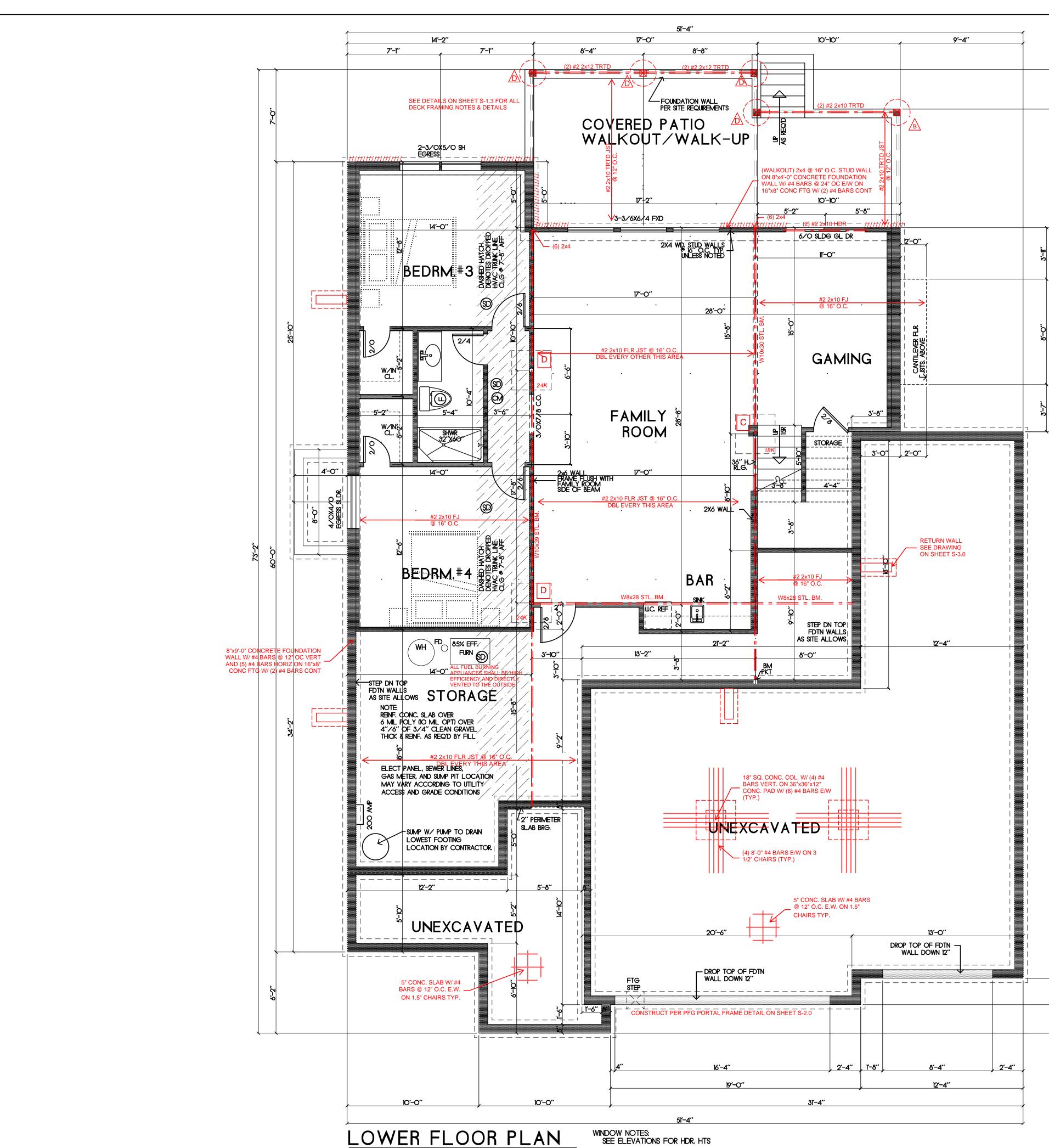


AS NOTED ON PLANS REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI

RELEASE FOR CONSTRUCTION



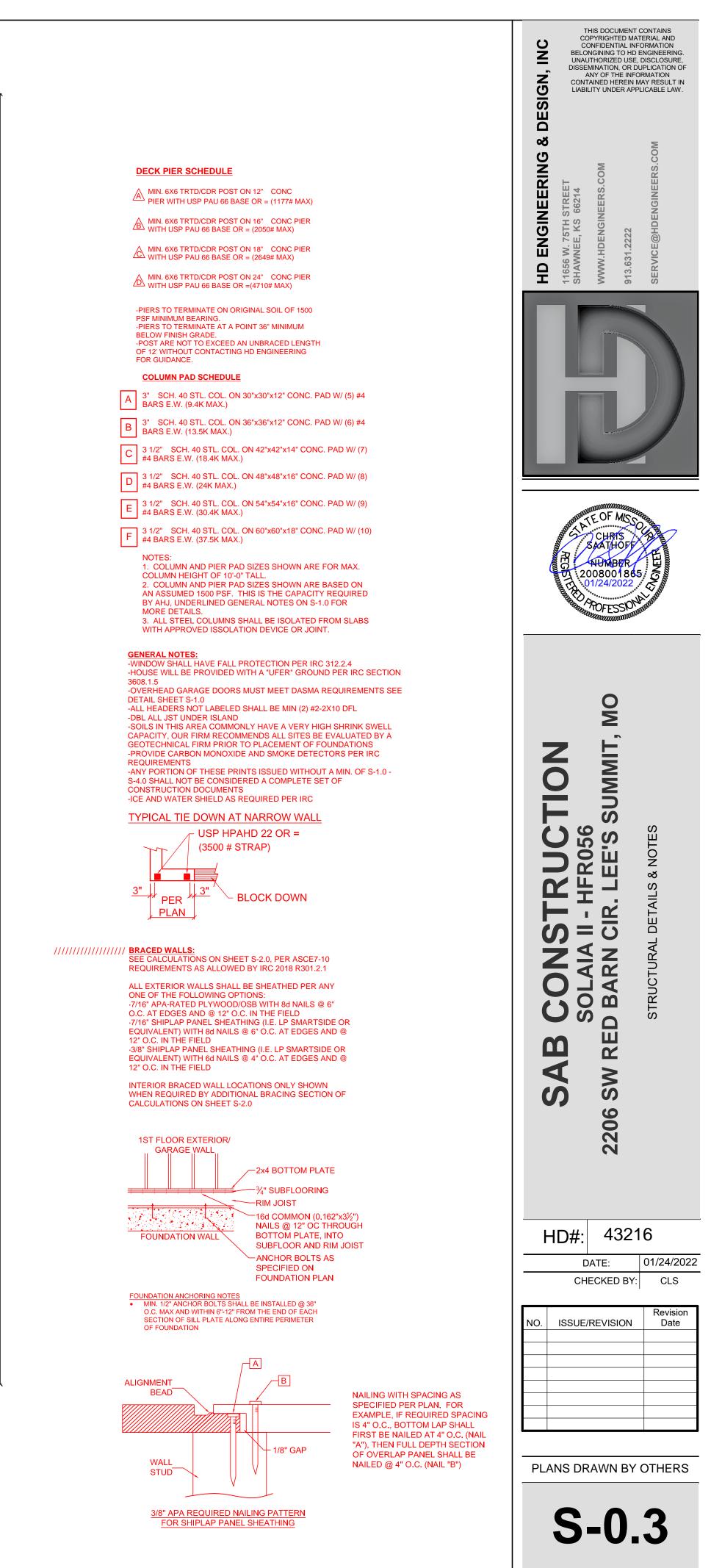


RELEASE FOR CONSTRUCTION **AS NOTED ON PLANS REVIEW** DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI

08/01/2023 1:07:57 05/15/2020 1:34:01 PM ChrisSaathoff

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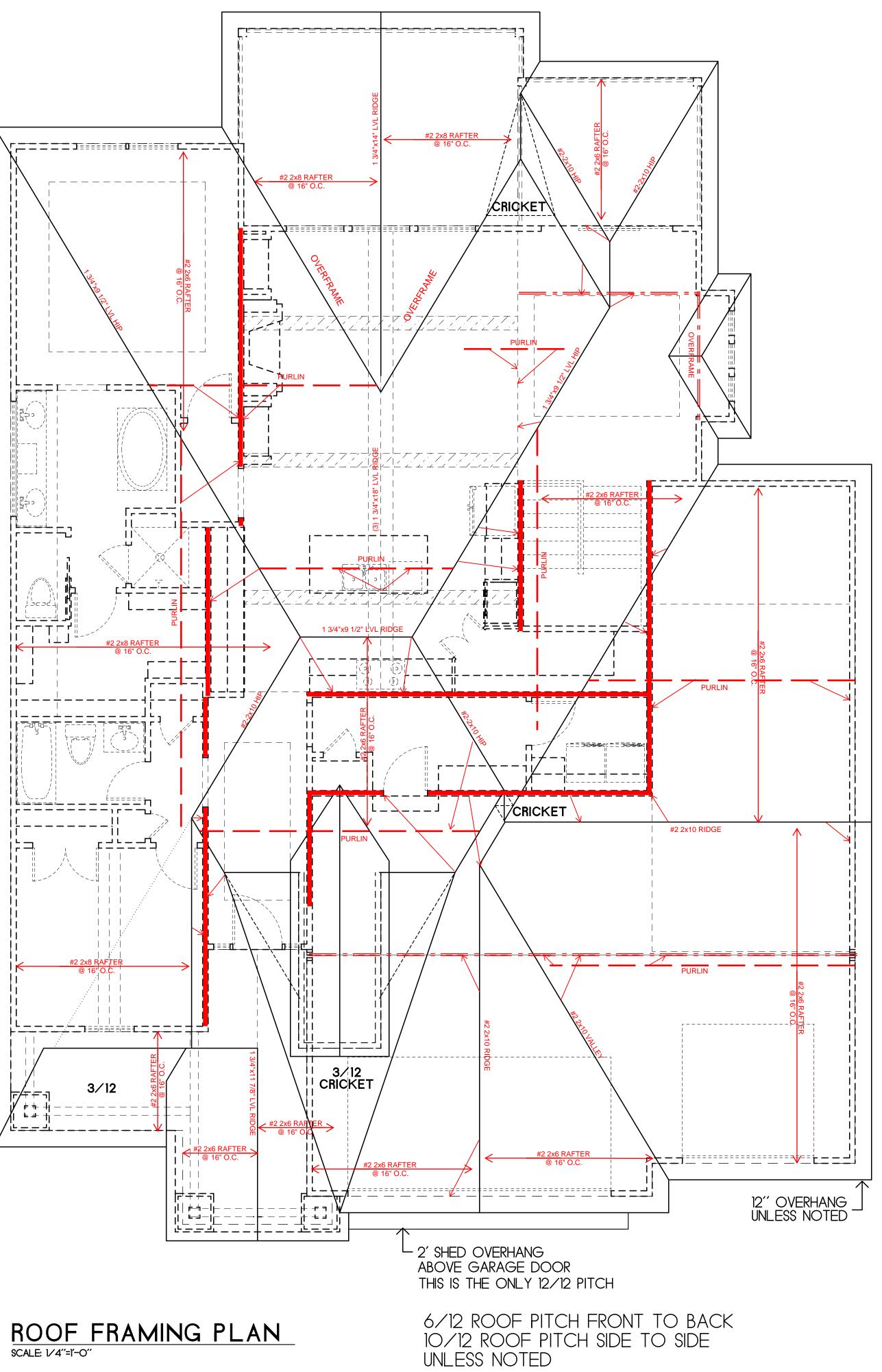
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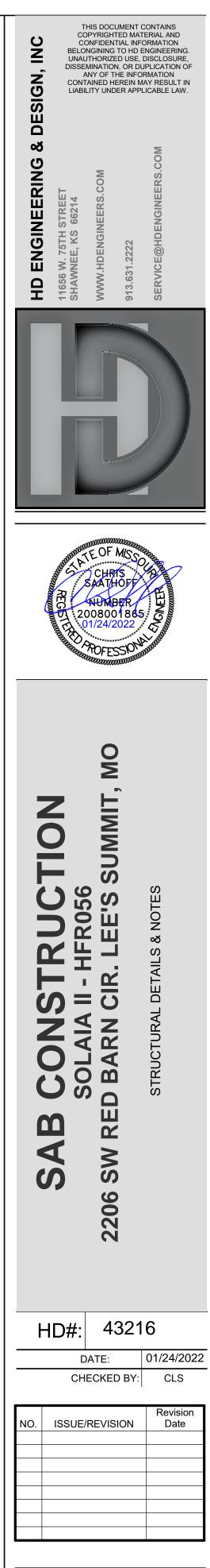


RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 08/01/2023 1:07:58 05/15/2020 1:34:01 PM ChrisSaathoff

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PLANS DRAWN BY OTHERS

S-0.4

ALLOWABLE LOADS FOR PNEUMATIC OR MECHANICALLY DRIVEN NAILS AND STAPLES

			PENETRATION	AL	LOWABLE LO	ADS (IN POUNE	DS)
FASTENER	NAIL GUN NAILS/	WIRE	REQUIRED INTO MAIN		STRENGTH	WITHDRAWA	
DESCRIPTION	WIRE DIA.	GA.	MEMBER FOR LATERAL STRENGTH (IN.)	SP	DF/L	SP	DF/L
16 GA. STAPLE	.063	16	1	51		36	32
15 GA. STAPLE	.072	15	1	64		42	37
14 GA. STAPLE	.080	14	1	75		46	41
6d COOLER NAIL							
6d SINKER NAIL	.092	13	1	46		27	23
6d BOX NAIL							
6d CASING NAIL	.099	12-1/2	1-1/8	61	55	31	24
7d COOLER NAIL							
6d COMMON NAIL							
8d COOLER NAIL							
8d SINKER NAIL	.113	11-1/2	1-1/4	79	72	35	28
8d BOX NAIL							
8d CASING NAIL							
6d RING SHANK NAIL							
6d SCREW SHANK NAIL	400		4.0/0	00	0.1	4.4	00
8d RING SHANK NAIL	.120	11	1-3/8	89	81	41	32
8d SCREW SHANK NAIL							
10d Cooler Nail							
10d Sinker Nail	.128	10-1/2	1-1/2	89	81	36	31
12d Short							
10d Box Nails							
12d Box Nails	.128	10-1/2	1-1/2	101	93	40	31
10d Casing Nails							
8d Common Nails							
16d Short	.131	10-1/4	1-1/2	106	97	41	32
12d Sinkers					100		
16d Box Nails	.135	10	1-1/2	113	103	42	33
10d Ring Shank Nails							
10d Screw Shank Nails	.135	10	1-5/8	113	103	46	36
12d Ring Shank Nails	. 100		1-0/0	115	100	70	
12d Screw Shank Nails							
10d Common Nails							
12d Common Nails							
16d Sinker Nails	.148	9	1-5/8	128	118	46	36
20d Box Nails							
30d Box Nails							
16d Ring Shank Nails	440	_	4.014	400	440	50	40
16d Screw Shank Nails	.148	9	1-3/4	128	118	50	40
16d Common Nails	160	0	1.0/4	151	4.4.4	E0	40
40d Box Nails	.162	8	1-3/4	154	141	50	40
20d Ring Shank Nails	.177	7	2-1/8	178	163	59	47
20d Screw Shank Nails	.177	′	2-1/0	170	103	28	47
20d Sinker Nails	.177	7	2-1/8	178	163	54	43
20d Common Nails	440	_	0.4/0	470	400	50	47
30d Sinker Nails	.148	9	2-1/8	170	166	59	47

SHEATHING SCHEDULE

ALL SHEATHING MATERIALS TO BE APPLIED PERPENDICULAR TO JOISTS AND ENDS STAGGERED

BUILDING COMPONENT	MATERIAL	FASTENING
ROOF SHEATHING	7/16" PLYWOOD	16 GA X 1 3/4" STAPLES @ 6" OC EDGES & 12" OC IN FIELD
ROOF SHEATHING	1x 4 #3 FURRING	1/2" CROWN STAPLES
	3/4" T&G YELLOW	14 GA X 1 3/4" STAPLES @ 6" OC EDGES & 12" OC IN FIELD
FLOOR SHEATHING	PINE PLYWOOD	12.5 GA X 1 1/2" RING OR SCREW SHANK NAILS @ 6" OC EDGES & 12" OC IN FIELD
WALL COVERING	1/2" GYPSUM SHEATHING	6D COMMON NAILS: 1 5/8" GALVANIZED STAPLES; 1 1/4" SCREWS, TYPE W OR S @ 4" OC EDGES & 8" OC IN FIELD
CEILING COVERING	1/2" GYPSUM SHEATHING	7" OC NAILED / 12" OC SCREWED W/ 13GA, 1 3/8" LONG, 19/64" HEAD; 0.098 Ø, 1 1/4" LONG, ANG-RINGED; 5D COOLER NAIL, 0.086 Ø, 1 5/8" LONG, 15/64" HEAD; OR GYP BD NAIL, 0.086 Ø, 1 5/8" LONG, 19/64" HEAD
EXTERIOR WALL	7/16" APA RATED SHEATHING	8D COMMON NAILS @ 6" OC EDGES & 12" OC IN THE FIELD
SHEATHING	RATED PANEL SIDING, RATED 16" O.C. 7/16" THICK	8D BOX OR SINKER NAILS @ 6" OC EDGES & 12" OC IN THE FIELD

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EXCEPTIONS: SEALS.

FRAME FASTENING SCHEDULE

BUILDING COMPONENT	FASTEN TO	FASTEN WITH
	RIDGE / VALLEY / HIP	TOENAIL W/ (4) 16D, FACENAIL W/ (3) 16D
	PLATE	TOENAIL W/ (3) 10D
RAFTERS	LEDGER STRIPS SUPPORTING JOISTS OR RAFTERS	FACENAIL W/ (3) 16D
	COLLAR TIE TO RAFTERS	FACENAIL W/ (3) 10D
	TOP PLATE	TOENAIL W/ (3) 8D @ EACH END
	WHERE CLG JST RUN PARALLEL TO RAFTERS FAC	ENAIL TO RAFTERS W/ (3) 10D MINIMUM
EILING JOISTS	LAPS OVER PARTITIONS	FACENAIL W/ (3) 10D
	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	TOENAIL W/ (3) 8D
	BUILT-UP BEAMS, 2" LUMBER LAYERS, FACENAIL OPPOSITE SIDES, (2) @ EACH END PLUS	10D @ 32" OC STAGGERED, TOP & BOTTOM, OPPOSITE SIDES
BEAMS	BUILT-UP BEAMS OF ENGINEERED LUMBER, FACE NAIL OPPOSITE SIDES	(2) ROWS @ 12" OC
	BUILT-UP HEADER, TWO PIECES W/ 1/2" SPACER	16D @16" OC ALONG EDGES
	BUILT-UP HEADER, TWO PIECES, NO 1/2" SPACER	3" x 0.131" NAILS @ 12" OC ALONG EDGES
	BEARING	TOENAIL W/ (2) 18D @ EACH END
	RIM JOIST TO SILL OR TOP PLATE	TOENAIL W/ 8D COMMON OR 10D BOX NAILS @ 6" OC
LOOR JOISTS	JOIST TO SILL OR GIRDER	TOENAIL W/ (3) 8D
	JOIST TO RIM JOIST	FACENAIL W/ (3) 16D
	BRIDGING TO JOIST	TOENAIL W/ (2) 8D
	I-JOIST TO BEARING PLATE	TOENAIL W/ (2) 8D - ONE INTO EACH SIDE AT LEAST 1 1/2" FROM THE END
	RIM JOIST TO I-JOIST	FACENAIL W/ (2) 10D BOX NAILS - ONE INTO EACH FLANGE
	SOLE PLATE TO LSL RIM BOARD	16D BOX NAILS @ 12" OC
	SINGLE JOIST HANGERS *	10D FACENAILS AND TOENAILS
	DOUBLE JOIST HANGERS *	16D FACENAILS AND TOENAILS
	TOP & SOLE PLATE TO STUD	END NAIL W/ (2) 16D
	STUD TO SOLE AND TOP PLATE	TOENAIL W/ (4) 8D
	DOUBLE TOP PLATES	FACENAIL W/ 16D @ 16" OC
	DOUBLE TOP PLATE LAP SPLICE	FACENAIL W/ (8) 16D
	TOP PLATE LAPS & INTERSECTIONS	FACENAIL W/ (2) 16D
	DOUBLE STUDS	FACENAIL W/ 16D @ 24" OC
	BUILT-UP CORNER STUDS	FACENAIL W/ 16D - 2 ROWS @ 24" OC
	STEEL "X" BRACING	FACENAIL W/ (2) 16D IN EACH TOP & BOTTOM PLATE & (1) 8D PER STUD
WALLS	SOLE PLATE TO JOIST OR BLOCKING	FACENAIL W/ 16D @ 16" OC
	SOLE PLATES TO JOIST OR BLOCKING AT BRACED WALL LINES, PERPENDICULAR TO FRAMING	FACENAIL W/ (3) 16D @ 16" OC ALONG BRACED WALL PANEL
	TOP PLATE TO JOIST OR BLOCKING AT BW LINES, PERPENDICULAR TO FRAMING	TOENAIL W/ 8D @ 6" OC ALONG BRACED WALL PANEL
	SOLE PLATES TO JOIST OR BLOCKING AT BW LINES PARALLEL TO FRAMING, BLOCKING @ 16" OC	FACENAIL W/ (3) 16D @ 16" OC ALONG BW PANEL & AT EACH BLOCK
	TOP PLATE TO JOIST OR BLOCKING AT BW LINES, PARALLEL TO FRAMING, BLOCKING @ 16" OC	TOENAIL W/ 8D @ 6" OC ALONG BW PANEL & AT EACH BLOCK
	NON-STRUCT. SIDING OVER STRUCT. SHEATHING	(1) 6D BOX NAIL IN EACH STUD
		· · · ·
	FIBER CEMENT PLANK SIDING	(1) 6D GALVANIZED NAIL IN EACH STUD

* JOIST HANGER NOTES: 1) NO JOIST HANGER NAILS ALLOWED FOR TOENAILS, 2) NO GUN NAILS OR SCREWS ALLOWED IN CONNECTORS, 3) TOENAILS SHALL ALWAYS BE A FULL 3" OR 3.5" NAIL

COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. 1/2"x2" BOLTS SHOULD THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

DUCT SEALING METHOD, PER IRC2018 W1103.3.2

N1103.2.2 (R403.2.2) SEALING (MANDATORY) DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF THIS CODE.

1. AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT

2. WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT. 3. CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURE LESS THAN 2 INCHES OF WATER COLUMN (500 Pa) PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.

DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:

1. POST CONSTRUCTION TEST: TOTAL LEAKAGE SHALL NOT BE LESS THAN OR EQUAL TO 4 CFM (113.3 L/MIN) PER 100FT² (9.29m²) OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. (25 Pa) ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST.

2. ROUGH-IN TEST: TOTAL AIR LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM (113.3 L/MIN) PER 100FT² (9.29m²) OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. (25 Pa) ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL AIR LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM (85 L/MIN) PER 100FT² (9.29m²) OF CONDITIONED FLOOR AREA. EXCEPTION: THE TOTAL LEAKAGE IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

OR DEVIATIONS ARE MADE FROM THESE PLANS THE CONTRACTOR SHALL NOTIFY THE APPROPRIATE AUTHORITY AND THE ENGINEER TO EVALUATE THE CHANGES AND MAKE ANY APPROPRIATE MODIFICATIONS TO THE PLANS. 2. WHERE DISCREPANCIES EXIST BETWEEN THE STANDARD COMMENTS, NOTES FOR THE DESIGN PROFESSIONAL OR THE CODE, THE MOST RESTRICTIVE SHALL APPLY. 3. THE CONTRACTUAL OBLIGATION OF THESE PLANS IS TO PROVIDE THE OWNER/BUILDER AND THE AHJ WITH A SET OF PLANS THAT MEET AHJ AND CODE REQUIREMENTS FOR A SINGLE SITE CONSTRUCTION PROJECT. UNLESS REQUESTED BY OUR CLIENT, CODE/AHJ MINIMUM DESIGNS WILL BE UTILIZED. ALSO, UNLESS REQUESTED BY THE OWNER, OUR FIRM CAN NOT AND WILL NOT BE AUTHORIZED TO VISIT THE SITE TO EVALUATE THE SITE OR ANY CONSTRUCTION FOR THIS PROJECT. IMPLEMENTATION OF ALTERNATES TO THE DESIGNS INCLUDING BUT NOT LIMITED TO PIER DESIGNS, FOUNDATION ALTERATIONS, OR ANY STRUCTURAL CHANGES NOT PROVIDED BY HD ENGINEERING OR A PROFESSIONAL REFERRED BY HD ENGINEERING SHALL RELEASE HD ENGINEERING FROM ALL LIABILITY ASSOCIATED WITH THIS DESIGN. 4. OUR FIRM HIGHLY RECOMMENDS THAT ANY SITE WITH GREATER THAN A 15% GRADE, ANY SITE WHERE A PREVIOUS STRUCTURE WAS LOCATED, OR ANY SITE WITH POTENTIAL FILL MATERIAL OR A POTENTIAL SOIL BEARING CAPACITY BELOW 1500 PSF SHOULD BE EVALUATED BY OUR FIRM OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACING FOOTINGS. THE ATTACHED PLANS HAVE BEEN DESIGNED WITH THE UNDERSTANDING THAT OUR FIRM HAS NOT AND CAN NOT VISIT OR INSPECT THE SITE WITHOUT WRITTEN CONSENT/REQUEST OF THE OWNER/BUILDER. DUE TO THIS FACT OUR FIRM CAN ONLY DESIGN THE ATTACHED PLANS TO CERTAIN CODE REQUIREMENTS WHICH ARE DETAILED THROUGHOUT THE PLAN AND ATTACHED DETAIL SHEETS, IF THE OWNER DESIRES GREATER THAN CODE DESIGNS THAT REQUEST MUST BE MADE CLEARLY AND IN WRITING PRIOR TO ENGINEERING OF THE PLAN. 5. DUE TO THE WIDE VARIETY OF SOIL CONDITIONS IN OUR AREA AND THE WIDE VARIETY OF PLASTICITY INDEX AND SOIL BEARING CAPACITIES OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY HD ENGINEERING OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF ANY "STANDARD" FOUNDATIONS .

FOUNDATION NOTES

REQUIREMENTS BASED ON ACTUAL SITE CONDITIONS 2. FOUNDATION WALLS SHALL BE DAMP-PROOFED PER IRC SECTION R406. 3. PROVIDE A MINIMUM 4" PERFORATED DRAIN AROUND USABLE SPACE BELOW GRADE OR OTHER EQUIVALENT MATERIALS PER IRC SECTION 405.1. THE PIPE SHALL BE COVERED WITH NOT LESS THAN 6" OF WASHED GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT TO THE EXTERIOR BELOW THE FLOOR LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT.

4. FOUNDATION DESIGN SHALL BE BASED ON A MINIMUM SOIL BEARING CAPACITY OF 1500 PSF. 5. FOOTINGS SHALL BE A MIN. OF 16" WIDE AND 8" DEEP W/ (2) #4 BARS CONTINUOUS, LOCATED A MIN. OF 3" CLEAR FROM BOTTOM. FOOTINGS SHALL BE A MINIMUM OF 36" BELOW GRADE FOR FROST PROTECTION.

6. COLUMN PADS SHALL BE A MINIMUM OF 24"X24"X8" WITH (3) #4 BARS EACH WAY. 7. FOUNDATION WALLS SHALL BE A MINIMUM 8" THICK W/ MINIMUM #4 BARS @ 24" O.C. HORIZONTAL AND VERTICAL W/ THE TOP BAR WITHIN 8" OF THE TOP OF THE WALL UNLESS NOTED OTHERWISE ON PLAN.

8. REINFORCEMENT SHALL LAP A MINIMUM OF 24"

9. INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB. 10. INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING, SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE BY A SEPARATION OF 1/2" 11. CONCRETE FLOOR SLABS ON GRADE, SHALL BE A MINIMUM 4" THICK OVER A MINIMUM 4" BASE OF SAND, GRAVEL, OR CRUSHED STONE. BASEMENT SLABS SHALL HAVE

A MIN. 6 MIL POLYETHYLENE OR APPROVED VAPOR RETARDER WITH JOINTS LAPPED NOT LESS THAN 6" SHALL BE PLACED BETWEEN THE FLOOR SLAB AND THE BASE COURSE

DESIGN.

SPACED NOT MORE THAN 3' ON CENTER AND WITHIN 12" OF EACH END PIECE PER IRC SECTION R403.1.6. 14. FOUNDATION WINDOW WELLS FOR SECONDARY MEANS OF EGRESS SHALL PROVIDE A MINIMUM 3'X3' HORIZONTAL AREA. 15. THE BASE OF ALL FOOTING EXCAVATIONS SHOULD BE FREE OF ALL WATER AND LOOSE MATERIAL PRIOR TO PLACING CONCRETE. CONCRETE SHOULD BE PLACED AS SOON AS POSSIBLE AFTER EXCAVATING SO THAT EXCESSIVE DRYING OR DISTURBANCE OF BEARING MATERIALS DOES NOT OCCUR. SHOULD THE MATERIALS AT BEARING LEVEL BECOME EXCESSIVELY DRY OR SATURATED, WE RECOMMEND THAT THE AFFECTED MATERIAL BE REMOVED PRIOR TO PLACING CONCRETE 16. IT IS RECOMMENDED THAT ALL FOOTING EXCAVATIONS BE EVALUATED AND TESTED BY A GEOTECHNICAL ENGINEER IMMEDIATELY PRIOR TO PLACEMENT OF FOUNDATION CONCRETE. UNSUITABLE AREAS IDENTIFIED AT THIS TIME SHOULD BE CORRECTED. CORRECTIVE PROCEDURES WOULD BE DEPENDENT UPON CONDITIONS ENCOUNTERED AND MAY INCLUDE DEEPENING OF FOUNDATION ELEMENTS, OR UNDERCUTTING OF UNSUITABLE MATERIALS AND REPLACEMENT WITH ENGINEERED FILL.

STAIRWAY NOTES:

1. STAIRWAYS SHALL PROVIDE A MAXIMUM 7 3/4" RISE AND MIN. 10" RUN. 2. PROVIDE MINIMUM 36" GUARDRAILS ON THE OPEN SIDES OF RAISED FLOORS, PORCHES AND BALCONIES. MINIMUM 34" GUARDRAILS ON THE OPEN SIDES OF STAIRWAYS LOCATED MORE THAN 30" ABOVE THE FLOOR OR GRADE BELOW. GUARDRAIL ENCLOSURES SHALL HAVE INTERMEDIATE RAILS OR ORNAMENTAL PATTERNS THAT DO NOT ALLOW PASSAGE OF A SPHERE 4" IN DIAMETER

3. EACH STAIRWAY OF 3 OR MORE RISERS SHALL PROVIDE A CONTINUOUS HANDRAIL ON AT LEAST ONE SIDE BETWEEN 34" AND 38" ABOVE THE NOSING OF THE THREADS. 4. HANDRAILS SHALL HAVE A CIRCULAR CROSS-SECTION OF 1 1/4" MINIMUM TO 2" MAXIMUM OR OTHER APPROVED GRASPABLE SHAPE PER IRC SECTION R311.7.8.5

5. PROVIDE A MINIMUM 6'-8" OF HEADROOM CLEARANCE IN STAIRWAYS. 6. ENCLOSED ACCESSIBLE SPACE UNDER STAIRWAYS SHALL HAVE WALLS AND THE UNDERSIDE OF THE STAIR AND LANDING PROTECTED WITH 1/2" GYPSUM BOARD ON ENCLOSURE SIDE 7. WINDERS SHALL PROVIDE A MINIMUM TREAD OF AT LEAST 6" AT ANY POINT WITHIN CLEAR WIDTH OF STAIRS. WINDER TREAD PROPORTION TO COMPLY WITH IRCR311.7.5.2.1. <u>GLAZING NOTES:</u> 1. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPERABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 24" ARCH OF THE DOOR IN A CLOSED POSITION AND WHOSE BOTTOM EDGE IS WITHIN 60" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 60" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS AND WHIRLPOOLS, GLAZING IN FIXED OR OPERABLE PANELS EXCEEDING 9 S.F. AND WHOSE BOTTOM EDGE IS LESS THAN 18" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 36" 2. IN DWELLING UNITS, WHERE THE OPENING OF AN OPERABLE WINDOW IS LOCATED MORE THAN 72 INCHES ABOVE THE FINISHED GRADE OR SURFACE BELOW, THE LOWEST PART OF THE CLEAR OPENING OF THE WINDOW SHALL BE A MINIMUM OF 24 INCHES ABOVE THE FINISHED FLOOR OF THE ROOM IN WHICH THE WINDOW IS LOCATED. OPERABLE SECTIONS OF WINDOWS SHALL NOT PERMIT OPENINGS THAT ALLOW PASSAGE OF A 4 INCH DIAMETER SPHERE WHERE SUCH OPENINGS ARE LOCATED WITHIN 24 INCHES OF THE FINISHED FLOOR.

FRAMING NOTES:

1. ALL LUMBER SIZES ARE FOR DOUGLAS FIR-LARCH UNLESS OTHERWISE NOTED. 2. ALL HEADERS TO BE A MINIMUM OF (2) #2-2X10'S UNLESS OTHERWISE NOTED.

3. BLOCK CANTILEVERS, DOOR JAMBS, AND OVER BEAMS. 4. ALL HEADERS/BEAMS TO BEAR ON A MINIMUM OF (1) 2X4 POSTS UNLESS NOTED OTHERWISE. 5. INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE 6. WHERE JOISTS RUN PARALLEL TO FOUNDATION WALLS, SOLID BLOCKING FOR A MINIMUM OF (2) JOIST SPACES SHALL BE PROVIDED AT A MAXIMUM OF 4' CENTERS TO TRANSFER LATERAL LOADS ON THE WALL TO THE FLOOR DIAPHRAGM. THE BLOCKING SHALL BE SECURELY NAILED TO THE JOISTS AND FLOORING. NAIL JOISTS AND

BLOCKING TO SILL PLATE WITH (4) 10D NAILS. 7. IF DUCTS ARE INSTALLED IN THE FIRST JOIST SPACE(S), NAIL 2X4'S FLAT AT 4' CENTERS WITHIN THE JOIST SPACE(S) AND THEN PROVIDE SOLID BLOCKING, INSTALLED UPRIGHT, IN THE NEXT TWO JOIST SPACES. SECURE THE 2X4'S TO THE SILL PLATE WITH (4) 10D NAILS. 8. ALL SILLS AND SLEEPERS SUPPORTED ON CONCRETE OR MASONRY AND FURRING ATTACHED TO CONCRETE OR MASONRY SHALL BE OF DECAY RESISTANT MATERIALS.

9. JOISTS UNDER BEARING PARTITIONS SHALL BE SIZED TO CARRY THE DESIGN LOAD IN ACCORDANCE WITH IRC SECTION R502.4. 10. JOISTS FRAMING FROM OPPOSITE SIDES OVER BEARING SUPPORTS SHALL LAP A MINIMUM OF 3" AND SHALL BE NAILED TOGETHER WITH A MINIMUM 10D FACE NAILS. 11. JOISTS FRAMING INTO A WOOD GIRDER OR BEAM SHALL BE SUPPORTED BY APPROVED FRAMING ANCHORS OR ON MINIMUM 2"X2" LEDGER STRIPS. 12. HEADER AND TRIMMERS SHALL BE OF SUFFICIENT CROSS SECTION TO SUPPORT THE FLOOR FRAMING. TRIMMER JOISTS SHALL BE DOUBLED WHEN THE HEADER IS

SUPPORTED MORE THAN 3' FROM THE TRIMMER JOIST BEARING. WHEN THE HEADER SPAN EXCEEDS 4', THE HEADER AND TRIMMER SHALL BE DOUBLED. 13. JOISTS AT SUPPORTS SHALL BE SUPPORTED LATERALLY AT THE ENDS BY FULL-DEPTH SOLID BLOCKING NOT LESS THAN 2" NOMINAL THICKNESS OR BY ATTACHMENT TO A HEADER, BAND OR RIM JOIST OR TO AN ADJOINING STUD OR OTHERWISE PROVIDED WITH LATERAL SUPPORT TO PREVENT ROTATION.

14. ALL WALL COVERINGS TO COMPLY WITH IRC SECTION 702 AND 703 15. ALL RAFTER / COLLAR TIES TO COMPLY WITH IRC SECTIONS 804

16. ALL RAFTERS TO HAVE 2x4 COLLAR TIES @ 48" OC IN UPPER 1/3 OF DISTANCE BETWEEN CEILING AND ROOF

17. BLOCKING BETWEEN JOISTS UNDER A PERPENDICULAR LOAD-BEARING WALL IS NOT REQUIRED 18. BOTTOM OF ALL FLOOR ASSEMBLIES SHALL BE PROVIDED WITH A 1/2" GYPSUM WALLBOARD MEMBRANE (IF REQUIRED BY LOCAL CODE)

19. I-JOIST AND FLOOR TRUSS SYSTEMS SHALL BE FIRE PROTECTED PER IRC AS ADOPTED BY AHJ 20. STUDS SHALL BE CONTINUOUS FROM THE FLOOR TO THE ROOF/ CEILING DIAPHRAGM PER IRC 602.3

CONCRETE NOTES:

. CONCRETE SHALL BE AIR-ENTRAINED (5%-7%) WITH A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS, 3000 PSI FOR BASEMENT AND FOUNDATION WALLS AND 3500 PSI FOR PORCHES, CARPORTS AND GARAGE FLOOR SLABS.

EMERGENCY EGRESS AND RESCUE NOTES

1. PROVIDE ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 S.F. WITH A MINIMUM OPENABLE HEIGHT OF 24" AND WIDTH OF 21". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 44" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP. 2. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR INCLUDING BASEMENTS. ALARMS SHALL BE INTERCONNECTED IN SUCH A MANNER THAT THE ACTIVATION OF ONE ALARM WILL ACTIVATE ALL OF THE ALARMS IN THE DWELLING. 3. PROVIDE CARBON MONOXIDE ALARMS AS REQUIRED PER IRC. CARBON MONOXIDE ALARMS SHALL BE INSTALLED OUTSIDE OF EACH SEPARATE SLEEPING AREA. WHERE FUEL-BURNING APPLIANCES ARE LOCATED WITHIN A BEDROOM OR ITS ATTACHED BATHROOM, A CARBON MONOXIDE ALARM SHALL BE INSTALLED IN THE BEDROOM.

GARAGE NOTES:

1. THE GARAGE FLOOR SHALL SLOPE TOWARDS THE GARAGE DOORWAYS OR SLOPE TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES DIRECTLY TO THE EXTERIOR ABOVE GRADE. 2. DOORS BETWEEN THE GARAGE AND DWELLING - MINIMUM 1 3/8" SOLID WOOD, SOLID OR HONEY-COMBED CORE STEEL DOOR NOT LESS THAN 1 3/8" THICK, OR 20 -

MINUTE FIRE - RATED EQUIPPED WITH SELF CLOSING DEVICE PER IRC2018 R302.5.1. 3. GARAGE VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC2018 R301.2.1

4. THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 5/8" GYPSUM BOARD APPLIED TO THE GARAGE SIDE. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE FLOOR CEILING ASSEMBLY SHALL BE PROTECTED WITH MINIMUM 5/8" TYPE X GYPSUM BOARD ON THE GARAGE CEILING. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH 5/8" GYPSUM BOARD OR EQUIVALENT.

5. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING ATTACHED WITH 1 3/4"X.120" NAILS AT 7" CENTERS STAGGERED WITH (7) 3 1/4"X.120" NAILS THRU THE JAMB INTO THE HEADER, MINIMUM 2X8 HEADER

FOR ATTACHMENT OF COUNTER BALANCE SYSTEM. 6. ANY ATTACHED GARAGE TO THE MAIN HOUSE SHALL BE PROVIDED WITH A SINGLE HEAT DETECTOR. HEAT DETECTOR SHALL BE HARDWIRED AND INTERCONNECTED WITH THE HOUSEHOLD SMOKE ALARM SYSTEM. HEAT DETECTOR SHALL BE LISTED FOR THE AMBIENT ENVIRONMENT AND INSTALLED PER MANF. INSTRUCTIONS.

MECHANICAL/INSULATION: 1. BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2018 IECC.

2. BUILDING THERMAL ENVELOPE IS REQUIRED TO BE SEALED PER 2018 IRC N1102.4.1 & TABLE N1102.4.1.1.

VENTILATION I. ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH. WITH 1/8" TO 1/4" OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/150 OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS AREA LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED THE REQUIRED AREA MAY BE REDUCED TO 1/300.

1. PLANS SHALL COMPLY WITH THE 2018 INTERNATIONAL RESIDENTIAL CODE, IECC AS ADOPTED BY AHJ, AND ALL AMENDMENTS AS ADOPTED BY THE AHJ. IF ANY CHANGES

1. THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION RESIDENTIAL FOUNDATION STANDARD IN LIEU OF ENGINEERING REPORT

12. FLOOR SLABS SUPPORTED BY FILL CONSISTING OF MORE THAN 24" OF GRANULAR FILL OR 8" OF EARTH SHALL BE REINFORCED PER A SEPARATE ENGINEERING

13. BASEMENT FOUNDATION SILL PLATES SHALL BE BOLTED TO THE FOUNDATION W/ A MINIMUM OF 1/2" ANCHOR BOLTS EMBEDDED AT LEAST 7" INTO THE CONCRETE AND

ONTAINED HEREIN MAY RESULT IN ILITY UNDER APPLICABLE LAW. ENGINEERI Ī

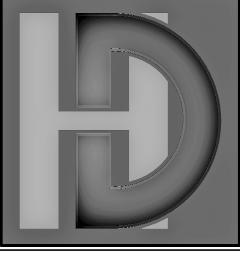
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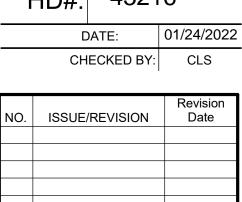
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GENERAL NOTES

TABLE R602.3(1) FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

									THE DWELLING SHALL COMPLY WITH THE FOLLOWIN	G LOAD CONDITIONS
ITEM	DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF ^{a,b,c} FASTENER	SPACING OF FASTENERS	ITEM	DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF ^{a,b} FASTENER		SPACING OF FASTENERS (INCHES)h INTERMEDIATE c. e SUPPORTS (INCHES)	AREA	MIN MIN DEAD LIVE LOAD LOAD
1	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOE NAIL	ROOF 4-8D BOX (2 1/2" X 0.113")	TOE NAIL		WOOD STRUCTURAL PANELS, SUBFLOOR, ROOF AND INTERIOR [SEE TABLE R602.3(3) FOR WOOD STRUC	R WALL SHEATHING TO FRAMING AND PARTIC TURAL PANEL EXTERIOR WALL SHEATHING			EXTERIOR BALCONIES DECKS, STAIRS	10 60 10 40
2	CEILING JOISTS TO PLATE, TOE NAIL	3-8D (2 1/2" X 0.113") 3-10D (3"X0.128") 3-3"X 0.131" NAILS	PER JOIST, TOE NAIL	30	3/8"- 1/2"	6D COMMON (2"X 0.113" NAIL (SUBFLOO 8D COMMON (2 1/2" X 0.131 NAIL (ROOF); or 3/8" X 0.113" NAIL (ROOF) j		6 12 f	CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE 3:12 OR LESS	10 10
3	CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS (SEE SECTION R802.5.2 AND TABLE R802.52	4-10D BOX (3"X 0.128") 3-16D COMMON (3 1/2"X 0.162") 4-3"X 0.131"NAILS	FACE NAIL	31	19/32" - 1"	8D COMMON NAIL (2 1/2" X 0.131; or RSRS- 0.113) NAIL ROOF ;	-01; 2 3/8" X	6 12 f	CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE OVER 3:12 CEILING JOISTS / ATTICS WITH STORAGE - DOOR	10 10
4	CEILING JOIST ATTACHED TO PARALLEL RAFTER (HEEL JOINT) SEE SECTION R802.5.2 AND TABLE R802.5.2)	TABLE R802.5.2	FACE NAIL	32	1 1/8" - 1 1/4"	10D COMMON NAIL (3" X 0.148) NAIL; or 8	D (2 1/2" X	6 12	PULL DOWN LADDER ACCESS ROOMS: NON-SLEEPING	10 20 10 40
5	COLLAR TIE TO RAFTER, FACE NAIL OR 1 1/4" X 20GA. RIDGE STRAP TO RAFTER	4-10D BOX (3" X 0.128") 3-10D COMMON (3" X 0.148") 4-3" X 0.131" NAILS	FACE NAILS EACH RAFTER			0.131") DEFORMED NAIL			ROOMS: SLEEPING ROOF: LIGHT ROOF COVERING	10 30 10 20
6	RAFTER OR ROOF TRUSS TO PLATE	3-16D BOX NAILS (3 1/2" X0.135") 3-10D COMMON NAILS (3" X 0.148" 4-10D BOX (3" X 0.128" 4-3" X0.131" NAILS	2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS ¹	33	1/2" STRUCTURAL CELLULOSE FIBERBOARD SHEATHING	1 1/2" GALVANIZED ROOF NAIL, 7/16" HEAD OR 1 1/4" LONG 16GA. STAPLE WITH 7/ CROWN	16" OR 1"	3 6	ROOF: HEAVY ROOF COVERING / CONCRETE / TILE / SLATE GUARDRAILS, HANDRAILS	20 20 200# LL NORMAL
	ROOF RAFTERS TO RIDGE, VALLEY OR HIP RAFTERS OR ROOF	4-16D(3 1/2" X 0.135"); OR 3-10D COMMON (3" X 0.148") 4-10D BOX (3" X 0.128"); OR 4-3" X 0.131" NAILS		34	25/32" STRUCTURAL CELLULOSE FIBERBOARD SHEATHING	1 3/4" GALVANIZED ROOF NAIL, 7/16" HEAD OR 1 1/2" LONG 16GA. STAPLE WITH 7/16" OR		3 6	HEAVY ROOF COVERING MATERIAL (TILE, CONCRETE, S BE USED UNLESS 20 PSF DEAD LOAD AND HEAVY ROOF ROOF PLAN. IF HEAVY ROOFING IS TO BE USED AND NO PLAN NOTIFY ENGINEER PRIOR TO ANY CONSTRUCTION	F IS NOTED ON THE OT NOTED ON THE RO
/	RAFTER TO MINIMUM 2" RIDGE BEAM	3-16D(3 1/2" X0.135"); OR 2-16D COMMON (3 1/2" X0.162") 3-10D BOX (3" X 0.128"); OR 3-3" X 0.131" NAILS	TOE NAIL	35	1/2" GYPSUM SHEATHING d	1 1/2" GALVANIZED ROOF NAIL, STAPLE GA 11/2" LONG; 1 1/4" SCREWS, TYPE W	V or S	7 7	FOUNDATION AND SITE WORK. IF THE PLAN HAS BEEN ROOF LOADS IT WILL BE NOTED IN THE ROOF NOTES O	DESIGNED FOR HEAV
		WALL 16D (3 1/2" X 0.162")	24" OC FACE NAIL	36	5/8" GYPSUM SHEATHING d	1 3/4" GALVANIZED ROOF NAIL; STAPLE GA 1 5/8" LONG; 1 5/8" SCREWS, TYPE V		7 7		
8	STUD TO STUD (NOT BRACED WALL PANELS)	10D BOX (3" X 0.128"); OR 3" X 0.131" NAILS	16" OC FACE NAIL		WOOD STRUCTURAL PANELS, CO	OMBINATION SUBFLOOR UNDERLAYMENT TO	O FRAMING			
	STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL		12" OC FACE NAIL	37		6D DEFORMED (2" X 0.120") NAIL (6 12	<u>COLUMN SCHI</u>	LUULE
9	CORNERS (AT BRACED WALL PANELS)	16D BOX (3 1/2" X 0.135"); OR 3" X 0.131" NAILS 16D COMMON (3 1/2" X 0.162")	12" OC FACE NAIL	31	3/4" AND LESS	8D COMMON (2 1/2" X 0.131") NA		0 12	BASED ON FOOTING SIZE (ASSUME	1500 PSF SOIL)
10	BUILT-UP HEADER (2" TO 2" HEADER WITH 1/2" SPACER)	16D COMMON (3 1/2" X 0.162")	16" OC EACH EDGE FACE NAIL	38	7/8" - 1"	8D COMMON (2 1/2" X 0.131") NAIL 8D DEFORMED (2 1/2" X 0.120") N/		6 12	PAD SIZE REINFORCEMENT COL. MIN.	COL. MAX TYPE LOAD
		16D BOX (3 1/2" X 0.135") 5-8D BOX (2 1/2" X 0.113") or 4-8D COMMON	12" OC EACH EDGE FACE NAIL			10D COMMON (3" X 0.148") NAIL 0	OR		24x24x12 (4) #4 BARS E/W 3"	SCH40 6K
11	CONTINUOUS HEADER TO STUD	(2 1/2" X 0.113") (2 1/2" X 0.131") 4-10D BOX (3" X 0.128")	TOE NAIL	39	1 1/8" - 1 1/4"	8D DEFORMED (2 1/2" X 0.120") N		υ 12	30x30x12 (5) #4 BARS E/W 3"	SCH40 9.4K
		16D COMMON (3 1/2" X 0.162")	16" OC FACE NAIL	For SI: 1 inc	h = 25.4mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1 ksi = 6.89	95 MPa.			36x36x12 (6) #4 BARS E/W 3"	SCH40 13.5k
12	TOP PLATE TO TOP PLATE	10D BOX (3" X 0.128") OR 3" X 0.131" NAILS	12" OC FACE NAIL						42x42x14 (7) #4 BARS E/W 3 1/2"	SCH40 18.4k
13	DOUBLE TOP PLATE SPLICE	8-16D COMMON (3 1/2" X 0.162"); or 12-16D BOX (3 1/2" X 0.135"); or 12-10D BOX (3" X 0.128"); or 12-3" X 0.131" NAILS	FACE NAIL ON EACH SIDE OF END JOINT (MINIMUM 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT)		TABLE R 602.3(5) SIZE, H	EIGHT, AND SPAC			54x54x16 (9) #4 BARS E/W 3 1/2"	SCH40 24.0k SCH40 30.4k
	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING	16D COMMON (3 1/2" X 0.162")	16" OC FACE NAIL		BEARING WALLS			NON-BEARING WALLS	60x60x18 (10) #4 BARS E/W 3 1/2"	SCH40 37.5k
14	(NOT AT BRACED WALL PANELS	16D BOX (3 1/2" X 0.135"); OR 3" X 0.131" NAILS	12" OC FACE NAIL		UNSUPPORTED WHERE SUPPORTING A WHERE SU	UPPORTING WHERE SUPPORTING WHE		LATERALLY LATER	RTED STUD	
15	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING (NOT AT BRACED WALL PANELS	3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162"); or 4-3" X 0.131" NAILS	3, 2, OR 4 EACH 16" OC FACE NAIL	STUD SIZ (IN)	(feet) ASSEMBLY OR A ROOF- HABITABLE ATTIC ASSEME ASSEMBLY, ONLY HABITAB	OR, PLUS A TWO FLOORS, PLUS A ONE CEILING ROOF-CEILING BLY OR A ASSEMBLY OR A BLE ATTIC HABITABLE ATTIC SLY (inches) ASSEMBLY (inches)	E FLOOR HEIGHTa (inches)	HEIGHT a HEIG (feet) (fee		OM FLANGE OF THE BE ED IN THE BOTTOM FL F THE PLATE. 1/2" X 2"
16	TOP OR BOTTOM PLATE TO STUD	4-8D BOX (2 1/2" X 0.113"); or 3-16D BOX (3 1/2" X0.135"); or 4-8D COMMON (2 1/2" X0.131");or 4-10D BOX (3" X0.128"); or 3-3" X 0.131" NAILS	TOE NAIL						EACH OF THE HOLES. THE POST CAP MAY BE ACCORDANCE WITH AWS D1.1-92 AS AN ALTER INSPECTED BY AN AWS-CERTIFIED INSPECTOR	WELDED TO THE STEE NATIVE, AND WOULD N
		3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162"); or 3-10D BOX (3" X0.128");or 3-3" X 0.131" NAILS	END NAIL			-				
17	TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	3-10D BOX (3" X 0.128"); or 2-16D COMMON (3 1/2" X0.162"); or 3-3" X 0.131" NAILS	FACE NAIL	2x3 ^b				10 16	6	
18	1" BRAVE TO EACH STUD AND PLATE	3-8D BOX (2 1/2" X 0.113"); or 2-8D COMMON (2 1/2" X0.131") or 2-10D BOX (3" X 0.128"); or 2 STAPLES 1 3/4"	FACE NAIL	2x4 3x4	10 24 2	16 _c 24 16	24 24	14 24 14 24 14 24	ENGINEERED LU	MBER
19	1" X 6" SHEATHING TO EACH BEARING	3-8D BOX (2 1/2" X 0.113"); or 2-8D COMMON (2 1/2" X0.131") or 2-10D BOX (3" X 0.128"); or 2 STAPLES 1" CROWN, 16GA., 1 3/4" LONG	FACE NAIL	2x5 2x6	10 24 2	24 24 16	24 24	16 24 20 24	4 MIN. DESIGN REQUIREMENTS	3
20	1" X 8" AND WIDER SHEATHING TO EACH BEARING	3-8D BOX (2 1/2" X 0.113"); or 3-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 3 STAPLES, 1" CROWN, 16GA., 1 3/4" LONG WIDER THAN 1" X 8" 4-8D BOX (2 1/2" X 0.113"); or 3-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 4 STAPLES, 1" CROWN, 16GA., 1 3/4" LONG	FACE NAIL	a. LISTED H ON NOT LES UNSUPPOR PRACTICES. b. SHALL N	CH = 25.4mm, 1 FOOT = 304.8mm HEIGHTS ARE DISTANCES BETWEEN POINTS OF LATERAL SUPPOF SS THAN ONE SIDE OR BRIDGING SHALL BE INSTALLED NOT GREA TED HEIGHT ARE PERMITTED WHERE IN COMPLIANCE WITH EXCE NOT BE USED IN EXTERIOR WALLS TABLE ATTIC ASSEMBLY SUPPORTED BY 2X4 STUDS IS LIMITED TO	ATER THAN 4 FEET APART MEASURED VERTIC EPTION 2 OF SECTION R602.3.1 OR DESIGNED	CALLY FROM EITHER EN IN ACCORDANCE WITH	ND OF THE STUD. INCREASES IN H ACCEPTED ENGINEERING	Fb(psi) E (psi) LVL 2600 1.8x10 GLULAM 2400 1.8x10	F _v (psi) 285 190
		FLOOR			TO 2X6 OR THE STUDS SHALL BE DESIGNED IN ACCORDANCE WI			,	PARALAM 2600 2.0x10	290
21	JOIST TO SILL, TOP PLATE OR GIRDER	4-8D BOX (2 1/2" X 0.113"); or 3-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 3-3" X 0.131: NAILS	TOE NAIL		IMUM MECHANICAL EQUI				RAL / VAULTED CEILING	
22	RIM JOIST, BAND JOIST OR BLOCKING TO SILL OR TOP PLATE (ROOF APPLICATIONS ALSO)	8D BOX (2 1/2" X 0.113") 8D COMMON (2 1/2" X 0.131"); or 10D BOX(3" X0.128") or 3-3" X 0.131" NAILS	4" OC TOE NAIL 6" OC TOE NAIL	VAL	UES BY COMPONENT, PE	R IRC2018 N1103.	<u>6.1</u>		NG AND INSULATION 38 INSULATION REQUIRED, <u>SEE DETAIL 14/S-1.2</u>	
23	1" X 6" SUBFLOOR OR LESS TO EACH JOIST	3-8D BOX (2 1/2" X 0.113"); or 2-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 2 STAPLES, 1" CROWN, 16GA., 1 3/4" LONG	FACE NAIL		FAN LOCATION MINIMUM (CFM) CFM/	AIR FLOW RATE WATT MAXIMUM (CFM)	BETV NOTE	WEEN THE TOP OF THE INSULATION E: RAFTER SIZES SPECIFIED ON PLA	LY TO THE BOTTOM OF THE RAFTERS, A MINIMUM 1" AIR SPACE AND THE SHEATHING FOR VENTILATION (R806.3) NS ARE THE MINIMUM REQUIRED FOR STRUCTURAL PURPOSE	
24	2" SUBFLOOR TO JOIST OR GIRDER	3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162")	BLIND AND FACE NAIL		RANGE HOOD ANY 2.8 CFM	M/WATT ANY M/WATT ANY	IF FU OR A ADDI	ADEQUATE FURRING SHALL BE USED ITION, IF THE RAFTER SIZE IS INCRE/	TE FOR MINIMUM INSULATION VALUE, RAFTER SIZES WILL NEED TO OBTAIN THE MINIMUM JOIST DEPTH FOR THE REQUIRED IN ASED IT SHALL BE VERIFIED THAT THE RIDGE BE A MINIMUM OF	ISULATION. IN
25	2" PLANKS (PLANK & BEAM-FLOOR AND ROOF)	3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162")	AT EACH BEARING, FACE NAIL		BATHROOM UTILITY FAN 10 1.4 CFM	M/WATT ANY M/WATT <90		GER THAN THE RAFTERS BEING REC		2x12
26	BAND OR RIM JOIST TO JOIST	3-16D COMMON (3 1/2" X 0.162"); or 4-10D BOX (3" X0.128") or 4-3" X 0.131" NAILS; or 4-3" X 14GA. STAPLES, 7/16" CROWN								R-38, 10 1/4"
		20D COMMON (4" X 0.192"); or	NAIL EACH LAYER AS FOLLOWS: 32" OC AT TIP AND BOTTOM AND STAGGERED 24" OC FACE NAIL AT TOP AND BOTTOM	<u>MI</u>	NIMUM INSULATION & FE	<u>NSTRATION VALU</u>	<u>JES BY C</u>	<u>OMPONENT, F</u>	<u> PER IRC2018 N1102.1.2</u>	
27	BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS	10D BOX (3" X 0.128"); or 3" X 0.131" NAILS	24" OC FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES	VALUES E	BELOW ARE PER 2018 IECC, ACTUAL VALUES MAY VARY BASED ON ALTERNATE ENERGY COMPL	LIANCE PATH CHOSEN (IN JURISDITIONS WHERE ALTERNATIVE PA	THS ARE AVAILABLE)			-
28	LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	AND: 2-20D COMMON (4" X 0.192"); or 3-10D BOX (3" X 0.128; or 3-3" X 0.131" NAILS 4-16D BOX (3 1/2" X 0.135"): or 3-26D COMMON (3 1/2" X 0.162"); or	FACE NAIL AT END AND AT EACH SPLICE AT EACH JOIST OR RAFTER, FACE NAIL	CLIMATE ZO	U-FACTOR U-FACTOR FENSTRATION DOOR U-V	ALUE DOOR OTALOL RUALOL III	OOD FRAMED FLOO ALL R-VALUE R-VAL	LUE WALL R-VALUE & DEPTH	WALL R-VALUE OUTSIDE R-VALUE OTHER) R-VALUE	
29	BRIDGING OR BLOCKING TO JOIST	4-10D BOX (3" X 0.128"); or 4-3" X 0.131" NAILS 2-10D BOX (3" X 0.128"): or 2-8D COMMON	EACH END, TOE NAIL	2) RE	JILDING THERMAL ENVELOPE IS REQUIRED TO BE SEALED WITH A	AN AIR BARRIER AS PER N1102.4.1 OF THE 20 IWEEN THE CONDITIONED SPACE AND UNCO	NDITIONED SPACE	OR 13 CAVITY	OR 13 CAVITY 8 6	
	ARE SMOOTH-COMMON, BOX OR DEFORMED SHANKS EXCEPT WHERE OTHERWISE STATED. NAILS U	(2 1/2" X 0.131" Or 2-3" X 0.131") NAILS		3) ALI	L DUCTS, AIR HANDLERS, FILTER BOXES, AND BUILDING CAVITIES	SUSED AS DUCTS SHALL BE SEALED AS PER	N1103.2 OF THE 2018 IF		E DI ANS REQUIRE THAT THE CONTRACTOR POSSESSES COMP	

a. ALL NAILS ARE SMOOTH-COMMON, BOX OR DEFORMED SHANKS EXCEPT WHERE OTHERWISE STATED. NAILS USED FOR FRAMING AND SHEATHING CONNECTIONS SHALL HAVE MINIMUM AVERAGE BENDING YIELD STRENGTHS AS SHOWN: 80 KSI FOR SHANK DIAMETER OF 0.192 INCH (20D COMMON), NAILS FOR SHANK DIAMETERS LARGER THANK 0.142 INCH BUT NOT LARGER THANK 0.177 INCH, AND 100 KSI FOR SHANK DIAMETER OF 0.142 INCH OR LESS. b. STAPLES ARE 16 GAGE WIRE AND HAVE A MINIMUM 7/16 - INCH ON DIAMETER CROWN WIDTH.

C. NAILS SHALL BE SPACED AT NOT MORE THAN 6 INCHES ON CENTER AT ALL SUPPORTS WHERE SPANS ARE 48 INCHES OR GREATER. d. FOUR-FOOT BY 8-FOOT OR 4-FOOT BY 9-FOOT PANELS SHALL BE APPLIED VERTICALLY.

e. SPACING OF FASTENERS NOT INCLUDED IN THIS TABLE SHALL BE BASED ON TABLE R602.3(2). f. FOR REGIONS HAVING BASIC WIND SPEED OF 110 MPH OR GREATER, 8D DEFORMED (2 1/2" X 0.120) NAILS SHALL BE USED FOR ATTACHING PLYWOOD AND WOOD STRUCTURAL PANEL ROOF SHEATHING TO FRAMING WITHIN MINIMUM 48-INCHES DISTANCE FROM GABLE END WALLS, IF MEAN ROOF

HEIGHT IS MORE THAN 25 FEET, UP TO 35 FEET MAXIMUM. g. FOR REGIONS HAVING BASIC WIND SPEED OF 100 MPH OR LESS, NAILS FOR ATTACHING WOOD STRUCTURAL PANEL ROOF SHEATHING TO GABLE END WALL FRAMING SHALL BE SPACED 6 INCHES ON CENTER. WHEN BASIC WIND SPEED IS GREATER THAN 100 MPH, NAILS FOR ATTACHING PANEL ROOF SHEATHING TO INTERMEDIATE SUPPORTS SHALL BE SPACED 6 INCHES ON CENTER FOR MINIMUM 48-INCH DISTANCE FROM RIDGES, EAVES AND GABLE END WALLS; AND 4 INCHES ON CENTER TO GABLE END WALL FRAMING. h. GYPSUM SHEATHING SHALL CONFORM TO ASTM C 1396 AND SHALL BE INSTALLED IN ACCORDANCE WITH GA 253. FIBERBOARD SHEATHING SHALL CONFORM TO ASTM C 208. I. SPACING OF FASTENERS ON FLOOR SHEATHING PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING AND AT ALL FLOOR PERIMETERS ON ROOF SHEATHING PANEL EDGES APPLIES TO PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING. BLOCKING OF FASTENERS ON ROOF SHEATHING PANEL EDGES APPLIES TO PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING. BLOCKING OF FASTENERS ON ROOF SHEATHING PANEL EDGES APPLIES TO PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING. BLOCKING OF FASTENERS ON ROOF SHEATHING PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING. BLOCKING OF ROOF OR FLOOR SHEATHING PANEL EDGES PERPENDICULAR TO THE FRAMING MEMBERS NEED NOT BE PROVIDED EXCEPT AS REQUIRED BY OTHER PROVISIONS OF THIS CODE. FLOOR PERIMETER SHALL BE SUPPORTED BY FRAMING MEMBERS OR SOLID BLOCKING. J. WHERE A RAFTER IS FASTENED TO AN ADJACENT PARALLEL CEILING JOIST IN ACCORDANCE WITH THIS SCHEDULE, PROVIDE TWO TOE NAILS ON ONE SIDE OF THE RAFTER AND TOE NAILS FROM CEILING JOIST TO TOP PLATE IN ACCORDANCE WITH THIS SCHEDULE. THE TOE NAIL ON THE OPPOSITE SIDE OF THE RAFTER SHALL NOT BE REQUIRED.

CONTINUED TABLE R602.3(1) FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

BUILDER'S PLANS: THE TERM "BUILDER'S PLANS" REFERS TO A CERTAIN LEVEL OF DEVELOPMENT OF THE DRAWINGS. AS THE NAME IMPLIES, THESE PLANS REQUIRE THAT THE CONTRACTOR POSSESSES COMPETENCE IN RESIDENTIAL CONSTRUCTION AND A THOROUGH UNDERSTANDING OF THE INTERNATIONAL RESIDENTIAL CODE (IRC). THE CONTRACTOR WARRANTS TO HD ENGINEERING & DESIGN THAT HE POSSESSES THE PARTICULAR COMPETENCE AND SKILL IN CONSTRUCTION NECESSARY TO BUILD THIS PROJECT WITHOUT FULL ENGINEERING AND DESIGN SERVICES, AND FOR THAT REASON THE CONTRACTOR OR HOME OWNER HAS RESTRICTED THE SCOPE OF PROFESSIONAL SERVICES. THE CONSTRUCTION DOCUMENTS PROVIDED BY THE LIMITED SERVICES SHALL BE TERMED "BUILDER'S PLANS" IN RECOGNITION OF THE CONTRACTOR'S SOPHISTICATION. ALTHOUGH HD ENGINEERING & DESIGN HAVE PERFORMED THEIR SERVICES WITH DUE CARE AND DILIGENCE, WE CANNOT GUARANTEE PERFECTION. ANY AMBIGUITY OR DISCREPANCY DISCOVERED BY THE USE OF THESE PLANS SHALL BE REPORTED IMMEDIATELY TO HD ENGINEERING. CONSTRUCTION MAY REQUIRE THAT THE CONTRACTOR ADAPT THE "BUILDER'S PLANS" TO THE FIELD CONDITIONS ENCOUNTERED AND MAKE LOGICAL ADJUSTMENTS IN FIT, FORM, DIMENSION AND QUANTITY. CHANGES MADE FROM THE PLANS WITHOUT THE CONSENT OF HD ENGINEERING & DESIGN ARE UNAUTHORIZED. IT IS ALSO UNDERSTOOD THAT THE CONTRACTOR WILL BE RESPONSIBLE FOR MEETING ALL APPLICABLE BUILDING CODES INCLUDING BUT NOT LIMITED TO MECHANICAL, ELECTRICAL, AND PLUMBING CODE REQUIREMENTS (WHICH IS EXCLUDED FROM THESE PLANS). IN THE EVENT ADDITIONAL DETAIL OR GUIDANCE IS NEEDED BY THE CONTRACTOR OR HOMEOWNER FOR CONSTRUCTION OF ANY ASPECT OF THE PROJECT, HD ENGINEERING & DESIGN OR A QUALIFIED ENGINEER SHALL IMMEDIATELY BE RETAINED. FAILURE TO NOTIFY US OF THESE NEEDS OR OF CHANGES TO THE PLANS SHALL RELIEVE HD ENGINEERING & DESIGN OF ALL RESPONSIBILITIES OF THE CONSEQUENCES.

AREA	MIN DEAD LOAD	MIN LIVE LOAD
EXTERIOR BALCONIES	10	60
DECKS, STAIRS	10	40
CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE 3:12 OR LESS	10	10
CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE OVER 3:12	10	10
CEILING JOISTS / ATTICS WITH STORAGE - DOOR PULL DOWN LADDER ACCESS	10	20
ROOMS: NON-SLEEPING	10	40
ROOMS: SLEEPING	10	30
ROOF: LIGHT ROOF COVERING	10	20
ROOF: HEAVY ROOF COVERING / CONCRETE / TILE / SLATE	20	20
GUARDRAILS, HANDRAILS	200# LL	NORMAL

PAD SIZE	REINFORCEMENT	COL. MIN.	COL. TYPE	MAX. LOAD
24x24x12	(4) #4 BARS E/W	3"	SCH40	6K
30x30x12	(5) #4 BARS E/W	3"	SCH40	9.4K
36x36x12	(6) #4 BARS E/W	3"	SCH40	13.5K
42x42x14	(7) #4 BARS E/W	3 1/2"	SCH40	18.4K
48x48x16	(8) #4 BARS E/W	3 1/2"	SCH40	24.0K
54x54x16	(9) #4 BARS E/W	3 1/2"	SCH40	30.4K
60x60x18	(10) #4 BARS E/W	3 1/2"	SCH40	37.5K

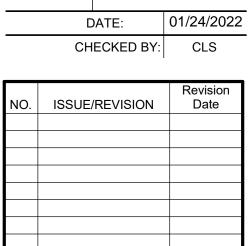
POST CAP WITH E BEAM. FOR A M FLANGE OF THE " X 2" BOLTS HER, AND A NUT IN STEEL BEAM IN ULD NEED TO BE

	F _b (psi)	E (psi)	F _∨ (psi)
LVL	2600	1.8x10	285
GLULAM	2400	1.8x10	190
PARALAM	2600	2.0x10	290



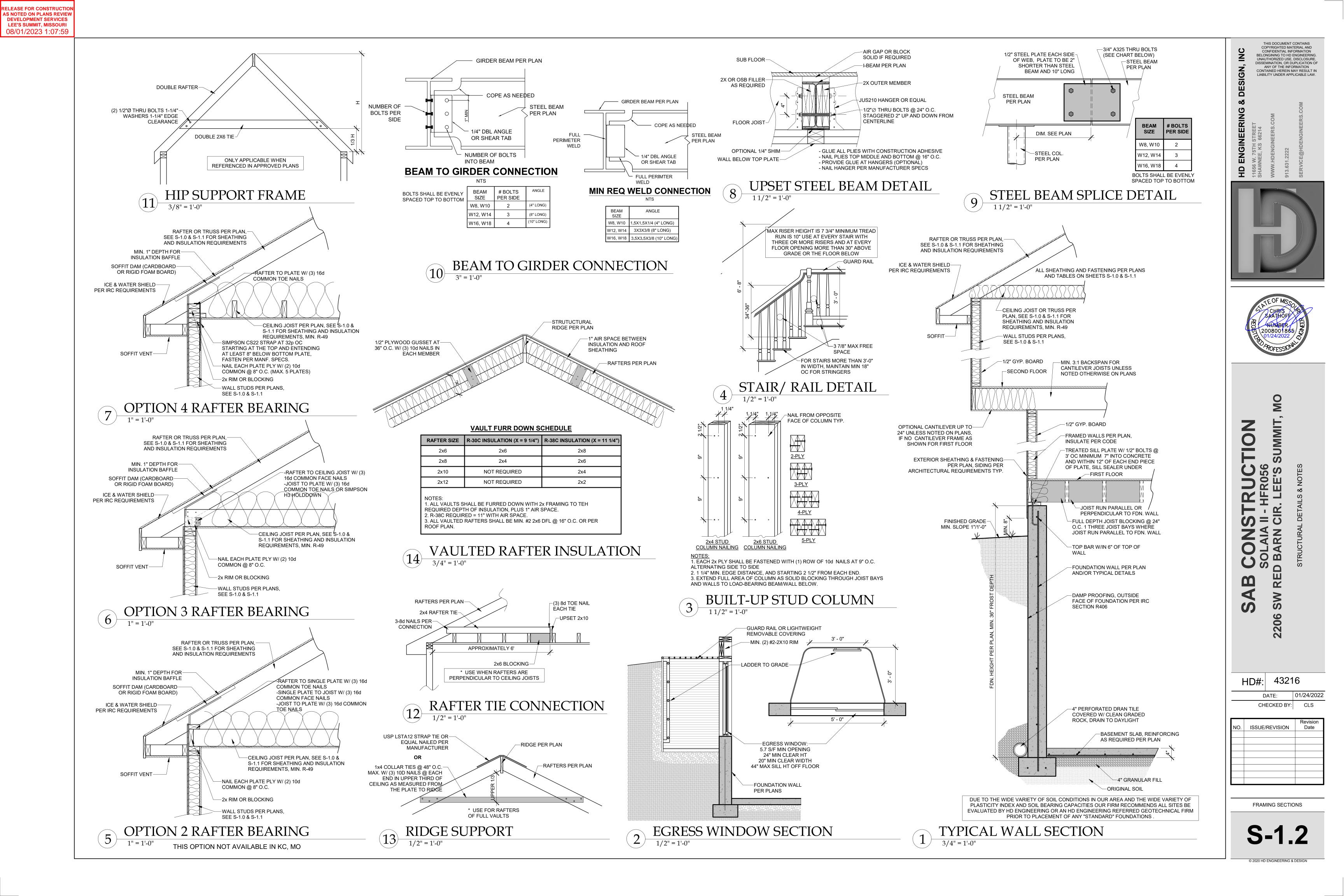


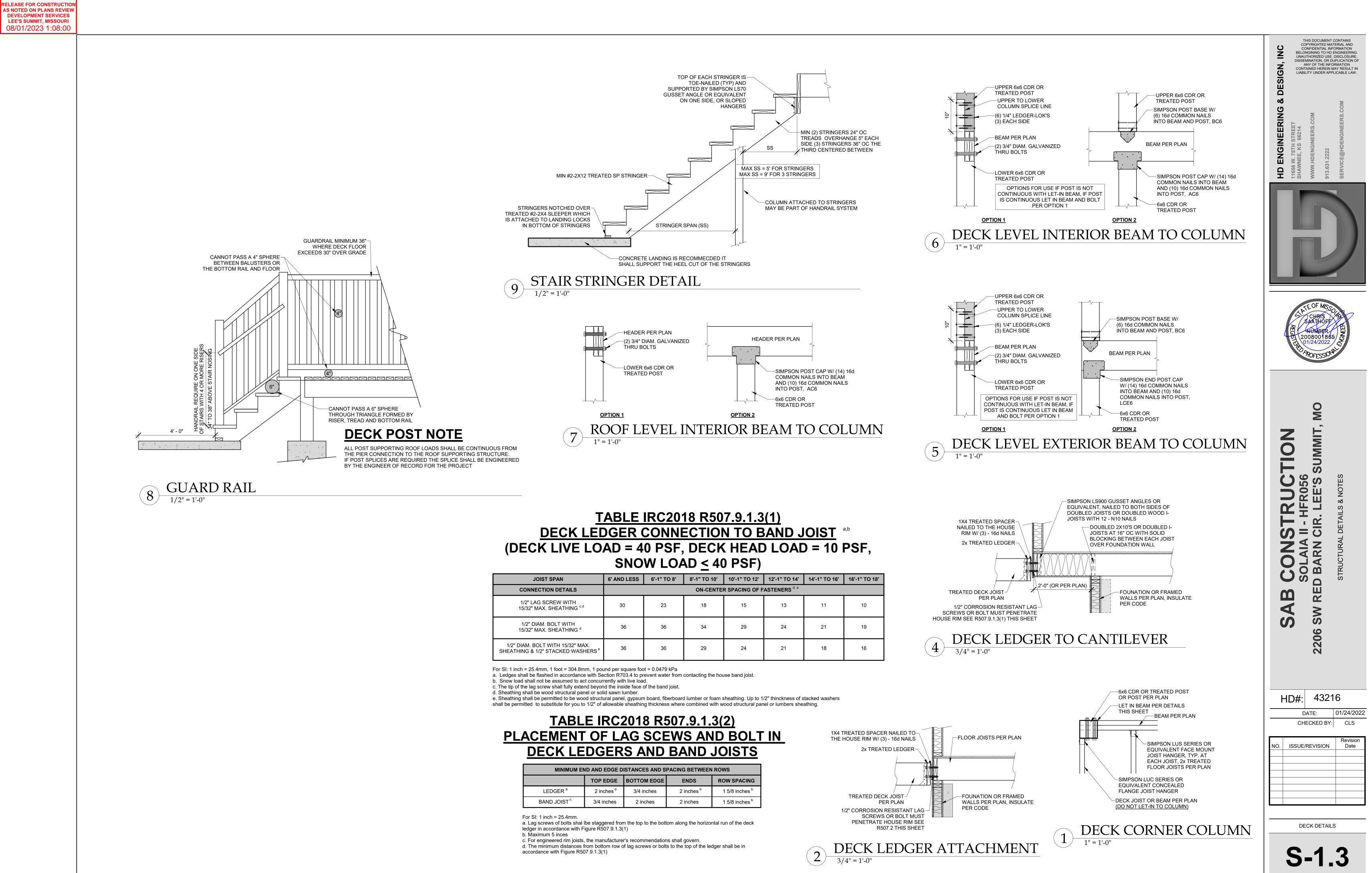
MO SUMMIT Ζ 0 F LOC R056 EE'S Щ R . CIR. S ARN ZO O M REI \mathbf{m} 4 \mathbf{i} S S 2206 43216 HD#:



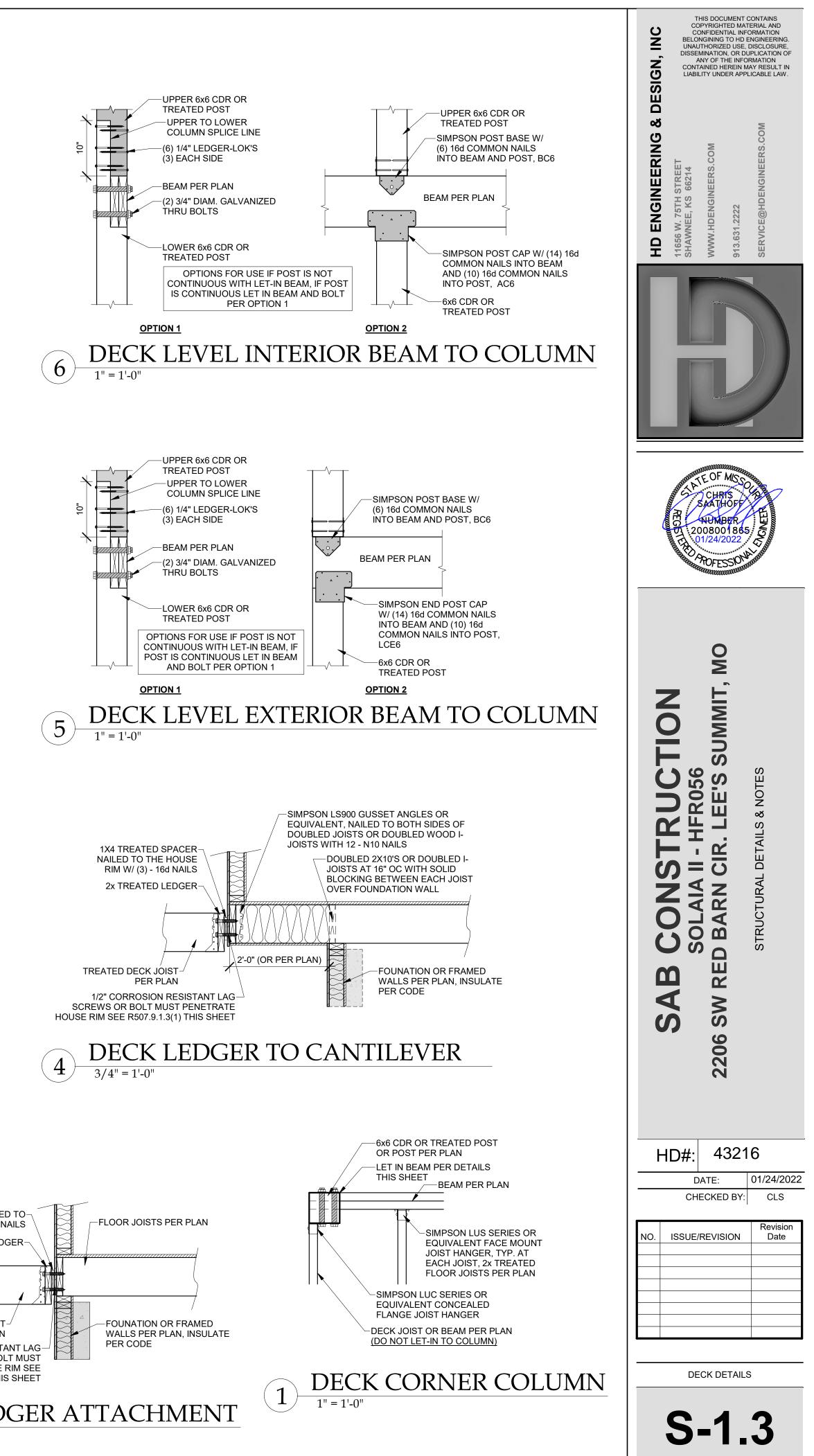
GENERAL NOTES



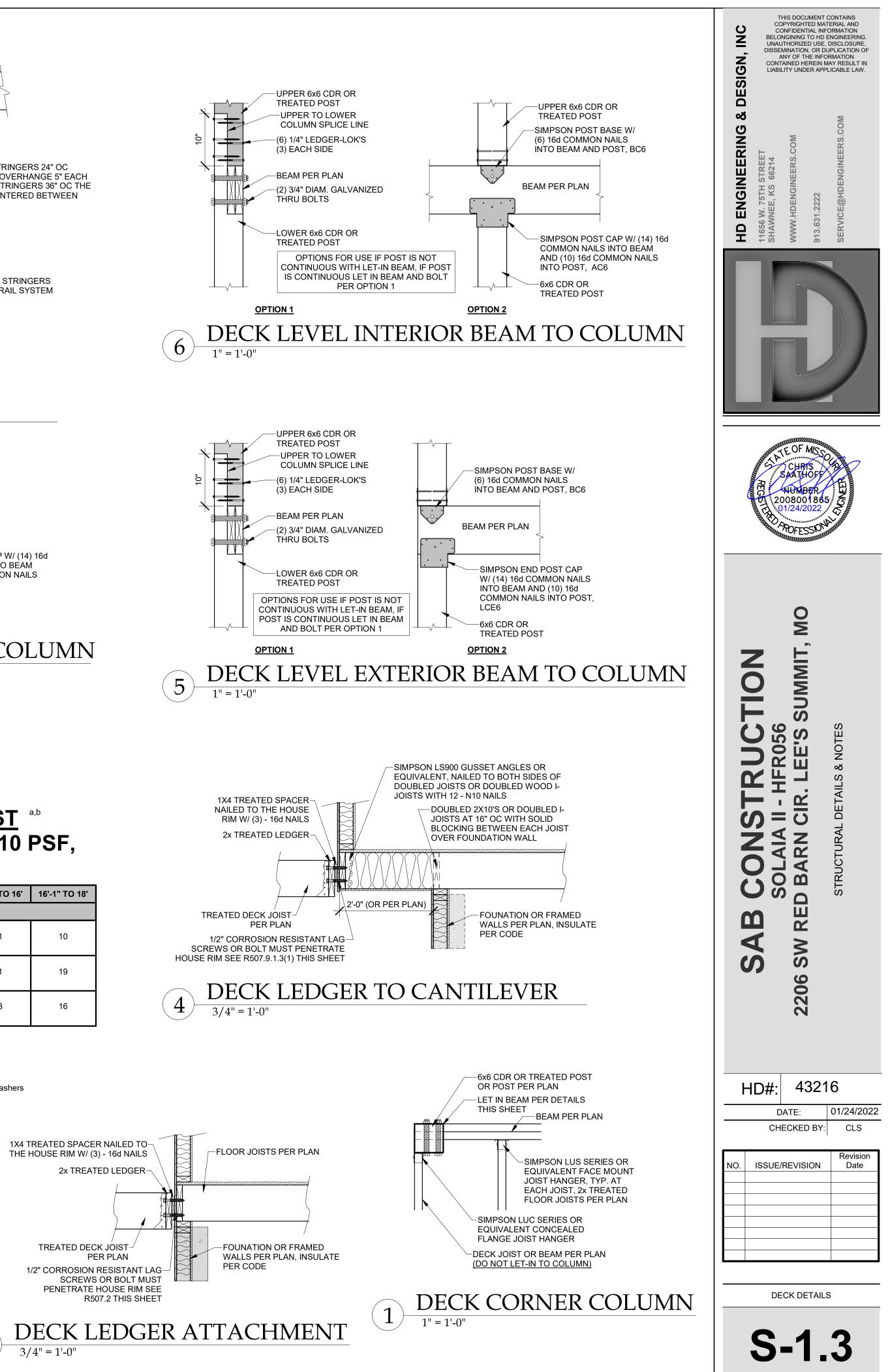




JOIST SPAN	6' AND LESS	6'-1" TO 8'	8'-1" TO 10'	10'-1" TO 12'	12'-1" TO 14'	14'-1" TO 16'	16'-1" TO 18'
CONNECTION DETAILS			ON-CENTE	R SPACING OF F	ASTENERS ^{d, e}		
1/2" LAG SCREW WITH 15/32" MAX. SHEATHING ^{c,d}	30	23	18	15	13	11	10
1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING ^d	36	36	34	29	24	21	19
1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING & 1/2" STACKED WASHERS [®]	36	36	29	24	21	18	16



	ND AND EDGE D	ISTANCES AND S	PACING BETWEEN	ROWS
	TOP EDGE	BOTTOM EDGE	ENDS	ROW SPACING
LEDGER ^a	2 inches ^d	3/4 inches	2 inches ^b	1 5/8 inches ^b
BAND JOIST [°]	3/4 inches	2 inches	2 inches	1 5/8 inches ^b



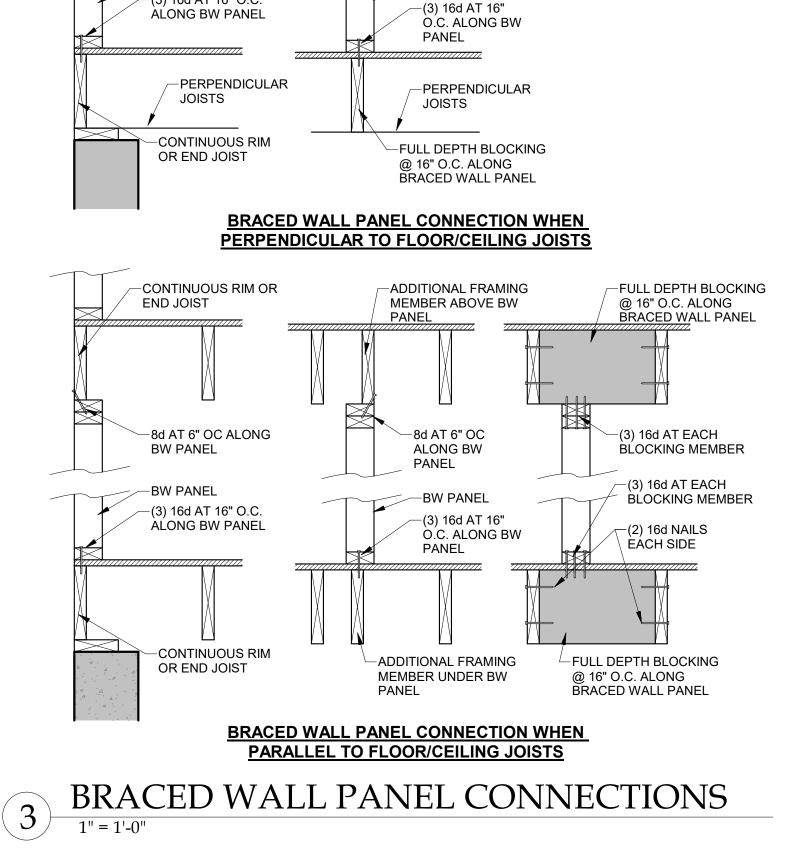
RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
08/01/2023 1:08:00

					<u>& WIND ANALYSIS</u>			
					<u> </u>		INPUT	
DETERMINE WEIGHT	T OF HOUSE:						CALCULATED VALUE	
LOCATION ROOF					DEAD LOAD (psf)	AREA (ft ²) 3464	WEIGHT (lbs.) 34640	
CEILING					10	3200	32000	
FIRST FLOOR					10	1717	17170	
				WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WEIGHT (lbs)	
FIRST FLOOR EXT. W	VALL DL			256	DEAD LOAD (psf)	10 AREA (ft2)	25600 WEIGHT (lbs)	
FIRST FLOOR INT. PA	ARTITION WALL DL				6	1717	10302	
	PRC	JECTED AREAS (WIND)	DESIGN PER 115 MPH (3-SECOND GUST EXPOS	URE C AND MEAN ROOF HEIGHT <= 3	0 FT ASSUMED)		1
		T-TO-BACK			SIDE-TO-S	,		
	AREA	LOAD			AREA	LOAD		
SLOPED ROOF VERT. ROOF	345 252	1519 3514	CUMULATIVE	SLOPED ROOF VERT. ROOF	809 0	<u>3323</u> 0	CUMULATIVE	
1ST	438	6107	11270	1ST	593	7857	11309	
			PRESSURE (PSF) - PER ASCE CH. 6				
	SLOPED ROOF WALL/VERT. ROOF	ZONE B ZONE A		5.9 17.4	ZONE C ZONE D	11.6 3.4	2a (FIG. 28.6-1, ASCE7) 10.2	
	MEAN ROOF HT., h		17.5	17.4	ZONE D	5.4	10.2	
		etermine tributary wind are						
q _{z10} =0.00256K _z K _{zt} K _d V ²	² (ASCE7-10 Velocity P	ressure)	q_{z10_ASD} =0.6 q_{z10} (Desig	n Velocity Pressure for ASE	D analysis under ASCE7-10 and IRC/IBC	2018)		
1ST FLOOR TRIBUTA	ARY WEIGHT						79440	
	OTION - %g - FROM AS	SCE7 SEISMIC MAP)					12.0%	
F _a (from ASCE7 Table	-	,					1.6	
S _{DS} (= 2/3 * S _S * F _a)							0.128	
R (from ASCE7 Table	12.2-1)						6.5	
				SEISMIC	SHEAR			
LOCATION				<u></u>		m ASCE7 (Eq. 12.8-1):	V (= 1.2 * S _{DS} * W /	R) (lbs.)
1ST FLOOR							1877	
Sheathing	ng Location	Min. Sheathi	ng Schedule	Fas	stening Schedule	Allowat	ble Shear (#/LF)	Code Reference
				8d Common Nails w/ 1-3/8				
Exterior <u>((</u>	Option #4)	7/16" APA Rated Plywoo sheathing, or 3/8" shipla tighter na	ap panel sheathing with	Field for 7/16" APA-rated p	" penetration @ 6" O.C. Edges, 12" O.C. plywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing		220	AF&PA SDPWS Table 4.3A
Exterior <u>((</u>	<u>Option #4)</u>	sheathing, or 3/8" shipla	ap panel sheathing with	Field for 7/16" APA-rated p	plywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel		220	
	<u>Option #4)</u> NG OPTION FOR FIRS	sheathing, or 3/8" shipla tighter na	ap panel sheathing with	Field for 7/16" APA-rated p	plywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing		220	
		sheathing, or 3/8" shipla tighter na	ap panel sheathing with il spacing	Field for 7/16" APA-rated p	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.)	51	220	
		sheathing, or 3/8" shipla tighter na	ap panel sheathing with il spacing	Field for 7/16" APA-rated p	plywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.)	51 73	220	
		sheathing, or 3/8" shipla tighter na	ap panel sheathing with il spacing 4	Field for 7/16" APA-rated p OR @ 4" O.C. Edges,	Diywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.)	51 73 19	220	
		sheathing, or 3/8" shipla tighter na	ap panel sheathing with il spacing 4	Field for 7/16" APA-rated p OR @ 4" O.C. Edges,	WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S	51 73 19	220	
		sheathing, or 3/8" shipla tighter na	ap panel sheathing with il spacing 4 EXTER	Field for 7/16" APA-rated p OR @ 4" O.C. Edges,	WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S	51 73 19 2	220 SIDE-TO-SIDE	Table 4.3A
	NG OPTION FOR FIRS	sheathing, or 3/8" shipla tighter na T FLOOR	ap panel sheathing with il spacing 4 EXTER	Field for 7/16" APA-rated p OR @ 4" O.C. Edges,	WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S	51 73 19 2 WIND		Table 4.3A
EXTERIOR SHEATHIN	NG OPTION FOR FIRS	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240	ap panel sheathing with il spacing 4 EXTER SISMIC SIDE-TO-SIDE 76	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, RIOR STRUCTURAL WALL RESISTANCE (lbs.)	VIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK	51 73 19 2 WIND RESISTANCE (lbs.)	SIDE-TO-SIDE	Table 4.3A RESISTANCE (lbs.
EXTERIOR SHEATHIN	NG OPTION FOR FIRS	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS	ap panel sheathing with il spacing 4 EXTER SISMIC SIDE-TO-SIDE 76 TANCE REQUIRED	Field for 7/16" APA-rated p OR @ 4" O.C. Edges,	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.)	SIDE-TO-SIDE 76 16d Nail Spacing req'd at I	Table 4.3A RESISTANCE (lbs. 29792 pottom plate (in.)
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC	ap panel sheathing with il spacing 4 EXTER EISMIC SIDE-TO-SIDE 76 TANCE REQUIRED WIND	Field for 7/16" APA-rated p OR @ 4" O.C. Edges,	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5	SIDE-TO-SIDE 76 16d Nail Spacing req'd at I 1st Floor F-B	Table 4.3A RESISTANCE (lbs. 29792 pottom plate (in.)
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS	ap panel sheathing with il spacing 4 EXTER SISMIC SIDE-TO-SIDE 76 TANCE REQUIRED	Field for 7/16" APA-rated p OR @ 4" O.C. Edges,	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944	SIDE-TO-SIDE 76 16d Nail Spacing req'd at I	Table 4.3A RESISTANCE (lbs. 29792 pottom plate (in.)
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0	Ap panel sheathing with il spacing 4 EXTER SIDE-TO-SIDE 76 STANCE REQUIRED WIND 0	Field for 7/16" APA-rated p OR @ 4" O.C. Edges,	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5	SIDE-TO-SIDE 76 16d Nail Spacing req'd at I 1st Floor F-B	Table 4.3A RESISTANCE (lbs. 29792 pottom plate (in.)
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0	Ap panel sheathing with il spacing 4 EXTER SIDE-TO-SIDE 76 STANCE REQUIRED WIND 0	Field for 7/16" APA-rated p OR @ 4" O.C. Edges,	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8	SIDE-TO-SIDE 76 16d Nail Spacing req'd at I 1st Floor F-B	Table 4.3A RESISTANCE (lbs. 29792 pottom plate (in.)
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0	4 EXTER SIDE-TO-SIDE TANCE REQUIRED WIND 0 0 0	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, NOR STRUCTURAL WALL RESISTANCE (lbs.) 21280	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5	SIDE-TO-SIDE 76 16d Nail Spacing req'd at I 1st Floor F-B	Table 4.3A RESISTANCE (lbs. 29792 pottom plate (in.)
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0	Ap panel sheathing with il spacing 4 EXTER ISMIC SIDE-TO-SIDE 76 TANCE REQUIRED WIND 0 0 0 RESISTANCE REQUI	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, NOR STRUCTURAL WALL RESISTANCE (lbs.) 21280	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S	Table 4.3A RESISTANCE (lbs. 29792 pottom plate (in.)
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0	4 EXTER SIDE-TO-SIDE TANCE REQUIRED WIND 0 0 0	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, NOR STRUCTURAL WALL RESISTANCE (lbs.) 21280	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 5215 (in.) 52 (in.)	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S Not Floor S-S	Table 4.3A RESISTANCE (lbs. 29792 pottom plate (in.)
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133 TO-BACK SIDE	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Ap panel sheathing with il spacing A EXTER SIDE-TO-SIDE TANCE REQUIRED WIND 0 0 0 RESISTANCE REQUI PORTAL FRAMES OR PERF. SHEAR WALL	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, NOR STRUCTURAL WALL RESISTANCE (lbs.) 21280 RED IN ADDITION TO RES INTERIOR X-BRACES	VIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5 /ALLS** INT. WALL LENGTH SHEATHED W/ OSB	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S Note that the second	Table 4.3A RESISTANCE (lbs. 29792 pottom plate (in.)
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133 TO-BACK SIDE	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0 0 0	Ap panel sheathing with il spacing A EXTER SIDE-TO-SIDE TANCE REQUIRED WIND 0 0 0 RESISTANCE REQUI PORTAL FRAMES OR PERF. SHEAR WALL	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, NOR STRUCTURAL WALL RESISTANCE (lbs.) 21280 RED IN ADDITION TO RES INTERIOR X-BRACES	VIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 5215 (in.) 52 (in.)	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S RESISTANCE PROVIDED BY ADDITIONAL METHODS	Table 4.3A RESISTANCE (lbs. 29792 bottom plate (in.)
EXTERIOR SHEATHIN 1ST FLOOR 1ST FLOOR FRONT-T 1ST FLOOR SIDE-TO- 1ST FLOOR SIDE-TO- **NOTES: 1) SEE ATT 2) SEE SHEET S1 FO	NG OPTION FOR FIRS FRONT-TO-BACK 133 TO-BACK D-SIDE TO-BACK D-SIDE TACHED CALCULATIO DR INTERIOR STEEL X-	Sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4 EXTER EXTER SIDE-TO-SIDE TANCE REQUIRED VIND 0 0 RESISTANCE REQUI PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE COR PERFORATED SHI 3) INTERIOR WALLS SH	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, IOR STRUCTURAL WALL RESISTANCE (lbs.) 21280 RED IN ADDITION TO RES INTERIOR X-BRACES (325#/BRACE) EAR WALL RESISTANCE O HEATHED WITH OSB SHAL	VIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5 /ALLS** INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S Note that the second	Table 4.3A RESISTANCE (lbs. 29792 bottom plate (in.) CK? YES
EXTERIOR SHEATHIN 1ST FLOOR 1ST FLOOR FRONT-T 1ST FLOOR SIDE-TO- 1ST FLOOR SIDE-TO- **NOTES: 1) SEE ATT 2) SEE SHEET S1 FO	NG OPTION FOR FIRS FRONT-TO-BACK 133 TO-BACK D-SIDE TO-BACK D-SIDE TACHED CALCULATIO DR INTERIOR STEEL X-	Sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4 EXTER EXTER SIDE-TO-SIDE TANCE REQUIRED VIND 0 0 RESISTANCE REQUI PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE COR PERFORATED SHI 3) INTERIOR WALLS SH	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, NOR STRUCTURAL WALL RESISTANCE (lbs.) 21280 21280 RED IN ADDITION TO RES INTERIOR X-BRACES (325#/BRACE) EAR WALL RESISTANCE O HEATHED WITH OSB SHAP PLICABLE FOR FULL-HEIG	Olywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches) SISTANCE PROVIDED BY EXTERIOR W INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) CAPACITIES (IF APPLICABLE), LL BE ATTACHED WITH SAME STAPLI HT SECTIONS OF 2'-8" OR LONGER	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5 /ALLS** INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S Note that the second	Table 4.3A RESISTANCE (lbs. 29792 bottom plate (in.) CK? YES
EXTERIOR SHEATHIN 1ST FLOOR 1ST FLOOR FRONT-T 1ST FLOOR SIDE-TO- 1ST FLOOR SIDE-TO- **NOTES: 1) SEE ATT 2) SEE SHEET S1 FO	NG OPTION FOR FIRS FRONT-TO-BACK 133 TO-BACK D-SIDE TO-BACK D-SIDE TACHED CALCULATIO DR INTERIOR STEEL X-	Sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4 EXTER EXTER SIDE-TO-SIDE TANCE REQUIRED VIND 0 0 RESISTANCE REQUI PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE COR PERFORATED SHI 3) INTERIOR WALLS SH	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, IOR STRUCTURAL WALL RESISTANCE (lbs.) 21280 RED IN ADDITION TO RES INTERIOR X-BRACES (325#/BRACE) EAR WALL RESISTANCE O HEATHED WITH OSB SHAL	Olywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches) SISTANCE PROVIDED BY EXTERIOR W INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) CAPACITIES (IF APPLICABLE), LL BE ATTACHED WITH SAME STAPLI HT SECTIONS OF 2'-8" OR LONGER	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5 /ALLS** INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S Note that the second	Table 4.3A RESISTANCE (lbs. 29792 bottom plate (in.) CK? YES
EXTERIOR SHEATHIN 1ST FLOOR 1ST FLOOR FRONT-T 1ST FLOOR SIDE-TO- 1ST FLOOR SIDE-TO- **NOTES: 1) SEE ATT 2) SEE SHEET S1 FO	NG OPTION FOR FIRS FRONT-TO-BACK 133 TO-BACK D-SIDE TO-BACK D-SIDE TACHED CALCULATIO JACHED CALCULATIO JACHED CALCULATIO R INTERIOR STEEL X. RIOR OSB ON SAME FL	sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Ap panel sheathing with il spacing 4 EXTER ISMIC SIDE-TO-SIDE 76 TANCE REQUIRED WIND 0 0 RESISTANCE REQUI PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE OR PERFORATED SHI 3) INTERIOR WALLS SH E) AND ARE ONLY APP	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, NOR STRUCTURAL WALL RESISTANCE (lbs.) 21280 21280 RED IN ADDITION TO RES INTERIOR X-BRACES (325#/BRACE) EAR WALL RESISTANCE O HEATHED WITH OSB SHAP PLICABLE FOR FULL-HEIG	Olywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches) SISTANCE PROVIDED BY EXTERIOR W INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) CAPACITIES (IF APPLICABLE), LL BE ATTACHED WITH SAME STAPLI HT SECTIONS OF 2'-8" OR LONGER	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5 /ALLS** INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S Note that the second	Table 4.3A RESISTANCE (lbs. 29792 bottom plate (in.) CK? YES
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133 TO-BACK SIDE TO-BACK SIDE TACHED CALCULATIO DR INTERIOR STEEL X- RIOR OSB ON SAME FL X/12 6	sheathing, or 3/8" shipla tighter na T FLOOR SE RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4 EXTER ISMIC SIDE-TO-SIDE 76 TANCE REQUIRED WIND 0 0 RESISTANCE REQUIRED WIND 0 0 COR PERFORATED SHI SIDE-TO-SIDE 76 TANCE REQUIRED WIND 0 0 0 0 0 0 0 0 0	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, REDIN ADDITION TO RES INTERIOR X-BRACES (325#/BRACE) EAR WALL RESISTANCE (HEATHED WITH OSB SHAP CLICABLE FOR FULL-HEIG WIND UPLIFT EOH -13.3, E -7.2, G -5.2	Olywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches) SISTANCE PROVIDED BY EXTERIOR W INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) CAPACITIES (IF APPLICABLE), LL BE ATTACHED WITH SAME STAPLI HT SECTIONS OF 2'-8" OR LONGER	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5 /ALLS** INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S Note that the second	Table 4.3A RESISTANCE (lbs. 29792 bottom plate (in.) OK? YES
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133 TO-BACK D-SIDE TO-BACK D-SIDE TACHED CALCULATIO DR INTERIOR STEEL X- NOR OSB ON SAME FL	sheathing, or 3/8" shipla tighter na T FLOOR SE RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0 0 0 0 NS FOR PORTAL FRAME BRACE INSTALLATION, OOR (SEE TABLE ABOV DEGREES 26.6 ASCE 7 PRESSURE (PSF)	4 EXTER ISMIC SIDE-TO-SIDE TANCE REQUIRED WIND 0 0 RESISTANCE REQUI PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE OR PERFORATED SHI 3) INTERIOR WALLS SH E) AND ARE ONLY APP PITCH OF 6 OR LESS: LINEAL FT. OF OH	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, INTERIOR STRUCTURAL WALL RESISTANCE (lbs.) 21280 RED IN ADDITION TO RES INTERIOR X-BRACES (325#/BRACE) EAR WALL RESISTANCE (HEATHED WITH OSB SHAP CLICABLE FOR FULL-HEIG WIND UPLIFT EOH -13.3, E -7.2, G -5.2 UPLIFT PER FT* (LBS)	Olywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches) SISTANCE PROVIDED BY EXTERIOR W INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) CAPACITIES (IF APPLICABLE), LL BE ATTACHED WITH SAME STAPLI HT SECTIONS OF 2'-8" OR LONGER	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5 /ALLS** INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S Note that the second	Table 4.3A RESISTANCE (lbs. 29792 bottom plate (in.) CK? YES
EXTERIOR SHEATHIN	NG OPTION FOR FIRS FRONT-TO-BACK 133 TO-BACK SIDE TO-BACK SIDE TACHED CALCULATIO DR INTERIOR STEEL X- RIOR OSB ON SAME FL X/12 6	sheathing, or 3/8" shipla tighter na T FLOOR SE RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	A panel sheathing with il spacing A EXTER SIDE-TO-SIDE TANCE REQUIRED WIND 0 0 0 RESISTANCE REQUI PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE OR PERFORATED SHI 3) INTERIOR WALLS SH E) AND ARE ONLY APP PITCH OF 6 OR LESS: LINEAL FT. OF OH 250	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, REDIN ADDITION TO RES INTERIOR X-BRACES (325#/BRACE) EAR WALL RESISTANCE (HEATHED WITH OSB SHAP CLICABLE FOR FULL-HEIG WIND UPLIFT EOH -13.3, E -7.2, G -5.2	Olywood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches) SISTANCE PROVIDED BY EXTERIOR W INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) CAPACITIES (IF APPLICABLE), LL BE ATTACHED WITH SAME STAPLI HT SECTIONS OF 2'-8" OR LONGER	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5 /ALLS** INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S Note that the second	Table 4.3A RESISTANCE (lbs. 29792 bottom plate (in.) CK? YES YES YES
EXTERIOR SHEATHIN	NG OPTION FOR FIRS	ADDITIONAL RESISTANCE (Ibs.) ADDITIONAL RESIS SEISMIC 0 0 0 0 0 0 0 0 0 0 0 0 0	A panel sheathing with il spacing A EXTER SIDE-TO-SIDE TANCE REQUIRED WIND 0 0 0 RESISTANCE REQUI PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE OR PERFORATED SHI 3) INTERIOR WALLS SH E) AND ARE ONLY APP PITCH OF 6 OR LESS: LINEAL FT. OF OH 250	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, RESISTANCE (lbs.) 21280 RED IN ADDITION TO RES INTERIOR X-BRACES (325#/BRACE) EAR WALL RESISTANCE (HEATHED WITH OSB SHAP PLICABLE FOR FULL-HEIG WIND UPLIFT EOH -13.3, E -7.2, G -5.2 UPLIFT PER FT* (LBS) 16.56	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches) SISTANCE PROVIDED BY EXTERIOR W INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) CAPACITIES (IF APPLICABLE), LL BE ATTACHED WITH SAME STAPLI HT SECTIONS OF 2'-8" OR LONGER FANALYSIS	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5 (ALLS** INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.) E/NAILING	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS) 0 0 0	Table 4.3A RESISTANCE (lbs. 29792 bottom plate (in.) OK? YES YES
EXTERIOR SHEATHIN IST FLOOR IST FLOOR FRONT-T IST FLOOR SIDE-TO- TST FLOOR SIDE-TO- **NOTES: 1) SEE ATT 2) SEE SHEET S1 FOI PATTERN AS EXTERI ROOF PITCH (MAX) OVERHANG	NG OPTION FOR FIRS	Sheathing, or 3/8" shipla tighter na T FLOOR RESISTANCE (lbs.) 37240 ADDITIONAL RESIS SEISMIC 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	A panel sheathing with il spacing A EXTER ISMIC SIDE-TO-SIDE 76 TANCE REQUIRED WIND 0 0 RESISTANCE REQUI PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE OR PERFORATED SHI 3) INTERIOR WALLS SF E) AND ARE ONLY APP PITCH OF 6 OR LESS: LINEAL FT. OF OH 250 ZONE G AREA (FT ²) 4098.36 FOOT ALONG EXTERIOR (PC	Field for 7/16" APA-rated p OR @ 4" O.C. Edges, REDIN ADDITION TO RES INTERIOR X-BRACES (325#/BRACE) EAR WALL RESISTANCE (HEATHED WITH OSB SHAD CLICABLE FOR FULL-HEIG WIND UPLIFT EOH -13.3, E -7.2, G -5.2 UPLIFT PER FT* (LBS) 16.56 PRESSURE ZN. E (PSF) 15.12	Divwood/OSB or shiplap panel sheathing 12" O.C. Field for 3/8" shiplap panel sheathing WIDTH OF 1ST STORY (FT.) DEPTH OF 1ST STORY (FT.) BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S LENGTHS (ft.) & RESISTANCES FRONT-TO-BACK 133 Anchor Bolt Spacing diameter (in.) Shear value (per NDS) Spacing F-B (inches) spacing S-S (inches) SISTANCE PROVIDED BY EXTERIOR W INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) CAPACITIES (IF APPLICABLE), LL BE ATTACHED WITH SAME STAPLI HT SECTIONS OF 2'-8" OR LONGER FANALYSIS PRESSURE ZN. G (PSF)	51 73 19 2 WIND RESISTANCE (lbs.) 52136 (in.) 0.5 944 234.8 163.5 /ALLS** INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.) E/NAILING	SIDE-TO-SIDE 76 16d Nail Spacing req'd at 1 1st Floor F-B 1st Floor S-S RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS) 0 0 0 0 0 0 0 0 0 0 0 0 0	Table 4.3A RESISTANCE (lbs. 29792 bottom plate (in.) CK? YES YES YES

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS NOTE FOR DESIGN:

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2018 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 220 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION



-PERPENDICULAR

-FULL DEPTH BLOCKING

@ 16" O.C. ALONG BRACED WALL PANEL

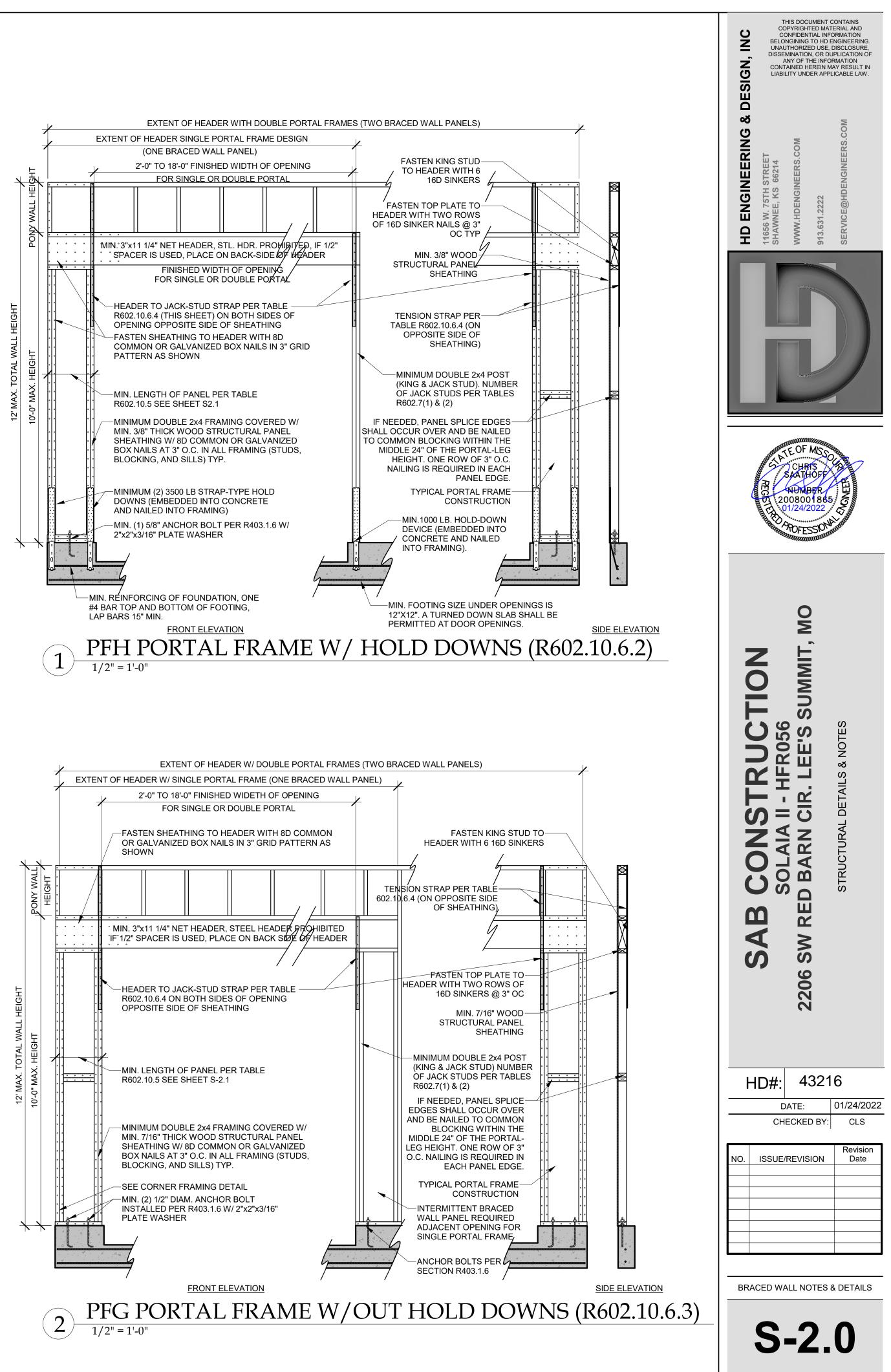
-8d AT 6" OC

ALONG BW

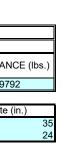
-BW PANEL

PANEL

JOISTS



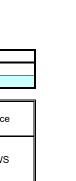














-CONTINUOUS RIM OR

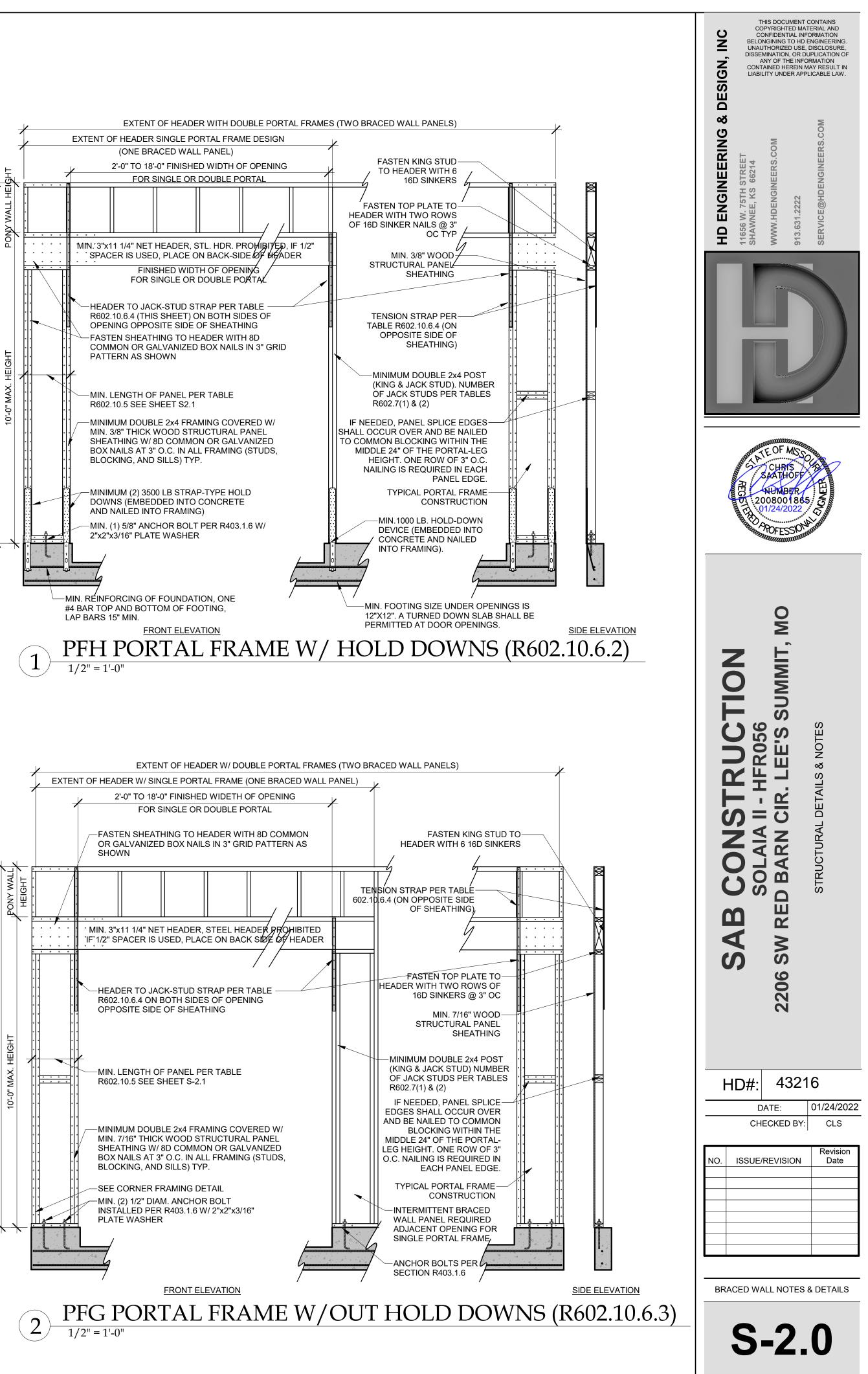
—8d AT 6" OC ALONG

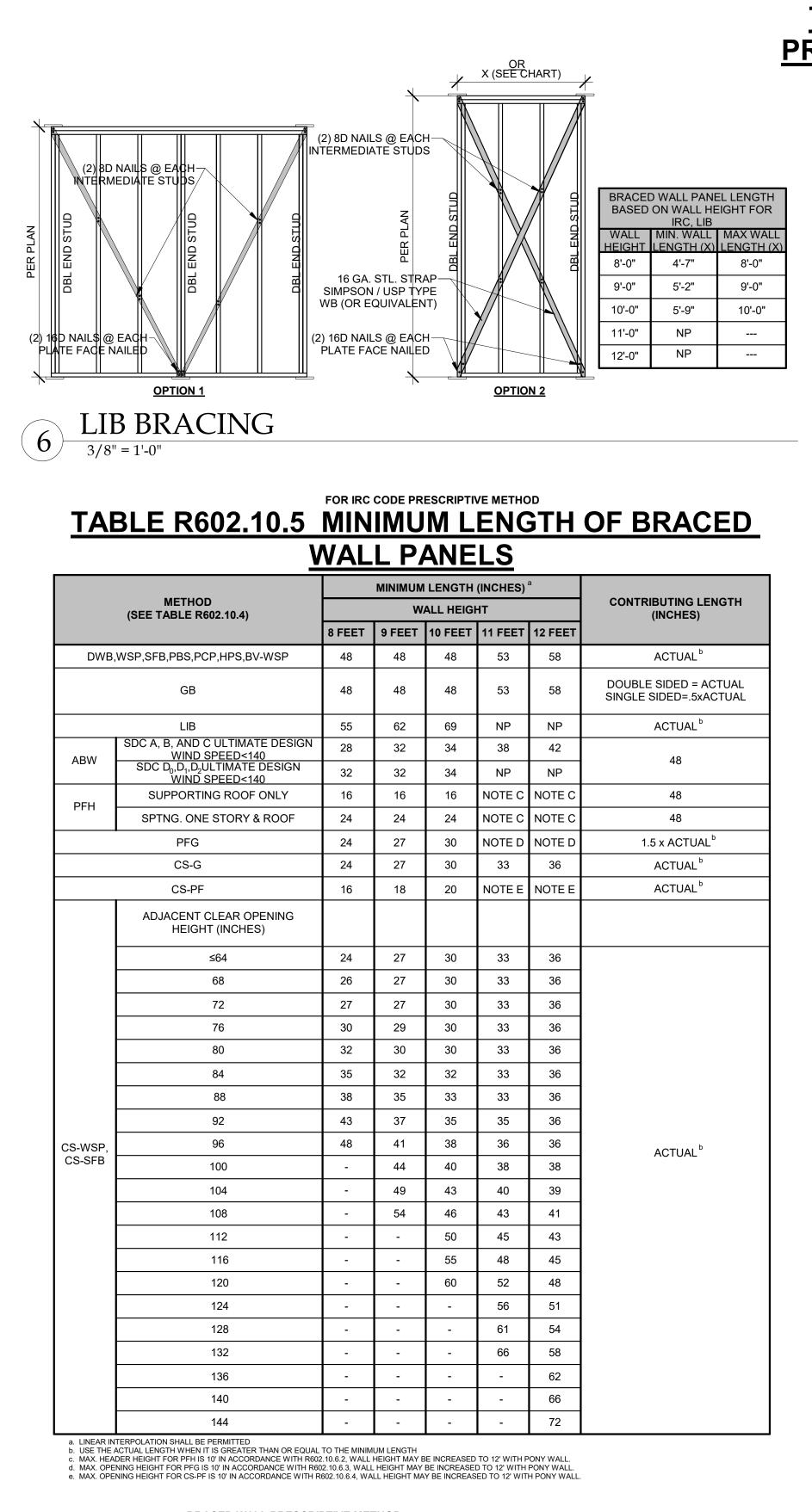
-(3) 16d AT 16" O.C.

BW PANEL

-BW PANEL

END JOIST





BRACED WALL PRESCRIPTIVE METHOD: CONTINOUS EXTERIOR SHEATHING (CS-WSP) PER WSP METHOD (BELOW) UNLESS OTHERWISE NOTED ON THE PLAN

EXTERIOR BRACED WALL METHOD: (SEE ON THIS SHEET)

WSP METHOD: WOOD STRUCUTRAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 3/8" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" O.C. STUD SPACING WITH 6d NAILS COMMON NAILS @ 6" O.C. EDGES AND 12" O.C. FIELD OR SHEATHING THICKNESS NOT LESS THANK 7/16" WITH MINIMUM SPAN RATING OF 24/16 FOR 24" O.C. SPACING WITH 8d COMMON NAILS @ 6" O.C. EDGES AND 12" O.C. IN FIELD (NOTE: FRAMING MEMBERS 16" O.C. MAX, UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS).

INTERIOR BRACED WALLS (SEE ON THIS SHEET)

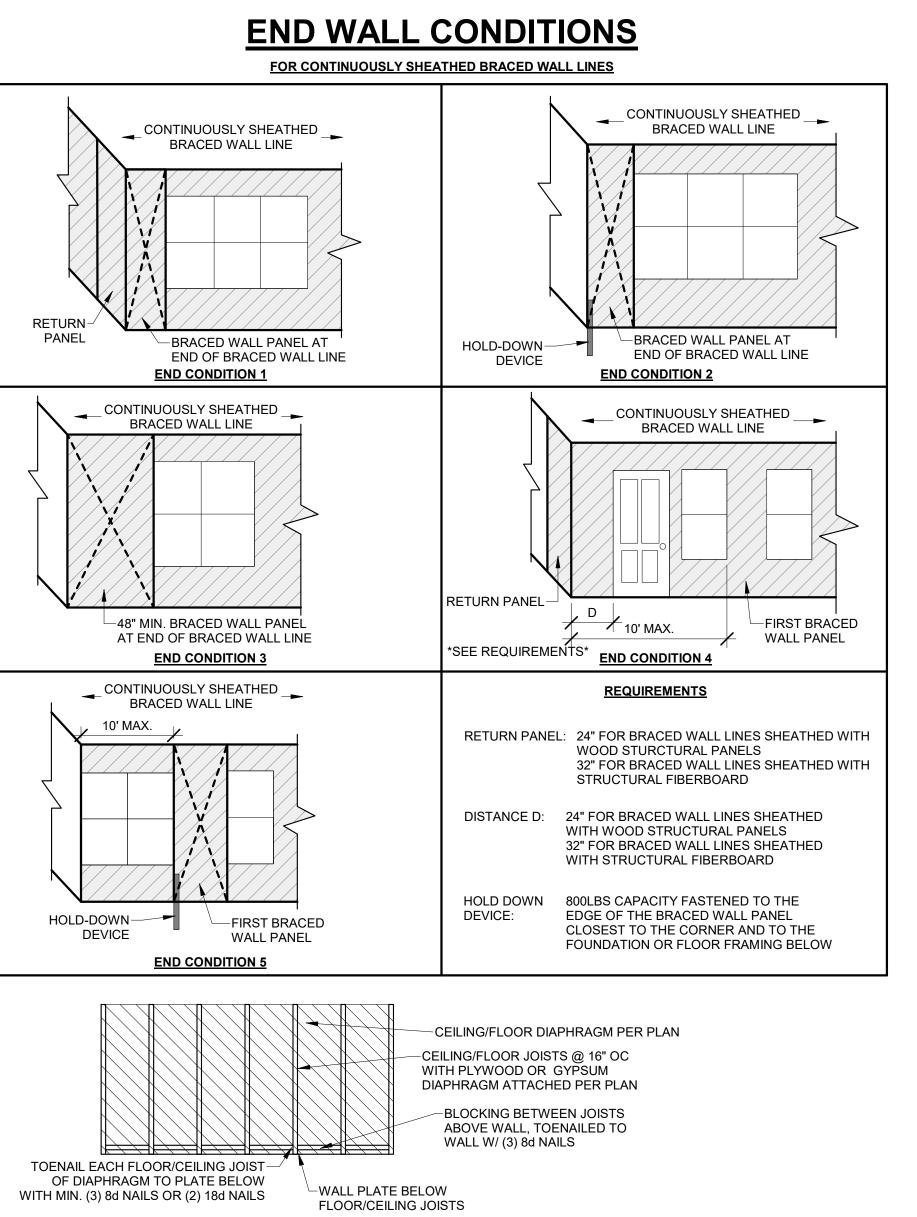
GB METHOD: 1/2" MINIMUM GYPSUM BOARD OVER STUDS SPACED @ 24" MAXIMUM FASTENED W/ #6- 1 1/4" TYPE "W" OR "S" DRYWALL SCREWS @ 7" O.C. EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES) OR

LIB METHOD: 1X4 WOOD FASTENED W/ (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUIVALENT) STL. X-BRACE(S) @ 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUDS FASTENED PER MANUF. SPECS.



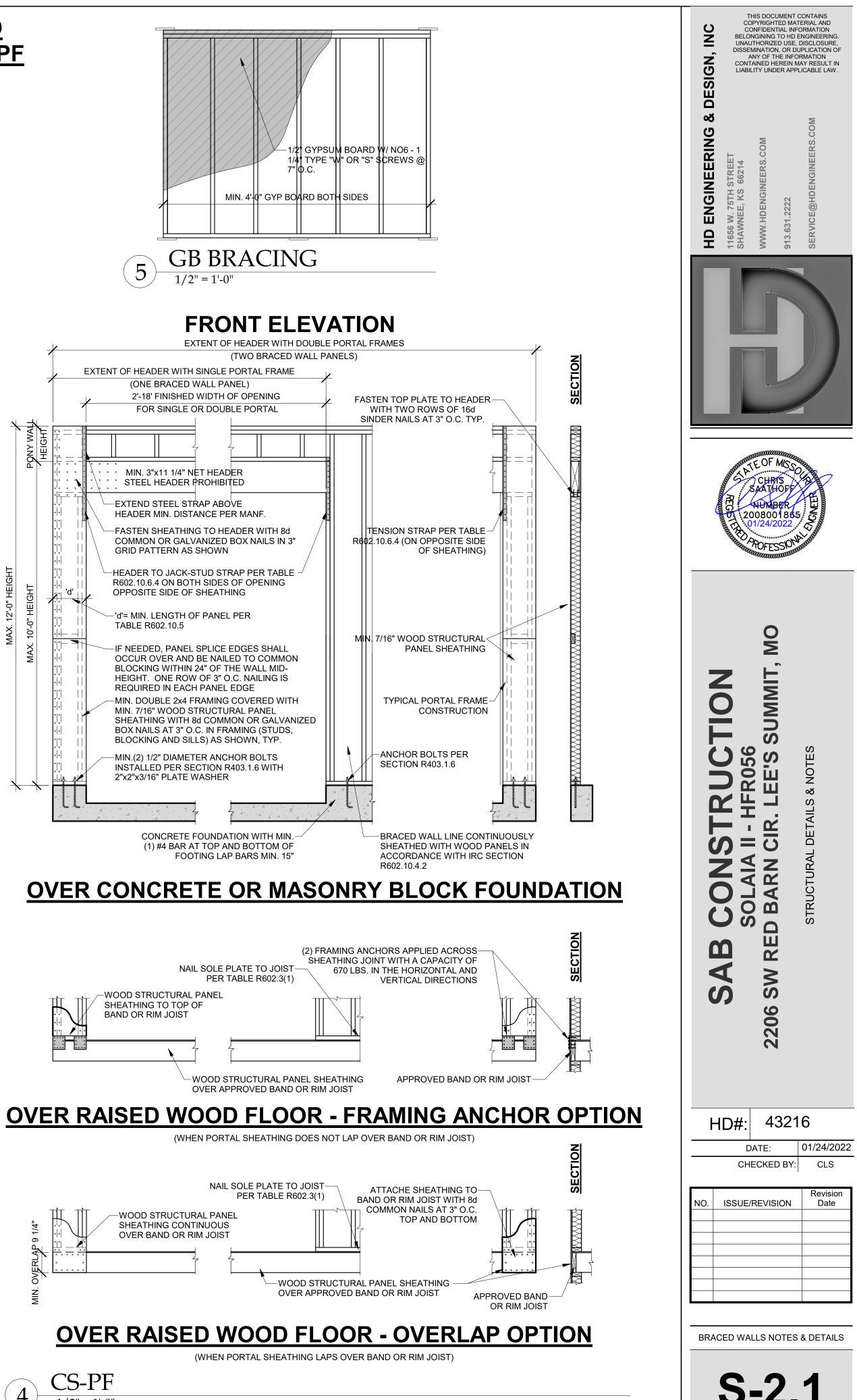
	MAX. PONY WALL HEIGHT (FEET)	MAX. TOTAL WALL HEIGHT (FEET)	MAX. OPENING WIDTH (FEET)	TENSION STRAP CAPACITY REQUIRED (POUNDS) ^a		
MINIMUM WALL STUD FRAMING NOMINAL SIZE & GRADE				ULTIMATE DESIGN WIND SPEED V (MPH)		
				115	115	
				EXPOSURE B	EXPOSURE C	
2X4 NO. 2 GRADE	0	10	18	1,000	1,000	
	1	10	9	1,000	1,000	
			16	1,025	2,500	
			18	1,275	2,850	
	2	10	9	1,000	1,875	
			16	2,175	4,125	
			18	2,500	DR	
	2	12	9	1,500	3,175	
			16	3,375	DR	
			18	3,975	DR	
	4	12	9	2,750	DR	
			12	3,775	DR	
2X6 STUD GRADE	2	12	9	1,000	2,025	
			16	2,150	3,675	
			18	2,550	DR	
	4	12	9	1,750	3,125	
			16	2,400	DR	
			18	3,800	DR	

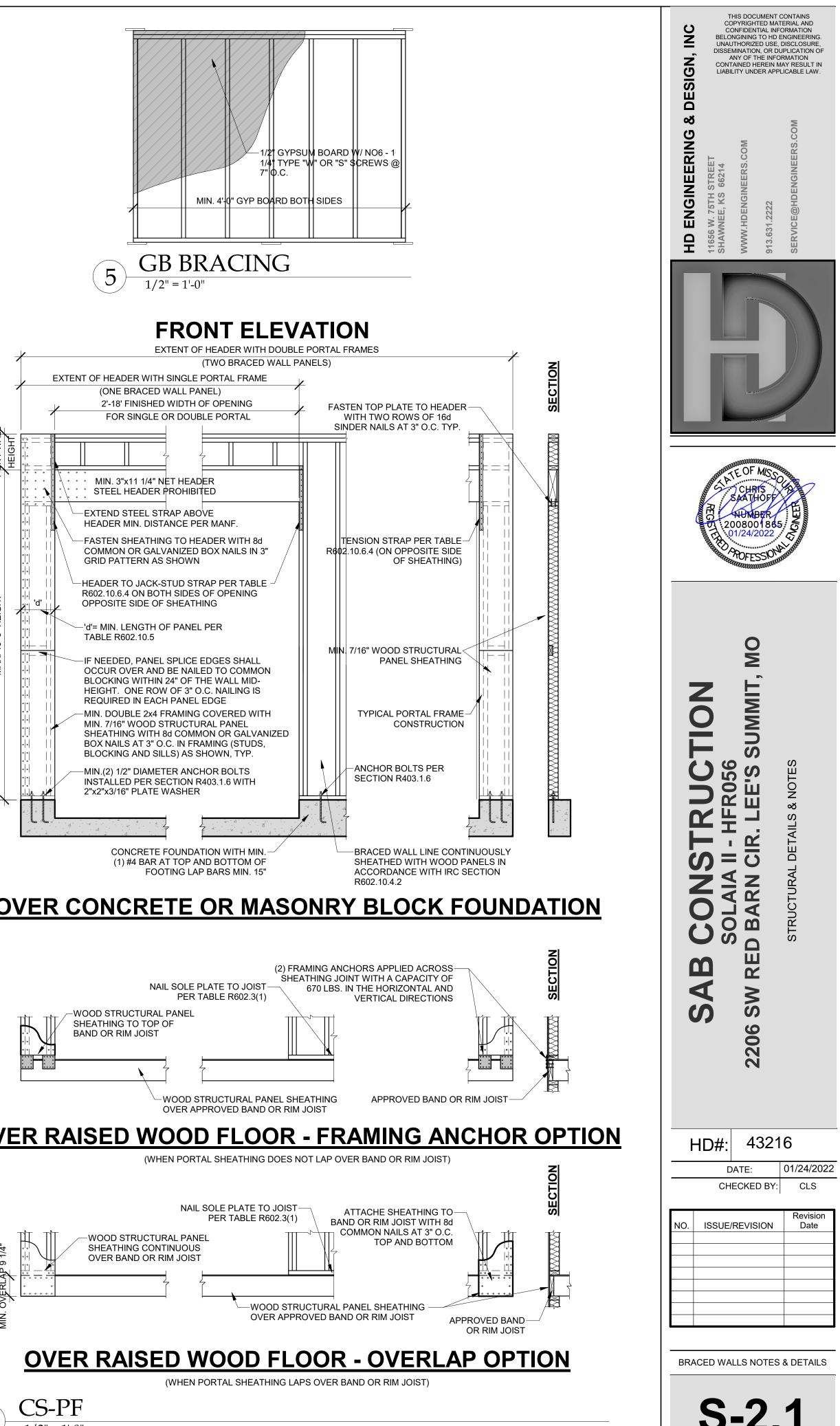
a. DR = DESIGN REQUIRED b. STRAP SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

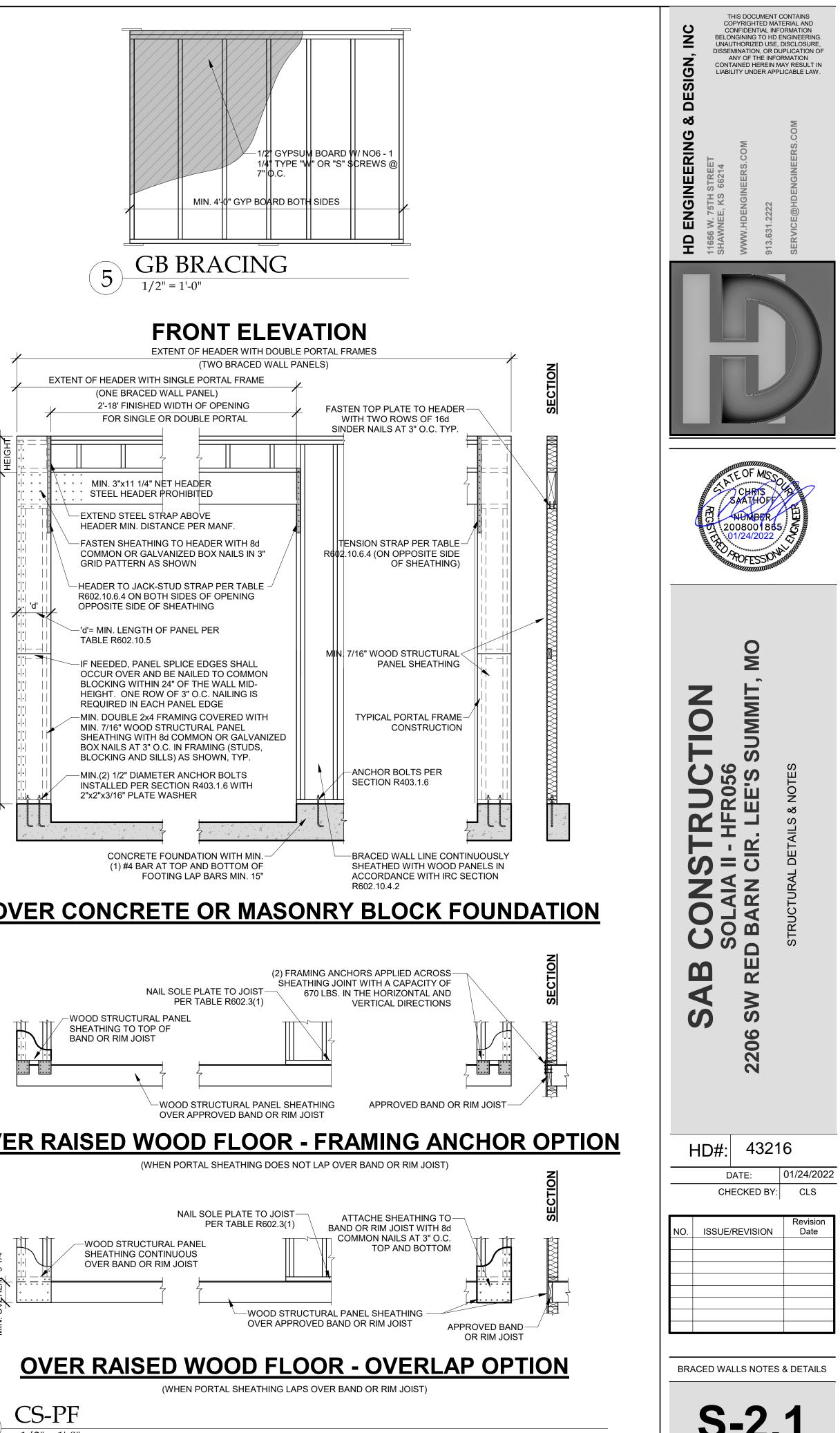


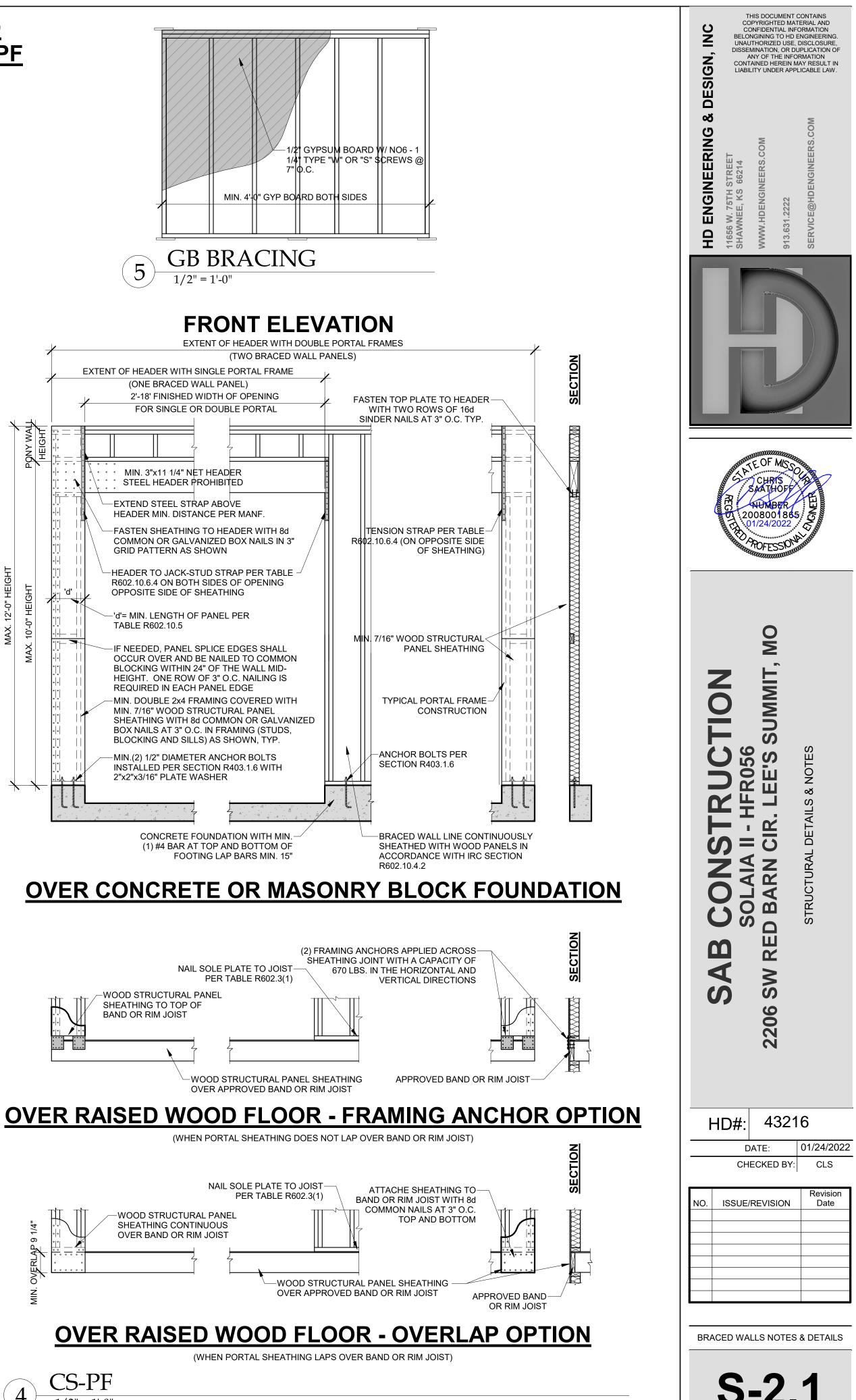
DIAPHRAGM CONNECTION TO INTERIOR WALL

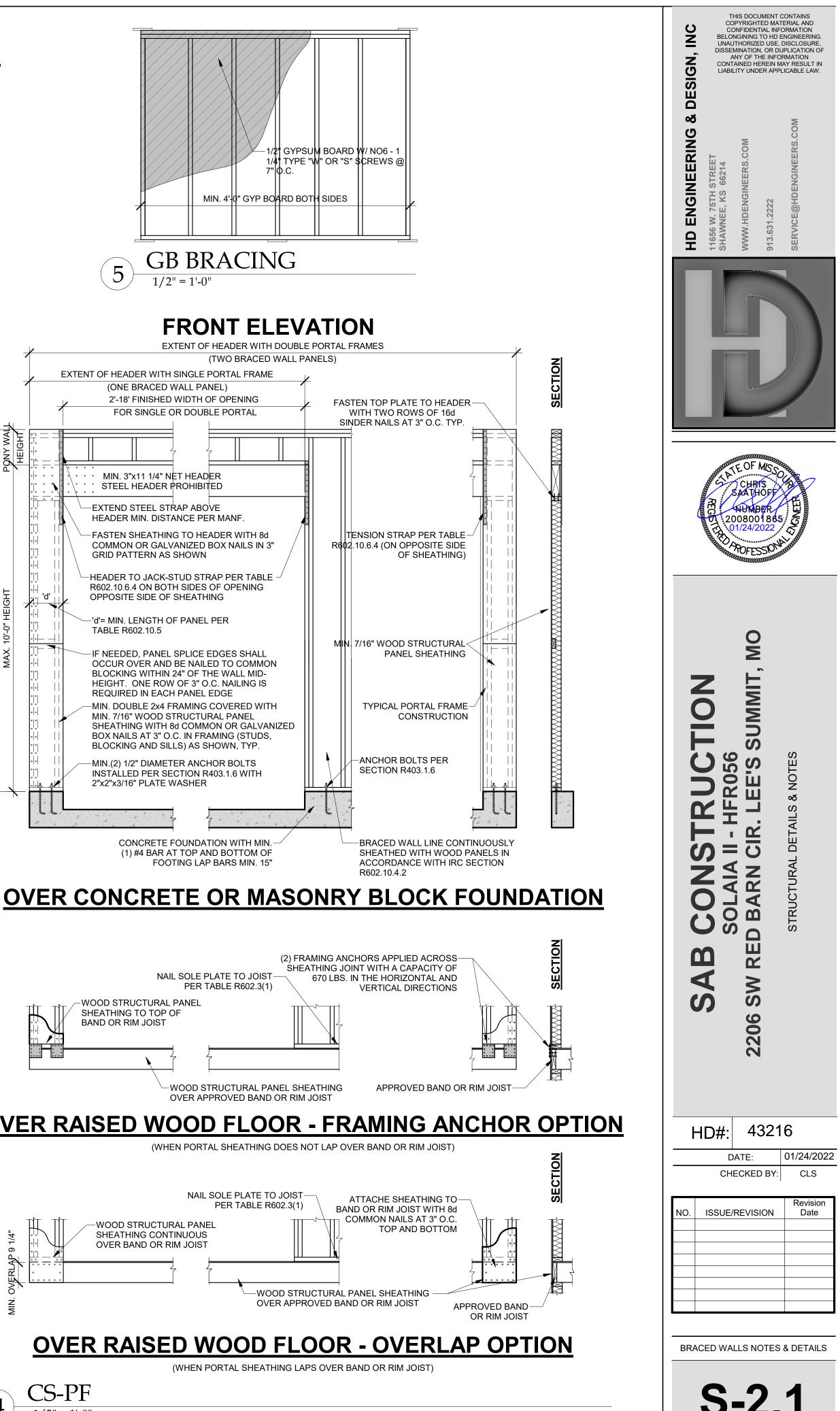
3/8" = 1'-0'

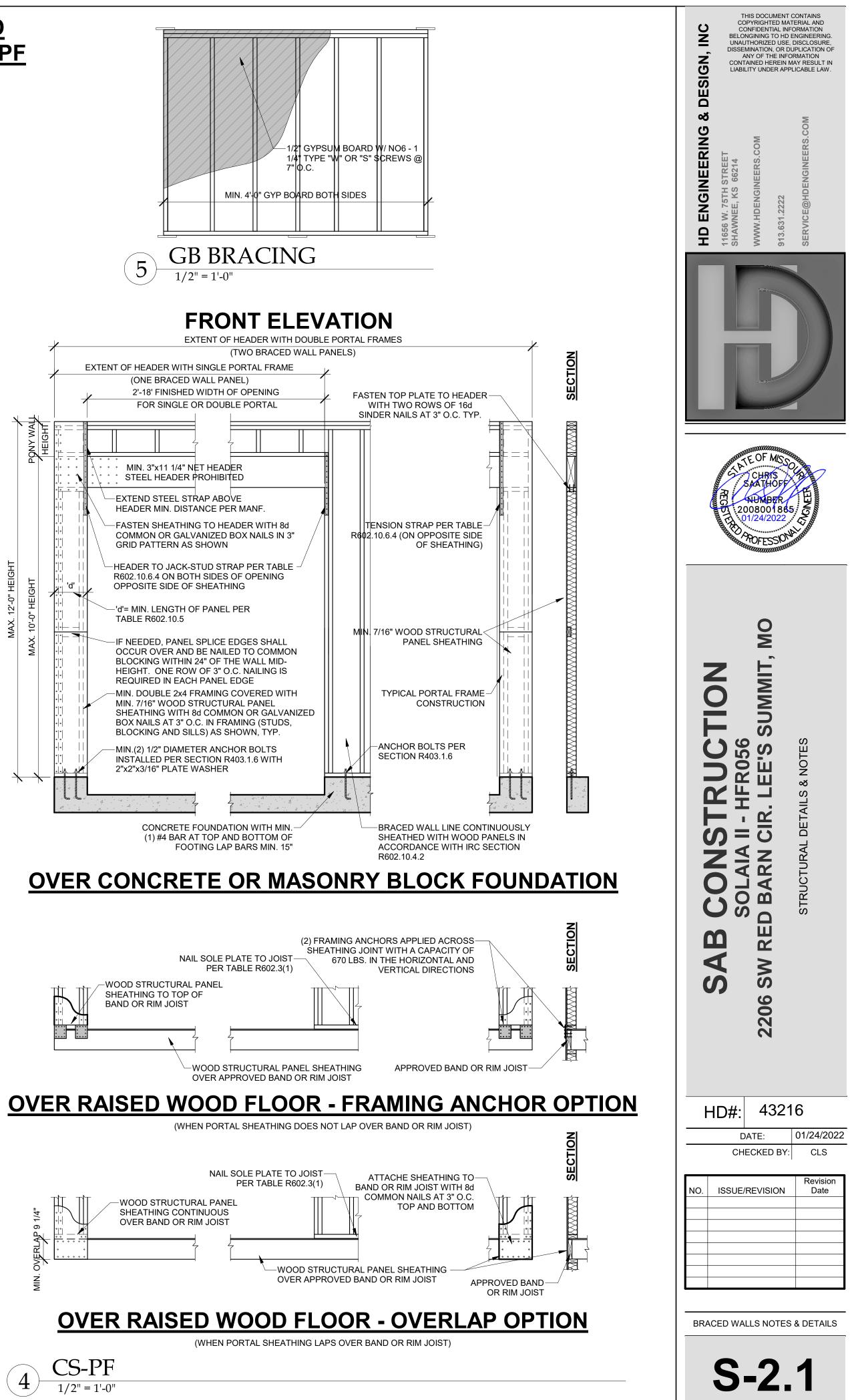


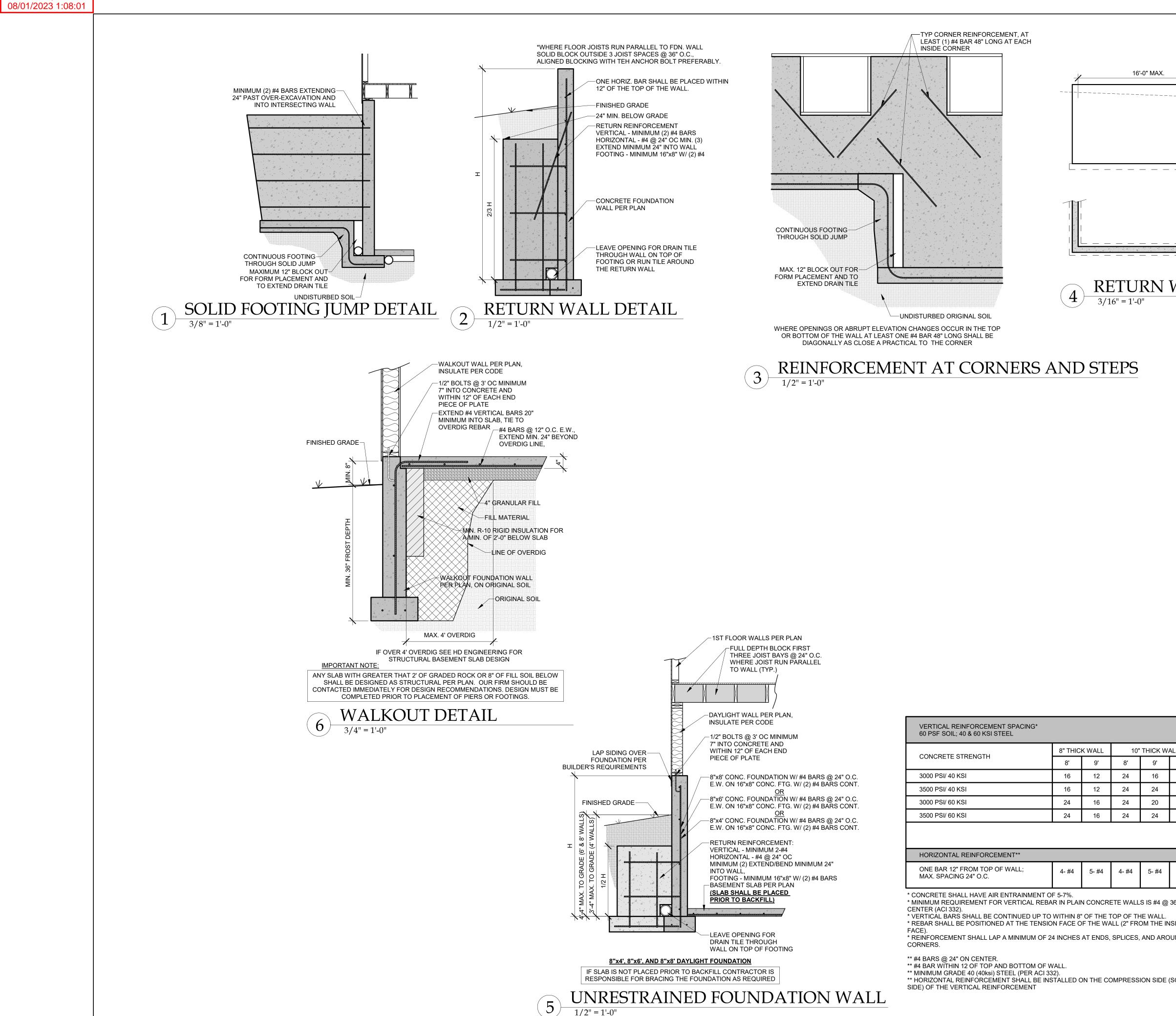












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UNRESTRAINED FOUNDATION WALL 1/2" = 1'-0"

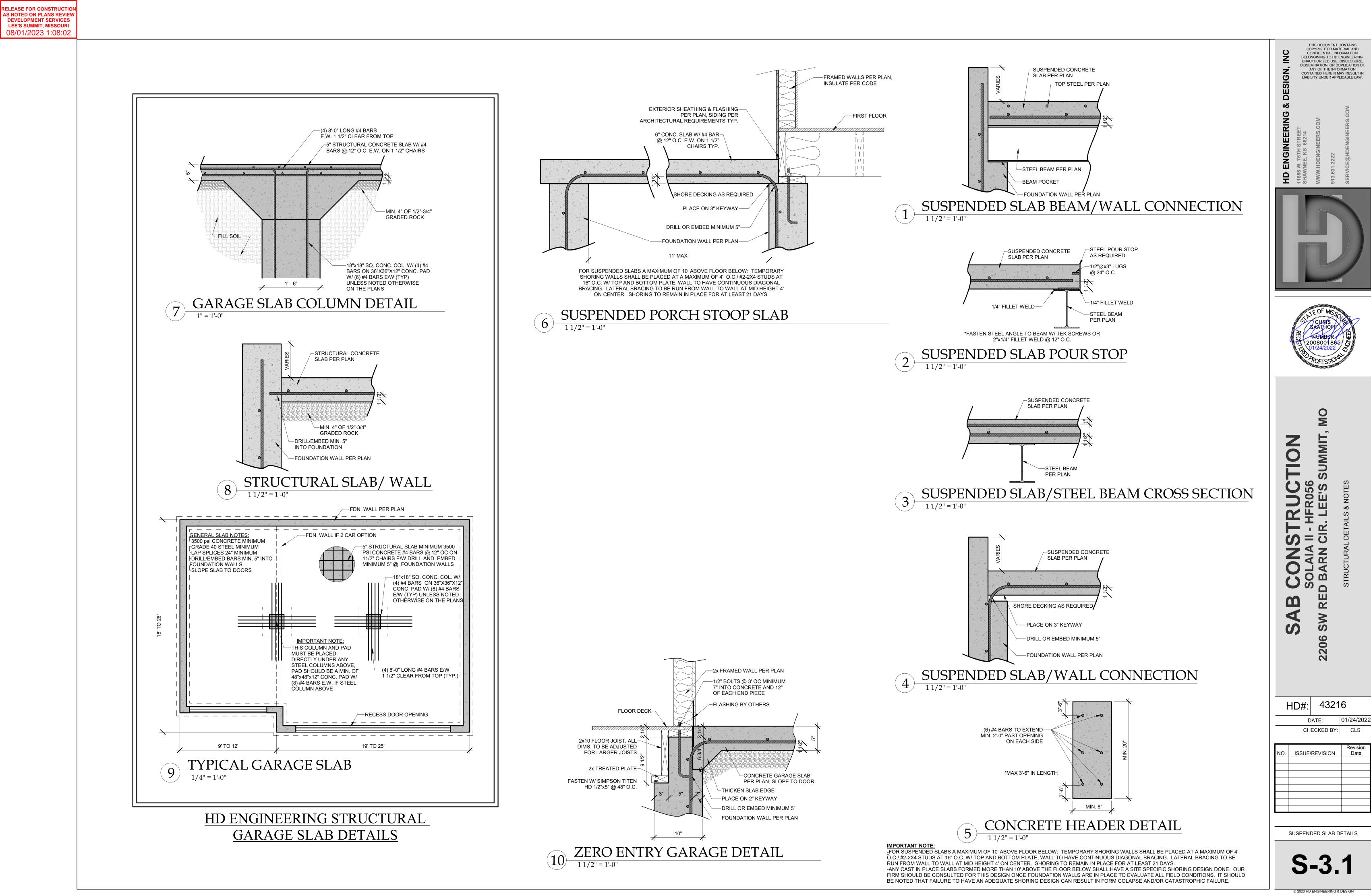
CONCRETE STRENGTH	8" THICK WALL		10" THICK WAL		
CONCRETE STRENGTH	8'	9'	8'	9'	
3000 PSI/ 40 KSI	16	12	24	16	
3500 PSI/ 40 KSI	16	12	24	24	
3000 PSI/ 60 KSI	24	16	24	20	
3500 PSI/ 60 KSI	24	16	24	24	

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* MINIMUM REQUIREMENT FOR VERTICAL REBAR IN PLAIN CONCRETE WALLS IS #4 @ 36" ON * VERTICÀL BARS SHALL BE CONTINUED UP TO WITHIN 8" OF THE TOP OF THE WALL. * REBAR SHALL BE POSITIONED AT THE TENSION FACE OF THE WALL (2" FROM THE INSIDE * REINFORCEMENT SHALL LAP A MINIMUM OF 24 INCHES AT ENDS, SPLICES, AND AROUND ** HORIZONTAL REINFORCEMENT SHALL BE INSTALLED ON THE COMPRESSION SIDE (SOIL

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