

ROOF Scale: 1/4" = 1'-0"

*ALL RAFTERS SHALL BE 2' X 6' #2 @ 16' D.C., UNLESS NOTED OTHERWISE.

see detail 7/s3.2 For alternate rafter bearing detail when rafters are required to bear higher than the vall double top plate.

Flashing note: DRIP Edge, valleys and flashings to be metal clad.

Roof Notes:

ROOF DESIGNED FOR LIGHT ROOF COVERING 30psf TOTAL LOAD [10psf DL, 20psf LL (SL)]

* RAFTERS (HEM-FIR, DDUG-FIR, DR EQUAL): SEE SPAN CHARTS BELDW

<u>Code minimum</u>

	RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN	
	#2-2x6	024" D.C.	11′-7 ″	
>>>	#2-2x6	@16″ D.C.	14'-2 '	‹‹‹
	#2-2x8	@24″ D.C.	14'-8 '	
	#2-2x8	@16″ D.C.	17'-11 '	
	#2-2x10	024 " D.C.	17'-10 '	
	#2-2x10	@16″ D.C.	21'-11 '	
	NOTE: CODE	E MINIMUM ALL	NVS END & DAETED DEELECTION	חבו

Note: Code Minimum Allows for a rafter deflection of L/180 total Load

HIGHER FERFORMANCE (RECOMMENDED)						
RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN				
#2-2x6	@24″ D.C.	8'-6 '				
#2-2x6	@16″ D.C.	9′-9 ′				
#2-2x8	024″ D.C.	11'-3"				
#2-2x8	@16″ D.C.	12'-9 '				
#2-2x10	024° D.C.	14'-3 '				
#2-2x10	@16″ D.C.	16'-3 '				
DEFLECTION = L/360 LIVE LOAD, L/240 TOTAL LOAD						

* VAULTS TO BE 2x10 DEPTH

* RIDGE BOARDS ARE: (UNLESS OTHERWISE NOTED)

- #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 OVER 10/12 PITCH

* ALL HIPS & VALLEYS ARE: (UNLESS OTHERWISE NOTED)

- #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 OVER 10/12 PITCH

* PURLINS ARE 2X6 MIN.

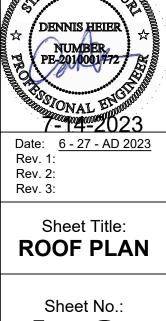
PURLIN STRUTS ARE AT 4'-0' D.C.
PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 DEGREE ANGLE WITH THE HORIZONTAL
ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED LENGTH DF 8'-0'
PURLINS STRUTS SHALL BE CONSTRUCTED IN A 'T' CONFIGURATION AND PER THE FOLLOWING CHART;

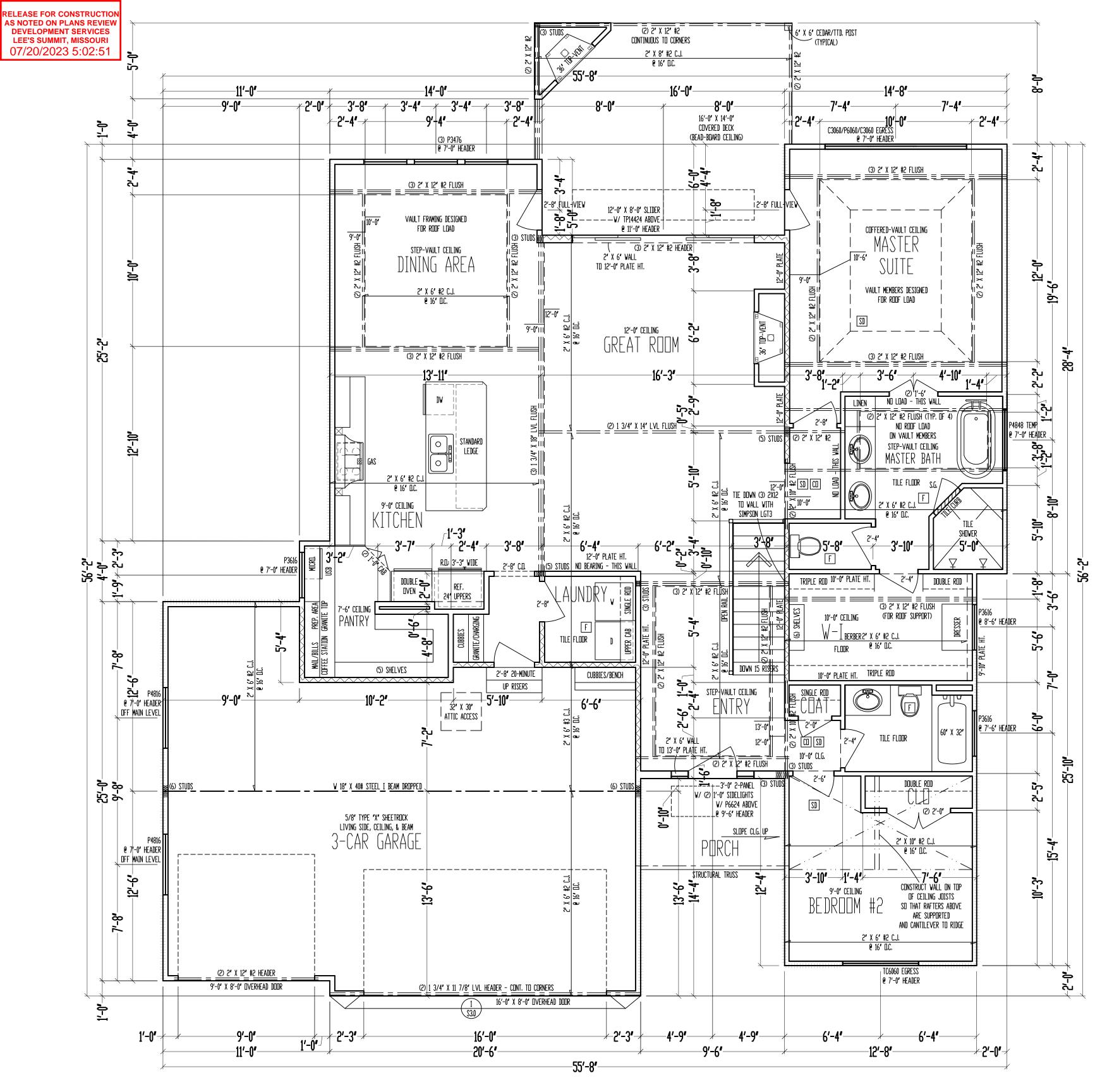
PURLIN STRUT	MAX PURLIN STRUT LENGTH
(2) 2x4	8'-0 '
(1) 2x4 & (1) 2x6	12'-0 '
(1) 2x6 & (1) 2x8	20'-0"
(2) 2x6 & (1) 2x8	30'-0"
CONSULT ARCH./ENGR. >	30'-0"

 * RIDGE BRACES ARE SAME AS PURLIN BRACES-SPACING, SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE)
 * HIP & VALLEY BRACES ARE SAME AS PURLIN SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE)

 * VERTICAL BRACE IF DOT IS UNDER HIP OR VALLEY
 * SLASH IS TOP END OF BRACE (/), DOT IS BOTTOM OF BRACE (o).

- * _____ DENOTES BEARING WALL





9'-0" CEILING **MAIN LEVEL** SCALE: 1/4" = 1'-0"

MAIN	LEVEL:	1789	SQ.	FT,
LOVER	LEVEL:	1225	SQ.	FT,
	TOTAL:	3014	SQ.	FT,

GARAGE: 698 SQ. FT. COV. DUT/LIV: 233 SQ. FT. UNFIN. BASEMENT: 460 SQ. FT.

+++++++++++++++++++++ = VALL BRACING PER FRAMING NOTE #1 AND PER CALCULATIONS ON SHEET \$1.1.

FRAMING NOTES

1. MAIN LEVEL EXTERIOR WALLS SHALL BE SHEATHED W/ 7/16' I.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS.

2. \ \ \ \ \ \ \ \ \ \ = G.B: 1/2" MIN. GYPSUM BDARD OVER STUDS SPACED 24" MAX FASTENED V/ ND. 6 - 1 1/4" TYPE V OR S DRYWALL SCREWS @ 7" D.C. EDGES & FIELD. (MIN. 8'-0' SECTIONS ONE SIDE OF VALL (OR) MIN. 4'-0" SECTION FOR BDTH SIDES) 3. /\/\/\/\/\/\/\/\/\ = LOAD BEARING INTERIOR VALL. 4. (2) 2" X 10" #2 HEADER AT ALL EXTERIOR AND LOAD BEARING VALLS, UNLESS NOTED DTHERVISE. 5. LOW TIES @ 4'-0" D.C. (TYPICAL)

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE VALLS. 7. BLOCK JDISTS ABDVE BEAMS, CANTILEVERS AND LDAD BEARING WALLS WITH JDIST MATERIAL (NOT REQUIRED WITH I-JDISTS). 8. PRDVIDE MULTIPLE STUDS FOR SOLID BEARING BELDW ALL BEAMS. 9. ALL DESIGNATED 2" X 6" VALLS SHALL HAVE DDUBLE KING STUDS AT DODR AND WINDOW OPENINGS.

10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. ALL WALLS TO BE FRAMED V/ MIN. STUD GRADE 2' X 4'S @ 16' L.C., UNLESS NOTED OTHERWISE.

12. Exterior Vall Bottom Plates Shall be nailed to framing Belov Vith 16d Common Nails @ 8' D.C. Max. (Where Applicable.) 13. LVL'S Shown on Plans May be replaced with DF/DF grade 24F-V4 glulam beams of the Same Depth, and the following VIDTHS:

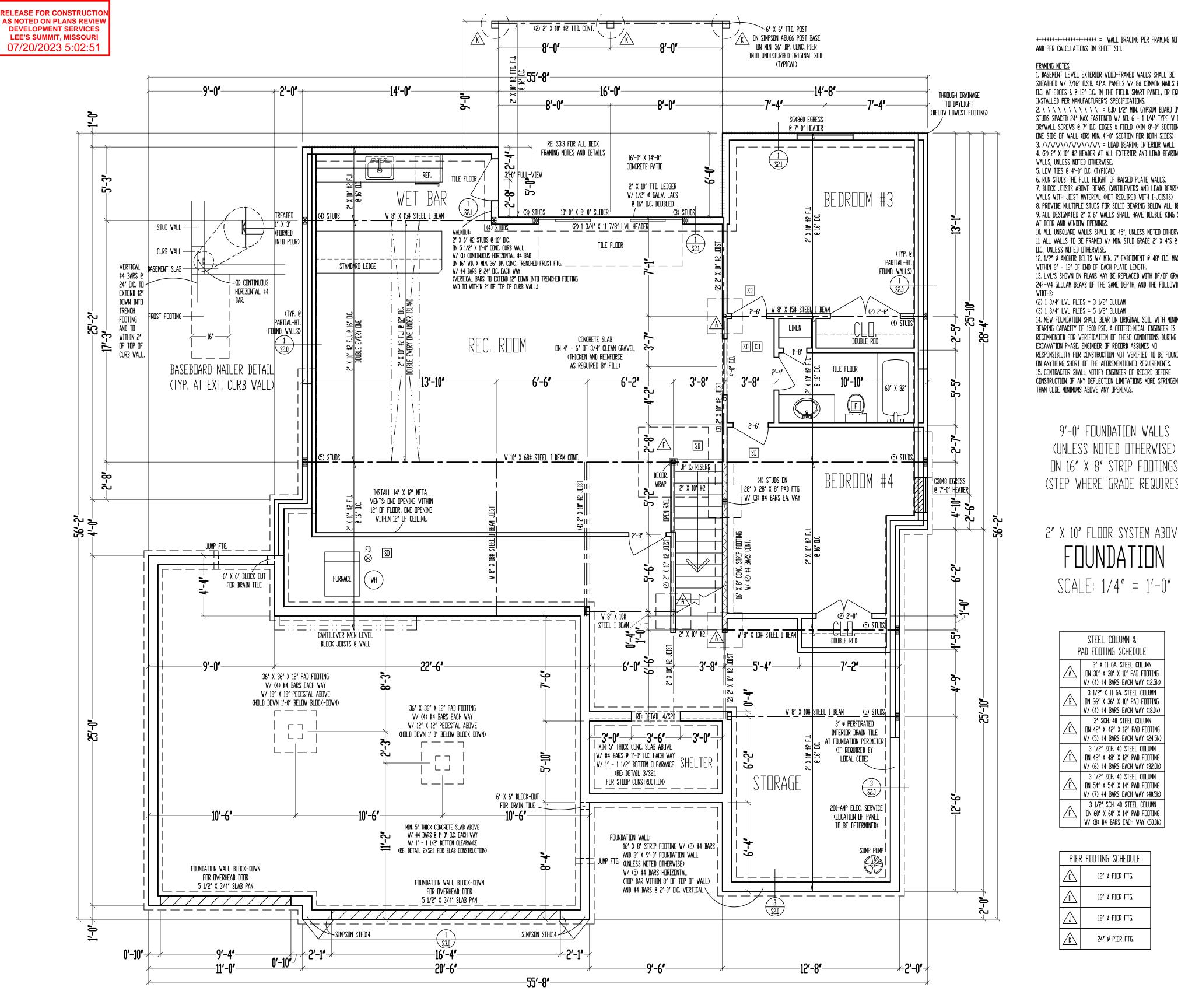
(2) 1 3/4" LVL PLIES = 3 1/2" GLULAM

(3) 1 3/4" LVL PLIES = 5 1/2" GLULAM
 14. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE
 CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT
 THAN CODE MINIMUMS ABOVE ANY DPENINGS.

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"For God so loved the world, that he	gave his only begotten Son, that whosoever believeth in him	but have but have everlasting life" (John 3:16).			
		Office: (816) 554-0400 Email: admin@viewpointdesign.net			
L Re Fa 273 Rd. W	Drawing Ti JF192 Site Descrip ot 192, treat at rms 2nd Property Add 5 SW Hea , Lee's S Missou General Contr (alker Cu Homes, L	Spec tion: The Hook Plat Iress: artland ummit, ri ractor: stom LC			
DENNIS HEIER PE-2014001772 PE-2014001772 DENNIS HEIER PE-2014001772 PE-2023 Date: 6-27-AD 2023 Rev. 1: Rev. 2: Rev. 3: Sheet Title:					



Sheet No.:



F ON 60' X 60' X 14' PAD FOOTING

r	STEEL COLUMN &
F	PAD FOOTING SCHEDULE
	3' X 11 GA. STEEL COLUMN DN 30' X 30' X 10' PAD FODTING W/ (4) #4 BARS EACH WAY (12.5k)
B	3 1/2" X 11 GA. STEEL COLUMN DN 36" X 36" X 10" PAD FODTING W/ (4) #4 BARS EACH WAY (18.0k)
<u>^</u>	3' SCH. 40 STEEL COLUMN DN 42' X 42' X 12' PAD FODTING W/ (5) #4 BARS EACH WAY (24.5k)
	3 1/2" SCH. 40 STEEL COLUMN DN 48" X 48" X 12" PAD FODTING W/ (6) #4 BARS EACH WAY (32.0k)
Ē	3 1/2" SCH. 40 STEEL COLUMN DN 54" X 54" X 14" PAD FODTING W/ (7) #4 BARS EACH WAY (40.5k)
	3 1/2" SCH. 40 STEEL COLUMN

2" X 10" FLOOR SYSTEM AB

<u> </u>	W/ (8) #4 BARS EACH WAY (5	
PIEF	R FOOTING SCHEDULE	
<u>G</u>	12" ø pier ftg.	
\mathbb{A}	16' ø pier ftg.	
\triangle	18 " ø pier ftg.	
\mathbb{R}	24" ø pier ftg.	

9'-0" FOUNDATION WALL (UNLESS NOTED OTHERWIS ON 16" X 8" STRIP FOOTIN STEP WHERE GRADE REQUI	SE) VGS
" x 10" floor system ab FOUNDATION SCALE: 1/4" = 1'-0)" Draw
STEEL COLUMN & PAD FOOTING SCHEDULE 3' X 11 GA. STEEL COLUMN DN 30' X 30' X 10' PAD FOOTING V/ (4) #4 BARS EACH VAY (12.5k) B J1/2' X 11 GA. STEEL COLUMN DN 36' X 36' X 10' PAD FOOTING V/ (4) #4 BARS EACH VAY (18.0k) S' X 36' X 10' PAD FOOTING V/ (4) #4 BARS EACH VAY (18.0k) S' SCH. 40 STEEL COLUMN DN 42' X 42' X 12' PAD FOOTING V/ (5) #4 BARS EACH VAY (24.5k)	RHF1 Site Du Lot 1 Retreat Farms Propert 2735 SW Rd., Lee Mis General Walken Homo



VIDTHS: (2) 1 3/4" LVL PLIES = 3 1/2" GLULAM (3) 1 3/4" LVL PLIES = 5 1/2" GLULAM 14. NEW FOUNDATION SHALL BEAR ON ORIGINAL SOIL WITH MINIMUM BEARING CAPACITY OF 1500 PSF. A GEOTECHNICAL ENGINEER IS RECOMMENDED FOR VERIFICATION OF THESE CONDITIONS DURING THE Excavation phase. Engineer of record assumes no RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED on anything short of the aforementioned requirements. 15. Contractor shall notify engineer of record before CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY OPENINGS.

11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2" X 4"S @ 16" 12. 1/2" Ø ANCHOR BOLTS W/ MIN. 7" EMBEDMENT @ 48" D.C. MAX. & WITHIN 6" - 12" OF END OF EACH PLATE LENGTH. 13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 Glulam beams of the same depth, and the following

D.C., UNLESS NOTED OTHERWISE

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. ALL DESIGNATED 2" X 6" WALLS SHALL HAVE DOUBLE KING STUDS At door and window openings. 10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE.

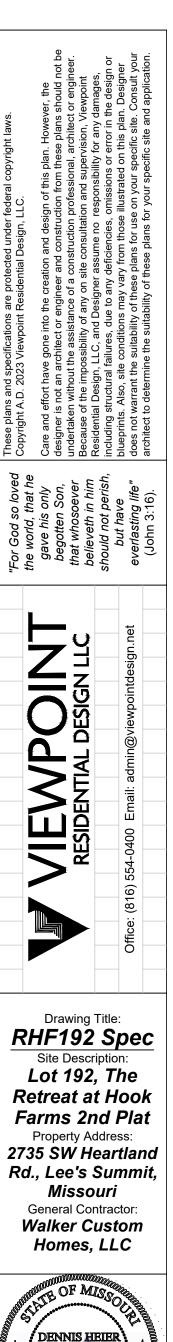
7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS).

WALLS, UNLESS NOTED OTHERWISE. 5. LOW TIES @ 4'-0" D.C. (TYPICAL) 6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS.

D.C. AT EDGES & @ 12" D.C. IN THE FIELD. SMART PANEL, DR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS. STUDS SPACED 24" MAX FASTENED W/ ND. 6 - 1 1/4" TYPE W DR S DRYWALL SCREWS @ 7" D.C. EDGES & FIELD. (MIN. 8'-0" SECTIONS one side of wall (or) min. 4'-0" section for both sides) 3. //////////// = LOAD BEARING INTERIOR WALL. 4. (2) 2" X 10" #2 HEADER AT ALL EXTERIOR AND LOAD BEARING

and per calculations on sheet s1.1.

Sheathed W/ 7/16" D.S.B. A.P.A. Panels W/ 8d Common Nails @ 6"



NUMBER PE-2010001772

ONAL BY

Date: <u>6 - 27</u> - AD 2023

Sheet Title:

FOUNDATION

PLAN

Sheet No.:

Rev. 1:

Rev. 2:

Rev. 3:

-14-2023

RELEASE FOR CONSTR AS NOTED ON PLANS					
DEVELOPMENT SER LEE'S SUMMIT, MISS	VICES	FASTENER SCHEDULE FOR STRUCTURAL MEMBERS			
07/20/2023 5:0	2.52 ^{ES}	CRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION	
			ROOF		
	BLOCKING	BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOE NAIL	4-8d (2 ½ ″ x 0.113")	TOENAIL	
	CE	ILING JOISTS TO PLATE, TOE NAIL	4-8d (2 ½ ″ x 0.113")	PER JOIST, TOENAIL	
		G JOISTS NOT ATTACHED TO PARALLEL R, LAPS OVER PARTITIONS, FACE NAIL	4-10d (3" x 0.128")	FACE NAIL	
	CEILING J	OIST TO PARALLEL RAFTER (HEEL JOINT)	TBLE R802.5.2	FACE NAIL	
	COLLAR T	IE TO RAFTER, FACE NAIL OR 1 ‡ " x 20 GA. RIDGE STRAP TO RAFTER	4-10d (3" x 0.128")	FACE NAIL, EACH RAFTER	
	RA	FTER OR ROOF TRUSS TO PLATE	3-16d BOX NAILS (3½" x 0.135") OR 3-10d COMMON NAILS (3" x 0.148")	2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS	
		RAFTERS TO RIDGE, VALLEY, OR HIP OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM	4-16d (3 ½" x 0.135") - TOENAIL; 3-16d BOX (3 ½" x 0.135") - END NAIL	TOENAIL, END NAIL	
			WALL		
	STUD TO	STUD (NOT AT BRACED WALL PANELS)	10d (3" x 0.128")	16" O.C. FACE NAIL	
		TO STUD AND ABUTTING STUDS AT TING WALL CORNERS (AT BRACED WALL PANELS)	16d (3 <mark>½</mark> " x 0.135")	12" O.C. FACE NAIL	
	BUILT-UP	HEADER, TWO PIECES WITH 🔏 "SPACER	16d (3 <mark>½</mark> " x 0.135")	12" O.C. EACH EDGE FACE NAIL	
	C	CONTINUOUS HEADER TO STUD	4-8d (2 ½ ″ x 0.131″)	TOENAIL	
		TOP PLATE TO TOP PLATE	10d (3" x 0.128")	12" O.C. FACE NAIL	
		DOUBLE TOP PLATE SPLICE	8-16d COMMON (3 ½" x 0.162")	FACE NAIL ON EACH SIDE OF END JOINT (MIN. 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT)	
		PLATE TO JOIST, RIM JOIST, BAND JOIST, CKING (NOT AT BRACED WALL PANELS)	16d COMMON (3 ½" x 0.162")	16" O.C. FACE NAIL	
		PLATE TO JOIST, RIM JOIST, BAND JOIST, LOCKING (AT BRACED WALL PANEL)	3-16d BOX (3 ½" x 0.135")	3 EACH 16" O.C. FACE NAIL	
	TOP	OR SOLE PLATE TO STUD, END NAIL	4-8d BOX (2 ½" x 0.113") - TOENAIL; 3-16d BOX (3 ½" x 0.135") - END NAIL	TOENAIL, END NAIL (SEE LEFT)	
	TOF	P PLATES, LAPS AT CORNERS AND INTERSECTIONS	3-10d BOX (3" x 0.128")	FACE NAIL	
	1" E	BRACE TO EACH STUD AND PLATE	3-8d BOX (2 ½" x 0.113")	FACE NAIL	
	1"x	6" SHEATHING TO EACH BEARING	3-8d BOX (2 1 /2" x 0.113")	FACE NAIL	
	1"x	8" SHEATHING TO EACH BEARING	3-8d BOX (2 ½" x 0.113") - FACE NAIL; WIDER THAN 1"x8" - 4-8d BOX (2 ½" x 0.113")	FACE NAIL	
			FLOOR		
	JOIS	ST TO SILL, TOP PLATE, OR GIRDER	4-8d BOX (2 2 ⁺ x 0.113")	TOE NAIL	
		, BAND JOIST, OR BLOCKING TO SILL OR PLATE (ROOF APPLICATIONS ALSO)	8d BOX (2 ½" x 0.113")	4" O.C. TOE NAIL	
	1" x 6"	SUBFLOOR OR LESS TO EACH JOIST	3-8d BOX (2 ½" x 0.113")	FACE NAIL	
	2"	SUBFLOOR TO JOIST OR GIRDER	3-16d BOX (3 ½" x 0.135")	BLIND AND FACE NAIL	
	2" PLAN	IKS (PLAN & BEAM - FLOOR AND ROOF)	3-16d BOX (3 ½" x 0.135")	AT EACH BEARING, FACE NAIL	
		BAND OR RIM JOIST TO JOIST	3-16d COMMON (3 ¹ / ₂ " x 0.162")	END NAIL	
	BUILT-UF	P GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS	10d BOX (3" x 0.128")	24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES	
	LEDGER S	STRIP SUPPORTING JOISTS OR RAFTERS	4-16d BOX (3 ½" x 0.135")	AT EACH JOIST OR RAFTER, FACE NAIL	
	BI	RIDGING OR BLOCKING TO JOIST	2-10d BOX (3" x 0.128")	EACH END, TOENAIL	

RIPTION OF BUILDING MATERIAL VOOD STRUCTURAL PANELS, SU	S DESCRIPTION OF FASTENER IBFLOOR, ROOF AND INTERIOR WALL SHEA	STRUCTURAL MEMBERS EDGE SPACING (INCHES) ATHING TO FRAMING AND PARTICLEBO	DARD WALL SHEATHING TO FRAMING ¹
K" - Z"	6d COMMON (2" x 0.113") NAIL (SUBFLOOR, WALL) 8d COMMON NAIL (ROOF)	6	12
¹ % ₃₂ " - 1"	8d COMMON NAIL (2 ¹ / ₂ " x 0.131")	6	12
1 % "- 1 % "	10d COMMON (3" x 0.148") NAIL OR 8d (2 ¹ / ₂ " x 0.131") DEFORMED NAIL	6	12
	OTHER WALL	. SHEATHING ¹	
STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	$1\frac{1}{2}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR $1\frac{1}{4}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6
₽ STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	$1\frac{2}{4}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR $1\frac{1}{2}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6
" GYPSUM SHEATHING	1½" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1¼" SCREWS, TYPE W OR S	7	1 7
% " GYPSUM SHEATHING	1 ³ / ₄ " GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1 ⁹ / ₈ " LONG; 1 ⁵ / ₈ " SCREWS, TYPE W OR S	7	7
w	OOD STRUCTURAL PANELS, COMBINATION	N SUBFLOOR UNDERLAYMENT TO FRA	MING
¥" AND LESS	6d DEFORMED (2" x 0.120") NAIL OR 8d COMMON (2 ½ " x 0.131") NAIL	6	12
% " - 1"	8d COMMON (2½" x 0.131") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12
11⁄8" - 11⁄4"	10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (22" x 0.120") NAIL	6	12

LISTED IN THIS TABLE

RELEASE FOR CONSTRUCTION

FOUNDATION NOTES

- CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLABS THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION
- STANDARDS PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT.
- FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.
- FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE FOR FROST PROTECTION.
- CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS
- AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0 REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE) INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND, GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB SHALL BE DESIGNED BY A LICENSED ENGINEER
- SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH ½" Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS) AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH. PER IRC SECTION R403.1.6 FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET 13.
- THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

- 15. ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 - 2x10's, UNLESS NOTED OTHERWISE 16. ON PLANS
- BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A
- MINIMUM OF 1/3 ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED 19.
- OTHERWISE WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS. ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT 21.
- MATERIAL JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.
- JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE 23. COLD-FORMED STEEL JOIST HANGERS
- JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 1%" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3
- ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3. 26. 27. ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER ½ OF VERTICAL DISTANCE BETWEEN CEILING AND ROOF
- BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED 28 PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH 29.
- A %" GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION) ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi
- ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND 32. THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. ½" x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.
- WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING
- ALL ROOF SHEATHING SHALL BE 16 OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD 34.

GLAZING NOTES

- GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0" ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2
- 36. ATTIC VENTILATION
- ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH %" TO ½" OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/50 OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

- 38. PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9", IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, 39. INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF
- ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

MASONRY VENEER

- 40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1/2", WITH NOT LESS
- THAN $\frac{5}{8}$ " MORTAR OR GROUT COVER TO OUTSIDE FACE. VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 🔏 CORRUGATED.
- EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE 42. THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY. VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL 43.
- OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

GARAGE NOTES

DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1%" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE 45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC 2018

DIMENSIO

MULTIPLE-PLY WOOD BEAM FASTENING SCHEDULE								
SIONAL LUMBER BEAM SIZE/TYPE	FASTENERS	LVL BEAM SIZE/TYPE	FASTENERS	LVL BEAM SIZE/TYPE	FASTENERS			
(2) 2x	(2) ROWS 10d @ 12" O.C. ONE SIDE	(2) 1 ³ ⁄ ₄ " UP TO 11 ⁷ ⁄ ₈ " DEPTH	(2) ROWS 16d @ 12" O.C. ONE SIDE	(3) 1 ¾" x 14"+ DEPTH	(3) ROWS 16d @ 12" O.C. BOTH SIDES			
(3) 2x	(2) ROWS 10d @ 12" O.C. BOTH SIDES	(2) 1 ⅔" 14"+ DEPTH	(3) ROWS 16d @ 12" O.C. ONE SIDE	(4) 1 ¾" UP TO 11 ½" DEPTH	(2) ROWS			
(4) 2x	(2) ROWS ½" x 5" SIMPSON SDS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM, BOTH SIDES	(3) 1 ¾" UP TO 11 ½" DEPTH	(2) ROWS OF 16d @ 12" O.C. BOTH SIDES	(4) 1 ¾" x 14"+ DEPTH	(3) ROWS ½" x 5" SIMPSON SDS OR SDWS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM BOTH SIDES			

GARAGE NOTES (CONTINUED)

45.

THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 5/8" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM 5/2" TYPE X GYP. BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH 5/8" GYP. BOARD. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING AND SHALL BE FASTENED WITH 21/2"" x 0.120" NAILS AT 7" O.C. STAGGERED WITH (7) 31/4" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER. MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

DESIGN LOADING (PER TABLE R301.5)

MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS (PSF)							
USE	LIVE LOAD	DEAD LOAD					
UNINHABITABLE ATTICS WITHOUT STORAGE	10	10					
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20	10					
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30	10					
BALCONIES (EXTERIOR) AND DECKS	40	10 ^d					
FIRE ESCAPES	40	10					
GUARDRAILS AND HANDRAILS ^a	200 [°]	-					
GUARDRAIL IN-FILL COMPONENTS ^b	50 ^c	-					
PASSENGER VEHICLE GARAGES	50	DEPENDENT UPON SLAB CONSTRUCTION					
ROOMS OTHER THAN SLEEPING ROOM	40	10 ^d					
SLEEPING ROOM	30	10 ^d					
STAIRS	40	10 ^d					

- a. A single concentrated load applied in any direction at any point along the top b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the
- infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load. d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An
- additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed

INSULATION/EFFICIENCY

- BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1.

INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT (TABLE N1102.1.1)

CLIMATE ZONE	4-A
FENESTRATION U-FACTOR	0.35
SKYLIGHT U-FACTOR	0.55
GLAZED FENSTRATION SHGC	0.40
CEILING R-VALUE	49
WOOD FRAME WALL R-VALUE	15
MASS WALL R-VALUE	8 / 13
FLOOR R-VALUE	19
BASEMENT WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
SLAB R-VALUE AND DEPTH	10 AT 2'-0"
CRAWL SPACE WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	8
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	6
CATHEDRAL VAULTED CEILING R-VALUE	38

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2018 IRC. EXCEPTIONS:

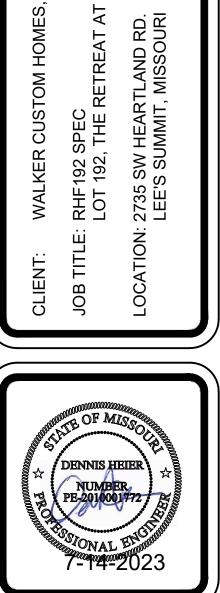
- AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT SEALS.
- WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE. THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:
- POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM 1. PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED
- OR OTHERWISE SEALED DURING THE TEST. 2. ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE
- DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE
- TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA. EXCEPTION: THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS
- LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

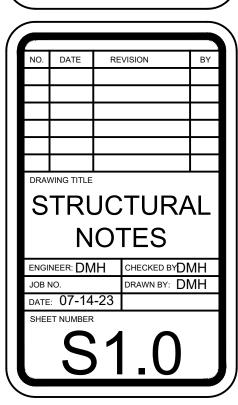
MECHANICAL VENTILATION SYSTEM FAN EFFICACY							
FAN LOCATION	AIR FLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY (CFM/WATT)	AIR FLOW RATE MAXIMUM (CFM)				
RANGE HOODS	ANY	2.8	ANY				
IN-LINE FAN	ANY	2.8	ANY				
BATHROOM, UTILITY ROOM	10	1.4	90				
BATHROOM, UTILITY ROOM	90	2.8	ANY				



HOOK

AT





RESIDENTIAL SEISMIC & WIND ANALYSIS

							INPUT	
DETERMINE WEIGHT	OF HOUSE:						CALCULATED VALUE	
LOCATION		·			DEAD LOAD (psf)	AREA (ft ²)	WEIGHT (lbs.)	
ROOF					10	2720	27200	
CEILING					10	2720	27200	
FIRST FLOOR					10	2720	27200	
				WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WEIGHT (lbs)	
FIRST FLOOR EXT. W	ALL DL			223.68	10	10	22368	
					DEAD LOAD (psf)	AREA (ft2)	WEIGHT (lbs)	
FIRST FLOOR INT. P/	ARTITION WALL DL				6	2720	16320	
	PRO	JECTED AREAS (WIND	DESIGN PER 115 MPH	3-SECOND GUST, EXPOSL	JRE C AND MEAN ROOF HEIGHT <= 30	FT ASSUMED)		
	FRONT	-TO-BACK			SIDE-TO-SIDE			
÷	AREA	LOAD			AREA	LOAD		
SLOPED ROOF	216	1838		SLOPED ROOF	726	6172		
VERT. ROOF	266	3307	CUMULATIVE	VERT. ROOF	0	0	CUMULATIVE	
1ST	612.37	7613	12833	1ST	617.87	7675	13922	
B \$MT ^a	0	0	0	BSMT ^a	100	1416	8377	
			PRESSURE (PSF	-) - PER ASCE CH. 6		•		
	SLOPED ROOF ZONE B 9.7			9.7	ZONE C	11.3	2a (FIG. 28.6-1, ASCE7)	
	WALL/VERT. ROOF	ZONE A		14.2	ZONE D	7.7	11.134	
	MEAN ROOF HT., h		23					

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area.

 $q_{z10}=0.00256K_zK_{zt}K_dV^2 (ASCE7-10 Velocity Pressure) \qquad \qquad q_{z10_ASD}=0.6q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)$

1ST FLOOR TRIBUTARY WEIGHT BASEMENT TRIBUTARY WEIGHT

RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 07/20/2023 5:02:52

 ${\rm S}_{\rm S}\,({\rm SITE}\,\,{\rm GROUND}\,\,{\rm MOTION}$ - ${\rm \%g}$ - FROM ASCE7 SEISMIC MAP)

F_a (from ASCE7 Table 11.4-1) S_{DS} (= 2/3 * S_S * F_a)

R (from ASCE7 Table 12.2-1)

		SEISMIC SHEAR				
ON OR INT	ASCE7 (Eq. 12.8-1):	1): V (= 1.2 * S _{DS} * W / R) (lbs.) 1550 1550				
Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable She	ear (#/LF)	Code Referer	
Exterior <u>(Option #1)</u>	7/16" APA Rated Plywcod/OSB	1-1/2" 16gs. Staples w/ 1" penetration@ 6" OC Edges, 8" OC Field For 24" stud spacing, 12" CC Field For 16" stud spacing	155		per IBC, Tab 2306.3(1)	
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga, Staples w/ 1" penetration@ 4" OC Edges, 5" OC Field For 24" stud spacing, 12" CC Field For 16" stud spacing	230		per IBC, Tab 2306.3(1)	
Exterior <u>(Option #3)</u>	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 5" OC Field For 24" stud spacing, 12" CC Field For 16" stud spacing	310		per IBC, Tab 2306.3(1)	
Exterior <u>(Option #4)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220		AF&PA SDF Table 4.3A	
Exterior (Option #5)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320		AF&PA SDF Table 4.3A	
Exterior (<i>Option #6)</i>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each pane edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410		AF&PA SDF Table 4.3A	
Interior	1/2" Gypsum Board	No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60		per IBC, Tat 2306.4.4	
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacture specifications - see detail on sheet S3)	325			

EXTERIOR SHEATHING OPTION FOR FIRST FLOOR EXTERIOR SHEATHING OPTION FOR BASEMENT WALLS
 WIDTH OF 1ST STORY (FT.)
 55.67

 DEPTH OF 1ST STORY (FT.)
 56.17

 BACK WALL OF GARAGE (FT.)
 22

 GAR. WALL: 1=F-B, 2=S-S
 2

WIDTH OF 2ND STORY (FT.) DEPTH OF 2ND STORY (FT.)

65584

65584

12.0%

1.6

0.128

6.5

EXTERIOR STRUCTURAL WALL LENGTHS (ft.) & RESISTANCES										
		SE	ISMIC			WIND				
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)		
1ST FLOOR	77	29260	28	10640	77	40964	28	14896		
BASEMENT	0	0	25	11750	0	0	25	16450		
		1		*	1 1	,	*			
		ADDITIONAL RESIS	TANCE REQUIRED]	Anchor Bolt Spacing (in.)		16d Nail Spacing req'd at bottom plate (in.)			
		SEISMIC	WIND		diameter (in.)	0.5	1st Floor F-B	24		
1ST FLOOR FRONT-T	O-BACK	0	0		Shear value (per NDS)	944	1st Floor S-S	22		
1ST FLOOR SIDE-TO-	SIDE	0	0		Spacing F-B (inches)	158.7				
BASEMENT FRONT-TO-BACK		0	0		spacing S-S (inches)	144.9				
BASEMENT SIDE-TO-	SIDE	0	0	1						

RESISTANCE REQUIRED IN ADDITION TO RESISTANCE PROVIDED BY EXTERIOR WALLS**										
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)	OK?			
1ST FLOOR FRONT-TO-BACK	0				1	0	YES			
1ST FLOOR SIDE-TO-SIDE	0					0	YES			
BASEMENT FRONT-TO-BACK	0					0	YES			
BASEMENT SIDE-TO-SIDE	0					0	YES			

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE), 2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING

PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRED

	WIND OFLIFT AWALTSIS									
	X/12	DEGREES								
ROOF PITCH (MAX)	10	39.8	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2						
		ASCE 7								
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)						
OVERHANG	1	-1.08	225.68	-1.08						
	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)			
MAIN ROOF**	3126.9839	-451.327824	3578.311724	-1.08	-0.36	-801	-3.6			
*ALONG PERIMETER	*ALONG PERIMETER TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS)					UPLIFT OK				
**INSIDE EXTERIOR WALLS RESISTANCE DUE TO DEAD WEIGHT & (3) 10d TOENAILS					251.6					

NOTE FOR CONSTRUCTION:

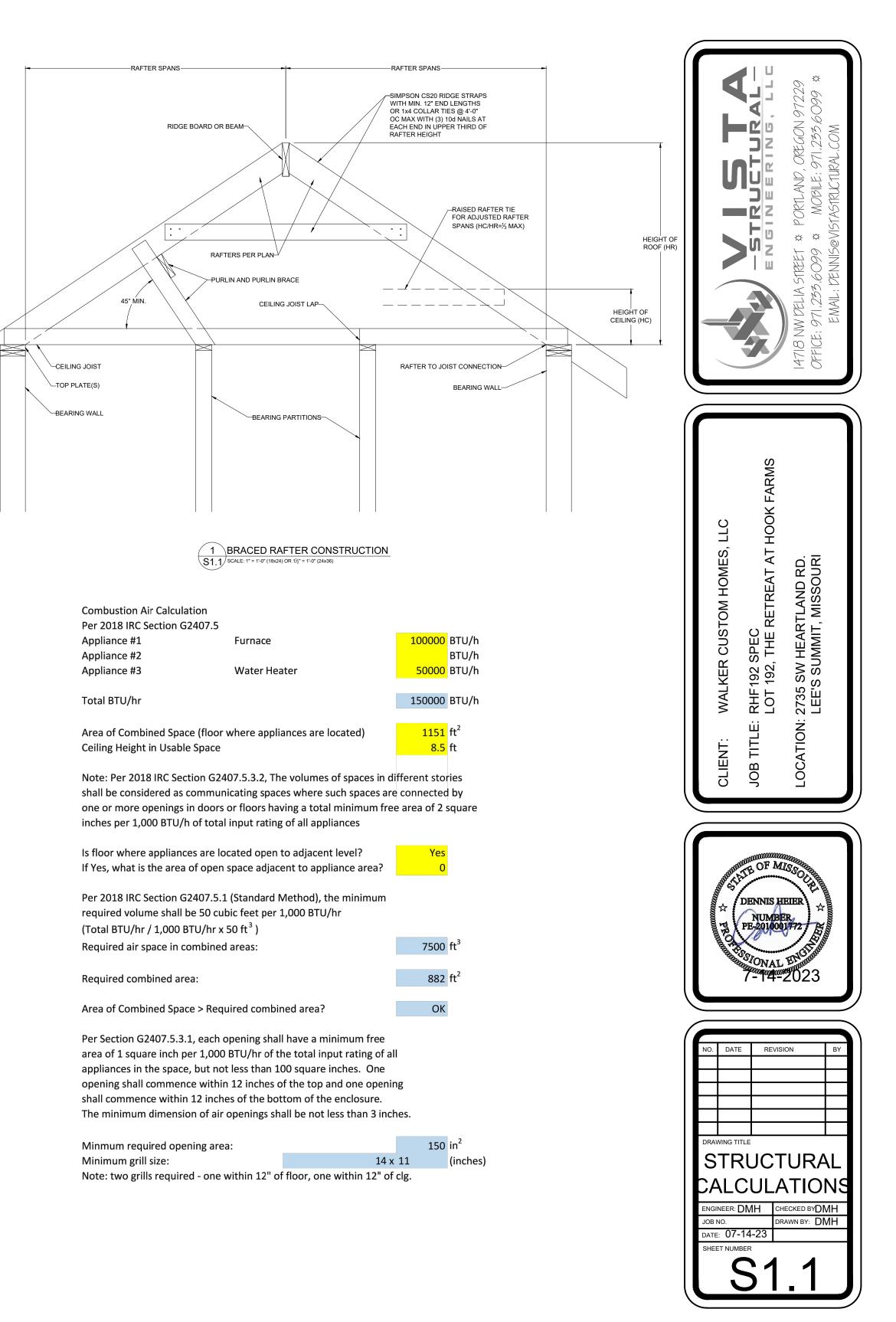
THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

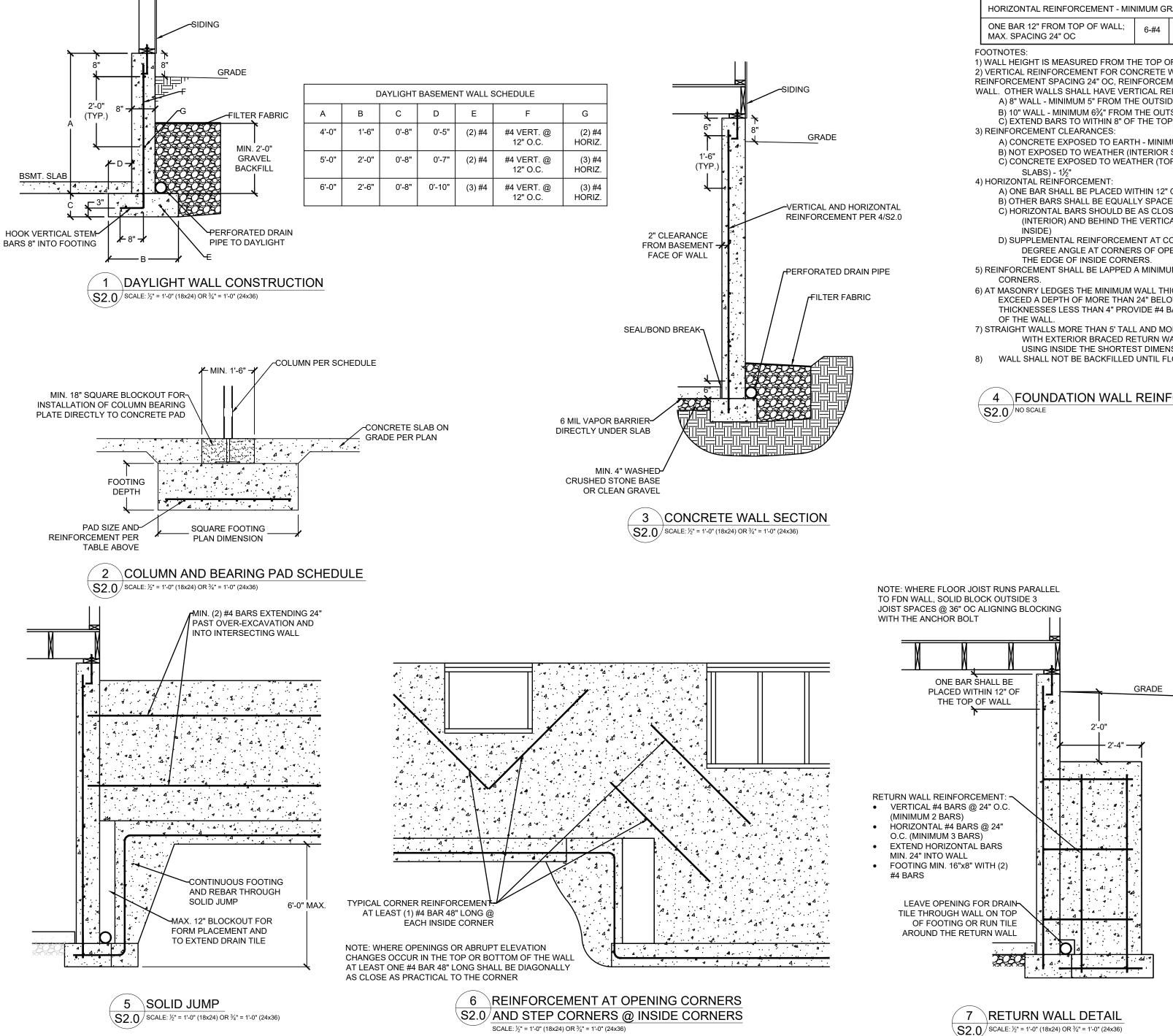
NOTE FOR DESIGN:

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

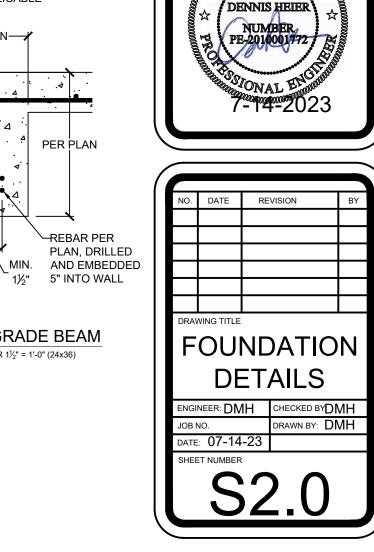
NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING

WITH CONSTRUCTION





	VERTICAL REINFORCEMENT SPAC	NG								I U	
	CONCRETE STRENGTH/GRADE REINFORCEMENT (#4 BARS)	8"	THICK W	VALL	10'	THICK V	/ALL				01
		8'	9'	10'	8'	9'	10'		- E	V.	EGON 9722 1,233,6099
	3,000 PSI/ GRADE 40	24	24	16	24	24	18				00000000000000000000000000000000000000
	3,500 PSI/ GRADE 40	24	24	16	24	24	18				
	3,000 PSI/ GRADE 60 3,500 PSI/ GRADE 60	24	24	16	24	24	18		l (r		20,0
					24	24	18				DR1LAND, MOBILE:
	HORIZONTAL REINFORCEMENT - M ONE BAR 12" FROM TOP OF WALL;	6-#4	7-#4	7-#4	6-#4	7-#4	7-#4		-	∎ CZ Z	PORTLAND MOBILE
	 FOOTNOTES: 1) WALL HEIGHT IS MEASURED FROM 2) VERTICAL REINFORCEMENT FOR COREINFORCEMENT SPACING 24" OC, REWALL. OTHER WALLS SHALL HAVE VEA) 8" WALL - MINIMUM 5" FROM TB) 10" WALL - MINIMUM 6% FROM TG) C) EXTEND BARS TO WITHIN 8" CONCRETE EXPOSED TO EAFB) NOT EXPOSED TO WEATHER C) CONCRETE BARS SHALL BE PLACED NB) OTHER BARS SHALL BE PLACED NB) OTHER BARS SHALL BE EQUAC) HORIZONTAL BARS SHOULD FUNCTION (INTERIOR) AND BEHIND TO INSIDE) D) SUPPLEMENTAL REINFORCEMENT SHALL BE LAPPEI CORNERS. 6) AT MASONRY LEDGES THE MINIMUM EXCEED A DEPTH OF MORE THAN THICKNESSES LESS THAN 4" PR OF THE WALL. 7) STRAIGHT WALLS MORE THAN 5' TA WITH EXTERIOR BRACED I USING INSIDE THE SHORT 8) WALL SHALL NOT BE BACKFILLE 4 FOUNDATION WALL STALL NOT BE BACKFILLE 	DNCRETE EINFORCE RTICAL R HE OUTSI M THE OU DF THE TO RTH - MINII (INTERIOF ATHER (TO WITHIN 12' LLY SPAC BE AS CLC HE VERTIC MENT AT C ERS OF OF NERS. D A MINIM M WALL TH N 24" BEL OVIDE #4 LL AND M RETURN V EST DIMEI D UNTIL F	WALLS 1 MENT M/ EINFORC DE FACE TSIDE FA P OF THI SIDE OF DP CLEAI COF THE ED WITH DSE TO T CAL REIN CORNERS PENINGS UM 24" A HICKNESS OW THE BARS AT ORE THA VALLS. V NSION BE LOOR SY	THAT ARE AY BE PLA CEMENT A E WALLS) FWALLS) RANCE IN TOP OF T I SPACING HE TENSI FORCEMI S - PLACE I T ENDS, S S SHALL E TOP OF T MAX. 24" N 16 FEE VALL LENG STEM AN	NOT FUL ACED IN 1 S FOLLO -3⁄4" GARAGE THE WALL GARAGE INT TO ON FACE ENT (I.E (1) #4 BA REINFOR SPLICES, BE 3½". L HE WALL OC TO W T LONG S GTH SHA NTERSEC ID DIAPH	L HEIGH HE MIDE WS: AND DR EXCEEL AS POSI 2" TOWA R 48" LO CEMENT AND ARC EDGES S FOR W (ITHIN 8" SHALL BE LL BE ME CTING W, RAGM AF	T, AND FOI DE OF THE IVEWAY 24" OC SIBLE RD THE NG AT 45 WITHIN 6" DUND CHALL NOT ALL OF THE TO PROVIDEI GASURED ALLS	QF PP D	CLIENT: WALKER CUSTOM HOMES, LLC	JOB TITLE: RHF192 SPEC LOT 192, THE RETREAT AT HOOK FARMS	LOCATION: 2735 SW HEARTLAND RD. LEE'S SUMMIT, MISSOURI
DN WALL, SOLIE	R JOIST RUNS PARALLEL) BLOCK OUTSIDE 3 ' OC ALIGNING BLOCKING										
		GRADE	_			IF	AB PER PI APPLICAB		A PRO	DENNIS H NUMBE PE-201000	EIER



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8 CONCRETE GRADE BEAM

SCALE: 1" = 1'-0" (18x24) OR 1¹/₂" = 1'-0" (24x36)

MIN. 1

3" CLEAR (TYP.

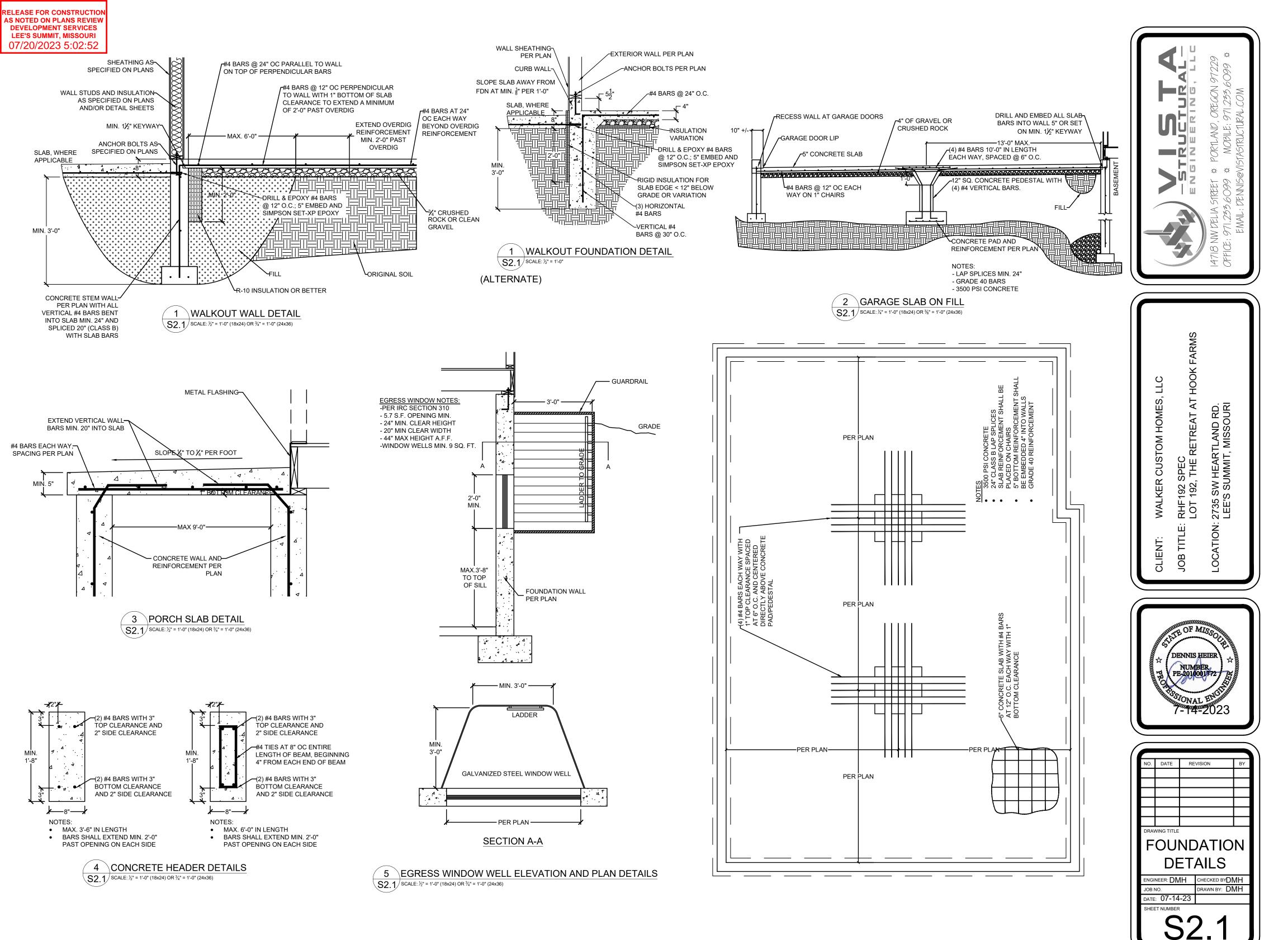
3'

CLEAR-

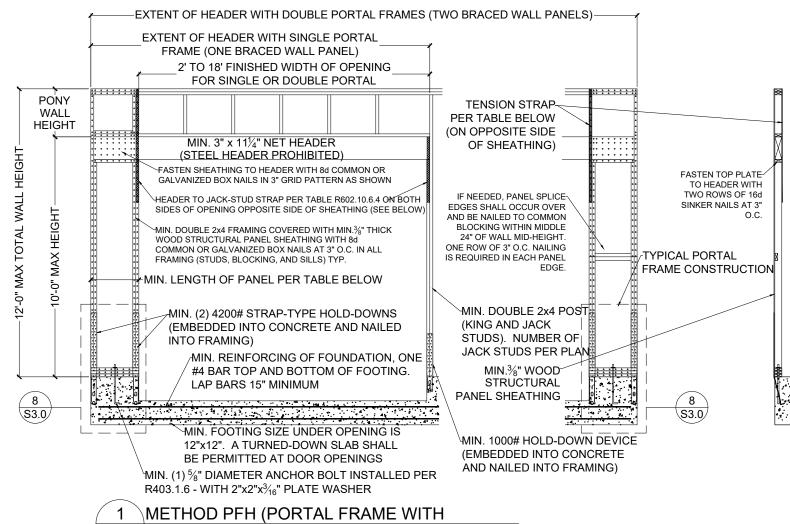
(TYP.)

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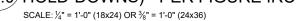
PER PLAN



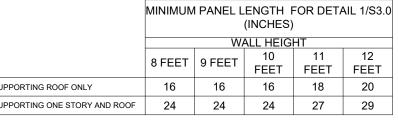
RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 07/20/2023 5:02:52



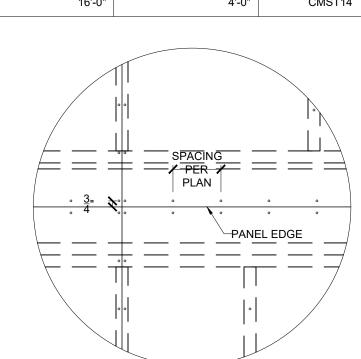




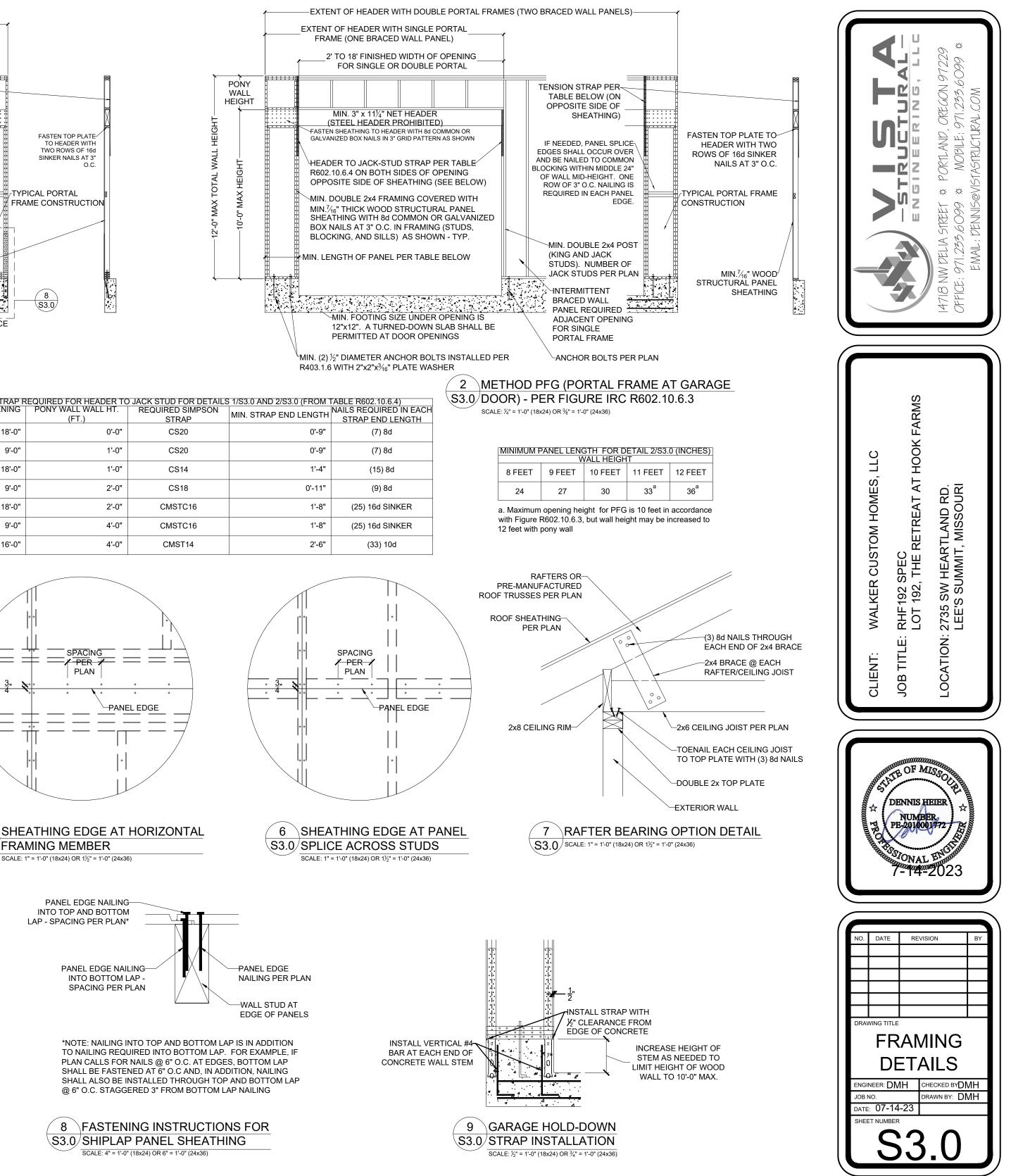
0.2				
1		REQUIRED FOR HEADER TO		1/S3.0 AND 2
		PONY WALL WALL HT.	REQUIRED SIMPSON	MIN. STRAP
	(FT.)	(FT.)	STRAP	
	18'-0"	0'-0"	CS20	
	9'-0"	1'-0"	CS20	
	18'-0"	1'-0"	CS14	
	9'-0"	2'-0"	CS18	
	18'-0"	2'-0"	CMSTC16	
	9'-0"	4'-0"	CMSTC16	
	16'-0"	4'-0"	CMST14	

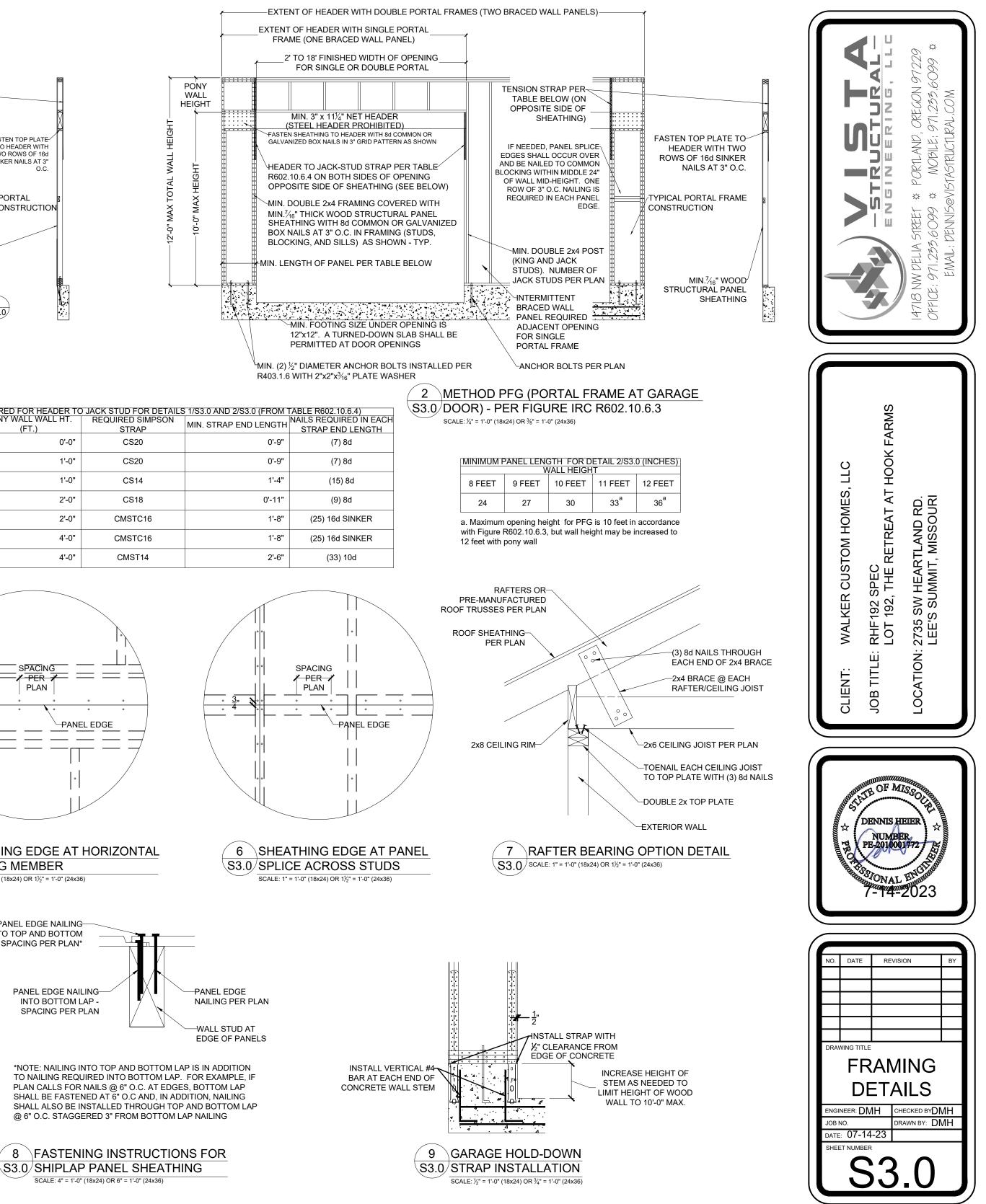


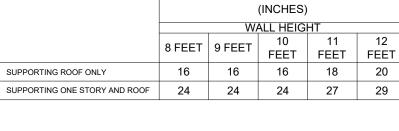
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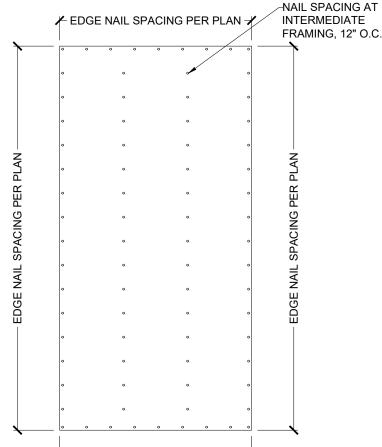






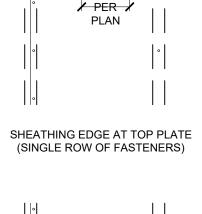




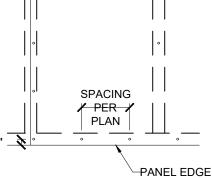


FEDGE NAIL SPACING PER PLAN



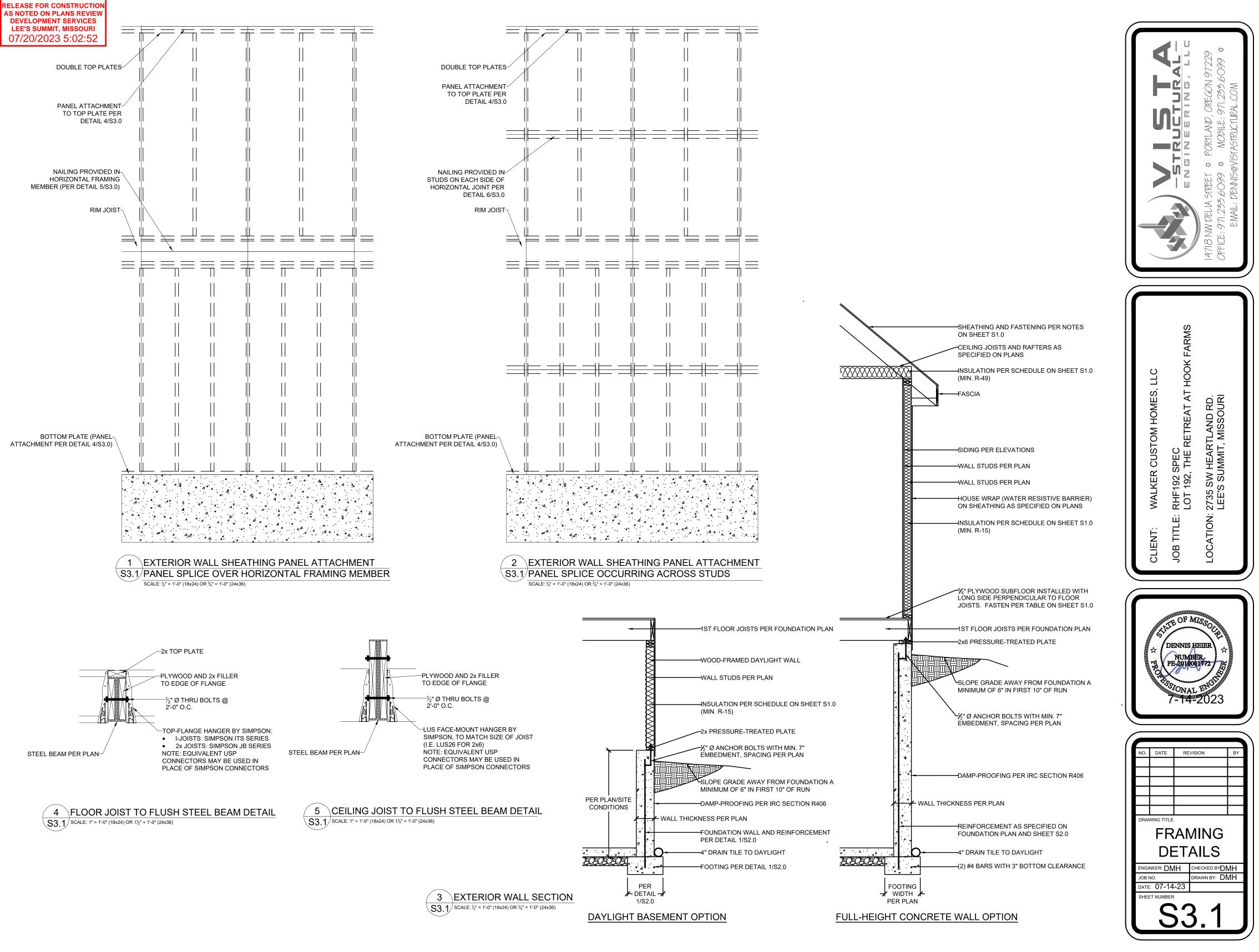


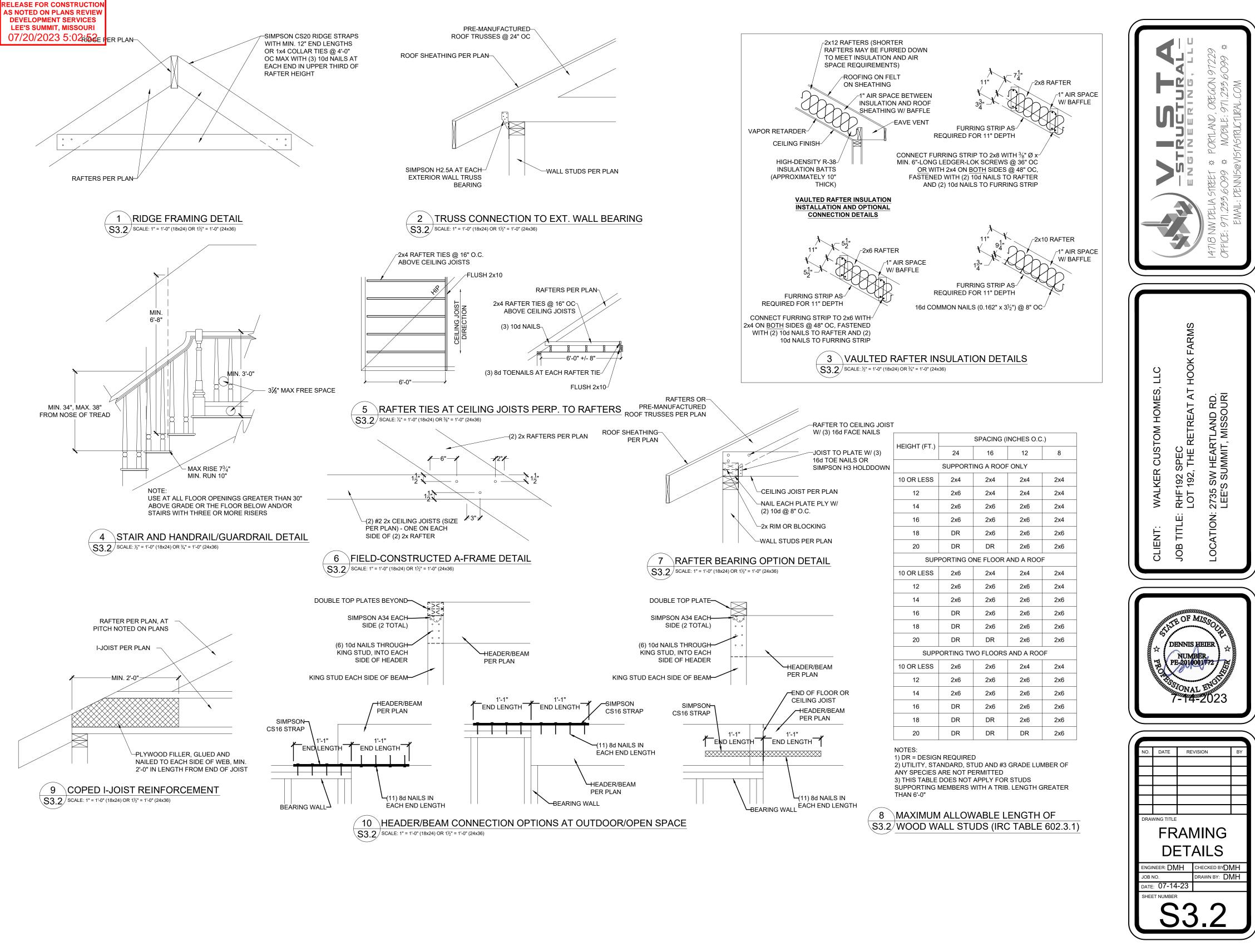
SPACING



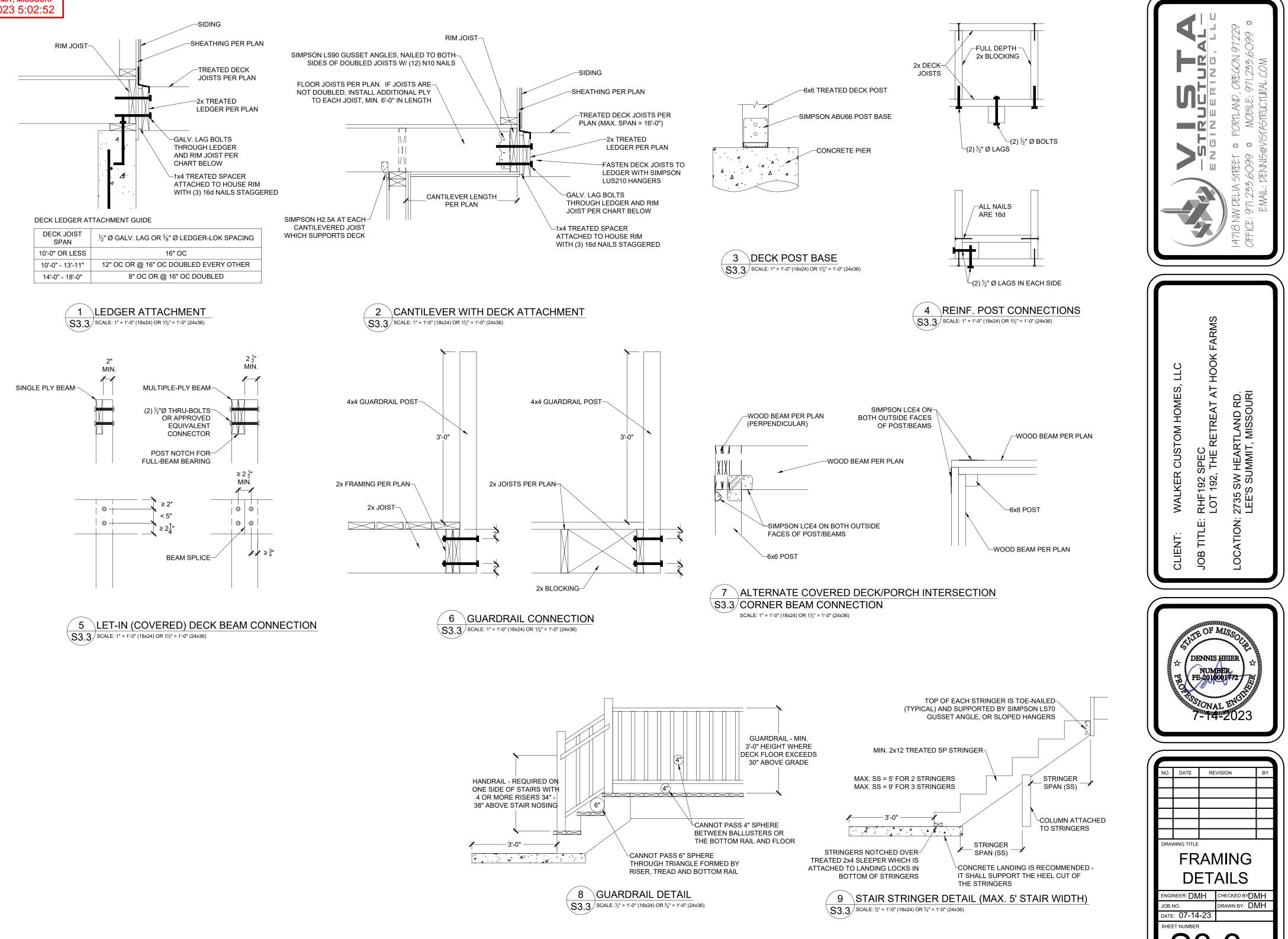
SHEATHING EDGE AT BOTTOM PLATE (SINGLE ROW OF FASTENERS)

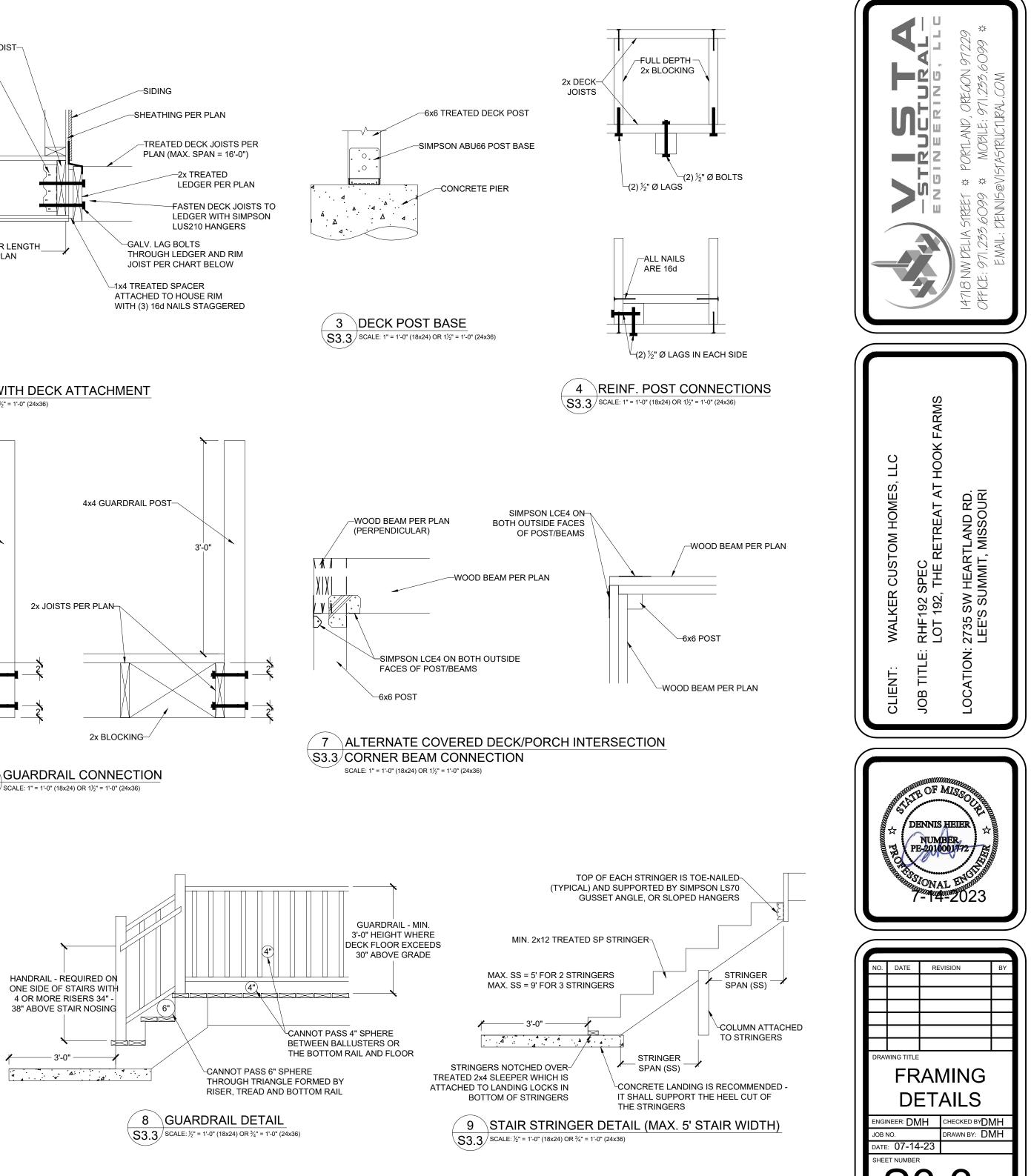






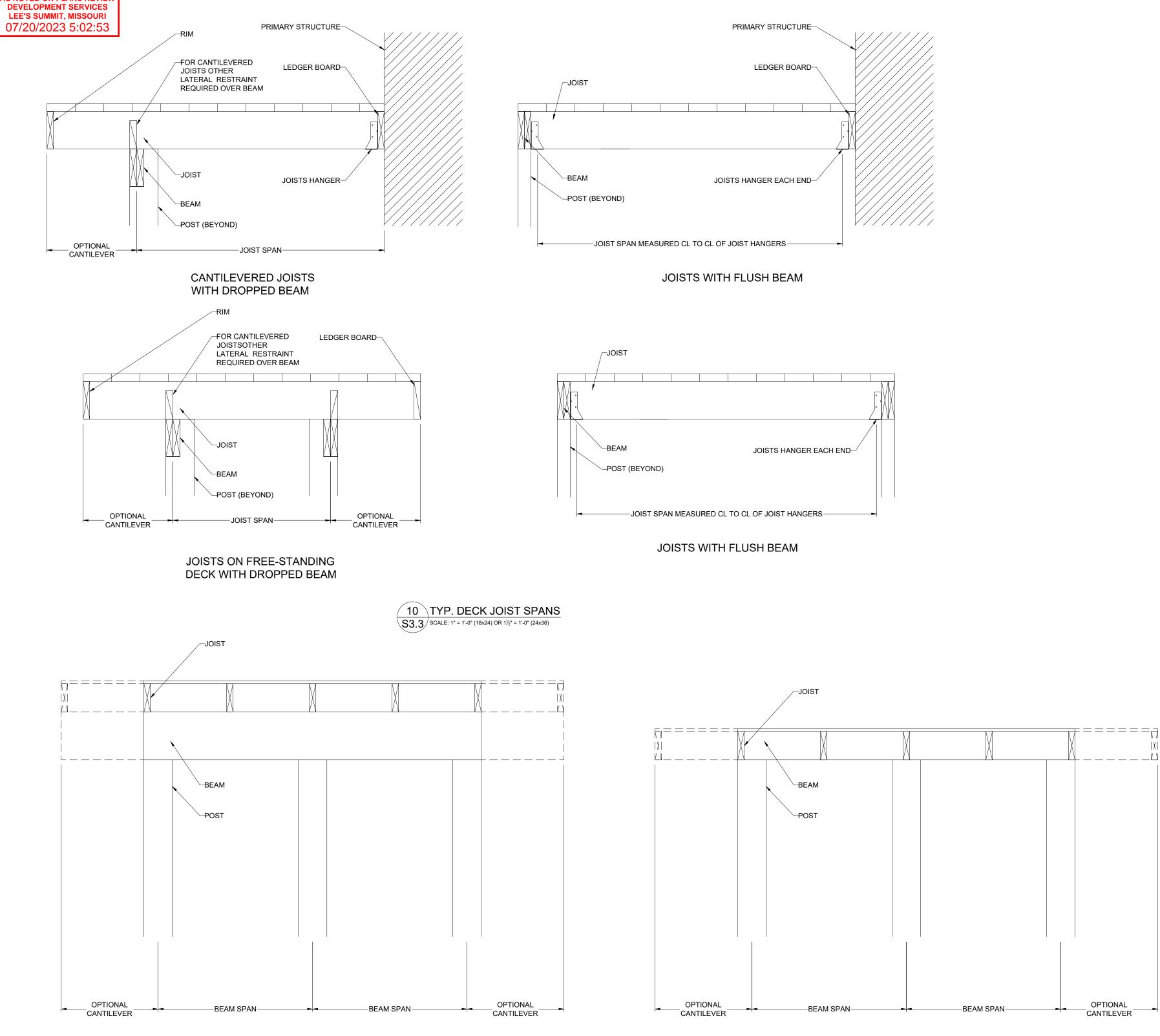
RELEASE FOR CONSTRUCTION **AS NOTED ON PLANS REVIEW** DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 07/20/2023 5:02:52





DROPPED BEAM

RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW



Þ 7229 99 * 6 GON LON • **Ľ** U R H È \neq ٦lu PORTL. \geq **2** | 2 U ¢ ¢ *77 77* Ш FARMS HOOK C Ľ WALKER CUSTOM HOMES, RHF192 SPEC LOT 192, THE RETREAT AT 2735 SW HEARTLAND RD. LEE'S SUMMIT, MISSOURI JOB TITLE: LOCATION: CLIENT: NOTE OF MISS DENNIS HEIER PE-2010001772 7-14-2023 REVISION DATE BY RAWING TITLE FRAMING DETAILS ENGINEER: DMH CHECKED BYDMH DRAWN BY: DMH JOB NO. DATE: 07-14-23 HEET NUMBER S3.3b