**PAINT BOOTH** LOCATION

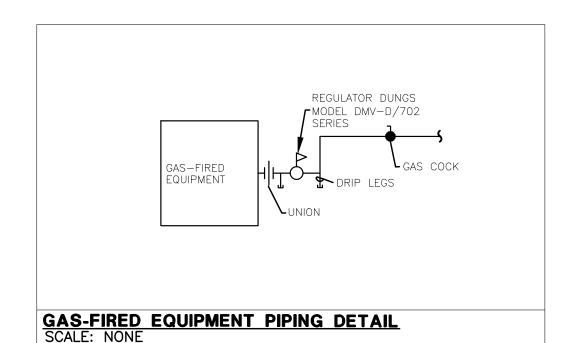
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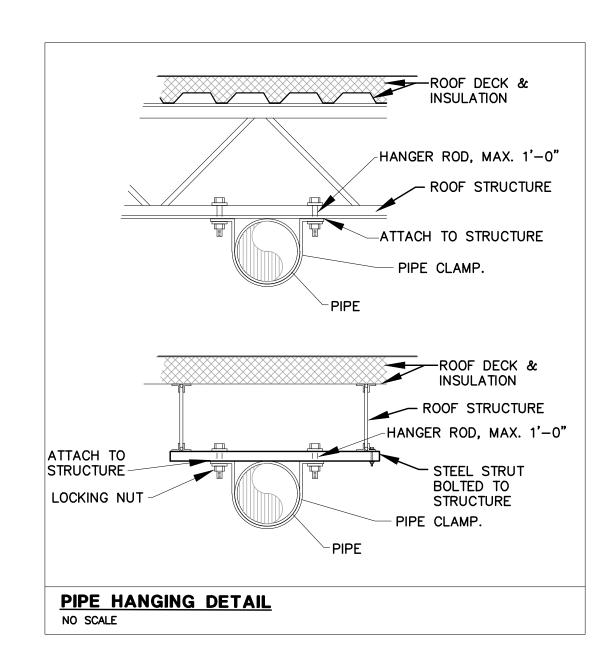
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Scale

EXHAUST MUST BE 10'-0" MININUM FROM AIR INTAKE 6'-0" MIN ABOVĘ ROOF ABOVĘ ROOF EXHAUST DUCT INTAKE DUCT

## EXHAUST/INTAKE DUCTS





## **GENERAL NOTES**

- 1. DOWNDRAFT BOOTH TO BE GLOBAL FINISHING SOLUTIONS.
- 2. PAINT BOOTH AND PRODUCT DOOR MEETS NFPA CODE 33.
- 3. EXHAUST FAN TO BE UL AND CSA CERTIFIED.
- 4. AIR VOLUME OF 4 CHANGES PER MIN @ 12,000 CFM FOR PAINT BOOTH. INTAKE FILTER POCKET STYLE, 95% AVERAGE ARRESTANCE. PLENUM FILTER 20"x48", 99% EFFICIENT AT REMOVING 10 MICRONS OR LARGER. EXHAUST FILER 20"x25", 99.6% AVERAGE ARRESTANCE.
- 5. FIRE SUPPRESSION SYSTEM BY OTHERS.
- 6. CONTRACTOR TO INSTALL NEW AIR SOLENOID VALVE AND ELECTRICALLY INTERLOCK VALVE IN THE PAINT BOOTH.
- 7. ALL INSTALLATION TO BE IN ACCORDANCE WITH LOCAL CODES, IMC,
- 8. VALVE TO PAINT BOOTH BY USE OF 3 PROXIMITY SWITCHES.
- 9. MIXING ROOM AND PRODUCT DOOR MEETS NFPA CODE 33.
- 10. AIR VOLUME OF 1 TO 2 CHANGES PER MIN @ 1150 CFM FOR MIXING ROOM.
- 11. SUPPLY AIR IS FILTERED THROUGH VILIDON 5606.

		SUPPLY	UNIT :	SCHE	DULE					
		EQUIPMENT		O.A.			GAS MB		ELECTRICAL	
MARK	MFR	SERVING	CFM	СҒМ	ESP	HP	IN	OUT	VOLT/PHASE	REMARKS
SF-1	GLOBAL FINISHING SOLUTIONS	PAINT BOOTH	15,205	15,205	1.0	15	1512.6	1323.5	208/3	1,2,3

VERIFY ELECTRICAL VOLTAGE AND PHASE WITH ELECTRICAL CONTRACTOR PRIOR TO ORDERING UNIT.

		EXHAUST FAI	N SCH	IEDU	LE		
	MANUFACTURER	EQUIPMENT SERVING	CFM	SP	HP	VOLTS/PH	REMARKS
EF-1	GLOBAL FINISHING SOLUTIONS	PAINT BOOTH	15,205	.75	7.5	208/3	1,2,3,4,5

## MECHANICAL KEYED NOTES

- $\langle 1 \rangle$  34"ø SUPPLY AIR DUCT UP THROUGH ROOF.
- (2) CONNECT 2" GAS LINE TO EXISTING 2" GAS IN CEILING SPACE. VERIFY EXACT LOCATION IN FIELD.
- 3 34"ø EXHAUST AIR DUCT UP THROUGH ROOF.
- FURNISH AND INSTALL NEW BOOTH MOUNTED EXHAUST FAN (EF-1), 7.5 HP. VERIFY VOLTAGE WITH OWNER REPRESENTATIVE. DUCT AND RAIN CAP TO TERMINATE ABOVE
- 5 ALL AIR INTAKES AND EXHAUSTS NEED TO BE SEPARATED BY A MINIMUM OF 10'-0".
- 6 FURNISH AND INSTALL NEW BOOTH MOUNTED SUPPLY FAN (SF-1), 15 HP. VERIFY VOLTAGE WITH OWNER REPRESENTATIVE. DUCT AND RAIN CAP TO TERMINATE ABOVE ROOF LINE.

PROPERTY LINE 50'-0" FROM BUILDING

30" ULTRA XL 2-ROW PIT

PAINT BOOTH

SEE SHEET M2 FOR MANUFACTURER'S DRAWINGS.

SERVES PAINT BOOTHS.
VERIFY ELECTRICAL VOLTAGE AND PHASE WITH ELECTRICAL CONTRACTOR PRIOR TO ORDERING UNIT. 4. SEE SHEET M2 THROUGH M3 FOR MANUFACTURER'S DRAWINGS.
5. FANS TO BE NON-SPARKING.

> FIRE PROTECTION MUST COMPLY WITH SECTION 2404.4 OF INTERNATIONAL FIRE

CODE. UNDER DEFERRED SUBMITTAL.

AIR DROP

EXISTING MIXING ROOM

STAT 3-STAGE

O AIR DROP

MECHANICAL FLOOR PLAN
SCALE: 1/4" = 1'0"

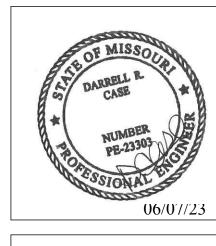
**EXISTING PAINT BOOTH** 

Engineering Inc

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**544 MAE COURT FENTON, MO 63026** 



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CARSTAR OF LEE SUMMIT
PAINT BOOTH
2509 NE INDEPENDENCI

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PAINT BOOTH SPECIFICATIONS

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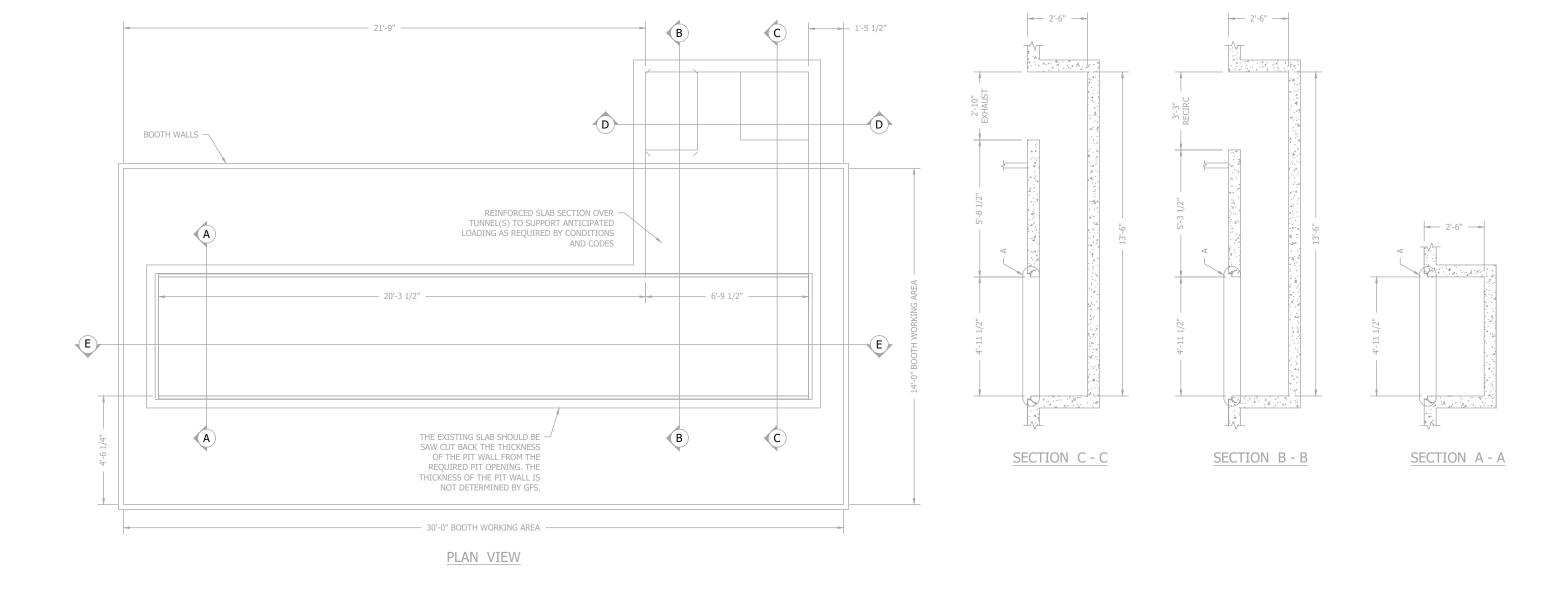
CIR	LIGHTIN				TY	CAPACIT	RCUIT (	NIMUM CI	MOTOR MII					EATER	AIR H			
CINCI	ANTITY OF	CITY	CAPACIT	CIRCUIT	JM CIRC	V /MINIMU	AMP DRA	FULL LOA	R SPECIFICATIONS	MOTOR	TAILET DIDE	TEMP	MAX	MIN INLET	MAX		INTAKE /	MAX AIR
SINGLE INIMUN		/ 3PH	575V 3F	OV 3PH	480V	230V 3PH	208V 3PH	230V 1PH	TAKE, 7.5HP EXHAUST	15HP INTA	INLET PIPE : SIZE NPT	RISE		PRESS. AT MAX		FUEL	EXHAUST	
APACIT		F									(IN)	(°F)		FIRING RATE	RATE	TULL	MOTOR	RATE
	(4-TUBE,										(===)		(PSI)	(INWC)	(BTU/HR)		(HP)	(CFM)
1 20\ /	TUBE, STD						73.4 /		ADVANCE CURE	NO A		0.1		12.0	1512605	NATURAL		
120V	OR LED)						92.0		ADVANCE CURE	NO A	1 1/4	91	5.0	13.0	1512605	GAS	15 / 7.5	15205
45	1.0										1 1/7	90	5.0	ГО	1222520	DDODANE	13 / 7.3	13203
45	18											80		5.0	1323529	PROPANE		

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**PAINT BOOTH SPECIFICATIONS** 

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SECTION D - D

1 3/4" X 1 3/4" X 1/4" ANGLE IRON WITH ANCHORS (TYP FOR DETAIL VIEW B)

- SEE FOUNDATION NOTE 4 -

5'-3" (OUTSIDE-TO-OUTSIDE OF ANGLE IRON) 5'-2 1/2" (INSIDE-TO-INSIDE OF ANGLE IRON) 4'-11 1/2" (BETWEEN PIT SIDE WALLS)

1 3/4" X 1 3/4" X 1/4" ANGLE IRON W/ ANCHORS (1 1/2" X 1 1/2" INSIDE) - SEE FOUNDATION NOTE 4 -

DETAIL A

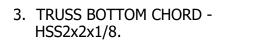
SECTION E - E

BAR SIZE (in)  $1 - \frac{1}{2} \times \frac{3}{16}$ LIGHT DUTY MAX WHEEL LOAD (lbs) 1,850 W SERIES 19-W-4 \* BASED ON A CONTACT LENGTH OF 9" (IE 11 BARS/FT OF GRATING WIDTH) AND A MAXIMUM CLEAR SPAN OF 2'-4 1/2"



DETAIL B

- 1. CONTRACTOR SHALL VERIFY THAT THERE ARE NO INTERFERENCES BETWEEN EXISTING FOUNDATION (FTG. PADS, CON. FTGS, GRADE BEAM, TIES, ETC) AND PROPOSED PIT FOUNDATION.
- 2. DO NOT PLACE BACKFILL AGAINST WALL UNTIL THE WALL HAS BEEN ADEQUATELY SHORED.
- 3. WALL LOCATIONS TO BE WITHIN 1/4" OF DIMENSIONS SHOWN.
- 4. ALL ANGLE IRON HAS BEEN SHOWN AS A REFERENCE, SHOULD BE IN THE SCOPE OF THE FOUNDATION DESIGN, AND PROVIDED BY OTHERS. ADJUSTMENTS MUST BE MADE FOR ANGLE THICKNESS THAT VARY FROM  $\frac{1}{4}$ " AS SHOWN IN DETAIL A.
- 5. THE DESIGN OF THE PIT GRATING AND ITS CAPACITY HAS BEEN PROVIDED IN A TABLE. DO NOT EXCEED THE WHEEL LOAD CAPACITY OF THE GRATING AS PROVIDED BY GFS. WHEELED VEHICLES WITH URETHANE TIRES SHOULD NEVER BE USED.
- 6. GRATINGS MUST BE INSTALLED WITH CROSS BARS ON TOP SIDE.
- 7. NOTCHING OF BEARING BARS AT SUPPORTS TO MAINTAIN PROPER ELEVATION IS GENERALLY NOT RECOMMENDED. IF NOTCHING IS REQUIRED FOR INSTALLATION, MANUFACTURER SHOULD BE CONSULTED.
- 8. METAL SHOULD ALWAYS BE USED FOR ALL GRATING SUPPORTS.
- 9. A MINIMUM OF 1" BEARING SHALL BE PROVIDED FOR ALUMINUM AND LIGHT DUTY STEEL GRATING. FOR HEAVY DUTY STEEL GRATING, 1" MINIMUM BEARING SHALL BE PROVIDED FOR BEARING BAR DEPTHS UP TO 2-1/4", AND 2" MINIMUM BEARING SHALL BE PROVIDED FOR DEPTHS OF 2-1/2" AND OVER. THIS BEARING SURFACE DOES NOT INCLUDE THE SUPPORT ANGLE FILLET RADIUS.
- 10. ALL DIMENSIONS ARE APPROXIMATE AND SUBJECT TO CHANGE. CUSTOMER MUST CHECK EQUIPMENT SIZE, LOCATION IN BUILDING AND ALL CLEARANCES TO BUILDING AND CONTENTS.
- 11. DEPTH DIMENSIONS ARE BASED ON HAVING A 6" SLAB OVER THE EXHAUST TUNNEL. IF STRUCTURAL ANALYSIS INDICATES THAT A THICKER SLAB IS REQUIRED, PIT DEPTH SHOULD BE INCREASED ACCORDINGLY AND GFS NOTIFIED SO TALLER PIT RAILS CAN BE PROVIDED.



- 4. VERTICAL TUBE WEB MEMBER -HSS2x2x1/4.
- 5. 3/8" STEEL WEB CUT OUT PLATE.
- 6. SYMMETRICAL ABOUT CENTERLINE.



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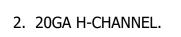
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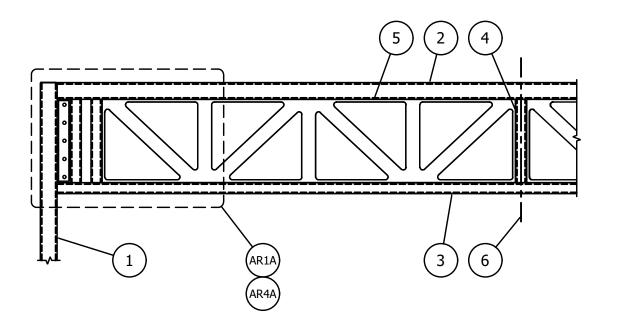
**PAINT BOOTH SPECIFICATIONS** 

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**M4** 

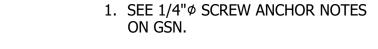
1. DUAL SKIN INSULATED PANEL.





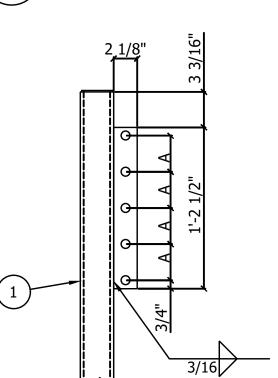
YPANEL TO PANEL CONNECTION

 $\frac{\text{SCALE}}{3" = 1'-0"}$ 



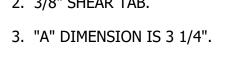
- 2. EXIST SLAB ON GRADE. 4" MIN CONC THICKNESS. VERIFICATION OF SLAB NOT BY GFS.
- 3. #10 x 3/4" TEK SCREW.SEE PANEL BOTTOM ELEVATION FOR SPACING INFORMATION.
- 4. DUAL SKIN INSULATED PANEL.
- 5. 1/2" MIN 3/4" MAX
- 6. 0" 9" PANEL WIDTH.
- 7. 10" 18" PANEL WIDTH.
- 8. 19" 36" PANEL WIDTH.



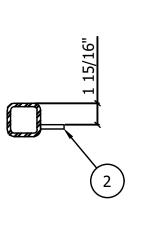


COLUMN SHEAR TAB

ELEVATION FOR SIZE. 2. 3/8" SHEAR TAB.



1. STEEL COLUMN - SEE FRAME



SECTION A-A

1. FRAME TOP CHORD - SEE FRAME ELEVATION FOR SIZE.

2. FRAME BOTTOM CHORD - SEE FRAME ELEVATION FOR SIZE.

3. VERTICAL WEB MEMBER -HSS2x2x1/4.

4. 3/8" PLATE WITH (5) 1/2"\$\phi\$ A325 BOLTS.

5. FRAME WEB PLATE - SEE FRAME ELEVATION FOR THICKNESS.

- 6. 2" LONG HORIZONTALLY FROM FRAME WEB PLATE TO CHORD AND 2" VERTICALLY FROM FRAME WEB PLATE TO VERTICAL WEB MEMBER CONTINUOUS THROUGH CORNER.
- 7. SEE BOTTOM CHORD ATTACHMENT DETAIL (AR2C), IF OCCURS, FOR CONNECTION INFORMATION.
- 8. SEE COLUMN SHEAR TAB DETAIL (AR4A) FOR COLUMN AND SHEAR TAB INFORMATION.

1. FRAME BOTTOM CHORD.

2. FRAME WEB PLATE.

3. 14GA "J"

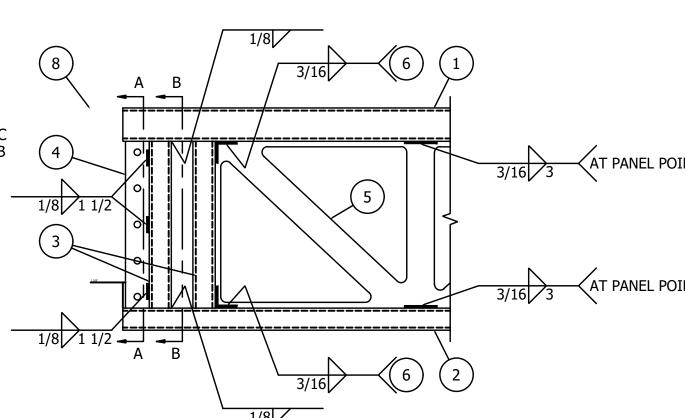
4. 14GA "L"





1. STEEL HSS COLUMN PER FRAME ELEVATION (AR8A).

- 2. 3/8" BASE PLATE.
- 3. SEE 3/8"Ø WEDGE ANCHOR NOTES ON GSN.
- 4. EXIST SLAB ON GRADE 4" MIN CONC THICKNESS. VERIFICATION OF SLAB NOT BY GFS.

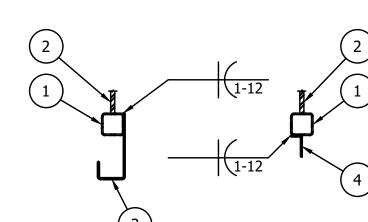


SECTION B-B

CORNER COLUMN

1. SINGLE SKIN PANEL.

2. 8MM CLASS 8.8 BOLTS AT 6" O.C. TYP.



TRUSS CONNECTION TO COLUMN

<u>SCALE</u> <u>3" = 1'-0"</u>

PANEL TO PANEL CONNECTION

BOTTOM CHORD ATTACHMENT DETAIL

PANEL BOTTOM ELEVATIONS

BASE CHANNEL

<u>SCALE</u> <u>3" = 1'-0"</u>

## - DESIGN INFORMATION:

**BOOTH AND EQUIPMENT HAS BEEN DESIGNED BASED ON THE CURRENT EDITION OF THE INTERNATIONAL BUILDING CODE** 

**RISK CATEGORY: II SEISMIC IMPORTANCE FACTOR: IE=1** MAPPED SPECTRAL RESPONSE ACCELERATION:  $S_s = 0.099$  $S_1 = 0.068$ **SITE CLASS: D (ASSUMED) SPECTRAL RESPONSE COEFFICIENT:**  $S_{DS} = 0.106$  $S_{D1} = 0.109$ **SEISMIC DESIGN CATEGORY: B SEISMIC-FORCE-RESISTING SYSTEMS:** STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMN SYSTEMS **RESPONSE MODIFICATION FACTOR: R=3.00** SEISMIC RESPONSE COEFFICIENT: C<sub>S</sub>=0.036 ALL OTHER SELF-SUPPORTING STRUCTURES, TANKS, OR VESSELS **NOT COVERED ELSEWHERE IN TABLE 15.4-2 RESPONSE MODIFICATION FACTOR: R=1.25** SEISMIC RESPONSE COEFFICIENT: C<sub>s</sub>=0.085 **ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE PROCEDURE BASIC WIND SPEED: N/A MPH** (PORTIONS OF EQUIPMENT THAT ARE OUTDOOR ONLY - IE

**STACKS AND STANDS) BUILDING CATEGORY: INDOOR EXPOSURE: C DEAD LOADS: SELF-WEIGHT OF STRUCTURAL STEEL** IN ADDITION TO THE LOADS LISTED BELOW **5.0 PSF (ROOF)** 

5.0 PSF (WALLS) 3.5 PSF (PLENUM)

**BOOTH ROOF LIVE LOADS: N/A PSF LIVE LOADS: 300 LBS AT MIDPOINT OF FRAME BEAM GROUND SNOW LOAD: N/A PSF** 

FDN INFORMATION:

CAPACITY OF THE FDN/SLAB TO SUPPORT GFS BOOTHS AND **EQUIPMENT IS NOT THE RESPONSIBILITY OF GFS.** 

ANCHORS INDICATED ARE BASED ON ASSUMPTIONS OF EXIST CONDITIONS (LISTED BELOW). THESE ASSUMPTIONS ARE MADE IN ORDER FOR GFS TO PROVIDE ANCHOR BOLT HOLES IN THE BASE PLATES AND PANELS. EXIST CONDITIONS SHOULD BE VERIFIED BY THE OWNER AND ANY DEVIATIONS SHOULD BE CONVEYED TO GFS PRIOR TO FABRICATION.

1/4" φ SCREW ANCHOR - 1/4" φ POWERS (DEWALT) SCREW-BOLT+ SCREW ANCHORS EMBEDDED 1 15/16" PER ICC ESR-3889 TO SECURE PANELS TO CONC. IN LIEU OF THE POWERS (DEWALT) ANCHOR,  $1/4^{\circ}$ HILTI KWIK HUS-EZ SCREW ANCHORS EMBEDDED 1 15/16" PER ICC ESR-3027 MAY BE USED. EACH WALL/BAY IS REQUIRED TO HAVE ANCHORS AT 18" O.C. MAX, U.N.O. EACH WALL SHALL HAVE (1) ANCHOR 3" MAX FROM END OR CORNER AND A MIN OF (2) ANCHOR PER WALL/BAY. INSTALL ANCHORS PER MFR'S RECOMMENDATION. SEE DETAILS FOR ADDITIONAL INFORMATION. A PREAPPROVED ANCHOR WITH A CAPACITY EQUAL TO OR GREATER THAN THE SPECIFIED ANCHOR AND WITH A CURRENT ICC REPORT MAY BE USED IN LIEU OF THE ANCHOR SPECIFIED. ALL OTHER RESTRICTIONS (INCLUDING BUT NOT LIMITED TO EDGE DISTANCE AND EMBEDMENT) SHALL BE CONSIDERED.

3/8"φ WEDGE ANCHOR - 3/8"φ POWERS (DEWALT) POWER-STUD+ SD1 WEDGE ANCHORS EMBEDDED 2" MIN PER ICC ESR-2818. IN LIEU OF THE POWERS (DEWALT) ANCHOR, 3/8" ## HILTI KWIK BOLT TZ2 WEDGE ANCHORS EMBEDDED 2" MIN PER ICC ESR-4266 MAY BE USED. SEE DETAILS FOR NUMBER OF ANCHORS REQUIRED AND ADDITIONAL INFORMATION. INSTALL ANCHORS PER MFR'S RECOMMENDATION. A PREAPPROVED ANCHOR WITH A CAPACITY EQUAL TO OR GREATER THAN THE SPECIFIED ANCHOR AND WITH A CURRENT ICC REPORT MAY BE USED IN LIEU OF THE ANCHOR SPECIFIED. ALL OTHER RESTRICTIONS (INCLUDING BUT NOT LIMITED TO EDGE DISTANCE AND EMBEDMENT) SHALL BE CONSIDERED.

ANCHOR SPECIFICATION IS BASED ON THE FOLLOWING ASSUMPTIONS **OF EXIST CONDITIONS:** 

- MIN CONC COMPRESSIVE STRENGTH IS 2500 PSI.
- MIN SLAB DEPTH IS 4".
- MIN SLAB DEPTH FOR PAINT KITCHEN IS 6".
- **COLD-FORMED STEEL:**

ALL COLD-FORMED STEEL MEETS THE REQUIREMENTS OF THE LATEST EDITION OF THE AISI SPECIFICATION FOR THE DESIGN OF **COLD-FORMED STEEL STRUCTURAL MEMBERS. ALL COLD-FORMED** STEEL IS COMMERCIAL GRADE WITH A YIELD STRENGTH OF 32KSI AND A TENSILE STRENGTH OF 40KSI. 304 AND 316 STAINLESS STEEL PER **ASTM A240 HAS A YIELD STRENGTH OF 25KSI AND A TENSILE** STRENGTH OF 70KSI.

STRUCTURAL STEEL:

ALL STRUCTURAL STEEL FABRICATION AND CONSTRUCTION COMPLY WITH THE LATEST AISC HANDBOOKS AND CODES. **ALL STEEL IS ASTM A36, EXCEPT AS FOLLOWS:** 

- -- WIDE FLANGE SECTIONS ASTM A992,
- **PIPE SECTIONS ASTM A53 GRADE B,**
- **HSS SECTIONS ASTM A500 GRADE B BOLTS ARE A325-N AND SHALL BE SNUG-TIGHTENED.**
- **WELDING:**

WELDERS HOLD CURRENT VALID CERTIFICATES AND HAVE CURRENT **EXPERIENCE IN TYPE OF WELD CALLED FOR. STRUCTURAL STEEL** WELDING WITH LOW HYDROGEN TYPE, E70 AND E60 FOR LIGHT GAUGE STEEL. STRUCTURAL STEEL WELDING CONFORMS TO THE "STRUCTURAL WELDING CODES-STEEL" AWS D1.1, CURRENT EDITION. **ROOF ACCESS RESTRICTIONS:** 

THE ROOFS OF GFS EQUIPMENT ARE NOT DESIGNED OR INTENDED TO BE WALKED UPON OR TO SUPPORT WEIGHT OF ANY KIND. AS DESIGNED AND MANUFACTURED, THE EQUIPMENT ROOFS DO NOT MEET THE MINIMUM REQUIREMENTS OF A SAFE WALKING AND/OR **WORKING SURFACE UNDER OSHA 1910.22. UNDER NO** CIRCUMSTANCES SHOULD THE ROOF BE USED BY MAINTENANCE PERSONNEL OR OTHERS FOR WALKING, STANDING, OR STORAGE OF ANY KIND.

WHEN NECESSARY, ROOF ACCESS SHOULD BE SECURED THROUGH THE **USE OF A PROPERLY SUPPORTED PLATFORM THAT SATISFIES THE** MINIMUM LOAD REQUIREMENTS SPECIFIED BY ASCE 7 (MINIMUM **DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND** OTHER STRUCTURES) AND ASCE 37 (DESIGN LOADS ON STRUCTURES **DURING CONSTRUCTION).** 

ADDITIONALLY, PERSONNEL SHOULD ALWAYS UTILIZE APPROPRIATE FALL SAFETY PROTOCOLS WHEN USING AN ELEVATED PLATFORM. USE OF THE ROOF IN A CONTRARY MANNER MAY RESULT IN INJURY AND/OR DEATH.

**SPECIAL INSPECTION INFORMATION:** 

SPECIAL INSPECTION SHALL BE REQUIRED FOR THE FOLLOWING TYPES OF WORK AND SHALL BE IN COMPLIANCE WITH IBC SECTION

- 1. POST-INSTALLED ANCHORS INTO HARDENED CONCRETE.
- 2. HIGH STRENGTH BOLTING.
- 3. FIELD WELDING.
- 4. STRUCTURAL STEEL IN THE SEISMIC FORCE-RESISTING **SYSTEMS**

**STATEMENT OF SPECIAL INSPECTION:** 

- A. THIS STATEMENT OF SPECIAL INSPECTIONS SHALL BE **SUBMITTED IN ACCORDANCE WITH SECTION 1704.3 OF THE**
- B. THIS STATEMENT SHALL INCLUDE A SCHEDULE OF SPECIAL INSPECTION SERVICES APPLICABLE TO THIS PROJECT.

THE SPECIAL INSPECTOR(S) SHALL KEEP RECORDS OF ALL INSPECTIONS AND SHALL FURNISH INTERIM INSPECTION REPORTS TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE ON A BI-WEEKLY BASIS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO COMPLETION OF THAT PHASE OF WORK. A FINAL REPORT OF SPECIAL INSPECTIONS DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTIONS OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AND REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AT THE CONCLUSION OF THE PROJECT.

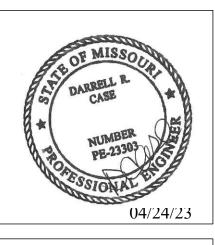
THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO COMPLY WITH THE CONTRACT DOCUMENTS. JOBSITE SAFETY AND MEANS AND METHOD OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

- OUTDOOR USE:

THE EQUIPMENT IN THESE DOCUMENTS ARE NOT DESIGNED FOR OUTDOOR USE. IN THE EVENT THAT SUCH APPLICATIONS AND/OR **USES ARE CONTEMPLATED (IE ANY PORTION OF THE EQUIPMENT IS EXPOSED TO THE ELEMENTS, NOT INCLUDING STACKS), THE** PURCHASER OF THE EQUIPMENT IS RESPONSIBLE FOR NOTIFYING GFS SO THAT ADDITIONAL STRUCTURAL ANALYSIS CAN BE PERFORMED AND THE NECESSARY MODIFICATIONS CAN BE MADE.

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**ABBREVIATIONS:** 

A.F.G. **ABOVE FINISH GRADE** AIR MAKE-UP UNIT AMU BLDG BUILDING CONC CONCRETE **ESOW** EACH SIDE OF WEB **EXIST** EXISTING FDN **FOUNDATION** GAUGE GR5 **GRADE 5** INTERNATIONAL BUILDING CODE POUNDS MAX **MAXIMUM** MFR MANUFACTURE MIN MINIMUM NS/FS NEAR SIDE AND FAR SIDE O.C. ON CENTER OSHA OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION

PLF POUNDS PER LINEAR FOOT **POUNDS PER SQUARE FOOT** PSF T/B TOP AND BOTTOM

**UNLESS NOTED OTHERWISE** U.N.O. OR UNO

WIDE FLANGE

**TYPICAL**