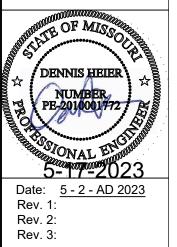


Site Description:

2, Summit View Farn
Street Address:
2311 SW Serena F

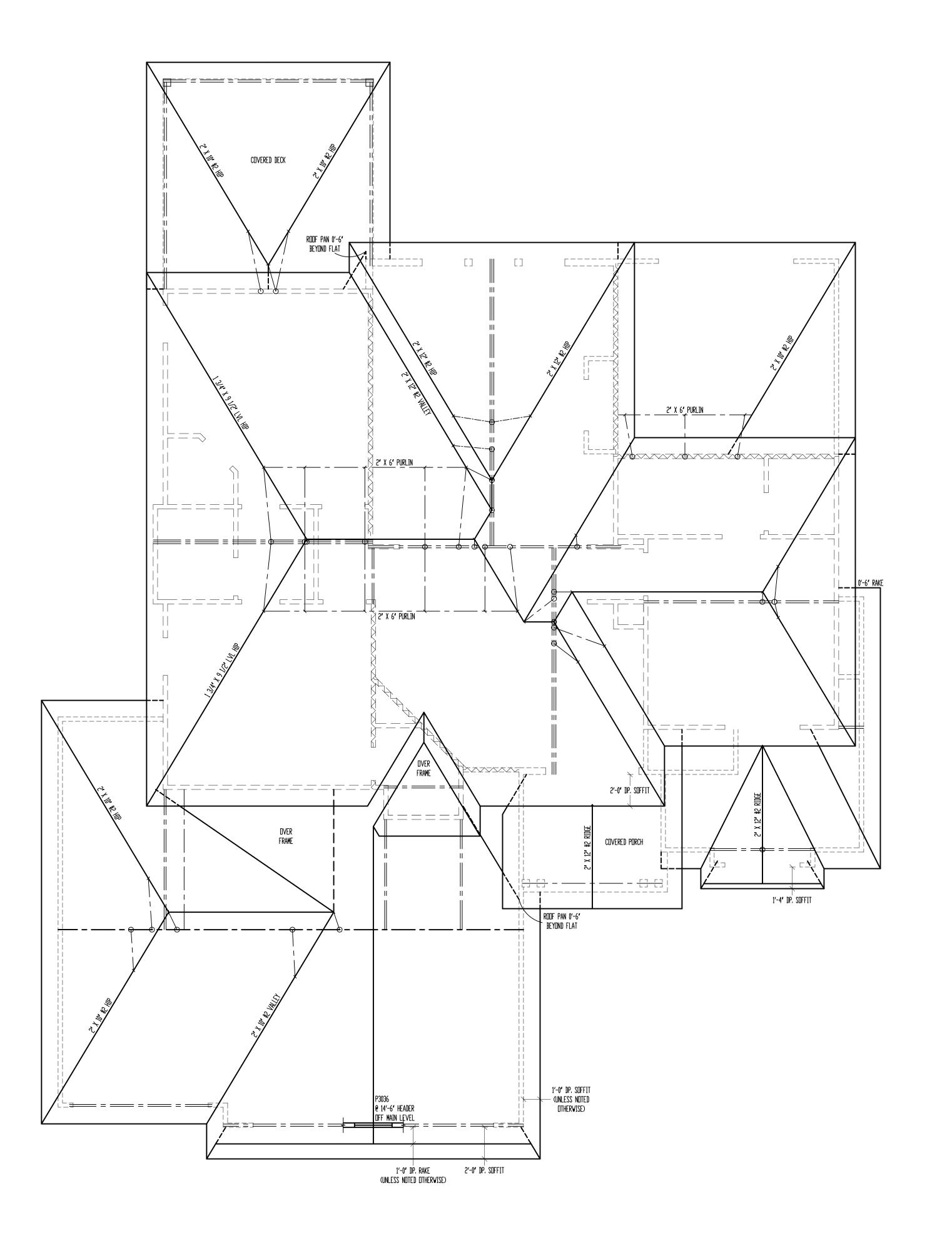
SVF082 Spec
General Contractor:

Iker Custom Homes, L



Sheet Title: **ELEVATIONS** 





SCALE: 1/4" = 1'-0"

\*ALL RAFTERS SHALL BE 2' X 6' #2 @ 16' D.C., UNLESS NOTED OTHERWISE. SEE DETAIL 7/S3.2 FOR ALTERNATE RAFTER BEARING DETAIL WHEN RAFTERS ARE REQUIRED TO BEAR HIGHER THAN THE WALL DOUBLE TOP PLATE.

FLASHING NOTE: DRIP EDGE, VALLEYS AND FLASHINGS TO BE METAL CLAD.

ROOF NOTES: ROOF DESIGNED FOR LIGHT ROOF COVERING 30psf TOTAL LOAD [10psf DL, 20psf LL (SL)] \* RAFTERS (HEM-FIR, DDUG-FIR, DR EQUAL): SEE SPAN CHARTS BELOW

	CODE MININ	1UM		_
	RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN	
	#2-2x6	@24 <b>′</b> □.C.	11'-7 <b>"</b>	
<b>〉</b> 〉	#2-2x6	<b>0</b> 16 <b>″</b> □.C.	14'-2 <b>'</b>	<b>\</b> (((
	#2-2x8	@24 <b>′</b> □.C.	14'-8 <b>'</b>	
	#2-2x8	<b>0</b> 16″ □.C.	17'-11 <b>'</b>	
	#2-2x10	@24 <b>′</b> □.C.	17′-10 <b>″</b>	
	#2-2x10	<b>0</b> 16″ □.C.	21′-11 <b>′</b>	
	NOTE: CODE	MINIMUM ALL	DWS FOR A RAFTER DEFLECTION	OF L/180 TOTAL LOAD

HIGHER PERFORMANCE (RECOMMENDED)							
RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN					
#2-2x6	@24″ □.C.	8'-6 <b>'</b>					
#2-2x6	<b>@</b> 16 <b>″</b> □.C.	9'-9 <b>'</b>					
#2-2x8	@24″ □.C.	11'-3 <b>'</b>					
#2-2x8	<b>€</b> 16 <b>″</b> □.C.	12'-9 <b>'</b>					
#2-2x10	@24″ □.C.	14'-3 <b>'</b>					
#2-2x10	<b>€16″</b> □.C.	16′-3 <b>′</b>					

DEFLECTION = L/360 LIVE LOAD, L/240 TOTAL LOAD \* VAULTS TO BE 2x10 DEPTH
\* RIDGE BOARDS ARE: (UNLESS OTHERWISE NOTED)

- #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 OVER 10/12 PITCH \* ALL HIPS & VALLEYS ARE: (UNLESS OTHERWISE NOTED) - #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 OVER 10/12 PITCH \* PURLINS ARE 2X6 MIN.

- PURLIN STRUTS ARE AT 4'-0" D.C. - PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 Degree angle vith the Horizontal - ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED LENGTH OF 8'-0' - PURLINS STRUTS SHALL BE CONSTRUCTED IN A "T" CONFIGURATION AND PER THE FOLLOWING CHART:

PURLIN STRUT	MAX PURLIN STRUT LENGTH
(2) 2x4	8′-0 <b>″</b>
(1) 2x4 & (1) 2x6	12'-0 <b>'</b>
(1) 2x6 & (1) 2x8	20'-0 <b>"</b>
(2) 2x6 & (1) 2x8	30'-0"
CONSULT ARCH,/ENGR. >	30'-0 <b>"</b>

\* RIDGE BRACES ARE SAME AS PURLIN BRACES-SPACING, SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE) \* HIP & VALLEY BRACES ARE SAME AS PURLIN SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE)

\* VERTICAL BRACE IF DOT IS UNDER HIP OR VALLEY \* SLASH IS TOP END OF BRACE ( / ), DOT IS BOTTOM OF BRACE ( o ). \* ~ DENUTES BEARING WALL \*---- DENOTES ROOF BRACE 

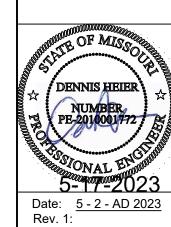
Site Description:

2, Summit View Farn Street Address:

2311 SW Serena I Lee's Summit, Miss

SVF082 Spec
General Contractor:

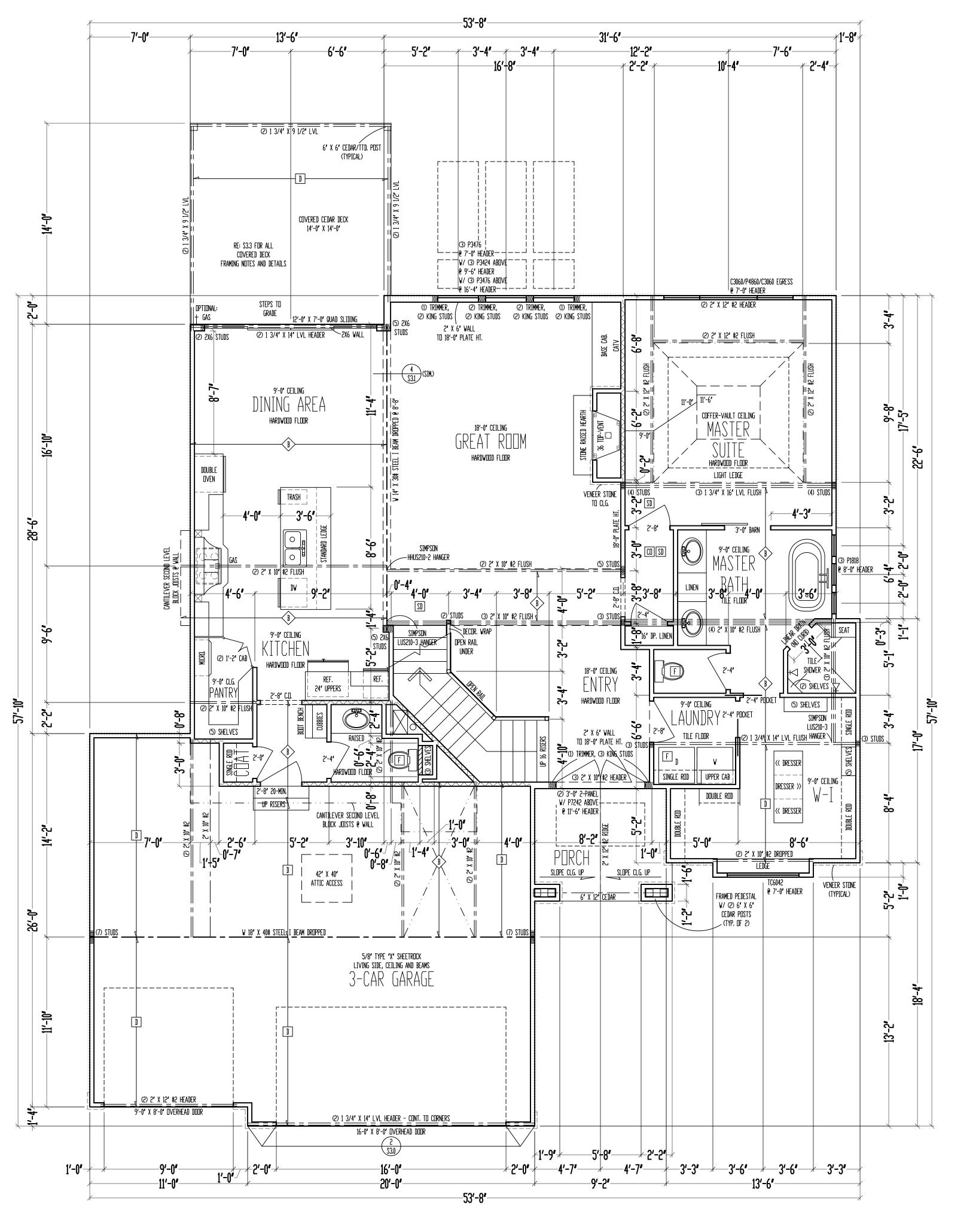
Iker Custom Homes, L



Rev. 2: Rev. 3:

> Sheet Title: **ROOF PLAN**

> > Sheet No.:



9'-0" CEILING
2" X 10" FLOOR SYSTEM ABOVE

MAIN LEVEL

SCALE: 1/4" = 1'-0"

MAIN LEVEL: 1673 SQ. FT. <u>SECOND LEVEL: 827 SQ. FT.</u> TOTAL: 2500 SQ. FT.

> GARAGE: 751 SQ. FT. COV. DUT/LIV: 195 SQ. FT. UNFIN. BASEMENT: 1426 SQ. FT.

CULATIONS ON SHEET S1.1.

3. /\/\/\/\/\/\/\ = LOAD BEARING INTERIOR WALL.
4. (2) 2' X 10' #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.
5. LOW TIES @ 4'-0' D.C. (TYPICAL)

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS.
7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS).
8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS.
9. ALL DESIGNATED 2" X 6" WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS.

10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE.
11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2′ X 4′S @ 16′ D.C., UNLESS NOTED OTHERWISE.
12. EXTERIOR WALL BOTTOM PLATES SHALL BE NATUED TO FRAMING BELOW WITH 16

12. EXTERIOR WALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW WITH 16d COMMON NAILS @ 8' D.C. MAX. (WHERE APPLICABLE.)

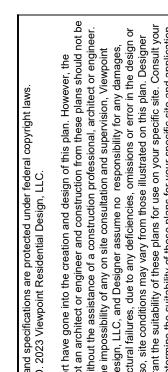
13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS:

(2) 1 3/4' LVL PLIES = 3 1/2' GLULAM

(3) 1 3/4' LVL PLIES = 5 1/2' GLULAM

14. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY OPENINGS,

	JOIST SCHEDULE
$\bigcirc$ B $>$	2" X 10" #2 FLOOR JOIST @ 16" D.C.
D	2" X 6" #2 CEILING JOIST @ 16" N.C.



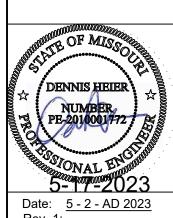
"For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, ut have everlasting life"

VIEWPOINT RESIDENTIAL DESIGN LLC

Site Description:

Lot 82, Summit View Farms 4th
Street Address:
2311 SW Serena PI.,
Lee's Summit, Missouri

SVF082 Spec
General Contractor:
Walker Custom Homes, LI

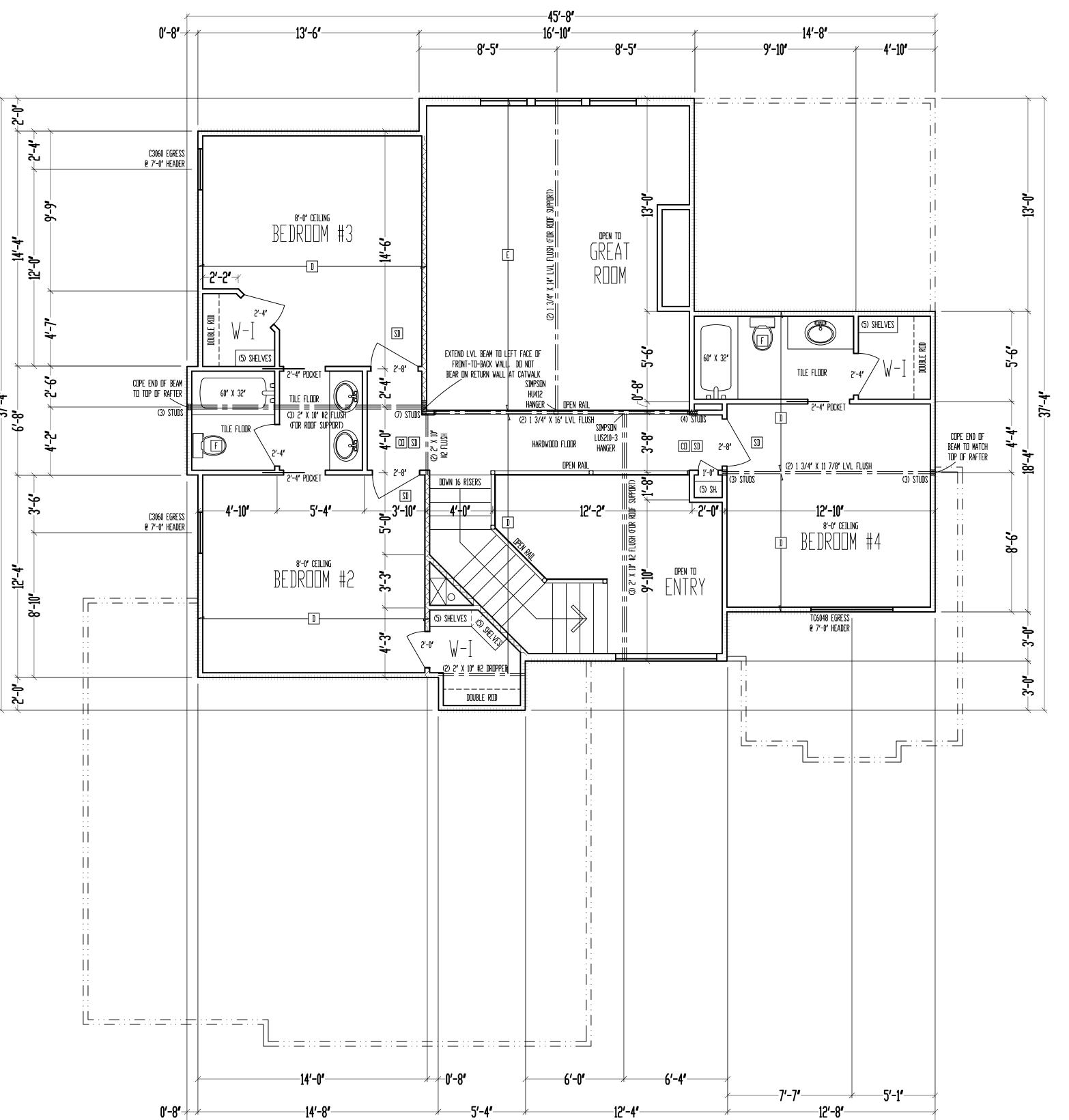


Date: <u>5 - 2</u> Rev. 1: Rev. 2: Rev. 3:

Sheet Title:
MAIN LEVEL
PLAN

Sheet No.:

A-3
of 8



## 8'-0" CEILING SCALE: 1/4" = 1'-0"

++++++ = Wall bracing per framing note #1 and per calculations

FRAMING NOTES

1. SECOND LEVEL EXTERIOR WALLS SHALL BE SHEATHED W/ 7/16' D.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS.

2. \ \ \ \ \ \ \ \ = G.B.: 1/2" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX

FASTENED W/ NO. 6 - 1 1/4" TYPE W OR S DRYWALL SCREWS @ 7" D.C. EDGES & FIELD. (MIN. 8"-0" SECTIONS ONE SIDE OF WALL (OR) MIN. 4"-0" SECTION FOR BOTH SIDES)

3. /\/\/\/\/\/\/\ = LOAD BEARING INTERIOR WALL.
4. (2) 2' X 10' #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED

DIHERWISE.

5. LOW TIES @ 4'-0' D.C. (TYPICAL)

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS.

7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS).

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. ALL DESIGNATED 2' X 6' WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW

10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERVISE. 11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2′ X 4′S @ 16′ D.C., UNLESS NOTED

12. EXTERIOR WALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW WITH 16d COMMON NAILS @ 16' D.C. MAX. (WHERE APPLICABLE.)

13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS: (2) 1 3/4" LVL PLIES = 3 1/2" GLULAM (3) 1 3/4" LVL PLIES = 5 1/2" GLULAM 14. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY

DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY OPENINGS.

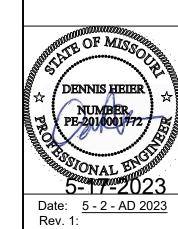
JOIST SCHEDULE 2' X 6' #2 CEILING JOIST @ 16' D.C.

E 2' X 8' #2 CEILING JOIST e 16' D.C.

Site Description:

2, Summit View Farn Street Address:

2311 SW Serena I Lee's Summit, Miss



Rev. 2: Rev. 3:

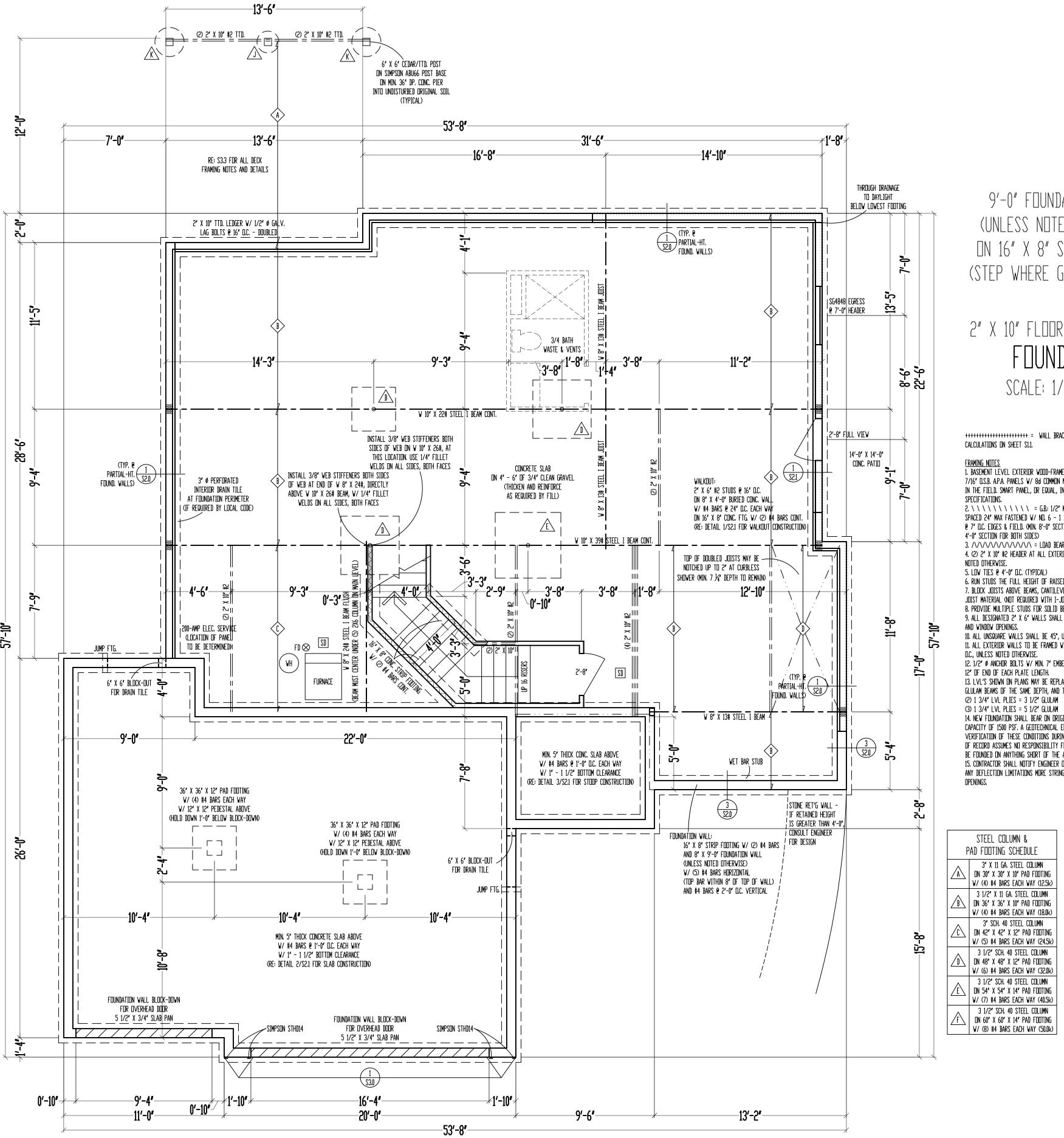
> Sheet Title: **SECOND LEVEL** PLAN

Sheet No.:

SVF082 Spec
General Contractor:

Iker Custom Homes, L

VIEWPOINT RESIDENTIAL DESIGN LLC



9'-0" FOUNDATION WALLS (UNLESS NOTED OTHERWISE) (STEP WHERE GRADE REQUIRES)

# 2" X 10" FLOOR SYSTEM ABOVE

SCALE: 1/4" = 1'-0"

1. BASEMENT LEVEL EXTERIOR WOOD-FRAMED WALLS SHALL BE SHEATHED W/ 7/16' D.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD, SMART PANEL, OR EQUAL, INSTALLED PER MANUFACTURER'S

2. \ \ \ \ \ \ \ \ = G.B; 1/2' MIN. GYPSUM BDARD DVER STUDS

SPACED 24' MAX FASTENED W/ ND. 6 - 1 1/4' TYPE W DR S DRYWALL SCREWS

9. 7' D.C. EDGES & FIELD. (MIN. 8'-0' SECTIONS DNE SIDE DF WALL (DR) MIN. 4'-0" SECTION FOR BOTH SIDES)

3. /\/\/\/\/\/\ = LOAD BEARING INTERIOR WALL. 4. (2) 2' X 10' #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS). 8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. ALL DESIGNATED 2' X 6' WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR

10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. ALL EXTERIOR WALLS TO BE FRAMED W/ MIN. STUD GRADE 2" X 6"S @ 16"

12. 1/2" Ø ANCHOR BOLTS W/ MIN. 7" EMBEDMENT @ 48" D.C. MAX. & WITHIN 6" -12" OF END OF EACH PLATE LENGTH. 13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS:

(3) 1 3/4" LVL PLIES = 5 1/2" GLULAM 14. NEV FOUNDATION SHALL BEAR ON ORIGINAL SOIL VITH MINIMUM BEARING CAPACITY OF 1500 PSF. A GEOTECHNICAL ENGINEER IS RECOMMENDED FOR VERIFICATION OF THESE CONDITIONS DURING THE EXCAVATION PHASE, ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANYTHING SHORT OF THE AFOREMENTIONED REQUIREMENTS. 15. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY

	STEEL COLUMN &		PIER FOOTING SCHEDULE		
PAD FOOTING SCHEDULE			<b>~</b>	12' Ø PIER FTG.	
Â	3' X 11 GA. STEEL COLUMN ON 30' X 30' X 10' PAD FOOTING W/ (4) #4 BARS EACH WAY (12.5k)			16' Ø PIER FTG.	
B	3 1/2' X 11 GA. STEEL COLUMN DN 36' X 36' X 10' PAD FOOTING W/ (4) #4 BARS EACH WAY (18.0k)		$\triangle$	18' Ø PIER FTG.	
Ĉ	3' SCH. 40 STEEL COLUMN ON 42' X 42' X 12' PAD FOOTING W/ (5) #4 BARS EACH WAY (24.5k)		<u>k</u>	24' Ø PIER FTG.	
$\bigwedge$	3 1/2' SCH. 40 STEEL COLUMN DN 48' X 48' X 12' PAD FOOTING W/ (6) #4 BARS EACH WAY (32.0k)			JOIST SCHEDULE	
			$\Diamond$	2" X 10" #2 TTD. FLOOR JOIS @ 16" D.C.	
	l 3 1/2" SCH, 40 STEEL COLUMN I				

ITING	$\sqrt{1}$	18° Ø PIER FIG.
18.0k) In Dting	K	24" Ø PIER FTG.
24.5k)		_
UMN Oting		JOIST SCHEDULE
32.0k) UMN	<b>⟨</b> ♣	2' X 10' #2 TTD. FLOOR JOIST @ 16' D.C.
OTING (40.5k)		2" X 10" #2 FLOOR JOIST @ 16" D.C.
UMN ITING	$\langle \diamondsuit \rangle$	2" X 10" #2 FLOOR JOIST @ 16" D.C DOUBLE EVERY OTHER
(50.0k)		2" X 10" #2 FLOOR JOIST @ 16" D.C DOUBLED

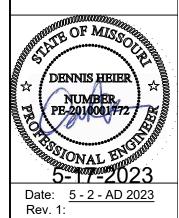
VIEWPOINT RESIDENTIAL DESIGN LLC

Site Description:

Summit View Farn Street Address:

2311 SW Serena I Lee's Summit, Miss

Spec SVF082



Rev. 2: Rev. 3:

> Sheet Title: **FOUNDATION** PLAN

Sheet No.:

	FASTENER SCHEDULE FOR STRUCTURAL MEMBERS	
DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
	ROOF <sup>1</sup>	
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOE NAIL	4-8d (2 <b>½</b> " x 0.113")	TOENAIL
CEILING JOISTS TO PLATE, TOE NAIL	4-8d (2 <b>½</b> " x 0.113")	PER JOIST, TOENAIL
CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS, FACE NAIL	4-10d (3" x 0.128")	FACE NAIL
CEILING JOIST TO PARALLEL RAFTER (HEEL JOINT)	TBLE R802.5.2	FACE NAIL
COLLAR TIE TO RAFTER, FACE NAIL OR 1 ½" x 20 GA. RIDGE STRAP TO RAFTER	4-10d (3" x 0.128")	FACE NAIL, EACH RAFTER
RAFTER OR ROOF TRUSS TO PLATE	3-16d BOX NAILS (3½" x 0.135") OR 3-10d COMMON NAILS (3" x 0.148")	2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS
ROOF RAFTERS TO RIDGE, VALLEY, OR HIP RAFTERS OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM	4-16d (3 ½" x 0.135") - TOENAIL; 3-16d BOX (3 ½" x 0.135") - END NAIL	TOENAIL, END NAIL
	WALL	
STUD TO STUD (NOT AT BRACED WALL PANELS)	10d (3" x 0.128")	16" O.C. FACE NAIL
STUD TO STUD AND ABUTTING STUDS AT NTERSECTING WALL CORNERS (AT BRACED WALL PANELS)	16d (3 <b>½</b> " x 0.135")	12" O.C. FACE NAIL
BUILT-UP HEADER, TWO PIECES WITH ½" SPACER	16d (3 <b>½</b> " x 0.135")	12" O.C. EACH EDGE FACE NAIL
CONTINUOUS HEADER TO STUD	4-8d (2½" x 0.131")	TOENAIL
TOP PLATE TO TOP PLATE	10d (3" x 0.128")	12" O.C. FACE NAIL
DOUBLE TOP PLATE SPLICE	8-16d COMMON (3 ½" x 0.162")	FACE NAIL ON EACH SIDE OF END JOINT (MIN. 24' LAP SPLICE LENGTH EACH SIDE OF END JOINT)
BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (NOT AT BRACED WALL PANELS)	16d COMMON (3 ½" x 0.162")	16" O.C. FACE NAIL
BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (AT BRACED WALL PANEL)	3-16d BOX (3 ½" x 0.135")	3 EACH 16" O.C. FACE NAIL
TOP OR SOLE PLATE TO STUD, END NAIL	4-8d BOX (2 $\frac{1}{2}$ " x 0.113") - TOENAIL; 3-16d BOX (3 $\frac{1}{2}$ " x 0.135") - END NAIL	TOENAIL, END NAIL (SEE LEFT)
TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	3-10d BOX (3" x 0.128")	FACE NAIL
1" BRACE TO EACH STUD AND PLATE	3-8d BOX (2 ½" x 0.113")	FACE NAIL
1"x6" SHEATHING TO EACH BEARING	3-8d BOX (2 ½" x 0.113")	FACE NAIL
1"x8" SHEATHING TO EACH BEARING	3-8d BOX ( $2\frac{1}{2}$ " x 0.113") - FACE NAIL; WIDER THAN 1"x8" - 4-8d BOX ( $2\frac{1}{2}$ " x 0.113")	FACE NAIL
	FLOOR	
JOIST TO SILL, TOP PLATE, OR GIRDER	4-8d BOX (2 ½" x 0.113")	TOE NAIL
RIM JOIST, BAND JOIST, OR BLOCKING TO SILL OR TOP PLATE (ROOF APPLICATIONS ALSO)	8d BOX (2 ½" x 0.113")	4" O.C. TOE NAIL
1" x 6" SUBFLOOR OR LESS TO EACH JOIST	3-8d BOX (2 ½" x 0.113")	FACE NAIL
2" SUBFLOOR TO JOIST OR GIRDER	3-16d BOX (3 ½" x 0.135")	BLIND AND FACE NAIL
2" PLANKS (PLAN & BEAM - FLOOR AND ROOF)	3-16d BOX (3 ½" x 0.135")	AT EACH BEARING, FACE NAIL
BAND OR RIM JOIST TO JOIST	3-16d COMMON (3 ½" x 0.162")	END NAIL
BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS	10d BOX (3" x 0.128")	24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	4-16d BOX (3 ½" x 0.135")	AT EACH JOIST OR RAFTER, FACE NAIL
BRIDGING OR BLOCKING TO JOIST	2-10d BOX (3" x 0.128")	EACH END, TOENAIL

CRIPTION OF BUILDING MATERIAL		EDGE SPACING (INCHES)	INTERMEDIATE SUPPORTS (INCH			
WOOD STRUCTURAL PANELS, SUBFLOOR, ROOF AND INTERIOR WALL SHEATHING TO FRAMING AND PARTICLEBOARD WALL SHEATHING TO FRAMING						
¾" - ½"	6d COMMON (2" x 0.113") NAIL (SUBFLOOR, WALL) 8d COMMON NAIL (ROOF)	6	12			
<sup>19</sup> / <sub>32</sub> " - 1"	8d COMMON NAIL (2 <b>½</b> " x 0.131")	6	12			
11/8" - 11/4"	10d COMMON (3" x 0.148") NAIL OR 8d (2 <b>½</b> " x 0.131") DEFORMED NAIL	6	12			
	OTHER WALL	SHEATHING 1				
½" STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	1 $\frac{1}{2}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR 1 $\frac{1}{4}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6			
25" STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	$1\frac{3}{4}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR $1\frac{1}{2}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6			
<b>½</b> " GYPSUM SHEATHING	1½" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1½" SCREWS, TYPE W OR S	7	7			
%" GYPSUM SHEATHING	1¾" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1¾" LONG; 1¾" SCREWS, TYPE W OR S	7	7			
v	VOOD STRUCTURAL PANELS, COMBINATIO	N SUBFLOOR UNDERLAYMENT TO FRAM	ING			
¾" AND LESS	6d DEFORMED (2" x 0.120") NAIL OR 8d COMMON (2½" x 0.131") NAIL	6	12			
<b>⅓</b> " - 1"	8d COMMON (2½" x 0.131") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12			
1 <b>½</b> " - 1 <b>½</b> "	10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12			

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION LISTED IN THIS TABLE

#### **FOUNDATION NOTES**

- CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLABS
- THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION STANDARDS
- PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A
- MINIMUM 20 GALLON SUMP PIT. FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY
- SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES. FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE
- 6. CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE
- FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0
- REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE) INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND, GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY
- VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES 11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB
- SHALL BE DESIGNED BY A LICENSED ENGINEER 12. SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH ½" Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS)

AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6

13. FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET 14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT

#### FRAMING NOTES

- 15. ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS 16. ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 - 2x10's, UNLESS NOTED OTHERWISE
- 17. BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS

DISCHARGES TO THE EXTERIOR, ABOVE GRADE

- 18. INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A
- 19. ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED
- 20. WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS.
- 21. ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT
- 22. JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.
- 23. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS
- 24. JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 11/8" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT
- 25. ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3 26. ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.
- 27. ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER ¼ OF VERTICAL DISTANCE BETWEEN CEILING AND
- 28. BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED
- 29. PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A 1/8" GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION)
- 30. ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi
- ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi 32. COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. ½" x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER. LOCK WASHER. AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY B WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.
- WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING.
- 34. ALL ROOF SHEATHING SHALL BE  $\frac{7}{16}$ " OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

### **GLAZING NOTES**

- 35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0"
- 36. ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

### ATTIC VENTILATION

37. ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH 1/8" TO 1/4" OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/50 OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

### **EMERGENCY EGRESS**

- PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP. 39. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF
- MASONRY VENEER

DETECTORS OUTSIDE EACH SLEEPING AREA.

LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC 2018

40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1½", WITH NOT LESS

ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE

- THAN 5/8" MORTAR OR GROUT COVER TO OUTSIDE FACE. 41. VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 🔏 "
- 42. EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE
- THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY. 43. VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

## **GARAGE NOTES**

- 44. DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 13/8" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE
- 45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST

#### **GARAGE NOTES (CONTINUED)**

- THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 5/8" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM 5/8" TYPE X GYP. BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS
- SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH %" GYP. BOARD. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING AND SHALL BE FASTENED WITH 2½"" x 0.120" NAILS AT 7" O.C. STAGGERED WITH (7) 31/4" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER. MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

#### DESIGN LOADING (PER TABLE R301.5)

MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS (PSF)					
USE	LIVE LOAD	DEAD LOAD			
UNINHABITABLE ATTICS WITHOUT STORAGE	10	10			
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20	10			
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30	10			
BALCONIES (EXTERIOR) AND DECKS	40	10 <sup>d</sup>			
FIRE ESCAPES	40	10			
GUARDRAILS AND HANDRAILS <sup>a</sup>	200 <sup>c</sup>	-			
GUARDRAIL IN-FILL COMPONENTS <sup>b</sup>	50 <sup>c</sup>	-			
PASSENGER VEHICLE GARAGES	50	DEPENDENT UPON SLAB CONSTRUCTION			
ROOMS OTHER THAN SLEEPING ROOM	40	10 <sup>d</sup>			
SLEEPING ROOM	30	10 <sup>d</sup>			
STAIRS	40	10 <sup>d</sup>			

a. A single concentrated load applied in any direction at any point along the top. b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement. c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load. d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed.

#### INSULATION/EFFICIENCY

- BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON

INSULATION AND FENESTRATION REQUIRE CLIMATE ZONE	EMENTS BY COMPONENT (TABLE N1102.1.1)  4-A
FENESTRATION U-FACTOR	0.35
SKYLIGHT U-FACTOR	0.55
GLAZED FENSTRATION SHGC	0.40
CEILING R-VALUE	49
WOOD FRAME WALL R-VALUE	15
MASS WALL R-VALUE	8 / 13
FLOOR R-VALUE	19
BASEMENT WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
SLAB R-VALUE AND DEPTH	10 AT 2'-0"
CRAWL SPACE WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	8
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	6
CATHEDRAL VAULTED CEILING R-VALUE	38

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2018 IRC.

- AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT SEALS.
- WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.

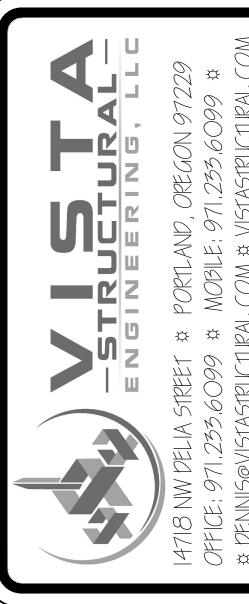
### DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:

- POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED
- OR OTHERWISE SEALED DURING THE TEST. ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA.

**EXCEPTION:** THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

NAT	COLLANDON A VENITURATIO	N OVOTEM EAN EEELOA	OV
FAN LOCATION	AIR FLOW RATE  MINIMUM (CFM)	N SYSTEM FAN EFFICA: MINIMUM EFFICACY (CFM/WATT)	AIR FLOW RATE  MAXIMUM (CFM)
RANGE HOODS	ANY	2.8	ANY
IN-LINE FAN	ANY	2.8	ANY
BATHROOM, UTILITY ROOM	10	1.4	90
BATHROOM, UTILITY ROOM	90	2.8	ANY

MULTIPLE-PLY WOOD BEAM FASTENING SCHEDULE							
DIMENSIONAL LUMBER BEAM SIZE/TYPE	FASTENERS	LVL BEAM SIZE/TYPE	FASTENERS	LVL BEAM SIZE/TYPE	FASTENERS		
(2) 2x	(2) ROWS 10d @ 12" O.C. ONE SIDE	(2) 1 $\frac{3}{4}$ " UP TO 11 $\frac{7}{8}$ " DEPTH	(2) ROWS 16d @ 12" O.C. ONE SIDE	(3) 1 ¾" x 14"+ DEPTH	(3) ROWS 16d @ 12" O.C. BOTH SIDES		
(3) 2x	(2) ROWS 10d @ 12" O.C. BOTH SIDES	(2) 1 ¾" 14"+ DEPTH	(3) ROWS 16d @ 12" O.C. ONE SIDE	(4) 1 $rac{3}{4}$ " UP TO 11 $rac{7}{8}$ " DEPTH	(2) ROWS ¼" x 5" SIMPSON SDS OR SDWS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM BOTH SIDES		
(4) 2x	(2) ROWS ¼" x 5" SIMPSON SDS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM, BOTH SIDES	(3) 1 $\frac{3}{4}$ " UP TO 11 $\frac{7}{8}$ " DEPTH	(2) ROWS OF 16d @ 12" O.C. BOTH SIDES	(4) 1 ¾" x 14"+ DEPTH	(3) ROWS ¼" x 5" SIMPSON SDS OR SDWS SCREWS @ 16" O.C. STAGGERED TOP & BOTTOM BOTH SIDES		



NO.	DATE	RE	VISION		BY	
DRAV	VINO TITLE					
	STRUCTURAL					
	N	O	ΓES			
ENGI	NEER: DIV	1H	CHECKED B	Y:D <b>I</b>	ИΗ	
JOB N			DRAWN BY:	DI	ИΗ	
		~ ~				

#### RESIDENTIAL SEISMIC & WIND ANALYSIS

			INPUT
			CALCULATED VALUE
	DEAD LOAD (psf)	AREA (ft <sup>2</sup> )	WEIGHT (lbs.)
	10	2692	26920
	10	2692	26920
	10	827	8270
	10	2692	26920
WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WEIGHT (lbs)
166.4	8	8	10649.6
223	10	10	22300
	DEAD LOAD (psf)	AREA (ft2)	WEIGHT (lbs)
	6	827	4962
	6	2692	16152
	166.4	10 10 10 10 10 WALL LENGTH (ft) WALL HEIGHT (ft)  166.4 8 223 10	10 2692 10 2692 10 827 10 827 10 2692 WALL LENGTH (ft) WALL HEIGHT (ft) WALL UNIT WT. (psf) 166.4 8 8 223 10 10 10 DEAD LOAD (psf) AREA (ft2) 6 827

	PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED)						
:	FRONT	-TO-BACK			SIDE-TO-SI	DE	
	AREA	LOAD			AREA	LOAD	
SLOPED ROOF	300	2552		SLOPED ROOF	442	3736	
VERT. ROOF	0	0	CUMULATIVE	VERT. ROOF	0	0	CUMULATIVE
2ND	412.83	5213	7766	2ND	335.97	4346	8082
1ST	590.37	7339	15105	1ST	636.13	7856	15938
BSMT <sup>a</sup>	0	0	0	BSMT <sup>a</sup>	157	2223	10192
			PRESSURE (PSF	) - PER ASCE CH. 6			
	SLOPED ROOF	ZONE B		9.7	ZONE C	11.3	2a (FIG. 28.6-1, ASCE7)
	WALL/VERT. ROOF	ZONE A		14.2	ZONE D	7.7	10.734
	MEAN ROOF HT., h		29	_			

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area.  $q_{z10}$ =0.00256 $K_zK_zt_dV^2$  (ASCE7-10 Velocity Pressure)  $q_{z10\_ASD}$ =0.6 $q_{z10}$  (Design Velocity Pressure for AS q<sub>z10\_ASD</sub>=0.6q<sub>z10</sub> (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)

2ND FLOOR TRIBUTARY WEIGHT 1ST FLOOR TRIBUTARY WEIGHT BASEMENT TRIBUTARY WEIGHT

S<sub>S</sub> (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP) F<sub>a</sub> (from ASCE7 Table 11.4-1)  $S_{DS}$  (= 2/3 \*  $S_{S}$  \*  $F_{a}$ )

EXTERIOR SHEATHING OPTION FOR SECOND FLOOR

EXTERIOR SHEATHING OPTION FOR FIRST FLOOR EXTERIOR SHEATHING OPTION FOR BASEMENT WALLS

88871.6 R (from ASCE7 Table 12.2-1) LOCATION From ASCE7 (Eq. 12.8-1):

2ND FLOOR					1398
1ST FLOOR BASEMENT					2100 2100
BAGEMENT		<del></del>			2100
Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowab	le Shear (#/LF)	Code Reference
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" (6ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing		155	per IBC, Table 2306.3(1)
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" (6ga. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing		230	per IBC, Table 2306.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" (6ga. Staples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing		310	per IBC, Table 2306.3(1)
Exterior (Option #4)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing		220	AF&PA SDPWS Table 4.3A
Exterior ( <i>Option #5</i> )	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing		320	AF&PA SDPWS Table 4.3A
Exterior (Option #6)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field		410	AF&PA SDPWS Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 1 <sup>1</sup> / <sub>4</sub> " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field		60	per IBC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)		325	

					GAR. WALL: 1=F-B, 2=S-S	2		
			EXTER	IOR STRUCTURAL WALL I	LENGTHS (ft.) & RESISTANCES	-		
		SE	ISMIC			WIND		
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)
2ND FLOOR	30	8400	42	11760	30	11760	42	16464
1ST FLOOR	70	26600	30	11400	70	37240	30	15960
BASEMENT	0	0	31.5	8820	0	0	31.5	12348

DEPTH OF 1ST STORY (FT.)

BACK WALL OF GARAGE (FT.)

	ADDITIONAL RESIS	TANCE REQUIRED
	SEISMIC	WIND
2ND FLOOR FRONT-TO-BACK	0	0
2ND FLOOR SIDE-TO-SIDE	0	0
1ST FLOOR FRONT-TO-BACK	0	0
1ST FLOOR SIDE-TO-SIDE	0	0
BASEMENT FRONT-TO-BACK	0	0
BASEMENT SIDE-TO-SIDE	0	0

Anchor Bolt Spacing	(in.)	16d Nail Spacing req'd at	bottom plate (in.)
diameter (in.)	0.5	2nd Floor F-B	
Shear value (per NDS)	944	2nd Floor S-S	
Spacing F-B (inches)	138.8	1st Floor F-B	
spacing S-S (inches	122.1	1st Floor S-S	

WIDTH OF 1ST STORY (FT.) 53.67 WIDTH OF 2ND STORY (FT.) 45.87

57.83

88871.6

12.0%

1.6

0.128

6.5

DEPTH OF 2ND STORY (FT.)

V (= 1.2 \* S<sub>DS</sub> \* W / R) (lbs.)

	RESISTANCE REQUIR	RED IN ADDITION TO RES	SISTANCE PROVIDED BY EXTERIOR V	VALLS**		
RESISTANCE	PERF. SHEAR WALL	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)	OK?
0					0	YES
0					0	YES
0					0	YES
0					Ō	YES
0					0	YES
0					0	YES
	RESISTANCE	ADDITIONAL PORTAL FRAMES OR RESISTANCE PERF. SHEAR WALL	ADDITIONAL PORTAL FRAMES OR RESISTANCE PERF. SHEAR WALL (325#/BRACE)	ADDITIONAL PORTAL FRAMES OR PERF. SHEAR WALL INTERIOR X-BRACES INTERIOR WALL LENGTH W/ 1/2"  (325#/BRACE) GYPSUM BOARD PER TABLE (FT.)	RESISTANCE	ADDITIONAL RESISTANCE PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE PERF. SHEAR WALL RESISTANCE PERF. SHEAR WALL RESISTANCE (325#/BRACE) RESISTANCE PROVIDED BY ADDITIONAL METHODS (POLINDS)

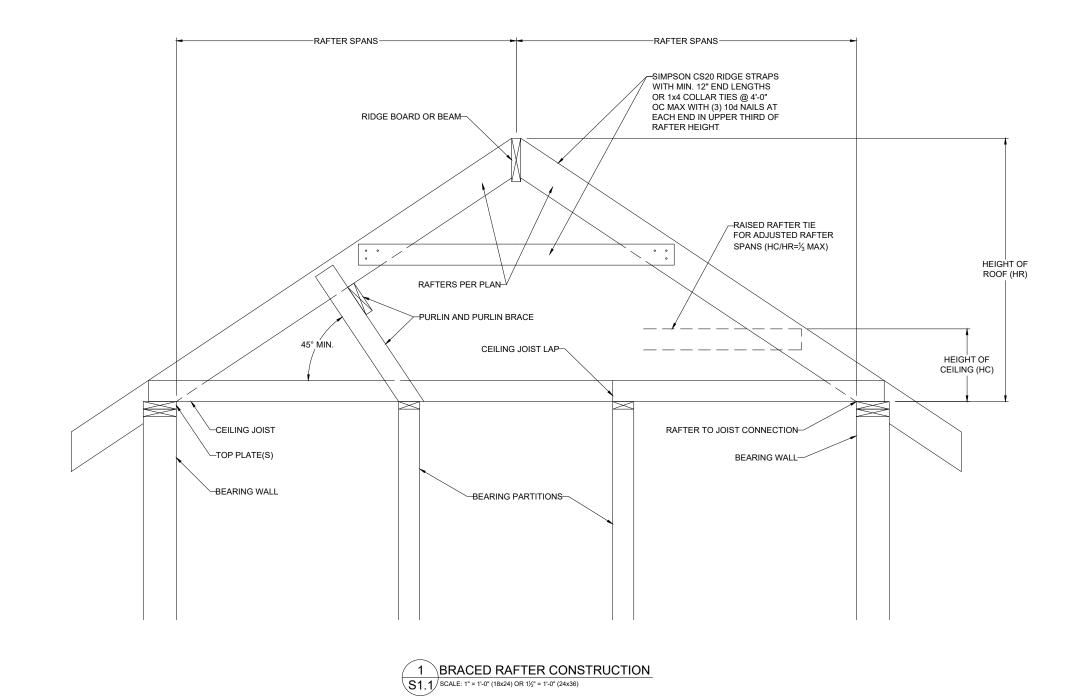
\*\*NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE),
2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING
PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER

ALL LATERAL BRACE	L LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRE							
	WIND UPLIFT ANALYSIS							
	X/12	DEGREES				· · · · · · · · · · · · · · · · · · ·		
ROOF PITCH (MAX)	10	39.8	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2				
		ASCE 7						
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)				
OVERHANG	1	-1.08	225	-1.08				
	TOTAL AREA (FT <sup>2</sup> )	ZONE E AREA (FT <sup>2</sup> )	ZONE G AREA (FT <sup>2</sup> )	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)	
MAIN ROOF**	3103.7361	1325.262576	1778.473524	-1.08	-0.36	-2072	-9.3	
	·							
*ALONG PERIMETER		TOTAL UPLIFT PER LINEAL	FOOT ALONG EXTERIOR (P	OUNDS)	-10.4	UPLIFT OK		
**INSIDE EXTERIOR V	**INSIDE EXTERIOR WALLS RESISTANCE DUE TO DEAD WEIGHT & (3) 10d TOENALLS				251.6			

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

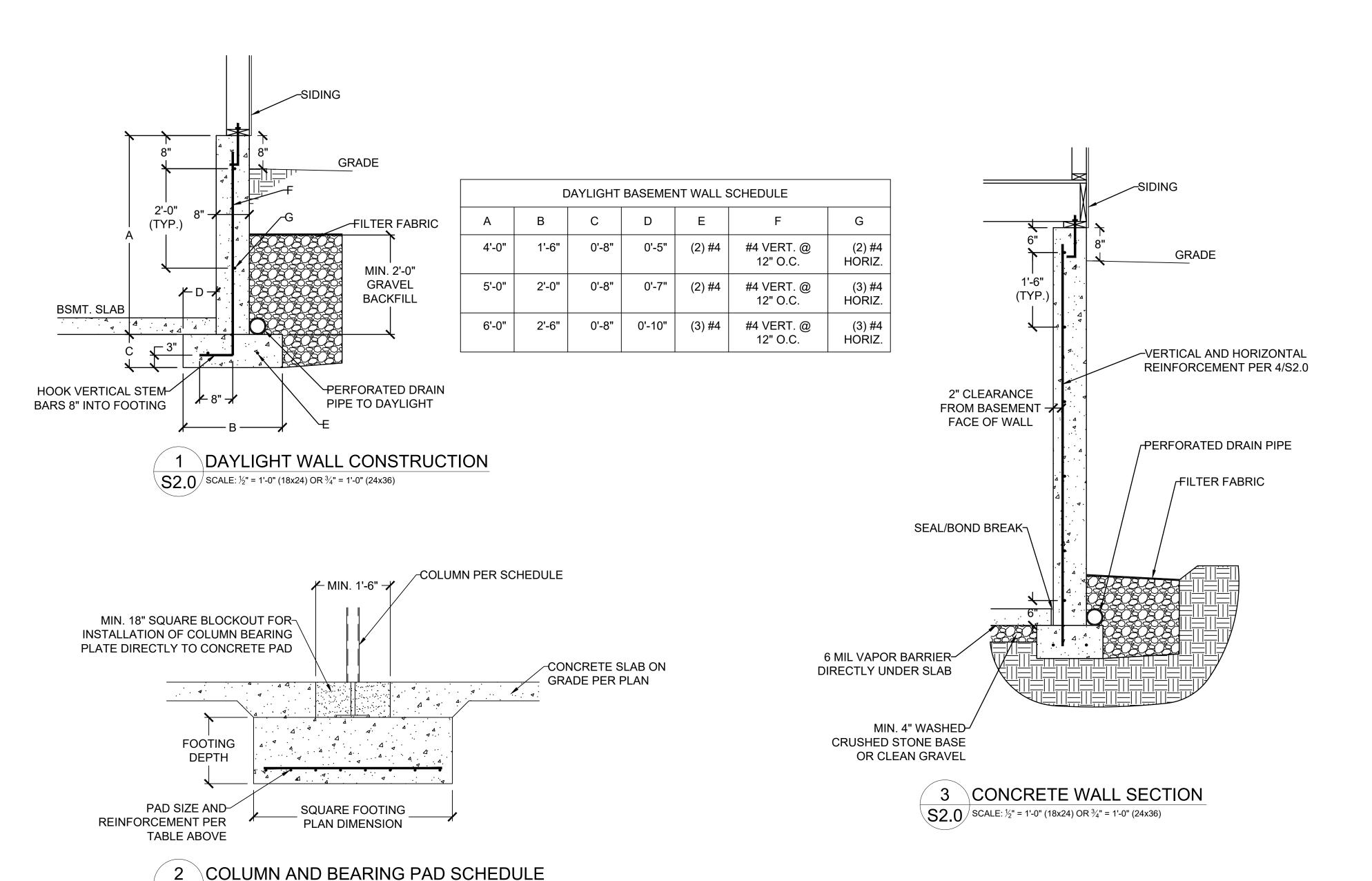
NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION







NO.	DATE	RE	VISION		В١
_					
				$\dashv$	
				$\neg$	
DRAW	ING TITLE			-	
S	TRI	IC	TUF	ξΔ	A.
SA	LCU	JL	ATI	OI	V
FNGIN	IEER: DM	<u>—</u>	CHECKED E	 3Y: <b>DI</b>	MН
		• •			
JOB N	Ο.		DRAWN BY:	: DI	ИΗ



TYPICAL CORNER REINFORCEMENT! - L

AT LEAST (1) #4 BAR 48" LONG @

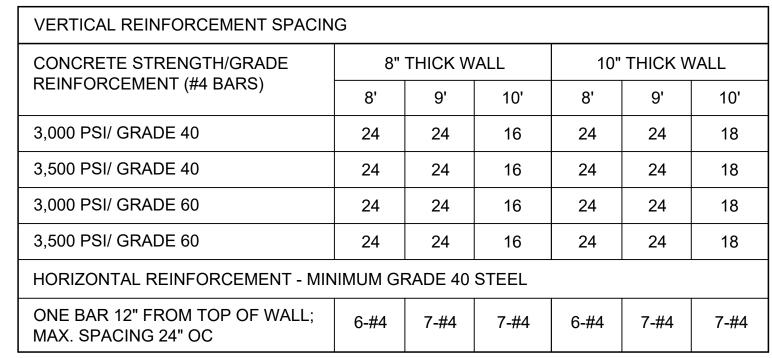
AS CLOSE AS PRACTICAL TO THE CORNER

**EACH INSIDE CORNER** 

NOTE: WHERE OPENINGS OR ABRUPT ELEVATION

CHANGES OCCUR IN THE TOP OR BOTTOM OF THE WALL

AT LEAST ONE #4 BAR 48" LONG SHALL BE DIAGONALLY



#### FOOTNOTES:

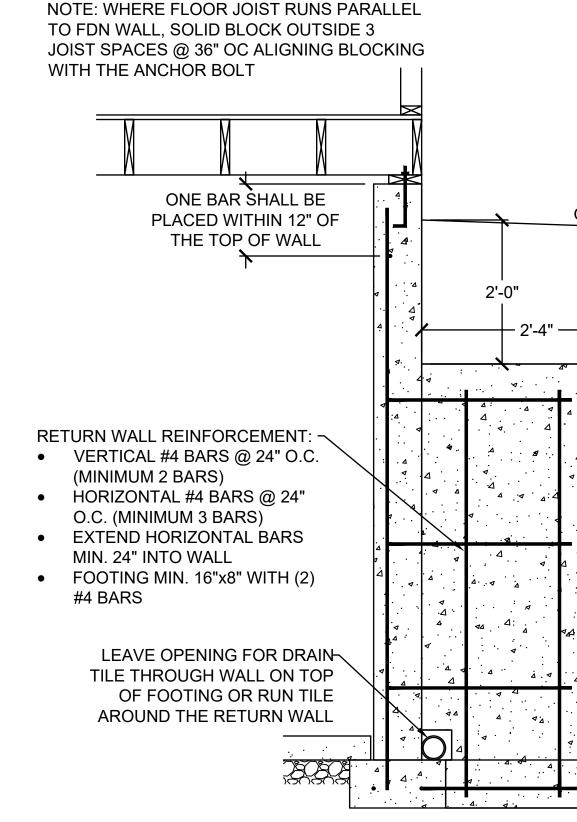
1) WALL HEIGHT IS MEASURED FROM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB 2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE WALL. OTHER WALLS SHALL HAVE VERTICAL REINFORCEMENT AS FOLLOWS:

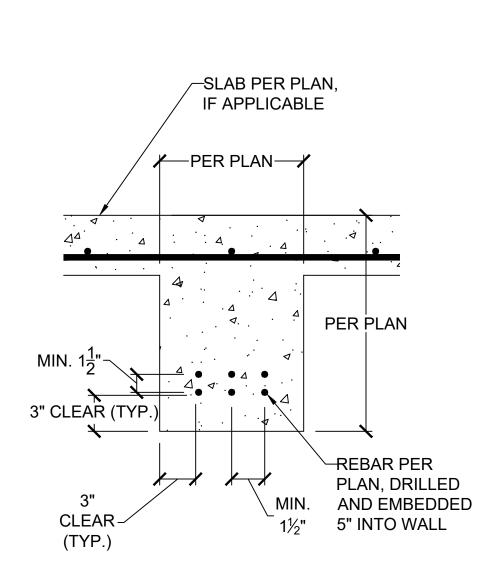
- A) 8" WALL MINIMUM 5" FROM THE OUTSIDE FACE
- B) 10" WALL MINIMUM 63/4" FROM THE OUTSIDE FACE
- C) EXTEND BARS TO WITHIN 8" OF THE TOP OF THE WALL

#### 3) REINFORCEMENT CLEARANCES:

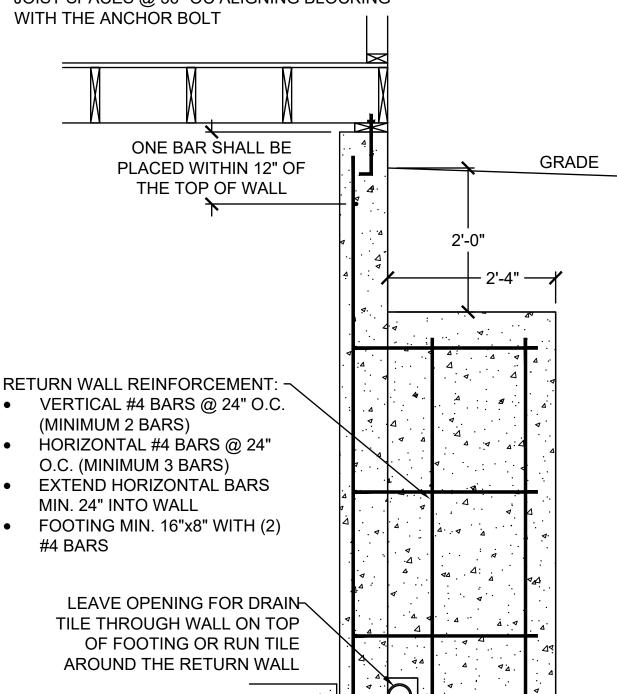
- A) CONCRETE EXPOSED TO EARTH MINIMUM 11/2"
- B) NOT EXPOSED TO WEATHER (INTERIOR SIDE OF WALLS) -3/4" C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY
- SLABS) 1½"
- 4) HORIZONTAL REINFORCEMENT: A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL
  - B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC
  - C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE
  - D) SUPPLEMENTAL REINFORCEMENT AT CORNERS PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF THE EDGE OF INSIDE CORNERS.
- 5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND CORNERS.
- 6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE 3½". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP
- 7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS
- WALL SHALL NOT BE BACKFILLED UNTIL FLOOR SYSTEM AND DIAPHRAGM ARE IN PLACE

4 FOUNDATION WALL REINFORCEMENT TABLE S2.0 NO SCALE





8 CONCRETE GRADE BEAM S2.0 SCALE: 1" = 1'-0" (18x24) OR  $1\frac{1}{2}$ " = 1'-0" (24x36)



5 SOLID JUMP S2.0 SCALE:  $\frac{1}{2}$ " = 1'-0" (18x24) OR  $\frac{3}{4}$ " = 1'-0" (24x36)

S2.0 SCALE:  $\frac{1}{2}$ " = 1'-0" (18x24) OR  $\frac{3}{4}$ " = 1'-0" (24x36)

MIN. (2) #4 BARS EXTENDING 24"

PAST OVER-EXCAVATION AND

INTO INTERSECTING WALL

-CONTINUOUS FOOTING

AND REBAR THROUGH

6'-0" MAX.

SOLID JUMP

MAX. 12" BLOCKOUT FOR

FORM PLACEMENT AND

TO EXTEND DRAIN TILE

6 REINFORCEMENT AT OPENING CORNERS S2.0 AND STEP CORNERS @ INSIDE CORNERS SCALE:  $\frac{1}{2}$ " = 1'-0" (18x24) OR  $\frac{3}{4}$ " = 1'-0" (24x36)

7 RETURN WALL DETAIL  $\frac{1}{52.0}$  SCALE:  $\frac{1}{2}$ " = 1'-0" (18x24) OR  $\frac{3}{4}$ " = 1'-0" (24x36)



REVISION

FOUNDATION

**DETAILS** 

ENGINEER: DMH | CHECKED BY:DMH

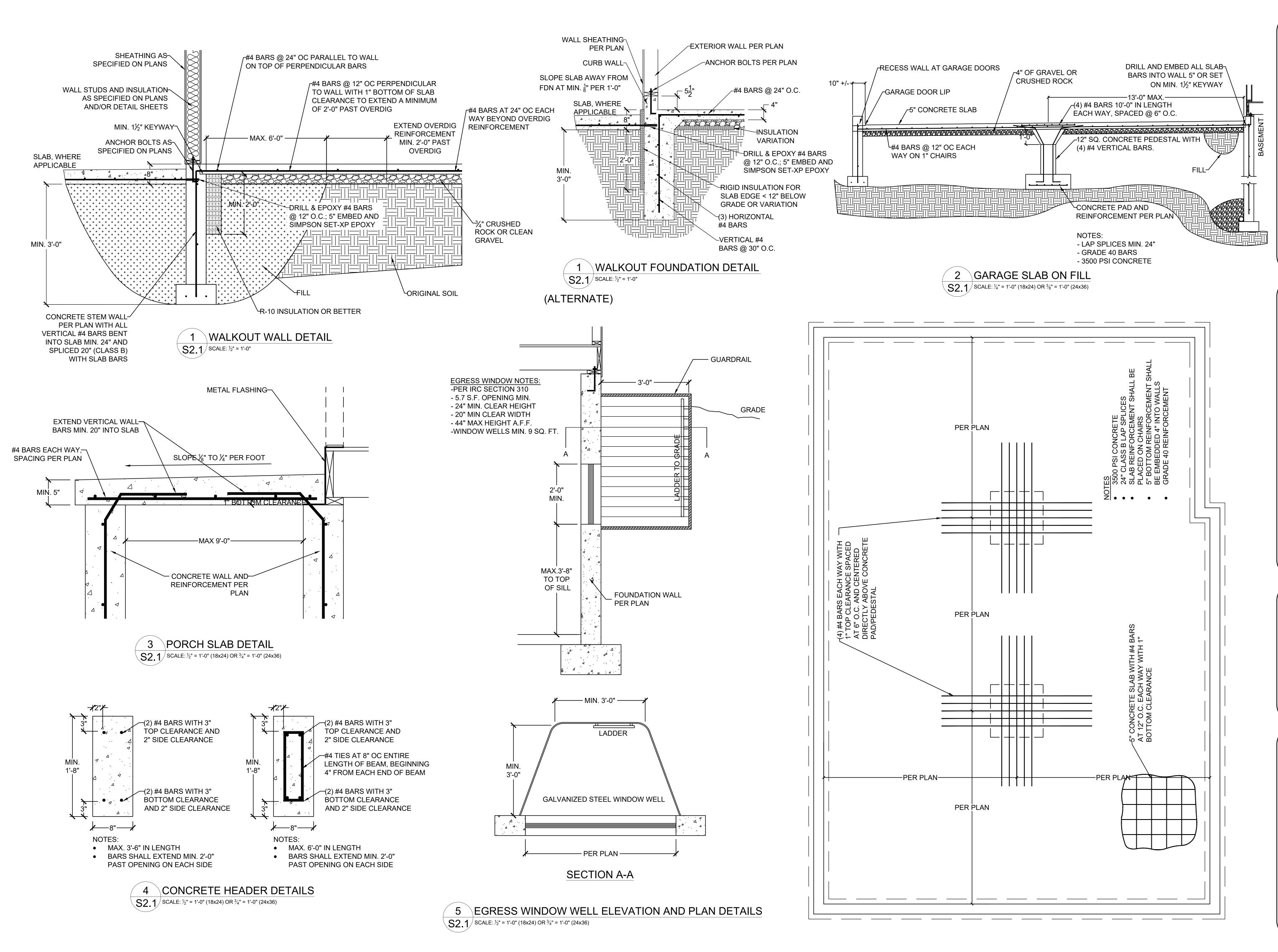
DRAWN BY: DMH

DATE

DRAWING TITLE

DATE: **5-17-23** 

SHEET NUMBER

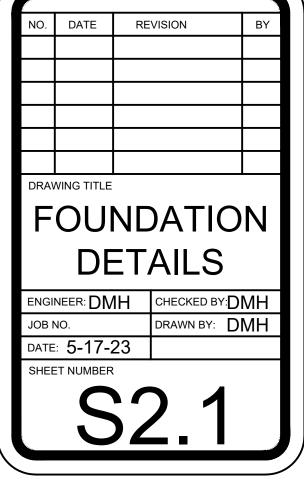


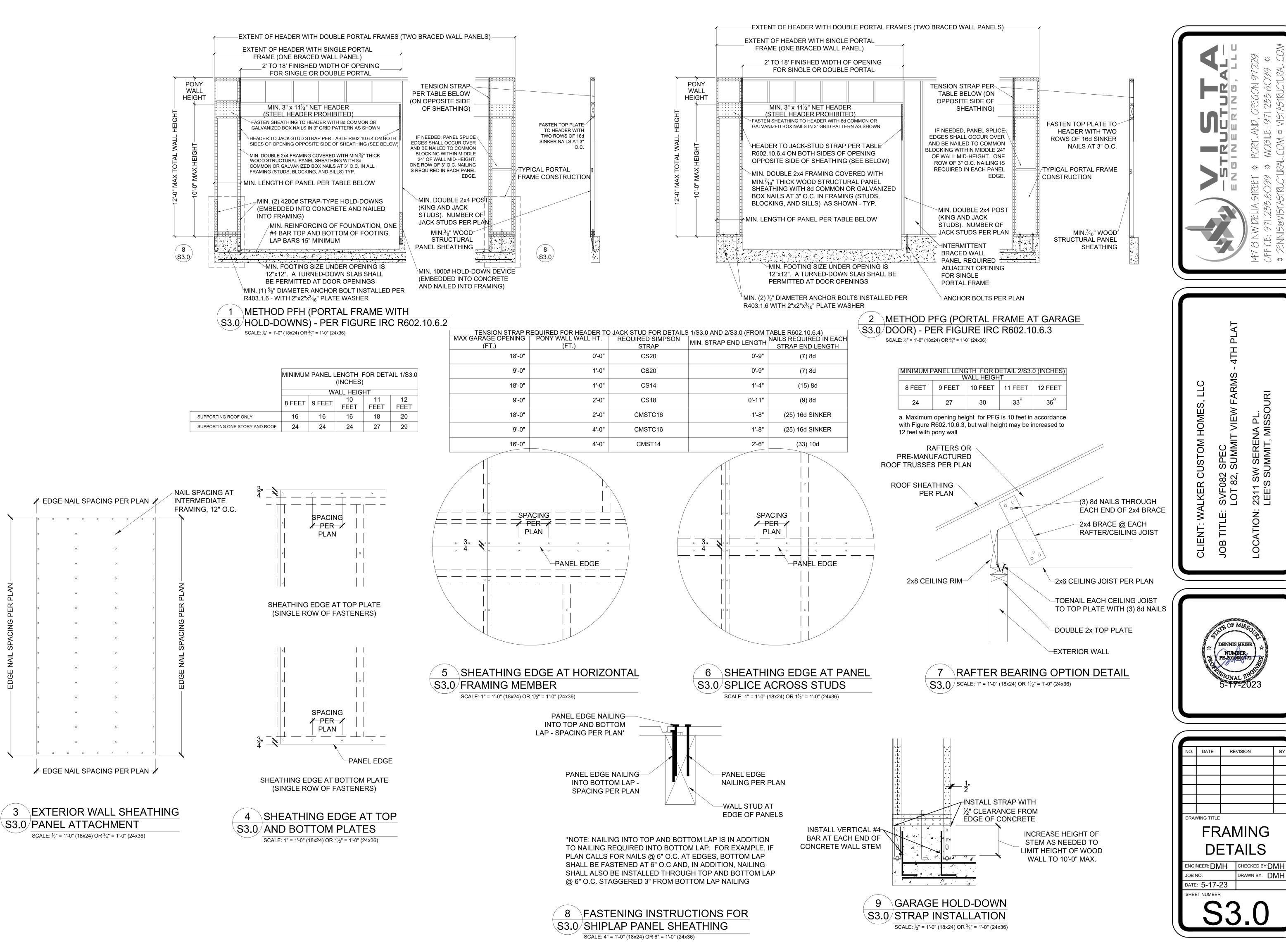
ENGINEERING, LLC

14718 NW PELIA STREET & PORTLAND, OREGON 97229

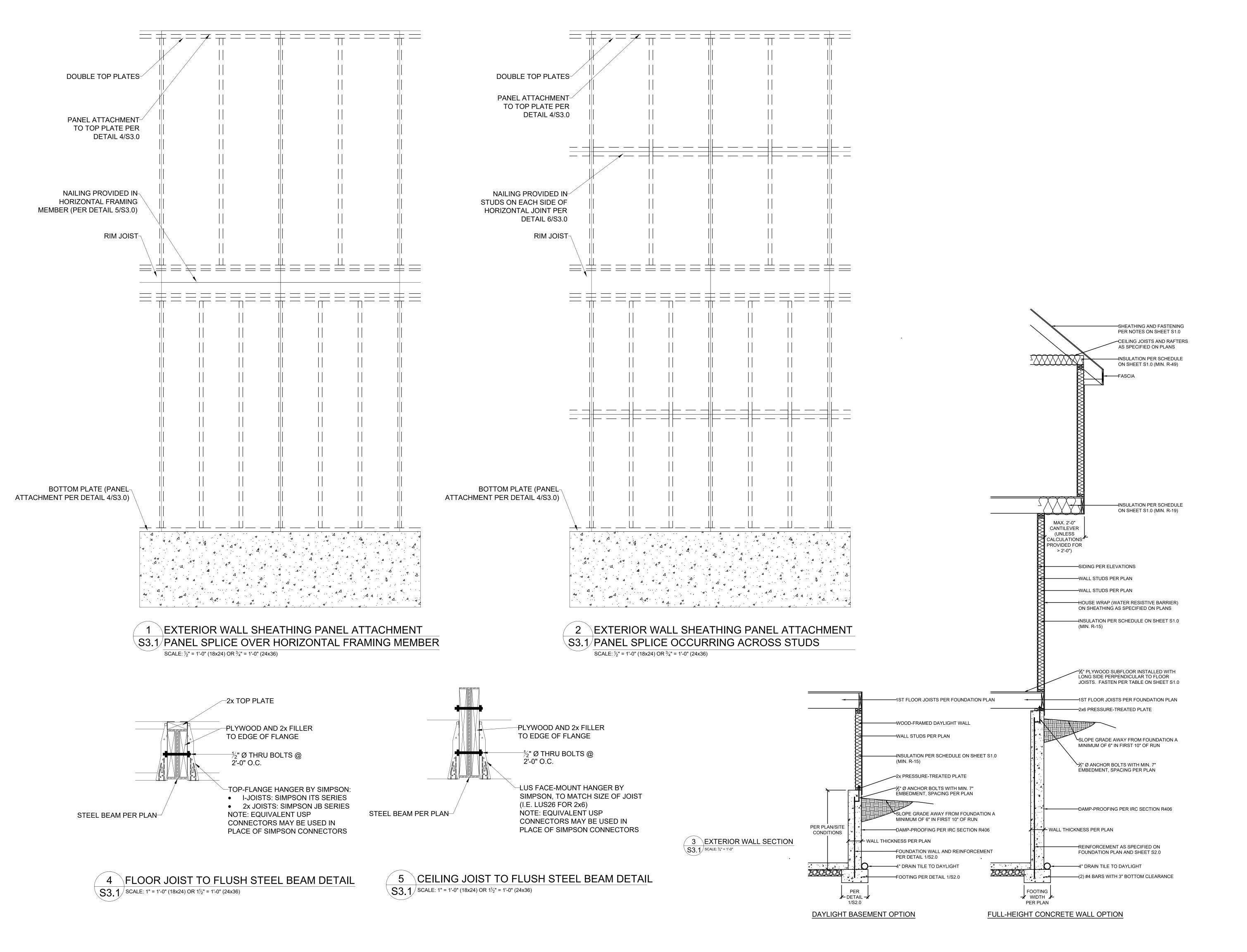
OFFICE; 971,255,6099 & MOBILE; 971,255,6099 & WISTASTRUCTURAL, COM

SVF082 SPEC OT 82, SUMMIT VIEW FARMS - 4TH PLAT





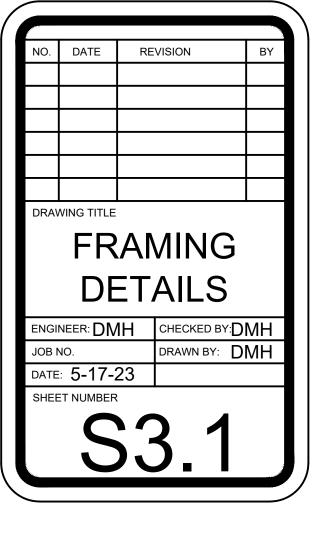
LE

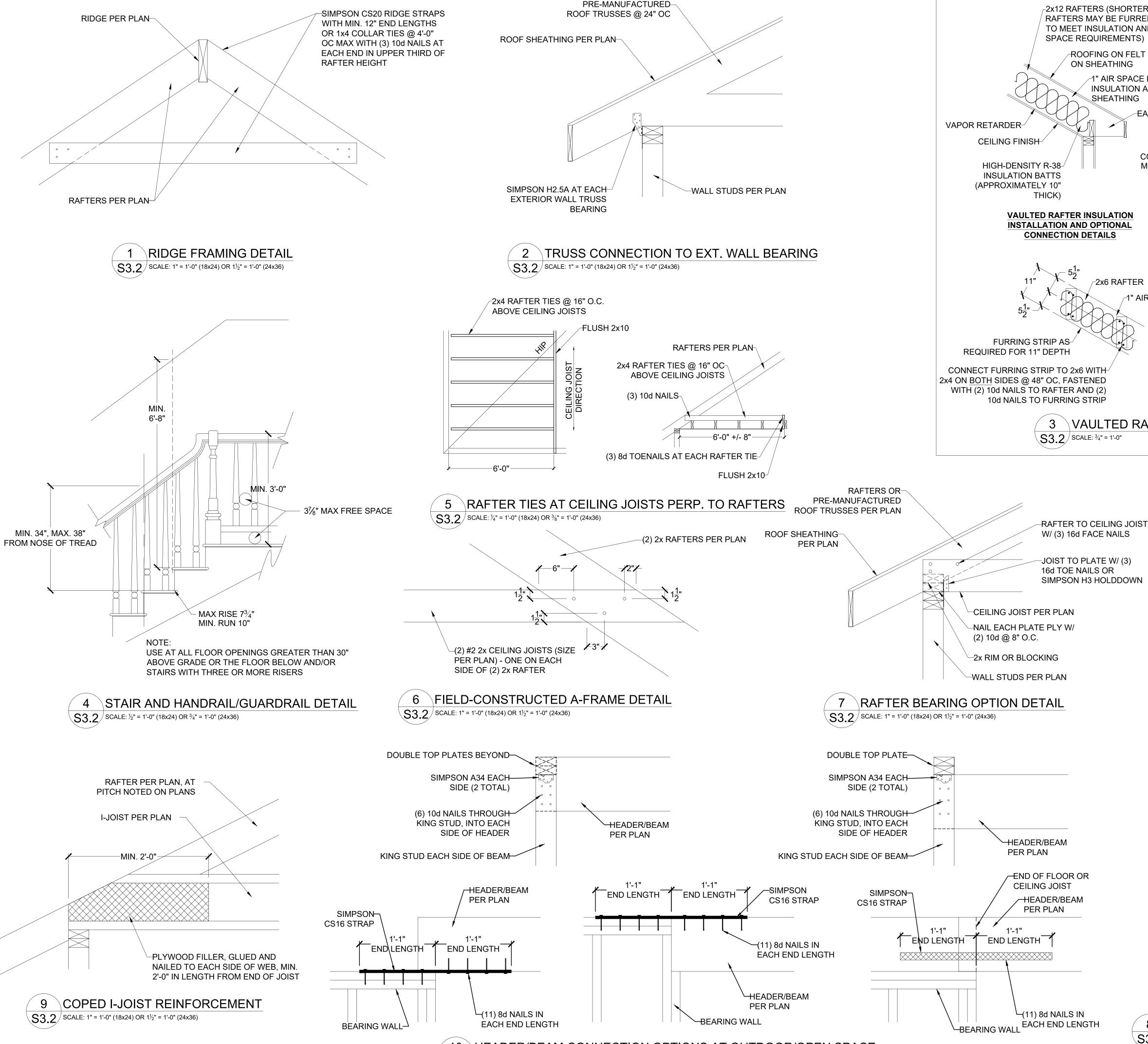


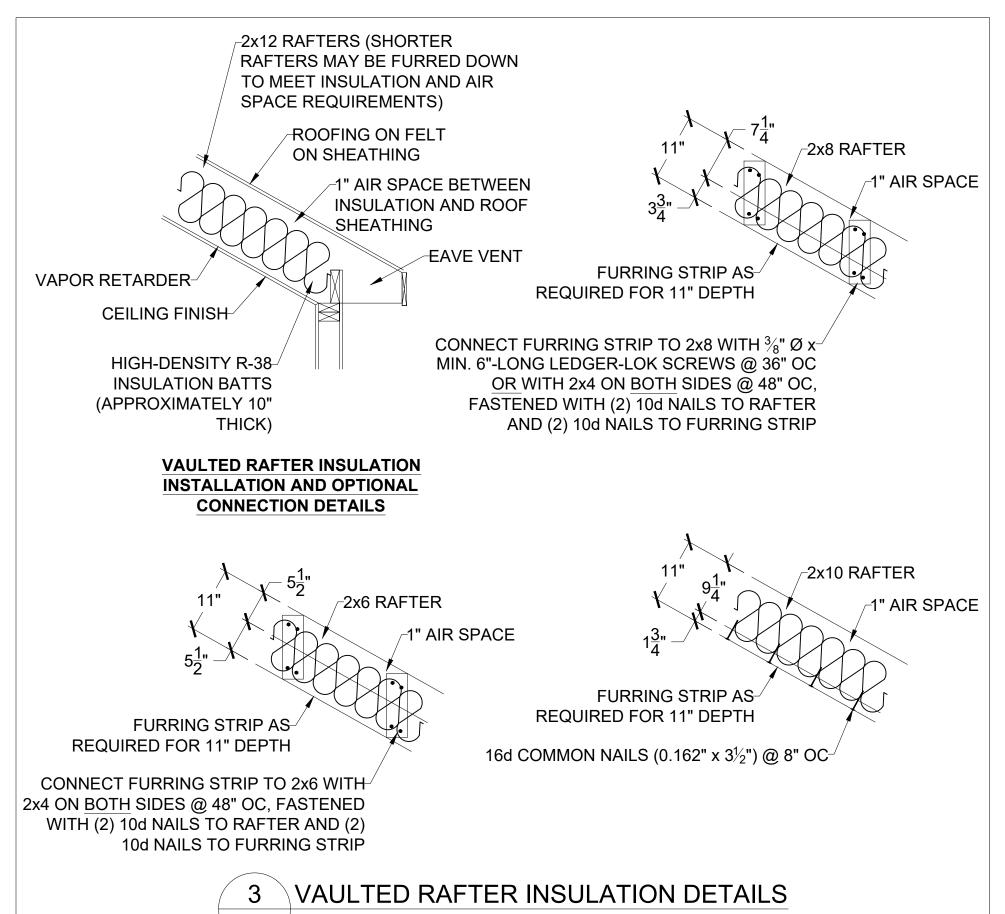


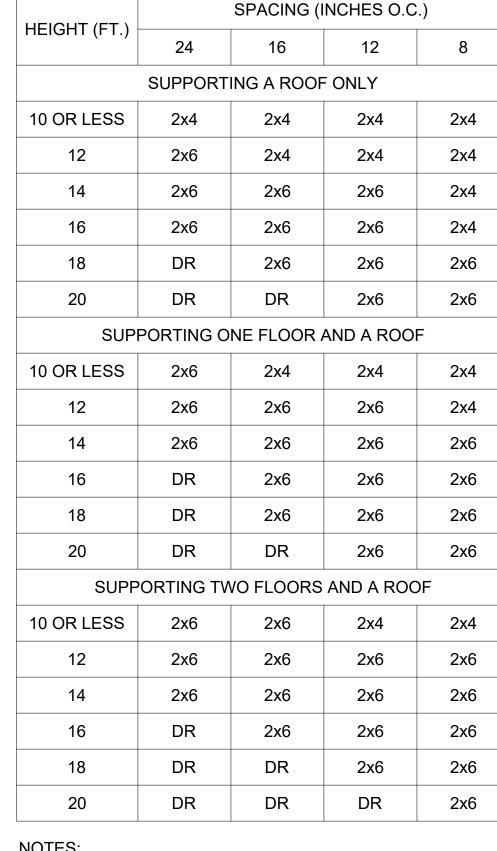
E: SVF082 SPEC











NOTES:

1) DR = DESIGN REQUIRED

2) UTILITY, STANDARD, STUD AND #3 GRADE LUMBER OF ANY SPECIES ARE NOT PERMITTED

3) THIS TABLE DOES NOT APPLY FOR STUDS SUPPORTING MEMBERS WITH A TRIB. LENGTH GREATER THAN 6'-0"

8 MAXIMUM ALLOWABLE LENGTH OF S3.2 WOOD WALL STUDS (IRC TABLE 602.3.1)

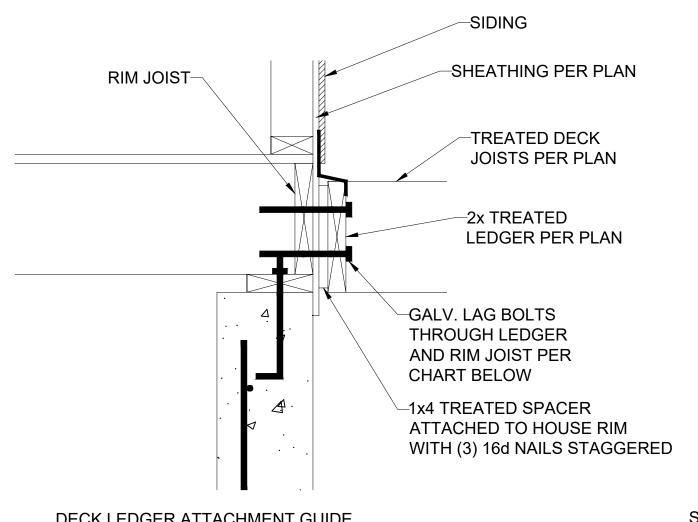


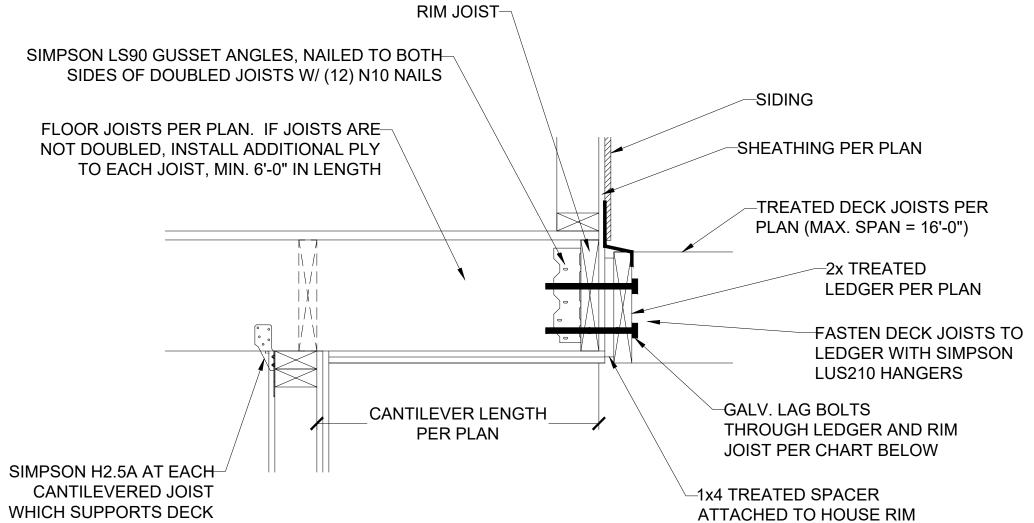
3 TITLE: SVF082 SPEC
LOT 82, SUMMIT VIEW FARMS - 4TH
SATION: 2311 SW SERENA PL.
LEE'S SUMMIT, MISSOURI

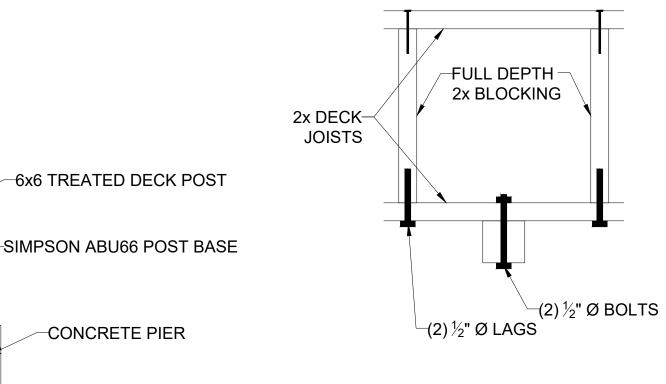


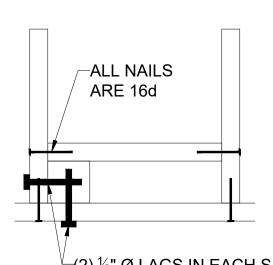
1					****
	NO.	DATE	RE	VISION	BY
	DRA	WING TITLE			
		FR	ΔΝ	MING	`
		DE	ΞΤ	AILS	
	ENGI	NEER: DN	1H	CHECKED BY	DMH
	JOB I	NO.		DRAWN BY:	DMH
	DATE	: 5-17-	23		
	SHE	ET NUMBER	2		

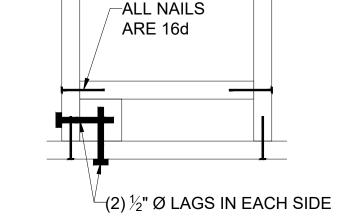
10 HEADER/BEAM CONNECTION OPTIONS AT OUTDOOR/OPEN SPACE S3.2 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)









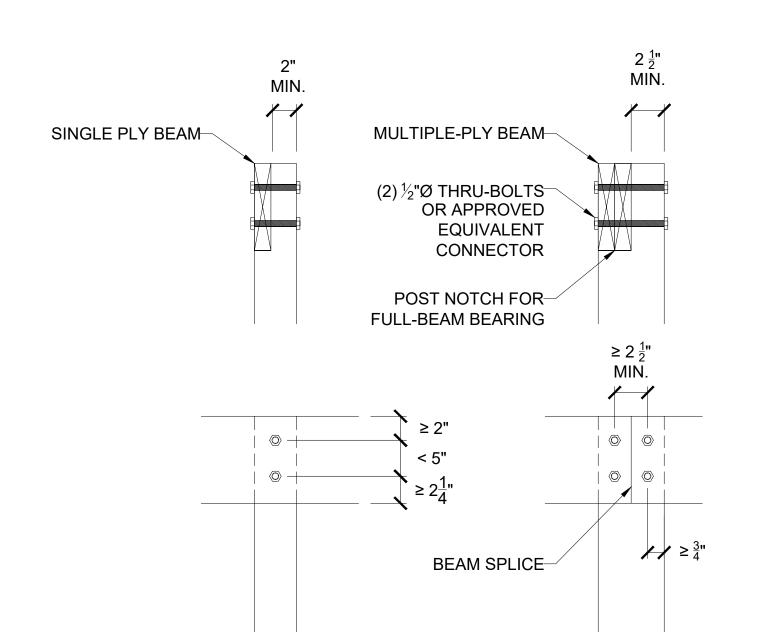




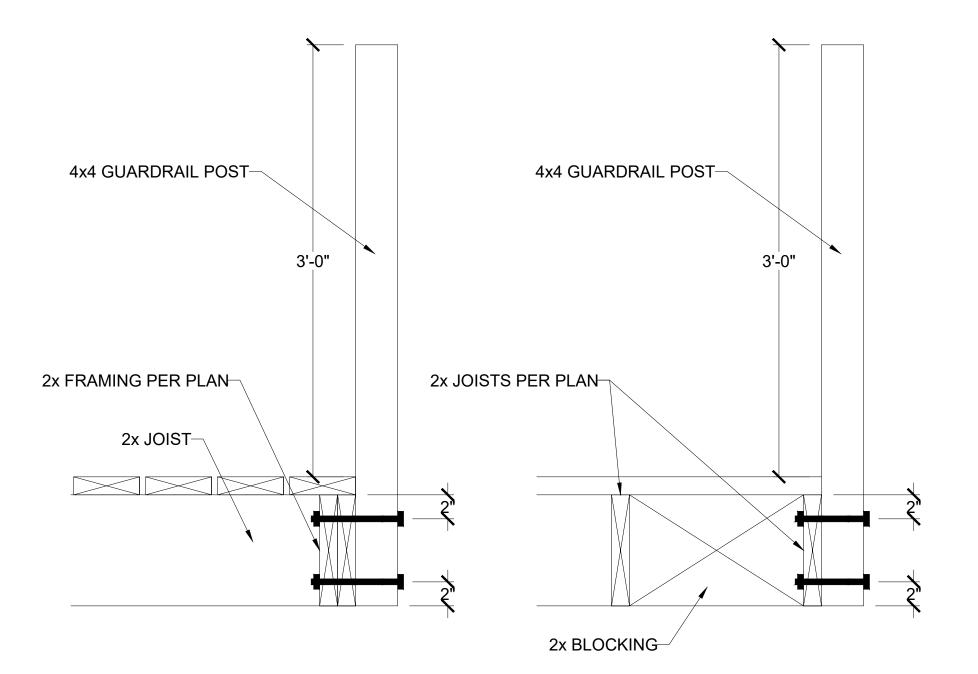
## DECK LEDGER ATTACHMENT GUIDE

DECK JOIST SPAN	$^{1\!\!/_{\!\!2}}$ " Ø GALV. LAG OR $^{3\!\!/_{\!\!8}}$ " Ø LEDGER-LOK SPACING
10'-0" OR LESS	16" OC
10'-0" - 13'-11"	12" OC OR @ 16" OC DOUBLED EVERY OTHER
14'-0" - 18'-0"	8" OC OR @ 16" OC DOUBLED

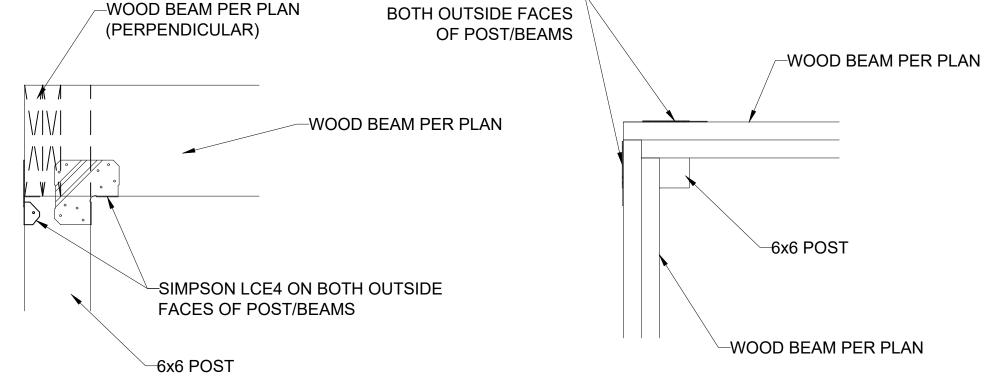
LEDGER ATTACHMENT 3.3 SCALE: 1" = 1'-0" (18x24) OR  $1\frac{1}{2}$ " = 1'-0" (24x36)



2 CANTILEVER WITH DECK ATTACHMENT 53.3 SCALE: 1" = 1'-0" (18x24) OR  $1\frac{1}{2}$ " = 1'-0" (24x36)



WITH (3) 16d NAILS STAGGERED



SIMPSON LCE4 ON-

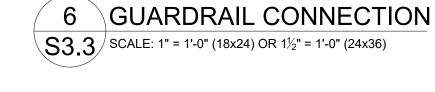
7 ALTERNATE COVERED DECK/PORCH INTERSECTION S3.3 CORNER BEAM CONNECTION

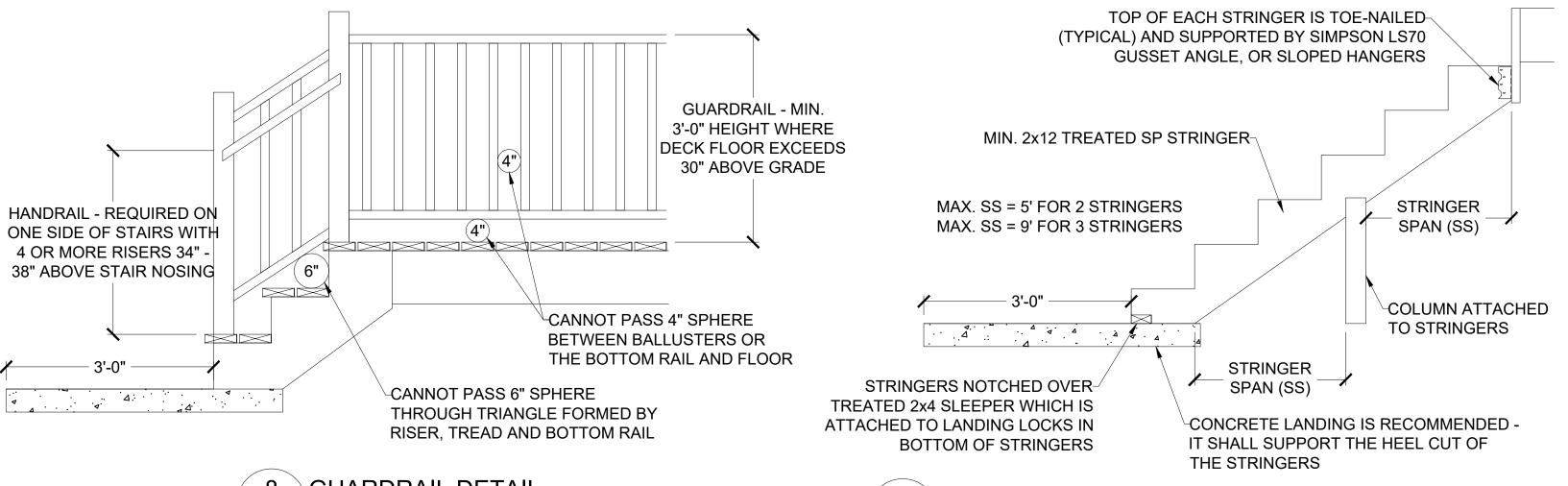
SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

3 DECK POST BASE

S3.3 SCALE: 1" = 1'-0" (18x24) OR  $1\frac{1}{2}$ " = 1'-0" (24x36)





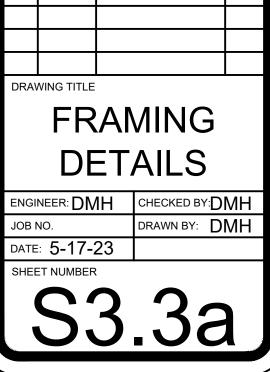


8 GUARDRAIL DETAIL 9 STAIR STRINGER DETAIL (MAX. 5' STAIR WIDTH) SCALE:  $\frac{1}{2}$ " = 1'-0" (18x24) OR  $\frac{3}{4}$ " = 1'-0" (24x36) S3.3 SCALE:  $\frac{1}{2}$ " = 1'-0" (18x24) OR  $\frac{3}{4}$ " = 1'-0" (24x36)

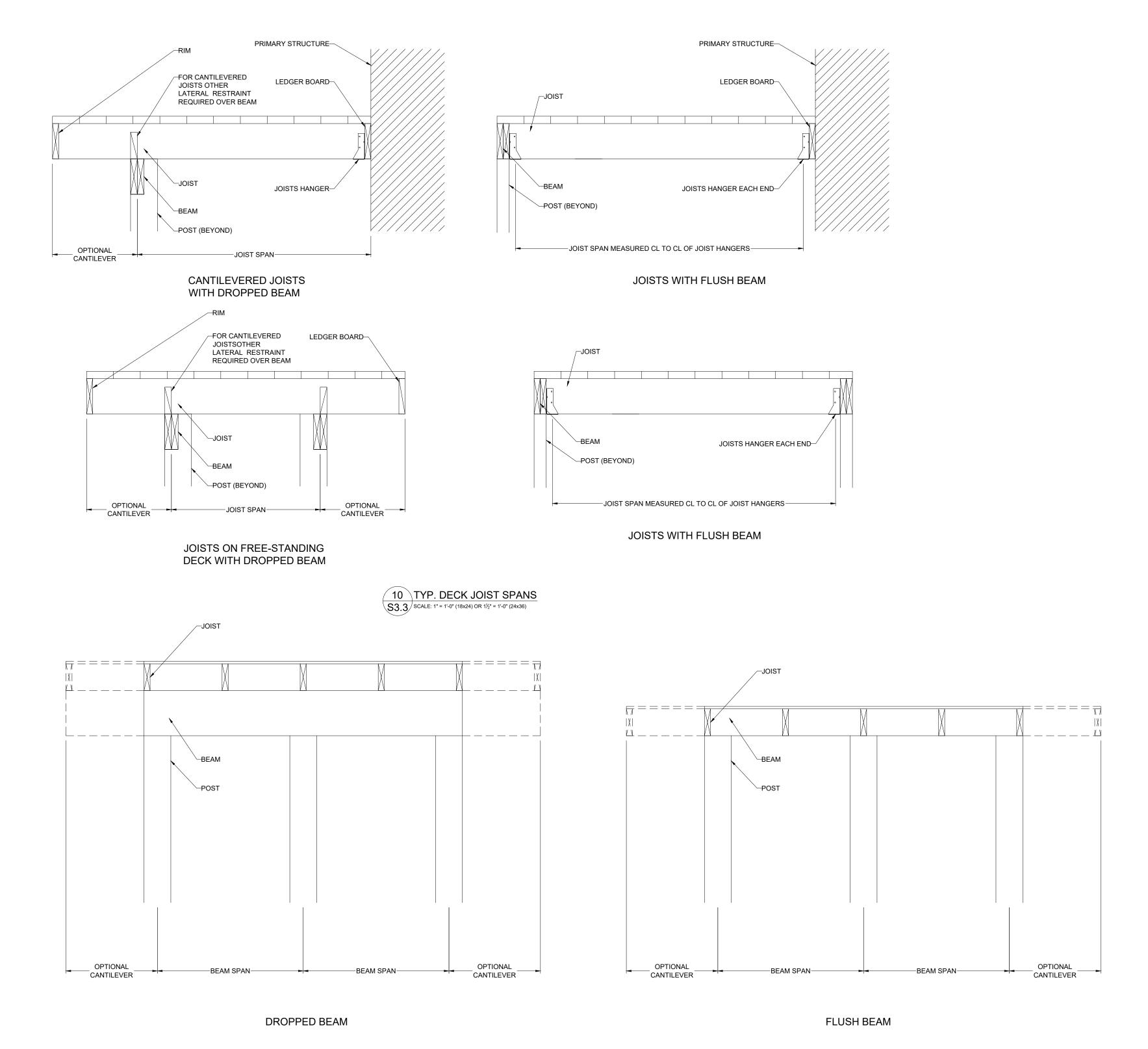
NO. DATE

23 LE





REVISION



11 TYP. DECK JOIST SPANS S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



VIEW FARMS - 4TH PLA

3 TITLE: SVF082 SPEC LOT 82, SUMMIT VI

JOB TITLE: SVI

ON THE PROPERTY OF THE PROPERT

NO.	DATE	REVISION	ВҮ
DBAM	VING TITLE		
		MING FAILS	
	IEED DIVIL	CHECKED BY	:DMH
ENGIN	NEER: DMH		
			DMF
JOB N		DRAWN BY:	DMH