affecting the work before proceeding. B. The contractor shall coordinate all disciplines, verifying size and location of all openings, whether shown on structural drawings or not, as called for on architectural, mechanical, or electrical drawings. In the case of work in an existing building the contractor shall scan existing structure to locate all rebar in the area of the new core/opening using ground penetrating radar and notify the engineer of record for review prior to coring/cutting. Conflicts, inconsistencies, or other difficulties affecting structural work shall be called to

C. All design and construction work for this project shall conform to the requirements of the following governing design codes: 1. International Building Code (IBC 2018) as amended by the City of Lee's

the architect or engineer's attention for direction before proceeding.

Summit, Missouri Minimum Design Loads for Buildings and Other Structures (ASCE7-16) 3. Specification for Structural Steel Buildings (AISC 360-16) Member Design Basis is Allowable Stress Design (ASD)

4. Structural Welding Code (AWS D1.4-17) Building Code Requirements for Structural Concrete (ACI 318-14) 6. Building Code Requirements for Masonry Structures (TMS 402-16) 7. North American Specification for the Design of Cold-Formed Steel

Connection Design Basis is Allowable Stress Design (ASD)

Structural Members (AISI S100-16) National Design Specification (NDS) for Wood Construction with 2018 upplements (ANSI/AWC NDS-2018)

9. Special Design Provisions for Wind and Seismic (AWC SDPWS-2015) D. These drawings are for this specific project and no other use is authorized.

2. Structural Design Load Criteria: A. Dead Loads:

= 35 psf Floor, Apartment = 55 psf Floor, Balcony Floor, Corridor = 30 psf = 25 psf Stair. Wood = 25 psf B. Live Loads: = 40 psfFloor, Apartment = 60 psfFloor, Balcony Floor, Corridor (Serving Apartment) = 40 psfFloor, Corridor (Serving Public) = 100 psfFloor, Public (Clubhouse) = 100 psf= 125 psf Floor, Storage = 20 psf Roof, MEP Equipment Zone = 45 psf = 100 psfC. Snow = Pg= 20 psf, Pf=14psf, Is = 1.0

Ce=1.0, Ct=1.0, Drift per ASCE/SEI 7-16 D. Lateral Loads:

. Wind V= 109 mph, exposure C Occupancy [Risk] Category II, lw=1.0 GCpi=+/-0.18 Design wind pressures to be used for the design of exterior

component and cladding materials on the designated zones of wall and roof surfaces shall be per section 30.7 and Table 30.7-2 of ASCE/SEI 7-16. Tabulated pressures shall be multiplied by effective area reduction factors, exposure adjustment factors, and topographic factors where applicable. 2. Seismic = Ss = 0.101g, S1 = 0.069g

Occupancy [Risk] Category II, le=1.0 Site Classification C; Sds=0.088g; Sd1=0.069g Seismic Design Category B Equivalent Lateral Force Procedure

> A.17 - Light framed walls with shear panels of all R = 2; Omega = 2.5; Cd = 2; V = 0.001W

A.2 - Ordinary Reinforced Concrete Shearwalls R = 4; Omega = 4; Cd = 2.5; V = 0.001W E. This project is designed to resist the most critical effects resulting from the load combinations of section 1605.3 of the 2018

International Building Code.

3. Concrete:

A. All concrete for foundations (grade beams, footings and piers) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 6 gallons of water per 100 pounds of cement and not over 4 inches of slump (excpet piers which shall have a 6"

B. All concrete for interior flat work and walls (including wall columns) shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 550 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.5 gallons of water per 100 pounds of cement and not over 4 inches of slump.

C. All concrete for exterior flatwork shall have a minimum design compressive strength of 4500 psi in 28 days, with not less than 560 pounds of cement per cubic yard of concrete, not over 5 gallons of water per 100 pounds of cement, with 6% +/- 1% air entrainment, and a maximum of 4 inches of slump. D. All concrete for elevated decks and columns shall develop a minimum

ultimate compressive design strength of 5000 psi in 28 days, but not less than 600 pounds of cement shall be used per cubic yard of concrete regardless of strength obtained, not over 5.5 gallons of water per 100 pounds of cement and not over 4 inches of slump.

E. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's dosage rates for improved workability.

F. The preceding minimum mix requirements may have up to 15% maximum of the cement content replaced with an approved ASTM C618 Class C fly ash, provided the total minimum cementitious content is not reduced. G. Combined aggregate (coarse plus fine) for all concrete shall be well graded from coarsest to finest with no more than 18 percent and not less than 8 percent retained on an individual sieve, except that less than 8 percent may be retained on coarsest sieve and on No. 50 and finer sieves. Submit this gradation report with the concrete mix design shop drawings.

H. All interior concrete slabs on grade shall be placed over 15 mil, Class A Vapor Barrier per ASTM E1745 with less than 0.01 perms, tested after mandatory conditioning. All joints shall be lapped and sealed per manufacturer's recommendations. All penetrations, as well as damaged vapor barrier material shall also be sealed per manufacturer's recommendation prior to concrete placement. Install barrier per manufacturer recommended details at all discontinuous edges (at interior columns, exterior edge of slab, etc.) to ensure terms of warranty are followed. The vapor barrier shall be placed over free-draining granular material as prescribed by the project soils report.

I. All concrete is reinforced concrete unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any details not shown shall be detailed per ACI 315 and meet requirements of ACI 318, current editions. Contractor shall verify that all concrete inserts, reinforcing and embedded

items are correctly located and rigidly secured prior to concrete placement.

K. No aluminum items shall be embedded in any concrete.

4. Reinforcing Steel:

A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel. Welded plain wire fabric shall be supplied in sheets and conform

to the requirements of ASTM A185. B. Clear minimum coverage of concrete over reinforcing steel shall be as follows: Concrete placed against earth: 3

Formed concrete against earth: 2" Beams or Columns: 1-1/2" Other

 All coverage shall be nominal bar diameter minimum. C. All dowels shall be the same size and spacing as adjoining main bars (splice lap 48 bar diameters or 24" minimum unless noted otherwise).

D. At corners of all walls, beams, and grade beams supply corner bars (minimum

2'-0" in each direction or 48 bar diameters) in outside face of wall, matching size and spacing of horizontal bars. Where there are no vertical bars in outside face of wall, supply 3 - #4 vertical support bars for corner bars. E. Bars marked continuous and all vertical steel shall be lapped 48 bar diameters (2'-0" minimum) at splices and embedments, unless shown otherwise. Splice

top bars near midspan and splice bottom bars over supports, unless noted

At all holes in concrete walls and slabs, add 2 - #5 bars (opening dimension plus 96 diameters long) at each of four sides and add 2 - #5 x 5'-0" diagonally at each of four corners of hole. Openings in 8" thick walls are reinforced similar, but with 1 - #5 instead of 2 - #5, respectively

G. Unless otherwise covered on architectural plans or specifications, vertical control joints in concrete wall shall be spaced at a maximum of 20'-0" on center and coordinated with the architect. Every other horizontal wall reinforcing bar shall be discontinuous at control joints except heavy top and bottom bars unless noted otherwise. Provide base seal waterstop style number 772 (by Greenstreak Inc. or approved equal) on dirt face side of wall at all walls below

H. Accessories shall be as specified in latest edition of the ACI Detailing Handbook and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet. All slabs and stairs not shown otherwise shall be 6" thick with #4 bars at 12" on center each way. All exterior porches and stoops not otherwise detailed may be constructed in any standard manner, solid or hollow, but must be reinforced with #4 bars at 12" on center each way minimum. Porches shall be doweled to adjacent walls or grade beams with #4 bars at 12" on center, hooked or

embedded 48 diameters into both members. Slope porches 1/8" per foot for

drainage unless noted otherwise. Allow 2 tons of reinforcing bars #4 or larger to be used as directed in the field for special conditions by the engineer of record (labor for placing same to be

. Structural Steel:

A. All structural steel beams and columns shall be ASTM A992, grade 50 steel and all miscellaneous steel shall be ASTM A36 grade steel. Hollow Structural Sections (HSS) shall be ASTM A500, grade B. Fabrication and erection shall be in accordance with AISC 303-05 "Code of Standard Practice for Steel Buildings and Bridges" in the 13th Edition of the AISC Steel Construction Manual. B. All welding shall conform to the recommendations of the AWS.

C. All exterior steel and connections, and brick relief angles shall be hot-dip D. All bolts not otherwise specified shall be 3/4" diameter high strength (ASTM

A325-N). All bolts shall be fully pretensioned. All beam connections shall be designed per the AISC Manual of Steel Construction "Framed Beam Connections" for the indicated reactions or at least 0.4 x beam total shear capacity, Vn/Omega, shown in the maximum total uniform load tables.whichever is greater; and, shall account for eccentricity when the bolt line is more than 2" from the center of the support. All connections must be two bolt minimum. Connection design and shop drawing preparation shall be completed under the direct supervision of a professional engineer licensed in the state the project is located and shop drawings and connection calculations shall bear his seal.

E. All anchor bolts shall be 3/4" diameter, ASTM F1554, Grade 36 unless noted otherwise. Washers of minimum size and thickness for the given anchor diameter in Table 14-2 of the AISC Steel Construction Manual shall be provided at every column anchor bolt. Washers shall have a standard size hole for the anchor bolt. At building perimeter columns and columns at braced frames washers shall be welded all around to the column base plate with 3/16" fillet

Handrails, guards and grab bars shall be designed to meet the requirements of the 2018 IBC. Refer to specifications for more explicit requirements. Submit structural calculations sealed by a lincensed engineer in the state of the project location.

6. Foundations:

A. The soil investigation was prepared by Terracon Consultants, Inc., and the project number is 02225094 and the telephone number is 913-492-7777. B. Spread footings, grade beams, and retaining walls are designed to bear on

undisturbed clay soil or geotechnical approved structural fill capable of safely

C. Retaining structures are designed for an active lateral load of 51 pcf equivalent fluid presssure and an at-rest lateral load of 72 pcf based on

geotechnical approved clay backfill. D. Contractor shall provide for dewatering at excavations from either surface

All concrete in the structural portion retaining the backfill shall have attained its design strength prior to being backfilled. All basement walls shall not be backfilled until the first floor slab or wood

decking is installed or the wall is temporarily braced by the contractor and the concrete has reached of its design strength. G. Moisture content in soils beneath building locations should not be allowed to change after footing excavations and after grading for slabs on grade are completed. If subgrade materials become desiccated or softened by water

content specified for engineered fill. Do not place concrete on frozen ground.

Concrete Masonry Units:

Concrete block used in exterior walls or load bearing walls shall meet the requirements of ASTM C90 and have a minimum net compressive strength of 2150 psi and laid up using type N mortar such that f'm equals 1500 psi. Mortar shall be volume proportion based cement lime mortar. Proportioning shall be completed by box measure. Any block in contact with earth shall be

normal weight units, laid using type "S" mortar and grouted solid. The contractor shall provide adequate temporary bracing for all masonry walls during construction. . All concrete block shall have 9 gage (or larger) horizontal joint reinforcing

(ladder or truss) per architectural drawings and specifications (16" maximum vertical spacing). . Cavity wall construction shall be reinforced as designed for specific concrete block used. The horizontal joint reinforcing shall be of the ladder or truss style per specification and continuous between brick and block, as

prescribed by the architectural drawings. E. Concrete block shall be reinforced as follows in 8" walls: 1. Vertical reinforcing shall be as indicated on S0.05, on center, at each corner, at each door and window jamb, each side of control joints and in the end void of each length of wall. Lap splices for masonry vertical

reinforcing shall be 48 bar diameters or 24" minimum. . Horizontal reinforcing: A. Horizontal joint reinforcing as noted above. B. Continuous horizontal bars shall be included per section or detail in bond beam or optional running bond beam where noted. Where bond beams are continuous at corners of walls, supply corner bars matching size of horizontal bars (minimum 2'-0" or 40 bar

diameters in each direction). Grout, where noted above, shall have a minimum design ultimate compressive strength of 2500 psi at 28 day test and 3/8" maximum aggregate size.

G. Unless otherwise covered on architectural plans or specifications, vertical control joints in masonry construction shall be 3/8" wide, full height of wall. Joints shall be spaced at a maximum of 24'-0" on center and coordinated with the architect. All horizontal joint reinforcing shall be discontinuous at control joints in masonry. All bond beam horizontal reinforcing shall be continuous through control joints.

Post-Installed Anchors:

A. Post-installed anchors shall be used only where specified on the drawings unless approved in writing by the engineer of record. See drawings for anchor diameter, spacing and embedment. Performance values of the anchors shall be obtained for specified products using appropriate design procedures and/or standards as required by the governing building code. Anchors installed in concrete shall have an ICC-ES Evaluation Service Report. Special inspection is required for all post installed anchors. The contractor shall coordinate an on-site meeting with the post installed anchor manufacturer field representative to educate the construction team on the anchor installation guidelines and requirements.

B. Mechanical anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ACI 355.2 and ICC-ES AC193. All anchors shall be installed per the anchor manufacturer's written instructions Adhesive anchors used in cracked and uncracked concrete shall have been

tested and qualified for use in accordance with ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions. Mechanical anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC01. All anchors shall be installed per the anchor manufacturer's written instructions.

Adhesive anchors used in solid grouted masonry shall have been tested and

qualified for use in accordance with ICC-ES AC58. All anchors shall be installed per the anchor manufacturer's written instructions. Anchors used in hollow concrete masonry shall have been tested and qualified in accordance with ICC-ES AC106 or ICC-ES AC58 as appropriate. All anchors shall be installed per the anchor manufacturer's written instructions with appropriate screen tubes used for adhesives.

9. Timber and Wood Framing:

Quality and construction of wood framing members and their fasteners for load supporting purposes not otherwise indicated on the drawings shall be in accordance with the 2018 International Building Code.

B. All studs and top and bottom plates shall be Douglas Fir No. 2 grade or Southern vellow pine No. 2 grade, visually graded, lumber, with an allowable fiber stress in bending of 900 psi minimum and an elastic modulus of 1,400,000 psi unless noted otherwise. All joist, truss members and headers to be No. 2 grade DF (unless noted otherwise) All lumber for exterior decks and balconies shall be preservative-treated Southern yellow pine No. 2 grade, visually grade unless noted otherwise. Bridging of stud bearing walls and shear walls shall be solid, matching

sheathing joints D. Joist blocking and bridging shall be solid wood or cross bridging of either wood or metal straps. Spacing, in any case, shall not exceed

E. Wood members and sheathing shall be fastened with number and size of fasteners not less than that set forth in Table 2304.9.1 of the 2018 International Building Code. Floor sheathing shall be APA rated tongue and groove Sturd-I-Floor, exposure 1, glued and nailed with 10d nails or # 10 screws at 6" on center to supports at edges and 12" on center field. Roof diaphragms shall be edge screwed with #10 screws at 6" on center and screwed to intermediate framing and/or blocking members with #10 screws at 12"on center unless noted on the drawings.

F. Sill plates shall be bolted to concrete slabs with 1/2" diameter bolts at 32" on center (UNO, Re: shearwall sched). Provide plate washers at sill plate anchors for shearwalls per shearwall sched. Plates in direct contact with concrete or masonry shall be treated lumber. G. All hangers, ties and connections shown are based on Simpson Strong

Tie as the basis of design, provide Simpson Strong Tie or an approved

equal. Joist hangers shall be equal to "LUS" for wood application and "LB" for steel weld-on application. Roof truss ties shall be equal to "H2.5A" and tie the roof truss to the top plate (provide 2) "H2.5A" Diagonally across from each other when uplift load shown in truss shop submittal exceeds 545 lbs). Roof girder ties shall be equal to a "LGT2". "LGT3" or "LGT4" tie (dependent on number of plies) and tie the truss girder to the top track. Provide "H2.5A" at the top of each stud to top plate when the top track has roof truss attached. Service condition - dry with moisture content at or below 19% in service.

Laminated strand lumber (LSL) shall have an allowable flexural stress (Fb) of 1,700 psi (reduced by size factor) and an elastic modulus (E) of 1,300,000 psi. J. Laminated veneer lumber (LVL) shall have an allowable flexural stress

(Fb) of 2,600 psi (reduced by size factor) and an elastic modulus (E) of 1,900,000 psi. K. Parallel Strand Lumber (PSL) shall have an allowable flexural stress (Fb) of 2,900 psi (reduced by size factor) and an elastic modulus (E) of

2,000,000 psi. ((E) = 2,200,000 psi for members > 18")Pre-engineered wood trusses shall be designed in accordance with the Truss Plate Institute's national design standard for metal-plate connected wood truss construction (ANSI/TPI-1 latest edition). Trusses shall be designed and manufactured by an authorized member of the Wood Truss Council of America (WTCA). Truss design shall conform to specified codes, allowable stress increases, deflection limitations and

other applicable criteria of the governing code. M. Shop drawings showing complete erection and fabrication details and calculations (including connections) shall be submitted to the project architect / engineer for review prior to fabrication and/or erection. Calculations shall bear the seal of a professional engineer, registered in the state of the project location. Shop drawings shall also be submitted to the local government controlling agency when requested by that

N. All trusses shall be securely braced both during erection and permanently, as indicated on the approved truss design drawings and in accordance with TPI's commentary and recommendations for handling, installing and bracing metal-plate connected wood trusses (HIB-91,

booklet) and the latest edition of ANSI/TPI-1. O. The truss manufacturer shall supply all hardware and fasteners for ioining truss members together and fastening truss members to their supports. Metal connector plates shall be manufactured by a member of the Wood Truss Council of America (WTCA) and shall be 20 gauge minimum. Connector plates shall meet or exceed ASTM A653, grade 33, with ASTM A924 galvanized coating designation G60.

P. Shipment, handling, and erection of trusses shall be by experienced, qualified persons and shall be performed in a manner so as not to endanger life or property. Apparent truss damage shall be reported to the truss manufacturer for evaluation prior to erection. Cutting or

alteration of trusses is not permitted. Q. Roof Truss Design criteria:

Top Chord Dead Load Top Chord Live Load = 20 psf. (Plus Rooftop Equipment) = 20 psf or 14 psf plus Drift Top Chord Snow Load Bottom Chord Dead Load = 10 psBottom Chord Live Load = 0 psf

Live Load Deflection = L/360Total Load Deflection = L/300 (1" MAX)Corridor Trusses Only = 250 # Point Load at any location on the top and bottom chord (nonconcurrent with each other or typical live load - concurrent with

MEP equipment)

R. Floor Truss Design Criteria: Top Chord Dead Load = 30 psfTop Chord Live Load = Per General Note 5B Bottom Chord Dead Load = 10 psf Live Load Deflection = L/480 (1/2" MAX)

Total Load Deflection = L/360 (3/4" MAX)S. Roof trusses shall be designed per IBC 2018 for net uplift resulting from wind loading based on component and cladding loading. Reference details 1, 2, 2A, 2B, 3, 3A, 3B, 4 and 5 on S0.20for net uplift holdown

detail and requirements. Refer to sheet S0.05 for wood shrinkage recommendations. U. Construction bracing shall be provided by the contractor as required to

keep the building and studs plumb. V. Structural members shall not be cut for pipes, etc., unless specifically detailed. Notching and boring of studs and top of plates shall conform to the provisions of section 2308.9.10 and 2308.9.11 of the IBC. Where top plates or sole plates are cut for pipes, ametal tie with minimum 0.058 inches thick and 1 1/2" inches wide shall be fastened to each plate across and to each side of the opening with not less than

(6) 16d nails, in accordance section 2308.9.8 of the IBC. W. All fasteners for wood to wood connections and wood connectors shall be as indicated in structural drawings or manufacturer literature to achieve full capacity of connector. Submittal must show that alternative will not reduce the capacity of the connection.

10. Shop Drawing Review:

A. Bob D. Campbell and Company, Inc. will review the General Contractor's (GC) shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to be a properly functioning integral element of the overall structural system designed by Bob D. Campbell and Company, Inc.

B. Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall: 1) Review each submission for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs incidental thereto, all of which are the sole responsibility of the GC.

Review and approve each submission. Stamp each submission as approved. C. Bob D. Campbell and Company, Inc. shall assume that no

submission comprises a variation unless the GC advises Bob D.

Campbell and Company, Inc. with written documentation.

D. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify the GC.

Structural steel shop drawings including erection drawings and piece details. Include connection submittals and miscellaneous framing.

Miscellaneous anchors shown on the structural drawings. Elevations of all reinforced concrete masonry walls at a scale of no smaller than 3/8"=1'-0" showing all required reinforcina Grout mix designs (for CMU). Construction and control joint plans and/or elevations

Wood truss desin and calculations and detailed erection and fabrication drawings. Wood shearwall holdown system. E. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has

met the above requirements. Bob D. Campbell and Company. Inc.

shall return without comment unrequired material or submissions

11.Structural Special Inspection:

without GC approval stamp.

official and structural engineer

A. The structural design for this project is based on completion of special inspections during construction in accordance with section 1704 of the International Building Code. The owner shall employ one or more qualified special inspectors to provide the required special inspections.

B. The special inspector shall furnish inspection reports to the building official, owner, architect and structural engineer, and any other designated person. C. All discrepancies shall be brought to the immediate attention of the contractor for correction, then, if uncorrected, to the proper design authority, building

D. The special inspector shall submit a final signed report stating that the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with the approved plans and specifications and the applicable workmanship provisions of the building code

E. The following inspections and tests are required with the frequency (continuous or periodic) as defined within the referenced section or standard listed below. The General Contractor shall provide notification to the inspector when items requiring inspection are ready to be inspected and provide access for those

2. Shop Fabrication – pre-engineered wood trusses per Section 1704.2.5 unless TPI certified shop 3. Steel Construction per Section 1705.2 and the quality assurance

requirements of AISC 341 Chapter J (as referenced by AISC 360)

1. Shop Fabrication – structural steel per Section 1704.2.5 unless AISC certified

4. Concrete Construction per Section 1705.3 and Table 1705.3 a. Reinforcing Steel Placement Reinforcing Steel Welding Cast in Place Anchors

d. Post Installed Anchors e. Design Mix Verification Concrete Sampling and Testing g. Concrete Placement Concrete Curing

Formwork Shape, Location and Dimensions 5. Masonry Construction per Section 1705.4 and the quality assurance requirements of TMS 402/ACI530/ASCE5 and TMS602/A530.1/ASCE6

6. Wood Lateral System (periodic) a. Wood shearwalls (include sheathing, rim board and bottom plate attachments) b. Portal frames

c. Shear wall and portal frame holdowns d. Shear wall tension rod system 7. Wood Gravity Framing and Placement (adjust frequency of random sampling

where indicated as required) a. Heavy timber/SCL/glulam beams and supports (periodic) b. Headers and jambs (random sampling) Bearing walls (random sampling)

d. Connector/hardware installation (random sampling) e. Floor and roof trusses (random sampling)

12. Structural Observation: A. The general contractor shall notify the engineer of record and allow for safe

B. The engineer of record shall be notified such that the following items can be a. Reinforcment at Elevated Concrete Floors/Roofs Reinforcement is placed and 48 hours (min) before a pour

access to the appropriate items requiring structural observation.

b. Structural Steel Framing Erection is completed, prior to covering steel c. Wood Framed Floors

or sheathing. C. At the discresion of the engineer of record a site obervation report will be issued to the general contractor and architect of record. D. The structural observations are performed at the discresion of the engineer of

record and are not required per the IBC for this project. 13. Copyright and Disclaimer:

copyrighted work of Bob D. Campbell and company, Inc. These drawings may not be photographed, traced, or copies in any manner without the written permission of Bob D. Campbell and Company, Inc. Exception: Original drawings may be printed for distribution to the owner, architect, and general contractor for coordination, bidding,

state for the structural design drawings consisting of S-series drawings. I hereby disclaim responsibility for all other drawings in other design professionals whose seals and signed statements may appear elsewhere in the construction document package.

LEGEND:

..... SPAN DIRECTION OF DECK - TYPE PER SCHED ON S0.01

BASE PLATE MARK - SEE SCHEDULE ON SHEET S0.04 BEAM OR HEADER PER SCHED ON S0.02

(A#-#U) UPSET BEAM OR HEADER PER SCHED ON S0.02 BEARING WALL TYPE PER SCHED ON S0.02 SHEARWALL HOLDDOWN TYPE PER SCHED ON S0.03 NUMBER OF RESPECTIVE JACK/KING STUDS IN A STUD PACK. REFER TO DETAIL 6 ON S1.11 NUMBER OF WALL STUDS IN STUD PACK EQUAL TO

ALL LOCATIONS WITHIN A PILASTER INDICATED PLAN NOTE PER SCHEDULE ON PLAN SHEET WHERE INDICATED

SHEARWALL TYPE PER SCHED ON S0.03

ROUND, DIAMETER FTG **FOOTING** FIELD VERIFY **ADDITIONAL** ABOVE FINISHED FLOOR GAGE **ALTERNATE** GALV GALVANIZE(D) ARCHITECTURAL GEN BUILDING GR HORIZ **BOTTOM OF**

AND

BOTTOM

BEARING

CAMBER

COLUMN

COVER

CENTER

DOUBLE

DETAIL

DIAMETER

DIMENSION

DEAD LOAD

EACH FACE

ELEVATION

ENGINEER

EQUIPMENT

EACH WAY

EXPANSION

EXTERIOR

FOUNDATION

EXISTING

FAR FACE

FINISH

EQUAL

EXPANSION JOINT

EDGE OF DECK

EDGE OF SLAB

FLOOR DECK TYPE

EMBEDMENT, EMBEDDED

ENGINEER OF RECORD

DRAWING

CONCRETE

CONNECTION

CONTINUOUS

COORDINATE

CENTERLINE

CONCRETE DECK TYPE

CONSTRUCTION/CONTROL JOIN

CONCRETE MASONRY UNIT

ADTL

ARCH

BLDG

BM

BOTT

CD-#

CMU

COL

CONC

CONN

CONT

CTR

DBL

DET

DWG

EL, ELEV

EMBED

ENGR

EOD

EOR

EOS

EQUIP

EXP

EXT

FD-#

FDN

FIN

EXTG, EXIST

COORD

COV, CVF

GENERAL GRADE HORIZONTAL HSS **HOLLOW STRUCTURAL SECTION** INSIDE FACE ISOLATION JOINT **INFORMATION** INTERIOR JOIST COMPLETE JOINT PENETRATION **JOINT** KIPS (1000 LBS) KIPS PER SQUARE FOOT KSF KIPS PER SQUARE INCH LBS, # POUNDS **DEVELOPMENT LENGTH** LIVE LOAD LONG LEG HORIZONTAL LLH LLV LONG LEG VERTICAL LONG LONGITUDINAL LSLT LONG-SLOTTED HOLE TRANSVERSE LTWT LIGHTWEIGHT MOMENT FORCE MAX MAXIMUM MECH **MECHANICAL MFGR** MANUFACTURER MINIMUM MISC **MISCELLANEOUS** MSRY **MASONRY** MTL METAL NEAR FACE **NEAR SIDE** NTS NOT TO SCALE

OPNG

OVS

PAF

PCF

PEMB

PERP

STRUCTURAL ABBREVIATIONS

FLOOR

FAR SIDE

NORMAL WEIGHT ON CENTER OUTSIDE FACE OPENING **OPPOSITE OVERSIZED HOLE** AXIAL FORCE POWDER ACTUATED FASTENER PRECAST POUNDS PER CUBIC FOOT PRE-ENGINEERED METAL BUILDING PERPENDICULAR

SQUARE STAINLESS STEEL SS SSLT SHORT-SLOTTED HOLE TRANSVERSE STD STANDARD STIFFENER STIFF STIR STIRRUP STL STEEL STRUCT STRUCTURE, STRUCTURAL SHEARWALL TOP OF THROUGH TOF TOP OF FOOTING TOS TOP OF STEEL, TOP OF SLAB **TRANS TRANSVERSE** TYPICAL **UNLESS NOTED OTHERWISE** UNO SHEAR FORCE **VERT** VERTICAL WITH W/0 WITHOUT WIDE FLANGE

PLATE

QUANTITY

REFERENCE

REQUIRED

REVISION

SECTION

SIMILAR

SPACING

SUPPORT

SAW JOINT

SNOE LOAD

SLAB-ON-GRADE

SPECIFICATION

SLAB-ON-GRADE TYPE

ROOF DECK TYPE

REINFORCEMENT

ROOF LIVE LOAD

SLIP CRITICAL

SCHEDULE(D)

SHEET

ROOF TOP UNIT

RADIUS

PJP

PSF

PSI

QTY

RAD

RD-#

REF

REINF

REQD

REV

RLL

RTU

SCHED

SECT

SHT

SIM

SOG-#

SPCG

SPEC

SPRT

W/

WP

Sheet Name

WOOD SHEARWALL SCHEDULES &

TYPICAL WOOD DETAILS &

SCHEDULES

DETAILS

PLAN

S2.12 CLUBHOUSE ROOF FRAMING PLAN

S3.30 WOOD FLOOR FRAMING SECTIONS

S3.31 WOOD FLOOR FRAMING SECTIONS

S3.40 WOOD ROOF FRAMING SECTIONS

S3.41 WOOD ROOF FRAMING SECTIONS

S3.42 WOOD ROOF FRAMING SECTIONS

S3.43 WOOD ROOF FRAMING SECTIONS

S3.44 CLUBHOUSE ROOF FRAMING

S3.45 CLUBHOUSE ROOF FRAMING

S4.00 DETACHED GARAGE PLANS &

S4.01 DETACHED GARAGE PLANS &

S4.02 | CARPORT PLANS & SECTIONS

S4.03 TRASH ENCLOSURE PLANS &

SECTIONS

SECTIONS

SECTIONS

SECTIONS

SECTIONS

S3.00 FOUNDATION SECTIONS

S3.01 | FOUNDATION SECTIONS

S3.02 | FOUNDATION SECTIONS

S3.03 FOUNDATION SECTIONS

S3.20 STEEL FRAMING SECTIONS

S3.10 | CONCRETE SECTIONS

WWF

SJ

POUNDS PER LINEAR FOOT

PARTIAL JOINT PENETRATION

POUNDS PER SQUARE FOOT

POUNDS PER SQUARE INCH

WIND LOAD **WORK POINT** WELDED WIRE FABRIC Current Current

Revision

Revision

Date

Sheet Number GENERAL NOTES

DESCRIPTION 3/4" GYPCRETE ATOP 23/32" T&G APA-RATED STURD-I FLOOR, EXP I SHEATHING. SHEATHING SHALL BE GLUED AND NAILED W/ 8d RING SHANK NAILS OR #10 SCREWS @ 6"o.c. @ EDGES & 12"o.c. AT FIELD 3" CONCRETE SLAB (4000psi) REINFORCE WITH 6x6 - W2.9xW2.9 WWF ATOP WATERPROOFING MEMBRANE (RE: ARCH.) ATOP 15/32" EXT. GRADE PLYWOOD SHEATHING SLOPE TO DRAIN PER ARCH. RE: NOTE 6 BELOW.

4" CONC. SLAB (4500psi, AIR ENTRAINED)

 Floor/Roof is erected, prior to MEP system routing MEP routing is completed and all hardward installed - prior to insulating

A. All drawings in the structural set (S-series drawings) are the

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HSS 6x6x1/4 COLUMN SIZE

SLAB SCHEDULE

STRUCTURAL DECK &

6" (MIN) CONCRETE SLAB (4,500psi, AIR-ENTRAINED) REINFORCE WITH #4 @ 12"oc LONGITUDINAL ATOP #4 @ 12"oc TRANSVERSE BOTTOM. T/SLAB EL. = 4" CONC. SLAB (4000psi) REINFORCE WITH 6x6-W2 9xW2 9 WWF ATOP 15 mil

SPECIFICATIONS. T/SLAB EL. = PER PLAN, SLOPE

SUITABLE SUBGRADE MATERIAL PER GEOTECH

KING & JAMB STUDS FROM HEADER ABOVE - TYP @

VAPOR BARRIER ATOP 6" OF 3/4" CLEAN GRANULAR LEVELING COURSE, ATOP SUITABLE SUBGRADE MATERIAL PER GEOTECH SPECIFICATIONS. T/SLAB EL. = PER PLAN, SLOPE TO DRAIN

REINFORCE WITH 6x6-W2.9xW2.9 WWF ATOP 6" OF 3/4" CLEAN GRANULAR LEVELING COURSE, ATOP SUITABLE SUBGRADE MATERIAL PER GEOTECH

6" CONC. SLAB (4500psi, AIR ENTRAINED) REINFORCE WITH #4 @ 12"oc EA WAY ATOP 6" OF 3/4" CLEAN GRANULAR LEVELING COURSE, ATOP

SPECIFICATIONS. T/SLAB EL. = PER PLAN 19/32" APA-RATED, EXP I SHEATHING ATTACHED WITH #10 SCREWS @ 6"o.c. AT EDGES & 12"o.c. AT

FIELD. (ATTACH WITH #8 SCREWS AT SAME SPACING AT FLAT ROOF AREAS) 1. FD = FLOOR DECK TYPE. 2. CD = CONCRETE DECK TYP.

FILL JOINT WITH SEALANT.

3. SOG = SLAB-ON-GRADE TYP. 4. RD = ROOF DECK TYP. 5. PROVIDE 1" DEEP TOOLED CONTROL JOINT (TRANSVERSE DIRECTION) AT MID-SPAN OF SINGLE BAY BALCONY OR AT THIRD POINTS OF DOUBLE BALCONY (8'-0" MAX SPACING).

CONCRETE FOOTING SCHEDULE BRG PRESSURE (PSF): CONCRETE (PSI): REBAR (KSI):

2.500 FOOTING SIZE (FT.) QTY/SIZE OF BARS THICKNESS (IN.) #4 @ 6"oc EA WAY BOTTOM 3'-0" x 3'-0" x 18" $\langle 3A \rangle$ 3'-0" x 3'-0" x 32" #4 @ 6"oc EA WAY TOP & BOTTOM 3/6 #4 @ 6"oc EA WAY TOP & BOTTOM 3'-0" x 6'-0" x 32" 4 #4 @ 6"oc EA WAY BOTTOM 4'-0" x 4'-0" x 18" $\langle 4A \rangle$ $\langle 4x2 \rangle$ 4'-0" x 4'-0" x 32" #4 @ 6"oc EA WAY TOP & BOTTOM 4'-0" x 2'-0" x 12" #4 @ 6"oc EA WAY BOTTOM 5'-0" x 5'-0" x 18" #4 @ 6"oc EA WAY BOTTOM

6'-0" x 6'-0" x 18" #4 @ 6"oc EA WAY BOTTOM 1) EXTERIOR FOOTINGS OR FOOTING AT GRADE BEAM SHALL MATCH GRADE BEAM DEPTH AND BE PLACED WITH GRADE BEAM. PROVIDE SPECIFIED REBAR TOP AND BOTTOM WITH 4 STANDEES TO SUPPORT MATS

2) CENTER FOOTINGS ON COLUMNS AND/OR WALL CENTER LINES PER PLAN, U.N.O.

3) SPREAD FOOTINGS LOCATED AT INTERIOR SHALL BE POURED MONOLITHIC W/

THE SLAB AS A THICKENED PORTION OF SLAB UNLESS THEY HAVE A STEEL COLUMN BEARING ATOP. 4) SPREAD FOOTINGS LOCATED AT INTERIOR WITH STEEL COLUMNS BEARING ATOP SHALL BE LOCATED AT 99'-0".

STEEL SCHEDULES WOOD SHRINKAGE & MOVEMENT CONCRETE SCHEDULES S0.20 TYPICAL WOOD DETAILS TYPICAL WOOD DETAILS S0.22 TYPICAL WOOD DETAILS S1.00 STAIR 1 FRAMING PLANS S1.01 STAIR 2 FRAMING PLANS S1.10 WOOD STAIR FRAMING SECTIONS S1.11 | ELEVATOR FRAMING SECTIONS S1.20 BALCONY FRAMING PLANS S1.21 BALCONY FRAMING PLANS S1.30 WOOD BALCONY FRAMING SECTIONS S2.00 LOWER LEVEL FOUNDATION PLAN S2.01 1ST FLOOR FRAMING & FOUNDATION PLAN S2.02 2ND FLOOR FRAMING PLAN S2.03 3RD FLOOR FRAMING PLAN S2.04 4TH FLOOR FRAMING PLAN S2.05 ROOF FRAMING PLAN S2.06 SHEARWALL PLAN S2.10 CLUBHOUSE FOUNDATION PLAN S2.11 CLUBHOUSE 1ST FLOOR FRAMING

DRAWING RELEASE LOG

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 \triangle REVISIONS:

DATE: 3/24/2023 JOB NO. 696521 DRAWN BY: CAB/JLF

CONNECTION

JOIST TO SILL OR GIRDER

SOLE PLATE TO JOIST OR

BRIDGING TO JOIST

BLOCKING

6 DOUBLE STUDS

4 TOP PLATE TO STUD

5 STUD TO SOLE PLATE

7 DOUBLED TOP PLATES

AND INTERSECTIONS

10 RIM JOIST TO TOP PLATE

TOP PLATE LAPS AND

CONTINUOUS HEADER,

13 CEILING JOISTS TO PLATE

CONTINUOUS HEADER

CEILING JOISTS TO

PARALLEL RAFTERS

1" BRACE TO EACH STUD

BUILT-UP CORNER AND

BUILT-UP GIRDER AND

BUILT-UP LAMINATED

23 RIM BOARD TO TRUSS

BUILD-UP STUD-PACK

3.) NAILING DESIGNATION:

4 - 3" x 0.131" NAILS

VENEER LUMBER BEAMS

CEILING JOISTS, LAPS OVER

INTERSECTIONS

TWO PIECES

TO STUD

PARTITIONS

17 RAFTER TO PLATE

AND PLATE

22 2" PLANKING

COLUMNS

ATTACHMENT

AT THIS FACE

EDGE DIST.

PER SCHED.

MULTIPLE STUDS

DOUBLE TOP PLATE LAPS

BLOCKING BETWEEN JOISTS

OR RAFTERS TO TOP PLATE

NAILING SCHEDULE (REFER TO NOTES #1 and #2)

3-3" x 0.131" NAILS-TOENAIL

3" x 0.131" NAILS AT 8"o.c.-

4-3" x 0.131" NAILS AT 6"o.c.

3- 3" x 0.131" NAILS-END NAIL

BRACED WALL PANELS

0.131" NAILS-END NAIL

12-3" x 0.131" NAILS

3-3" x 0.131" NAILS -TOENAIL

3- 3" x 0.131" NAILS-FACE NAIL

5- 3" x 0.131" NAILS-TOENAIL

4-3" x 0.131" NAILS-TOENAIL

4- 3" x 0.131" NAILS-FACE NAIL

4-3" x 0.131" NAILS-FACE NAIL

2-3" x 0.131" NAILS-FACE NAIL

3" x 0.131" NAILS AT 24"o.c. FACE NAILED

OPPOSITE SIDES 3- 3" x 0.131" NAILS AT

TOP AND BOTTOM STAGGERED ON

3" x 0.131" NAILS AT 6"o.c. TOP AND

4-3" x 0.131" NAILS AT EACH SUPPORT

2- 3" x 0.131" FACE NAILS (IT/IB @ EA.

1.) ALL NAILS SHALL BE AS NOTED UNLESS OTHERWISE SPECIFIED ON STRUCTURAL DRAWINGS OR

2.) CONDITIONS NOT SPECIFIED SHALL BE IN ACCORDANCE WITH CURRENT INTERNATIONAL BUILDING CODE

3- 3" x 0.131" NAILS-TOENAIL

3" x 0.131" NAILS AT 16"o.c.

ENDS AND EACH SPLICE

BOTTOM ALONG EDGE

REFER TO DETAIL 5/S0.02

- ATTACHMENT

OPPOSITE FACE

5A <u>DETAIL</u>

ALTERNATE PROVIDED BY ENGINEER IN WRITING.

— NAIL LENGTH

— QUANITY

3" x 0.131" NAILS AT 6"o.c.-TOENAIL

3" x 0.131" NAILS AT 10"o.c. ALONG EACH

TYPICAL FACE NAIL

2- 3" x 0.131" NAILS-TOENAIL EACH END

4- 3" x 0.131" NAILS-TOENAIL OR 3- 3" x

3" x 0.131" NAILS AT 8"o.c.-FACE NAIL

3" x 0.131" NAILS AT 12"o.c.-FACE NAIL

ATTACHMENTS

(REF NOTE #3 and #4)

3-8d NAILS-TOENAIL

MAX. FACE NAILING

BRACED WALL PANEL

2-16d NAILS-END NAIL

3-8d NAILS-TOENAIL

2-16d NAILS-FACE NAIL

16d NAILS AT 16"o.c. MAX. ALONG EACH EDGE-TOENAIL

3-8d NAILS-TOENAIL

4-8d NAILS-TOENAIL

3-16d NAILS-FACE NAIL

3-16d NAILS-FACE NAIL

3-8d NAILS-TOENAIL

2-8d NAILS-FACE NAIL

16d NAILS AT 24"o.c. MAX.

ENDS AND EACH SPLICE

BOTTOM ALONG EDGE

20d NAILS AT 32"o.c. MAX. TOP

AND BOTTOM, STAGGERED ON

OPPSITE SIDES. 2-20d NAILS AT

16d NAILS AT 12"o.c. TOP AND

16d NAILS AT EACH SUPPORT

2- 10d NAILS - FACE NAILS

REFER TO DETAIL 5/S0.02

(IT/IB @ EA. TRUSS)

NAILS-END NAIL

FACE NAIL

FACE NAIL

8-16d NAILS

2-8d NAILS-TOENAIL EACH END

16d BOX NAILSZ AT 16"o.c.

3-16d BOX NAILS AT 16"o.c.

4-8d NAILS-TOENAIL OR 2-16d

16d BOX NAILS AT 24"o.c. MAX.

16d BOX NAILS AT 16"o.c. MAX.

8d NAILS AT 6"o.c. MAX.-TOENAIL

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DATE:

3/24/2023

JOB NO.

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6" APART UNDERNEATH. DESIGN EACH JAMB TRUSS PER GENERAL NOTES ON S0.01 PLUS POINT LOAD OF DL=4,600lb, LL=3,850lb FOR EACH JAMB PRE-ENGINEERED ROOF TRUSSES @ 24"oc MAX

(C1) 2x10 @ 16"oc

ABOVE. ALL LOADS UNFACTORED.

ABOVE. ALL LOADS UNFACTORED.

2x8 @ 16"oc

8x8 WOOD COLUMN ATOP SIMPSON ABU88Z W/ (2) 5/8"Øx6"Lg SIMPSON TITEN HD SCREW ANCHORS UPSET (3) 2x10 BEAM w/ (2) KING STUDS & LSTA12 TWIST STRAP, EA END

(TO KING STUDS) UPSET (2) 2x12 BEAM w/ (2) KING STUDS & LSTA12 TWIST STRAP, EA END (TO KING STUDS) UPSET (3) 1 3/4"x9 1/4" LVL BEAM w/ (3) KING STUDS & LSTA12 TWIST STRAP

PLAN NOTES

18" DEEP PRE-ENGINEERED FLOOR TRUSSES @ 16"oc MAX. ALIGN WALL

STUDS ABOVE w/ FLOOR TRUSSES. TRUSS DESIGNER TO DESIGN TRUSS

1,000lb FOR OFFSET WALL ABOVE. AT JAMB ABOVE, PROVIDE (2) TRUSSES

6" APART UNDERNEATH. DESIGN EACH JAMB TRUSS PER GENERAL NOTES

PER GENERAL NOTES ON S0.01 PLUS POINT LOAD OF DL = 1,800lb, LL=

ON S0.01 PLUS POINT LOAD OF DL=3,000lb, LL=1,500lb FOR EACH JAMB

18" DEEP PRE-ENGINEERED FLOOR TRUSSES @ 16"oc MAX. ALIGN WALL

STUDS ABOVE w/ FLOOR TRUSSES. TRUSS DESIGNER TO DESIGN TRUSS

2,200lb FOR OFFSET WALL ABOVE. AT JAMB ABOVE, PROVIDE (2) TRUSSES

PER GENERAL NOTES ON S0.01 PLUS POINT LOAD OF DL = 2,600lb, LL=

18" DEEP PRE-ENGINEERED FLOOR TRUSSES @ 24"oc MAX

UPSET (3) 1 3/4"x11 1/4" LVL BEAM w/ (3) KING STUDS & LSTA12 TWIST STRAP, EA END (TO KING STUDS)

UPSET (3) 1 3/4"x18" LVL BEAM w/ (4) KING STUDS OR (3) 2x6 KING STUDS @ STUD WALL (MATCH WALL STUD SIZE)

UPSET (4) 1 3/4"x18" LVL BEAM w/ (6) 2x4 KING STUDS (3 IN EA WALL) @ DBL WALL OR HHGU7.25-SDS (H=18") HANGER @ BEAM CONNECTION

UPSET (4) 1 3/4"x18" LVL BEAM w/ (5) 2x6 KING STUDS EA END UPSET (3) 1 3/4"x18" LVL BEAM w/ (8) 2x4 KING STUDS EA END U.N.O. ON UPSET (3) 1 3/4"x18" LVL BEAM w/ (8) 2x4 KING STUDS EA END U.N.O. ON PLAN (4 IN EA SIDE OF DBL WALL). EXTEND BEAM TO BEAR FULLY ACROSS

UPSET (2) 1 3/4"x18" LVL BEAM w/ (3) KING STUDS @ STUD WALL OR HUC416 HANGER @ BEAM CONNECTIONS

GROUT VOID OF BASE SOLID w/ 6,000psi NON-SHRINK GROUT PRIOR TO

UPSET (2) 1 3/4"x18" LVL BEAM w/ (3) KING STUDS @ STUD WALL OR HGUS414 HANGER @ BEAM CONNECTIONS

UPSET (4) 1 3/4"x18" LVL BEAM

EA END (TO KING STUDS)

UPSET (4) 1 3/4"x14" LVL BEAM 7"x7" PSL COLUMN w/ ABW7-7Z POST BASE & CCQ6-7.13SDS2.5 POST CAP.

COLUMN INSTALLATION 3 1/2"x9 1/4" PSL COLUMN BTWN WALL PLATES (RE: 5/S0.21)

(2) 5 1/4"x5 1/4" PSL COLUMNS w/ ABU65 POST BASE & CCQ7.1-6SDS2.5

ALIGN (2) JOISTS w/ CL OF DBL SHEARWALL & PROVIDE TENSION TIE PER 8A/S3.02

SIMPSON HUCQ412-SDS HANGER

SIMPSON HHUS410 HANGER

ALUMINUM BOLT-ON BALCONY PER DEFERRED SUBMITTAL. PROVIDE ADDT'L FRAMING PER 8 & 8A ON S1.30

UPSET (3) 1 3/4"x18" LVL OUTRIGGER w/ (4) KING STUDS @ EXTERIOR BRG WALL & BACKSPAN OF BEAM. NOTCH OUTRIGGER @ CANTILEVERED TRANSITION PER TO MATCH ROTATED JOIST FRAMING PER 4/S3.31 (DO NOT OVERCUT

(3) 1 3/4"x7 1/4" LVL w/ SIMPSON HUC68 (MAX) EA END (3) 1 3/4"x7 1/4" x 16'-0" OUTRIGGER w/ (3) KING @ EXT WALL

PERMIT SET

	FLOOR/ROOF FRAMING HEADERS/BEAMS SCHEDULE								
	HEADER								
MARK		" 5 " (1st FLR FRAMING, TYP. U.N.O.)	" 4 " (2nd FLR FRAMING, TYP. U.N.O.)	" 3 " (3rd FLR FRAMING, TYP. U.N.O.)	" 2 " (4th FLR FRAMING, TYP. U.N.O.)	" 1 " (ROOF FRAMING, TYP. U.N.O.)	NOTES		
B1-#	(2) 2x10	HUC210-2 HANGER w/ (2) KING		REFER TO DTLS 10 & 10A ON S3.30					
C1-#	(2) 2x12					1 JACK / 1 KING	AT GARAGE HEADER w/ (2) BEARING STUDS EA END. EXTEND HEADER ACROSS FULL GARAGE BAY & BEAR PER 8/S4.00		
D1-#	(3) 2x8	1 JACK / 1 KING	1 JACK / 1 KING						
D2-#	(3) 2x8	1 JACK / 2 KING	1 JACK / 2 KING	1 JACK / 1 KING	1 JACK / 1 KING	1 JACK / 1 KING			
D3-#	(3) 2x8	1 JACK / 2 KING		REFER TO DTLS 10 & 10A ON S3.30					
E1-#	(3) 2x10			4 KING	3 KING	1 JACK / 1 KING			
E2-#	(3) 2x10					2 JACK / 1 KING			
F1-#	(3) 2x12	2 JACK / 2 KING	1 JACK / 2 KING	1 JACK / 2 KING	1 JACK / 1 KING	1 JACK / 2 KING			
J1-#	(3) 1 3/4"x7 1/4" LVL				3 KING				
L1-#	(3) 1 3/4"x9 1/4" LVL	3 KING				1 JACK / 2 KING	REFER TO DTLS 10 & 10A ON S3.30		
M1-#	(3) 1 3/4"x11 1/4" LVL	2 JACK / 2 KING					AT GARAGE HEADER w/ (2) BEARING STUDS EA END. EXTEND HEADER ACROSS FULL GARAGE BAY & BEAR PER 8/S4.00		
N1-#	(4) 1 3/4"x11 1/4" LVL					1 JACK / 2 KING	REFER TO DTL 8 ON S4.00		
T1-#	(3) 1 3/4"x18" LVL		2 KING	2 KING					

FLOOR/ROOF FRAMING HEADERS/REAMS SCHEDLILE

OVER-CUT STUDS WITHOUT PRIOR APPROVAL BY EOR 5. SLOT HOLES VERTICALLY FOR SHRINKAGE ALLOWANCE.

CENTER HOLES IN STUD

(VERTICALLY SLOT FOR

(MAX BORED HOLE Ø IS

(MAX NOTCH DEPTH IS

40% OF STUD WIDTH)

60% OF STUD WIDTH)

2x4 = 2 1/8" Ø MAX

2x6 = 3 1/4" Ø MAX

- 2x4 = 1 7/16" Ø MAX

 $2x6 = 2 \frac{3}{16}$ Ø MAX

NON-BEARING PARTITION WALL

SHRINKAGE ALLOWANCE)

ALLOWABLE HOLES/NOTCHES IN WALL STUDS

5/8" MIN TO

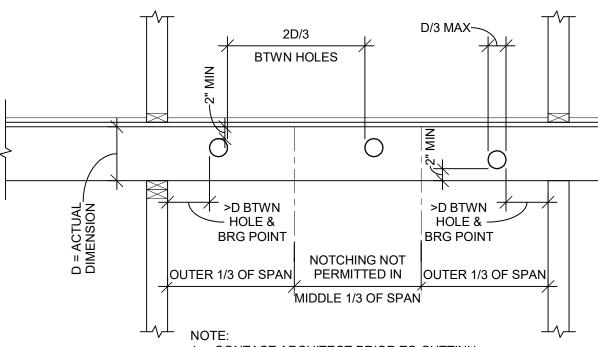
HOLES SHALL NOT BE LOCATED IN THE SAME STUD AS A CUT OR NOTCH

2. CONTACT ARCHITECT PRIOR TO CUTTING OR NOTCHING TO VERIFY SIZE AND LOCATION IF HOLE IS GREATER THAN 20% STUD WIDTH OR NOTCHES GREATER

THAN 10% STUD WIDTH ARE REQUIRED IN TWO OR MORE CONSECUTIVE STUDS 3. NOTCHES OR HOLES NOT PERMITTED IN JAMBS, STUD PACKS AND AT ENDS OF

STUD SHOES ARE NOTE AN ACCEPTABLE REMEDIATION OF OVER-NOTCHED OR

STUD FACE



CENTER HOLES IN STUD

2x4 = 1 3/8" Ø MAX

 $2x6 = 2 \frac{1}{8}$ Ø MAX

STUDS ALLOWED)

(MAX NOTCH DEPTH IS

25% OF STUD WIDTH)

2x6 = 1 3/8" Ø MAX

EXTERIOR OR BEARING WALL

SHEARWALLS

(MAX BORED HOLE Ø IS 40% OF

IF STUD IS DOUBLED NO MORE

STUD WIDTH)(60 % MAX ALLOWABLE

THAN TWO SUCCESSIVE DOUBLED

1. CONTACT ARCHITECT PRIOR TO CUTTINH JOISTS TO VERIFY SIZE AND LOCATION DETAIL APPLIES TO 2x FRAMING ONLY. REFER TO ENGINEERED OR COMPOSITE LUMBER MANUFACTURER'S RECOMMENDATIONS AT PSLs, LVLS, LSLs & GLULAM

KING STUD(S). FASTEN MULTIPLE KING STUDS TOGETHER PER BUILT-UP STUD PACK ATTACHMENT SCHEDULE INDEPENDENT OF JACK STUDS ADDITIONAL JACK STUD(S) (SEE SCHEDULE OR PLAN NOTES FOR EXACT **QUANTITIES OF JACK** JACK STUD 0.131"x3" OR 10d COMMON NAILS. FACE NAIL TO KING STUDS @ 12"oc STAGGERED. **FASTEN SUCCESSIVE JACK** STUDS w/ 0.131"x3" OR 10d COMMON NAILS. FACE NAILS @ 12"oc STAGGERED AND MIRRORED FROM PREVIOUS

TYPICAL JACK STUD ATTACHMENT

4 DETAIL

1. JAMB STUDS SHALL MATCH SIZE & GRADE OF WALL STUDS U.N.O.

11. ALL EXTERIOR LUMBER TO BE TREATED PER GENERAL NOTE '9B'

REFER TO DETAIL 5/S0.02 FOR MULTI-PLY MEMBER CONNECTION REQUIREMENTS.

9. REFER TO DETAILS 1/S0.04 FOR TYPICAL HEADER CONDITION AT FLOOR FRAMING.

ATTACH KING STUDS TOGETHER PER DETAIL 3/S0.02 WITH JACKS ATTACHED TO KINGS PER 4/S0.02.

10. REFER TO DETAILS 2 THRU 4 ON S0.21 FOR TYPICAL HEADER CONDITIONS AT ROOF FRAMING.

TOP PLATE.

LAST SCHEDULED LEVEL.

2. WHERE BEAM/HEADER MARK ENDS WITH "-U", THE BEAM SHALL BE UPSET. WHERE BEAM IS NOTED "UPSET", ALL JAMB STUDS NOTED WILL EXTEND TO DOUBLE

AT CONTRACTOR'S OPTION, PROVIDE GLULAM IN LIEU OF PSLs OF EQUAL OR GREATER STRUCTURAL PROPERTIES AS LISTED IN GENERAL NOTES.

PROVIDE 1/2" PLYWOOD SPACER PLs AT HEADERS CONSTRUCTED WITH 2x LUMBER. ATTACH PLY'S TOGETHER PER 5B/S0.02

3. PROVIDE STUDS UNDER ALL JAMBS BETWEEN HEADER/BEAM AND FOUNDATIONS/SLAB. WHEN JAMB IS DISCONT. AT A LEVEL, ALL STUD PACKS BELOW TO MATCH

PROVIDE SQUASH BLOCKS AT TRUSSES @ BLOCKING FRAMING WHERE JAMBS OR STUD PACKS ARE DISCONT. QUANTITY TO MATCH JAMB OR STUD PACK ABOVE

HANGER SCHEDULE MEMBER TYPE/SIZE CONNECTION TYPE HANGER SIZE NOTES FACE MOUNT TO WOOD LUS28 LEDGER/RIMBOARD/BEAM FACE MOUNT TO WOOD LEDGER/RIMBOARD/BEAM FACE MOUNT TO WOOD 2x12 LUS210 TYP @ LOW ROOF LEDGER/RIMBOARD/BEAM 18" DP PRE-ENG FACE MOUNT TO WOOD LUS410 LEDGER/RIMBOARD/BEAM FLOOR TRUSS 18" DP PRE-ENG TOP MOUNT TO 2x NAILER THA426 FLOOR TRUSS ATOP STEEL BEAM 18" DP PRE-ENG FACE MOUNT TO WOOD *@ LOCATIONS WHERE FLR TRUSS HGUS414 LEDGER/RIMBOARD/BEAM FLOOR TRUSS* SUPPORTS OFFSET BRG WALL ABOVE *@ LOCATIONS WHERE FLR TRUSS 18" DP PRE-ENG FACE MOUNT TO WOOD HB3.56/18 LEDGER/RIMBOARD/BEAM SUPPORTS OFFSET BRG WALL ABOVE

1. HANGERS APPLY TO ALL LOCATIONS WHERE NOT OTHERWISE SPECIFIED IN DETAIL OR PLAN NOTE

4.) ALL NAILS NOTED AS 8d, 10d, 16d, ETC. SHALL BE COMMON NAILS UNLESS NOTED BOX BUILT-UP STUD PACK COLUMN ATTACHMENT SCHEDULE NUMBER OF PLIES ATTACHMENT AT JAMB STUD PACKS ATTACHMENT AT WALL STUD PACKS 8d NAILS AT 12"oc, 1" FROM EDGE, w/ 8d NAILS AT 12"oc, 1" FROM EDGE, w/ OPPOSITE OPPOSITE EDGE NAILED FROM EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", **MEMBERS** OPPOSITE SIDE OFFSET 6", @ 12"oc w/ @ 12"oc w/ FIRST NAIL 2" FROM EA. END FIRST NAIL 2" FROM EA. END 20d NAILS AT 16"oc, 1 1/2" FROM EDGE 8d NAILS AT 12"oc, 1" FROM EDGE, w/ OPPOSITE w/ OPPOSITE EDGE NAILED FROM EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", **MEMBERS** OPPOSITE SIDE OFFSET 8", @ 16"oc w/ @ 12"oc w/ FIRST NAIL 2" FROM EA. END FIRST NAIL 4" FROM EA. END 1/4"Øx5" SIMPSON SDS SCREWS AT 3 PLIES ATTACHED PER 3-PLY ATTACHMENT 16"oc, 1 1/2" FROM EDGE w/ OPPOSITE w/ 4th PLY ATTACHED w/ 8d NAILS AT 12"oc IN 2 **MEMBERS** EDGE SCREWED FROM OPPOSITE SIDE ROWS, 1 1/2" FROM EDGE, OFFSET ROW 6" OFFSET 8", @ 16"oc w/ FIRST SCREW 4" FROM EA. END 1/4"Øx6" SIMPSON SDS SCREWS AT 3 PLIES ATTACHED PER 3-PLY ATTACHMENT 5-PLY 12"oc, 1 1/2" FROM EDGE w/ OPPOSITE w/ 4th & 5th PLY ATTACHED w/ 8d NAILS AT **MEMBERS** EDGE SCREWED FROM OPPOSITE 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET SIDE OFFSET 6", @ 12"oc w/ FIRST ROW 6" SCREW 4" FROM EA. END 1/4"Øx8" SIMPSON SDS SCREWS AT 3 PLIES ATTACHED PER 3-PLY ATTACHMENT w/ 4th PLY 12"oc, 1 1/2" FROM EDGE w/ OPPOSITE ATTACHED w/8d NAILS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, **MEMBERS** OFFSET ROW 6" AND 5th AND 6th PLIES ATTACHED w/ 1/4"Øx5" SIDE OFFSET 6", @ 12"oc w/ FIRST SIMPSON SDS SCREWS @ 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, SCREW 4" FROM EA. END OFFSET ROSS 6"oc w/ FIRST SCREW 4" FROM EA. END

3 <u>DETAIL</u>

TYPICAL MULTI-PLY

STUD CONNECTION

ALL BUILT-UP STUD PACKS MUST ALIGN FLOOR-TO-FLOOR WITH SOLID BLOCKING (SQUASH BLOCKS) AT FLOOR CAVITIES. 2. EXTEND ALL STUD PACKS TO COLUMNS UNLESS NOTED OTHERWISE

3. ALL NAILS ARE COMMON NAILS UNLESS NOTED OTHERWISE. 4. JAMB STUD PACKS ARE STUDS SUPPORTING STRUCTURAL MEMBERS SUCH AS BEAMS, HEADERS, GIRDER TRUSSES, ETC. 5. WALL STUD PACKS ARE REPETITIVE STUDS BETWEEN WALL PLATES AS SCHEDULED IN THE "STUD BEARING WALL SCHEDULE".

3 ROWS 16d 2 ROWS SDS 2 ROWS 16d NAILS @ 12"oc NAILS @ 12"oc 1/4"x6 " @ 16"oc 2 ROWS 16d 3 ROWS 16d (ONE SIDE) (EA. SIDE) (EA. SIDE) NAILS @ 12"oc NAILS @ 12"oc (ONE SIDE) (EA. SIDE) 4-PLY TOP LOADED 2-PLY TOP LOADED 3-PLY TOP LOADED 2 ROWS SDS 2 ROWS SDS 2 ROWS SDS 1/4"x3 1/2 " @ 1/4 "x3 1/2" @ 1/4"x6 " @ 12"oc 2 ROWS SDWS22300 16"oc (ONE SIDE) 16"oc (EA (EA. SIDE) SDWS2240 @ 16"oc SIDE) 0 @ 16"oc (ONE SIDE) (EA SIDE) 4-PLY SIDE LOADED 3-PLY SIDE LOADED TYPICAL MULTI-PLY BEAM CONNECTION

5B DETAIL

BASEMENT WALLS BASEMENT WALLS 2nd FLOOR WALLS 3rd FLOOR WALLS 4th FLOOR WALLS **WALL TYPE** (1st FLOOR FRAMING) (1st FLOOR FRAMING) (3rd FLOOR FRAMING) (4th FLOOR FRAMING) (ROOF FRAMING) 2x6 @ 16"oc 2x6 @ 16"oc 2x6 @ 16"oc 2x6 @ 16"oc EXTERIOR WALL BALCONY 2x6 @ 16"oc w/ 2x6 @ 16"oc 2x6 @ 16"oc 2x6 @ 16"oc 2x6 @ 16"oc **EXTERIOR WALL** ADDT'L 2x6 @ 32"oc 2x6 @ 16"oc CORRIDOR WALL TYPICAL UNIT 2x6 @ 16"oc w/ 2x6 @ 16"oc 2x6 @ 16"oc 2x6 @ 16"oc 2x6 @ 16"oc **PARTITION WALL** ADDT'L 2x6 @ 32"oc TYPICAL UNIT 2x4 @ 16"oc w/ 2x4 @ 16"oc w/ (2) 2x4 @ 16"oc 2x4 @ 16"oc 2x4 @ 16"oc **DEMISING WALL** ADDT'L 2x4 @ 32"oc ADDT'L 2x4 @ 32"oc

STUD BEARING WALL SCHEDULE

1. PROVIDE 2x BLOCKINH AT MID HEIGHT (5'-0" MAX) AT ALL LOAD BEARING WALLS NOT SHEATHED ON BOTH SIDES AND ALL STUDS LARGER THAN 2x6.

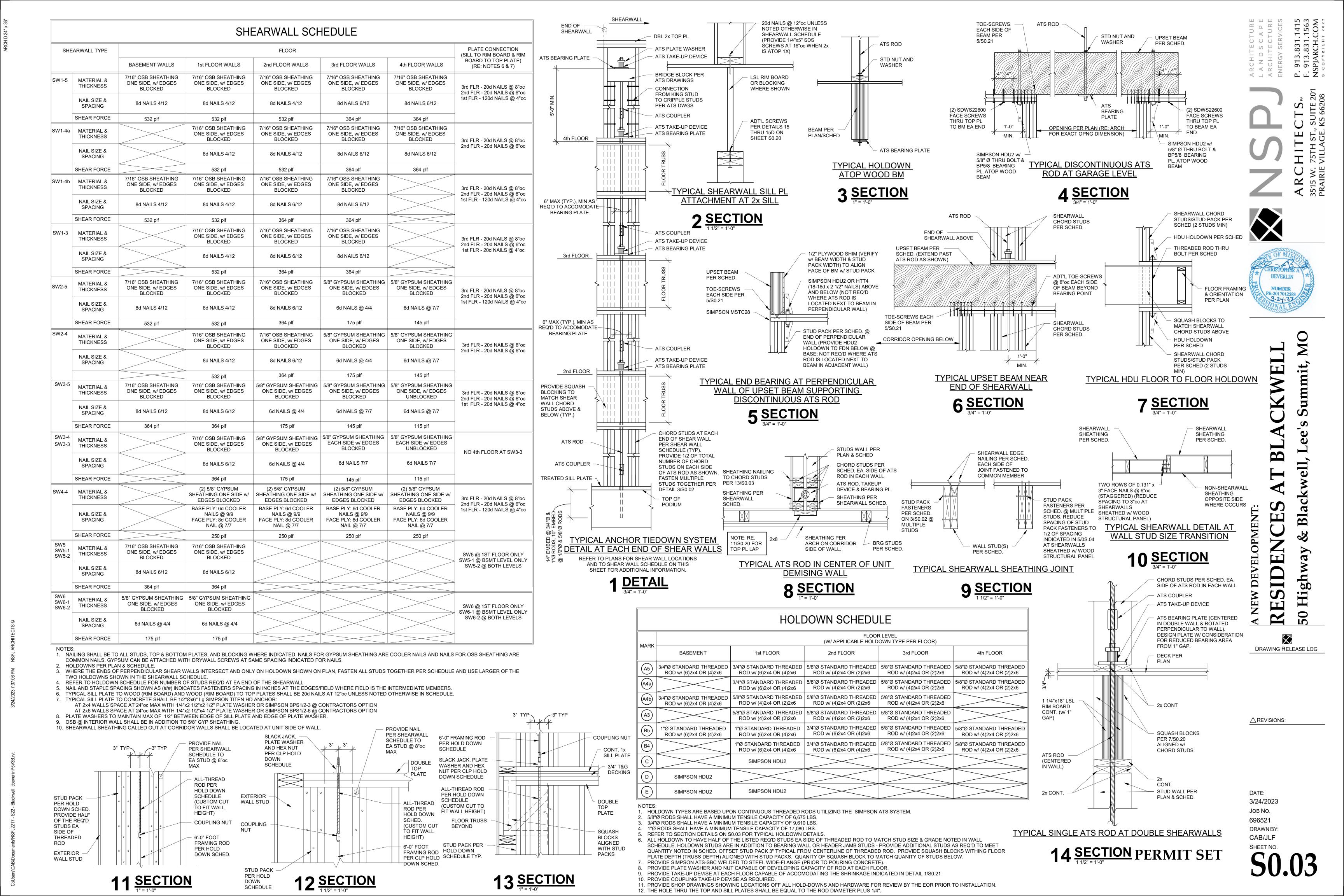
2. ALL STUDS TO BE No. 2 GRADE U.N.O. 3. RE: 3/S0.02 FOR NAILING OF MULTIPLE STUDS.

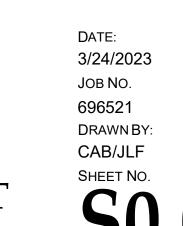
4. REFER TO ARCH/MEP DRAWING FOR LOCATIONS OF FURRED OUT WALLS TO ACCOMODATE PLUMBING OR MEP ITEMS.

5. REFER TO FRAMING PLANS AND ARCH PLANS FOR LEVEL(S) AT WHICH WALLS OCCUR. 6. WHERE SCHEDULE LISTS DIFFERENT WALL SIZES WITH AN "OR", REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS WHERE EACH SIZE IS TO BE USED

TYPICAL MULTI-PLY HEADER CONNECTION

DRAWN BY: CAB/JLF





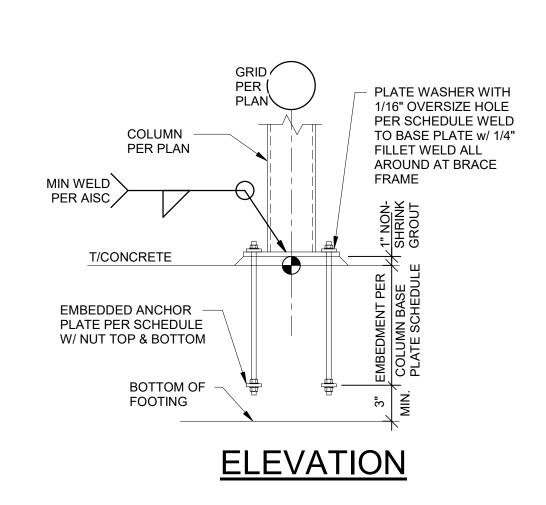


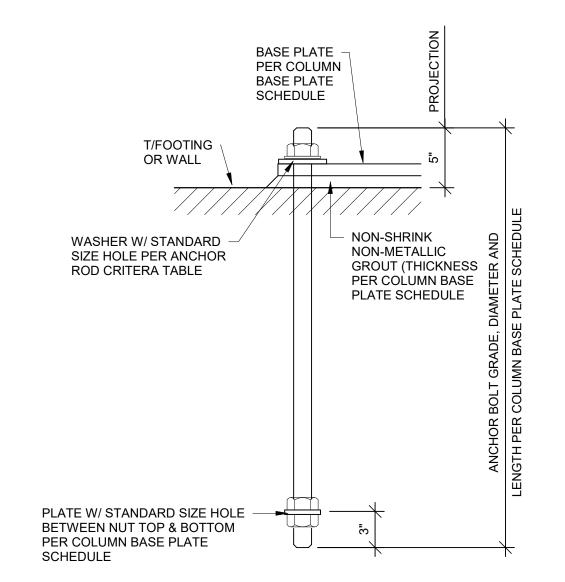


COLUMN BASE PLATE SCHEDULE

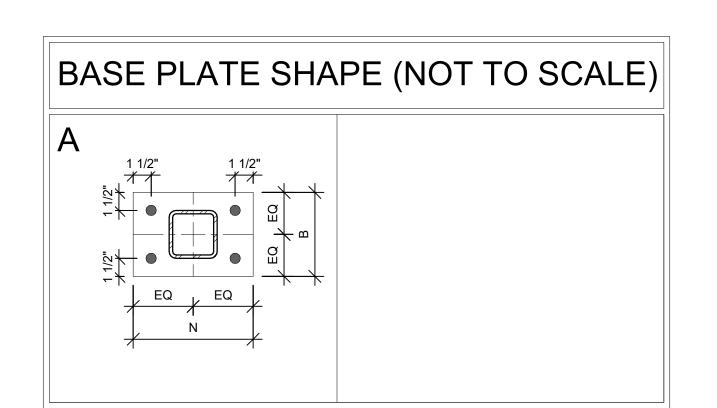
EMBEDMENT COLUMN ANCHOR BOLTS PER PLAN 3/4"x11"x11" 4- 3/4" DIA. 12" PER PLAN 4- 3/4" DIA. 3/4"x12"x12" 12"

C		ASE PLAT DD CRITEF	_ ,	R-
ANCHOR-ROD DIAMETER.	MAX. HOLE DIAMETER.	MIN. WASHER SIZE.	MIN. WASHER THICKNESS	EMBEDDED ANCHOR
3/4"	1 5/16"	2"	1/4"	1/2"x2 1/2"x2 1/2"

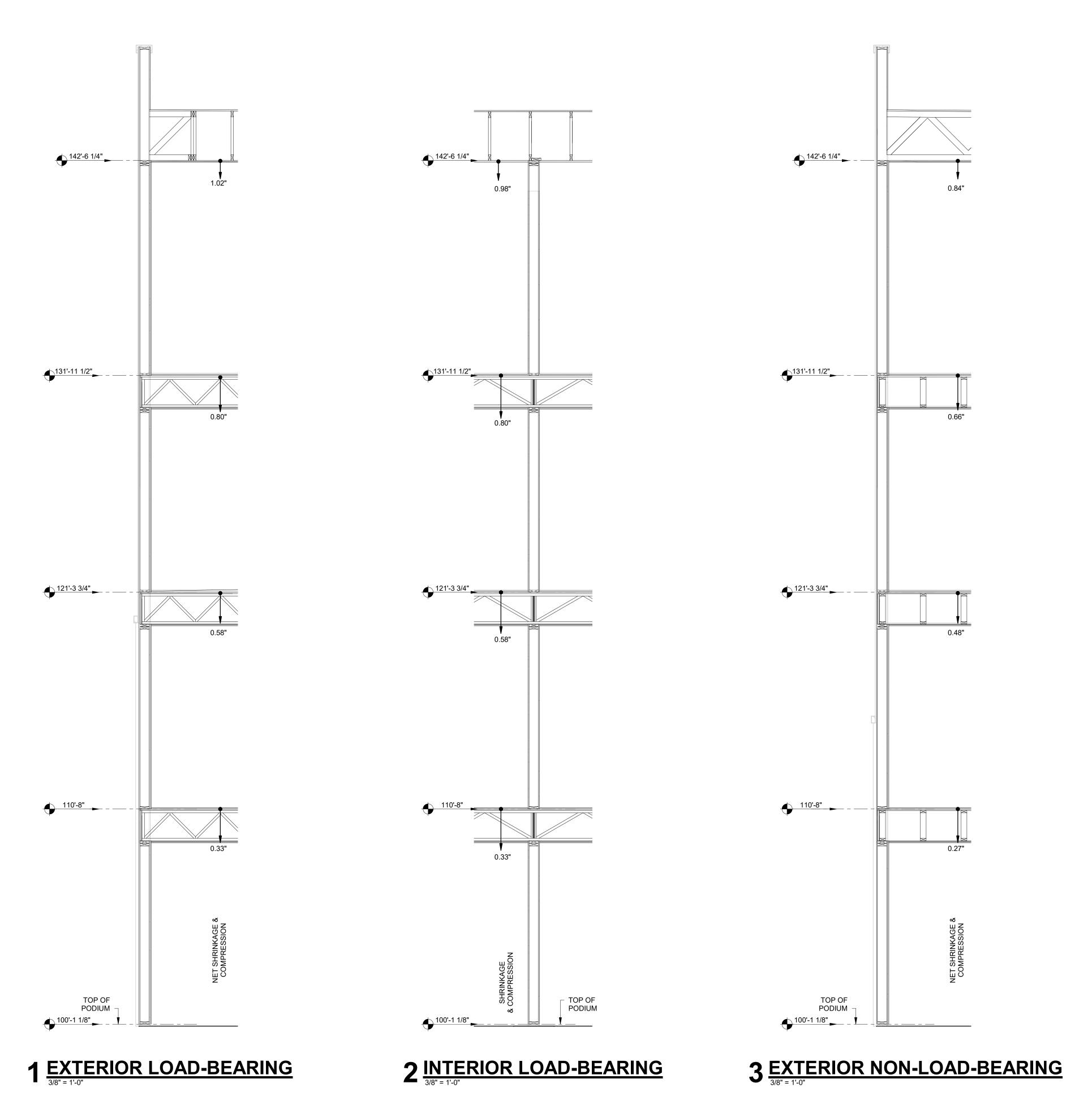




1 TYPICAL ANCHOR BOLT



TYPE NOTES: SEE PLAN FOR ORIENTATION OF COLUMNS.
 PROVIDE PLATE WASHER & EMBEDDED PLATE PER SCHEDULE @ ALL ANCHOR BOLTS.
 U.N.O. ALL THREADED ROD A.B's SHALL BE F1554 (36ksi) MATERIAL.



Wood Shrinkage Notes:

Bob D. Campbell & Company takes no responsibility for the naturally-occurring shrinkage that will occur in a wood structure or the impact the movement will have on the architectural, mechanical, electrical and plumbing systems that are designed by others. The analysis provided below are estimated values in accordance with IBC Section 2304.3.3 and indicate the systems and/or routing of the systems shall be designed to accommodate the movement. Failure to follow the considerations below can result in a failure of the impacted components within the system.

Estimated values are based on the following moisture content in the framing a. MC at delivery to site = 19%

b. Average Outdoor EMC (Time of Systems Being Installed) = 13% c. Average Indoor EMC (During Building Life) = 8%

The following is a list of recommendations to minimize potential issues related to wood shrinkage and veneer expansion. Veneer expansion is seasonable and variable depending on sun exposure. The majority of wood shrinkage will occur in the first 24 months of occupancy with minor seasonal

Reference wall sections on this sheet for estimated cumulative values per floor.

- MEP System Considerations
- a. Postpone MEP installation as long as possible to allow as much dead
- load to be applied--allowing construction gaps to close. b. Provide oversized and vertically slotted holes at pipe horizontal penetration and notches. Refer to typical notching and cutting of stud

wall detail for additional considerations on size limitations.

- c. Plumbing pipe and electrical conduit joints and connections shall be flexible and allow for
- expansion/contraction to prevent a rigid assembly.
- d. Hangers and necessary rigid connections shall be adjusted prior to completion of construction or closing of wall/ceiling assembly.
- e. Horizontal vent penetrations through exterior veneers shall be provided with double flashing.
- f. All sheet metal vertical down spouts shall have intermediate slip joints. g. Roof drains shall utilize adjustable fittings that are adjusted back to the
- roof finish sheathing elevation at the completion of construction and then shall be adjusted as required to maintain proper drainage. 2. Architectural System Considerations
- a. At stucco, EIFS and thin set veneer systems provide horizontal expansion joints, slip joints with appropriate
- flashing, this includes transitions between changes in veneer material. b. At brick and stone veneers provide veneers ties designed
- to accommodate differential movement. c. Refer to architectural window and door head and sill;
- parapet; and horizontal material changes for specific horizontal gap requirements between materials.
- d. Around rigid (concrete/CMU) stair and elevator towers and at fire seperation walls provide adjustable thresholds or transitions.
- 3. Construction Tolerance Considerations a. All studs shall be cut level, square and tight to top and bottom plates to reduce any additional shortening of the
- building due to nesting. b. All wood structural panels on the walls shall have a 1/2" relief
- gap at each floor level to reduce the potential for bulging.
- c. All floor sheathing shall have 1/8" gaps around all four sides at time of install to allow for expannsion.
- d. All shearwall holdown shall be checked and retighten immediately prior to sheathing of the walls. If a continuous rod system is utilized for holdowns or uniform uplift anchors, the take-up devise pins shall be
- verified to have been pulled prior to sheathing the walls. e. Delay placement of gyp topping around rigid (concrete/CMU) stair and elevator towers until completing of construction.
- 4. Material Storage and Protection a. All stored material shall remain covered and elevated from
- the elements to reduce the potential for an increase in
- b. Do not allow water to pond on the floor sheathing. Provide drain holes in the floor sheathing as required to relieve any water that might temporary pond.
- 5. Post Occupancy Consideration a. Recommend a review of roof drains every 3 months for the first 24
- months of occupancy and then annually and adjusted as needed. b. Recommend a review of vertical joints at exterior doors, windows and

or CMU stair and elevator towers as needed as shrinkage occurs.

- at changes in materials. Caulked as needed as shrinkage occurs and
- original joint fails. c. Remedial self-leveling work may be required around concrete





△REVISIONS:

DATE: 3/24/2023 JOB NO. 696521 DRAWN BY: CAB/JLF

	REBAR DEVELOPMENT LENGTH AND LAP SPLICE SCHEDULE													
CONCRETE STRENGTH = 5000 psi CONC						CONCRETE STRENGTH = 4000 psi			CONCRETE STRENGTH = 3500 psi					
CASE	LENG	OPMENT TH OR S A LAP	CLASS	B LAP	CASE	LENG	VELOPMENT ENGTH OR CLASS B LAP LASS A LAP		B LAP	CASE DEVELOPMENT LENGTH OR CLASS A LAP		CLASS B LAP		
BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	24	24	24	24	#3	24	24	24	24	#3	24	24	26	24
#4	24	24	29	24	#4	25	24	33	25	#4	27	24	35	27
#5	28	24	36	28	#5	31	24	41	31	#5	33	26	43	33
#6	34	26	43	34	#6	37	29	49	37	#6	40	31	52	40
#7	49	38	63	49	#7	54	42	71	54	#7	58	45	75	58
#8	56	43	72	56	#8	62	48	81	62	#8	66	51	86	66
#9	63	48	81	63	#9	70	54	91	70	#9	75	58	97	75
#10	71	54	92	70	#10	79	61	102	79	#10	84	65	109	84
#11	78	60	102	78	#11	87	67	113	87	#11	93	72	121	93

NOTES:

1. UNLESS SPECIFICALLY INDICATED OTHERWISE, USE THE MINIMUM LENGTH FOR A CLASS B LAP SPLICE OR THE MINIMUM DEVELOPMENT LENGTH INDICATED IN THE TABLES ABOVE MULTIPLIED BY THE

APPLICABLE FACTOR(S) LISTED BELOW.
2. WHERE THE CLEAR SPACING BETWEEN BARS LAP SPLICED OR EMBEDDED AT ANY SECTION IS LESS THAN 2 BAR DIAMETERS, OR WHERE THE BAR COVER IS LESS THAN OR EQUAL TO THE BAR DIAMETER.

INCREASE THE INDICATED BAR SPLICE OR DEVELOPMENT LENGTH BY 50%.

3. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.

4. MECHANICAL COUPLERS MAY BE SUBSTITUTED FOR TENSION LAP SPLICED BARS PROVIDED THAT THEY MEET THE REQUIREMENTS OF ACI 318-11, 12.14.

4. MECHANICAL COUPLERS MAY BE SUBSTITUTED FOR TENSION LAP SPLICED BARS PROVIDED THAT THEY MEET THE REQUIREMENTS OF ACI 318-11, 12.14.

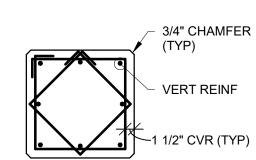
5. AT LOCATIONS WHERE REINFORCING WITHIN A STRUCTURAL ELEMENT WILL BE SPLICED, ALTERNATING SPLICES SHALL BE STAGGERED A MINIMUM OF THE CLASS B SPLICE LENGTH UNLESS INDICATED
OTHERWISE

CONCRETE COLUMN SCHEDULE					
COLUMN SIZE	REINFORCEMENT				
18x18	(8) #8 VERTICAL (2) #3 TIES @ 12"oc				

NOTE

1) PROVIDE (4) SETS OF TIES AT 3"oc TOP & BOTTOM OF EACH COLUMN

2) ALL COLUMNS TO CENTER ON GRIDLINE AND PIER/FOUNDATION U.N.O.



18"x18" COLUMN

1 COLUMN DETAILS

3/4" = 1'-0"

SLAB NOTES

- SEE GENERAL NOTES (STRUCTURAL) ON SHEET S0.01.
 PODIUM SLAB IS 12" THICK AND REINFORCED WITH A
 CONTINUOUS (12" LAP AT COLUMN CENTERLINE OF COLUMN
 STRIPS AND 24" LAP AT COLUMN CENTERLINE OF MID-STRIPS)
 BOTTOM MAT OF #5 @ 12" EACH WAY. THE BOTTOM MAT
 EXTENDING NORTH-SOUTH (PLAN EAST-WEST) SHALL BE
 SUPPORTED ON 1" SLAB BOLSTERS @ 4'-0"oc MAX. TOP OF
 CONCRETE ELEVATION PER PLAN (STEP TOP OF SLAB AND
 SLOPE AS INDICATED AT EXTERIOR LOCATIONS.)
 - EXTRA REINFORCING BARS PLACING SEQUENCE:

 14 A5 19'-9"
 - TOTAL LENGTH OF BAR IN FEET AND INCHES
 SIZE OF BAR AND LOCATION IN SLAB AS NOTED BELOW
 TOTAL NUMBER OF EXTRA BARS IN STRIP DEFINED ON PLAN
 - #5 EXTRA BOTTOM BARS WITH 1" CLEAR COVER BOTTOM. (PLACE WITH 1" CLEAR COVER BOTTOM MAT BARS.)
 #5 EXTRA BOTTOM BARS WITH 1 5/8" CLEAR COVER BOTTOM. (PLACE
 - WITH 1 5/8" CLEAR COVER BOTTOM MAT BARS.) PLACE ON TOP OF PERPENDICULAR (1" CLEAR COVER) BOTTOM MAT AND "A" BARS.

 C. #5 TOP BARS WITH 1 5/8" CLEAR COVER WHERE TWO LAYERS OF BARS OCCUR AND 1" CLEAR COVER WHERE ONE LAYER OF BARS
 - OCCUR ON IHC @4'-0" o.c. AND #5 SUPPORT BARS @4'-0"o.c.

 D. #5 TOP BARS WITH 1" CLEAR COVER TOP. PLACE ON TOP OF "C"
 BARS WHERE THEY OCCUR OR OTHERWISE PLACE ON IHC AT 4'-0"o.c.
 AND #5 SUPPORT BARS AT 4'-0" o.c.
 #5 TOP BARS WITH 1" CLEAR COVER TOP, PLACE ON IHC AT 4' 0"o.c.
 - AND #5 SUPPORT BARS AT 4'-0" o.c.

 E. #5 TOP BARS WITH 1" CLEAR COVER TOP. PLACE ON IHC AT 4'-0"o.c.
 AND #5 SUPPORT BARS AT 4'-0"o.c.
- 4. REINFORCING SHALL BE SPLAYED AROUND OPENINGS LESS THAN 18"
 WIDE. REINFORCING SHALL BE CUT AT OPENINGS GREATER THAN 18"
 WIDE WITH EQUAL CONTINUOUS BARS ADDED ONE-HALF EACH SIDE OF
 OPENING. PROVIDE REINFORCING PER GENERAL NOTE 4F AT ALL
 OPENINGS LAGER THAN 8".
- STRIP LINES ARE LOCATED AT 1/4 POINTS BETWEEN COLUMN
- CENTERLINES UNLESS NOTED ON PLAN OTHERWISE.

 6. SEE DETAIL 2/S3.10 FOR PLACING PATTERN FOR TOP REINFORCING
- BARS OVER INTERIOR COLUMN AS NOTED.

 7. TOP BARS SHOWN STAGGERED ON PLAN SHALL BE STAGGERED WHEN PLACED; THE END OF EVERY OTHER BAR TO BE PLACED AT RELATIVE
- STRIP LINE, UNLESS NOTED ON PLAN.

 8. BOTTOM BARS ARE SHOWN THUS ———————
 TOP BARS ARE SHOWN THUS ———————
- UNLESS SHOWN ON "S" SERIES DRAWINGS, NO HOLES LARGER THAN TEN INCH DIAMETER SHALL BE PLACED THROUGH SLAB. NOT MORE THAN ONE, SIX TO EIGHT INCH DIAMETER HOLES, OR TWO FOUR INCH DIAMETER HOLES, OR THREE TWO INCH DIAMETER OR SMALLER HOLES SHALL BE PLACE WITHIN 20"OF THE FACE OF THE COLUMNS.
- SHALL BE PLACE WITHIN 20"OF THE FACE OF THE COLUMNS.

 CAMBER ALL SPANS BETWEEN 16'-0" AND 24'-0" (CENTERLINE TO CENTERLINEOF SUPPORTS) FOR L/600 MINIMUM AT MIDSPAN (WITH L = SPAN IN INCHES) (I.E., 3/8 AT MIDSPAN FOR 18'-0" SPAN. CAMBER ALL SPANS LONGER THAN 24'-0" FOR L/480 (I.E., 3/4" AT MIDSPAN FOR 30'-0" SPAN.) DO NOT CAMBER SLAB IN COURTYARD AREA WHEN DRAIN IS
- LOCATED AT CENTER OF SPAN.

 11. AT TERMINATION OF COLUMN STRIP AT COLUMN, WALL, BEAM, PROVIDE 90 DEGREE STANDARD ACI HOOKED END AT (4) BOTTOM BARS NEAREST COLUMN CENTERLINE PER 6/S3.10



RESIDENCES AT BLACKWELL
50 Highway & Blackwell, Lee's Summit, MO

DRAWING RELEASE LOG

△REVISIONS:

DATE:
3/24/2023
JOB NO.
696521
DRAWN BY:
CAB/JLF
SHEET NO.

SET SHEET NO.

TRUSS BEARING

15 <u>SECTION</u>

TRUSS BEARING

15B <u>SECTION</u>

15A <u>SECTION</u>

CONTINUOUS PAST NON-LOAD BRG WALL

14 <u>SECTION</u>

AND UNIT DEMISING WALL w/ PLUMBING WALL

12 **SECTION**

CAB/JLF SHEET NO.

PERMIT SET

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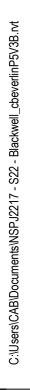
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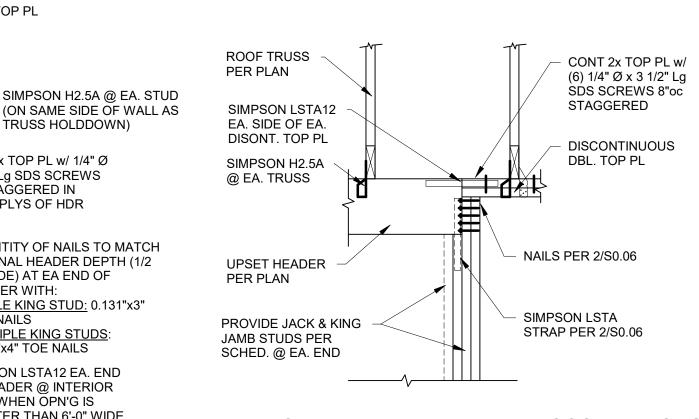
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14 **SECTION**

15 **SECTION**



TYPICAL HEADER DETAIL AT DISCONTINUOUS TOP PLATE AT ROOF

END LENGTH

(TYP)

PLAN VIEW

PLAN VIEW AT END OF WALL

3 **SECTION**3/4" = 1'-0"

PSL COL PER PLAN (w/ COL

CAP WHERE INDICATED)

PSL COL PER

PLAN (w/ COL

CAP WHERE

INDICATED)

1/4" x 6" Lg SDS SCREWS @ 8"oc STAGGERED BTWN CTR OF OUTER PLY'S NAILS PER 2/S0.06 HEADER PER PLAN (DIRECTLY BELOW DBL TOP PL) NOTE: SIMPSON H10S HOLDOWNS SECURING TRUSS DIRECTLY TO HEADER SIMPSON LSTA MAY BE USED IN LIEU OF STRAP PER 2/S0.06 SCREWS & HOLDOWN TO TOP PL ONLY

HOLDOWNS

PER 2/S0.06

WALL DBL TOP PL

(2) 2x6 TOP PL WHERE GEOMETRY

ALLOWS

TYPICAL HEADER DEATIL AT ROOF TRUSS BRG LOCATIONS W/HEADER DIRECTLY BELOW DBL TOP

4 **SECTION**3/4" = 1'-0"

MULTI - PLY

SCHED.

HEADER PER

(2) ROWS OF 1/4" x 4 1/2" Lg

ROWS ON OPP. SIDE BY 6")

SDS SCREWS @ 12"o.c.

EACH SIDE (STAGGER

PROVIDE BLOCKING EACH SIDE OF BEAM @ BEARING FULL WALL WIDTH BRG REQ'D (NOT SHOWN FOR CLARITY) EXTEND ACROSS ENTIRE STUD PACK-BELOW @ MINIMUM @ PARALLEL WALLS PROVIDE (1) SDWS22600 TOE-SCREW EACH SIDE OF BEAM FOR EACH 1 3/4" OF BEAM BEARING LENGTH (WHERE BEAM OVERHANGS FACE OF PARALLEL WALL, INSTALL TOE-**UPSET PSL OR GLULAM** SCREWS THRU WALL TOP PL TO BEAM PER PLAN/SCHED. GLULAM OR PSL (2) 16d END NAILS FOR EVERY 1 3/4" OF COL. WIDTH (TOP & BOTT. COL. PER PLAN/SCHED. USE (3) 16d NAILS @ 7" WIDE COL. (TYPICAL WHERE COL. SITS BTWN WALL TOP & BOTTOM PLATES) 3 1/2" PDL @ 2x4 WALLS-5 1/4" PSL @ 2x6 WALLS—

TYPICAL PSL OR GLULAM BEAM AND/OR COLUMN DETAIL

5 **SECTION**

NUMBER 3.24.23

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Black

Highway

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RESIDE

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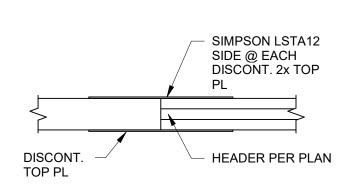
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3/24/2023

PROVIDE (2) PROVIDE (2) ON-CENTER SPACING **FASTENERS TOP** FASTENERS TOP **ON-CENTER SPACING** OF FASTENERS PER & BOTT AT EA. - OF FASTENERS PER & BOTT AT EA. DECK DETAILS **END OF LEDGER** DECK DETAILS **END OF LEDGER** A STAGGERED FASTENER ATTACHMENT OF LEDGER (2) ROW FASTENER
ATTACHMENT OF LEDGER NOTE: LEDGER SIZE, FASTENER SIZE AND FASTENER SPACING PER DECK DTLS

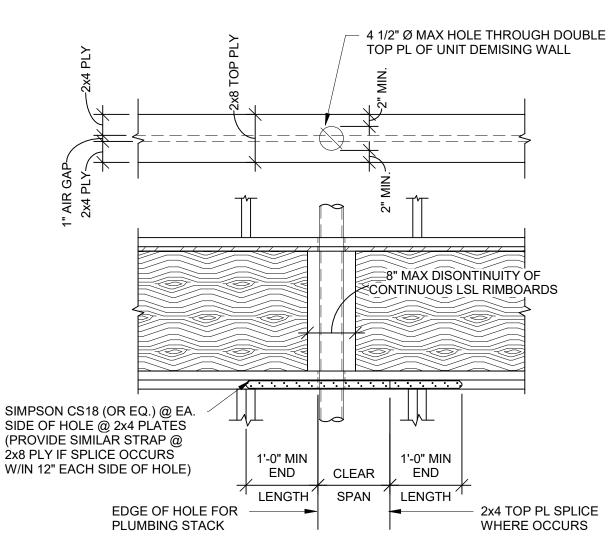
TYPICAL LEDGER CONNECTION

8 **SECTION**



TYPICAL SPLICE AT DISCONTINUOUS TOP PL

10 **SECTION**



DOUBLE UNIT DEMISING WALLS

13 **SECTION**

PERMIT SET

TYPICAL DETAIL AT PLUMBING STACK IN

JOB NO. 696521 DRAWN BY: CAB/JLF

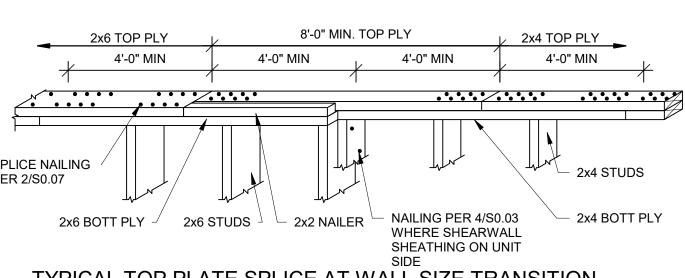
SHEET NO.

 $7 \frac{\text{SECTION}}{1 \cdot 1/2" = 1' - 0"}$ **FASTENERSON CENTER** SPACING PER BALCONY - 6" MAX. HOLE **FASTENER EDGE DISTANCE** PER 8/S0.06 ─ BALCONY JOIST , EQ∖ |, ∕ÉQ PER PLAN FASTENER ON CENTER STAGGERED SPACING -2 FASTENERS EA. PER BALCONY DETAILS SIDE OF HOLE TYPICAL HOLE THRU LEDGER DETAIL

TYPICAL MULTI-PLY EXTERIOR ROOF HEADER WHERE

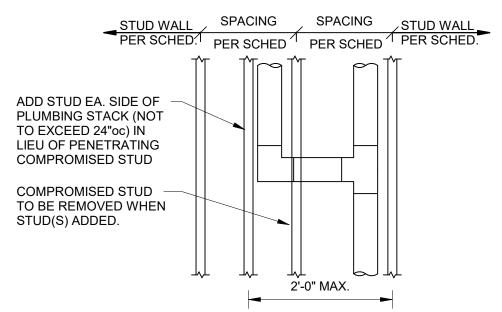
GEOMETRY DOES NOT ALLOW 2x PL BELOW HEADER

9 **SECTION**

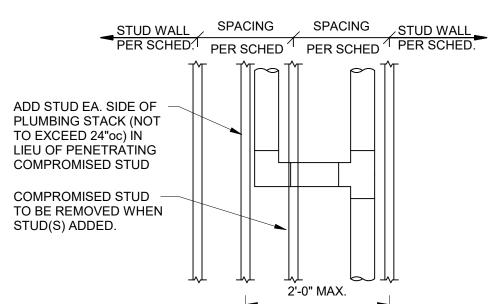


TYPICAL TOP PLATE SPLICE AT WALL SIZE TRANSITION

12 **SECTION**



SPLICE NAILING PER 2/S0.07



TYPICAL WHERE PLUMBING STACK HAS HORIZONTAL JOG

16 **SECTION**

SHEARWALL EDGE NAILING 2x4 STUDS PER SCHED. AS SHOWN (PROVIDE FASTENERS @ 7"o.c. @ NON-SHEARWALLS) (FIELD NAILING & INTERIOR EDGE NAILING @ PANEL EDGES NOT SHOWN FOR CLARITY) TYPICAL TOP PLATE SPLICE

SPLICE POINT

11 <u>SECTION</u>

4'-0" MIN (TYP)

BTWN SPLICES

OPTION 1: PROVIDE NEW STUDS (QUANTITY TO MATCH SCHED.) BELOW TRUSS ADJACENT TO COMPROMISED BLOCKING LOCATION

TYPICAL WHERE MECHANICAL DUCT INTERRUPTS LSL

SIMPSON H2.5A @ EA. ROOF TRUSS

HEADER PER PLAN

PROVIDE JACK & KING

JAMB STUDS PER

SCHED. @ EA. END

ELEVATION VIEW

PLAN VIEW AT CORNER

GENERAL NOTE

PROVIDE CRIPPLE

JAMB ABOVE @ EA.

2x STUD

END AS REQ'D

AT CONTRACTOR'S OPTION

STRAP FROM STUD TO HEADER

HOLDOWN) IN LIEU OF LOWER

PROVIDE SIMPSON LSTA9

(ON SAME SIDE AS TRUSS

H2.5A AND SDS SCREWS

FLOOR FRAMING PER

PLAN (ALIGN w/ STUDS

WHERE INDICATED IN

SCHED OR DETAILS)

KING STUDS (RE: SCHED)

CS20 COIL STRAP w/ 6" MIN END

AT SHEARWALLS)

LENGTH w/ (6) 10d x 1 1/2" Lg NAILS EA.

END EA. SIDE OF WALL (SPLICE SINGLE

TOP PL AT TYPICAL WALL & BOTH PL's

WALL DBL TOP PL

COMPRESSION,

SHIM AS REQ'D)

COL FOR

(INSTALL TIGHT TO

PSL COL PER PLAN

WHERE INDICATED)

(w/ COL CAP

HEADER PER PLAN

JACK STUDS

(RE: SCHED)

UPSET PSL OR GLULAM

BEAM PER PLAN/SCHED.

STUDS TO MATCH

DBL TOP PL

TRUSS HOLDDOWN)

QUANTITY OF NAILS TO MATCH

NOMINAL HEADER DEPTH (1/2

SINGLE KING STUD: 0.131"x3"

EA SIDE) AT EA END OF

MULTIPLE KING STUDS: 0.148"x4" TOE NAILS

SIMPSON LSTA12 EA. END

OF HEADER @ INTERIOR

GREATER THAN 6'-0" WIDE

CS20 COIL STRAP w/ 6" MIN END

AT SHEARWALLS)

LENGTH w/ (6) 10d x 1 1/2" Lg NAILS EA.

END EA. SIDE OF WALL (SPLICE SINGLE

TOP PL AT TYPICAL WALL & BOTH PL's

WALL DBL TOP PL

COMPRESSION,

SHIM AS REQ'D)

COL FOR

TYPICAL STRAPS @ DISCONTINUOUS TOP PL @ COL

6 SECTION

MECHANICAL

PENETRATION

(INSTALL TIGHT TO

CS20 COIL STRAP w/ 6" MIN END

LENGTH w/ (6) 10d x 1 1/2" Lg

WALL (SPLICE SINGLE TOP PL

AT TYPICAL WALL & BOTH PL's

NAILS EA. END EA. SIDE OF

AT SHEARWALLS)

FACE WHEN OPN'G IS

CONT 2x TOP PL w/ 1/4" Ø

x 3 1/2" Lg SDS SCREWS

8"oc STAGGERED IN

HEADER WITH:

END NAILS

TYPICAL HEADER DETAIL @ ROOF TRUSS BRG LOCATIONS

2 **SECTION**3/4" = 1'-0"

PSL COL PER

PLAN (w/ COL

CAP WHERE

INDICATED)

CS20 COIL STRAP w/ 6" MIN

END LENGTH w/ (6) 10d x 1 1/2"

Lg NAILS EA. END EA. SIDE OF

WALL (SPLICE SINGLE TOP PL

AT TYPICAL WALL & BOTH PL's

AT SHEARWALLS)

(2) 2x TOP

PL (TYP.)

ADJACENT STUDS)

,END LENGTH,

OUTER PLYS OF HDR

PROVIDE (2) 2x (MATCH STUD SIZE) BLOCKING ALLIGNED BELÓW MECH. PENETRATION AND EXTENDED 2 BAYS MINIMUM EACH SIDE OF PENETRATION. NAIL PLATES TOGETHER W/ 0.131"x3" FACE

LOCATION OF CUT TOP PL (TRUSS BEARING

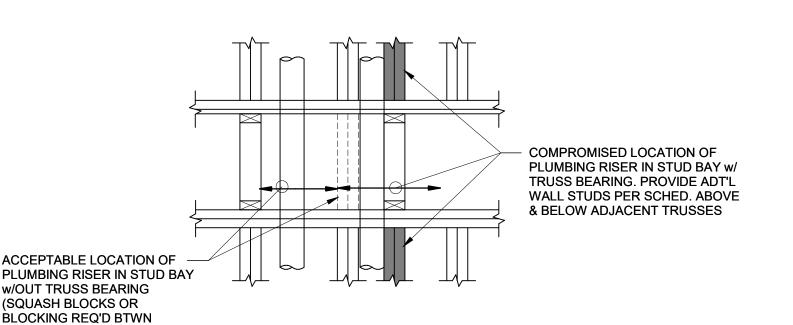
-IS NOT ALLOWED TO OCCUR W/IN STUD BAY

OF CUT OR PARTIALLY CUT TOP PLATE)

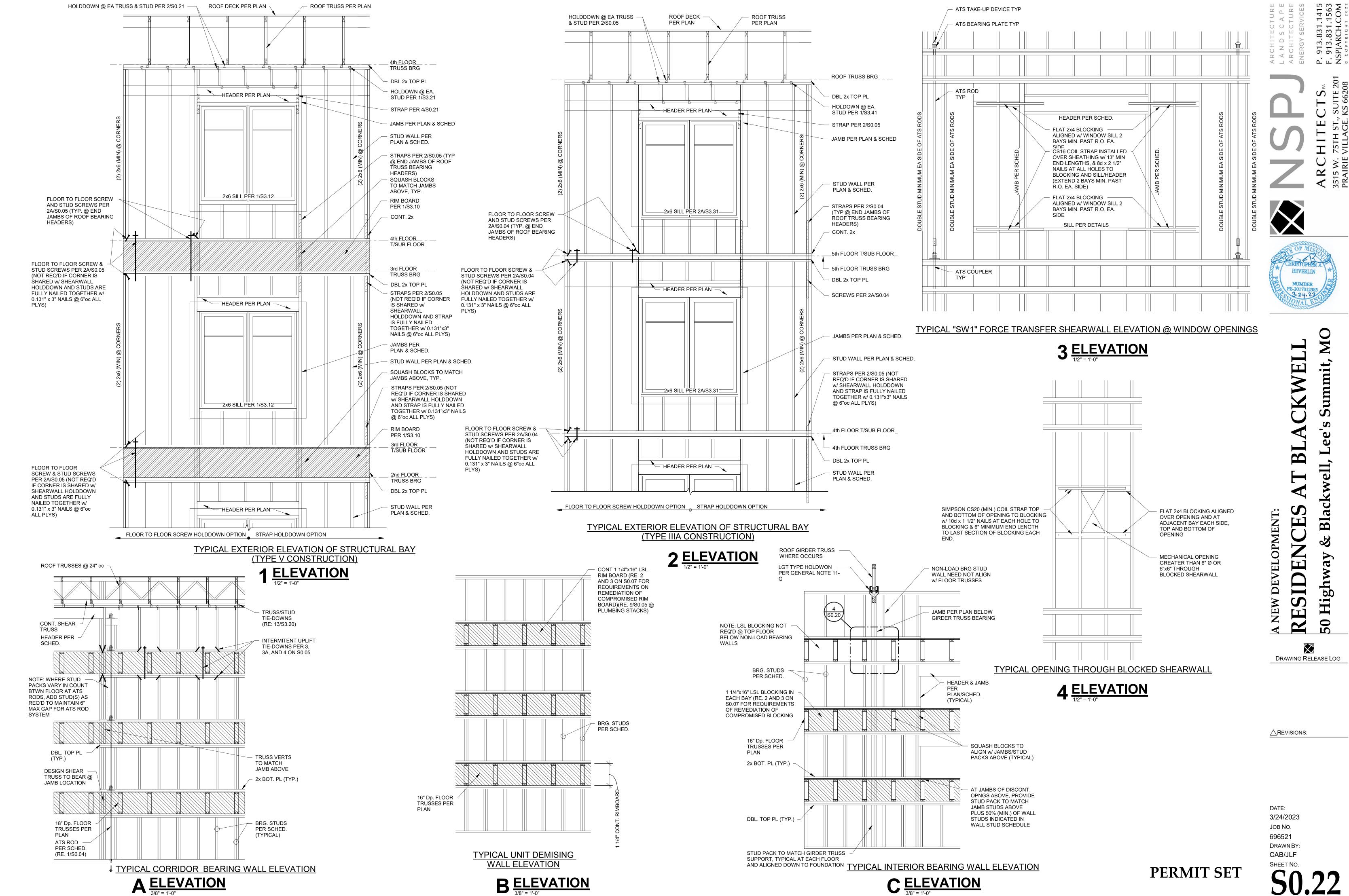
NAILS @ 6"o.c SIMPSON CS20 (MIN.) COIL STRAP EACH SIDE OF WALL TO LOWER PLY OF BLOCKING (TWO PLY BLOCKING MAY BE ELIMINATED IF STRAP IS APPLIED OVER WALL SHEATHING) W/ 10d x 1 1/2" NAILS TO EACH HOLE TO BLOCKING & 6" MINIMUM

> END LENGTH TO LAST BAY OF BLOCKING TYPICAL TOP PLATE SPLICE AT SHEARWALL OR BEARING WALL DUE TO MECH. PENETRATION

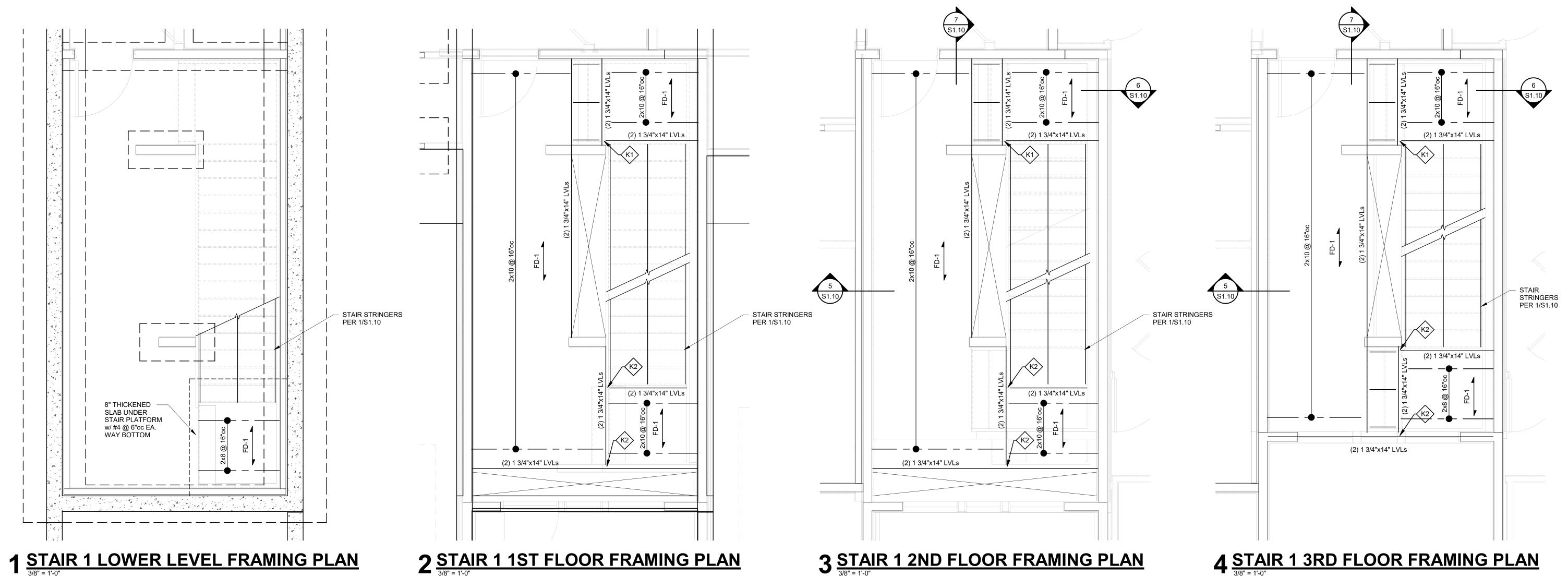
11A <u>SECTION</u>



TYPICAL WHERE PLUMBING INTERRUPTS TOP PL



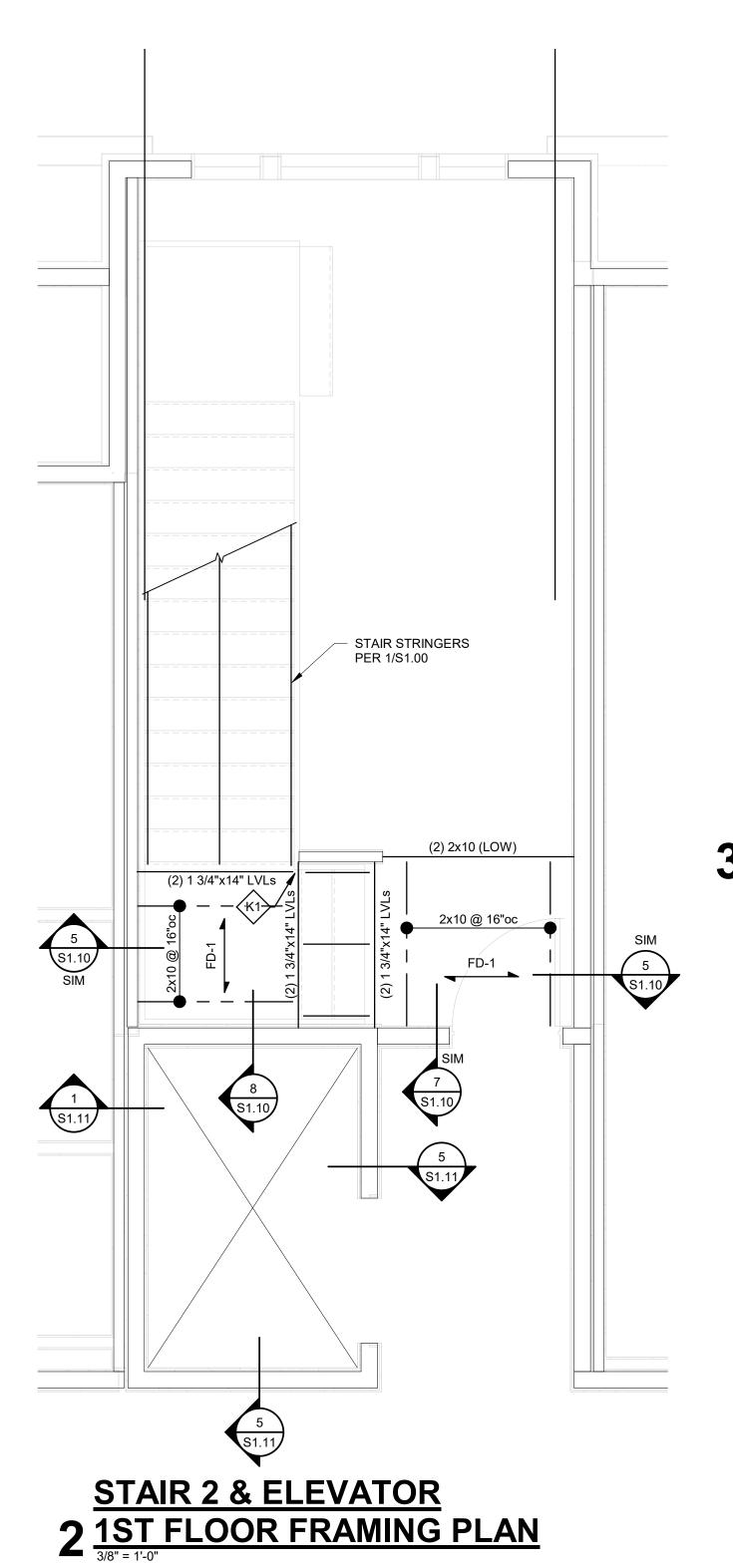
696521 DRAWN BY: CAB/JLF



(2) 1 3/4"x14" LVLs

STAIR STRINGERS
PER 1/S1.10

l (2) 2x10 (LOW)



8" THICKENED SLAB
 UNDER STAIR
 PLATFORM w/ #4 @
 6"oc EA. WAY BOTTOM

- STAIR STRINGERS PER 1/S1.00

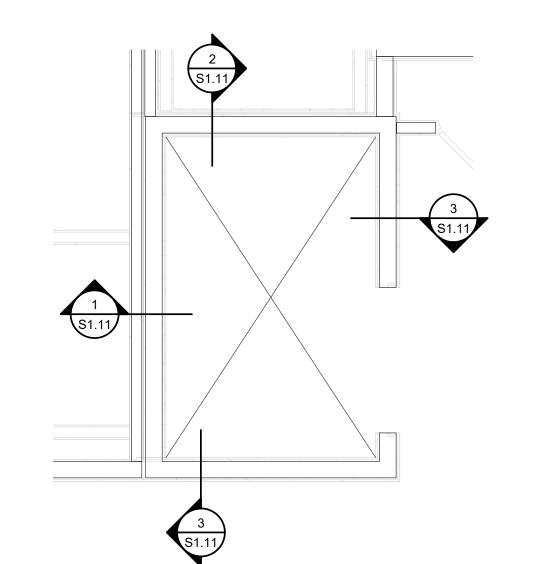
L_____

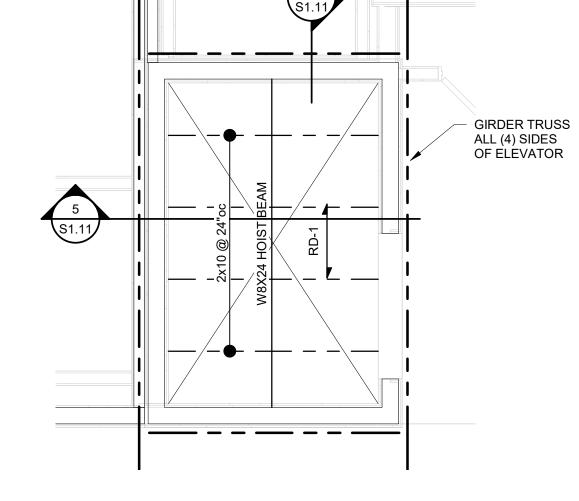
STAIR 2 & ELEVATOR LOWER

1 LEVEL FRAMING PLAN

3/8" = 1'-0"

3 ELEVATOR 2ND/3RD/4TH FLOOR FRAMING PLAN





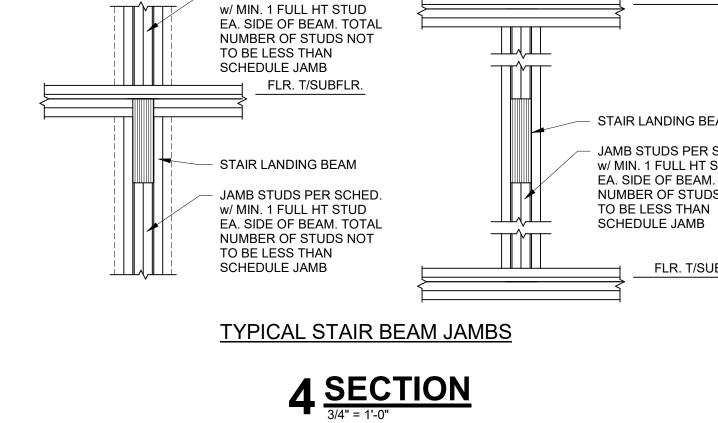
4 ELEVATOR ROOF FRAMING PLAN
3/8" = 1'-0"

DATE: 3/24/2023 JOB NO.

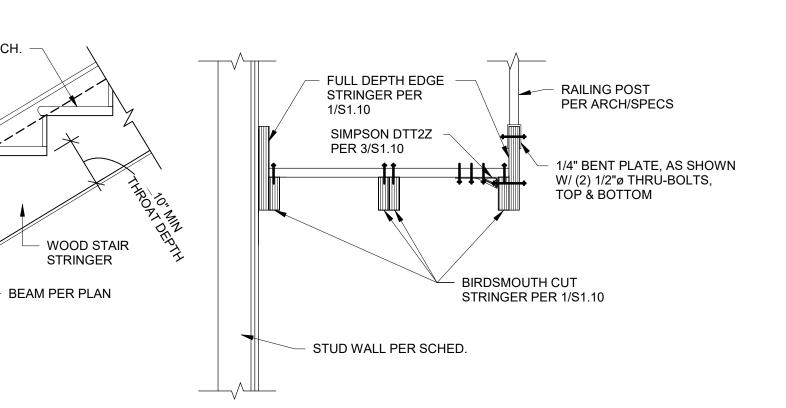
DATE: 3/24/2023 JOB NO.

△REVISIONS:





JAMB STUDS PER SCHED.



TYPICAL STAIR RUN TYPICAL THRU SECTION

2 **SECTION**3/4" = 1'-0"

TYPICAL STAIR AND STRINGER CONNECTION DETAIL

1 **SECTION**3/4" = 1'-0"

- JOIST PER PLAN w/ HANGER PER JOIST HANGER SCHED. SHEET ON S0.02

(DO NOT OVER CUT)

- 3/4" T&G PLYWOOD

DECK

WOOD STAIR

STAIR

ON S0.02

5 SECTION 3/4" = 1'-0"

(3) SDWS22600DB

SCREWS @ 16"oc

STRINGER

UNIT

BEAM PER PLAN -

SIMPSON HUC48 w/

STUD WALL PER

DECK PER PLAN & SCHED.

PRE-ENG FLOOR -TRUSS PER PLAN

HANGER PER PLAN

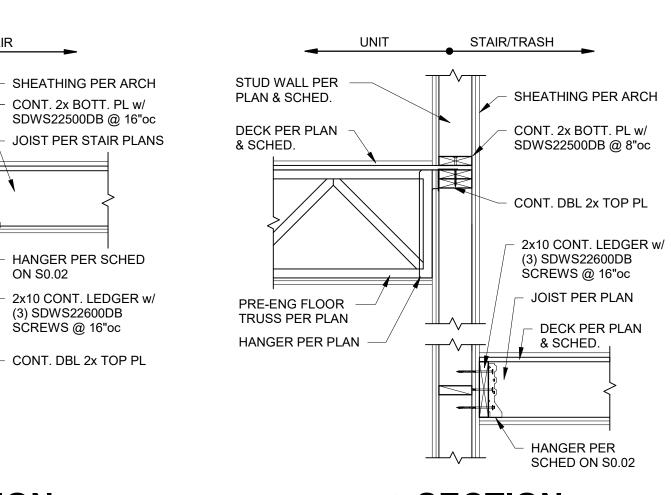
CONT. DBL 2x TOP PL

2x BLOCKING @ MID-HEIGHT (TYP. @ WALLS SHEATHED ON ONE SIDE ONLY)

PLAN & SCHED.

SLOPED SEAT @ EA. STRINGER

TREADS & RISERS PER ARCH.



TREADS & RISERS PER ARCH.

SIMPSON HUC48 w/ SLOPED

SEAT @ EA. STRINGER

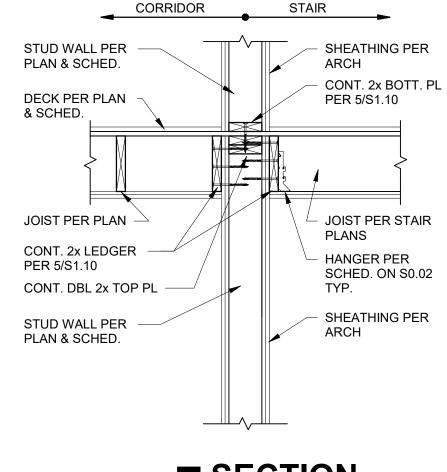
3/4" T&G PLYWOOD DECK

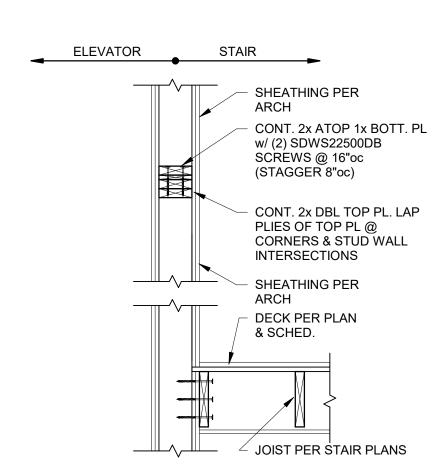
JOIST PER PLAN w/ HANGER PER JOIST HANGER SCHED.

SHEET ON S0.02

WOOD STAIR STRINGERS: 1 3/4" x 14" LVL @ CENTER SPAN: PROVIDE (2) BIRDSMOUTH CUT STRINGERS

@ END SPAN: PROVIDE (1) BRIDSMOUTH CUT STRINGER w/ (1) UNCUT STRINGER EA. SIDE (DO NOT OVER CUT)

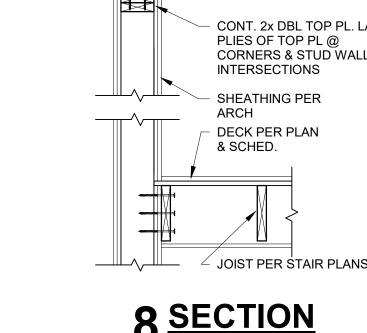




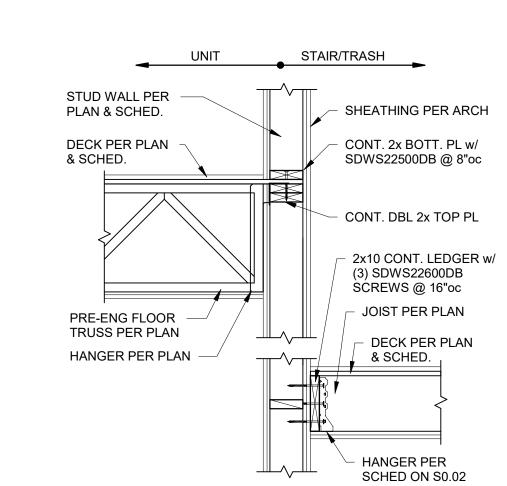
3 **SECTION**3/4" = 1'-0"

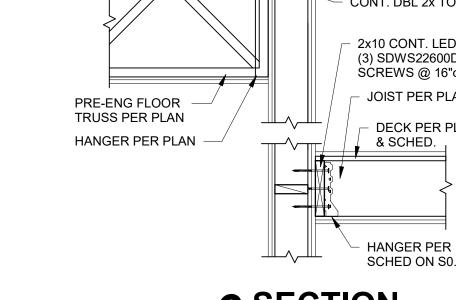
RAILING POST PER ARCH/SPECS

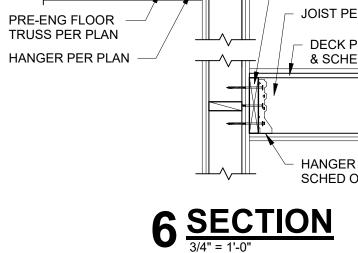
BENT PLATE PER 2/S1.10













7 SECTION 3/4" = 1'-0"

8 <u>SECTION</u>
3/4" = 1'-0"

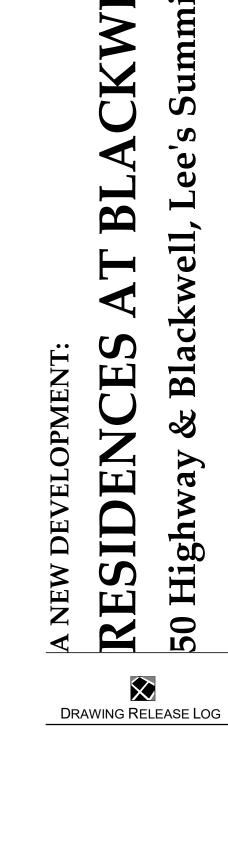
SIMPSON DTT2Z ATTACHED TO STAIR TREAD W/ 1/2" THRU-BOLT THRU DOUBLE STRINGER EACH SIDE OF RAILING POST

- STAIR LANDING BEAM JAMB STUDS PER SCHED. w/ MIN. 1 FULL HT STUD EA. SIDE OF BEAM. TOTAL NUMBER OF STUDS NOT

FLR. T/SUBFLR.

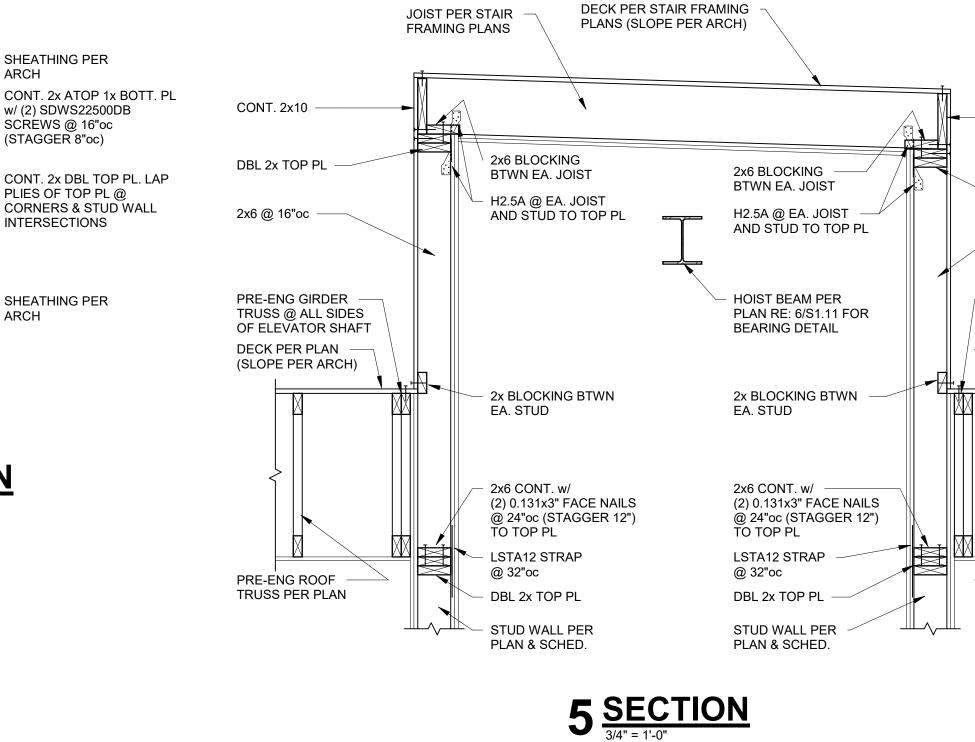
FLR. T/SUBFLR.

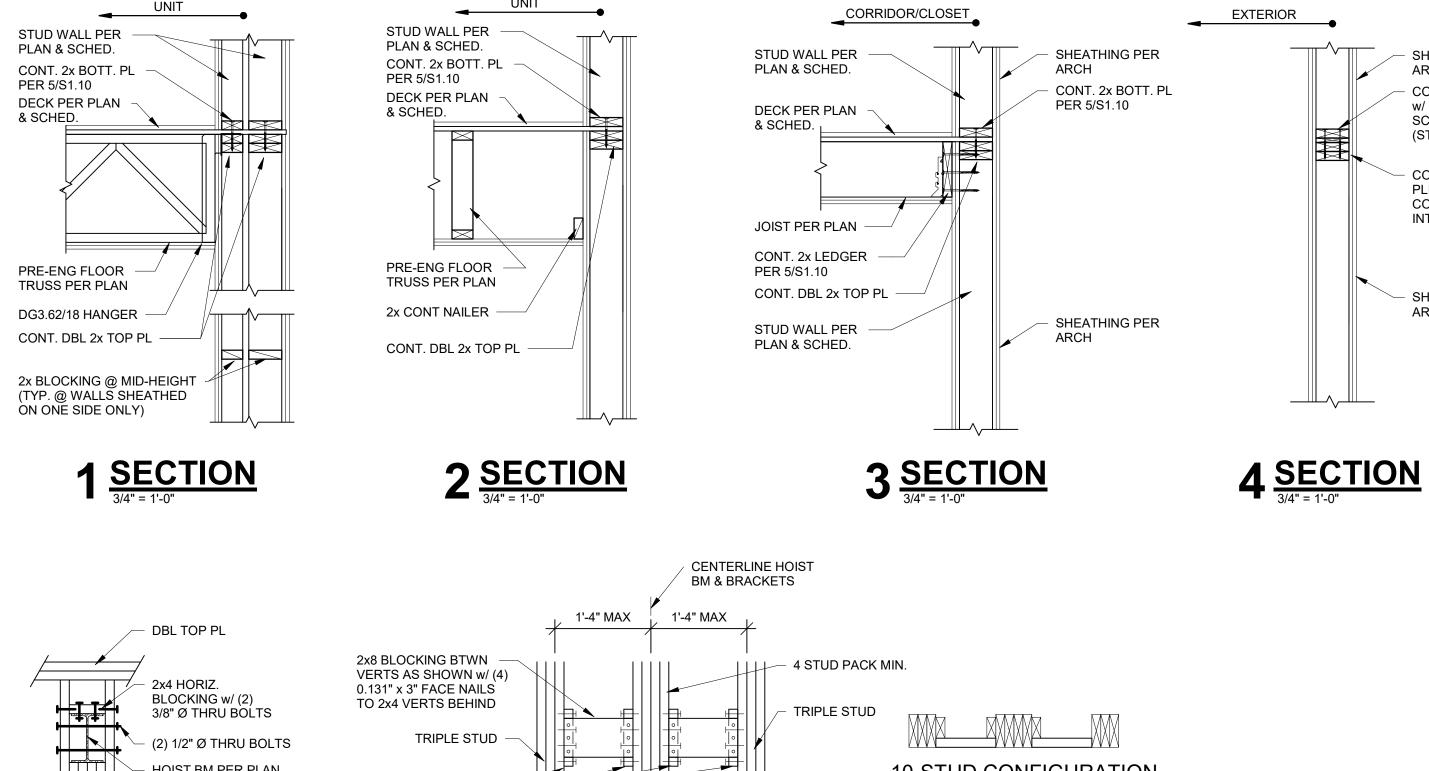




△REVISIONS:

DATE: 3/24/2023 JOB NO.





CORRIDOR/CLOSET

SHEATHING PER

w/ (2) SDWS22500DB

PLIES OF TOP PL @

INTERSECTIONS

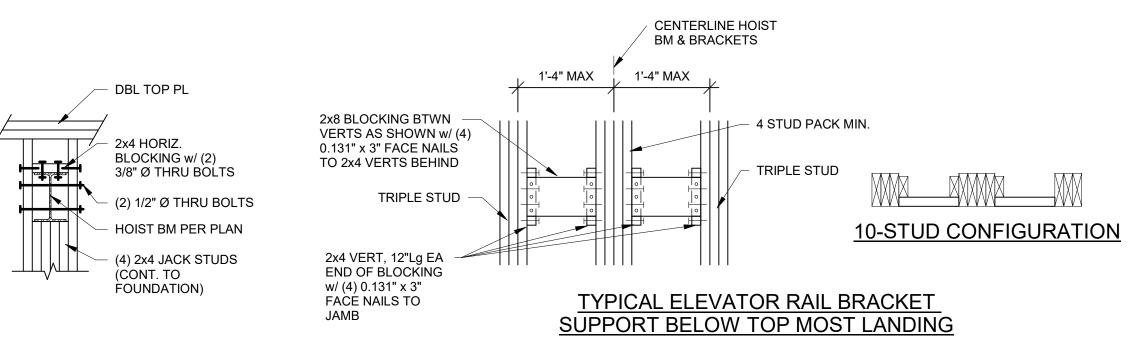
SHEATHING PER

CORNERS & STUD WALL

SCREWS @ 16"oc

(STAGGER 8"oc)

ARCH



6 <u>SECTION</u>
3/4" = 1'-0"

7 SECTION 3/4" = 1'-0"

TRUSS SUPPLIER **5A** <u>SECTION</u> 3/4" = 1'-0"

CONT. TAPERED 2x12

2x6 @ 16"oc

PRE-ENG GIRDER — TRUSS @ ALL SIDES OF ELEVATOR SHAFT

DECK PER PLAN —— (SLOPE PER ARCH)

PRE-ENG ROOF

HANGER PER

TRUSS PER PLAN

PRE-ENG ROOF TRUSS PER PLAN

- CONT. 2x10

- DBL 2x TOP PL

2x6 @ 16"oc

PRE-ENG GIRDER

DECK PER PLAN

(SLOPE PER ARCH)

TRUSS @ ALL SIDES

OF ELEVATOR SHAFT

2x BLOCKING BTWN EA. STUD 2x6 CONT. w/
 (2) 0.131x3" FACE NAILS
 @ 24"oc (STAGGER 12")
 TO TOP PL LSTA12 STRAP @ 32"oc DBL 2x TOP PL - STUD WALL PER BEVERLIN PLAN & SCHED.

DECK PER STAIR

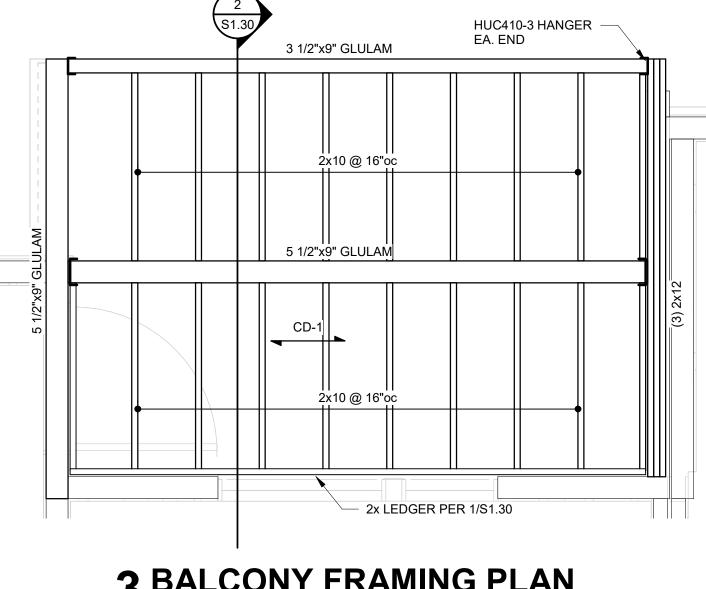
FRAMING PLANS

(SLOPE PER ARCH)

JOIST PER STAIR

FRAMING PLANS

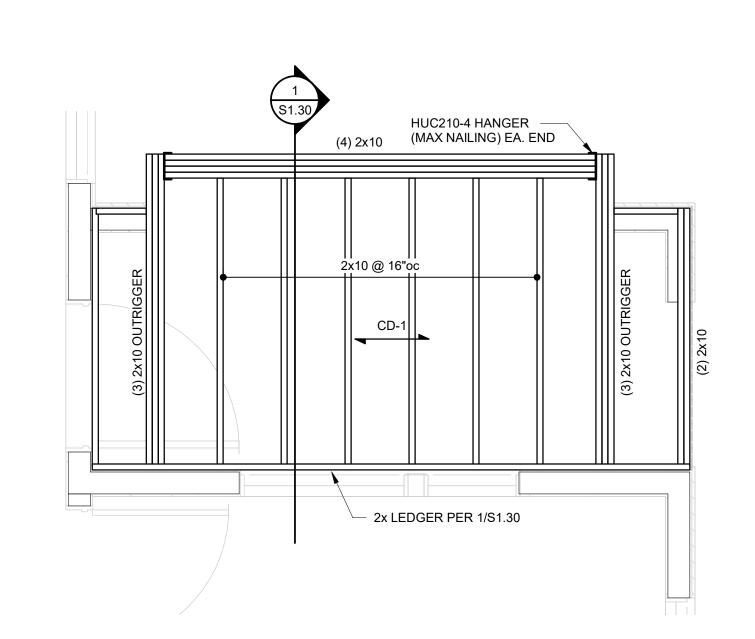
2x LEDGER PER 1/S1.30



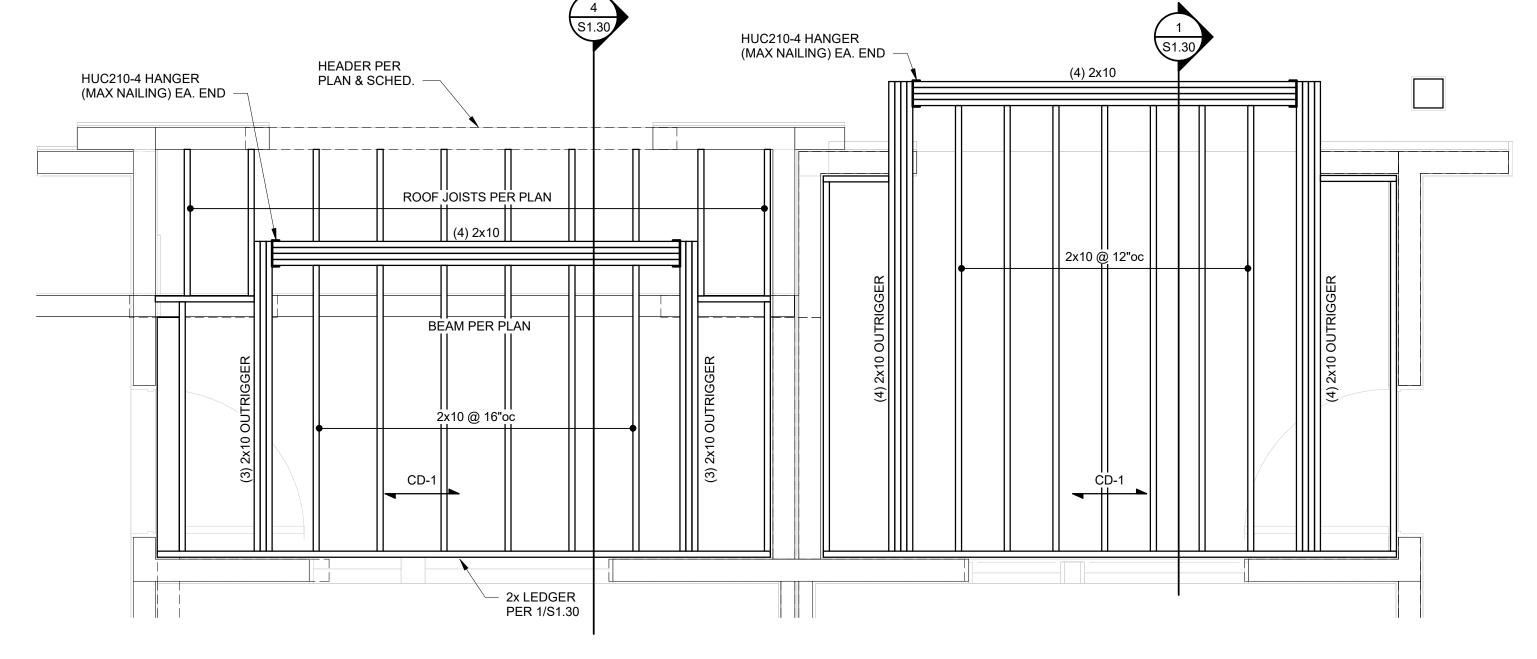
1 BALCONY FRAMING PLAN 1/2" = 1'-0"

2 BALCONY FRAMING PLAN 1/2" = 1'-0"

3 BALCONY FRAMING PLAN
1/2" = 1'-0"



2x LEDGER PER 1/S1.30

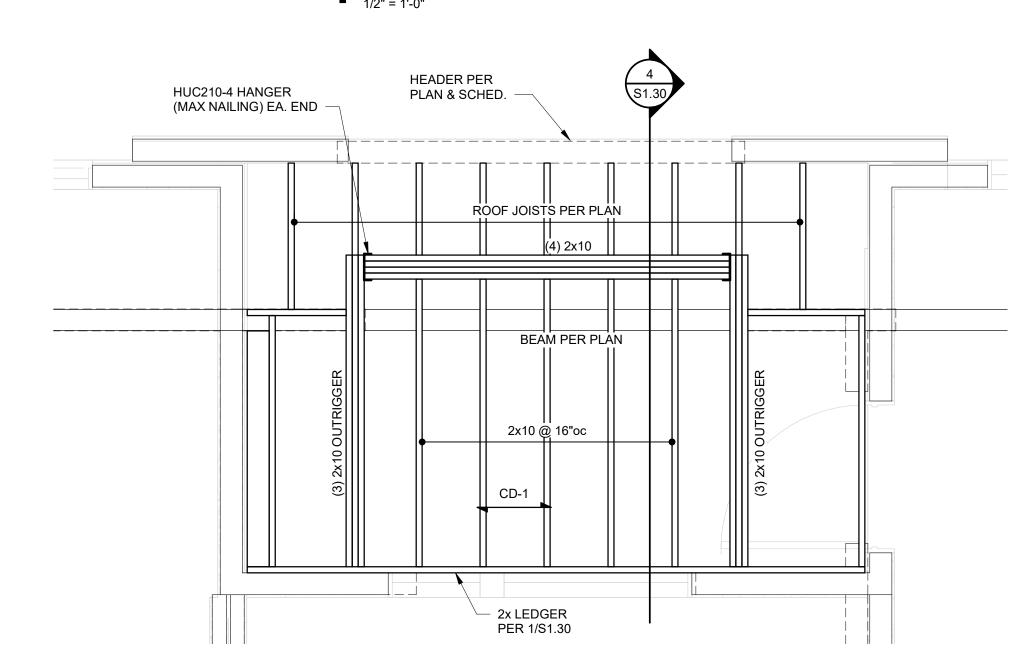


4 BALCONY FRAMING PLAN

1/2" = 1'-0"

5 BALCONY FRAMING PLAN

1/2" = 1'-0"



6 BALCONY FRAMING PLAN
1/2" = 1'-0"

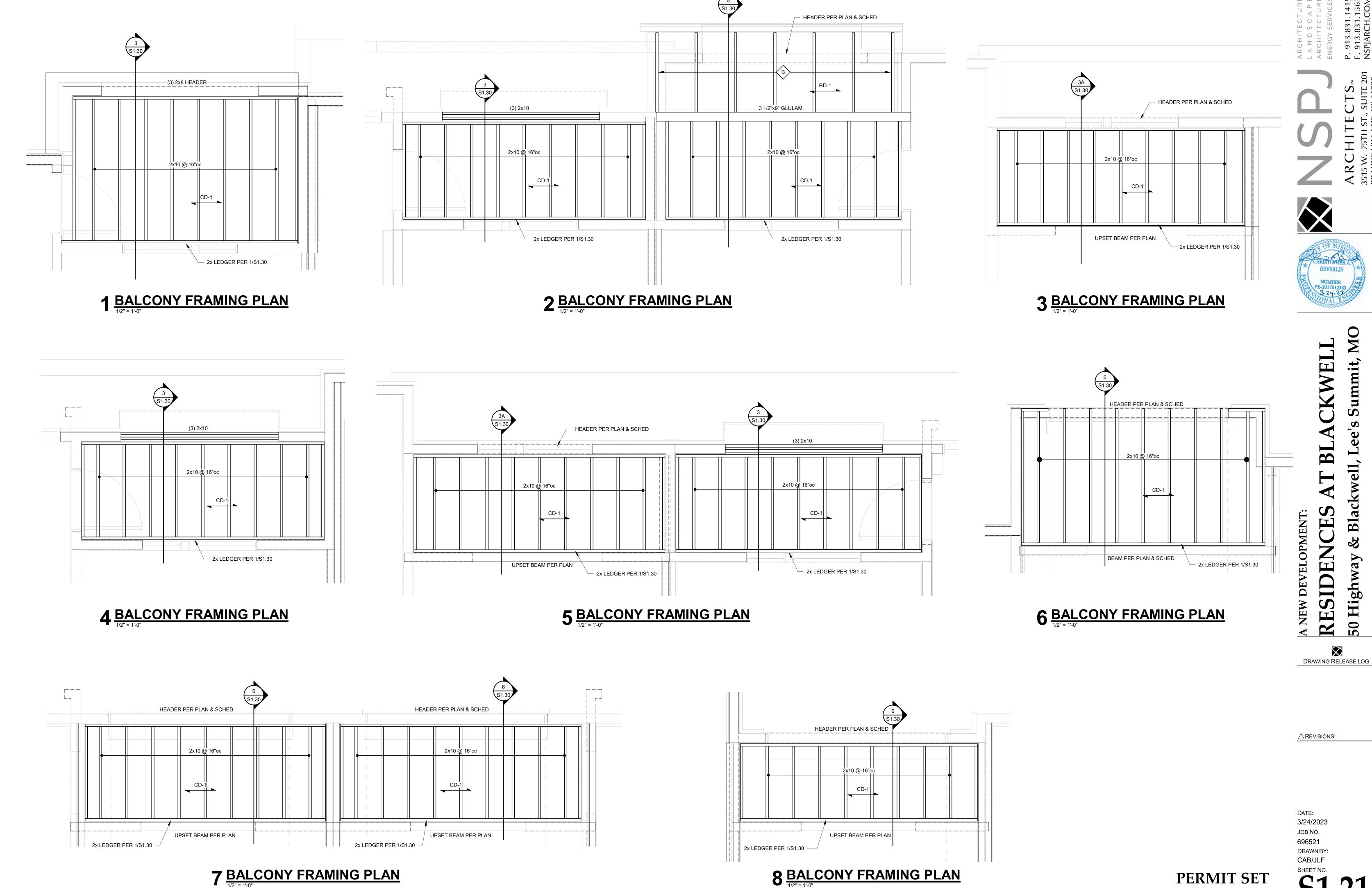
DATE: 3/24/2023 JOB NO. 696521 DRAWN BY: CAB/JLF

△REVISIONS:

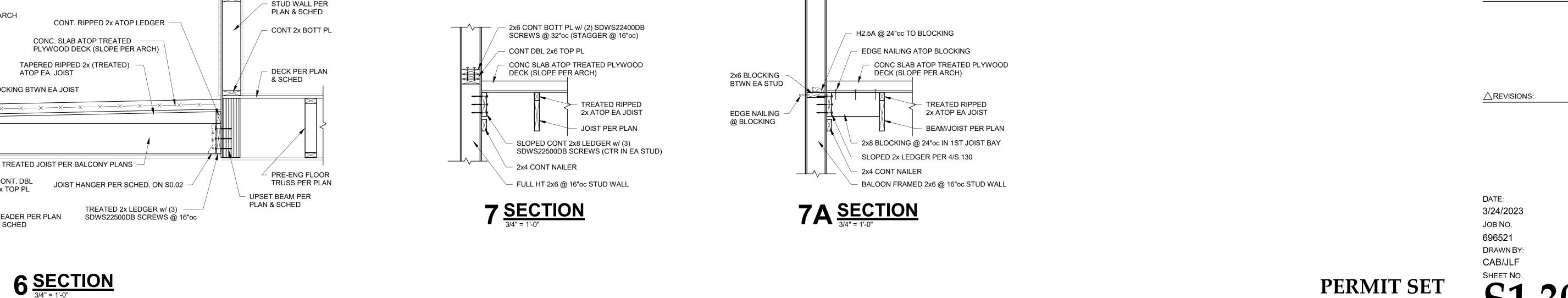
RESIDENCES

DRAWING RELEASE LOG

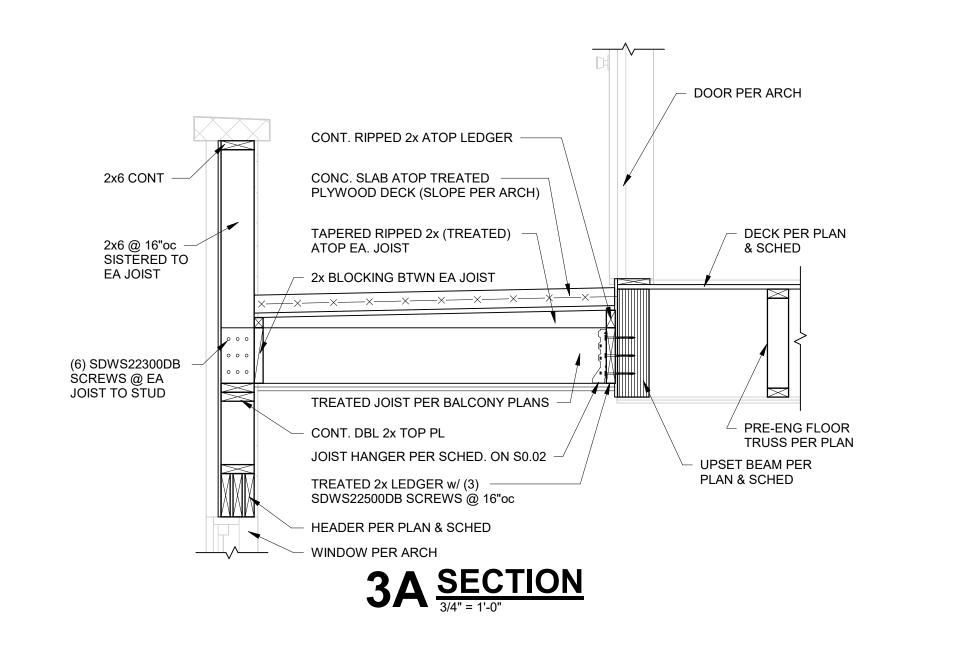
50 Highway



PERMIT SET



(2) 2x6 CONT



RAILING PER ARCH

2x10 CONT

ATOP EA. JOIST

2x BLOCKING BTWN EA JOIST

2x TOP PL

& SCHED

HEADER PER PLAN

EXTERIOR VENEER VARIES

TREATED PLYWOOD DECK

(TREATED) ATOP EA. JOIST

CONT. RIPPED 2x ATOP LEDGER

PER ARCH

- RAILING PER

TREATED BEAM

PER BALCONY

ARCH

CONC. SLAB ATOP

(SLOPE PER ARCH)

TAPERED RIPPED 2x

TREATED JOIST PER

JOIST HANGER PER

SCHED. ON S0.02, TYP.

TREATED 2x LEDGER w/ (3)

SDWS22500DB SCREWS @

1 **SECTION**3/4" = 1'-0"

BALCONY PLANS

WINDOW PER ARCH

2x SILL PL PER 1A/S3.31

STUD WALL PER PLAN

CONT. 2x BOTT. PL

CONT. 1 1/4"x18" LSL

2x SQUASH BLOCKS

2x6 KICKER & BRACING

FRAMING PER 3/S3.31

CONT. DBL 2x TOP PL

WINDOW PER ARCH

SILL PL PER 1/S3.31

HEADER PER PLAN

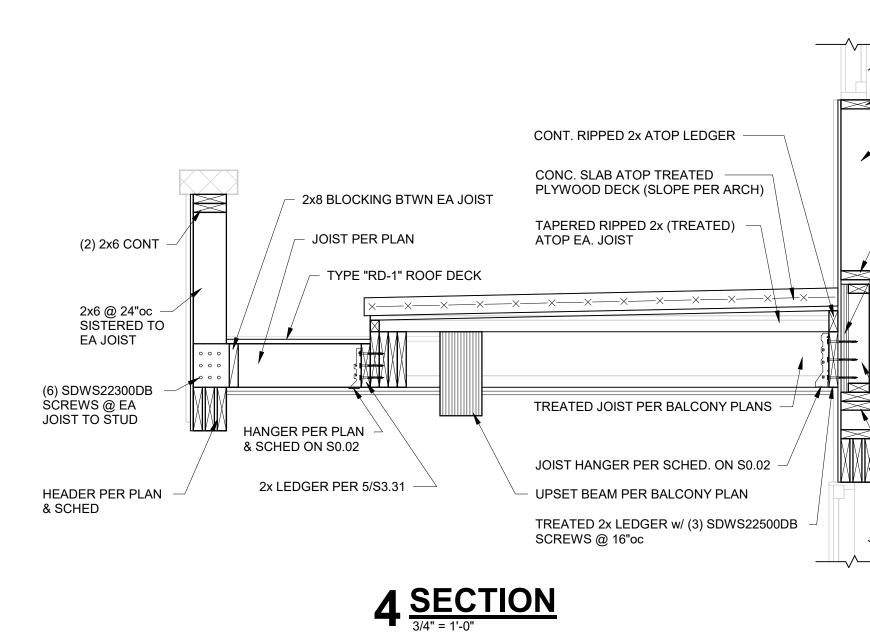
& SCHED

& SCHED.

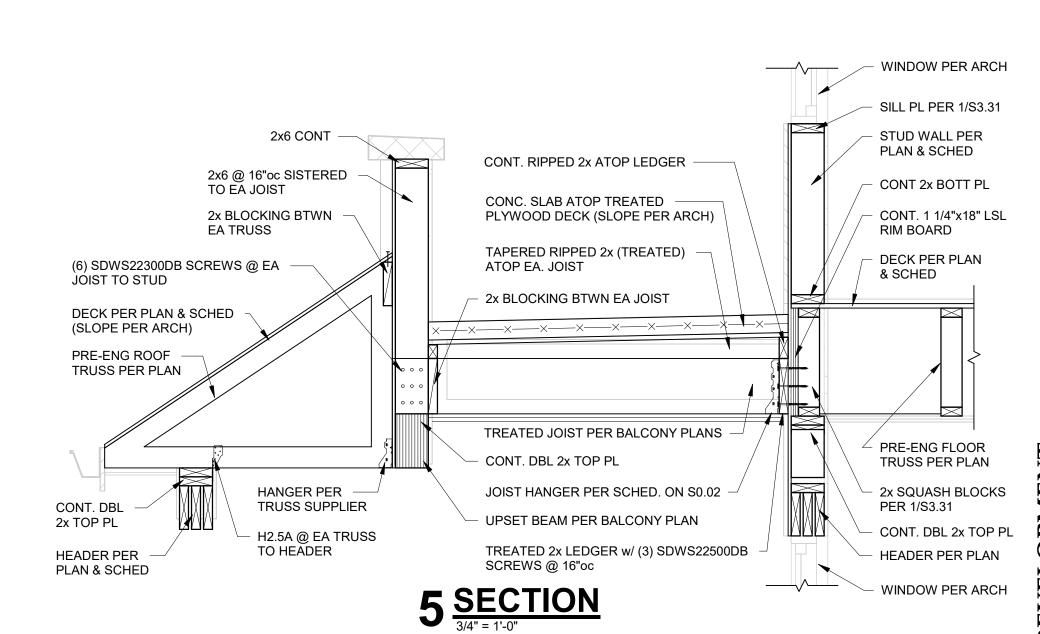
RIM BOARD

PER 1/S3.31

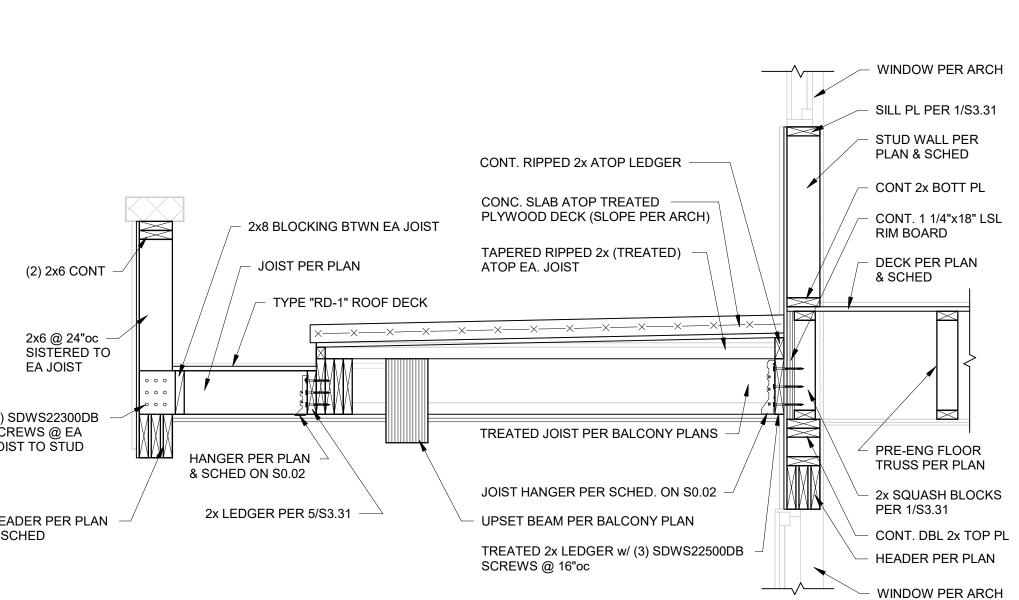
DECK PER PLAN

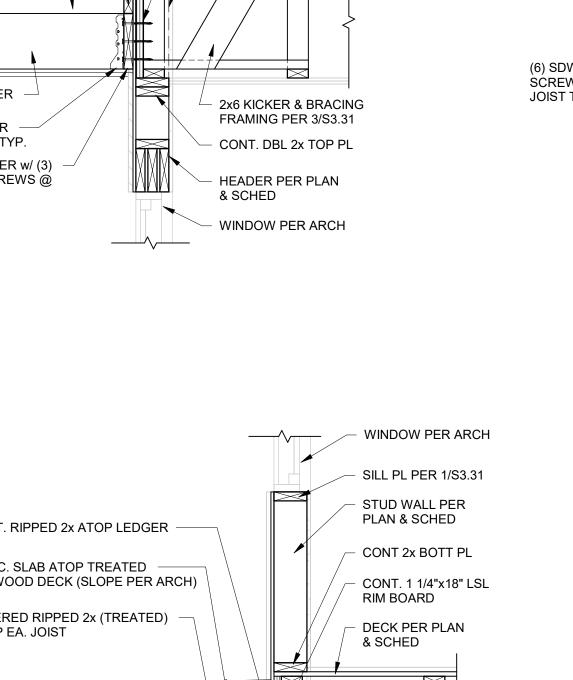


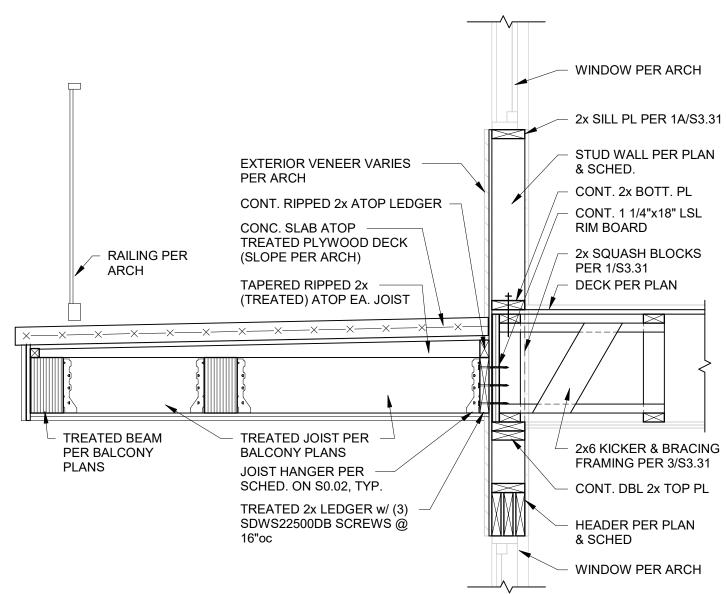
2 **SECTION**3/4" = 1'-0"

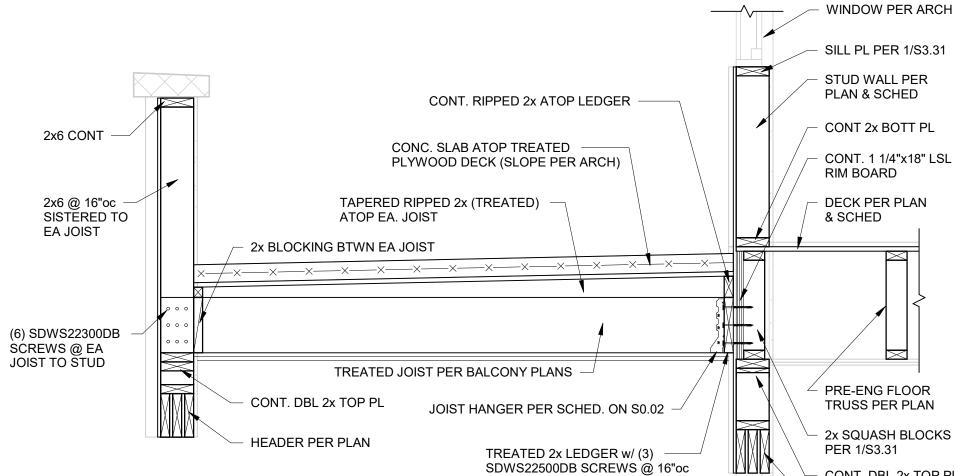


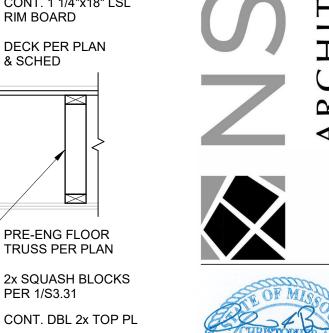
3 **SECTION**3/4" = 1'-0"











HEADER PER PLAN

WINDOW PER ARCH

& SCHED



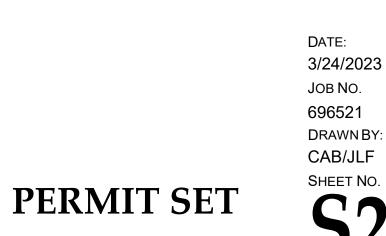
3.24.23

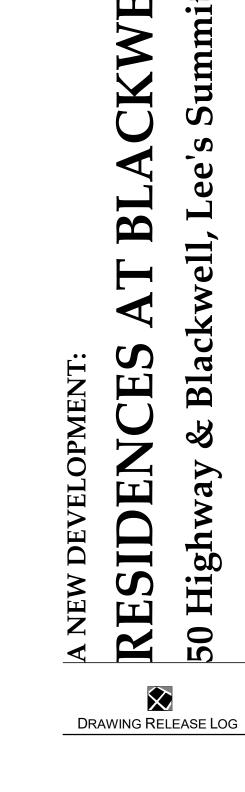
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DRAWING RELEASE LOG

PERMIT SET





 \triangle REVISIONS:

1 LOWER LEVEL FOUNDATION PLAN
3/32" = 1'-0"

FOUNDATION NOTES:

1) REFER TO GENERAL NOTES ON SHEET S0.01.

2) REFER TO CIVIL AND ARCH DRAWING FOR SLAB ELEVATIONS.

3) ELEVATION 100'-0" VARIES PER BUILDING - RE: CIVIL. 1) TOP OF FOOTING ELEVATIONS PER PLAN.

REFER TO FOOTING SCHEDULE ON S0.10.) REFER TO ARCH AND MECH DRAWINGS FOR LOCATIONS OF SPOT AND TRENCH DRAINS.) REFER TO CONCRETE COLUMN SCHEDULE ON S0.10.

8) REFER TO \$3.00 SERIES DRAWINGS FOR TYPICAL FOUNDATION DETAILS.
9) PROVIDE POCKET AT TOP OF FOUNDATION WALL TO RECEIVE GRADE BEAM ABOVE PER 15/S3.00

SITE —— WALLS BY OTHERS

40'-6"

26'-0"

39'-0"

2 LOWER LEVEL FOUNDATION PLAN
3/32" = 1'-0"

WALL TYPE 1

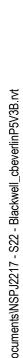


39'-6"

39'-6"

(M.7)

- SITE WALLS BY OTHERS







_SW3-3

@ BLDG 2 2 ONLY S2.01



T/SLAB =

ਨੀ ^{110'-8 3/4}" ਨੀ।

26'-0"

(D)

26'-0"

1 1ST FLOOR FRAMING & FOUNDATION PLAN 3/32" = 1'-0"

26'-0"

FOUNDATION NOTES:

1) REFER TO GENERAL NOTES ON SHEET S0.01.

2) REFER TO CIVIL AND ARCH DRAWING FOR SLAB ELEVATIONS. 3) ELEVATION 100'-0" VARIES PER BUILDING - RE: CIVIL. 4) TOP OF FOOTING ELEVATIONS PER PLAN. 5) REFER TO FOOTING SCHEDULE ON S0.10. 6) REFER TO ARCH AND MECH DRAWINGS FOR LOCATIONS OF SPOT AND TRENCH DRAINS.

7) REFER TO CONCRETE COLUMN SCHEDULE ON S0.10.8) REFER TO S3.00 SERIES DRAWINGS FOR TYPICAL FOUNDATION DETAILS. 9) PROVIDE POCKET AT TOP OF FOUNDATION WALL TO RECEIVE GRADE BEAM ABOVE PER 15/S3.00

WOOD FLOOR FRAMING NOTES:

1) REFER TO GENERAL NOTES ON SHEET S0.01 2) REFER TO STUD BEARING WALL SCHEDULE TO SHEET S0.02

3) REFER TO HEADER/BEAM SCHEDULE ON SHEET S0.02

4) REFER TO SHEARWALL SCHEDULE ON SHEET S0.03 5) REFER TO STAIR FRAMING PLANS ON SHEET S2.00 6) REFER TO BALCONY FRAMING PLANS ON SHEET S1.20

REFER TO S3.30-SERIES DRAWINGS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE 8) PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSETS; REFER TO ARCH & MEP DRAWINGS FOR EXACT LOCATIONS

9) - STORAGE AREA: DESIGN FOR LL PER GENERAL NOTE 2.B ON S0.01 10) TOP OF STEEL ELEVATION FOR STEEL BEARING ON WOOD MEMBERS SHALL BE FIELD COORDINATED BY THE CONTRACTOR & ARCHITECTURAL ROUGH OPENING SCHEDULE.

CONCRETE FRAMING NOTES:

1) REFER TO GENERAL NOTES ON SHEET S0.01. 2) REFER TO CIVIL AND ARCH DRAWINGS FOR SLAB ELEVATIONS.

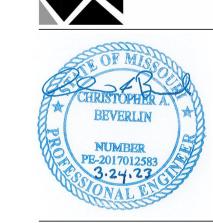
5) REFER TO STRUCTURAL SLAB (PODIUM SLAB) NOTES ON S0.10.

3) REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS OF SPOT AND TRENCH DRAINS. 4) REFER TO CONCRETE COLUMN SCHEDULE ON S0.10. 6) ELEVATIONS AND SLAB STEPS INDICATED OCCUR IN THE STRUCTURAL SLAB. REFER TO ARCH FOR SLOPES OF TOPPING SLAB.

7) ALL ADDITIONAL REINFORCEMENT (PER PLAN) SHALL PROJECT TO OPPOSITE FACE OR STRIP & LAP WITH SAME SIZE BAR.











DRAWING RELEASE LOG

△REVISIONS:

DATE:

3/24/2023 JOB NO.

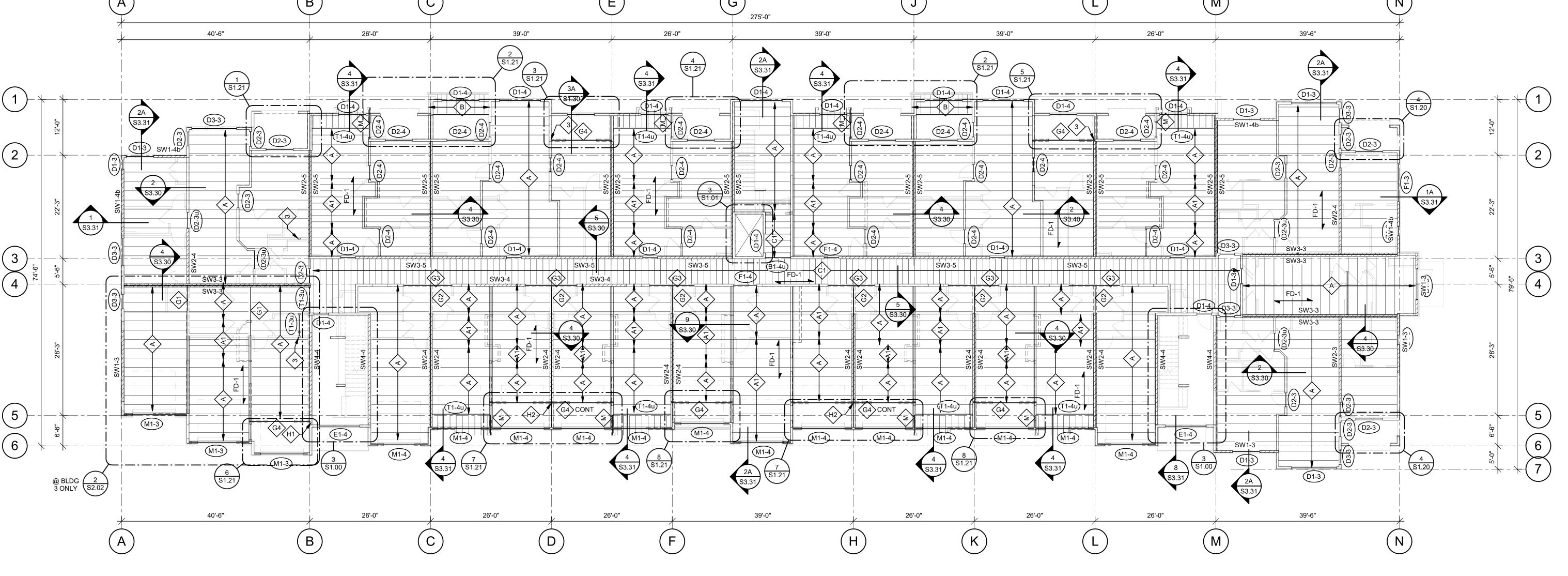
696521

DRAWN BY:

12" CONCRETE 2-WAY SLAB T/SLAB = 110'-8 3/4"

(M.7)

CAB/JLF PERMIT SET



1 2ND FLOOR FRAMING PLAN 3/32" = 1'-0"

- WOOD FLOOR FRAMING NOTES:

 1) REFER TO GENERAL NOTES ON SHEET S0.01

 2) REFER TO STUD BEARING WALL SCHEDULE TO SHEET S0.02

 3) REFER TO HEADER/BEAM SCHEDULE ON SHEET S0.02
- 4) REFER TO SHEARWALL SCHEDULE ON SHEET S0.03 REFER TO STAIR FRAMING PLANS ON SHEET S2.00
- REFER TO BALCONY FRAMING PLANS ON SHEET S1.20 7) REFER TO S3.30-SERIES DRAWINGS FOR ADDITONAL FLOOR FRAMING DETAILS NOT INDICATED HERE 8) PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSETS; REFER TO ARCH &
- MEP DRAWINGS FOR EXACT LOCATIONS
- 9) STORAGE AREA: DESIGN FOR LL PER GENERAL NOTE 2.B ON S0.01 10) TOP OF STEEL ELEVATION FOR STEEL BEARING ON WOOD MEMBERS SHALL BE FIELD COORDINATED BY THE CONTRACTOR & ARCHITECTURAL ROUGH OPENING SCHEDULE.

2 2ND FLOOR FRAMING PLAN
3/32" = 1'-0"

_SW3-3

BUCKET ON

_WF BEAM

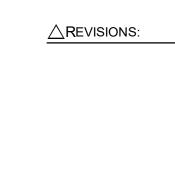
_PER 4/S3.20

DATE: 3/24/2023 JOB NO. 696521 DRAWN BY: CAB/JLF

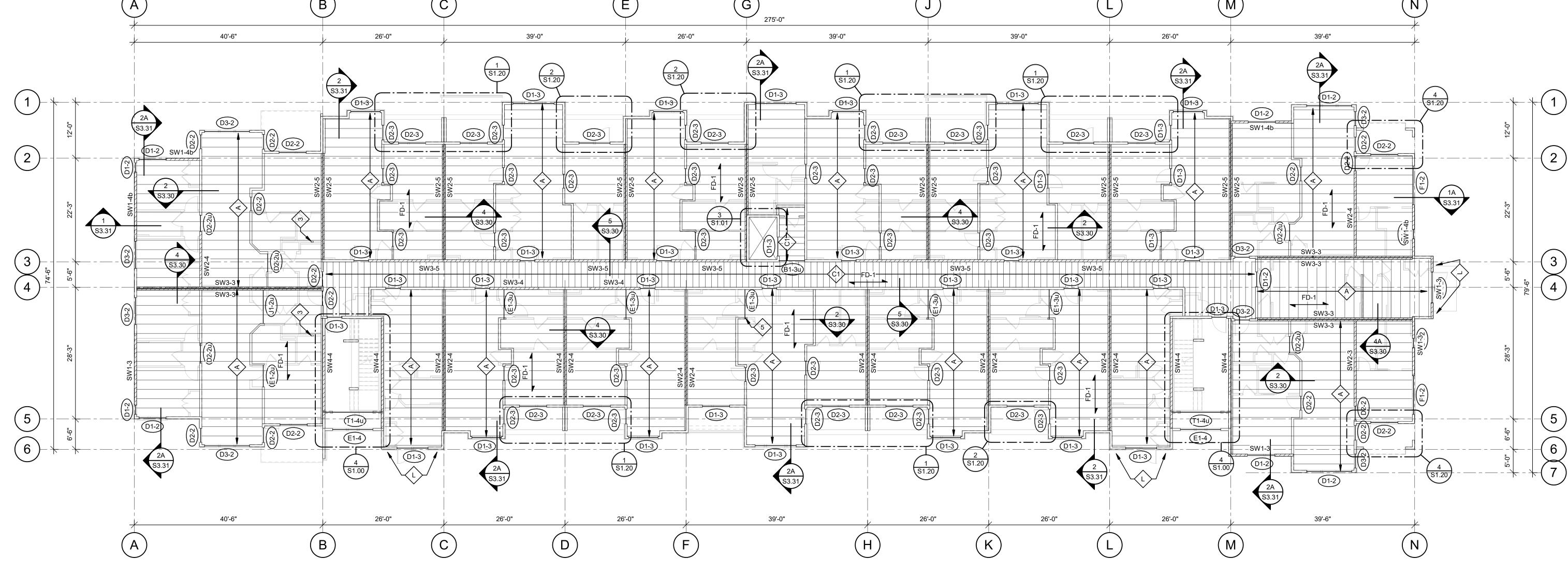
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DRAWING RELEASE LOG

ENCES



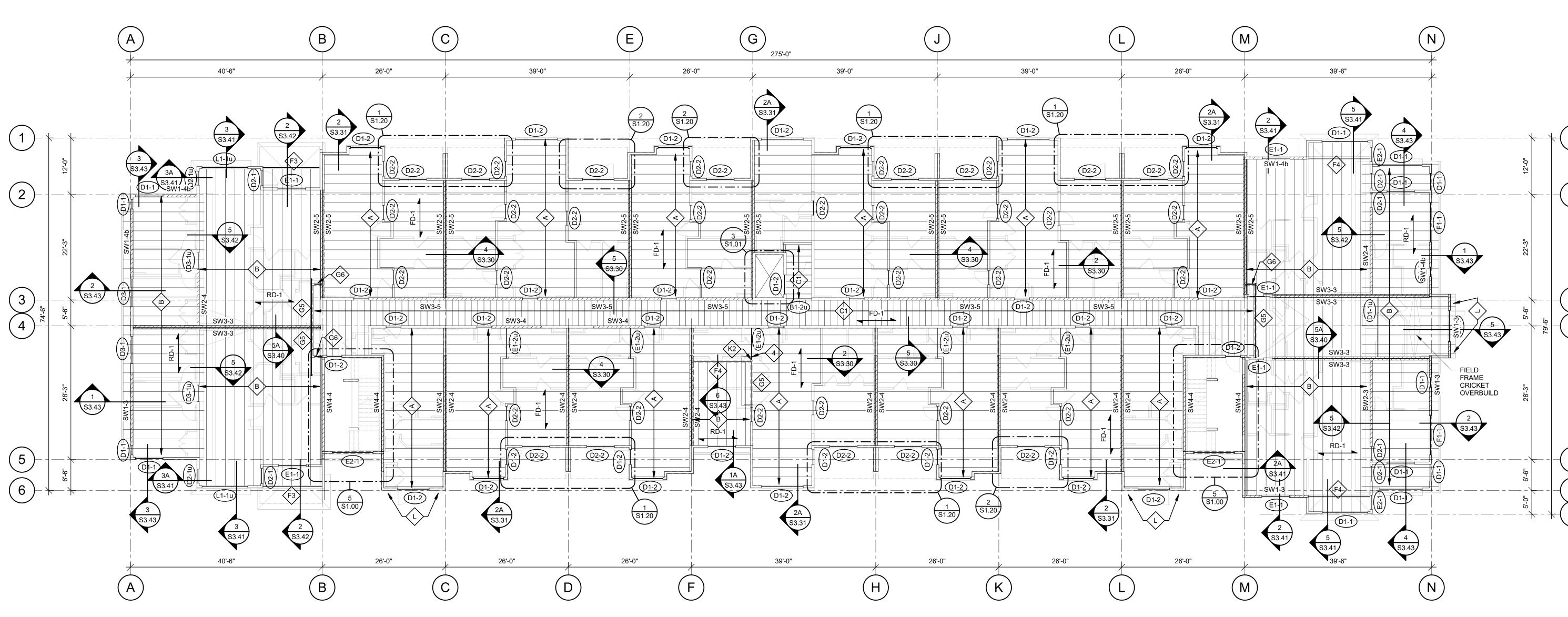
DATE: 3/24/2023 JOB NO. 696521



1 3RD FLOOR FRAMING PLAN 3/32" = 1'-0"

- WOOD FLOOR FRAMING NOTES:

 1) REFER TO GENERAL NOTES ON SHEET S0.01
- 2) REFER TO STUD BEARING WALL SCHEDULE TO SHEET S0.02 S) REFER TO HEADER/BEAM SCHEDULE ON SHEET S0.02 4) REFER TO SHEARWALL SCHEDULE ON SHEET S0.03
- REFER TO STAIR FRAMING PLANS ON SHEET S2.00 6) REFER TO BALCONY FRAMING PLANS ON SHEET S1.20 7) REFER TO S3.30-SERIES DRAWINGS FOR ADDITONAL FLOOR FRAMING DETAILS NOT INDICATED HERE 8) PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSETS; REFER TO ARCH &
- MEP DRAWINGS FOR EXACT LOCATIONS
- 9) STORAGE AREA: DESIGN FOR LL PER GENERAL NOTE 2.B ON S0.01 10) TOP OF STEEL ELEVATION FOR STEEL BEARING ON WOOD MEMBERS SHALL BE FIELD COORDINATED BY THE CONTRACTOR & ARCHITECTURAL ROUGH OPENING SCHEDULE.



1 4TH FLOOR FRAMING PLAN 3/32" = 1'-0"

- WOOD FLOOR FRAMING NOTES:

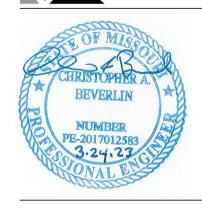
 1) REFER TO GENERAL NOTES ON SHEET S0.01 2) REFER TO STUD BEARING WALL SCHEDULE TO SHEET S0.02 3) REFER TO HEADER/BEAM SCHEDULE ON SHEET S0.02
- 4) REFER TO SHEARWALL SCHEDULE ON SHEET S0.03 REFER TO STAIR FRAMING PLANS ON SHEET S2.00
- 6) REFER TO BALCONY FRAMING PLANS ON SHEET S1.20 7) REFER TO S3.30-SERIES DRAWINGS FOR ADDITONAL FLOOR FRAMING DETAILS NOT INDICATED HERE 8) PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSETS; REFER TO ARCH &
- MEP DRAWINGS FOR EXACT LOCATIONS 9) - STORAGE AREA: DESIGN FOR LL PER GENERAL NOTE 2.B ON S0.01
- 10) TOP OF STEEL ELEVATION FOR STEEL BEARING ON WOOD MEMBERS SHALL BE FIELD COORDINATED BY THE CONTRACTOR & ARCHITECTURAL ROUGH OPENING SCHEDULE.

DRAWING RELEASE LOG

 \triangle REVISIONS:

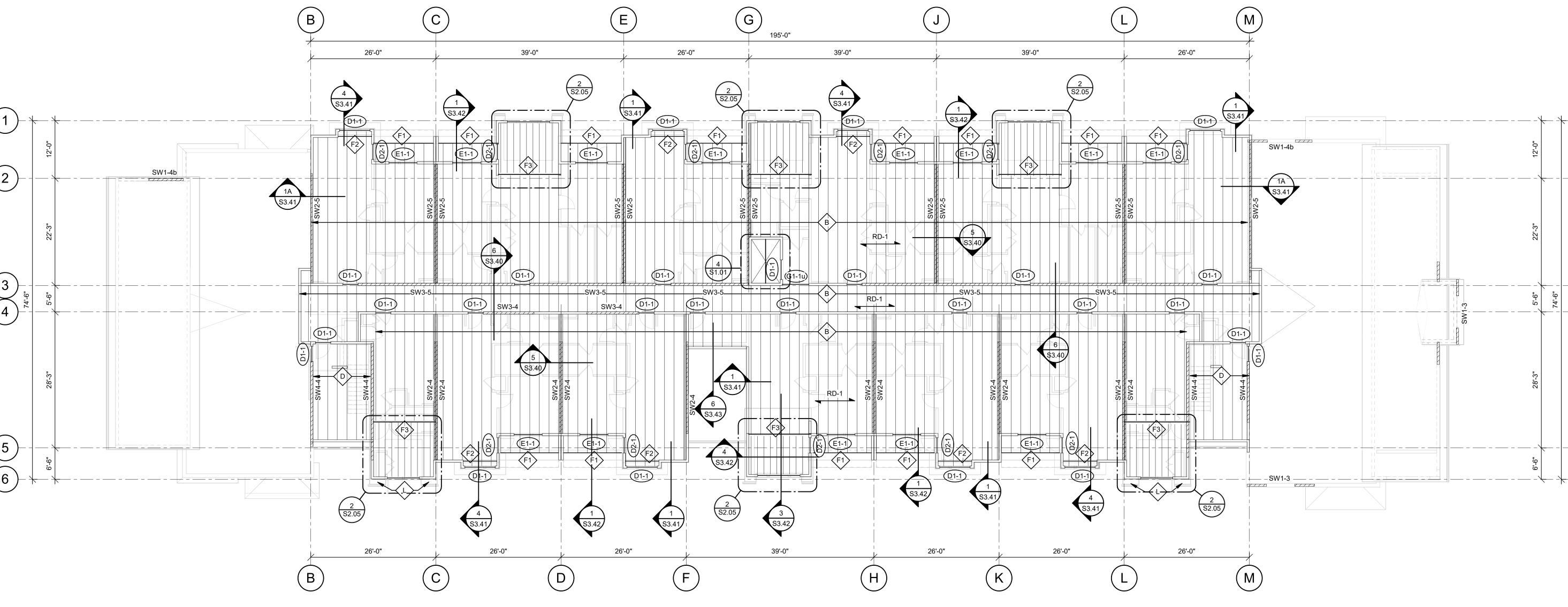
DATE:

3/24/2023 JOB NO.







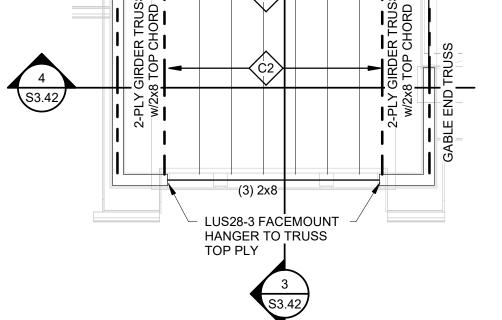


1 ROOF FRAMING PLAN 3/32" = 1'-0"

- WOOD ROOF FRAMING NOTES:

 1) REFER TO GENERAL NOTES ON SHEET S0.01
- 2) REFER TO STUD BEARING WALL SCHEDULE TO SHEET S0.02 3) REFER TO HEADER/BEAM SCHEDULE ON SHEET S0.02 4) REFER TO SHEARWALL SCHEDULE ON SHEET S0.03
- 5) PROVIDE (3) STUD (MINIMUM) ALIGNED UNDER EACH END OF GIRDER TRUSS (CONTINUOUS FOUNDATION) FINAL QUANTITY TO MATCH NUMBER OF PLIES OF GIRDER TRUSS. PROVIDE SIMPSON LSTA-STYLE HOLDOWN AT EACH END
- OF GIRDER TRUSS.
- 6) REFER TO S3.40-SERIES DRAWINGS FOR ADDITIONAL ROOF FRAMING DETAILS NOT INDICATED HERE.
 7) PROVIDE UNIFORM UPLIFT SCREWS AT UPPER FLOOR PER DETAILS 2, 2A, 3, 3A, 3B, 4 AND 5 ON SHEET S0.20.
- 8) PRE-ENGINEERED TRUSSES TO HAVE A MINIMUM DEPTH OF 24". SLOPE TOP CHORD PER ARCHITECTUAL DRAWINGS.
- 9) INDICATES AREA ON ROOF THAT IS REQ'D TO BE DESIGNED FOR MEP EQUIPMENT ZONE PER GENERAL NOTE "2.B" ON SHEET S0.01
- 10) TOP OF STEEL ELEVATION FOR STEEL BEARING ON WOOD MEMBERS SHALL BE FIELD COORDINATED BY THE CONTRACTOR & ARCHITECTURAL ROUGH OPENING SCHEDULE.

11) PRE-ENG TRUSSES TO HAVE SLOPING TOP CHORD WITH MINIMUM TRUSS DEPTH OF 24".



2 ROOF FRAMING PLAN

1/4" = 1'-0"

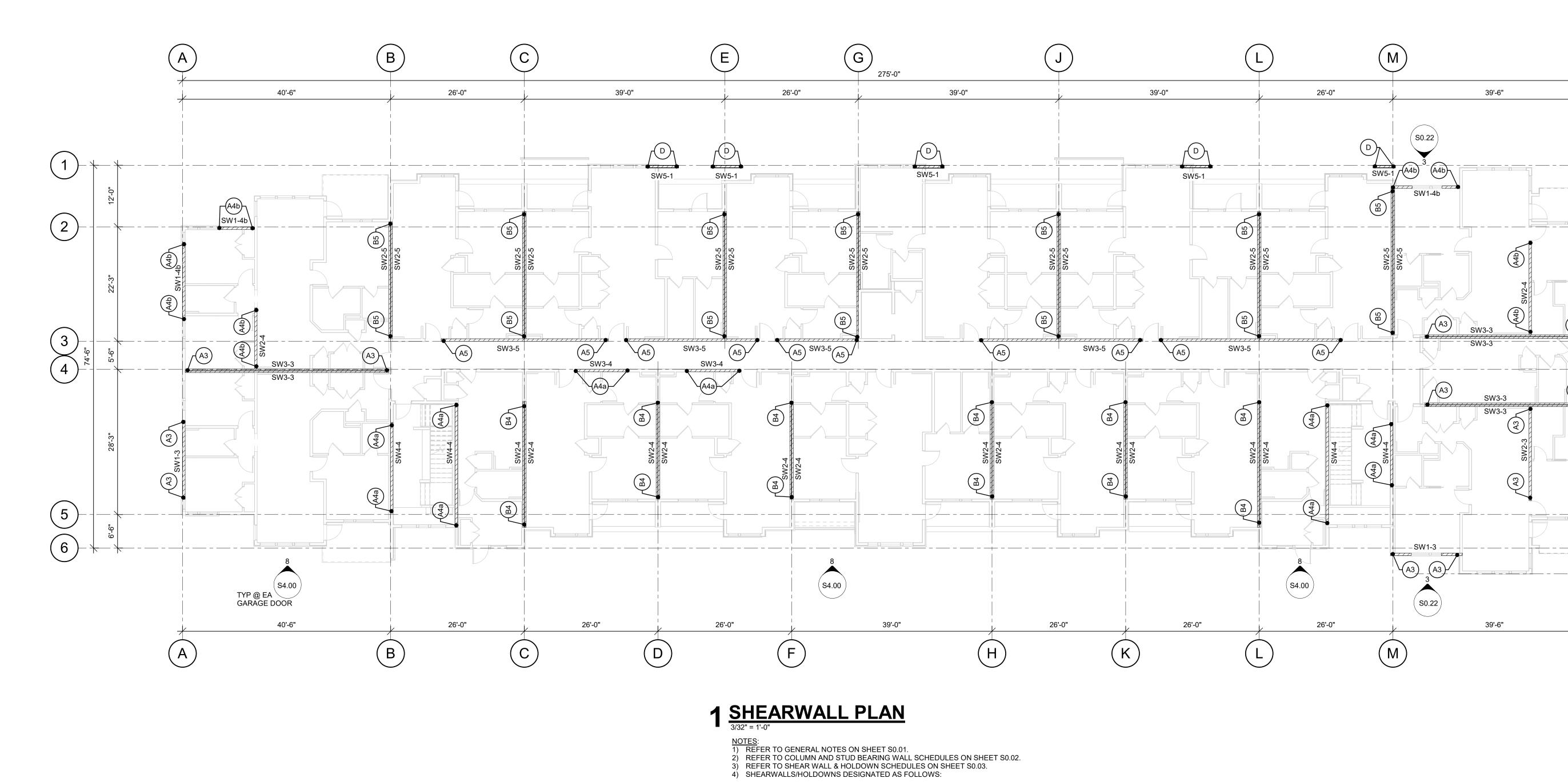
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- SHEAR WALL TYPE

ATTACHMENT AT EXTENTS OF SHEARWALLS.

— SHEARWALL EXTENTS INDICATED WITH HATCHED AREA

5) ALL EXTERIOR WALLS NOT SPECIFICALLY DESIGNATED AS A STRUCTURAL SHEARWALL SHALL BE SHEATHED w/ 7/16" OSB w/ 8d NAILS @ 6"oc EDGES @ 12"oc FIELD.

6) REFER TO DETAILS 15 THRU 15D ON S0.20 FOR SILL PLATE AND RIM BOARD

HOLDOWN TYPE MARK: (1) HOLDOWN TYPICAL EACH END OF SHEARWALL (OF TYPE INDICATED) U.N.O. PER SHEARWALL SCHED. RE: SCHED. FOR ADDT'L SPECIFIC REQ'S

RESIDENCES A NEW DEVELOPMENT:

RESIDENCES A

50 Highway & Black

 \triangle REVISIONS:

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DENCES AT BLACKWELL
hway & Blackwell, Lee's Summit, MC



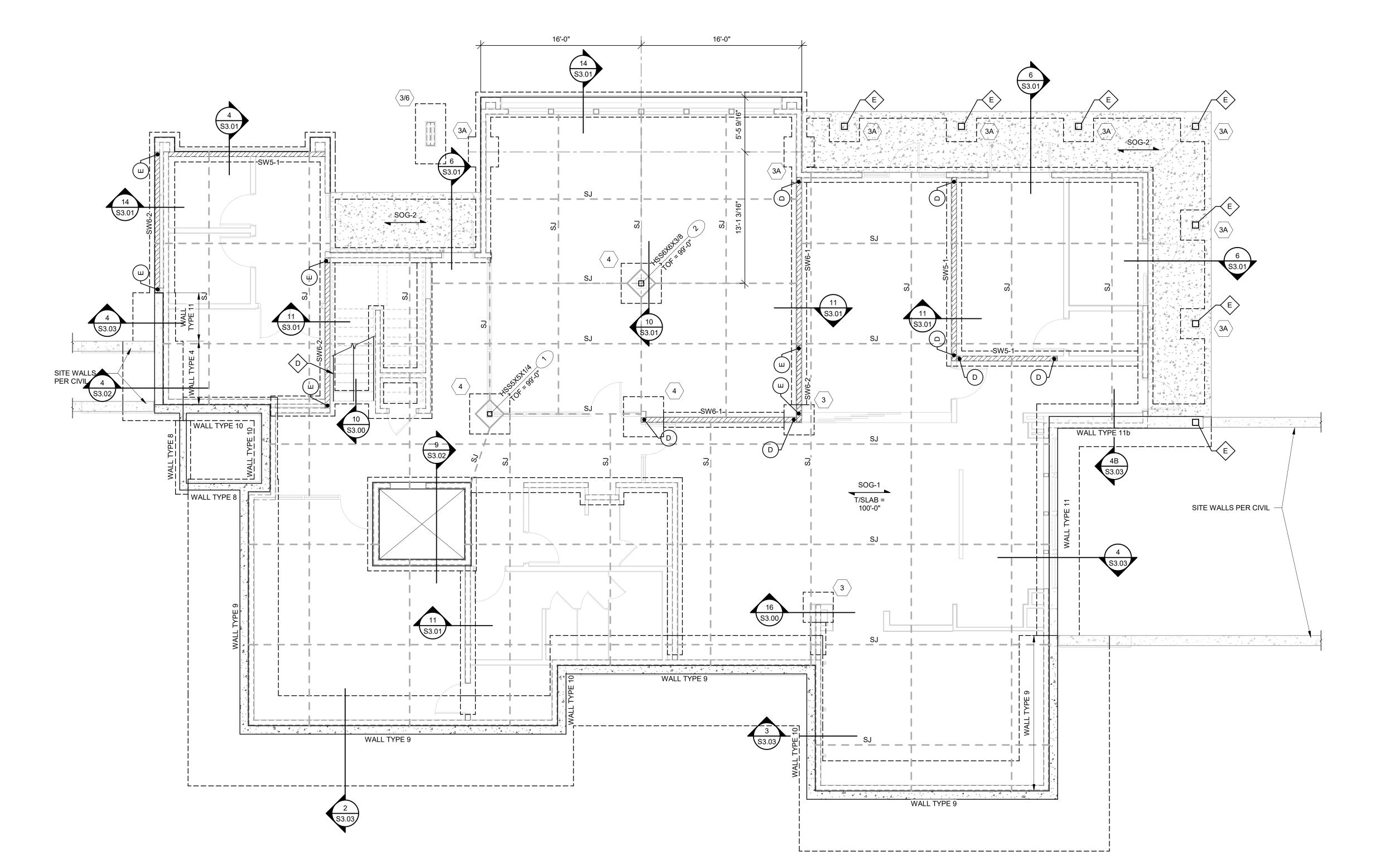
TECTS PARCHITECT ENERGY SERV

TECTS PARCHITECT ENERGY SERV

F. 913.831.7

H ST., SUITE 201 NSPJARCH.C

LAGE. KS 66208 COPTRIGHT



1 CLUBHOUSE FOUNDATION PLAN 3/16" = 1'-0"

FOUNDATION NOTES:

- 1) REFER TO GENERAL NOTES ON SHEET S0.01.
- 2) REFER TO CIVIL AND ARCH DRAWING FOR SLAB ELEVATIONS. 3) ELEVATION 100'-0" VARIES PER BUILDING - RE: CIVIL.
- 4) TOP OF FOOTING ELEVATIONS PER PLAN. 5) REFER TO FOOTING SCHEDULE ON S0.10. 6) REFER TO ARCH AND MECH DRAWINGS FOR LOCATIONS OF SPOT AND TRENCH DRAINS.
- 7) REFER TO CONCRETE COLUMN SCHEDULE ON S0.10. 8) REFER TO \$3.00 SERIES DRAWINGS FOR TYPICAL FOUNDATION DETAILS. 9) PROVIDE POCKET AT TOP OF FOUNDATION WALL TO RECEIVE GRADE BEAM ABOVE PER 15/S3.00

△REVISIONS:

I		
	V1	(4) 1 3/4"x18" LV
	X1	(5) 1 3/4"x18" LV
	Y1	(3) 1 3/4"x24" LV
·	 WHEF ALL E RETA PROV 	STUDS SHALL MAT RE BEAM IS NOTED XTERIOR LUMBER T RDANT TREATED H RIDE SQUASH BLOC

MARK

D1

D2

E1

E2

F2

ATCH SIZE & GRADE OF WALL STUDS U.N.O.

CLUBHOUSE PLAN NOTES

18" DEEP PRE-ENGINEERED FLOOR TRUSSES @ 24"oc MAX

18" DEEP PRE-ENGINEERED FLOOR TRUSSES @ 16"oc MAX

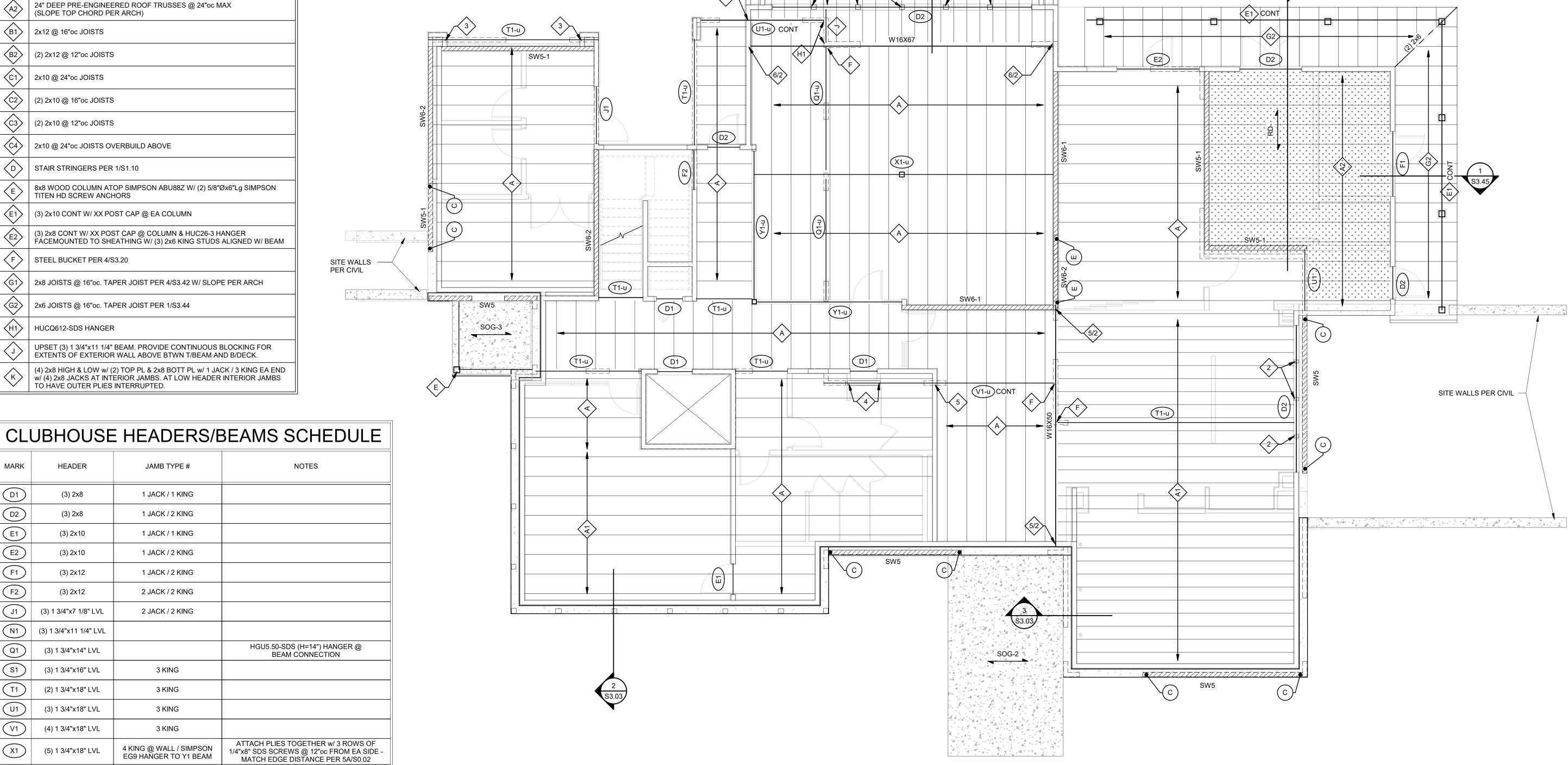
D "-u", ALL JAMB STUDS NOTED WILL EXTEND TO DOUBLE TOP PLATE. R TO BE TREATED AGAINST MOISTURE. REFER TO NOTE 12.T ON SHEET S0.01 FOR FIRE HEADER AND STUD REQUIREMENTS.

CKS AT TRUSSES & BLOCKING FRAMING WHERE JAMBS OR STUD PACKS ARE DISCONT AND IN TRUSS CAVITY. QUANTITY TO MATCH JAMB OR STUD PACK ABOVE.

PROVIDE 1/2" PLYWOOD SPACER PLATES AT INTERIOR HEADERS CONSTRUCTED WITH 2x LUMBER. 6. AT CONTRACTOR'S OPTION, PROVIDE GLULAM IN LIEU OF PSL OF EQUAL OR GREATER STRENGTH.

4 KING

REFER TO DETAIL 4/S0.02 FOR MULTI-PLY MEMBER CONNECTION REQUIREMENTS. 8. ATTACH JAMB & KING STUDS TOGETHER PER CONNECTION TYPE 24 ON NAILING SCHEDULE ON S0.01.



1 CLUBHOUSE 1ST FLOOR FRAMING PLAN 3/16" = 1'-0"

WOOD FLOOR FRAMING NOTES:

1) REFER TO GENERAL NOTES ON SHEET S0.01

2) REFER TO STUD BEARING WALL SCHEDULE TO SHEET S0.02

3) REFER TO HEADER/BEAM SCHEDULE ON SHEET S0.02

4) REFER TO SHEARWALL SCHEDULE ON SHEET S0.03 5) REFER TO STAIR FRAMING PLANS ON SHEET S2.00 6) REFER TO BALCONY FRAMING PLANS ON SHEET S1.20

7) REFER TO S3.30-SERIES DRAWINGS FOR ADDITONAL FLOOR FRAMING DETAILS NOT INDICATED HERE 8) PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSETS; REFER TO ARCH &

MEP DRAWINGS FOR EXACT LOCATIONS 9) - STORAGE AREA: DESIGN FOR LL PER GENERAL NOTE 2.B ON S0.01

10) TOP OF STEEL ELEVATION FOR STEEL BEARING ON WOOD MEMBERS SHALL BE FIELD COORDINATED BY THE CONTRACTOR & ARCHITECTURAL ROUGH OPENING SCHEDULE.

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JAMB STUDS SHALL MATCH SIZE & GRADE OF WALL STUDS U.N.O.

AND IN TRUSS CAVITY. QUANTITY TO MATCH JAMB OR STUD PACK ABOVE.

REFER TO DETAIL 4/S0.02 FOR MULTI-PLY MEMBER CONNECTION REQUIREMENTS.

RETARDANT TREATED HEADER AND STUD REQUIREMENTS.

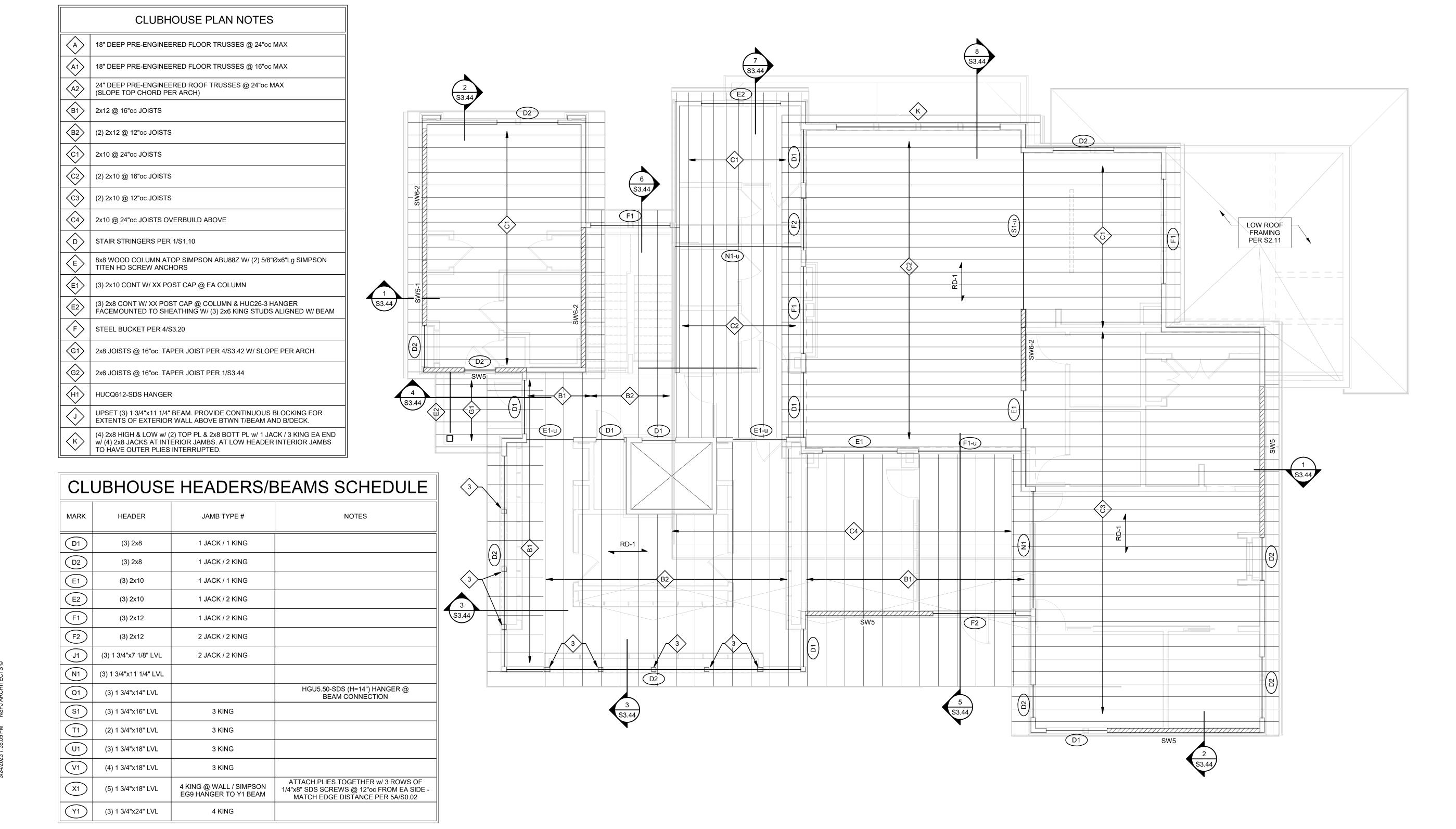
WHERE BEAM IS NOTED "-u", ALL JAMB STUDS NOTED WILL EXTEND TO DOUBLE TOP PLATE.

ALL EXTERIOR LUMBER TO BE TREATED AGAINST MOISTURE. REFER TO NOTE 12.T ON SHEET S0.01 FOR FIRE

4. PROVIDE SQUASH BLOCKS AT TRUSSES & BLOCKING FRAMING WHERE JAMBS OR STUD PACKS ARE DISCONT

PROVIDE 1/2" PLYWOOD SPACER PLATES AT INTERIOR HEADERS CONSTRUCTED WITH 2x LUMBER. 6. AT CONTRACTOR'S OPTION, PROVIDE GLULAM IN LIEU OF PSL OF EQUAL OR GREATER STRENGTH.

8. ATTACH JAMB & KING STUDS TOGETHER PER CONNECTION TYPE 24 ON NAILING SCHEDULE ON S0.01.



CLUBHOUSE ROOF FRAMING PLAN 3/16" = 1'-0"

WOOD ROOF FRAMING NOTES:

1) REFER TO GENERAL NOTES ON SHEET S0.01

NOTE "2.B" ON SHEET S0.01

2) REFER TO STUD BEARING WALL SCHEDULE TO SHEET S0.02

3) REFER TO HEADER/BEAM SCHEDULE ON SHEET S0.02 4) REFER TO SHEARWALL SCHEDULE ON SHEET S0.03 5) PROVIDE (3) STUD (MINIMUM) ALIGNED UNDER EACH END OF GIRDER TRUSS (CONTINUOUS FOUNDATION) - FINAL

QUANTITY TO MATCH NUMBÉR OF PLIES OF GIRDER TRUSS. PROVIDE SIMPSON LSTA-STYLE HOLDOWN AT EACH END 6) REFER TO \$3.40-SERIES DRAWINGS FOR ADDITIONAL ROOF FRAMING DETAILS NOT INDICATED HERE.

7) PROVIDE UNIFORM UPLIFT SCREWS AT UPPER FLOOR PER DETAILS 2, 2A, 3, 3A, 3B, 4 AND 5 ON SHEET S0.20. 8) PRE-ENGINEERED TRUSSES TO HAVE A MINIMUM DEPTH OF 24". SLOPE TOP CHORD PER ARCHITECTUAL DRAWINGS. 9) XXXX-INDICATES AREA ON ROOF THAT IS REQ'D TO BE DESIGNED FOR MEP EQUIPMENT ZONE PER GENERAL

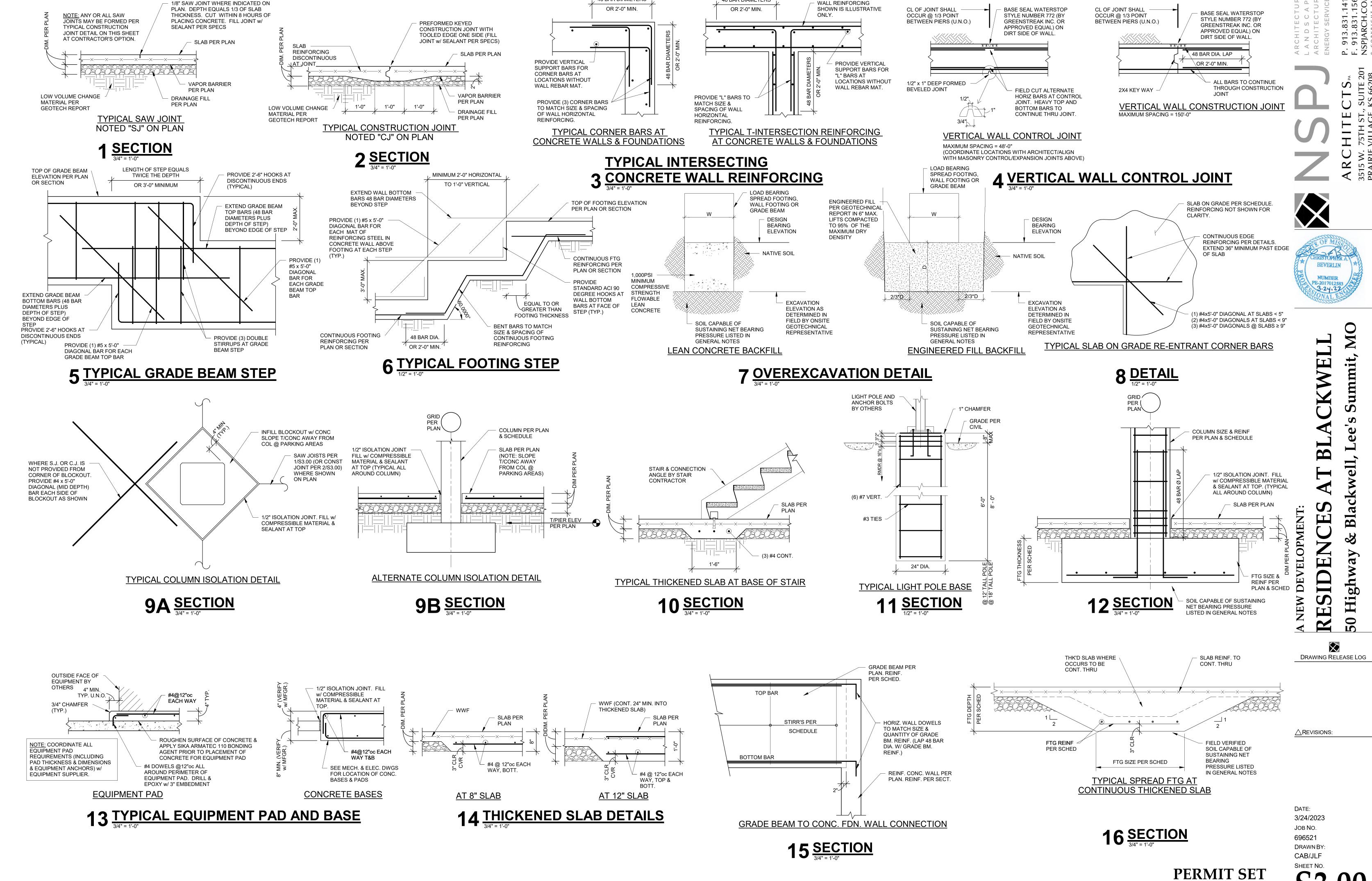
10) TOP OF STEEL ELEVATION FOR STEEL BEARING ON WOOD MEMBERS SHALL BE FIELD COORDINATED BY THE CONTRACTOR & ARCHITECTURAL ROUGH OPENING SCHEDULE. 11) PRE-ENG TRUSSES TO HAVE SLOPING TOP CHORD WITH MINIMUM TRUSS DEPTH OF 24".

PERMIT SET

DATE: 3/24/2023 JOB NO. 696521 DRAWN BY: CAB/JLF

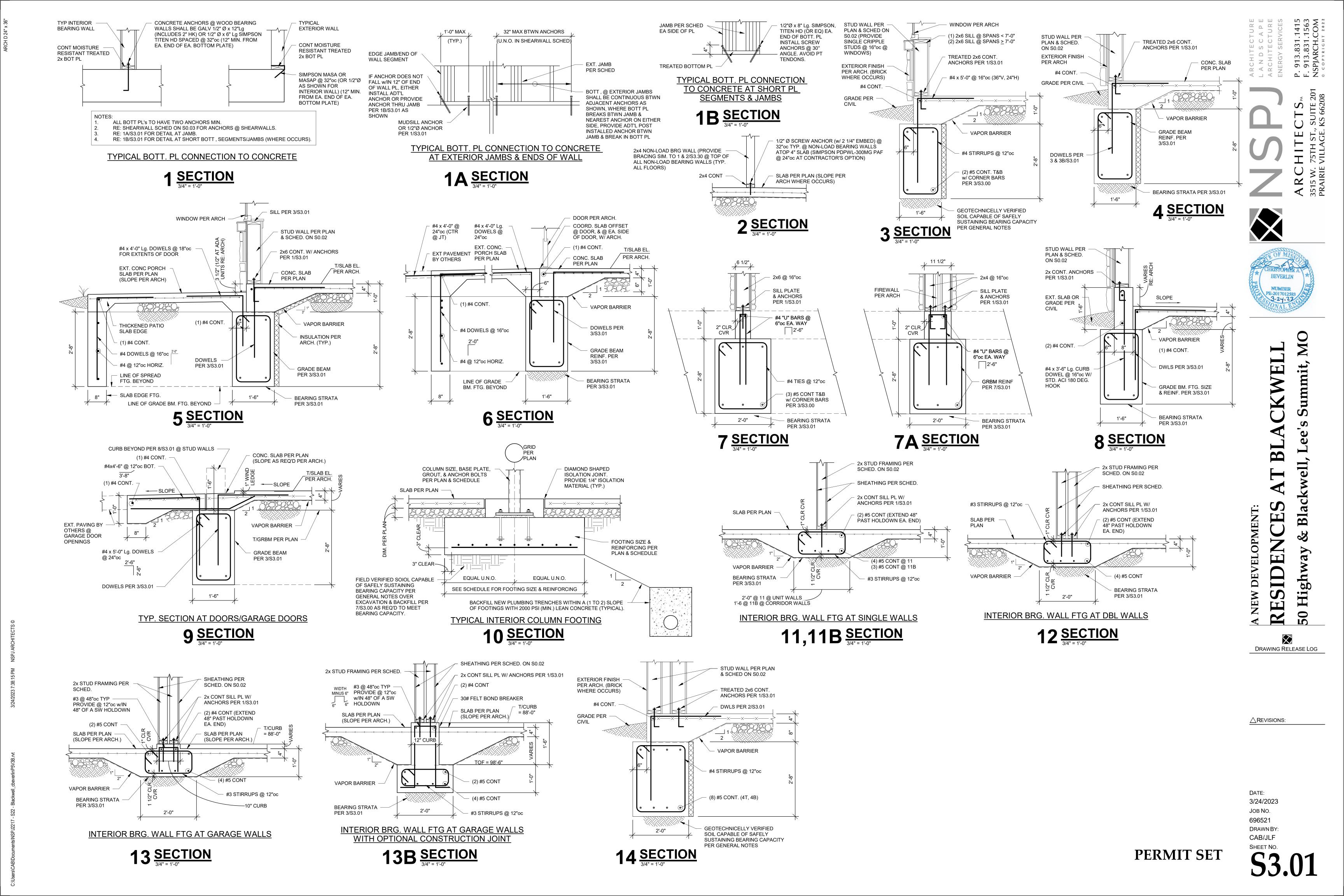
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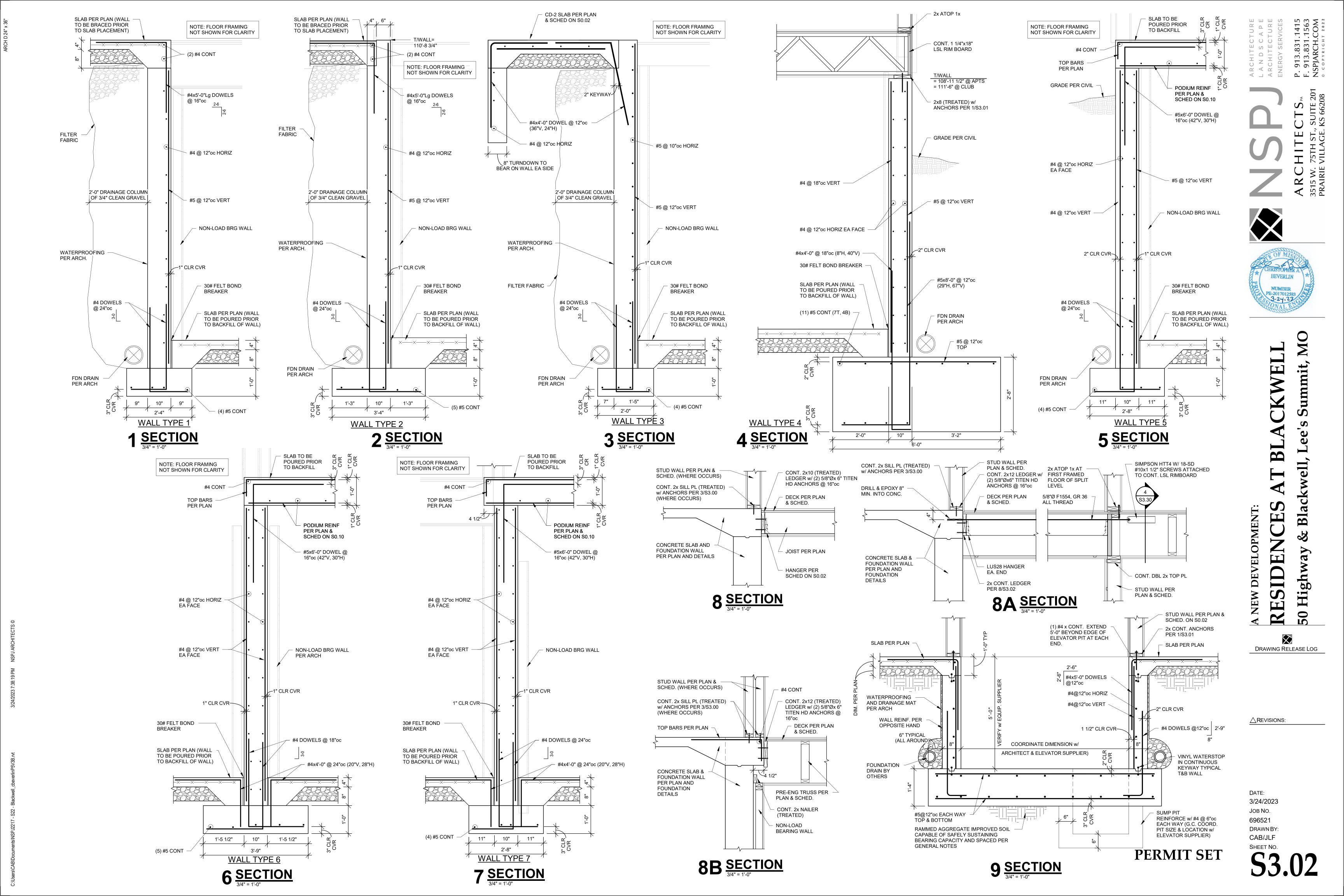
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48 BAR DIAMETERS

48 BAR DIAMETERS





GUARDRAIL PER ARCH

- CD-2 SLAB PER PLAN & SCHED ON S0.02

(36"V, 36"H)

#4 @ 12"oc EA WAYCTR IN WALL

#4x4'-0" @ 12"oc

(3) #5 CONT.

5" 8" 5"

WALL TYPE 8

(8"H, 40"V)

GRADE — PER CIVIL

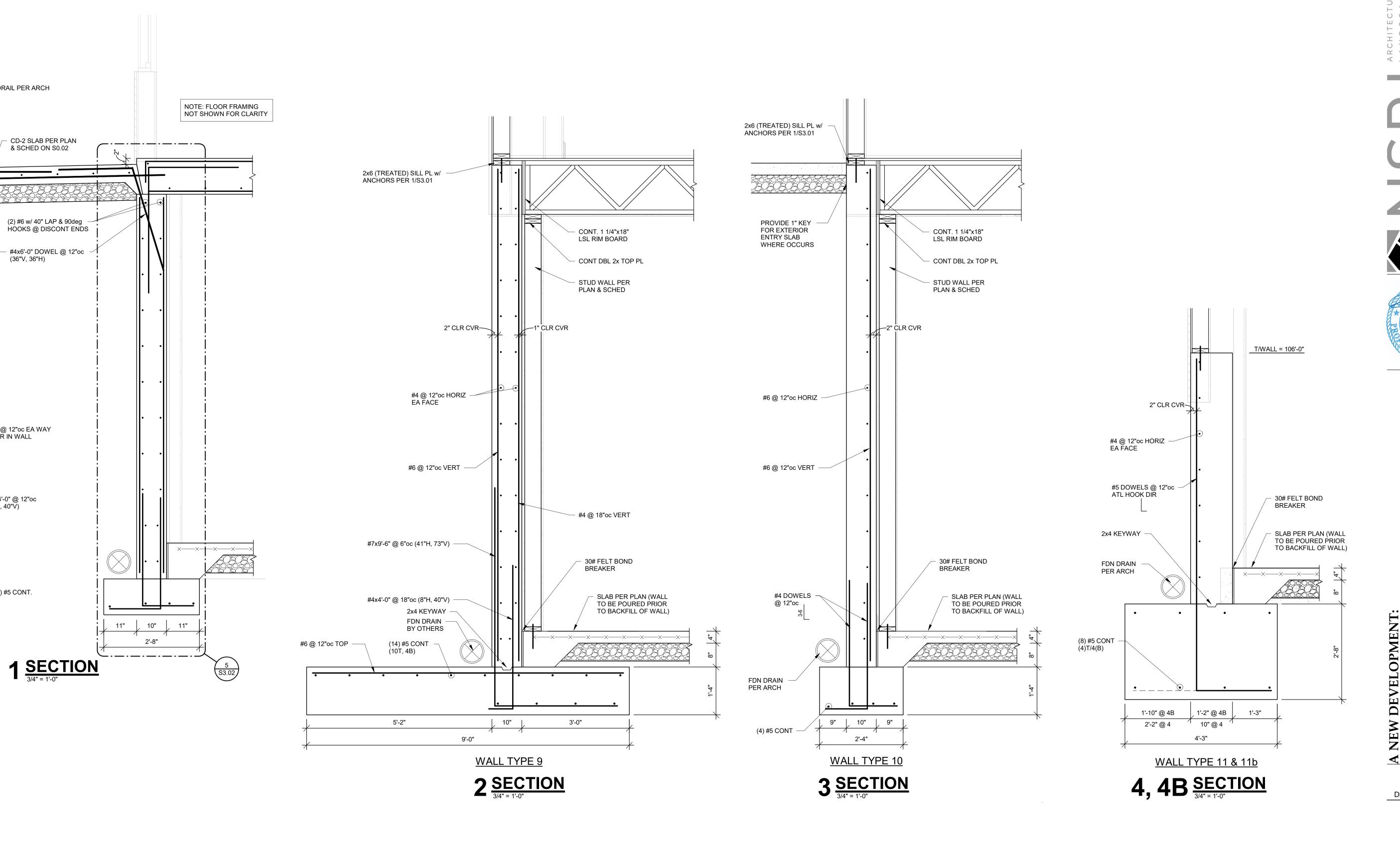
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SHEET NO. **53.03**

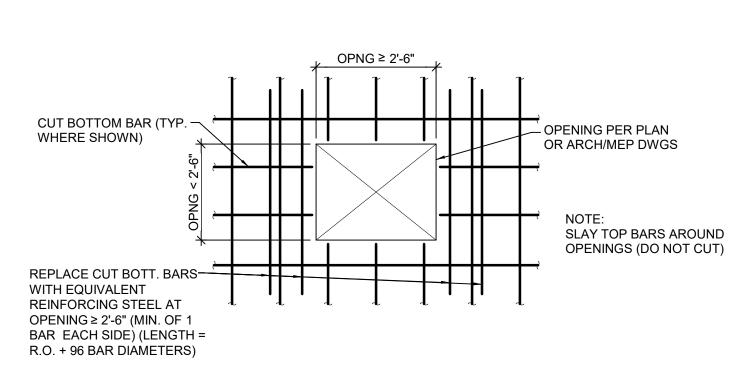


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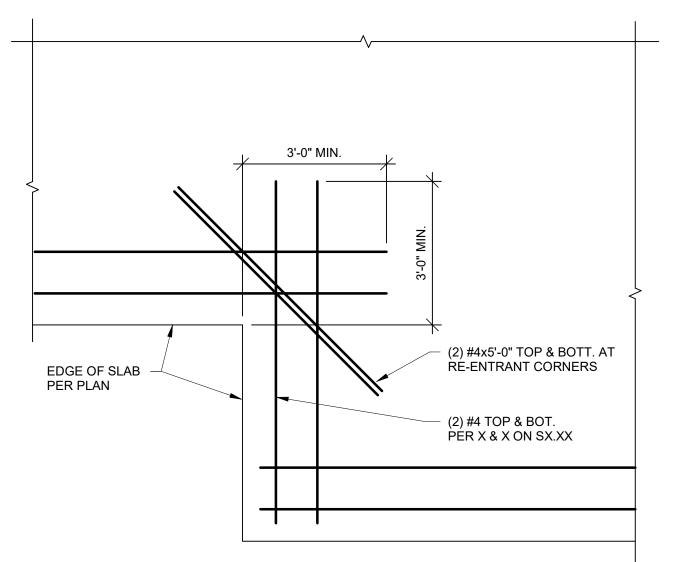
TYPICAL SLEEVE IN PODIUM SLAB RESTRICTIONS

1 **DETAIL** 3/8" = 1'-0"



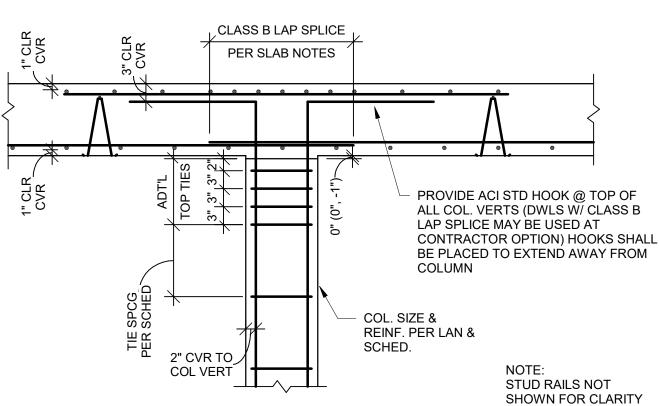
TYPICAL BOTTOM REINFORCING AT INTERIOR OPENING IN PODIUM

5 DETAIL 1/2" = 1'-0"



TYPICAL REINFORCING AT RE-ENTRANT **CORNERS OF ELEVATED SLAB**

8 **SECTION**



REFER TO SHEET S3.19

#5x5'-0" (USE (2) #5 @ WALLS > 8") 2'-6" TYP. (3'-6" @ #6s) (2) #5 MIN. (U.N.O.) (USE / (2) #6 @ WALLS > 8")/ **DWLS TO MATCH** JAMB REINF w/ CLASS B LAP MATCHING SPLICE EA. END **DOWELS**

STRIP LINES

1/4 OF BARS

SPACED EQUALLY

1/2 OF BARS

SPACED AT 4"oc

2 TYPICAL TOP BAR LAYOUT OVER INTERIOR COLUMN
3/8" = 1'-0"

PER PLAN

WHERE BARS ARE SHOWN WITH DIFFERENT

LENGTHS OR EXTENDED TO EDGE OF SLAB,

ALTERNATE BARS AS SHOWN OR SIMILAR

EDGE OF —

SLAB WHERE

OCCURS

REINFORCING STEEL AND ARE NOT

CLOSER THAN 4" ON CENTER

TYPICAL WALL OPENING REINFORCING

1. THIS DETAIL APPLIES TO OPENINGS ≤ 4'-0"oc DO NOT LOCATE VERTICAL WALL CONSTRUCTION JOINTS W/IN 5'-0" OF OPENINGS 3. THIS DETAIL DOES NOT APPLY TO OPENINGS DIRECTLY BELOW POINT LOADS OR

9 ELEVATION

#5x5'-0" (USE (2) 2'-6" TYP. #5 @ WALLS > 8") (3'-6" @ #6s) (Ú.N.O.) (USE (2) #6 @ WALLS > 8") (2) #5x5'-0" DWLS TO MATCH JAMB REINF w/ CLASS B LAP SPLICE EA. END DOWELS

1/4 OF BARS SPACED EQUALLY

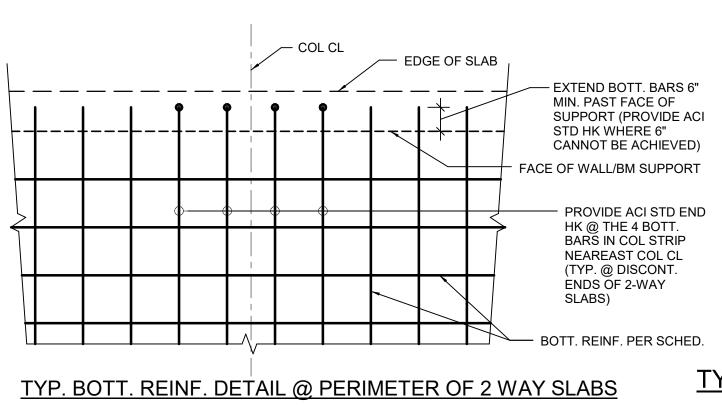
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PER PLAN

TYPICAL WALL OPENING REINFORCING

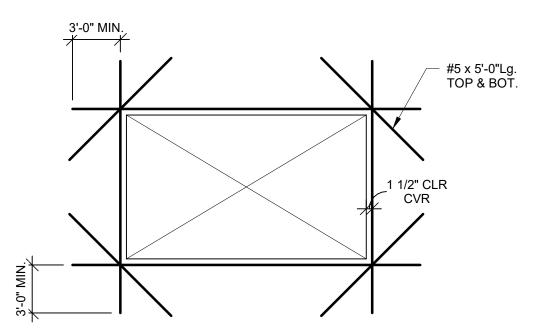
1. THIS DETAIL APPLIES TO OPENINGS ≤ 4'-0"oc 2. DO NOT LOCATE VERTICAL WALL CONSTRUCTION JOINTS W/IN 5'-0" OF OPENINGS 3. THIS DETAIL DOES NOT APPLY TO OPENINGS DIRECTLY BELOW POINT LOADS OR

10 ELEVATION



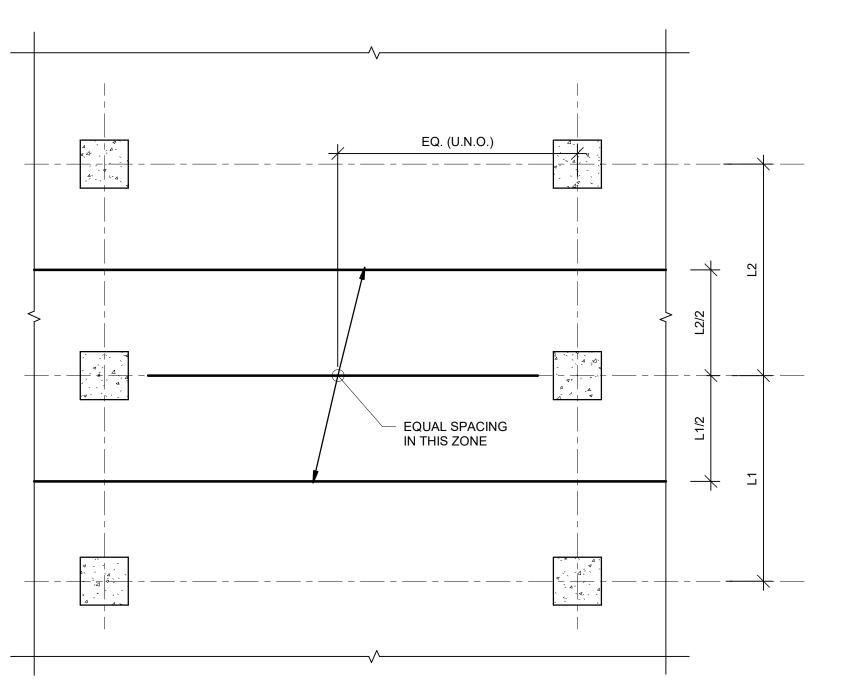
TYPICAL CONDUIT EMBEDMENT IN SLAB

3 **DETAIL**1/2" = 1'-0"



MAX. OPENING DIM.	REINFORCING
12" TO 18"	(1) #5 EA. SIDE
18" TO 2'-6"	(1) #5 EA. SIDE
2'-6" & LARGER	(2) #5 TOP & BOT. EA. SIDE

 ALL OPENINGS LARGER THAN 12" SHALL BE TRIMMED AS SHOWN.
 THIS REINF. IS IN ADDITION TO REINF. SHOWN ON PLANS 3. FOR MULTIPLE OPENINGS SEE7/S3.10



TYPICAL MIDSPAN BOTTOM BAR PLACEMENT (WHERE SHOWN ON PLAN, U.N.O.)

11 **SECTION**

4 DETAIL 3/4" = 1'-0"

1. MAX. CONDUIT DIAMETER SHALL NOT EXCEED 1/3 OF

2. CONDUITS SHALL NOT BE SPACED CLOSER THAN 3

3. CONDUITS ARE NOT PERMITTED WITHIN 24" OF THE

4. CONDUITS SHALL BE PLACED AS CLOSE TO MID-DEPTH

THECROSSING OF CONDUITS SHOULD BE AVOIDED

6. CONCRETE COVERAGE FOR CONDUITS SHALL BE THE

SAME AS THE REQUIREMENTS FOR REINFORCING

NOTE 2 NOTE 2 NOTE 2

CONDUIT EMBEDED IN SLAB (RE: NOTES ON THIS SHEET)

- SLAB THICKNESS T

PER PLAN

(THIS INCLUDES WHERE CONDUITS CROSS; RE: NOTE

WHERE POSSIBLE. RE-ROUTE CONDUITS WHERE

POSSIBLE TO AVOID CROSSING W/IN THE SLAB)

7. ALUMINUM CONDUITS & PIPES SHALL NOT BE

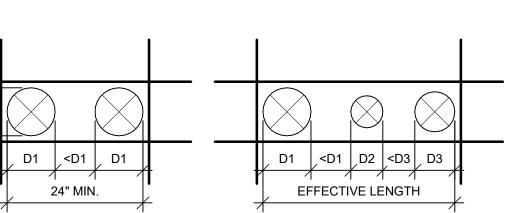
THE OVERALL THICKNESS OF THE SLAB

DIAMETERS ON CENTER

FACE OF A COLUMN

OF SLAB AS POSSIBLE

EMBEDDED IN CONCRETE.



OPENINGS WHICH ARE CLOSER TO ONE ANOTHER THAN THE DIAMETER OF THE LARGER OF THE TWO ARE CONSIDERED TO FORM A

- IF THE COMBINED OPENING IS LESS THAN 12" NO TRIM BARS ARE 24", PROVIDE (1) #5 (TOP & BOT.) W/ 2'-0" EMBEDMENT PAST
- IF COMBINED OPENING IS LARGER THAN 24" SEE 6/S3.10

7 SECTION 1/2" = 1'-0"

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NUMBER

3.24.23

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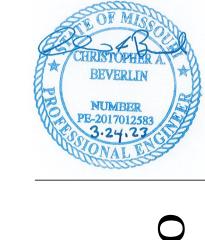
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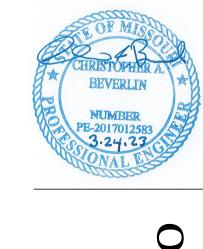
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TYPICAL INTERIOR COLUMN 12 **SECTION**











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DATE:

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3/24/2023 JOB NO. CAB/JLF

WHERE ARCHITECTURE
OR BLDG GEOMETRY
DOES NOT ALLOW BEAM
CONNECTION) WHERE PERPENDICULAR BEAM OCCURS) COLUMN PER PLAN TYPICAL BEAM TO COLUMN CONNECTION 1 **DETAIL** 3/4" = 1'-0"

CONTINUOUS BEAM WHERE OCCURS

1/2" CAP PL w/ (4) 3/4" -BOLTS (RE: 5/S3.20

BEAM PERPLAN

3/8" STIFF. PL EACH SIDE @ COL CL (ELIMINATE STIFFENER IN FAVOR OF SHEAR CONNECTION PER 2/S310

BM PER PLAN 3/8" STIFF PL -EA. SIDE / 3/4"Ø THREADED ROD (OR THREADED STUD) WELDED TO CAP PL (2) 3/4"Ø — THRU-BOLTS — COL. PER PLAN W/ 1/2" CAP PL TYPICAL BEAM TO COLUMN
CONNECTION @ DISCONTINOUS BEAMS

2 **SECTION**3/4" = 1'-0"

NOTE: WOOD FRAMING NOT

SHOWN FOR CLARITY

TRUSS HANGER BY — TRUSS SUPPLIER RE: SCHED ON S0.02 - STL. BEAM PER PLAN

2x NAILER CUT TO MATCH — BEAM FLANGE WIDTH PLUS 3/8", ATTACH TO BEAM W/ (2) SHALL NOT OVERHANG STEEL 1/2"Ø A307 BOLTS @ 24"oc STAGGERED BEAM FLANGE BY MORE THAN 1/4" ON EITHER SIDE REF. ARCH. FOR FLOOR TOPPING WOOD TRUSSES PER PLAN

NOTE: 2x TOP NAILER

2x NAILER & CARRIAGE -BOLTS PER 5/S3.20

3/8" STIFF PL OPP SIDE CENTER

PERPENDICULAR WOOD BEAM

HSS12x8x3/8 CUT AND USE AS STIFFENER CENTERED AT BEAM

WELD TO BEAM W/ 1/4" FILLET ALL

AROUND (12" DIM PARALLEL STEEL

1/4"x16"x6" SIDE PLATE EA SIDE WELD TO BENT

PL WITH 1/4" FILLET

BTWN SIDE PLATES

4 **SECTION**3/4" = 1'-0"

- 3/4"Ø THRU BOLTS

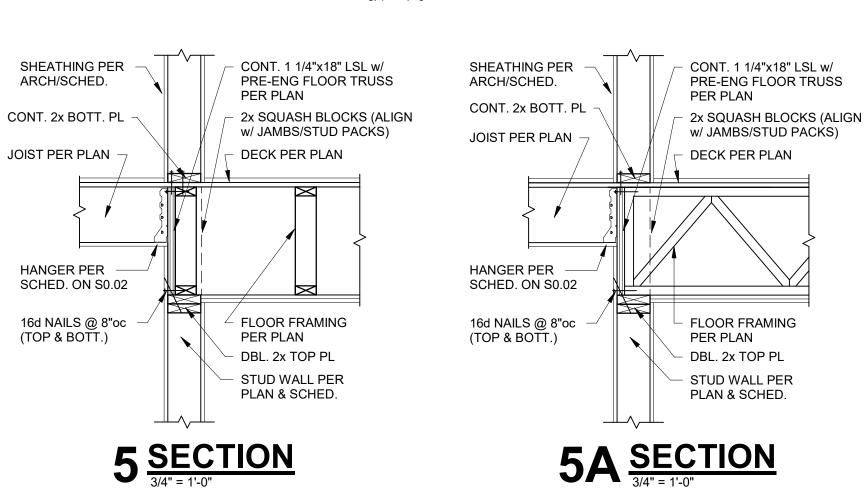
- 3/8" BENT PLATE WELD TO HSS W/ 1/4" FILLET EA SIDE.
- PROVIDE CLEAR SPACE EQUAL TO BEAM WIDTH + 1/4"

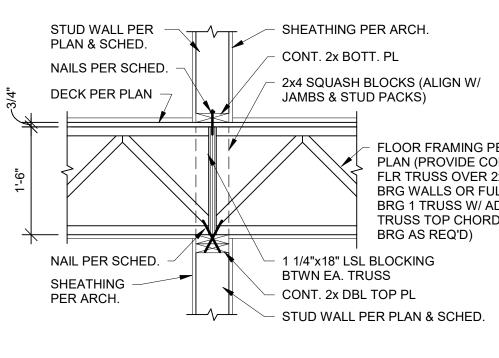
UPSET BEAM PER PLAN

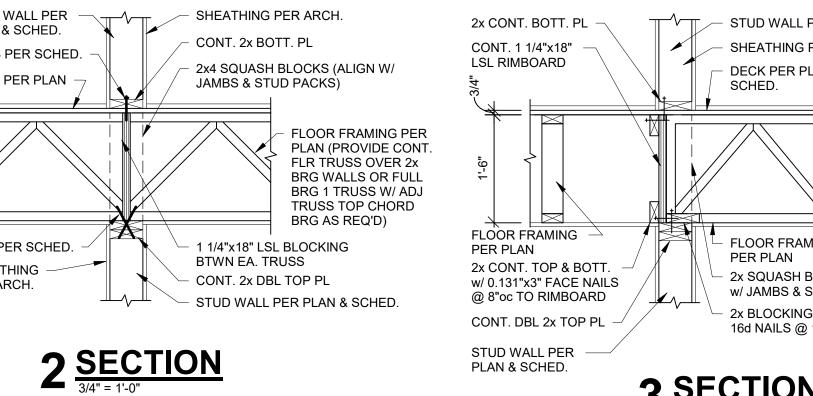
3 **SECTION**3/4" = 1'-0"

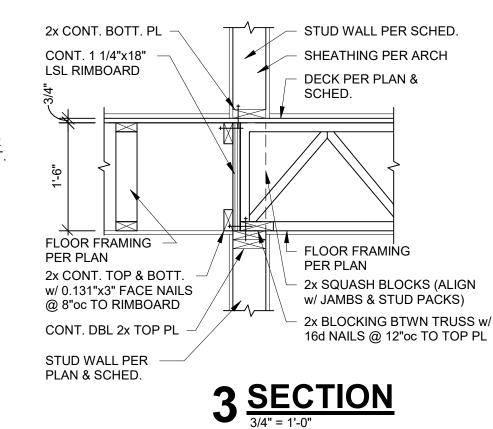
NON-LOAD BEARING WALLS

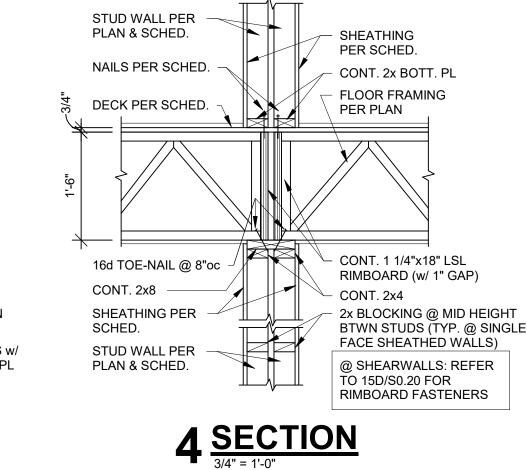
1 **SECTION**3/4" = 1'-0"

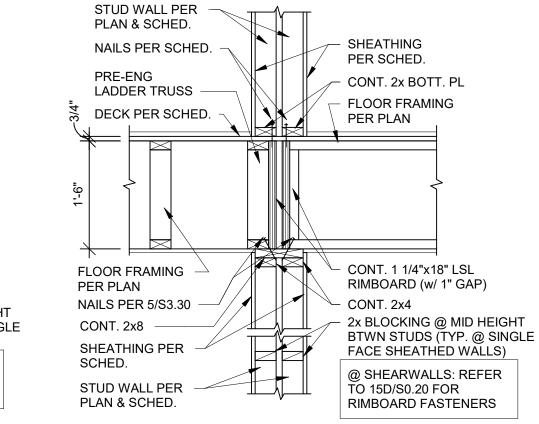




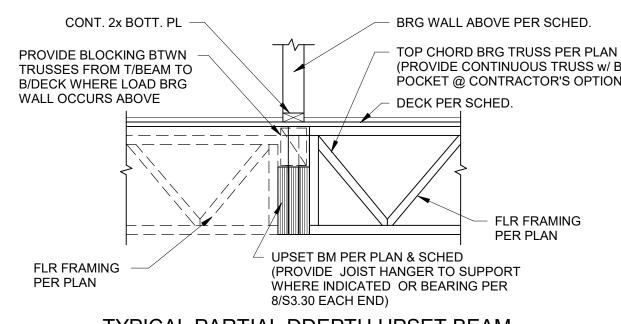




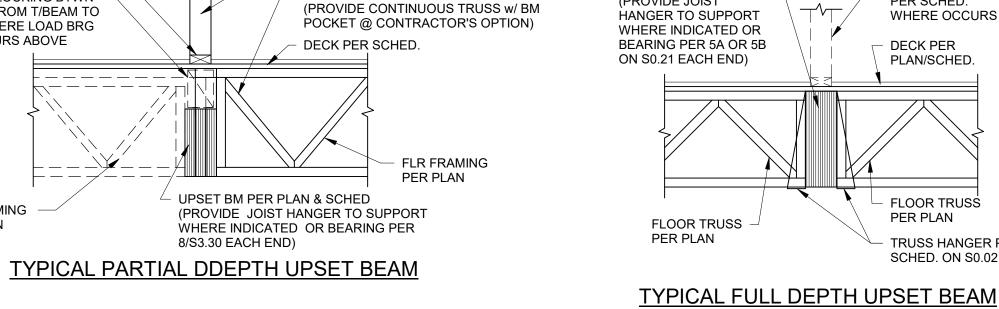


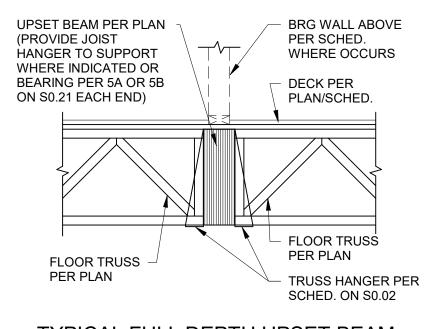


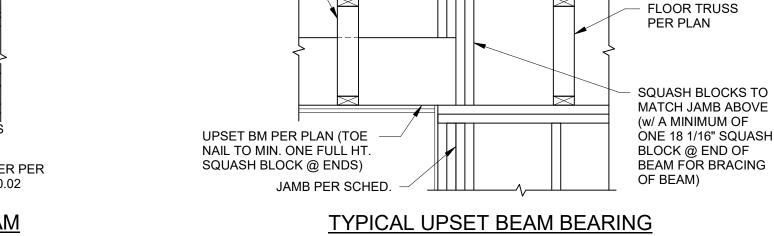
4A SECTION 3/4" = 1'-0"



 $6 \frac{\text{SECTION}}{3/4" = 1'-0"}$







JAMB ABOVE WHERE OCCURS

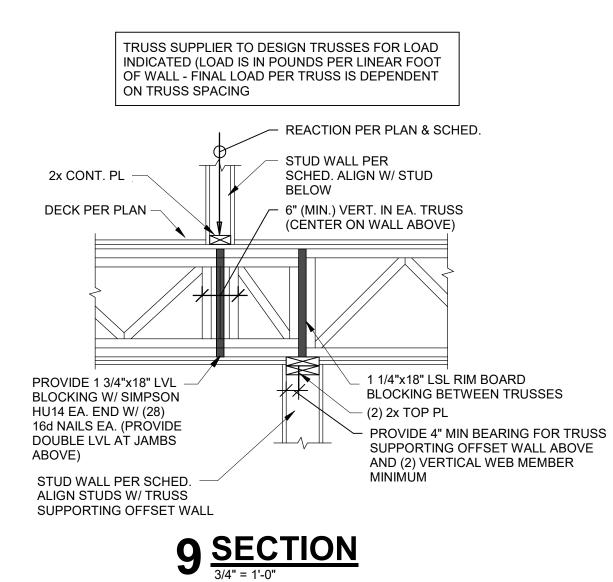
DECK PER SCHED.

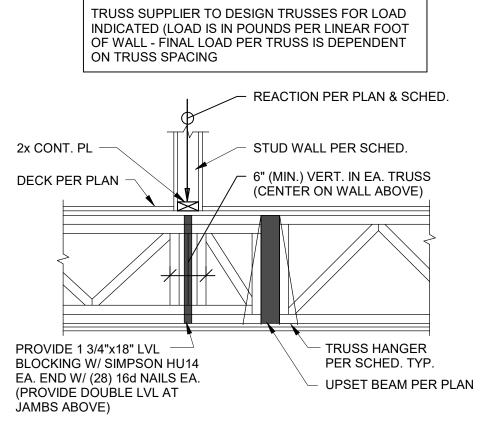
TOP CHORD

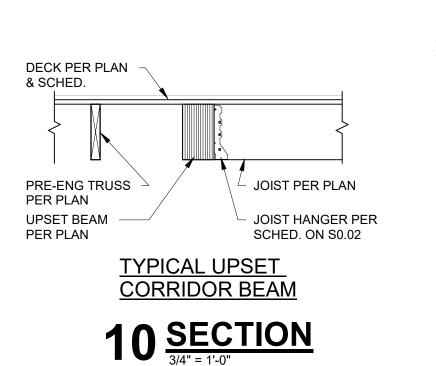
BEARING TRUSS

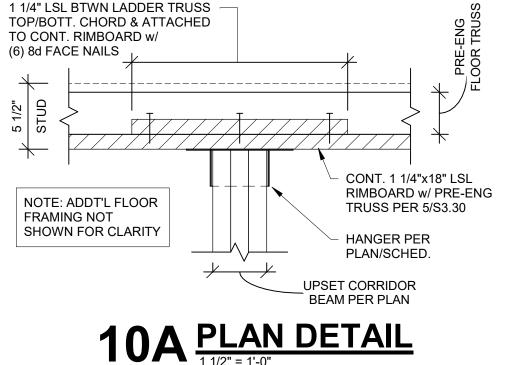
7 SECTION 3/4" = 1'-0"

8 <u>SECTION</u>
3/4" = 1'-0"









9A SECTION 3/4" = 1'-0"

RESIDE

50 \bigotimes DRAWING RELEASE LOG

way

High

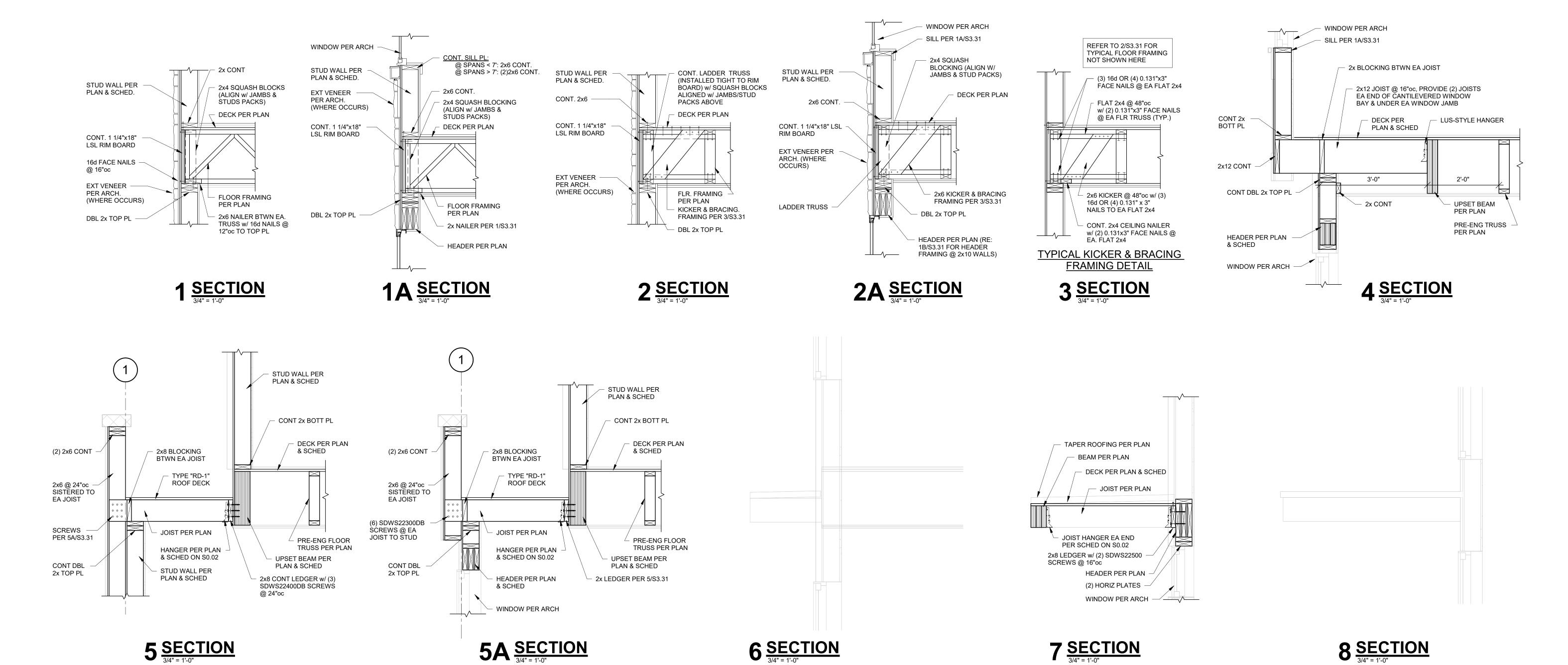
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DATE: 3/24/2023 JOB NO. 696521 DRAWN BY: CAB/JLF



Blackwe] RESIDENCES 50 Highway

BEVERLIN

DRAWING RELEASE LOG

△REVISIONS:

DATE: 3/24/2023 JOB NO. 696521 DRAWN BY: CAB/JLF SHEET NO. **53.31**

PRE-ENG

ROOF TRUSS PER PLAN

> PRE-ENG GIRDER TRUSS PER PLAN

7A <u>SECTION</u>

ROOF DECK PER PLAN

PER SCHED

EDGE FASTENERS

PRE-ENG ROOF — TRUSS PER PLAN

PER TRUSS SUPPLIER

TRUSS HANGER

SIMPSON LGT TYPE HOLDOWN TO JAMB (RE: GENERAL NOTE

11G) (USE LSTA15 STRAP EACH

PARALLEL TO BEARING WALLS)

CONT 2x TOP PI

SIDE @ GIRDER TRUSS

JAMB PER SCHED. -(RE: 4/S0.05 @ BASE

TYPICAL DETAIL AT ROOF

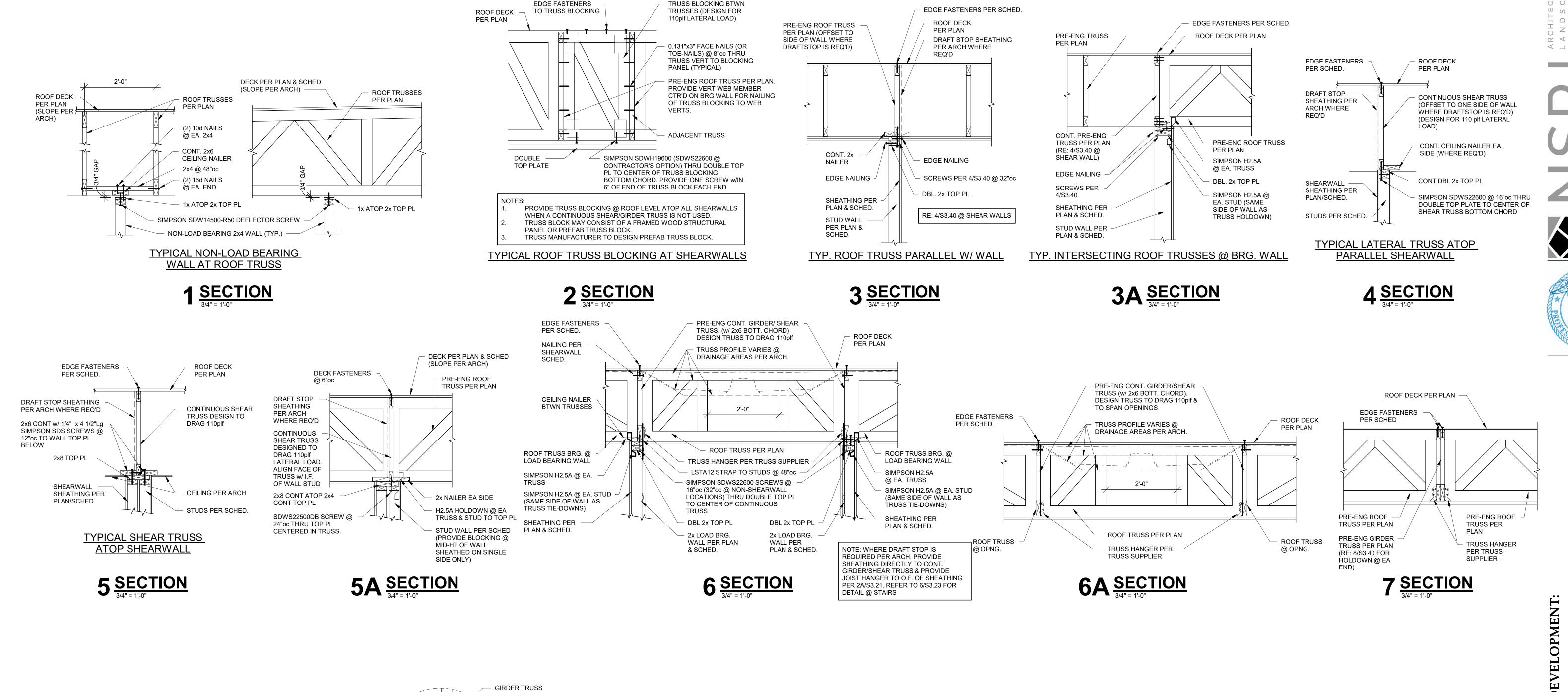
GIRDER TRUSS BEARING

8 **SECTION**

ÒF STUD

PACK/WALL)





CONT 2x TOP PL

PL TRANSITION

SIMPSON LSTA12 STRAP ACROSS EA STUD FOR EXTENTS

OF BLOCKING

STUD WALL PER SCHED

9 **SECTION**3/4" = 1'-0"

2x BLOCKING ALIGNED

w/ LOWER WALL TOP

PL FOR 32" MIN PAST

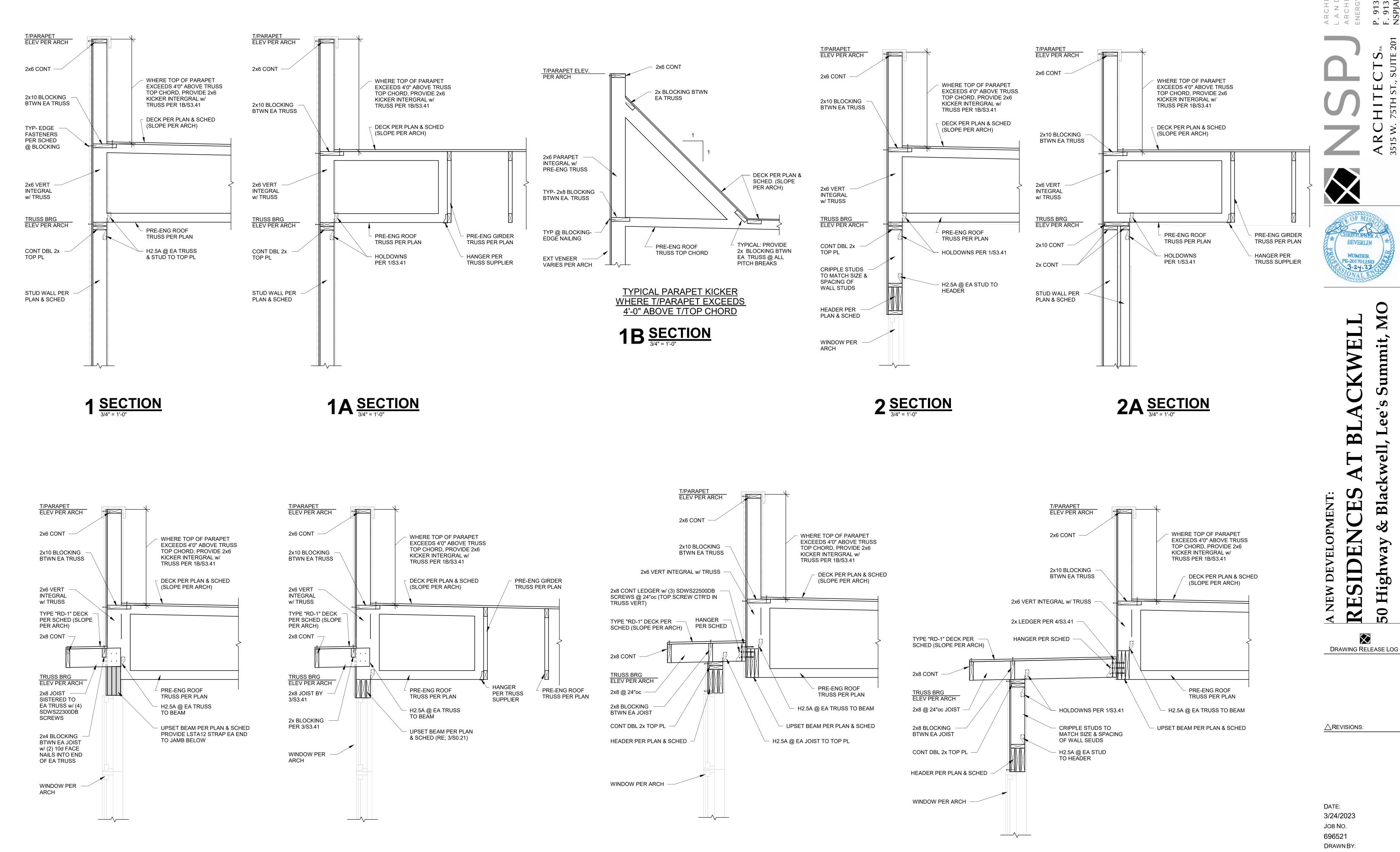
RESIDENCE

So Highway & BI

BEVERLIN

△REVISIONS:

DATE: 3/24/2023 JOB NO. 696521



4 **SECTION**3/4" = 1'-0"

3 <u>SECTION</u>

3A SECTION 3/4" = 1'-0"

CAB/JLF

PERMIT SET

5 **SECTION**3/4" = 1'-0"

SHEET NO.

CAB/JLF
SHEET NO.

S142

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50

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DRAWING RELEASE LOG

△REVISIONS:

DATE:

3/24/2023 JOB NO. 696521 DRAWN BY:

PERMIT SET

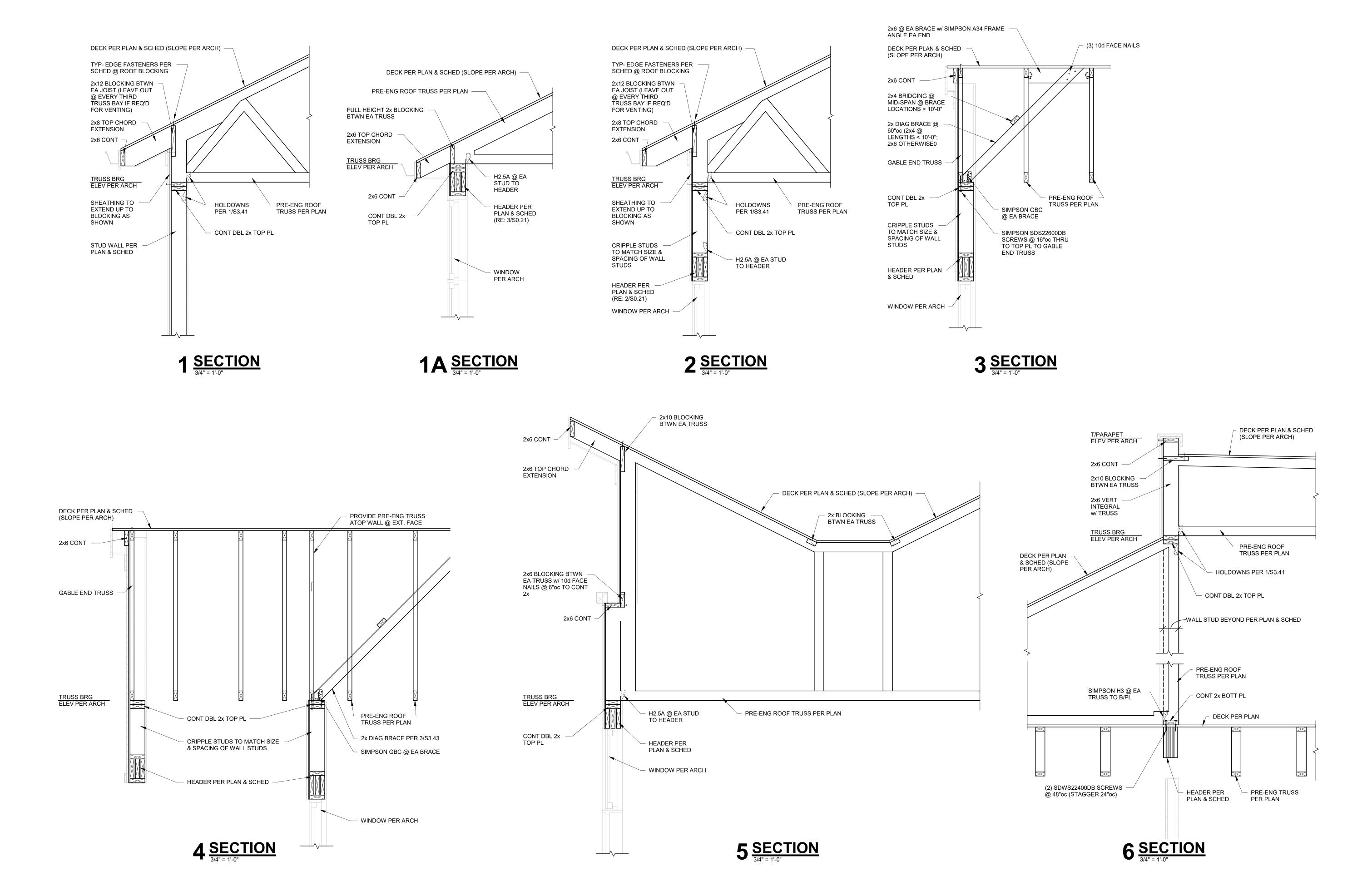
Highway

RESIDE 50 \bigotimes DRAWING RELEASE LOG

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DATE:

3/24/2023 JOB NO. 696521 DRAWN BY: CAB/JLF

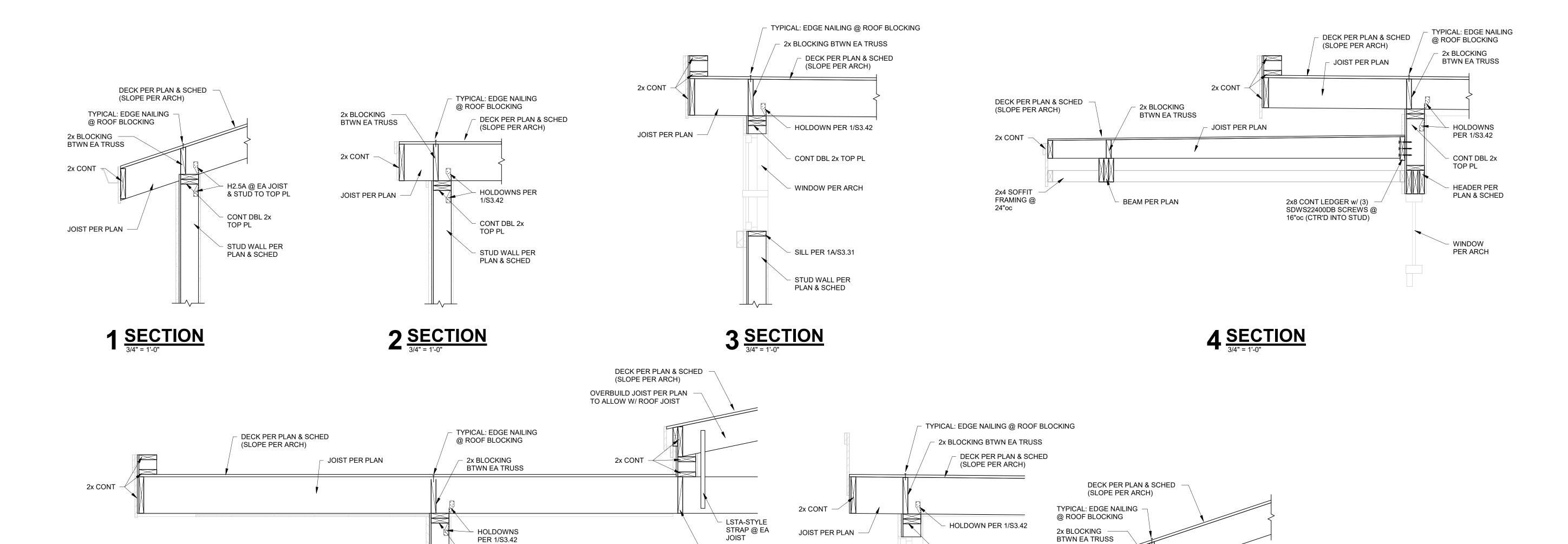


 \bigotimes DRAWING RELEASE LOG

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DATE: 3/24/2023 JOB NO. 696521 DRAWN BY: CAB/JLF

SHEET NO. **53.44**

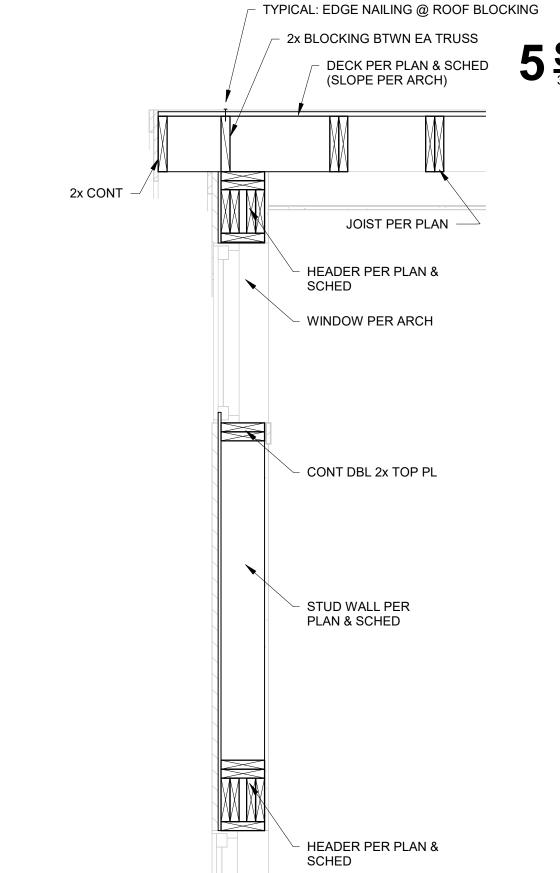


JOIST PER PLAN

JOIST

BTWN EA TRUSS

2x BLOCKING



WINDOW PER ARCH

8 <u>SECTION</u>
3/4" = 1'-0"

- HOLDOWNS

PER 1/S3.42

CONT DBL 2x TOP PL

HEADER PER

PLAN & SCHED

PER ARCH

6 **SECTION**3/4" = 1'-0"

CONT DBL 2x TOP PL

WINDOW PER ARCH

2x CONT

JOIST PER PLAN

HOLDOWN PER

HEADER PER PLAN & SCHED (RE: 3/S0.21)

1/S3.42

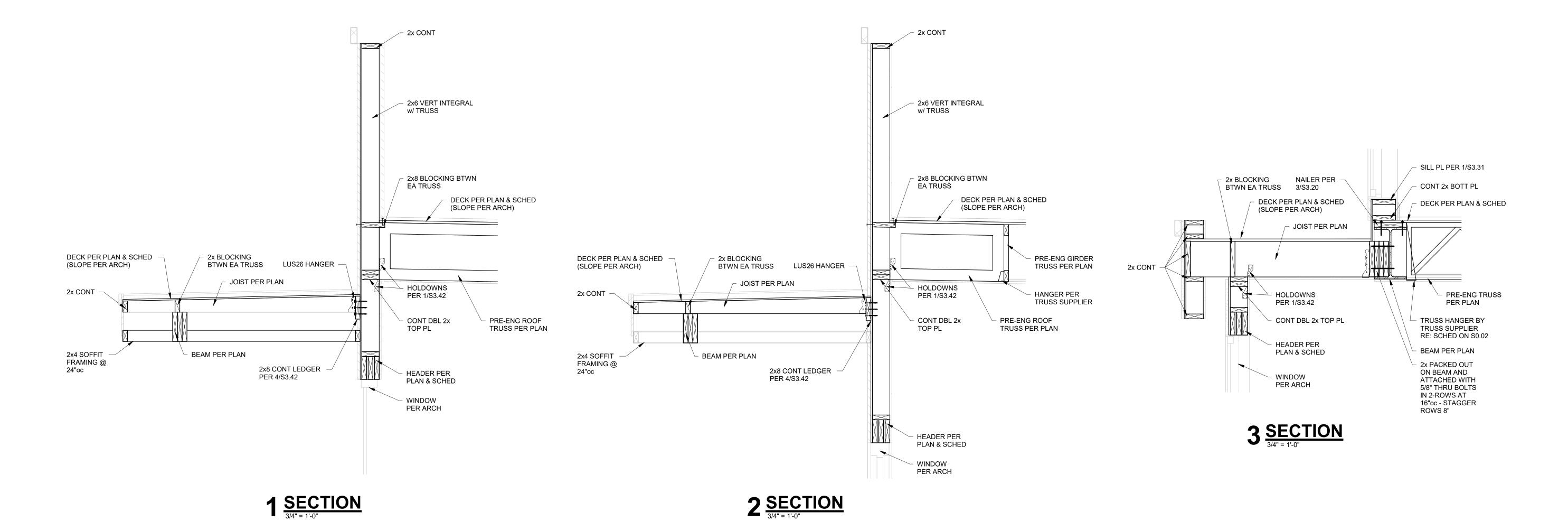
7 **SECTION**3/4" = 1'-0"

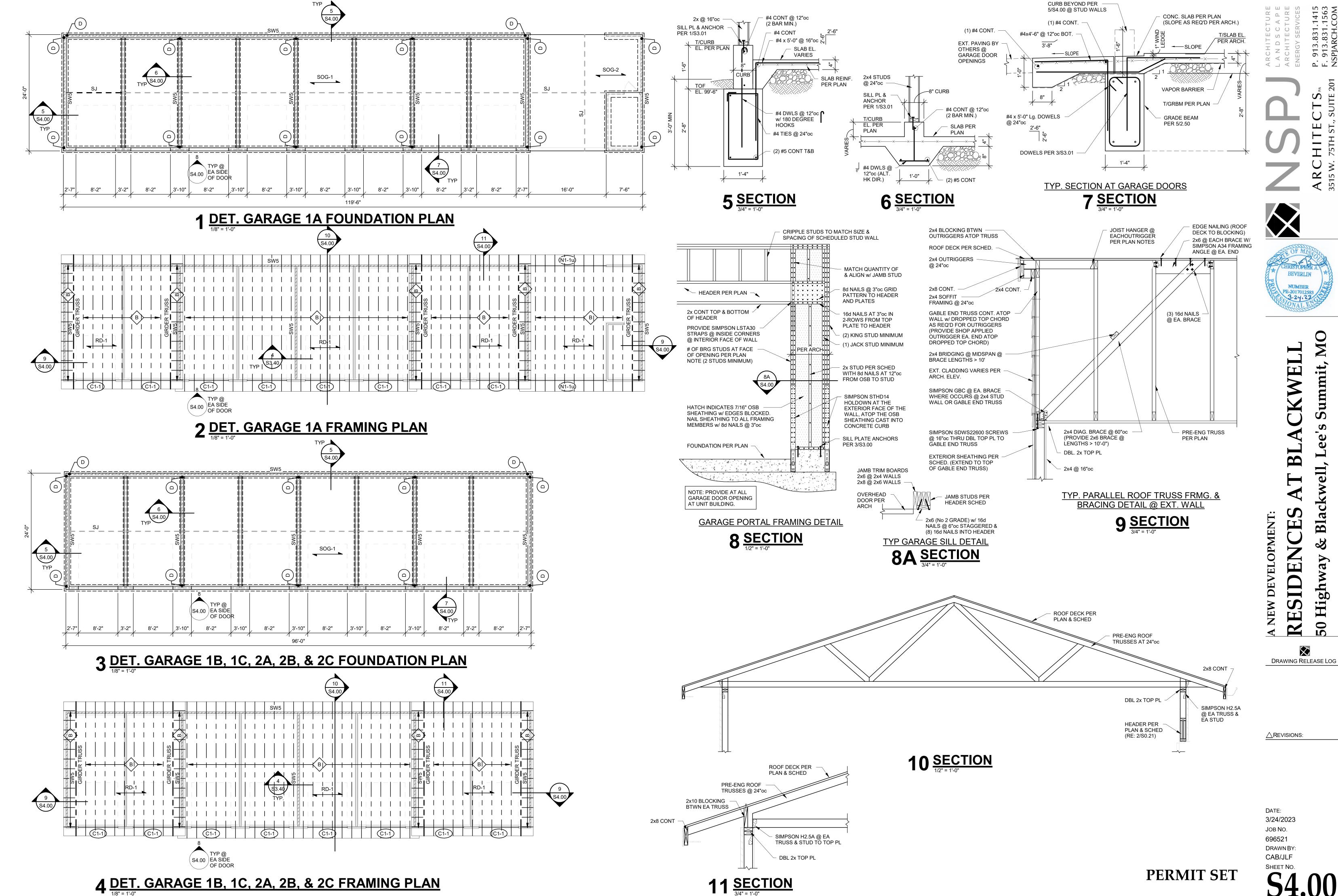
PERMIT SET

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DATE: 3/24/2023 JOB NO. 696521

CAB/JLF





4 DET. GARAGE 3C, 4A, & 4B FRAMING PLAN

RESIDENCES AT BLACKWELL

50 Highway & Blackwell, Lee's Summit, MO

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DRAWING RELEASE LOG

△REVISIONS:

DATE: 3/24/2023 JOB NO. 696521

PERMIT SET

CAB/JLF
SHEET NO.

S4.01

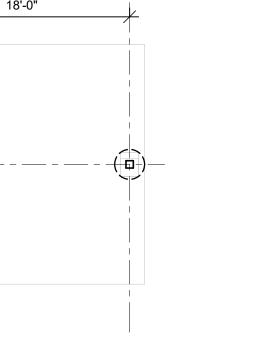
DATE: 3/24/2023 JOB NO.

DRAWN BY: CAB/JLF









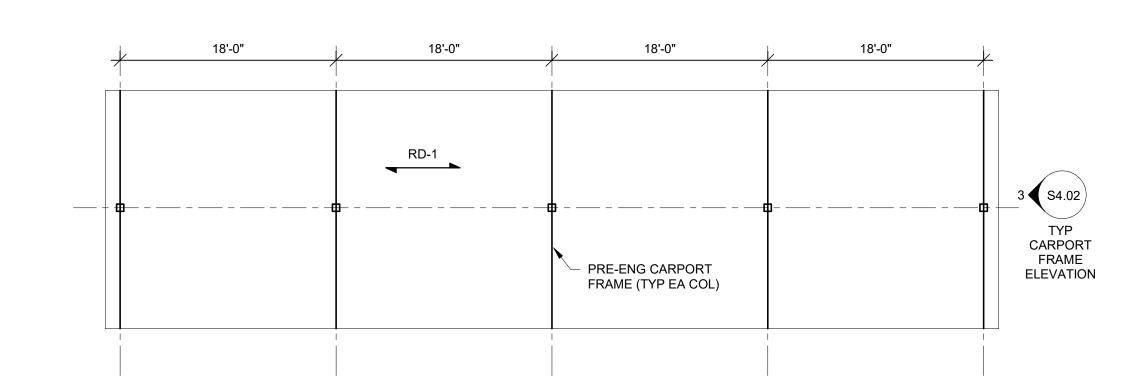
1 CARPORT FOUNDATION PLAN 1/8" = 1'-0"

PRE-ENG CARPORT COL TYP

18'-0"

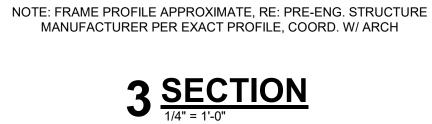
- (| |)

30"Øx7'-0" Min Dp.REINF CONCPIER TYP



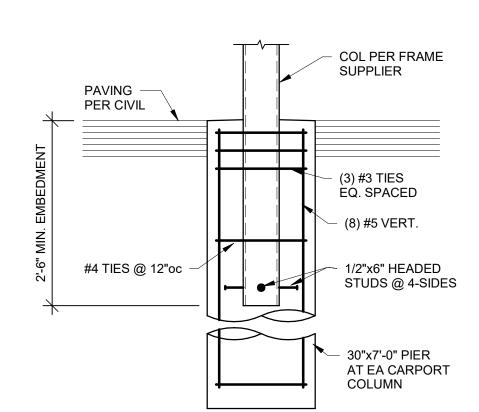
2 CARPORT FRAMING PLAN

1/8" = 1'-0"

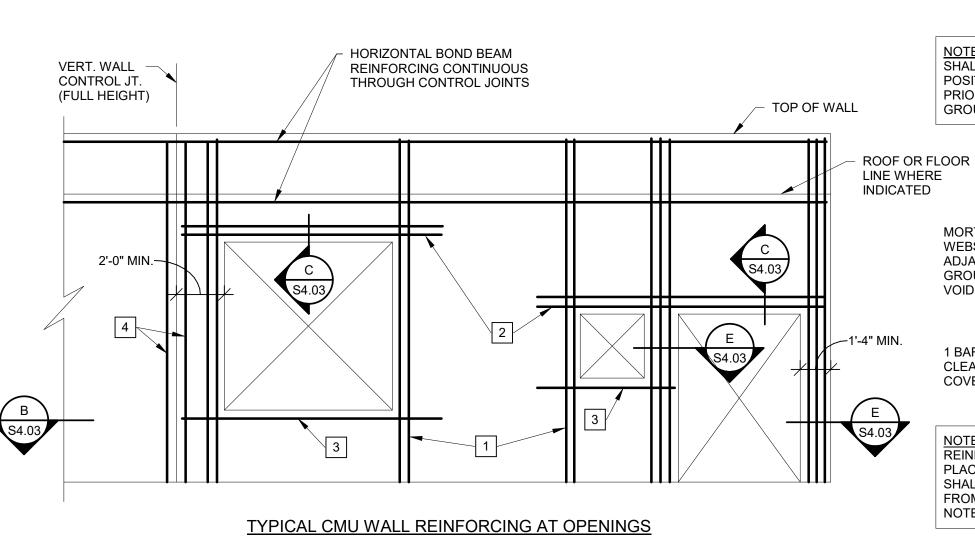


TYPICAL CARPORT FRAME

PRE-ENG METAL CARPORT



4 **SECTION**3/4" = 1'-0"



LEGEND:

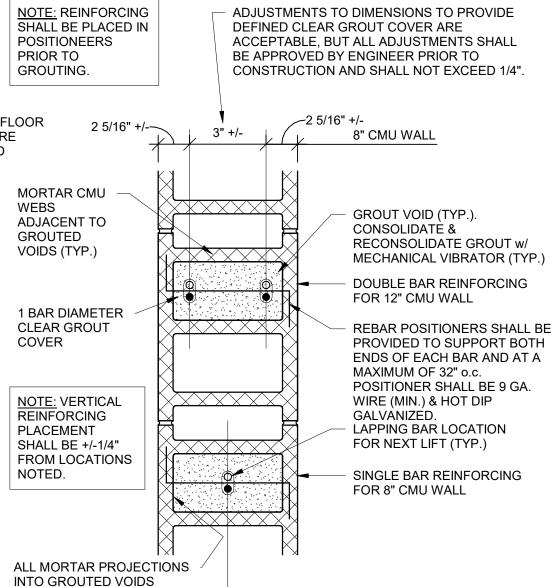
- FULL HEIGHT VERTICAL BARS AS JAMB REINFORCING IN FIRST 2 CELLS ADJACENT TO OPENING. REINFORCE EACH CELL WITH SIZE & QUANTITY OF BAR TO MATCH WALL REINFORCING (1 BAR TYPICAL IN 8" WALLS AND 2 BARS TYPICAL IN 12" WALLS).
- 2 LINTEL REINFORCING PER SECTION C. EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).
- 2-#5 CONTINUOUS HORIZONTAL BARS AS SILL REINFORCING IN 8" COURSE BELOW OPENING (U.N.O.). EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).
- FULL HEIGHT VERTICAL BARS PER MASONRY VERTICAL REINFORCING SCHEDULE LOCATED IN END CELL AT EACH SIDE OF VERTICAL WALL CONTROL JOINTS.

GENERAL CRITERIA: (SECTION A CONTINUED):

- 1. VERTICAL REINFORCING BARS SHALL BE DOWELED TO FOUNDATION WITH A DOWEL OF MATCHING SIZE
- AND SPACING. 2. CONTRACTOR SHALL COORDINATE AND VERIFY OPENINGS IN MASONRY WALLS. OPENINGS SHALL BE
- DETAILED ON REINFORCING STEEL SHOP DRAWING ELEVATIONS. 3. VERTICAL CONTROL JOINTS IN MASONRY WALLS SHALL BE 3/8" WIDE, FULL HEIGHT OF WALL. JOINTS SHALL BE SPACED AT A MAXIMUM OF 24'-0" ON CENTER AND NOT LESS THAN 2'-0" FROM THE EDGE OF ANY OPENING. ALL HORIZONTAL JOINT REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS. ALL BOND BEAM HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL JOINTS. CONTRACTOR SHALL COORDINATE AND VERIFY ALL CONTROL JOINT LOCATIONS.

MASONRY VERTICAL REINFORCING SCHEDULE FOR LOAD BEARING MASONRY (CMU) WALLS								
WALL THICKNESS	LOCATION	VERTICAL REINF. (IN GROUTED CELLS)	SPACING					
8"	TRASH ENCLOSURE	1- #5	24"oc					
NOTES: 1. IN ADDITION TO SPACING SHOWN IN SCHEDULE, VERTICAL REINFORCING SHALL BE PROVIDED IN GROUTED CELLS AT THE FOLLOWING LOCATIONS A.) IN THE FIRST 2 CELLS ADJACENT TO EACH OPENING B.) IN THE END CELLS ON EACH SIDE OF VERTICAL CONTROL JOINTS C.) IN THE END CELLS OF EACH LENGTH OF WALL D.) AT EACH CORNER OF WALLS								
 ALL MASONRY VOIDS AND BOND BEAMS TO BE GROUTED SHALL BE FREE OF DEBRIS AND MORTAR DROPPINGS PRIOR TO GROUTING. ANY MASONRY w/ DROPPINGS OR DEBRIS OBSERVED IN VOIDS SHALL BE REJECTED. 								

A CMU WALL ELEVATION
1 1/2" = 1'-0"



— CMU WALL CENTERLINE

NOTE: ALL MASONRY VOIDS AND BOND BEAMS TO BE GROUTED SHALL BE FREE OF DEBRIS AND MORTAR DROPPINGS PRIOR TO GROUTING. ANY MASONRY w/ DROPPINGS OR DEBRIS OBSERVED IN VOIDS SHALL BE REJECTED.

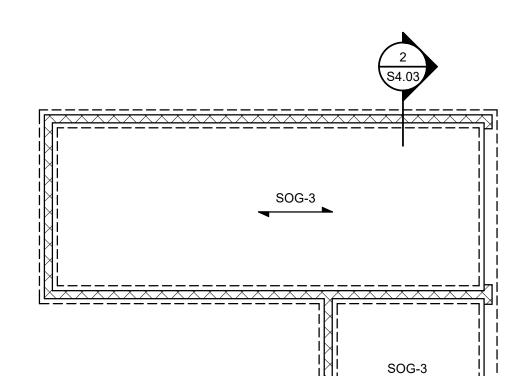
SHALL BE LESS THAN 1/2"

BEYOND INSIDE FACE OF

MASONRY.

TYPICAL REBAR POSITIONING DETAIL

B SECTION

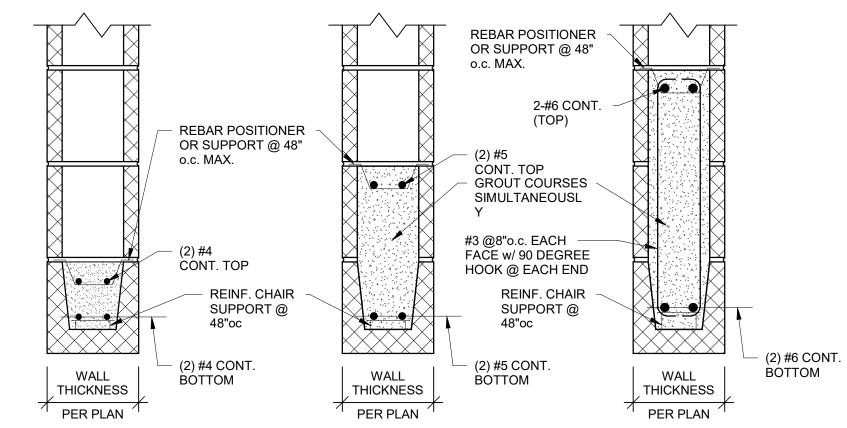


1 TRASH ENCLOSURE FOUNDATION PLAN

1/8" = 1'-0"

TYPICAL MASONRY REINFORCING NOTE:

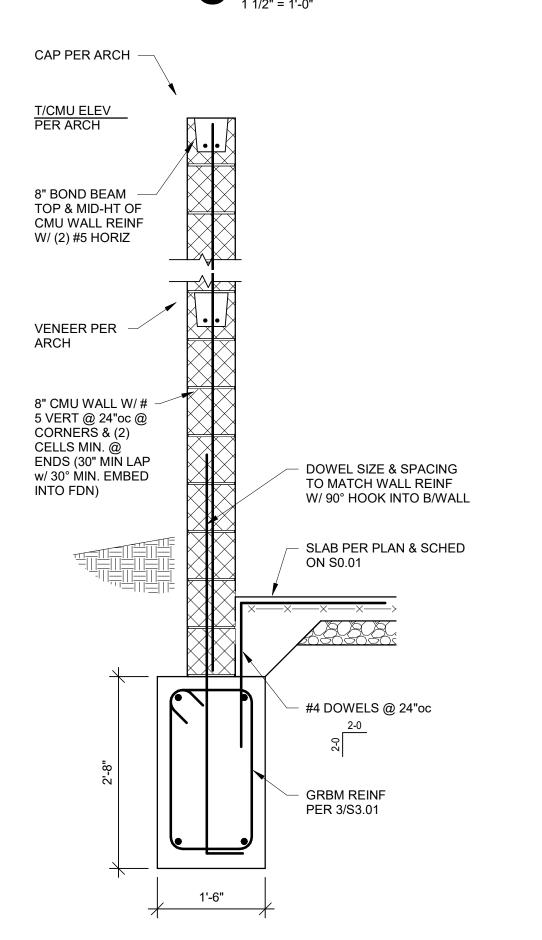
ALL INTERIOR & EXTERIOR MASONRY WALLS SHOWN ON ARCHITECTURAL AND STRUCTURAL DRAWINGS ARE TO BE REINFORCED HORIZONTALLY WITH BOND BEAMS (2 - #5 BOTTOM) AT BOTTOM COURSE, TOP COURSE, JOIST BEARING ELEVATION AND AT 8'-0" MAXIMUM O.C. AND VERTICALLY AS INDICATED ON DRAWINGS. THESE WALLS ARE TO BE ANCHORED TOP AND BOTTOM TO THE FOUNDATION, FLOOR, OR ROOF PER TYPICAL DETAILS. THE VERTICAL REINFORCING IS CONTINUOUS (IN 6'-6" MAXIMUM LENGTHS, LAPPED 2'-6" MINIMUM). FILL BLOCK CELLS AND BOND BEAMS WITH 2,500psi GROUT. RE: DETAILS "A" THROUGH "E" ON THIS SHEET.



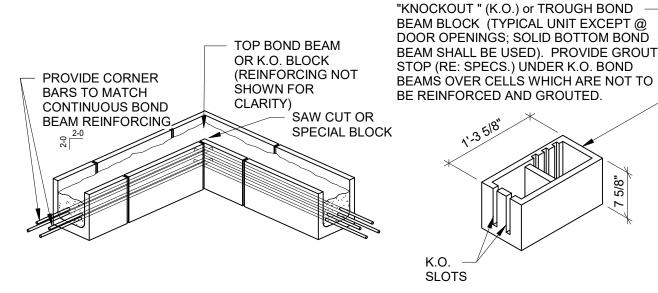
OPENINGS UP TO 4'-0" OPENINGS 7'-4" TO 12'-0" **OPENINGS 4'-0" TO 7'-4"**

TYPICAL LINTELS AT ALL CMU WALLS (U.N.O.)

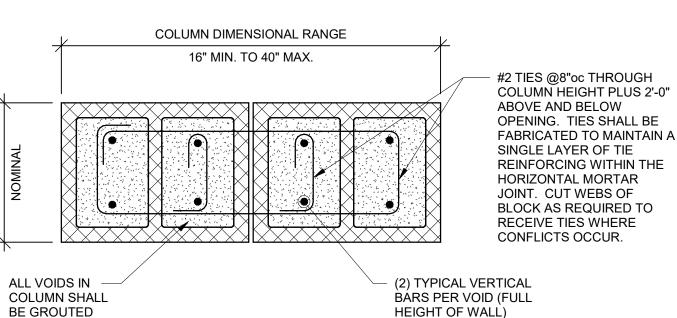
C SECTION



2 **SECTION**



TYPICAL BOND BEAM DETAIL AT CORNER OF CMU WALL



SOLID

HEIGHT OF WALL) **TYPICAL MASONRY COLUMN**

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