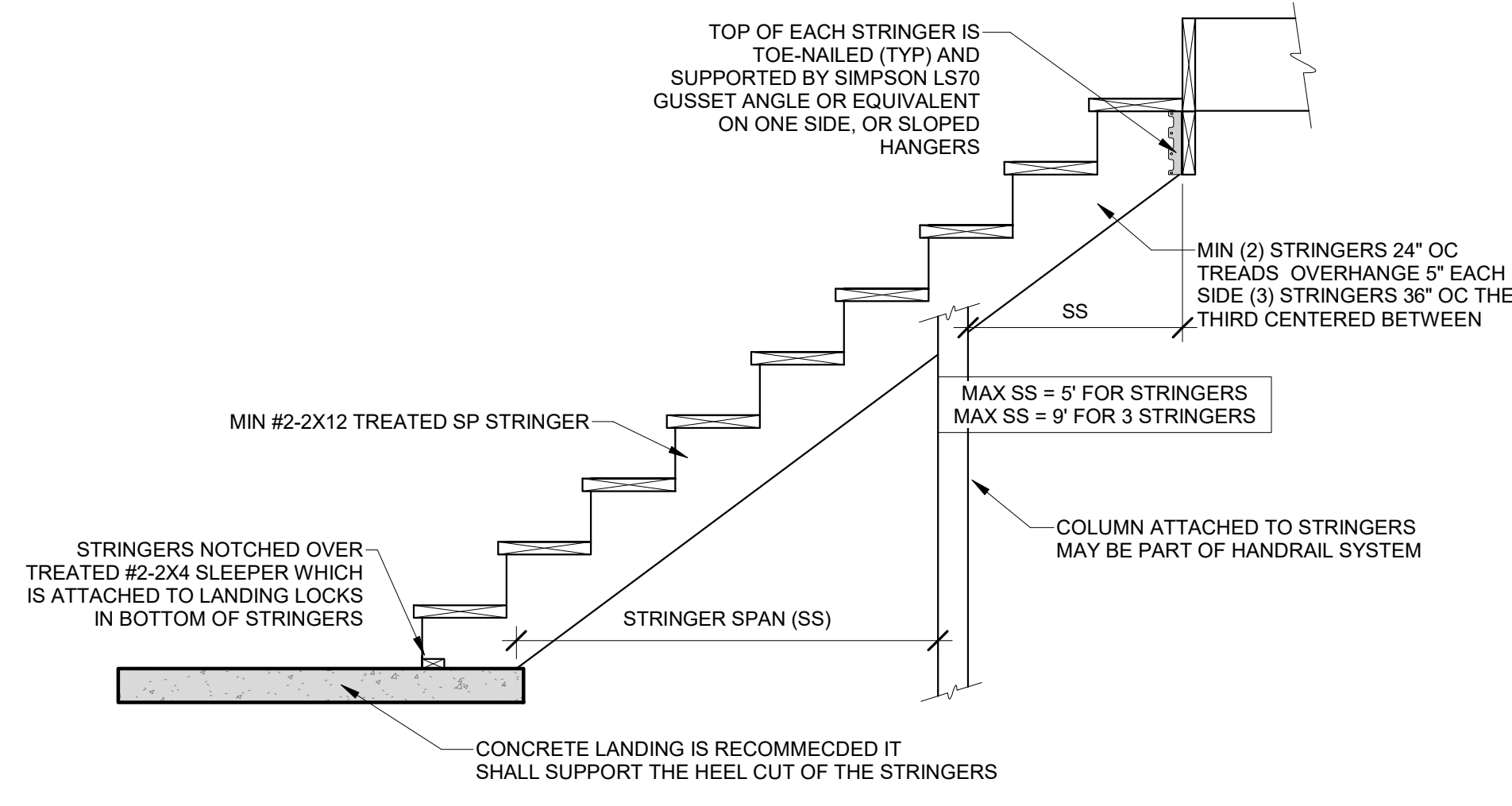
NO.	ISSUE/REVISION	Revision Date

DECK DETAILS

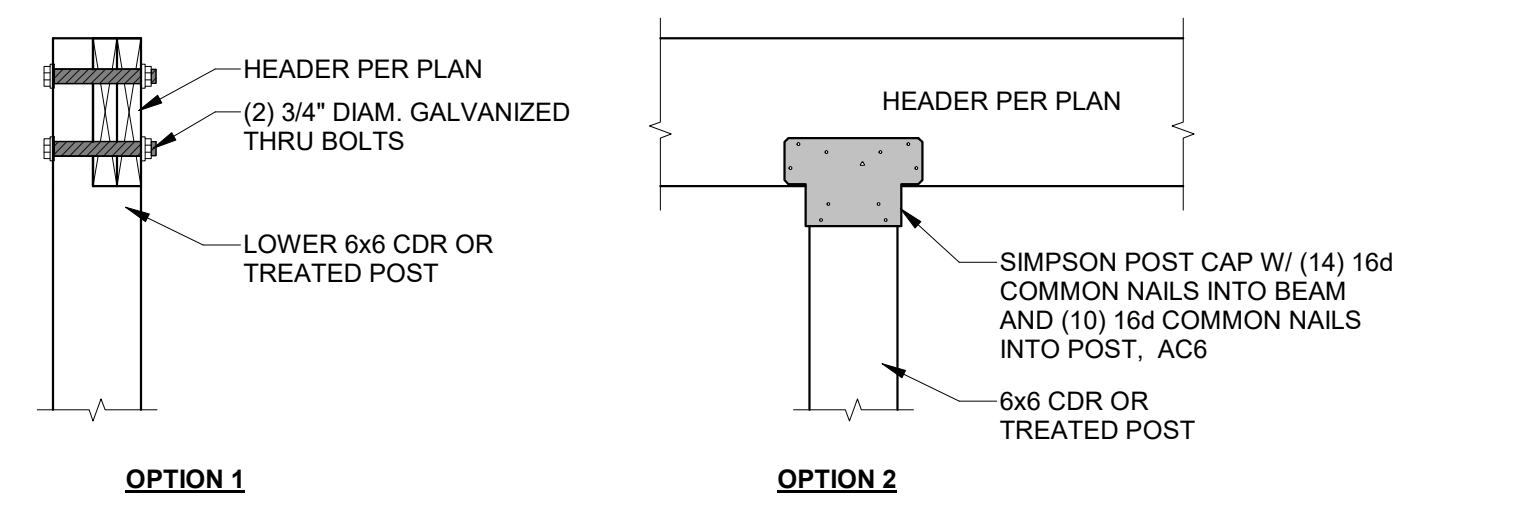
S-1.3



8 GUARD RAIL
 1/2" = 1'-0"



9 STAIR STRINGER DETAIL
 1/2" = 1'-0"



7 ROOF LEVEL INTERIOR BEAM TO COLUMN
 1" = 1'-0"

TABLE IRC2018 R507.9.1.3(1)
DECK LEDGER CONNECTION TO BAND JOIST^{a,b}
 (DECK LIVE LOAD = 40 PSF, DECK HEAD LOAD = 10 PSF, SNOW LOAD ≤ 40 PSF)

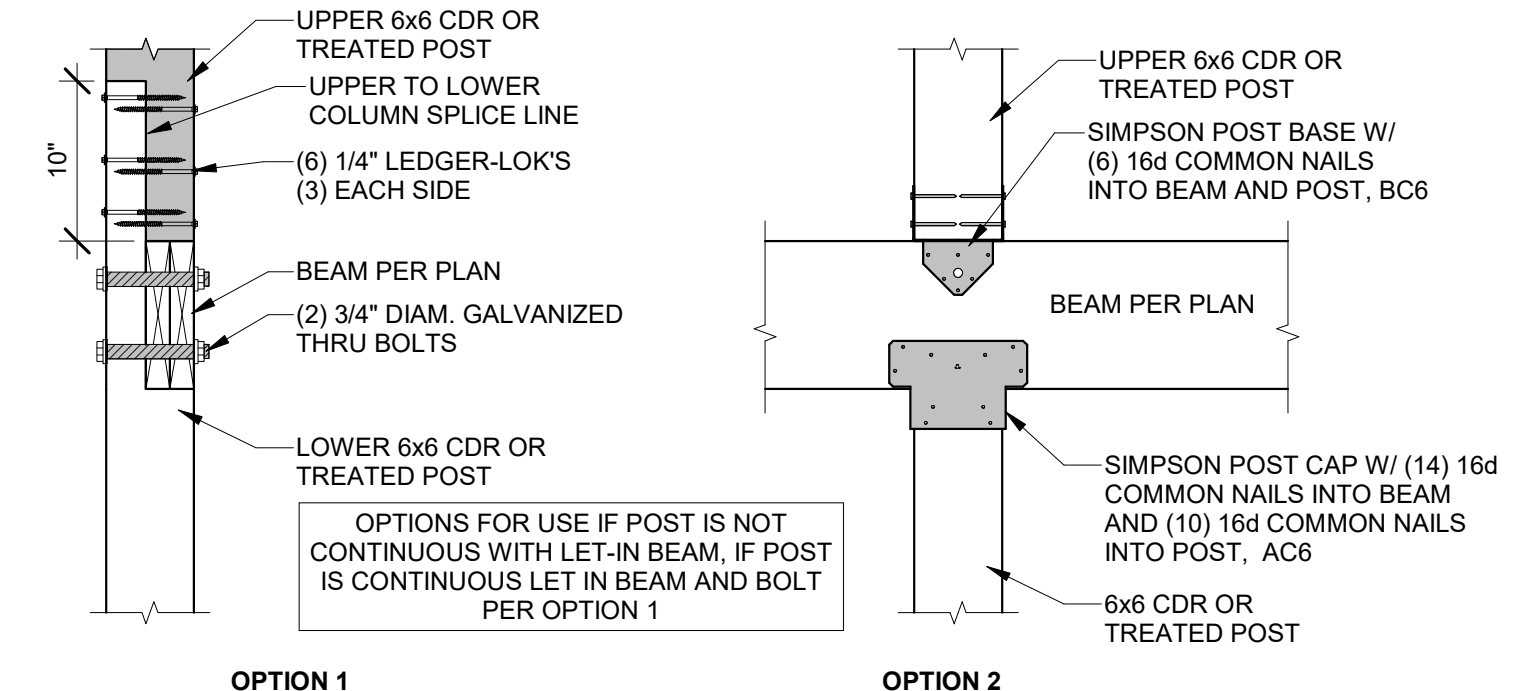
JOIST SPAN	6' AND LESS	6'-1" TO 8'	8'-1" TO 10'	10'-1" TO 12'	12'-1" TO 14'	14'-1" TO 16'	16'-1" TO 18'
CONNECTION DETAILS	ON-CENTER SPACING OF FASTENERS ^{c,d}						
1/2" LAG SCREW WITH 15/32" MAX. SHEATHING ^{c,d}	30	23	18	15	13	11	10
1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING ^d	36	36	34	29	24	21	19
1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING & 1/2" STACKED WASHERS ^e	36	36	29	24	21	18	16

For SI: 1 inch = 25.4mm, 1 foot = 304.8mm, 1 pound per square foot = 0.0479 kPa
 a. Ledges shall be flashed in accordance with Section R703.4 to prevent water from contacting the house band joist.
 b. Snow load shall not be assumed to act concurrently with live load.
 c. The tip of the lag screw shall fully extend beyond the inside face of the band joist.
 d. Sheathing shall be wood structural panel or solid sawn lumber.
 e. Sheathing shall be permitted to be wood structural panel, gypsum board, fiberboard lumber or foam sheathing. Up to 1/2" thickness of stacked washers shall be permitted to substitute for you to 1/2" of allowable sheathing thickness where combined with wood structural panel or lumbers sheathing.

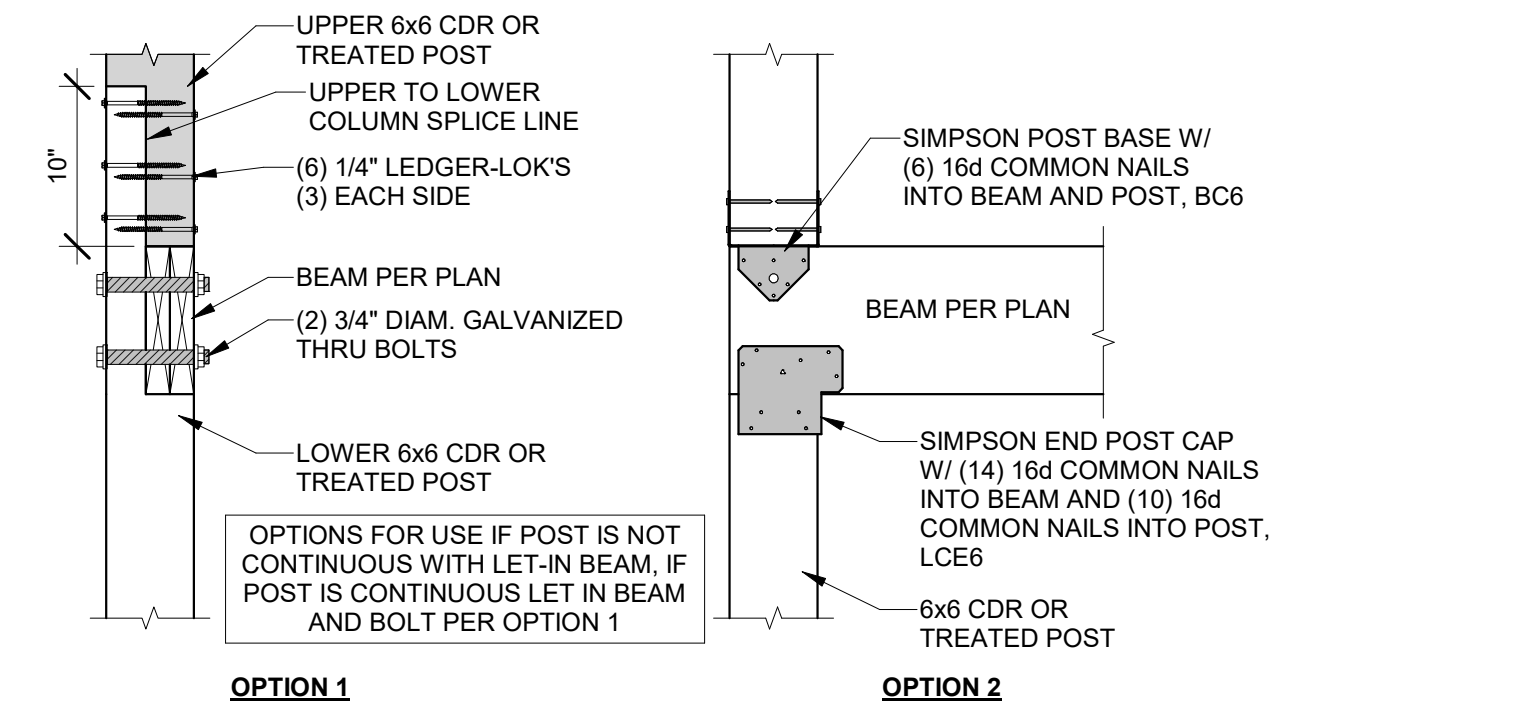
TABLE IRC2018 R507.9.1.3(2)
PLACEMENT OF LAG SCREWS AND BOLT IN DECK LEDGERS AND BAND JOISTS

MINIMUM END AND EDGE DISTANCES AND SPACING BETWEEN ROWS				
	TOP EDGE	BOTTOM EDGE	ENDS	ROW SPACING
LEDGER ^a	2 inches ^d	3/4 inches	2 inches ^b	1 5/8 inches ^b
BAND JOIST ^c	3/4 inches	2 inches	2 inches	1 5/8 inches ^b

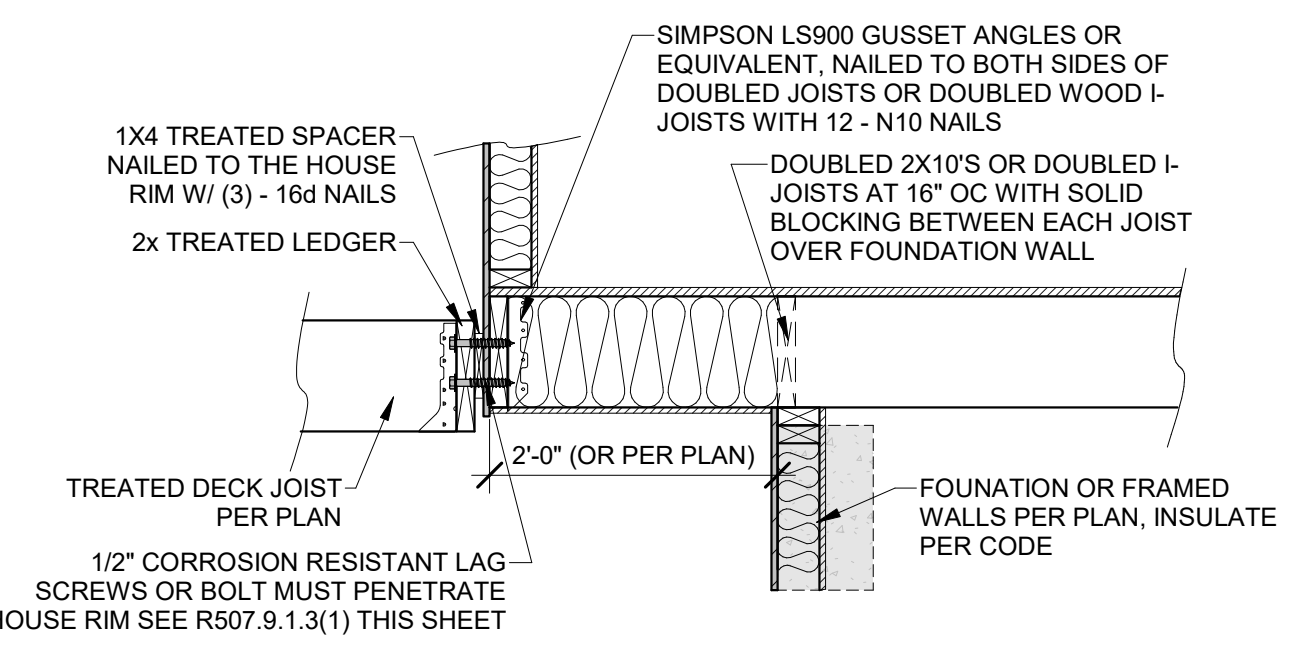
For SI: 1 inch = 25.4mm.
 a. Lag screws or bolts shall be staggered from the top to the bottom along the horizontal run of the deck ledger in accordance with Figure R507.9.1.3(1)
 b. Maximum: 5 inches
 c. For engineered rim joists, the manufacturer's recommendations shall govern.
 d. The minimum distances from bottom row of lag screws or bolts to the top of the ledger shall be in accordance with Figure R507.9.1.3(1)



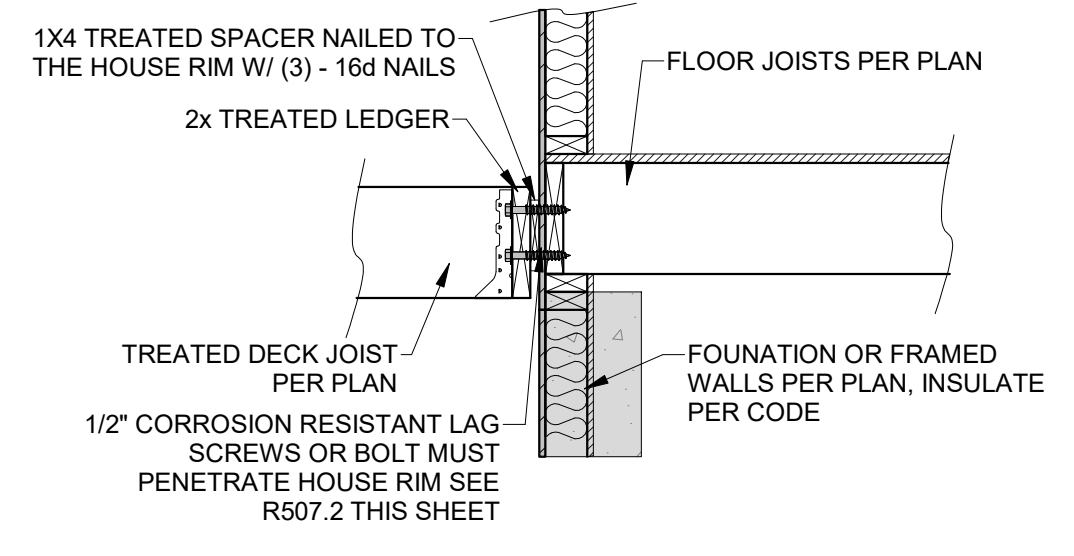
6 DECK LEVEL INTERIOR BEAM TO COLUMN
 1" = 1'-0"



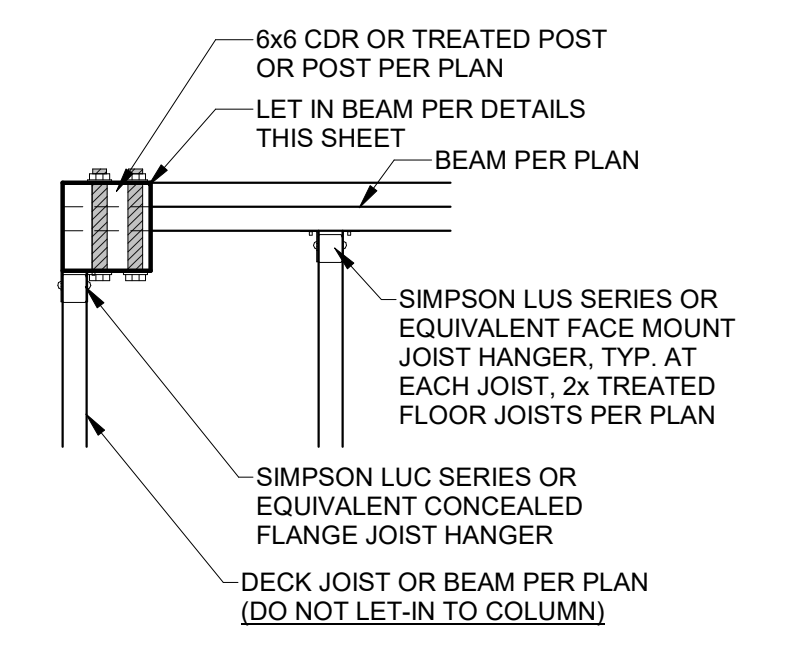
5 DECK LEVEL EXTERIOR BEAM TO COLUMN
 1" = 1'-0"



4 DECK LEDGER TO CANTILEVER
 3/4" = 1'-0"



2 DECK LEDGER ATTACHMENT
 3/4" = 1'-0"



1 DECK CORNER COLUMN
 1" = 1'-0"

RESIDENTIAL SEISMIC & WIND ANALYSIS				INPUT		CALCULATED VALUE	
DETERMINE WEIGHT OF HOUSE:							
LOCATION	DEAD LOAD (psf)	AREA (ft ²)	WEIGHT (lbs.)				
ROOF	10	2800	28000				
CEILING	10	2700	27000				
FIRST FLOOR	10	1500	15000				
FIRST FLOOR EXT. WALL DL	WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT (psf)	WEIGHT (lbs)			
	561.5	10	10	55150			
FIRST FLOOR INT. PARTITION WALL DL	DEAD LOAD (psf)	AREA (ft ²)	WEIGHT (lbs)				
	6	1850	11100				
PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT = 30 FT ASSUMED)							
FRONT-TO-BACK				SIDE-TO-SIDE			
SLOPED ROOF	AREA	LOAD	WEIGHT	SLOPED ROOF	AREA	LOAD	WEIGHT
VERT. ROOF	44	551	29817	0	0	0	0
1ST	1962.62	24568	28917	1070.63	14186	17299	17299
SLOPED ROOF				ZONE C			
WALL/VERT. ROOF	ZONE A	5.9	11.6				
MEAN ROOF HT.	ZONE B	17.4	3.4				
	ZONE D	17	13.6				

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area.
 $q_{w1} = 0.00256K_d K_{e1} V^2$ (ASCE7-10 Velocity Pressure) $q_{w1,adj} = 0.6 q_{w1}$ (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)

1ST FLOOR TRIBUTARY WEIGHT
 S_s (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP)
 F_a (from ASCE7 Table 11.4-1)
 S_{ps} ($= 2.5 \cdot S_s \cdot F_a$)
 R (from ASCE7 Table 12.2-1)

SEISMIC SHEAR			
LOCATION	From ASCE7 (Eq. 12.8-1)	$V = 1.2 \cdot S_{ps} \cdot W / R$ (lbs.)	
1ST FLOOR		1953	

Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable Shear (#/LF)	Code Reference
Exterior (Option #4)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220	AF&PA SDPWS Table 4.3A
Exterior (Option #5)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320	AF&PA SDPWS Table 4.3A
Exterior (Option #6)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410	AF&PA SDPWS Table 4.3A
Interior	1/2" Gypsum Board	No. 6-1 1/2" Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60	per IRC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)	325	

EXTERIOR SHEATHING OPTION FOR FIRST FLOOR		WIDTH OF 1ST STORY (FT.)		DEPTH OF 1ST STORY (FT.)	
4		178.42		97.33	
				30	
				2	

EXTERIOR STRUCTURAL WALL LENGTHS (ft.) & RESISTANCES					
1ST FLOOR	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	WIND
	303.84	85075	58.66	16425	303.84
					119105
					58.66
					22995

ADDITIONAL RESISTANCE REQUIRED			
1ST FLOOR FRONT-TO-BACK	SEISMIC	WIND	
1ST FLOOR SIDE-TO-SIDE	0	0	

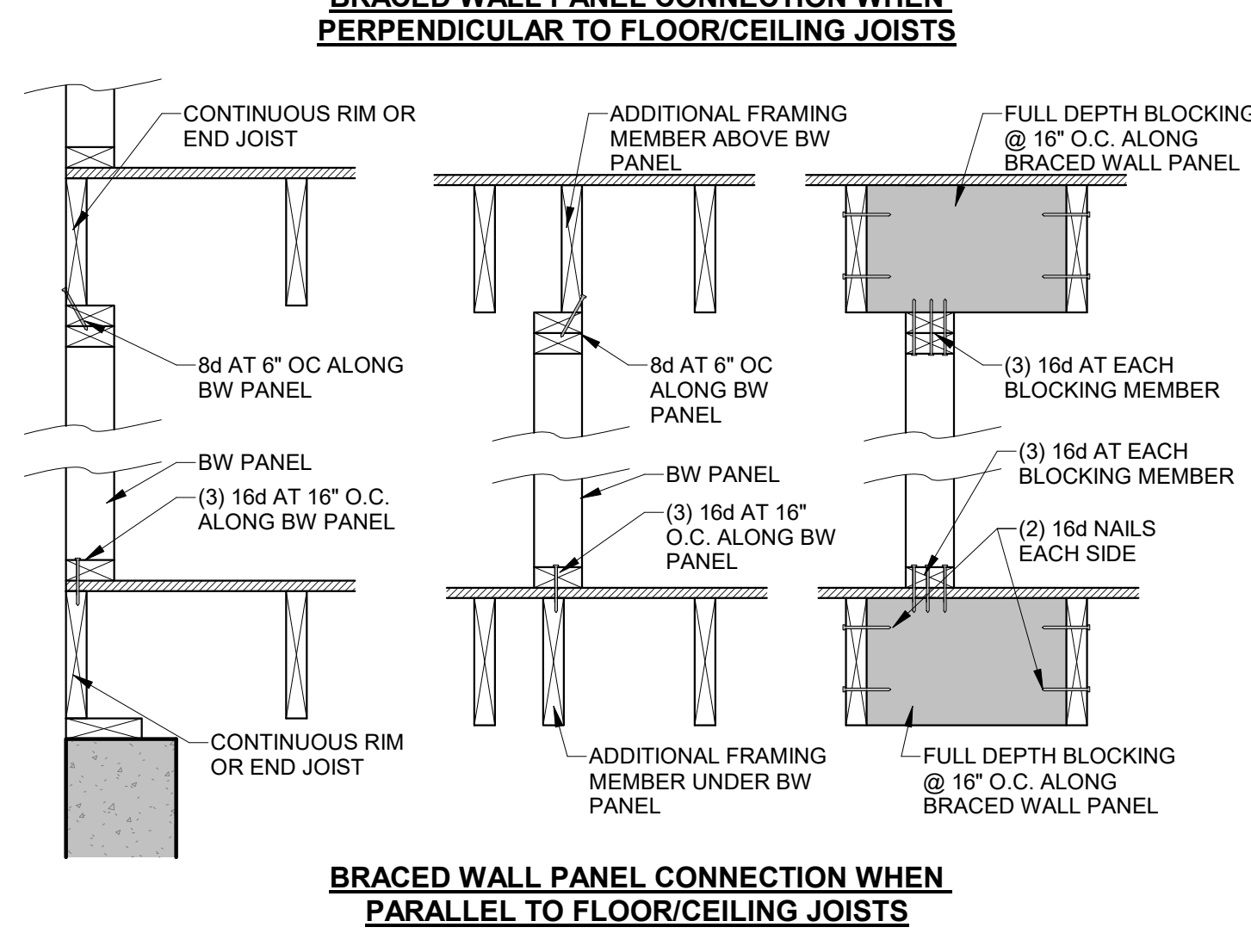
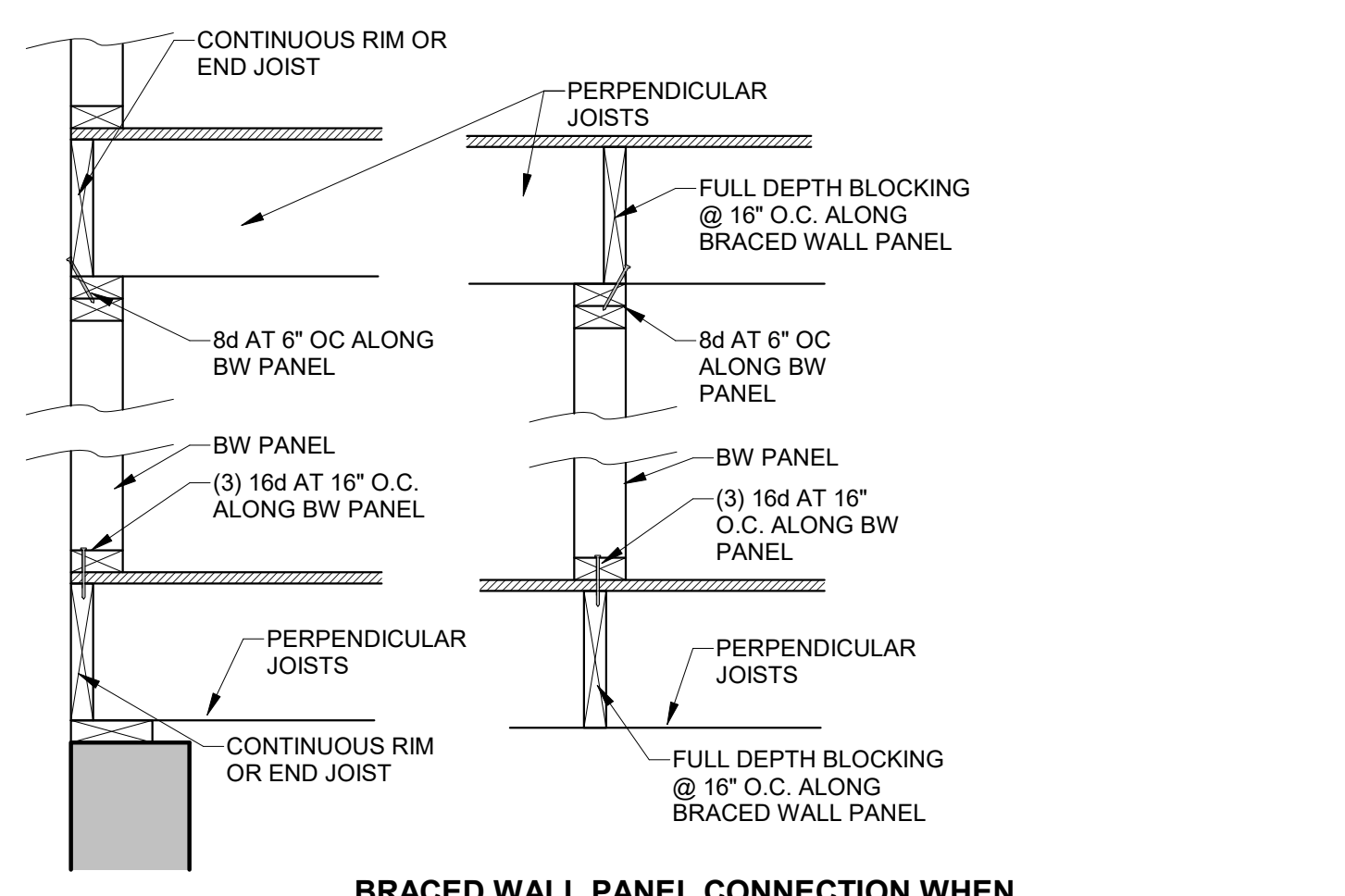
RESISTANCE REQUIRED IN ADDITION TO RESISTANCE PROVIDED BY EXTERIOR WALLS*			
1ST FLOOR FRONT-TO-BACK	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERP. SHEAR WALL RESISTANCE	OK?
1ST FLOOR SIDE-TO-SIDE	0	0	YES

WIND UPLIFT ANALYSIS										
ROOF PITCH (MAX)	X/12	DEGREES	22	PITCH OF 6 OR LESS: EOH-13.3, E-7.2, G-5.2						
OVERHANG	LENGTH (FT.)	PRESSURE (PSF)	18.56	LINAL FT. OF OH	18.56	UPLIFT PER FT. (LBS.)				
MAIN ROOF**	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	17365.6186	18051.0586	15.12	PRESSURE ZN. E (PSF)	10.5	TOTAL FORCE (LBS.)	179172	FORCE PER LINEAL FT. @ PERIMETER (LBS.)
										324.9

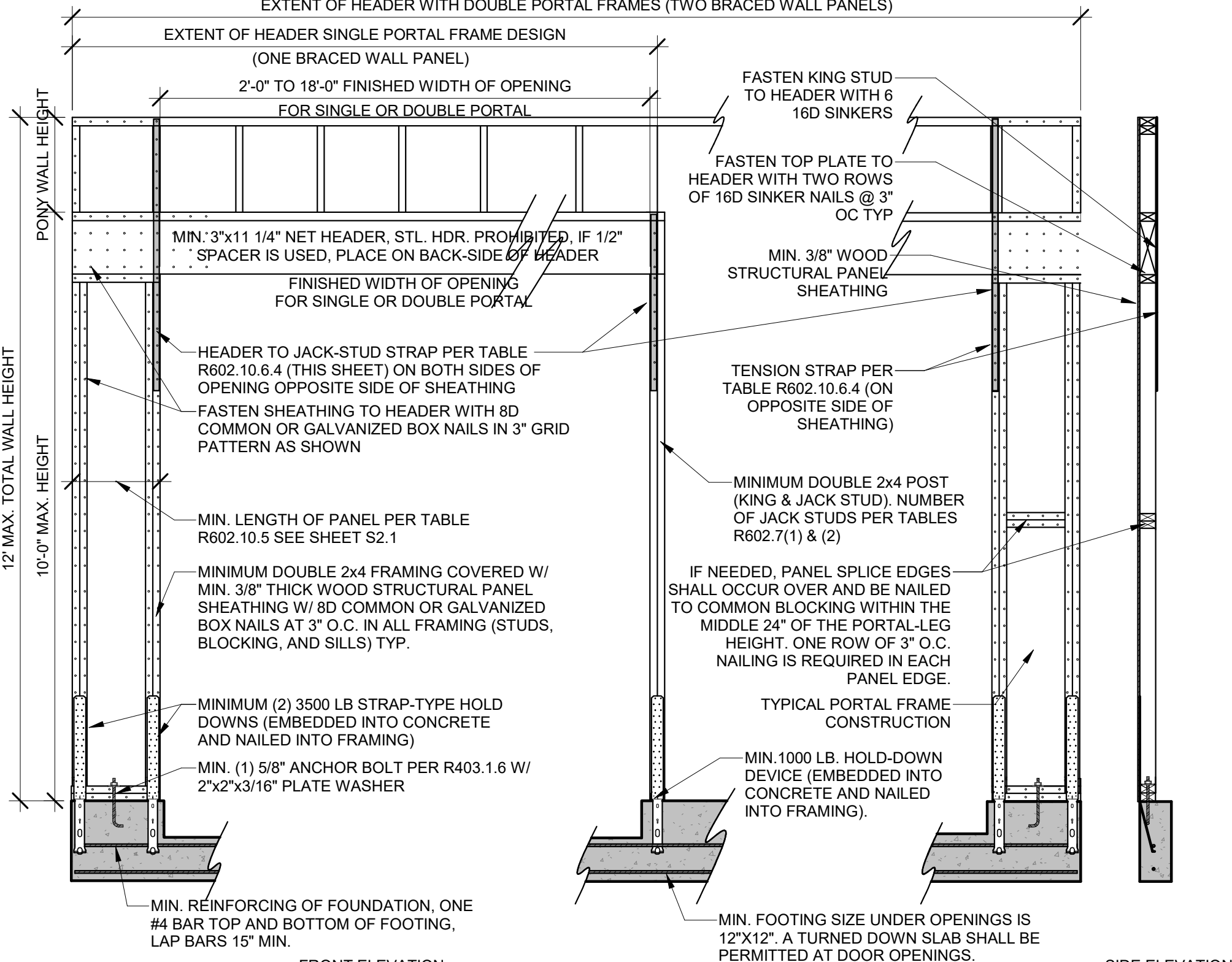
NOTE FOR CONSTRUCTION:
 THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX. UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:
 ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 6'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 220 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC

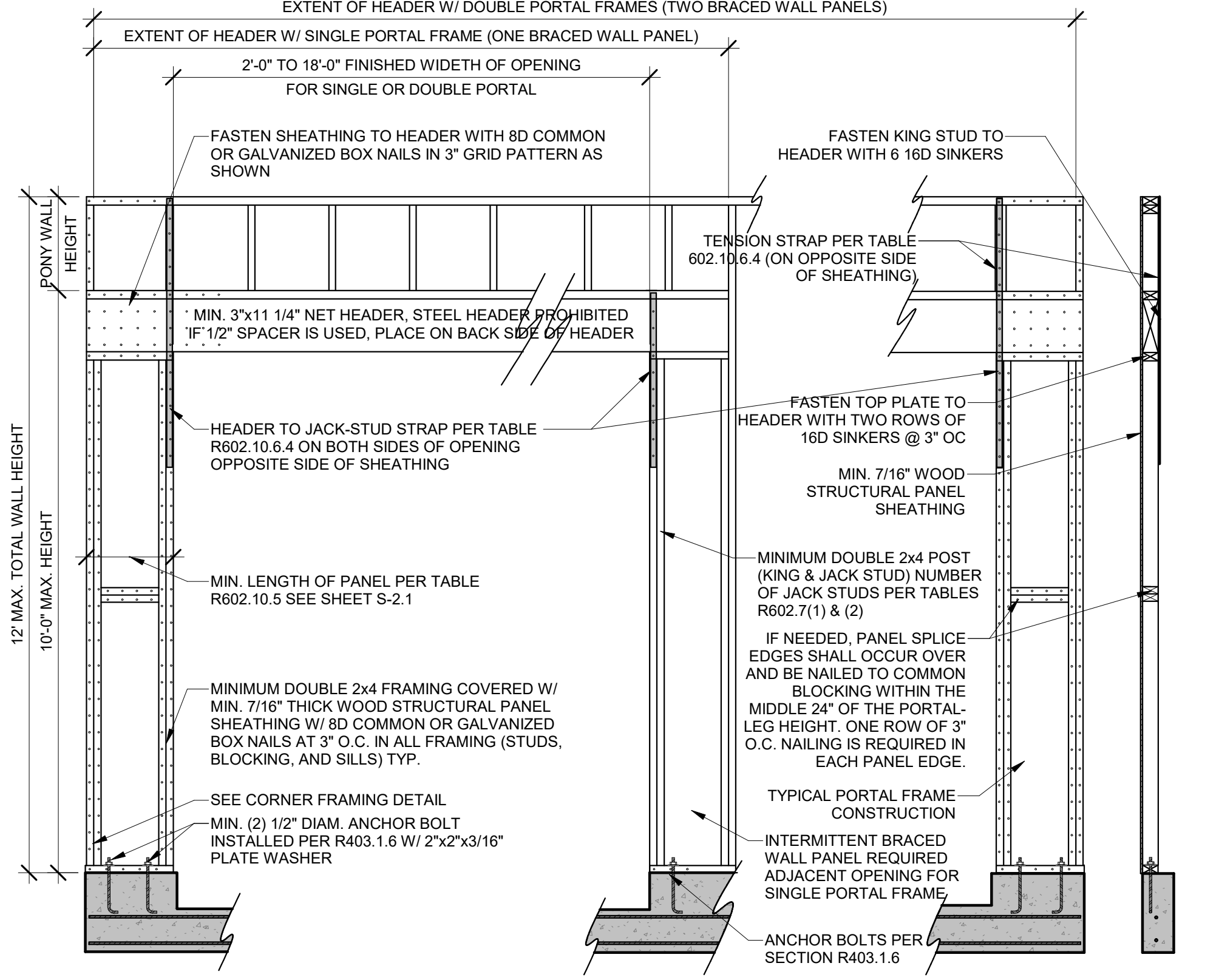
NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION



3 BRACED WALL PANEL CONNECTIONS
 1" = 1'-0"

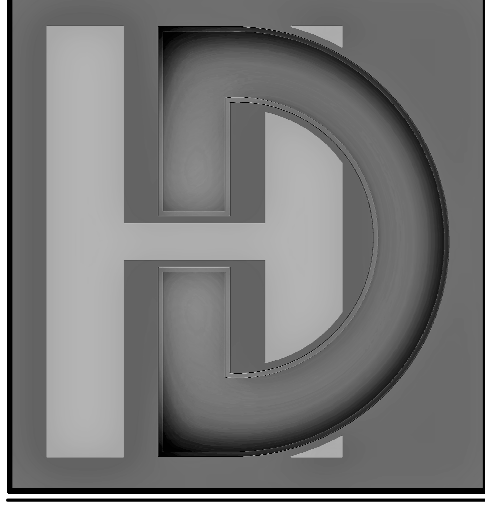


1 PFG PORTAL FRAME W/ HOLD DOWNS (R602.10.6.2)
 1/2" = 1'-0"



2 PFG PORTAL FRAME W/OUT HOLD DOWNS (R602.10.6.3)
 1/2" = 1'-0"

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STRUCTURAL DETAILS & NOTES

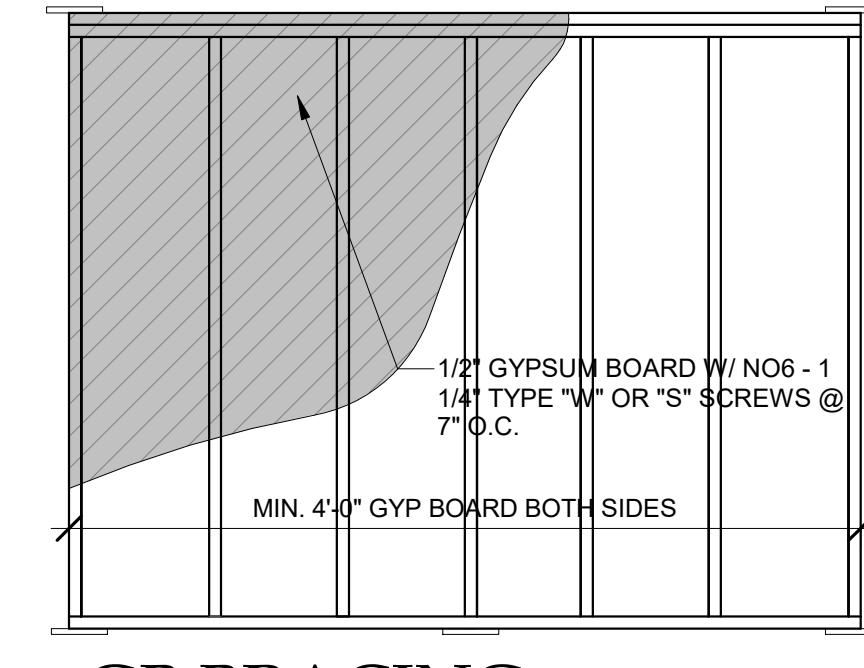
HD#:	42639	
DATE:	10/11/2021	
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NO.	ISSUE/REVISION	Revision Date

BRACED WALL NOTES & DETAILS

S-2.0

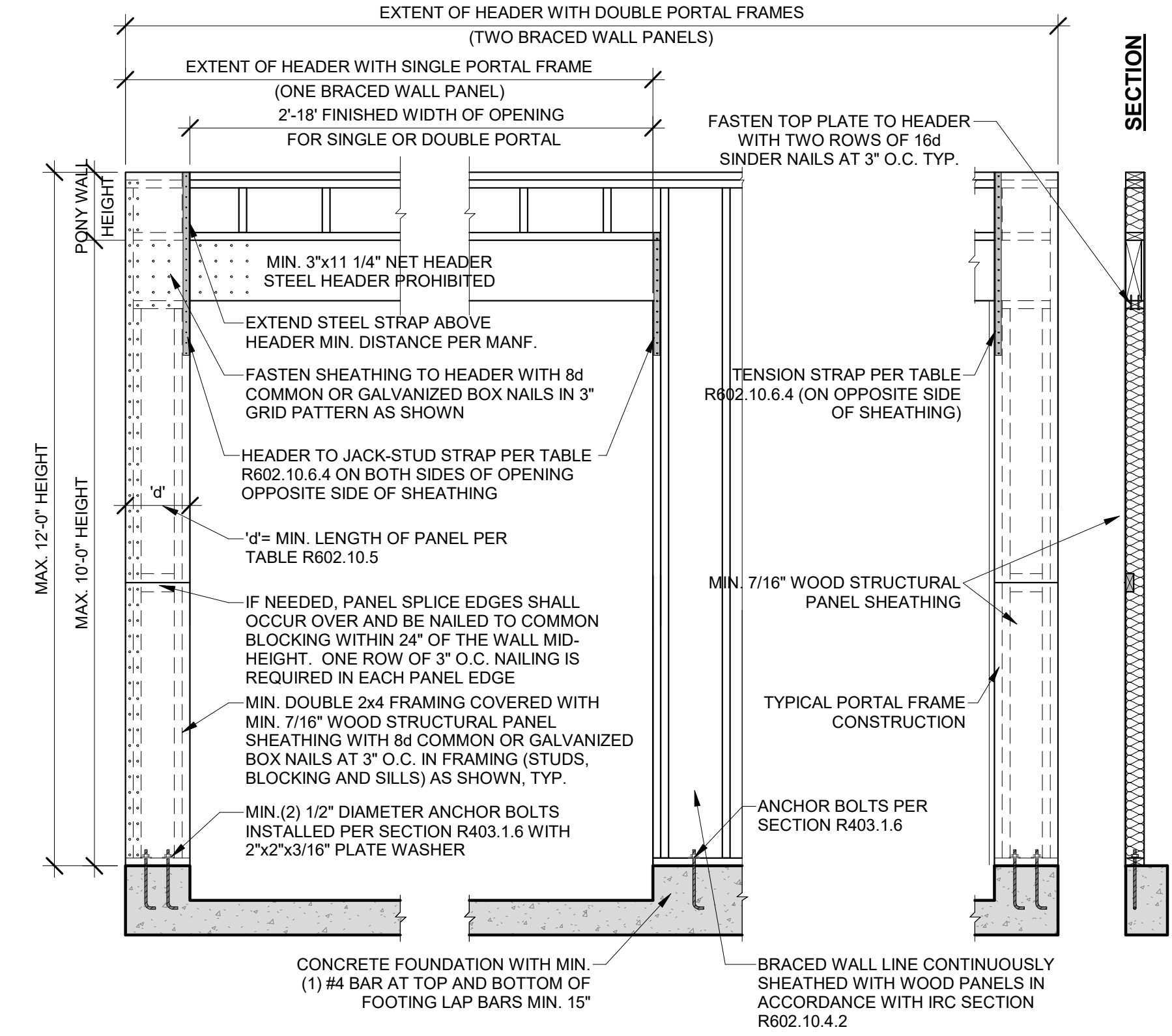
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 RELEASE FOR CONSTRUCTION AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES
 LEE'S SUMMIT, MISSOURI
 08/24/2023

TENSION STRAP CAPACITY REQUIRED FOR RESISTING WIND PRESSURES PERPENDICULAR TO METHOD PFH, PFG AND CS-PF BRACED WALL PANELS IRC2018 TABLE R602.10.6.4

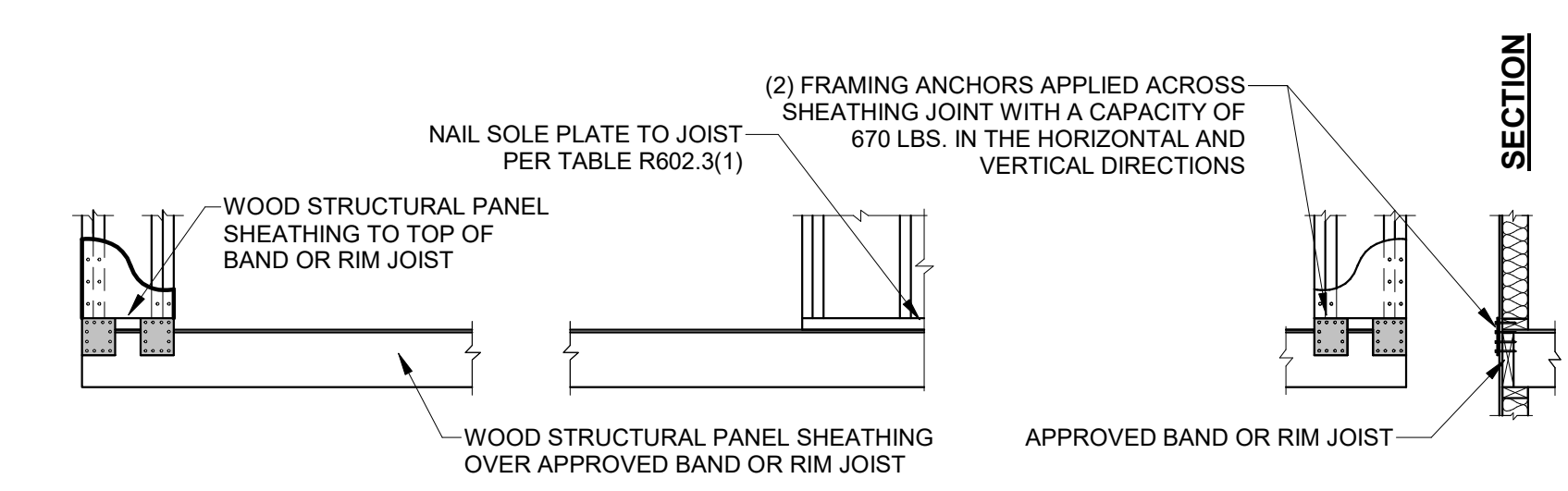


5 GB BRACING
1/2" = 1'-0"

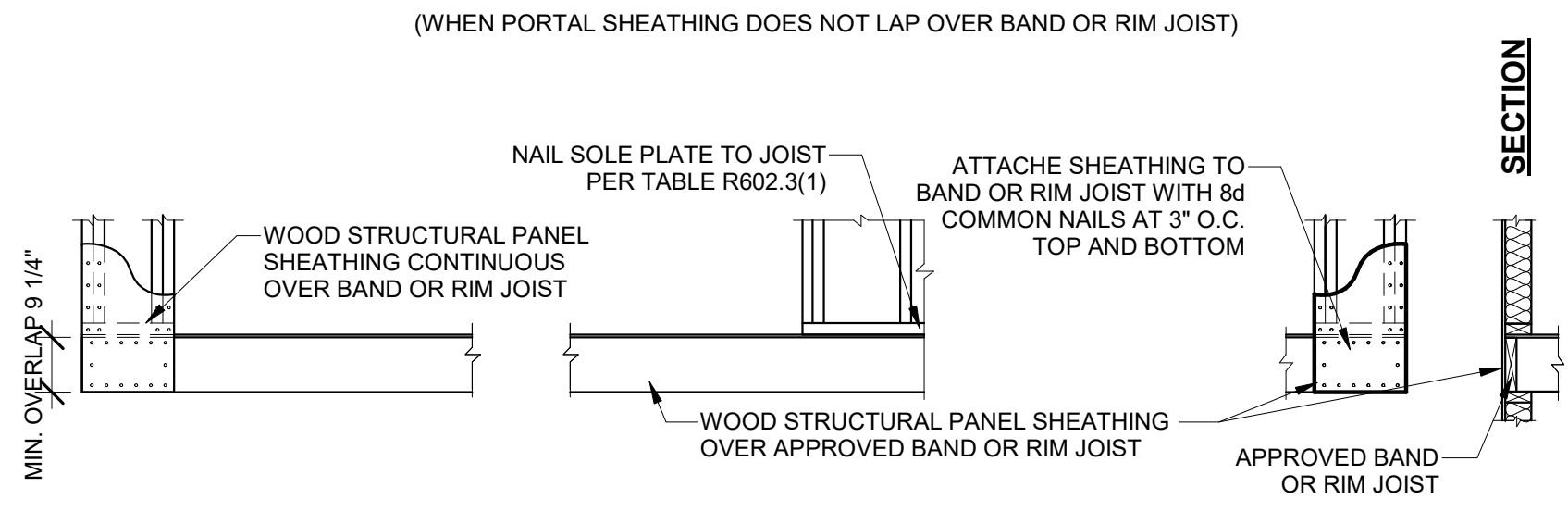
FRONT ELEVATION



OVER CONCRETE OR MASONRY BLOCK FOUNDATION



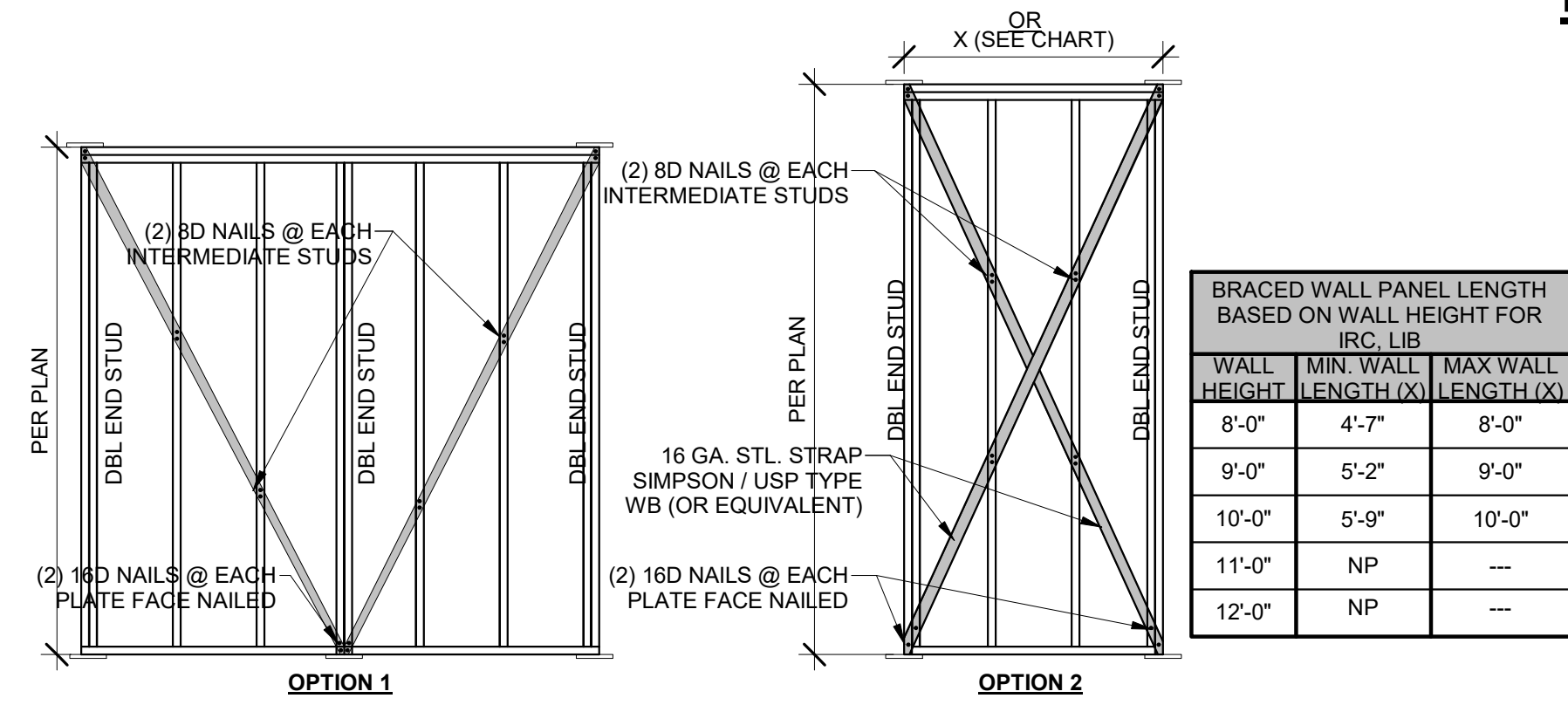
OVER RAISED WOOD FLOOR - FRAMING ANCHOR OPTION



OVER RAISED WOOD FLOOR - OVERLAP OPTION

(WHEN PORTAL SHEATHING LAPS OVER BAND OR RIM JOIST)

4 CS-PF
1/2" = 1'-0"



6 LIB BRACING
3/8" = 1'-0"

TABLE R602.10.5 MINIMUM LENGTH OF BRACED WALL PANELS

METHOD (SEE TABLE R602.10.4)	MINIMUM LENGTH (INCHES) ^a					CONTRIBUTING LENGTH (INCHES)	
	WALL HEIGHT						
	8 FEET	9 FEET	10 FEET	11 FEET	12 FEET		
DWB,WSP,SFB,PBS,PCP,HPS,BV-WSP	48	48	48	53	58	ACTUAL ^b	
GB	48	48	48	53	58	DOUBLE SIDED = ACTUAL SINGLE SIDED = .5xACTUAL	
LIB	55	62	69	NP	NP	ACTUAL ^b	
ABW	SDC A, B, AND C ULTIMATE DESIGN WIND SPEED < 140	28	32	34	38	42	48
	SDC D, D ₁ , D ₂ ULTIMATE DESIGN WIND SPEED < 140	32	32	34	NP	NP	
PFH	SUPPORTING ROOF ONLY	16	16	16	NOTE C	NOTE C	48
	SPTNG. ONE STORY & ROOF	24	24	24	NOTE C	NOTE C	
PFG		24	27	30	NOTE D	NOTE D	1.5 x ACTUAL ^b
CS-G		24	27	30	33	36	ACTUAL ^b
CS-PF		16	18	20	NOTE E	NOTE E	ACTUAL ^b
CS-WSP, CS-SFB	ADJACENT CLEAR OPENING HEIGHT (INCHES)						ACTUAL ^b
	≤64	24	27	30	33	36	
	68	26	27	30	33	36	
	72	27	27	30	33	36	
	76	30	29	30	33	36	
	80	32	30	30	33	36	
	84	35	32	32	33	36	
	88	38	35	33	33	36	
	92	43	37	35	35	36	
	96	48	41	38	36	36	
	100	-	44	40	38	38	
	104	-	49	43	40	39	
	108	-	54	46	43	41	
	112	-	-	50	45	43	
	116	-	-	55	48	45	
	120	-	-	60	52	48	
	124	-	-	-	56	51	
128	-	-	-	61	54		
132	-	-	-	66	58		
136	-	-	-	-	62		
140	-	-	-	-	66		
144	-	-	-	-	72		

a. LINEAR INTERPOLATION SHALL BE PERMITTED.
 b. USE THE ACTUAL LENGTH WHEN IT IS GREATER THAN OR EQUAL TO THE MINIMUM LENGTH.
 c. MAX. HEADER HEIGHT FOR PFH IS 10' IN ACCORDANCE WITH R602.10.6.2. WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.
 d. MAX. OPENING HEIGHT FOR PFG IS 10' IN ACCORDANCE WITH R602.10.6.3. WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.
 e. MAX. OPENING HEIGHT FOR CS-PF IS 10' IN ACCORDANCE WITH R602.10.6.4. WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.

BRACED WALL PRESRIPTIVE METHOD:
CONTINUOUS EXTERIOR SHEATHING (CS-WSP) PER WSP METHOD (BELOW) UNLESS OTHERWISE NOTED ON THE PLAN

EXTERIOR BRACED WALL METHOD: (SEE ON THIS SHEET)
WSP METHOD
WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 3/8" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" O.C. STUD SPACING WITH 6d COMMON NAILS @ 8" O.C. EDGES AND 12" O.C. FIELD OR SHEATHING THICKNESS NOT LESS THAN 7/16" WITH MINIMUM SPAN RATING OF 24/16 FOR 24" O.C. SPACING WITH 8d COMMON NAILS @ 8" O.C. EDGES AND 12" O.C. IN FIELD (NOTE: FRAMING MEMBERS 16" O.C. MAX. UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS).

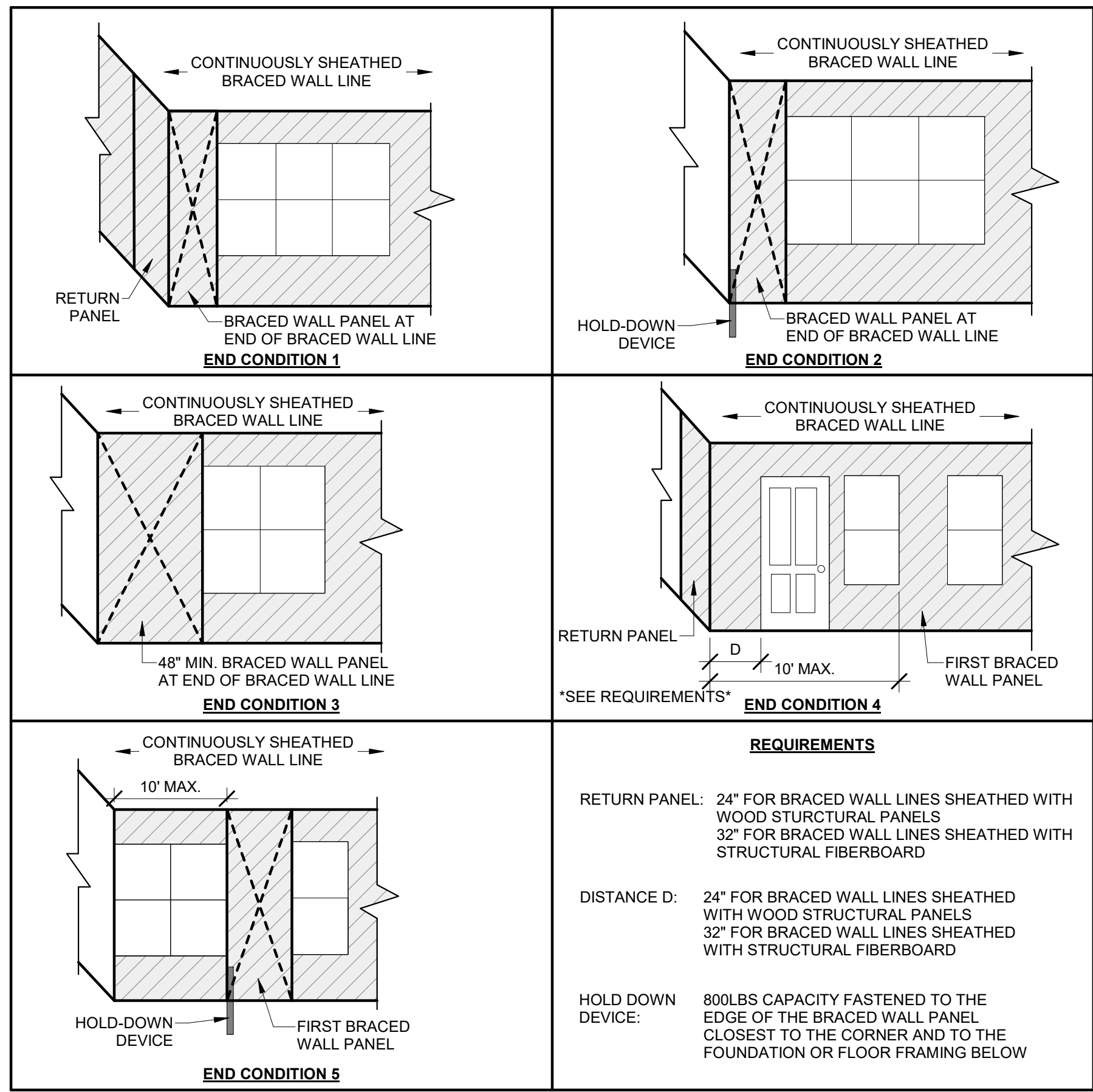
INTERIOR BRACED WALLS (SEE ON THIS SHEET)
GB METHOD:
1/2" MINIMUM GYPSUM BOARD OVER STUDS SPACED @ 24" MAXIMUM FASTENED W/ #6- 1 1/4" TYPE "W" OR "S" DRYWALL SCREWS @ 7" O.C. EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES)
OR
LIB METHOD:
1x4 WOOD FASTENED W/ (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUIVALENT) STL. X-BRACE(S) @ 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUDS FASTENED PER MANUF. SPECS.

MINIMUM WALL STUD FRAMING NOMINAL SIZE & GRADE	MAX. PONY WALL HEIGHT (FEET)	MAX. TOTAL WALL HEIGHT (FEET)	MAX. OPENING WIDTH (FEET)	TENSION STRAP CAPACITY REQUIRED (POUNDS) ^a	
				ULTIMATE DESIGN WIND SPEED V (MPH)	
				115	115
				EXPOSURE B	EXPOSURE C
2X4 NO. 2 GRADE	0	10	18	1,000	1,000
			9	1,000	1,000
			16	1,025	2,500
			18	1,275	2,850
			9	1,000	1,875
			16	2,175	4,125
	2	10	18	2,500	DR
			9	1,500	3,175
			16	3,375	DR
			18	3,975	DR
			9	2,750	DR
			12	3,775	DR
2X6 STUD GRADE	2	12	9	1,000	2,025
			16	2,150	3,675
			18	2,550	DR
			9	1,750	3,125
			16	2,400	DR
			18	3,800	DR
	4	12	9	1,750	3,125
			16	2,400	DR
			18	3,800	DR

a. DR = DESIGN REQUIRED
 b. STRAP SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

END WALL CONDITIONS

FOR CONTINUOUSLY SHEATHED BRACED WALL LINES

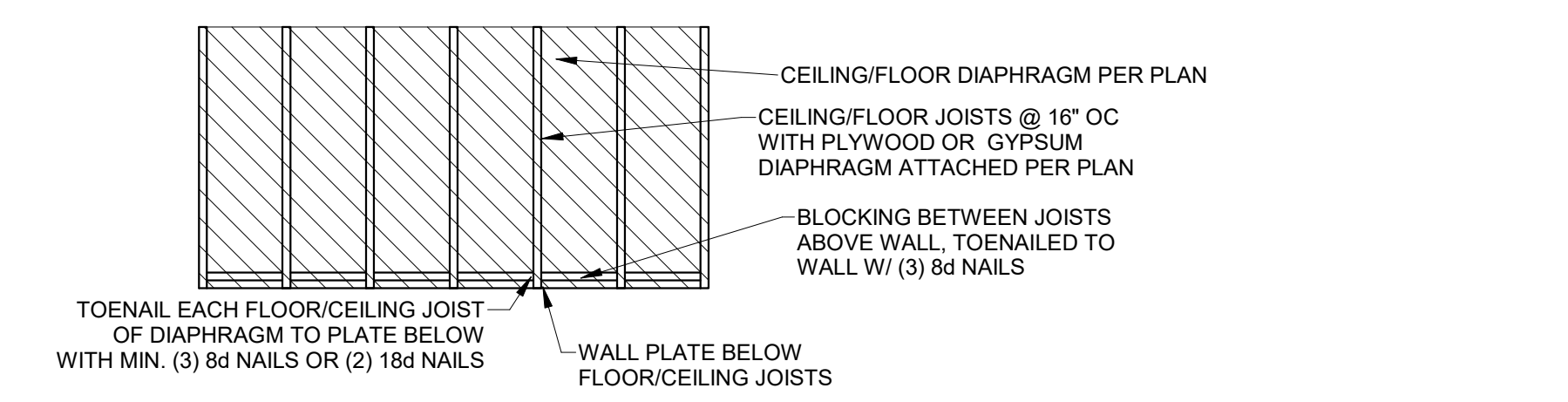


REQUIREMENTS

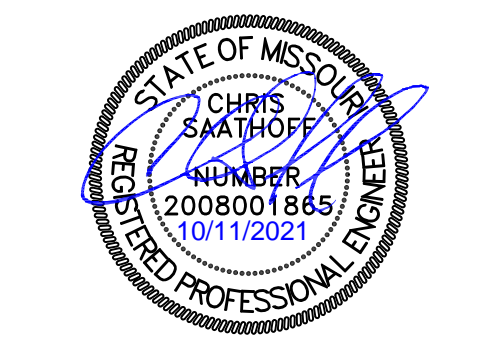
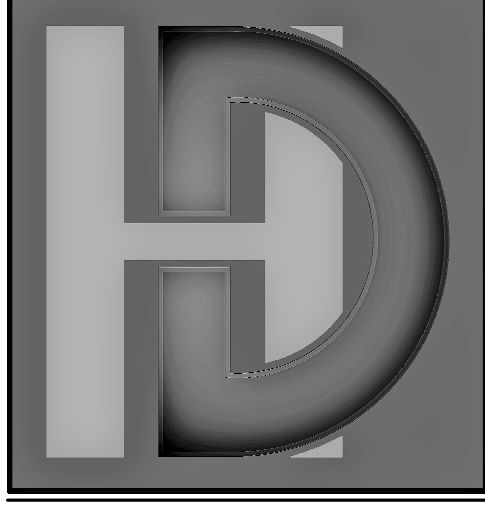
RETURN PANEL: 24" FOR BRACED WALL LINES SHEATHED WITH WOOD STRUCTURAL PANELS
32" FOR BRACED WALL LINES SHEATHED WITH STRUCTURAL FIBERBOARD

DISTANCE D: 24" FOR BRACED WALL LINES SHEATHED WITH WOOD STRUCTURAL PANELS
32" FOR BRACED WALL LINES SHEATHED WITH STRUCTURAL FIBERBOARD

HOLD DOWN DEVICE: 800LBS CAPACITY FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FLOOR FRAMING BELOW



1 DIAPHRAGM CONNECTION TO INTERIOR WALL
3/8" = 1'-0"



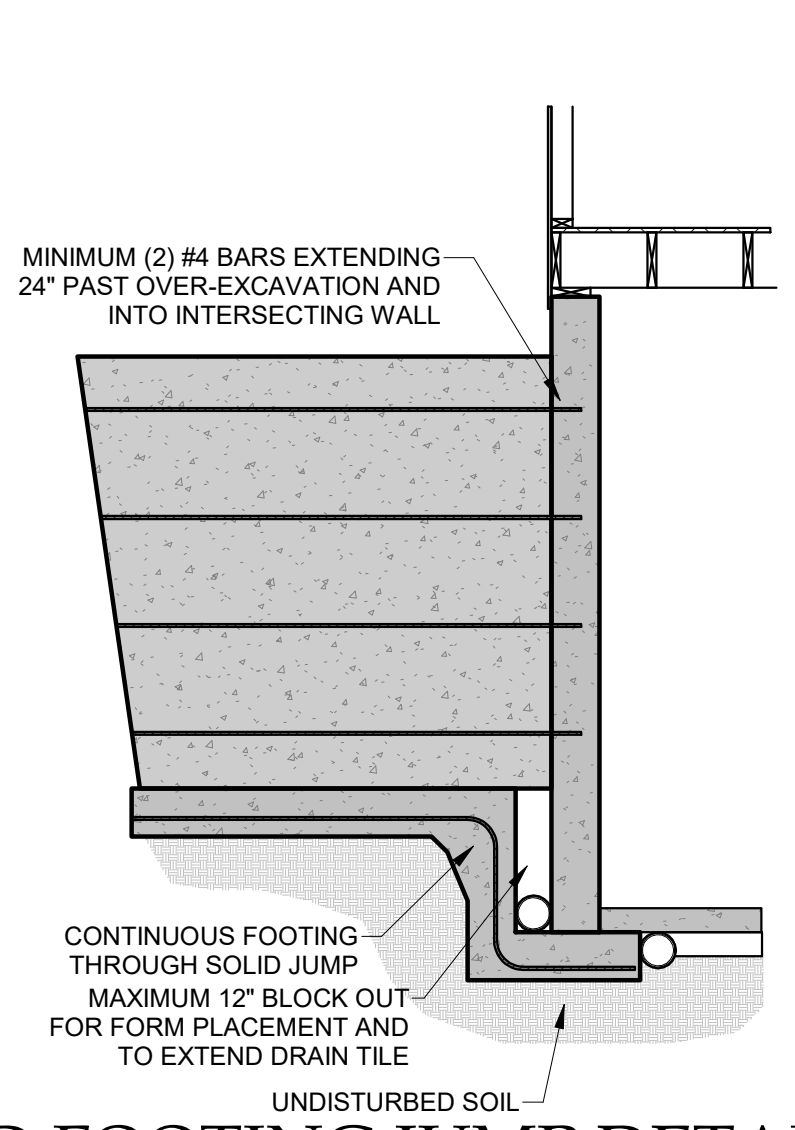
STARR HOMES, LLC
 TYLER & ERIN RESIDENCE - LOT 3A & 4E
 PROMISED VIEW DR, LEE'S SUMMIT, MO
 STRUCTURAL DETAILS & NOTES

HD#: 42639
 DATE: 10/11/2021
 CHECKED BY: CLS

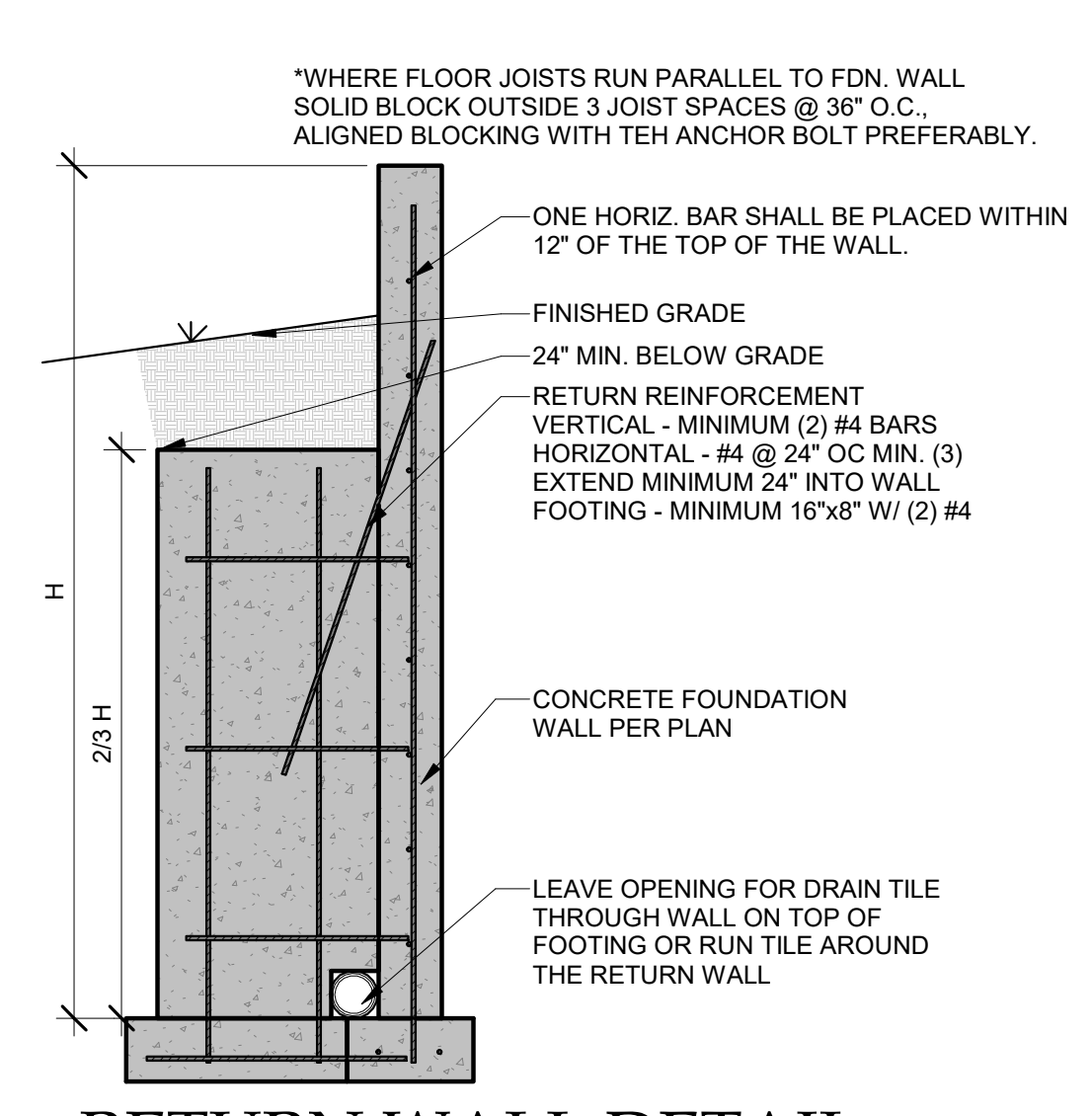
NO.	ISSUE/REVISION	Revision Date

BRACED WALLS NOTES & DETAILS

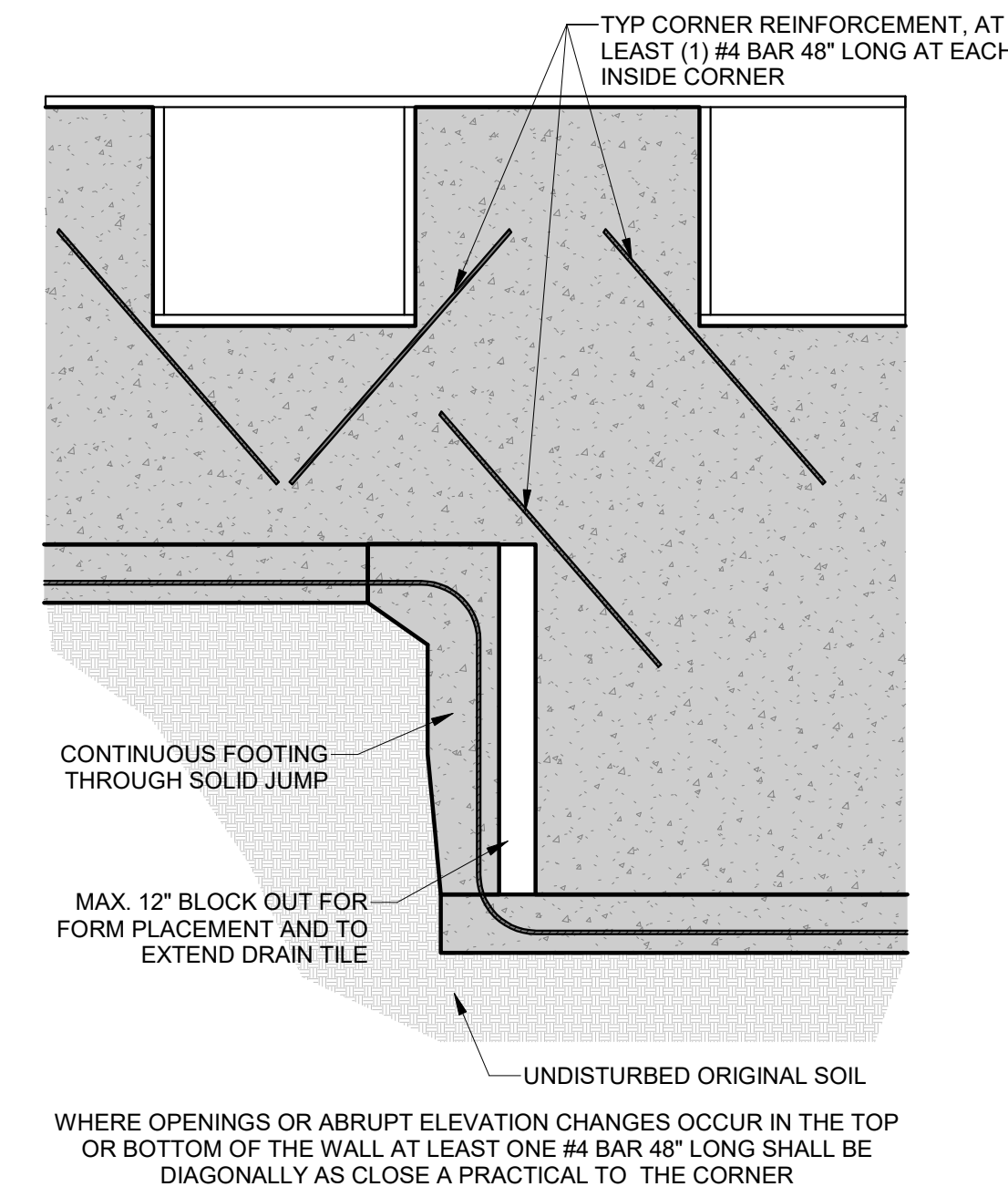
S-2.1



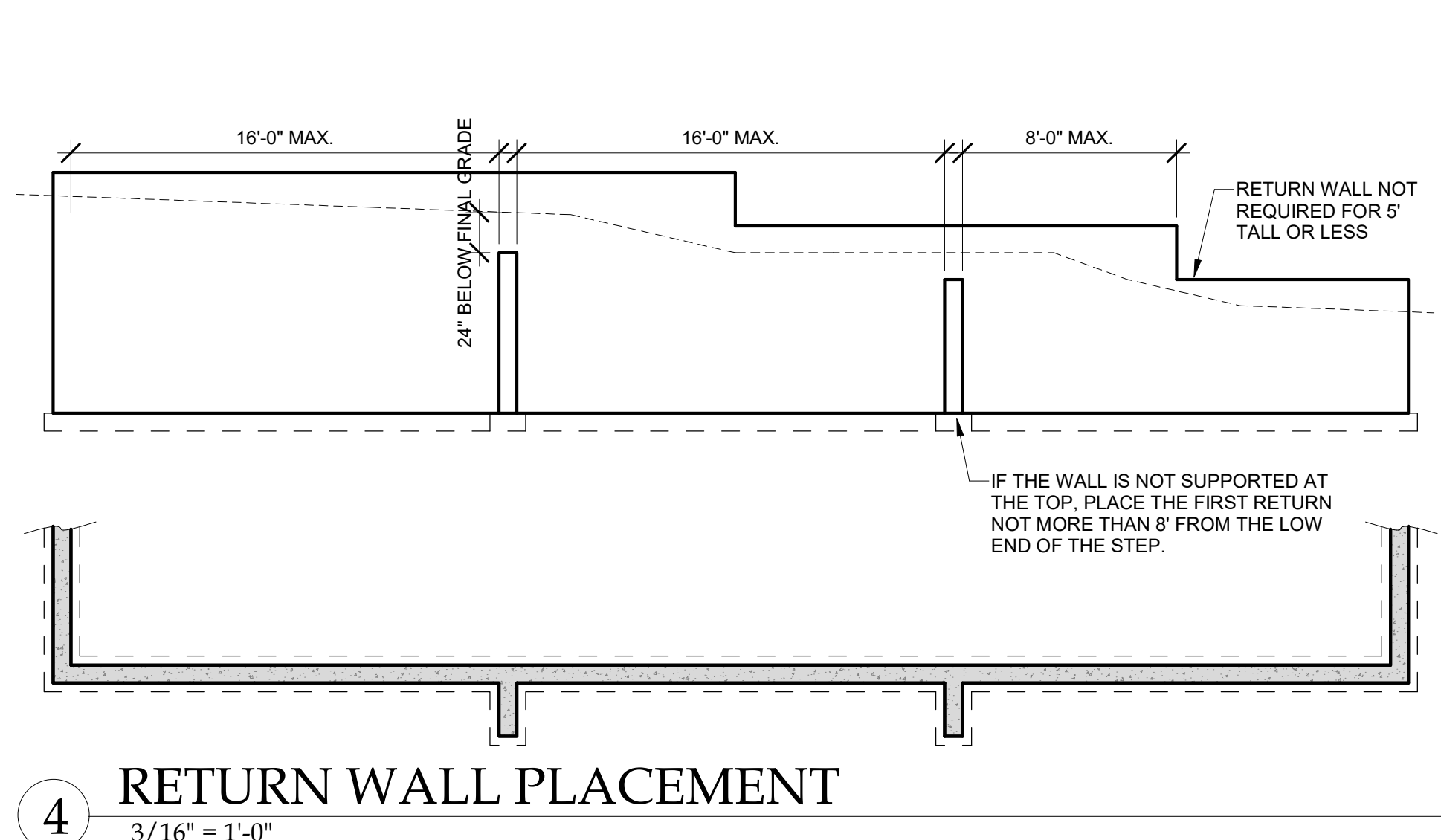
1 SOLID FOOTING JUMP DETAIL
3/8" = 1'-0"



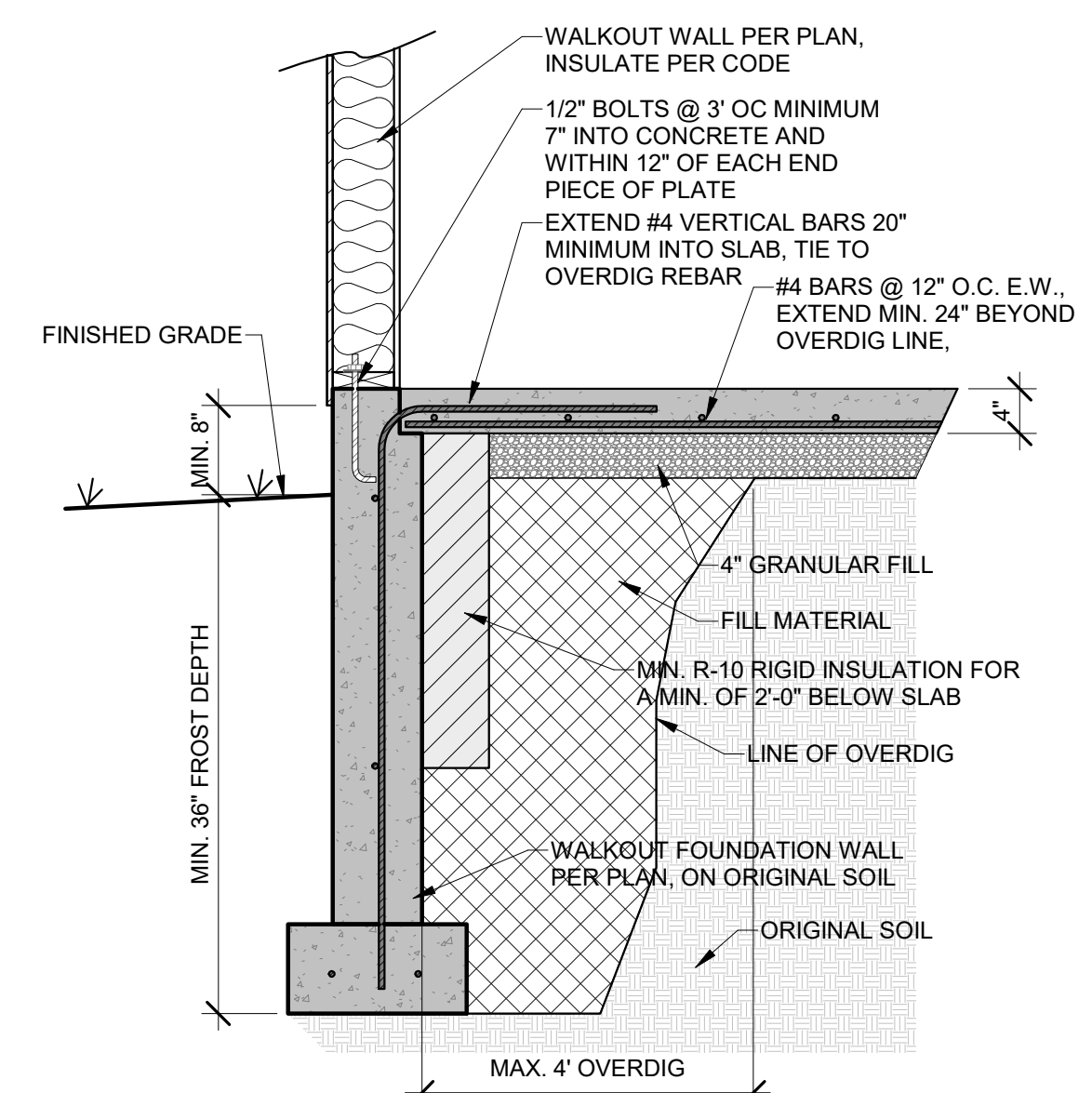
2 RETURN WALL DETAIL
1/2" = 1'-0"



3 REINFORCEMENT AT CORNERS AND STEPS
1/2" = 1'-0"

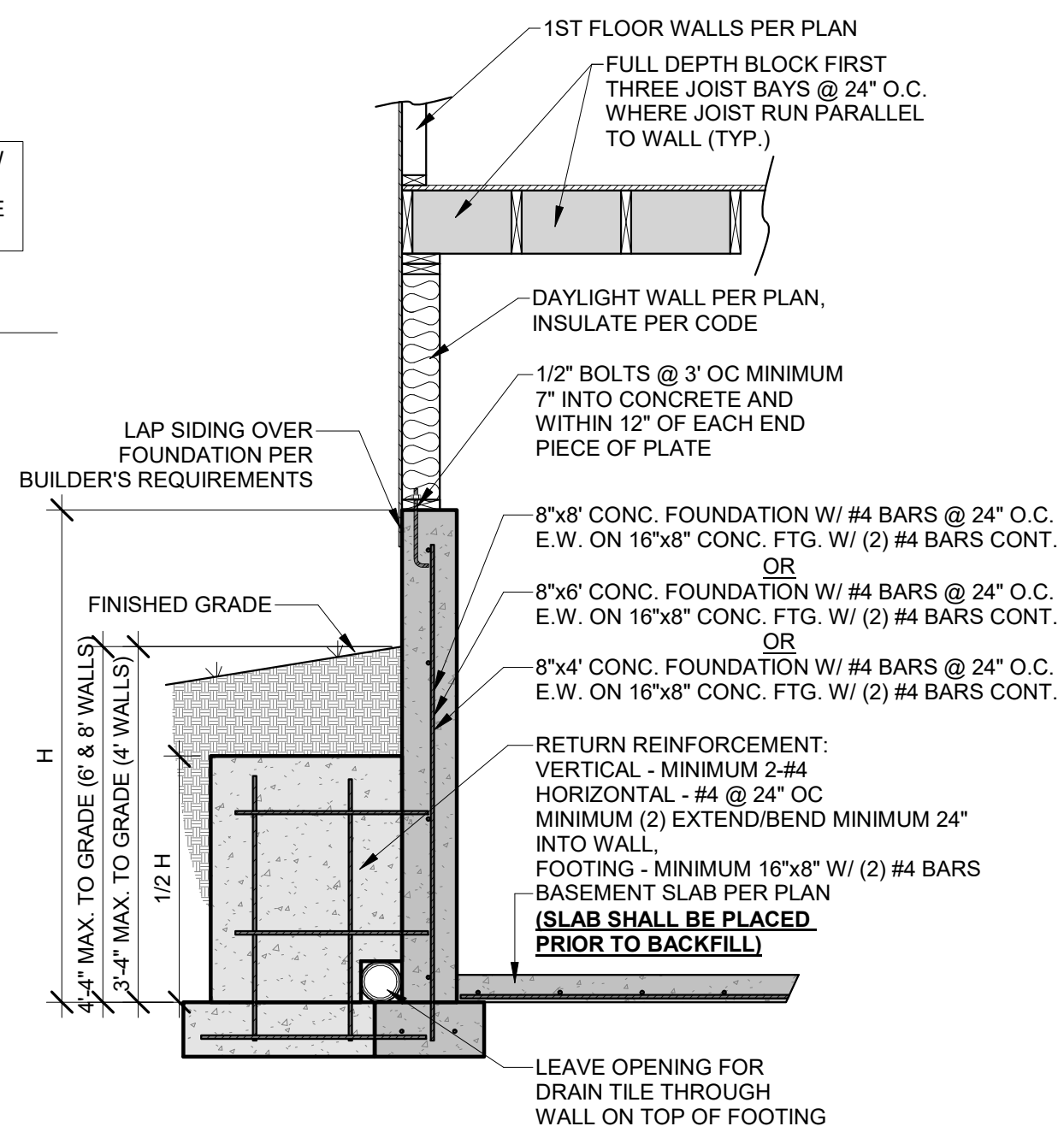


4 RETURN WALL PLACEMENT
3/16" = 1'-0"

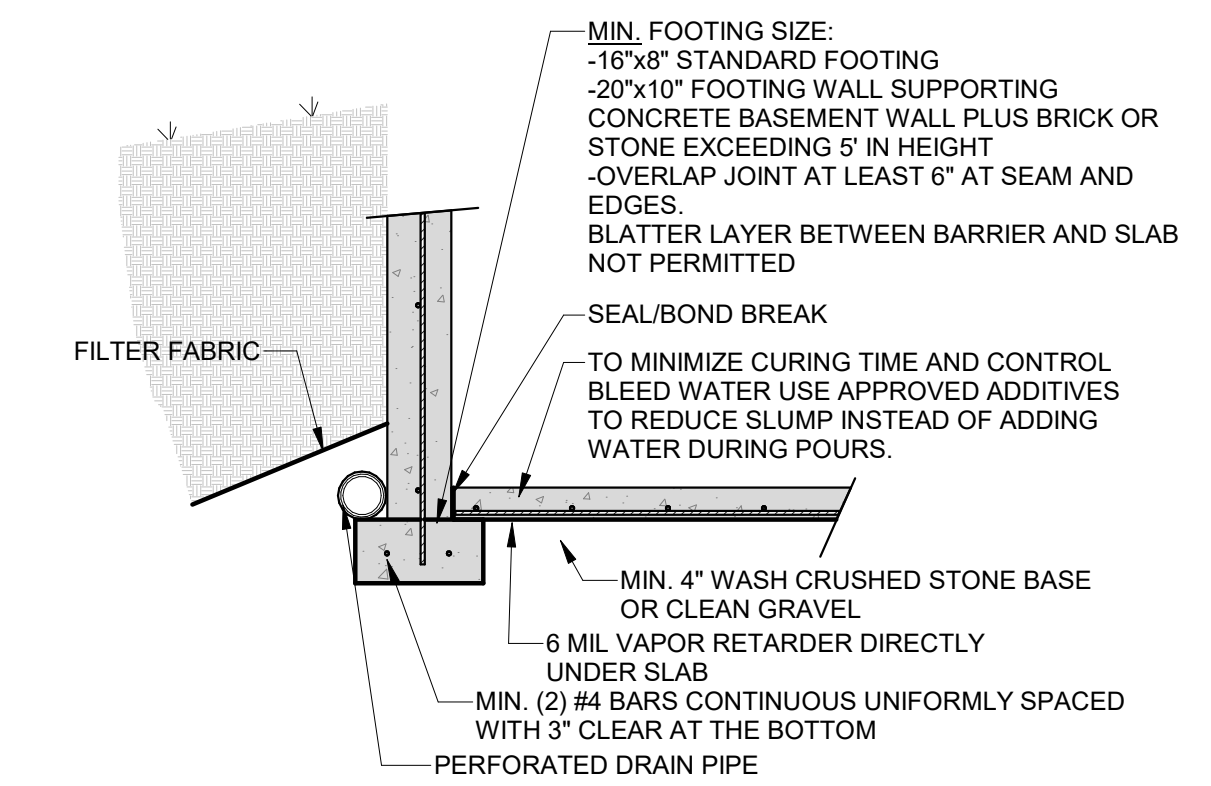


IMPORTANT NOTE:
ANY SLAB WITH GREATER THAN 2' OF GRADED ROCK OR 8' OF FILL SOIL BELOW SHALL BE DESIGNED AS STRUCTURAL PER PLAN. OUR FIRM SHOULD BE CONTACTED IMMEDIATELY FOR DESIGN RECOMMENDATIONS. DESIGN MUST BE COMPLETED PRIOR TO PLACEMENT OF PIERS OR FOOTINGS.

6 WALKOUT DETAIL
3/4" = 1'-0"



5 UNRESTRAINED FOUNDATION WALL
1/2" = 1'-0"



7 FOUNDATION FOOTINGS
1/2" = 1'-0"

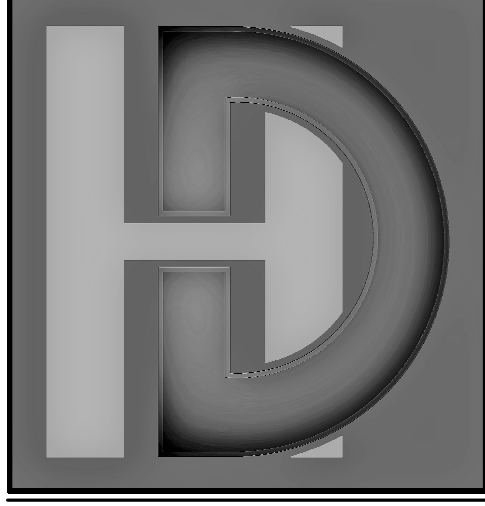
CONCRETE STRENGTH	8" THICK WALL		10" THICK WALL		
	8'	9'	8'	9'	10'
3000 PSI/ 40 KSI	16	12	24	16	12
3500 PSI/ 40 KSI	16	12	24	24	12
3000 PSI/ 60 KSI	24	16	24	20	16
3500 PSI/ 60 KSI	24	16	24	24	16

HORIZONTAL REINFORCEMENT**	4'-#4		5'-#4		6'-#4	
	4'-#4	5'-#4	4'-#4	5'-#4	6'-#4	6'-#4
ONE BAR 12" FROM TOP OF WALL; MAX. SPACING 24" O.C.	4-#4	5-#4	4-#4	5-#4	6-#4	6-#4

* CONCRETE SHALL HAVE AIR ENTRAINMENT OF 5-7%.
 * MINIMUM REQUIREMENT FOR VERTICAL REBAR IN PLAIN CONCRETE WALLS IS #4 @ 36" ON CENTER (ACI 332).
 * VERTICAL BARS SHALL BE CONTINUED UP TO WITHIN 8" OF THE TOP OF THE WALL.
 * REBAR SHALL BE POSITIONED AT THE TENSION FACE OF THE WALL (2" FROM THE INSIDE FACE).
 * REINFORCEMENT SHALL LAP A MINIMUM OF 24 INCHES AT ENDS, SPLICES, AND AROUND CORNERS.
 ** #4 BARS @ 24" ON CENTER.
 ** #4 BAR WITHIN 12 OF TOP AND BOTTOM OF WALL.
 ** MINIMUM GRADE 40 (40ksi) STEEL (PER ACI 332).
 ** HORIZONTAL REINFORCEMENT SHALL BE INSTALLED ON THE COMPRESSION SIDE (SOIL SIDE) OF THE VERTICAL REINFORCEMENT

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 SERVICE@HDENGINEERS.COM



STARR HOMES, LLC
 TYLER & ERIN RESIDENCE - LOT 3A & 4E
 PROMISED VIEW DR, LEE'S SUMMIT, MO

STRUCTURAL DETAILS & NOTES

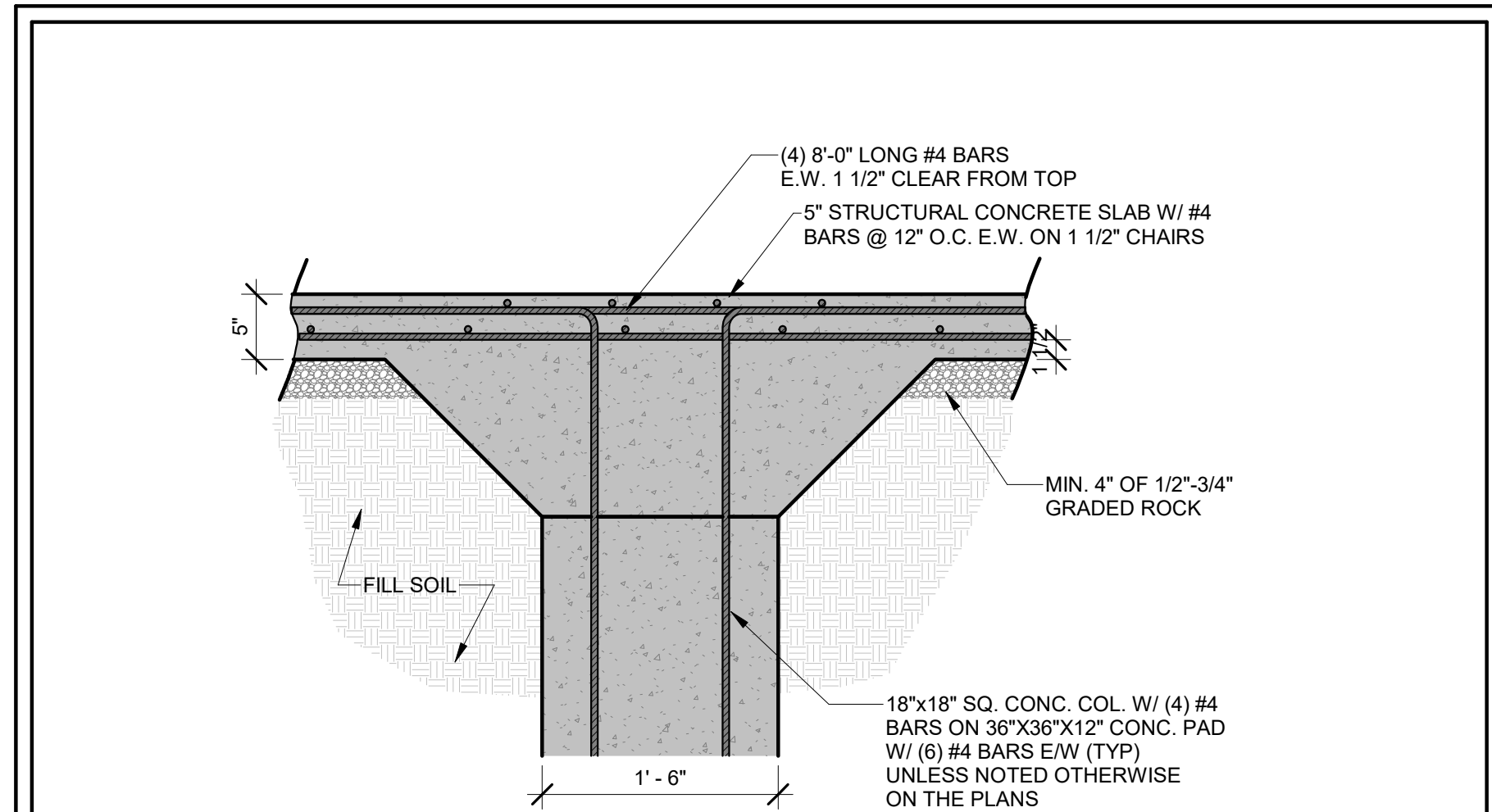
HD#: 42639
 DATE: 10/11/2021
 CHECKED BY: CLS

NO.	ISSUE/REVISION	Revision Date

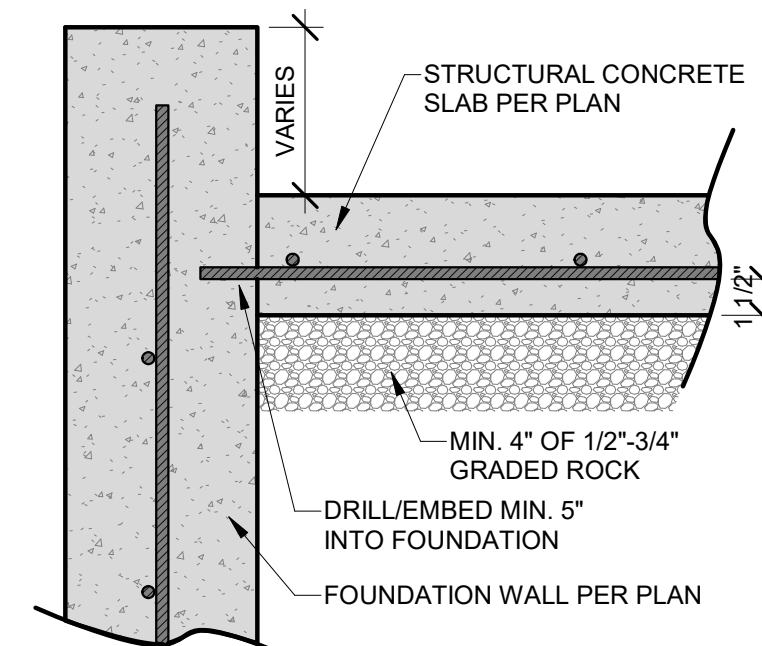
CONCRETE DETAILS

S-3.0

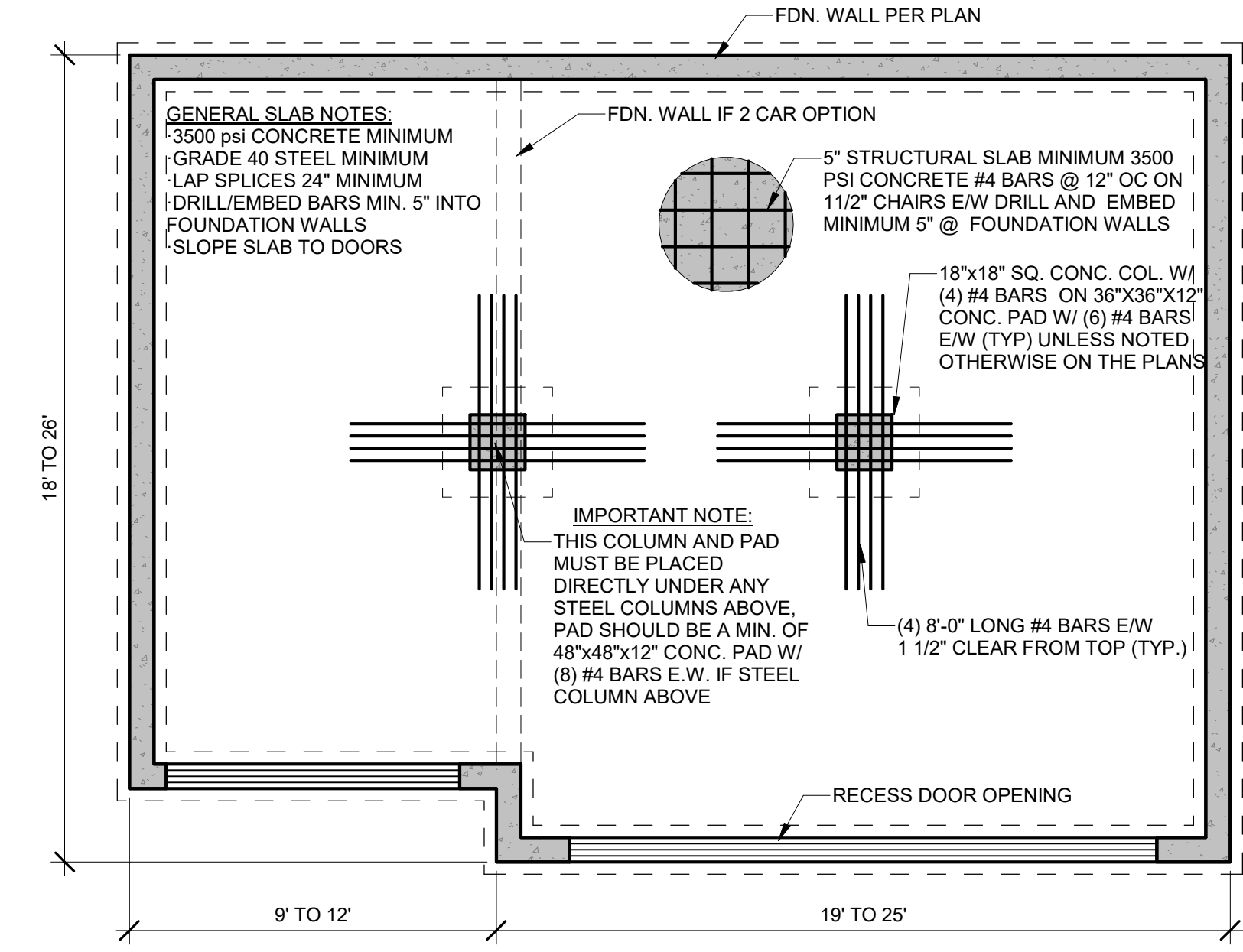
DETAILS PROVIDED ARE DERIVED FROM JOHNSON COUNTY RESIDENTIAL FOUNDATION GUIDELINE



7 GARAGE SLAB COLUMN DETAIL
1" = 1'-0"

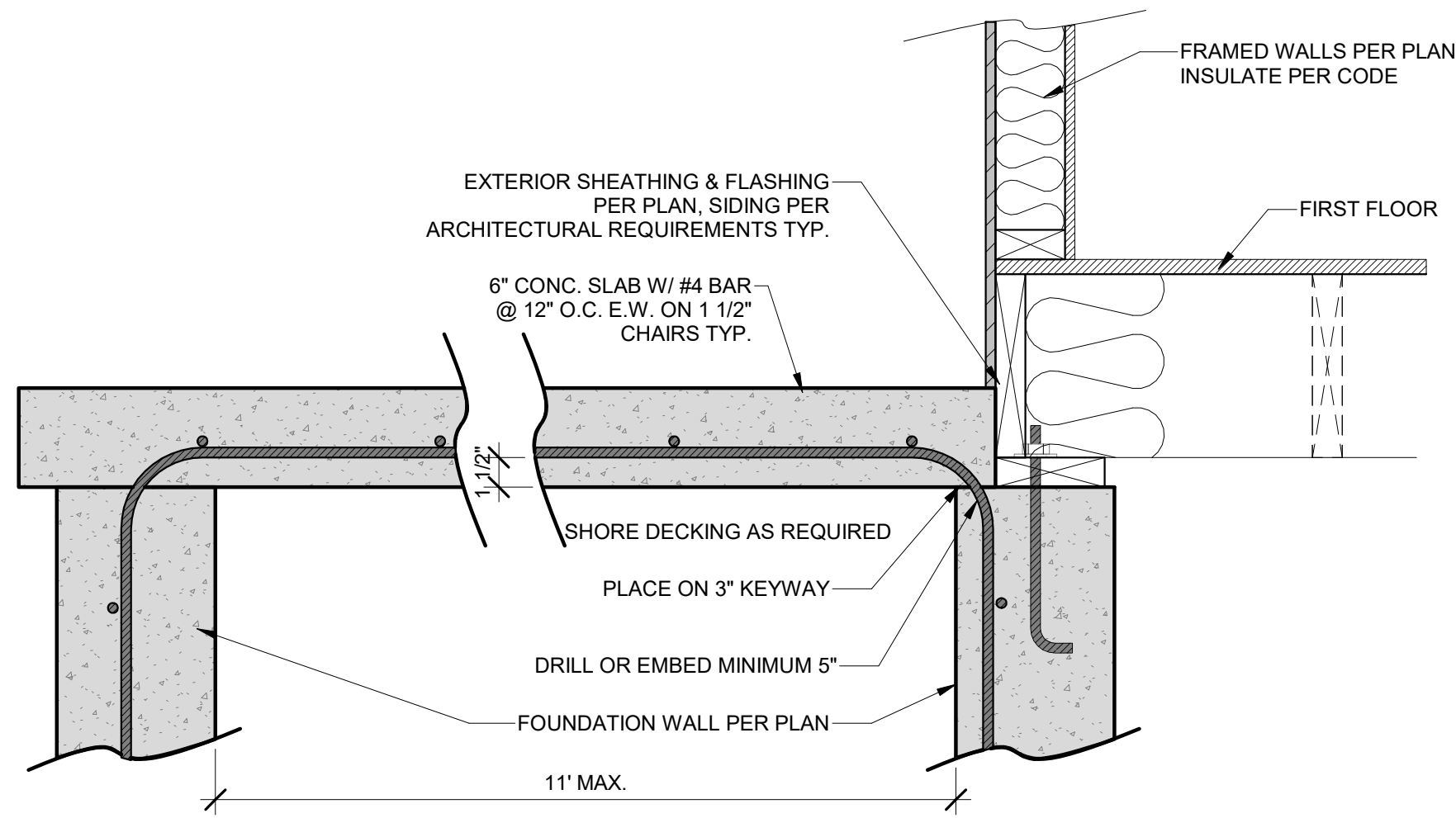


8 STRUCTURAL SLAB/ WALL
1 1/2" = 1'-0"



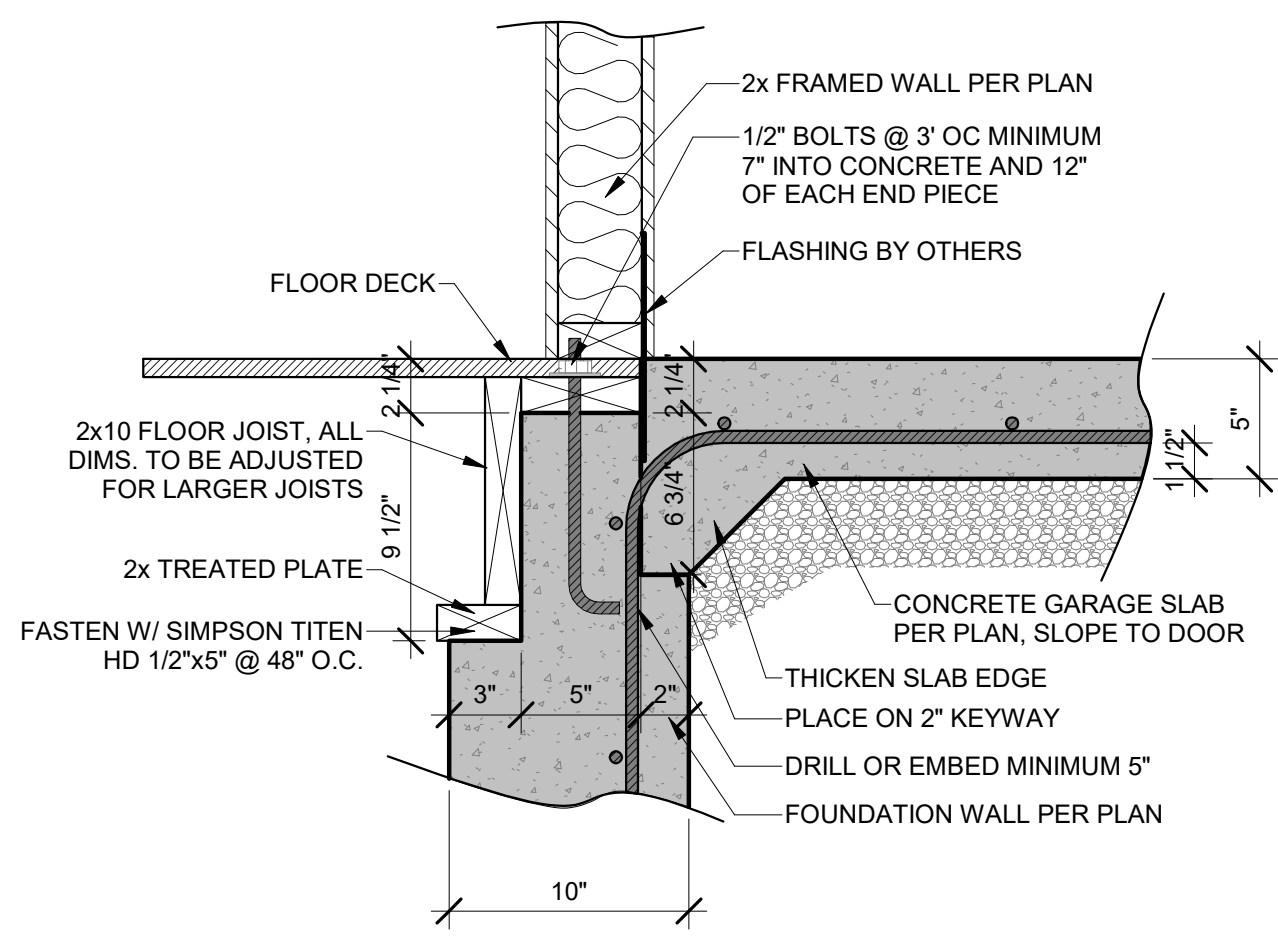
9 TYPICAL GARAGE SLAB
1/4" = 1'-0"

HD ENGINEERING STRUCTURAL GARAGE SLAB DETAILS

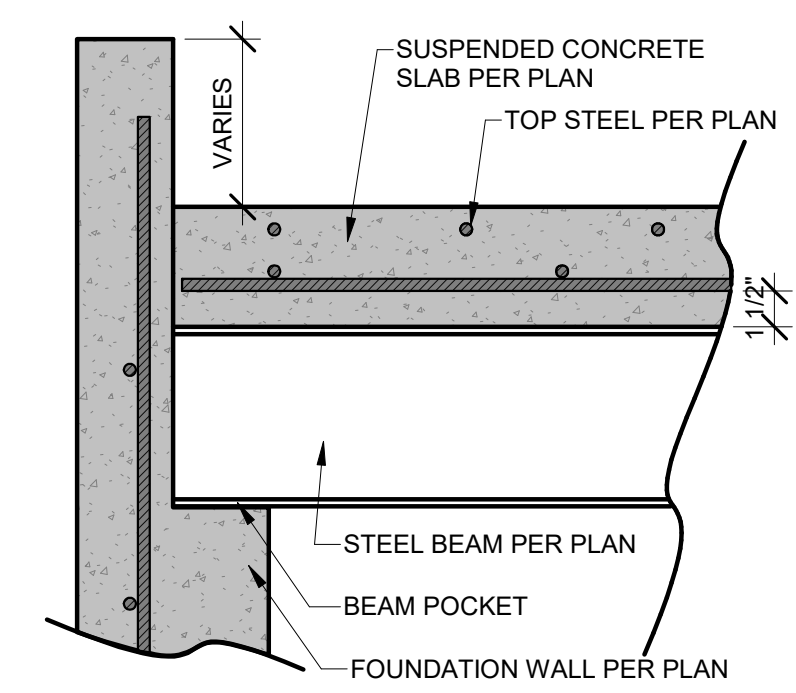


FOR SUSPENDED SLABS A MAXIMUM OF 10' ABOVE FLOOR BELOW: TEMPORARY SHORING WALLS SHALL BE PLACED AT A MAXIMUM OF 4' O.C./#2-2X4 STUDS AT 16" O.C. W/ TOP AND BOTTOM PLATE, WALL TO HAVE CONTINUOUS DIAGONAL BRACING. LATERAL BRACING TO BE RUN FROM WALL TO WALL AT MID HEIGHT 4' ON CENTER. SHORING TO REMAIN IN PLACE FOR AT LEAST 21 DAYS.

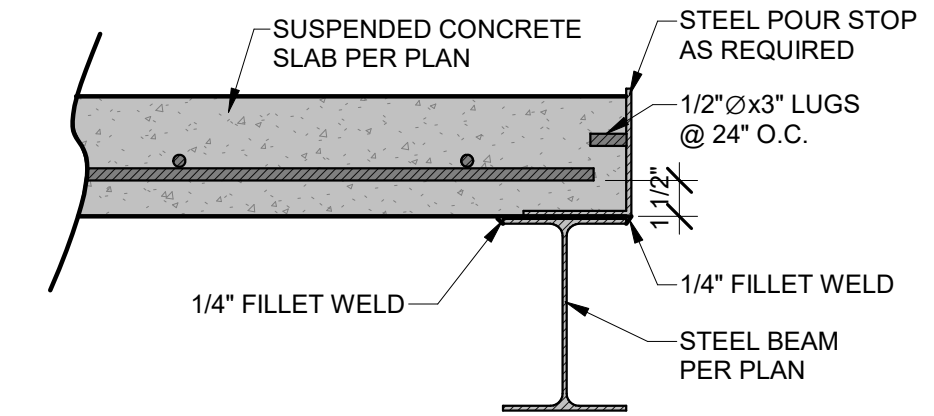
6 SUSPENDED PORCH STOOP SLAB
1 1/2" = 1'-0"



10 ZERO ENTRY GARAGE DETAIL
1 1/2" = 1'-0"

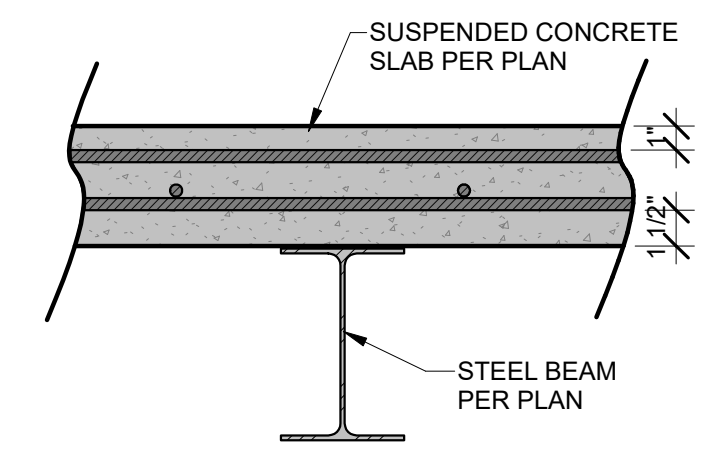


1 SUSPENDED SLAB BEAM/WALL CONNECTION
1 1/2" = 1'-0"

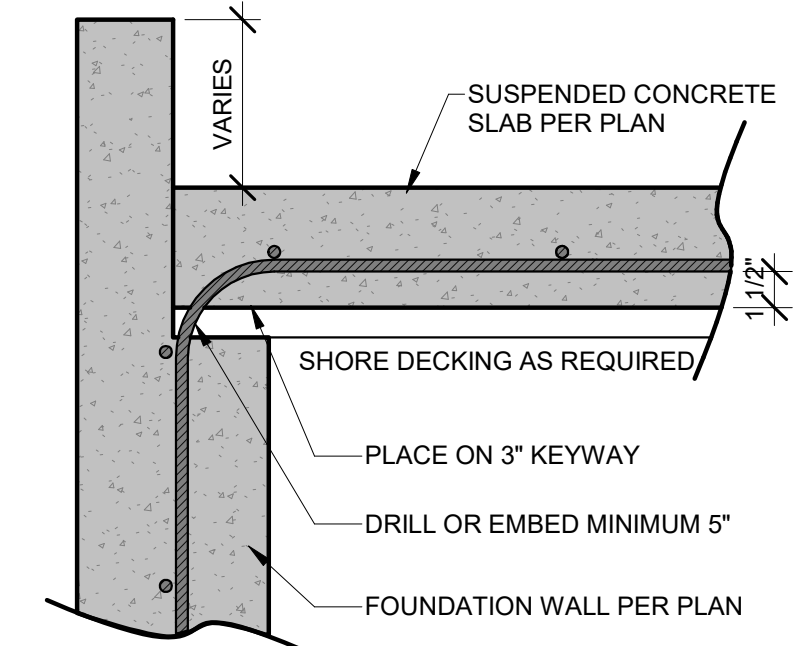


*FASTEN STEEL ANGLE TO BEAM W/ TEK SCREWS OR 2"x1/4" FILLET WELD @ 12" O.C.

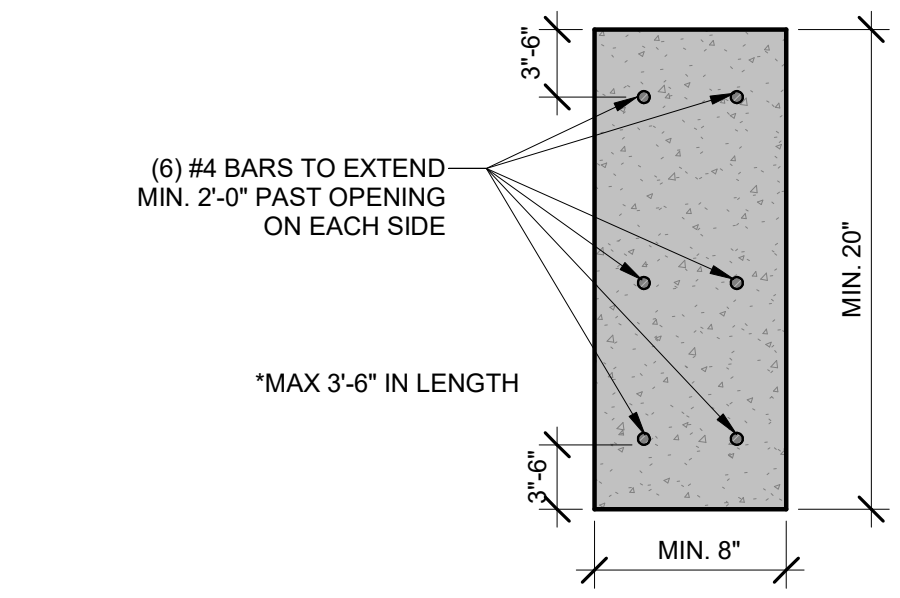
2 SUSPENDED SLAB POUR STOP
1 1/2" = 1'-0"



3 SUSPENDED SLAB/STEEL BEAM CROSS SECTION
1 1/2" = 1'-0"

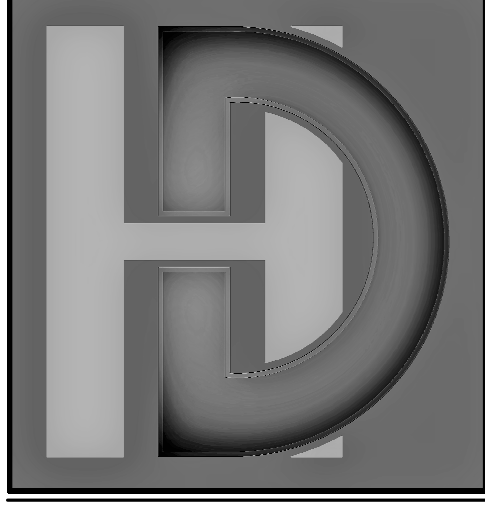


4 SUSPENDED SLAB/WALL CONNECTION
1 1/2" = 1'-0"



5 CONCRETE HEADER DETAIL
1 1/2" = 1'-0"

IMPORTANT NOTE:
FOR SUSPENDED SLABS A MAXIMUM OF 10' ABOVE FLOOR BELOW: TEMPORARY SHORING WALLS SHALL BE PLACED AT A MAXIMUM OF 4' O.C./#2-2X4 STUDS AT 16" O.C. W/ TOP AND BOTTOM PLATE, WALL TO HAVE CONTINUOUS DIAGONAL BRACING. LATERAL BRACING TO BE RUN FROM WALL TO WALL AT MID HEIGHT 4' ON CENTER. SHORING TO REMAIN IN PLACE FOR AT LEAST 21 DAYS.
ANY CAST IN PLACE SLABS FORMED MORE THAN 10' ABOVE THE FLOOR BELOW SHALL HAVE A SITE SPECIFIC SHORING DESIGN DONE. OUR FIRM SHOULD BE CONSULTED FOR THIS DESIGN ONCE FOUNDATION WALLS ARE IN PLACE TO EVALUATE ALL FIELD CONDITIONS. IT SHOULD BE NOTED THAT FAILURE TO HAVE AN ADEQUATE SHORING DESIGN CAN RESULT IN FORM COLAPSE AND/OR CATASTROPHIC FAILURE.



STARR HOMES, LLC
TYLER & ERIN RESIDENCE - LOT 3A & 4E
PROMISED VIEW DR, LEE'S SUMMIT, MO

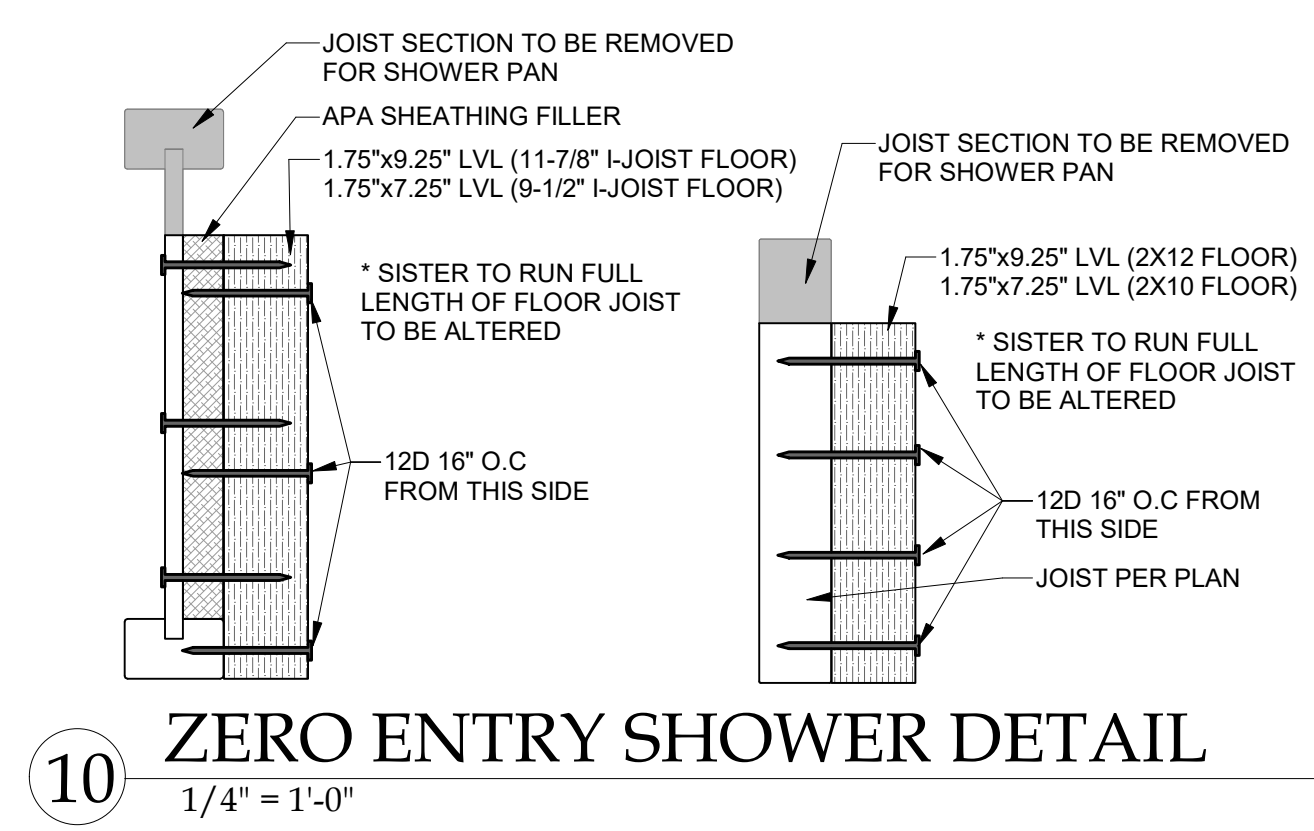
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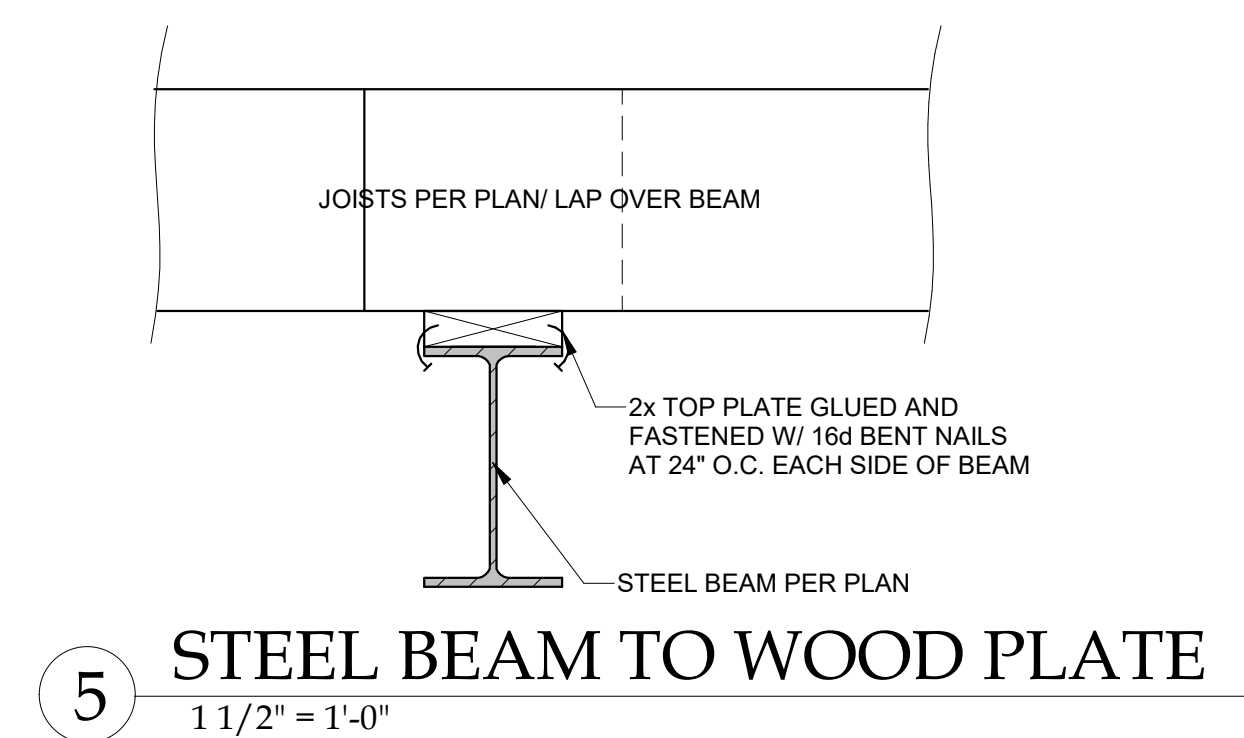
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SUSPENDED SLAB DETAILS

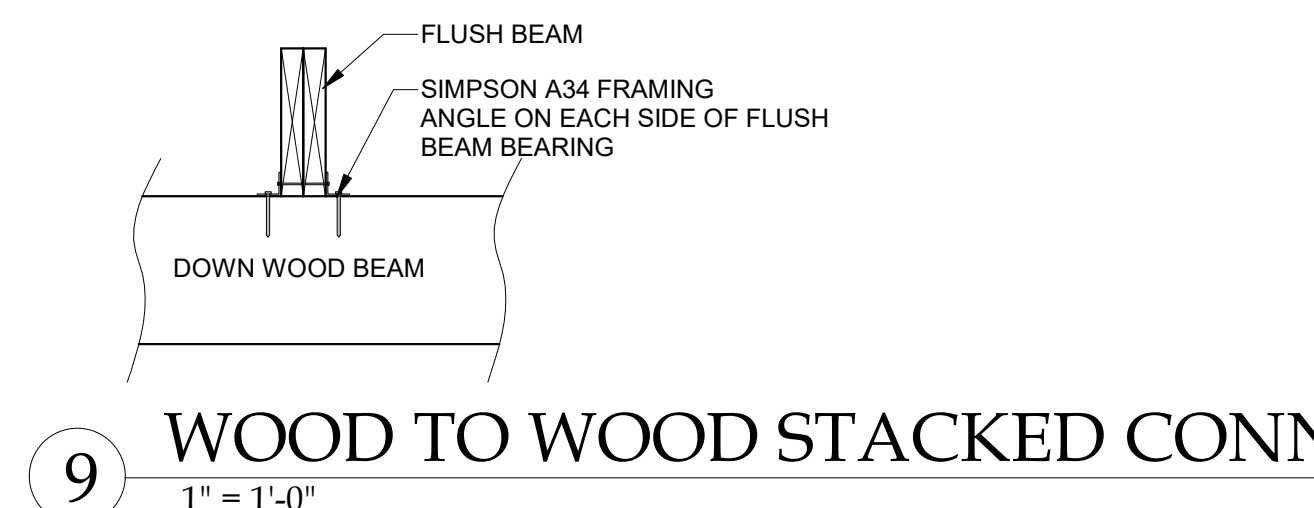
S-3.1



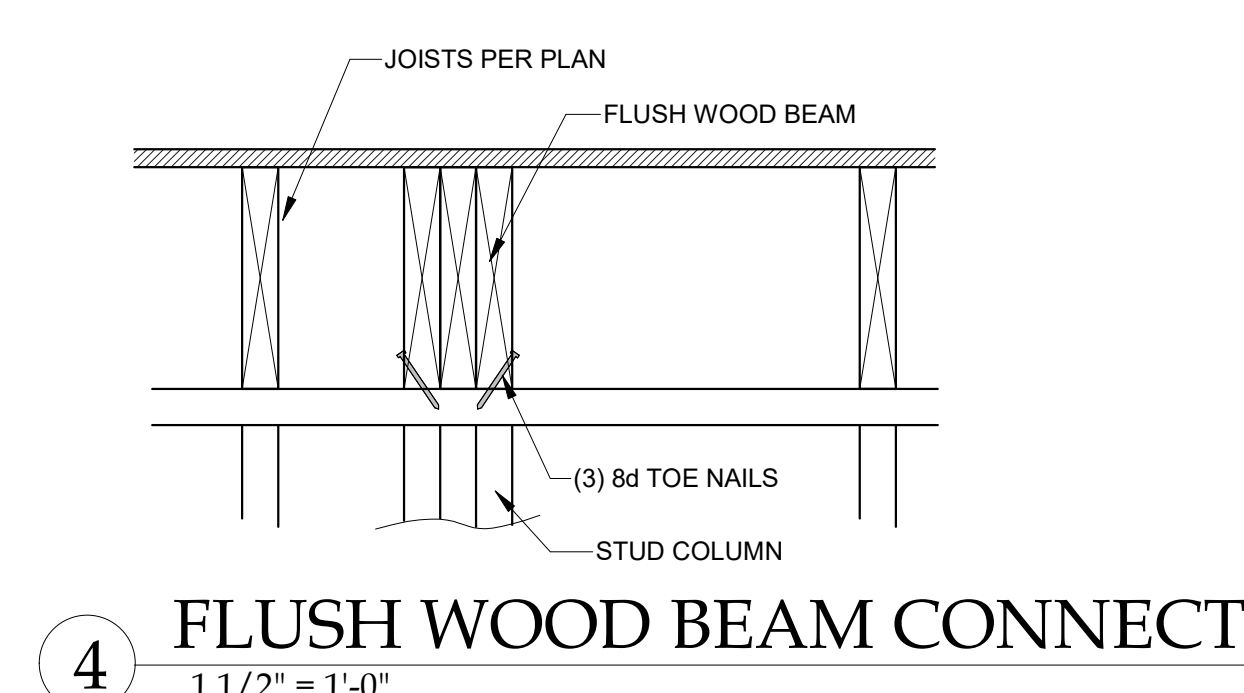
10 ZERO ENTRY SHOWER DETAIL
1/4" = 1'-0"



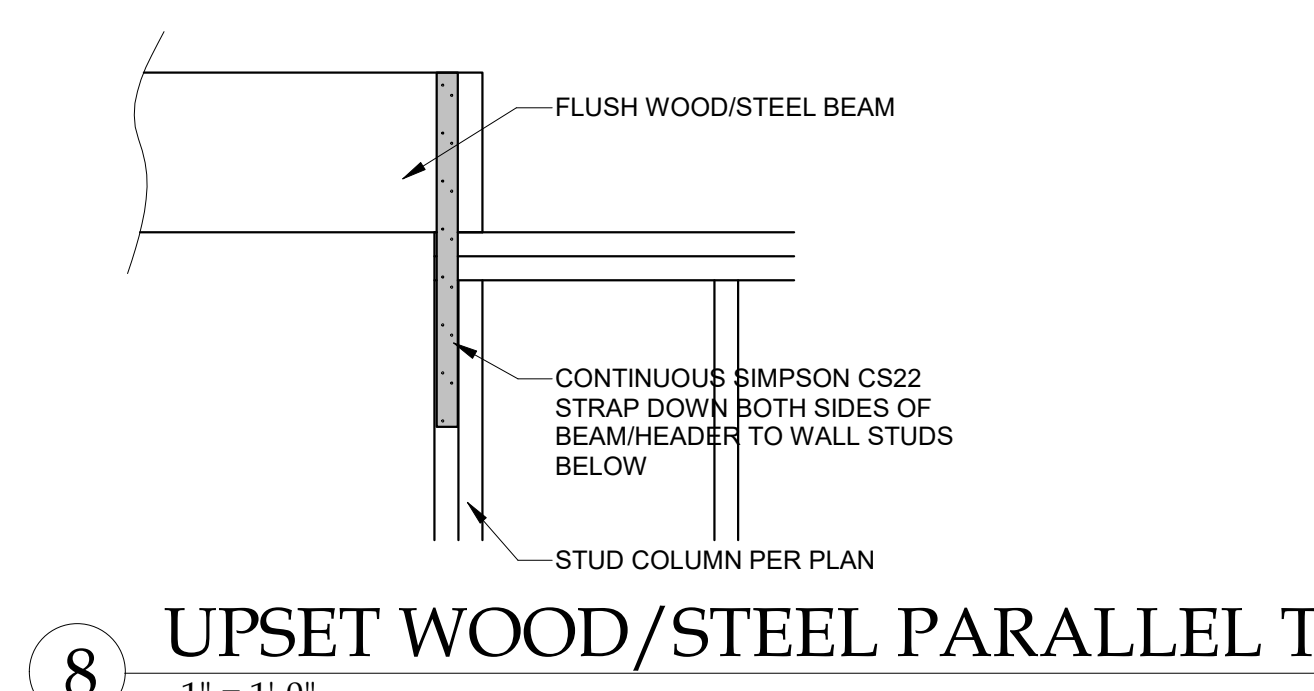
5 STEEL BEAM TO WOOD PLATE
1 1/2" = 1'-0"



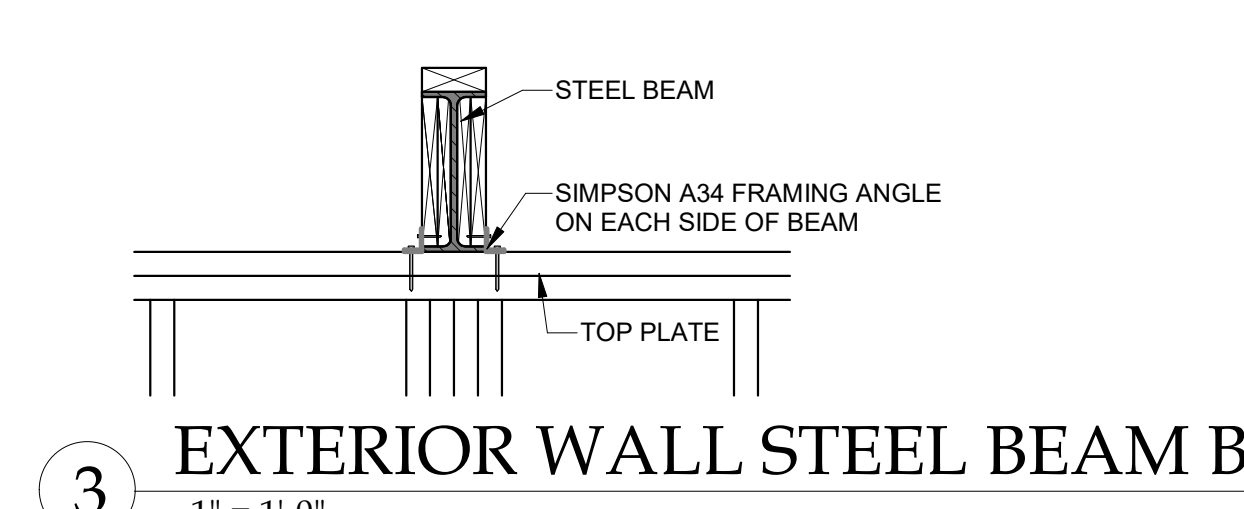
9 WOOD TO WOOD STACKED CONNECTION
1" = 1'-0"



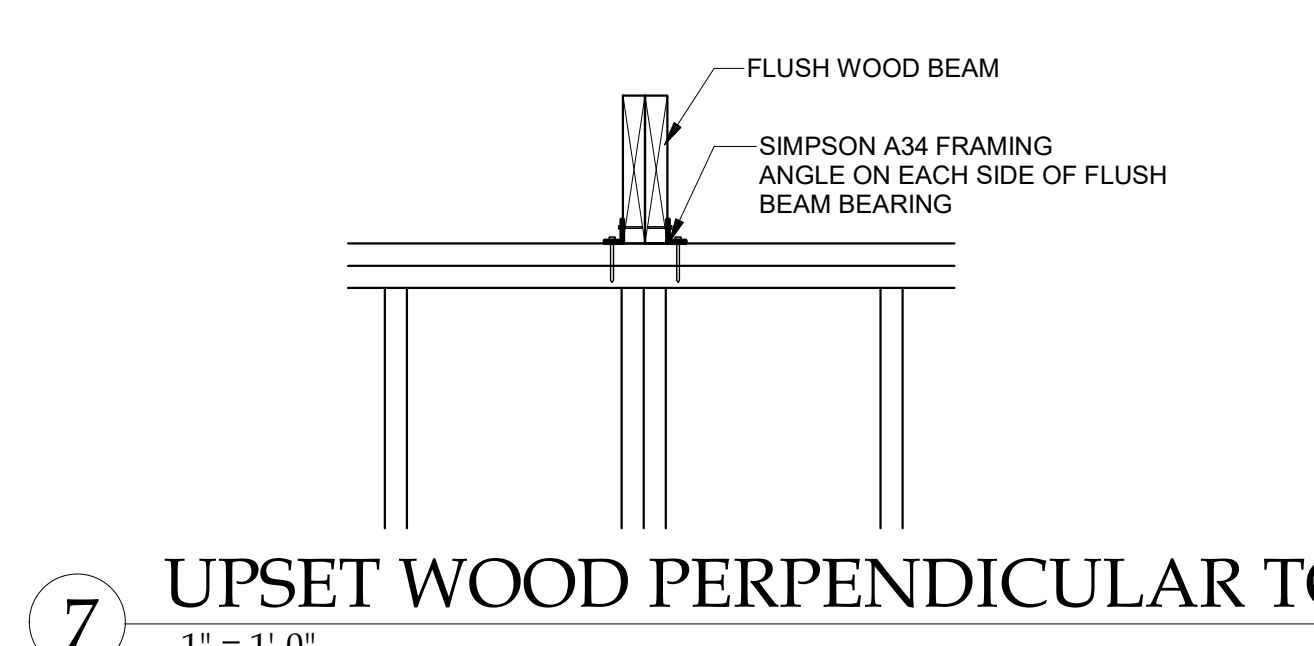
4 FLUSH WOOD BEAM CONNECTION
1 1/2" = 1'-0"



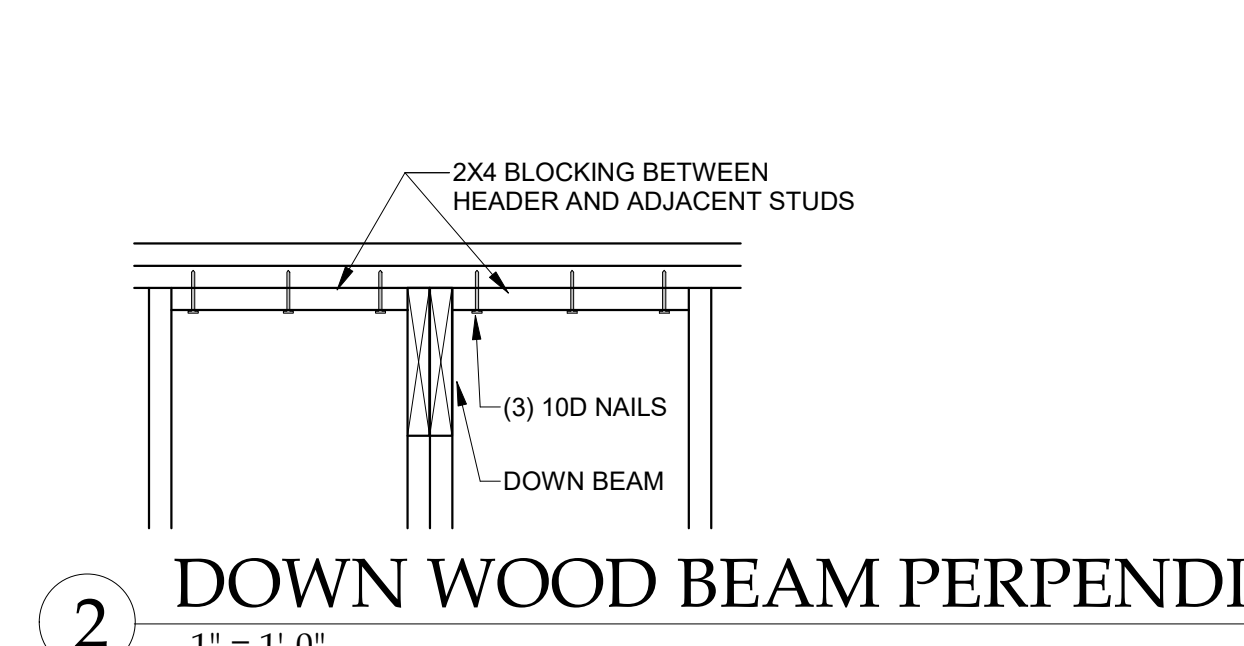
8 UPSET WOOD/STEEL PARALLEL TO WALL
1" = 1'-0"



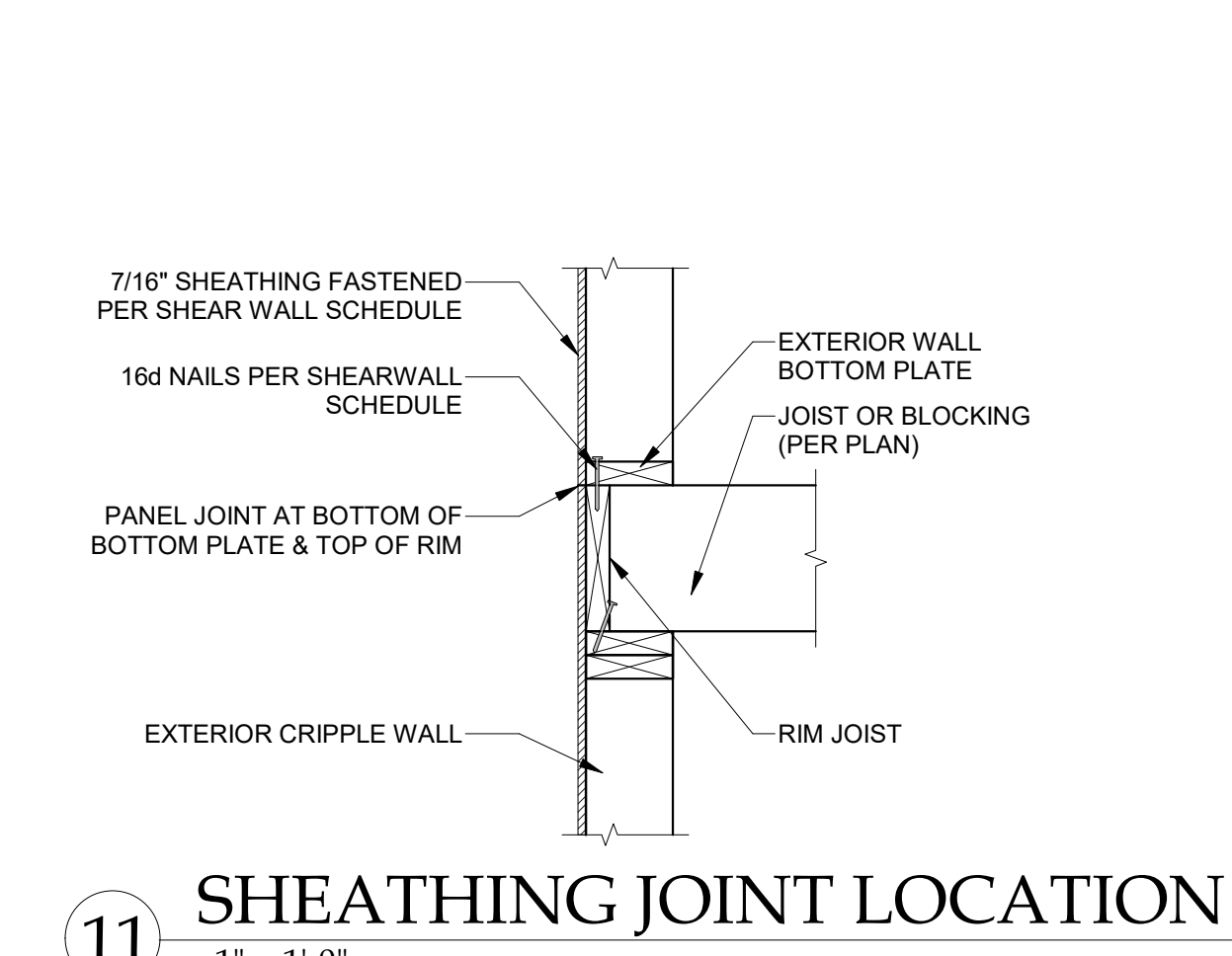
3 EXTERIOR WALL STEEL BEAM BEARING
1" = 1'-0"



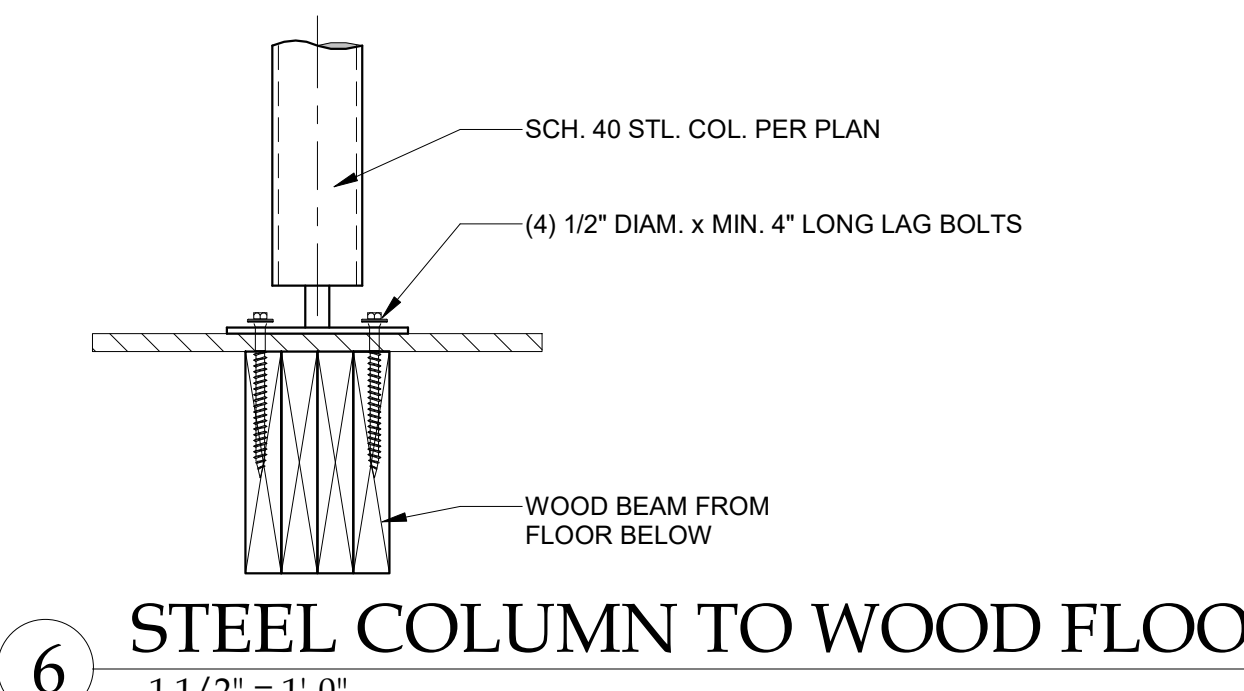
7 UPSET WOOD PERPENDICULAR TO WALL
1" = 1'-0"



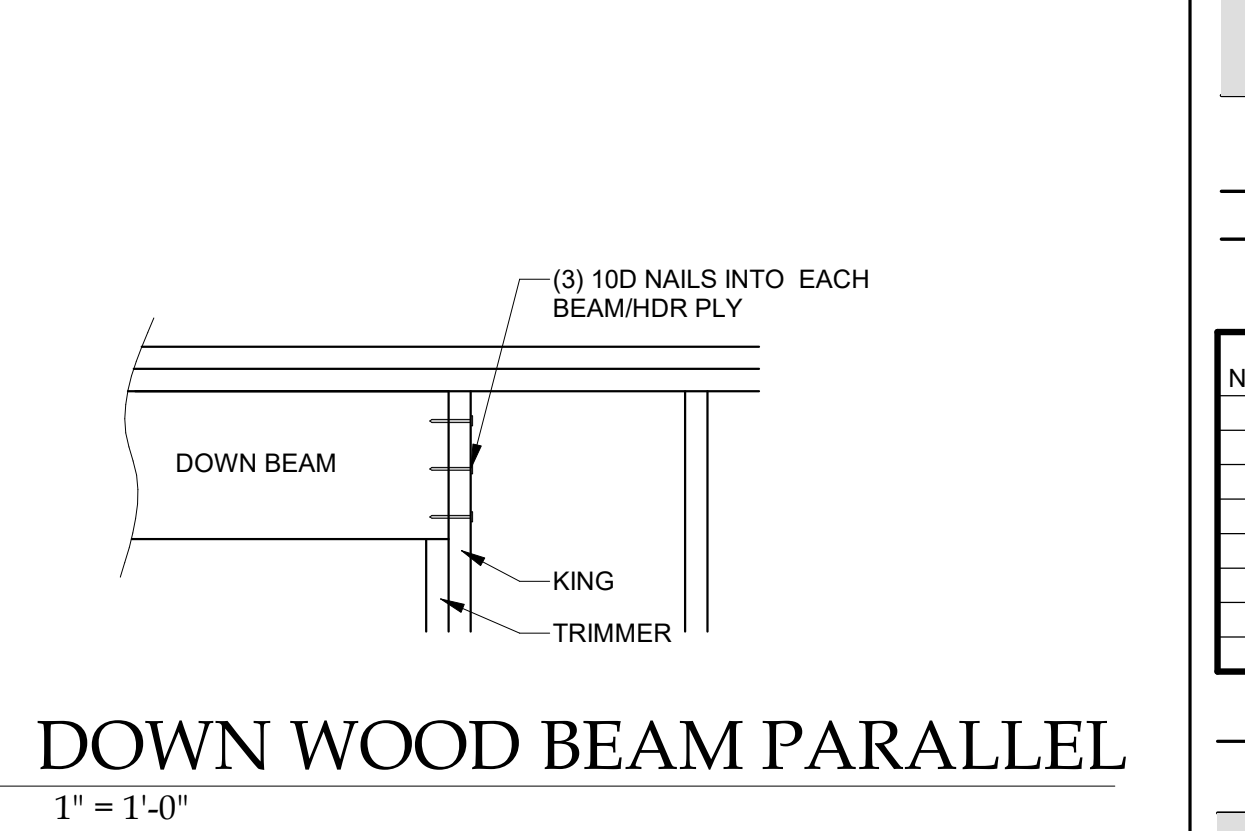
2 DOWN WOOD BEAM PERPENDICULAR
1" = 1'-0"



11 SHEATHING JOINT LOCATION
1" = 1'-0"



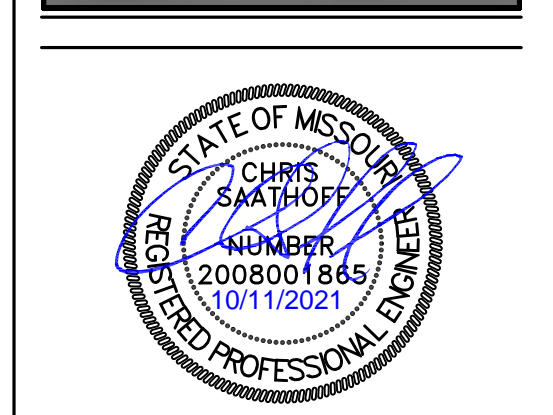
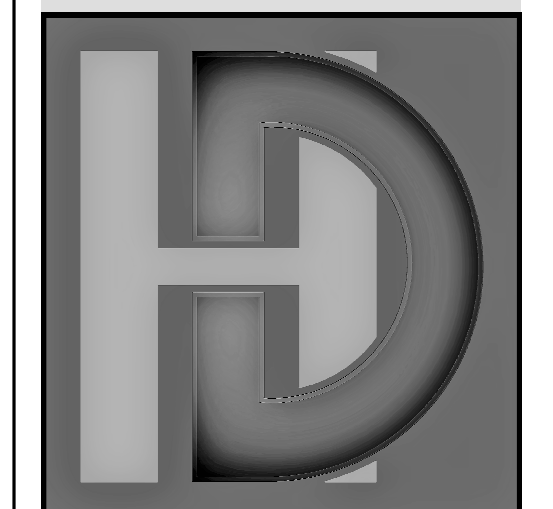
6 STEEL COLUMN TO WOOD FLOOR
1 1/2" = 1'-0"



1 DOWN WOOD BEAM PARALLEL
1" = 1'-0"

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GENERAL DETAILS

S-4.0