

RE: P220321-P220321-02 Roof - Osage Lot 50 MiTek USA, Inc. 16023 Swingley Ridge Rd Chesterfield, MO 63017 314-434-1200

Site Information: Customer: Clover & Hive Project Name: P220321-P220321-02 Lot/Block: 50 Model: Address: 2109 / 2101 South West Osage Stivelivision: Osage City: Lee's Summit State: MO

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014 Wind Code: ASCE 7-16 Roof Load: 45.0 psf Design Program: MiTek 20/20 8.5 Wind Speed: 115 mph Floor Load: N/A psf

This package includes 48 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	153048638	A1	7/13/2022	21	153048658	D3	7/13/2022
2	153048639	A2	7/13/2022	22	153048659	E1	7/13/2022
3	153048640	A3	7/13/2022	23	153048660	E2	7/13/2022
4	153048641	A4	7/13/2022	24	153048661	E3	7/13/2022
5	153048642	A5	7/13/2022	25	153048662	E4	7/13/2022
6	153048643	A6	7/13/2022	26	153048663	E5	7/13/2022
7	153048644	A7	7/13/2022	27	153048664	J1	7/13/2022
8	153048645	A8	7/13/2022	28	153048665	J2	7/13/2022
9	153048646	A9	7/13/2022	29	153048666	J3	7/13/2022
10	153048647	A10	7/13/2022	30	153048667	J4	7/13/2022
11	153048648	A11	7/13/2022	31	153048668	J5	7/13/2022
12	153048649	B1	7/13/2022	32	153048669	J6	7/13/2022
13	153048650	B2	7/13/2022	33	153048670	J7	7/13/2022
14	153048651	B3	7/13/2022	34	153048671	J8	7/13/2022
15	153048652	C1	7/13/2022	35	153048672	J9	7/13/2022
16	153048653	C2	7/13/2022	36	153048673	J10	7/13/2022
17	153048654	C3	7/13/2022	37	153048674	J11	7/13/2022
18	153048655	CJ1	7/13/2022	38	153048675	PG1	7/13/2022
19	153048656	D1	7/13/2022	39	153048676	PG2	7/13/2022
20	153048657	D2	7/13/2022	40	153048677	PG3	7/13/2022

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc under my direct supervision based on the parameters provided by . Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of Missouri is December 31, 2023. Missouri COA: 001193

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek. Any project specific information included is for MiTek customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





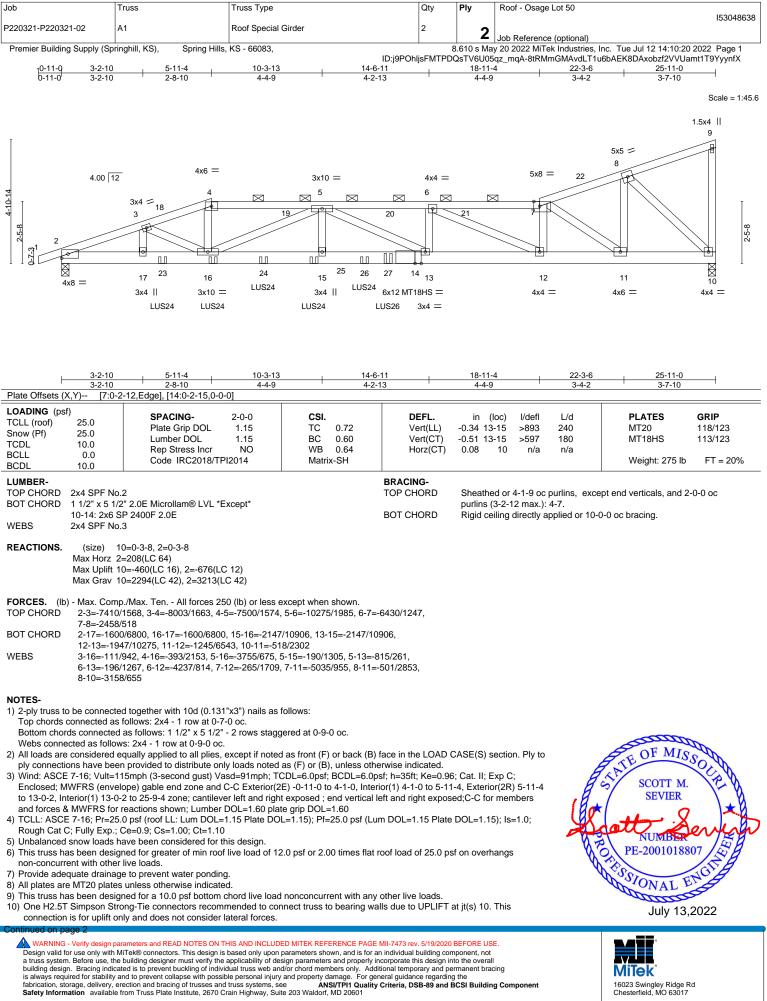
RE: P220321-P220321-02 - Roof - Osage Lot 50

MiTek USA, Inc. 16023 Swingley Ridge Rd Chesterfield, MO 63017 314-434-1200

Site Information:

Project Customer: Clover & HiveProject Name:P220321-P220321-02Lot/Block: 50Subdivision:OsageAddress: 2109 / 2101 South West Osage DriveState:MO

No.	Seal#	Truss Name	Date
41	153048678	V1	7/13/2022
42	153048679	V2	7/13/2022
43	153048680	V3	7/13/2022
44	153048681	V4	7/13/2022
45	153048682	V5	7/13/2022
46	153048683	V6	7/13/2022
47	153048684	V7	7/13/2022
48	153048685	V8	7/13/2022



¹⁶⁰²³ Swingley Ridge Rd Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 50
					153048638
P220321-P220321-02	A1	Roof Special Girder	2	2	
				2	Job Reference (optional)
Premier Building Supply (Spi	ringhill, KS), Spring Hills,	KS - 66083,	8	.610 s May	/ 20 2022 MiTek Industries, Inc. Tue Jul 12 14:10:21 2022 Page 2
		ID:j9PC	hlisFMTP	DQsTV6U0	5qz_mqA-c3?k_cNogwTKe1hokyrNmNTzLN_HEykk?Xn0i_yynfW

NOTES-

- 11) Two H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 4-0-0 from the left end to 12-0-0 to connect truss(es) to front face of bottom chord.
- 15) Use Simpson Strong-Tie LUS26 (4-SD9112 Girder, 4-SD9212 Truss, Single Ply Girder) or equivalent at 12-11-4 from the left end to connect truss(es) to front face of bottom chord.
- 16) Fill all nail holes where hanger is in contact with lumber.

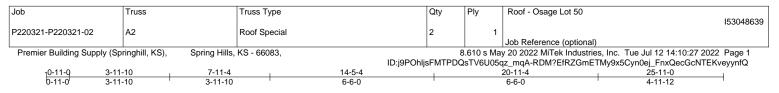
LOAD CASE(S) Standard

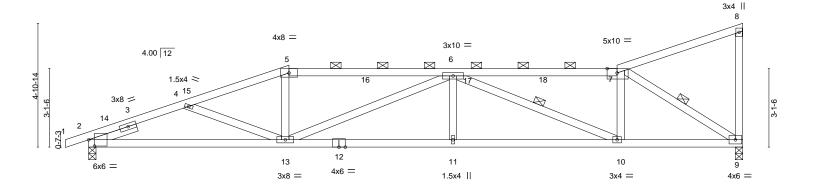
- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-4=-70, 4-7=-70, 7-9=-70, 2-10=-20

Concentrated Loads (lb)

Vert: 16=-356(F) 23=-558(F) 24=-356(F) 25=-356(F) 26=-356(F) 27=-1052(F)





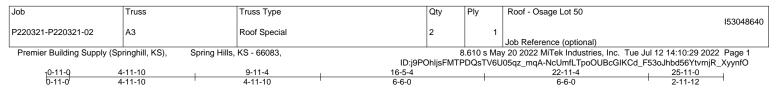


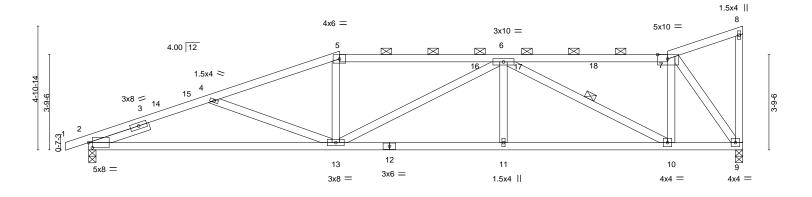
	7-11-4	14-5-4		20-11-4		25-11-0	
	7-11-4	6-6-0		6-6-0	ł	4-11-12	
Plate Offsets (X,Y) [2:0-2	-12,0-3-0], [7:0-4-12,Edge]						
LOADING (psf) TCLL (roof) 25.0	SPACING- 2-0-0	CSI.	DEFL.	in (loc) l/defl	L/d	PLATES	GRIP
Snow (Pf) 25.0	Plate Grip DOL 1.15	TC 0.82	Vert(LL)	-0.25 11-13 >999	240	MT20	197/144
TCDL 10.0	Lumber DOL 1.15	BC 1.00	Vert(CT)	-0.40 11-13 >780	180		
BCLL 0.0	Rep Stress Incr NO	WB 0.96	Horz(CT)	0.12 9 n/a	n/a		
BCDL 10.0	Code IRC2018/TPI2014	Matrix-SH				Weight: 108 lb	FT = 20%
	0 * 5		BRACING-	Oh			0 0 0
TOP CHORD 2x4 SPF No.			TOP CHORD	Sheathed or 2-11-8 or		ept end verticals, and	2-0-0 oc
5-7: 2x4 SP BOT CHORD 2x4 SPF No.			BOT CHORD	purlins (3-4-7 max.): 5 Rigid ceiling directly a			
WEBS 2x4 SPF No.			WEBS	1 Row at midpt	6-10. 7-	0	
	S No.2 1-11-11		WEDS	T ROW at midpt	6-10, 7-	9	
	10.2 1-11-11						
Max Grav 9 FORCES. (Ib) - Max. Comp TOP CHORD 2-4=-2654/ 3OT CHORD 2-13=-796/	=-243(LC 16), 2=-268(LC 12) =1255(LC 41), 2=1350(LC 42) o./Max. Ten All forces 250 (lb) or less 651, 4-5=-2729/580, 5-6=-2583/574, 6- 2388, 11-13=-667/3409, 10-11=-667/34 227, 5-13=0/414, 6-13=-901/120, 6-11=	7=-1912/379, 8-9=-253/1 09, 9-10=-388/1946					
 Enclosed; MWFRS (envel to 15-0-2, Interior(1) 15-0- and forces & MWFRS for TCLL: ASCE 7-16; Pr=25. Rough Cat C; Fully Exp.; Unbalanced snow loads h This truss has been desig non-concurrent with other Provide adequate drainag This truss has been desig 	ave been considered for this design. ned for greater of min roof live load of 1 live loads.	2E) -0-11-0 to 4-1-0, Internet texposed ; end vertical I tegrip DOL=1.60 DL=1.15); Pf=25.0 psf (Lu 2.0 psf or 2.00 times flat d nonconcurrent with any nect truss to bearing wall	ior(1) 4-1-0 to 7-11 eft and right expos m DOL=1.15 Plate roof load of 25.0 pe other live loads. s due to UPLIFT a	-4, Exterior(2R) 7-11-4 ed;C-C for members DOL=1.15); Is=1.0; sf on overhangs		STATE OF MISS	S OLIPI



July 13,2022

MiTek 16023 Swingley Ridge Rd Chesterfield, MO 63017

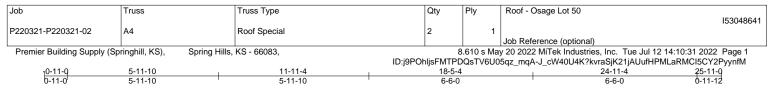


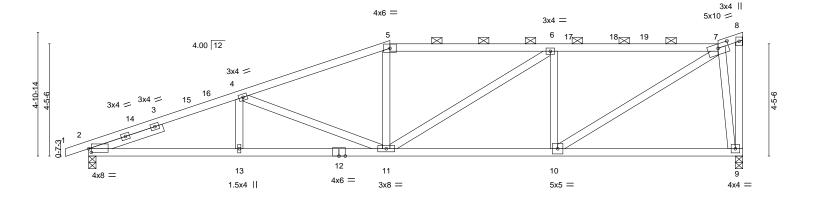


F		9-11-4		16-5-4		22-11-4	25-11-0
Plate Offsets ()	V V) [2:0.1	9-11-4 12,0-2-8], [7:0-4-12,Edge]	1	6-6-0	1	6-6-0	2-11-12
		12,0-2-6], [7.0-4-12,Euge]					
LOADING (pst TCLL (roof) Snow (Pf) TCDL	f) 25.0 25.0 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15	CSI. TC 0.89 BC 0.99	DEFL. Vert(LL) Vert(CT)	in (loc -0.18 2-1 -0.40 2-1	3 >999 240 3 >770 180	PLATES GRIP MT20 197/144
BCLL BCDL	0.0 10.0	Rep Stress Incr NO Code IRC2018/TPI2014	WB 0.76 Matrix-SH	Horz(CT)	0.10	9 n/a n/a	Weight: 112 lb FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS SLIDER	5-7: 2x4 SP 1	650F 1.5E		BRACING- TOP CHORD BOT CHORD WEBS	purlins (3-	8-11 max.): 5-7. ng directly applied or 6-	xcept end verticals, and 2-0-0 oc 5-11 oc bracing.
REACTIONS.	Max Horz 2= Max Uplift 9=	-0-3-8, 2=0-3-8 -210(LC 13) -243(LC 16), 2=-268(LC 12) -1331(LC 41), 2=1425(LC 42)					
FORCES. (Ib) TOP CHORD BOT CHORD WEBS	2-4=-2825/6 2-13=-804/2	/Max. Ten All forces 250 (lb) or less 6 52, 4-5=-2404/526, 5-6=-2268/526, 6-7 564, 11-13=-526/2571, 10-11=-526/25 38, 5-13=0/353, 6-13=-347/284, 6-10= 12	′=-1023/231 71, 9-10=-220/1057	932,			
Enclosed; M to 17-0-2, In and forces & 2) TCLL: ASCE Rough Cat (2) 3) Unbalanced 4) This truss ha non-concurr 5) Provide ade 6) This truss ha 7) One H2.5T S connection i 8) This truss is referenced s	IWFRS (envelo terior(1) 17-0-2 & MWFRS for re E 7-16; Pr=25.0 C; Fully Exp.; C snow loads ha as been design ent with other I quate drainage as been design Simpson Strong s for uplift only designed in ac standard ANSI/	to prevent water ponding. ed for a 10.0 psf bottom chord live load g-Tie connectors recommended to conr and does not consider lateral forces. cordance with the 2018 International R	E) -0-11-0 to 4-1-0, Int t exposed ; end vertica e grip DOL=1.60 L=1.15); Pf=25.0 psf (L 2.0 psf or 2.00 times fla nonconcurrent with ar nect truss to bearing we esidential Code sectio	erior(1) 4-1-0 to 9-1 I left and right expos Lum DOL=1.15 Plate at roof load of 25.0 p ny other live loads. alls due to UPLIFT a ns R502.11.1 and R	I-4, Exterior(ed;C-C for n DOL=1.15); of on overhau t jt(s) 9 and 2 302.10.2 and	2R) 9-11-4 nembers Is=1.0; ngs 2. This	STATE OF MISSOL



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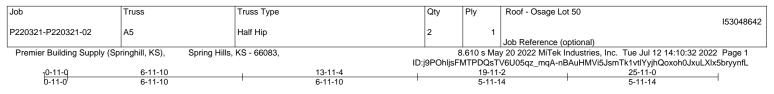


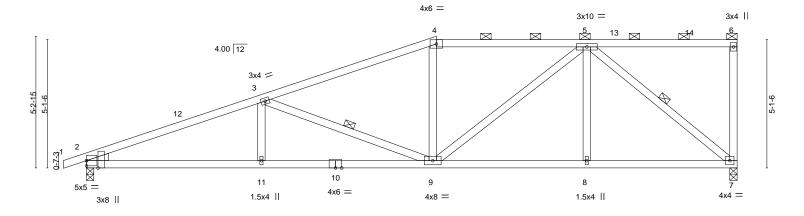


		5-11-10		1-11-4		18-5-4				25-11-0	
Plate Offsets		5-11-10 -4,0-1-12], [7:0-5-0,0-2		-11-10		6-6-0)			7-5-12	·
LOADING (i,o i izj, [i.o o o,o z	<u></u>								
TCLL (roof) Snow (Pf) TCDL	25.0 25.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 NO	CSI. TC BC WB	0.82 0.81 0.82	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.14 11-13 -0.25 11-13 0.07 9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCLL BCDL	0.0 10.0	Code IRC2018		Matri		1012(01)	0.07 9	n/a	11/a	Weight: 122 lb	FT = 20%
LUMBER-						BRACING-				I.	
TOP CHORE		F 1.5E *Except*				TOP CHORD				cept end verticals, and 2	2-0-0 oc
BOT CHORE	7-8: 2x4 SPF 2x4 SP 1650 9-12: 2x4 SP	F 1.5E *Except*				BOT CHORD	purlins (4-2-2 Rigid ceiling			-10-12 oc bracing.	
WEBS SLIDER	2x4 SPF No. Left 2x4 SPF										
	Max Horz 2= Max Uplift 9= Max Grav 9=	=-243(LC 16), 2=-268(l =1407(LC 41), 2=1501((LC 42)		a haven						
TOP CHORE BOT CHORE WEBS	2-4=-3011/ 2-13=-739/2	n/Max. Ten All forces 585, 4-5=-2030/488, 5- 2734, 11-13=-739/2732 228, 5-11=0/279, 6-11= 341	6=-1912/492, 6- 4, 10-11=-400/18	7=-1882/436 882, 9-10=-10	5/313	63,					
NOTES-											
 Wind: ASC Enclosed; 11-11-4 to members TCLL: AS' Rough Ca Unbalance This truss non-conce. Provide actions This truss This truss One H2.5' connection 	MWFRS (envelo 19-0-2, Interior(and forces & MV CE 7-16; Pr=25.1 t C; Fully Exp.; C ed snow loads ha has been desigr urrent with other dequate drainagy has been desigr T Simpson Stron n is for uplift only	5mph (3-second gust) ope) gable end zone ar (1) 19-0-2 to 25-9-4 zor VFRS for reactions sho O psf (roof LL: Lum DO Ce=0.9; Cs=1.00; Ct=1 ave been considered for ned for greater of min r live loads. e to prevent water pond hed for a 10.0 psf botto g-Tie connectors recor and does not conside coordance with the 201	nd C-C Exterior(ne; cantilever lef wn; Lumber DO L=1.15 Plate DO 10 or this design. oof live load of 1 ding. m chord live load nmended to con r lateral forces.	2E) -0-11-0 tc and right exit L=1.60 plate DL=1.15); Pf= 2.0 psf or 2.0 d nonconcurr nect truss to	 0 4-1-0, Interpretended in the possed ; end grip DOL=1 25.0 psf (Lu 0 times flat ent with any bearing wal 	rior(1) 4-1-0 to 11-1 vertical left and righ .60 um DOL=1.15 Plate roof load of 25.0 ps / other live loads. Is due to UPLIFT at	1-4, Exterior(2I ht exposed;C-C DOL=1.15); Is of on overhangs if (s) 9 and 2. T	R) ; for =1.0;		STATE OF MIS SCOTT M SEVIER	SOUR T

referenced standard ANSI/TPI 1. 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





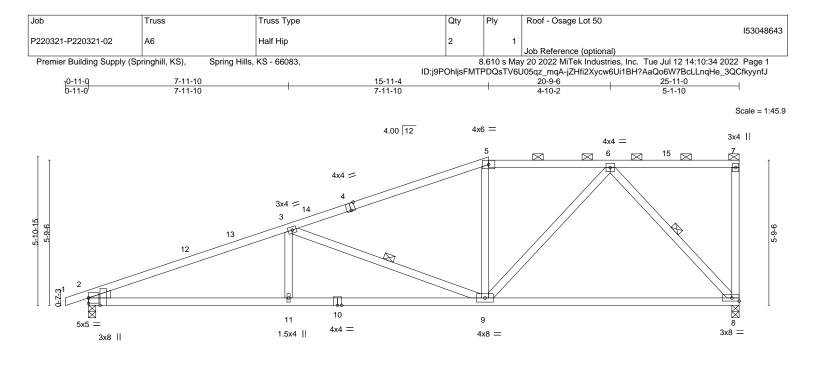


 	6-11-10 6-11-10	<u>13-11-4</u> 6-11-10		<u>19-11-2</u> 5-11-14		<u>25-11-0</u> 5-11-14	
Plate Offsets (X,Y) [2:0-3	3-8,Edge]	01110		01111		0.11.11	
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI. TC 0.95 BC 0.84 WB 0.61 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.18 2-11 >999 -0.31 2-11 >999 0.08 7 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 114 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP 1650 4-6: 2x4 SP BOT CHORD 2x4 SP 1650 7-10: 2x4 SP WEBS 2x4 SPF No WEDGE Left: 2x4 SPF No.2	DF 1.5E *Except* PF No.2		BRACING- TOP CHORD BOT CHORD WEBS	Sheathed or 2-10-6 of purlins (3-8-4 max.): Rigid ceiling directly 1 Row at midpt	4-6.		2-0-0 oc
Max Horz 2 Max Uplift 7 Max Grav 7 FORCES. (Ib) - Max. Com TOP CHORD 2-3=-3255 BOT CHORD 2-11=-749	'=0-3-8, 2=0-3-8 !=221(LC 13) '=-243(LC 12), 2=-267(LC 12) '=1393(LC 37), 2=1575(LC 38) p./Max. Ten All forces 250 (lb) or less /567, 3-4=-1890/431, 4-5=-1666/439, 6- /2937, 9-11=-749/2937, 8-9=-350/1325, 9, 3-9=-1358/286, 5-9=-171/797, 5-8=0/	7=-264/102 7-8=-350/1325					
NOTES- 1) Unbalanced roof live load 2) Wind: ASCE 7-16; Vult=1 Enclosed; MWFRS (enve 13-11-4 to 21-0-2, Interior members and forces & M 3) TCLL: ASCE 7-16; PT=25 Rough Cat C; Fully Exp.; 4) Unbalanced snow loads H 5) This truss has been desig non-concurrent with other 6) Provide adequate drainag 7) This truss has been desig 8) One H2.5T Simpson Stro connection is for uplift onl	Is have been considered for this design. 15mph (3-second gust) Vasd=91mph; T lope) gable end zone and C-C Exterior(2 r(1) 21-0-2 to 25-9-4 zone; cantilever left WFRS for reactions shown; Lumber DOI 0 psf (roof LL: Lum DOL=1.15 Plate DC Ce=0.9; Cs=1.00; Ct=1.10 have been considered for this design. Ined for greater of min roof live load of 1 live loads. ge to prevent water ponding. Ined for a 10.0 psf bottom chord live load ng-Tie connectors recommended to con y and does not consider lateral forces. accordance with the 2018 International F	CDL=6.0psf; BCDL=6.0p E) -0-11-0 to 4-1-0, Inter and right exposed ; end L=1.60 plate grip DOL=1. L=1.15); Pf=25.0 psf (Lu 2.0 psf or 2.00 times flat n d nonconcurrent with any nect truss to bearing wall	ior(1) 4-1-0 to 13-1 vertical left and rigi 60 m DOL=1.15 Plate roof load of 25.0 ps other live loads. s due to UPLIFT at	1-4, Exterior(2R) ht exposed;C-C for DOL=1.15); Is=1.0; of on overhangs		STATE OF MIS SCOTT M. SEVIER PE-20010188	tient

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



MiTek 16023 Swingley Ridge Rd Chesterfield, MO 63017



	L	7-11-10	15-11-4		_			25-11-0		
Plate Offsets	(X Y) [2·0-?	7-11-10 ' 3-8,Edge], [4:0-2-0,Edge]	7-11-10					9-11-12	2	
LOADING (p TCLL (roof) Snow (Pf) TCDL BCLL BCDL		SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI. TC 0.97 BC 0.92 WB 0.68 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.27 -0.55 0.09	(loc) 8-9 8-9 8	l/defl >999 >558 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 118 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS WEDGE Left: 2x4 SPF	 2x4 SP 1650 5-7: 2x4 SP 1650 2x4 SP 1650 2x4 SPF No. 			BRACING- TOP CHORD BOT CHORD WEBS	5-7. Rigid	,	directly a	,	nd 2-0-0 oc purlins (4-4 -15 oc bracing. 8	-8 max.):
TOP CHORD BOT CHORD	Max Horz 2 Max Uplift 8 Max Grav 8 Ib) - Max. Comp 2-3=-3354/ 2-11=-754/	=0-3-8, 2=0-3-8 =252(LC 13) =-246(LC 12), 2=-265(LC 12) =1306(LC 37), 2=1638(LC 38) 0./Max. Ten All forces 250 (lb) or less 549, 3-5=-1748/366, 5-6=-1509/380 3025, 9-11=-754/3025, 8-9=-326/920	·							
 2) Wind: ASC Enclosed; 15-11-4 to members a 3) TCLL: ASC Rough Cat 	ed roof live load CE 7-16; Vult=1 MWFRS (envel 23-0-2, Interior and forces & M CE 7-16; Pr=25 t C; Fully Exp.;	3, 3-9=-1621/353, 6-9=-149/996, 6-8=-1 s have been considered for this design. 15mph (3-second gust) Vasd=91mph; T ope) gable end zone and C-C Exterior((1) 23-0-2 to 25-9-4 zone; cantilever lef WFRS for reactions shown; Lumber DC 0 psf (roof LL: Lum DOL=1.15 Plate DC Ce=0.9; Cs=1.00; Ct=1.10 ave been considered for this design	CDL=6.0psf; BCDL=6.0p (2E) -0-11-0 to 4-1-0, Inter t and right exposed ; end DL=1.60 plate grip DOL=1.	rior(1) 4-1-0 to 15-1 vertical left and rig .60	1-4, Ext ht expos	erior(21 ed;C-C	R) C for		STE OF MIS	

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 2. This connection is for uplift only and does not consider lateral forces.

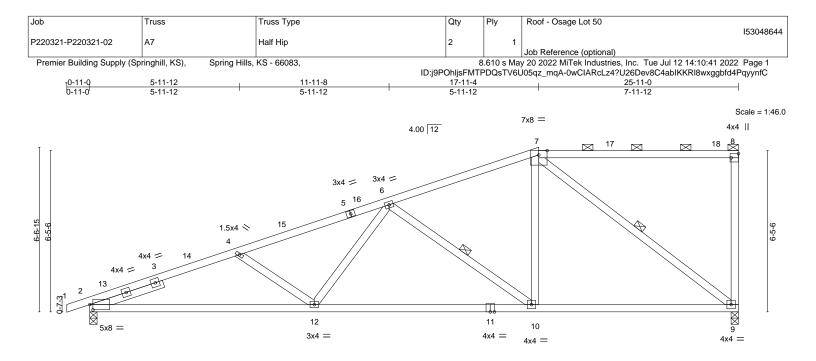
9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



July 13,2022

NITEK* 16023 Swingley Ridge Rd Chesterfield, MO 63017



	H	8-11-10	-	17-11-4				25-11-0	
Plate Offsets	(X Y) [2·0-1	8-11-10 -8,0-2-8], [8:Edge,0-3-8]	·	8-11-10				7-11-12	
LOADING (I TCLL (roof) Snow (Pf) TCDL BCLL BCDL		SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI. TC 0.97 BC 0.84 WB 0.90 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.19 10-12 -0.33 10-12 0.09 9	l/defl >999 >946 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 116 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORE BOT CHORE WEBS SLIDER REACTIONS	7-8: 2x4 SP 2 2x4 SPF No. 2-11: 2x4 SP 2x4 SPF No. Left 2x4 SPF 3. (size) 9 Max Horz 2: Max Uplift 9	2400F 2.0E 2 *Except* 1650F 1.5E 3 No.2 3-0-11 =0-3-8, 2=0-3-8		BRACING- TOP CHORD BOT CHORD WEBS	purlins (6-0-0) max.): 7 directly a	-8.	cept end verticals, and . -5 oc bracing. 7-9	2-0-0 oc
FORCES. TOP CHORE BOT CHORE WEBS	2-4=-3480/ 2-12=-768/	o./Max. Ten All forces 250 (lb) or less (565, 4-6=-2933/462, 6-7=-1455/327, 8-5 3175, 10-12=-614/2332, 9-10=-375/125 214, 6-12=-20/630, 6-10=-1311/295, 7-1	9=-426/145 3	91					
 Wind: AS(Enclosed; 17-11-4 tc members TCLL: AS Rough Ca Unbalance This truss non-conct Provide as This truss 	CE 7-16; Vult=11 MWFRS (envel-) 25-0-2, Interiorr and forces & MV CE 7-16; Pr=25. at C; Fully Exp.; (ed snow loads h has been desig urrent with other dequate drainag has been desig	s have been considered for this design. 15mph (3-second gust) Vasd=91mph; Ti ope) gable end zone and C-C Exterior(2 (1) 25-0-2 to 25-9-4 zone; cantilever left WFRS for reactions shown; Lumber DOL 0 psf (roof LL: Lum DOL=1.15 Plate DO Ce=0.9; Cs=1.00; Ct=1.10 ave been considered for this design. ned for greater of min roof live load of 12 live loads. e to prevent water ponding. ned for a 10.0 psf bottom chord live load	 E) -0-11-0 to 4-1-0, International texposed; end ==1.60 plate grip DOL=1 L=1.15); Pf=25.0 psf (Lu 2.0 psf or 2.00 times flat I nonconcurrent with any 	rior(1) 4-1-0 to 17-1 vertical left and rig .60 im DOL=1.15 Plate roof load of 25.0 ps	1-4, Exterior(21 ht exposed;C-C DOL=1.15); Is sf on overhangs	R) C for =1.0;	4	SCOTT M. SEVIER	SOURI *

8) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9 and 2. This

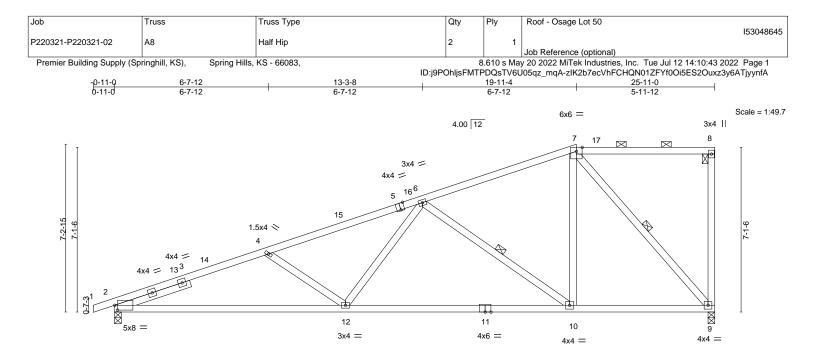
connection is for uplift only and does not consider lateral forces. 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and

referenced standard ANSI/TPI 1.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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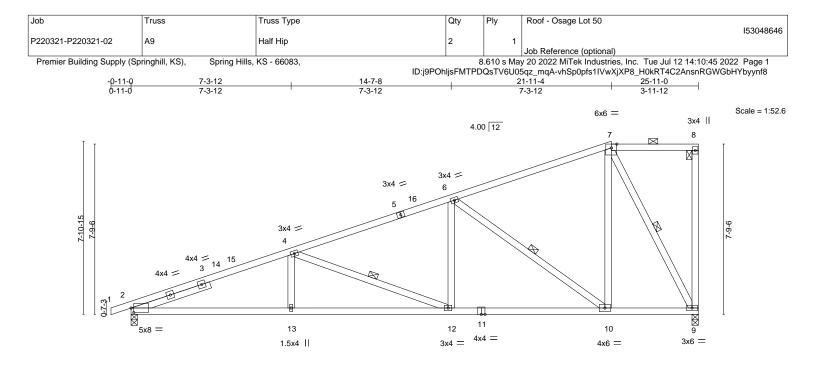


L	9-11-10	1	19-11-4		1	25-11-0	
	9-11-10		9-11-10		I	5-11-12	
	-8,0-2-8], [5:0-2-0,Edge]		1				
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO	CSI. TC 0.89 BC 0.82 WB 0.72	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.18 10-12 >999 -0.38 10-12 >810 0.09 9 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCLL 0.0 BCDL 10.0	Code IRC2018/TPI2014	Matrix-SH	1012(01)	0.09 9 11/a	1#a	Weight: 123 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP 1650 7-8: 2x4 SP BOT CHORD 2x4 SP 1650 9-11: 2x4 SP	F No.2 F 1.5E *Except*		BRACING- TOP CHORD BOT CHORD WEBS	Sheathed or 3-4-2 oc purlins (6-0-0 max.): Rigid ceiling directly : 1 Row at midpt	7-8.		0-0 oc
WEBS 2x4 SPF No. SLIDER Left 2x4 SPF	3		11200		0 10, 1	0	
Max Horz 2: Max Uplift 9 Max Grav 9 FORCES. (lb) - Max. Comp TOP CHORD 2-4=-3472/ BOT CHORD 2-12=-758/	=0-3-8, 2=0-3-8 =314(LC 13) =-252(LC 12), 2=-258(LC 12) =1281(LC 38), 2=1587(LC 38) b./Max. Ten All forces 250 (lb) or less 6 548, 4-6=-2880/432, 6-7=-1190/277, 8-5 3187, 10-12=-580/2214, 9-10=-311/986 241, 6-12=-19/726, 6-10=-1491/330, 7-	9=-313/126	270				
 NOTES- 1) Unbalanced roof live loads 2) Wind: ASCE 7-16; Vult=11 Enclosed; MWFRS (envel 19-11-4 to 25-9-4 zone; ca for reactions shown; Lumb 3) TCLL: ASCE 7-16; PT=25. Rough Cat C; Fully Exp.; (4) Unbalanced snow loads h 5) This truss has been desig non-concurrent with other 6) Provide adequate drainag 7) This truss has been desig 8) One H2.5T Simpson Stror connection is for uplift only 	s have been considered for this design. 15mph (3-second gust) Vasd=91mph; Tr ope) gable end zone and C-C Exterior(2 antilever left and right exposed ; end ver ber DOL=1.60 plate grip DOL=1.60 0 psf (roof LL: Lum DOL=1.15 Plate DO Ce=0.9; Cs=1.00; Ct=1.10 ave been considered for this design. ned for greater of min roof live load of 12 live loads. e to prevent water ponding. ned for a 10.0 psf bottom chord live load ng-Tie connectors recommended to coni y and does not consider lateral forces. ccordance with the 2018 International R	CDL=6.0psf; BCDL=6.0p (E) -0-11-0 to 4-1-0, Inter tical left and right expose L=1.15); Pf=25.0 psf (Lu 2.0 psf or 2.00 times flat r I nonconcurrent with any nect truss to bearing wall	sf; h=35ft; Ke=0.96 ior(1) 4-1-0 to 19-1 d;C-C for member: m DOL=1.15 Plate roof load of 25.0 ps other live loads. s due to UPLIFT at	1-4, Exterior(2E) s and forces & MWFRS DOL=1.15); Is=1.0; if on overhangs ; jt(s) 9 and 2. This		State OF MIS SCOTT M. SEVIER NUMBER PE-20010188	enter

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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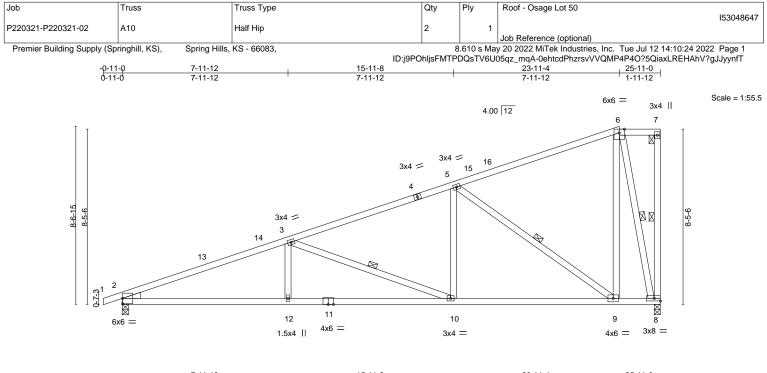
L	7-3-12	14-7-8		21-11-4	25-11-0	
Plate Offsets (X,Y) [2:0-1	7-3-12	7-3-12		7-3-12	3-11-12	
	-8,0-2-8]					
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO	CSI. TC 0.72 BC 0.67 WB 0.78	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d -0.20 12-13 >999 240 -0.33 12-13 >944 180 0.10 9 n/a n/a	PLATES MT20	GRIP 197/144
BCLL 0.0 BCDL 10.0	Code IRC2018/TPI2014	Matrix-SH	1012(01)	0.10 <i>3 1/a 1/a</i>	Weight: 132 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP 2400	F No.2, 1-5: 2x4 SP 1650F 1.5E 2 *Except* 2 1650F 1.5E 3		BRACING- TOP CHORD BOT CHORD WEBS	Sheathed or 3-8-1 oc purlins, of purlins (6-0-0 max.): 7-8. Rigid ceiling directly applied or 1 Row at midpt 4-1	, ,	-0-0 oc
Max Horz 2 Max Uplift 9	=0-3-8, 2=0-3-8 =344(LC 13) =-256(LC 12), 2=-255(LC 12) =1391(LC 38), 2=1546(LC 38)					
TOP CHORD 2-4=-3418/ BOT CHORD 2-13=-706/	o./Max. Ten All forces 250 (lb) or less (491, 4-6=-2314/362, 6-7=-882/225 (3140, 12-13=-706/3140, 10-12=-516/20 7, 4-12=-1123/236, 6-12=0/550, 6-10=-1 (368	89, 9-10=-242/679	,			
 Wind: ASCE 7-16; Vult=1 Enclosed; MWFRS (envel 21-11-4 to 25-9-4 zone; cr for reactions shown; Lumk TCLL: ASCE 7-16; Pr=25. Rough Cat C; Fully Exp.; Unbalanced snow loads h 	s have been considered for this design. 15mph (3-second gust) Vasd=91mph; T lope) gable end zone and C-C Exterior(2 antilever left and right exposed ; end ver ber DOL=1.60 plate grip DOL=1.60 .0 psf (roof LL: Lum DOL=1.15 Plate DC Ce=0.9; Cs=1.00; Ct=1.10 ave been considered for this design. ned for greater of min roof live load of 1: bits load	2E) -0-11-0 to 4-1-0, Interio tical left and right exposed 0L=1.15); Pf=25.0 psf (Lun	or(1) 4-1-0 to 21-1 d;C-C for member n DOL=1.15 Plate	1-4, Exterior(2E)	SCOTT M.	SOUR

- b) I his truss has been designed for greater of min root live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhand non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9 and 2. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



July 13,2022





	7-11-12	<u> </u>	+ <u>23-11-4</u> 7-11-12	<u>25-11-0</u> 1-11-12
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI. DEFL. TC 0.97 Vert(LL) BC 0.96 Vert(CT) WB 0.97 Horz(CT) Matrix-SH Horz(CT)	in (loc) l/defl L/d -0.22 2-12 >999 240 -0.39 2-12 >799 180 0.10 8 n/a n/a	PLATES GRIP MT20 197/144 Weight: 132 lb FT = 20%
LUMBER-		BRACING-		

LUMBER-		BRACING-		
TOP CHORD	2x4 SP 2400F 2.0E *Except*	TOP CHORD	Sheathed or 2-9-5 oc p	ourlins, except end verticals, and 2-0-0 oc
	6-7: 2x4 SPF No.2, 1-4: 2x4 SP 1650F 1.5E		purlins (6-0-0 max.): 6-	-7.
BOT CHORD	2x4 SPF No.2 *Except*	BOT CHORD	Rigid ceiling directly ap	oplied or 7-1-5 oc bracing.
	2-11: 2x4 SP 1650F 1.5E	WEBS	1 Row at midpt	7-8, 3-10, 5-9, 6-8
WEBS	2x4 SPF No.3			

WEDGE

Left: 2x4 SPF No.2

REACTIONS.	(size)	8=0-3-8, 2=0-3-8
	Max Horz	2=376(LC 13)
	Max Uplift	8=-260(LC 12), 2=-251(LC 12)
	Max Grav	8=1507(LC 38), 2=1501(LC 38)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3376/478, 3-5=-2110/325, 5-6=-553/173

BOT CHORD 2-12=-683/3087, 10-12=-683/3087, 9-10=-455/1879, 8-9=-163/361

WEBS 3-12=0/344, 3-10=-1289/279, 5-10=0/610, 5-9=-1842/357, 6-9=-162/1189, 6-8=-1634/377

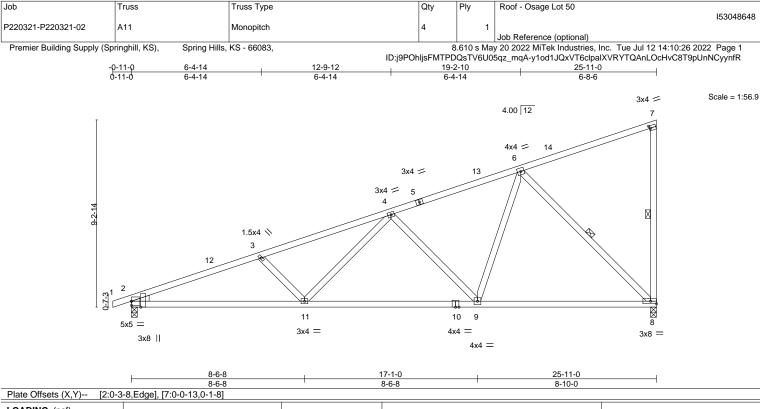
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior(1) 4-1-0 to 23-11-4, Exterior(2E) 23-11-4 to 25-9-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 2. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 BCLL 0.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2018/TPI2014	CSI. TC 0.96 BC 0.99 WB 0.76 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.17 8-9 -0.35 8-9 0.08 8	l/defl >999 >881 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 108 lb	GRIP 197/144 FT = 20%
BCDL10.0LUMBER- TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.3WEDGE			BRACING- TOP CHORD BOT CHORD WEBS		directly a	• /	ppt end verticals. D-11 oc bracing. B	

Left: 2x4 SPF No.2

REACTIONS.	(size)	8=0-3-8, 2=0-3-8
	Max Horz	2=410(LC 13)
	Max Uplift	8=-268(LC 16), 2=-246(LC 12)
	Max Grav	8=1443(LC 23), 2=1281(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2754/514, 3-4=-2458/451, 4-6=-1426/296, 7-8=-305/149

BOT CHORD 2-11=-720/2526, 9-11=-516/1822, 8-9=-333/1053

WEBS 3-11=-392/220, 4-11=-84/647, 4-9=-793/275, 6-9=-102/801, 6-8=-1483/349

NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior(1) 4-1-0 to 25-9-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

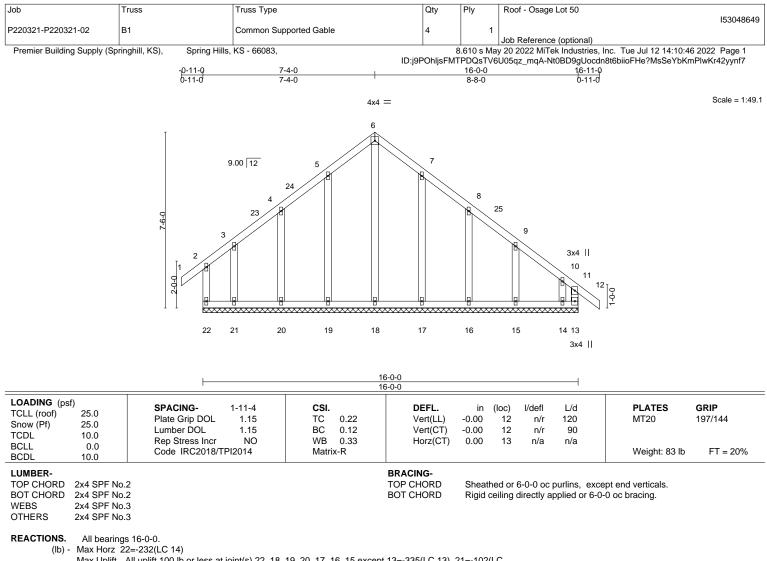
2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 2. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







Max Uplift All uplift 100 b or less at joint(s) 22, 18, 19, 20, 17, 16, 15 except 13=-335(LC 13), 21=-102(LC 16), 14=-275(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 22, 18, 21, 15 except 13=364(LC 14), 19=290(LC 23), 20=260(LC 23), 17=292(LC 24), 16=250(LC 24), 14=332(LC 15)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 4-5=-133/255, 5-6=-175/337, 6-7=-175/337, 7-8=-139/255, 10-11=-255/245

WEBS 6-18=-316/110, 5-19=-251/115, 7-17=-253/116

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-11-0 to 4-1-0, Exterior(2N) 4-1-0 to 7-4-0, Corner(3R) 7-4-0 to 12-4-0, Exterior(2N) 12-4-0 to 16-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.

7) All plates are 1.5x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

10) Gable studs spaced at 2-0-0 oc.

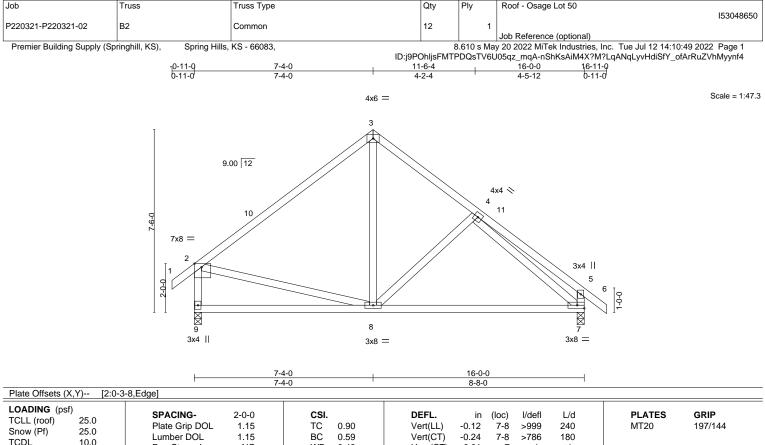
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

12) N/A

13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







10.0 0.0 10.0	Rep Stress Incr NO Code IRC2018/TPI2014	WB 0.48 Matrix-SH	Horz(CT)	0.01	7	n/a	n/a	Weight: 79 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

Sheathed or 5-2-3 oc purlins. except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

BCDL LUMBER-

BCLL

- 2x4 SP 1650F 1.5E *Except* TOP CHORD 3-6: 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.3 *Except* 2-9: 2x4 SPF No.2
- REACTIONS. (size) 9=0-3-8, 7=0-3-8 Max Horz 9=-239(LC 14) Max Uplift 9=-101(LC 16), 7=-111(LC 17) Max Grav 9=866(LC 23), 7=869(LC 24)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-754/154, 3-4=-708/187, 4-5=-318/118, 2-9=-804/193, 5-7=-359/149
- BOT CHORD 8-9=-233/342, 7-8=-43/655
- WEBS 3-8=-38/334, 4-8=-298/199, 2-8=-43/357, 4-7=-685/79
- NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior(1) 4-1-0 to 7-4-0, Exterior(2R) 7-4-0 to 12-4-0, Interior(1) 12-4-0 to 16-11-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9 and 7. This connection is for uplift only and does not consider lateral forces.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







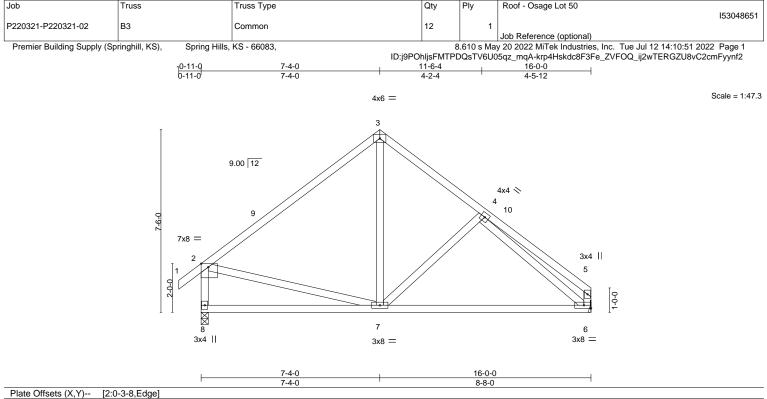
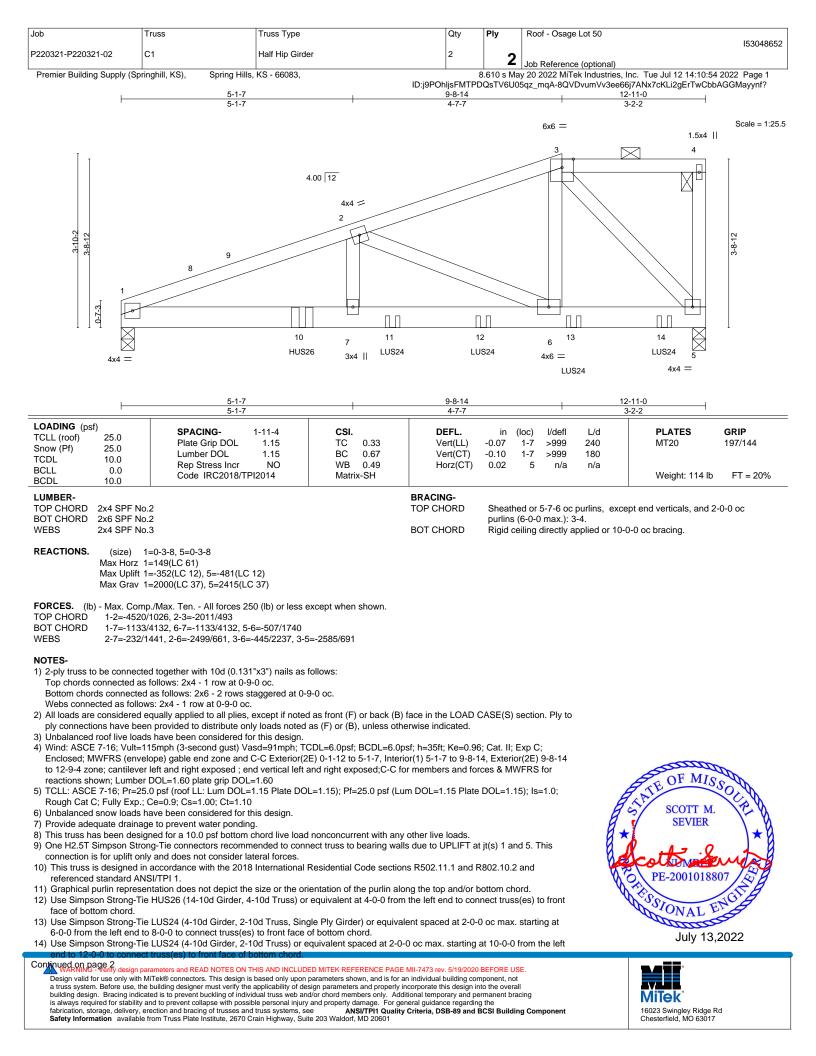


Plate Offsets (X,Y) [2:0-3	-8,Edgej							
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2018/TPI2014	CSI. TC 0.90 BC 0.59 WB 0.49 Matrix-SH		in (loc) -0.12 6-7 -0.24 6-7 0.01 6	l/defl >999 >786 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 77 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP 1650 3-5: 2x4 SPF BOT CHORD 2x4 SPF No. WEBS 2x4 SPF No. 2-8: 2x4 SPF	2 3 *Except*		BRACING- TOP CHORD BOT CHORD				ept end verticals. D-0 oc bracing.	
Max Horz 8= Max Uplift 8= Max Grav 8= FORCES. (Ib) - Max. Comp	=0-3-8, 6=Mechanical =-225(LC 14) =-101(LC 16), 6=-85(LC 17) =867(LC 23), 6=792(LC 24) -/Max. Ten All forces 250 (lb) or less 54, 3-4=-713/188, 4-5=-294/85, 2-8=-80							
WEBS 3-7=-40/336 NOTES- 1) Unbalanced roof live loads 2) Wind: ASCE 7-16; Vult=11	32, 6-7=-91/665 6, 4-7=-308/202, 2-7=-43/359, 4-6=-701 s have been considered for this design. .5mph (3-second gust) Vasd=91mph; T ope) gable end zone and C-C Exterior(2	CDL=6.0psf; BCDL=6.0						
forces & MWFRS for react 3) TCLL: ASCE 7-16; Pr=25.0 Rough Cat C; Fully Exp.; C 4) Unbalanced snow loads ha 5) This truss has been design non-concurrent with other 6) This truss has been design	ave been considered for this design. hed for greater of min roof live load of 1 live loads. hed for a 10.0 psf bottom chord live load	ip DOL=1.60 L=1.15); Pf=25.0 psf (Lu 2.0 psf or 2.00 times flat	um DOL=1.15 Plate I roof load of 25.0 psf	DOL=1.15); Is	s=1.0;	ł.	STATE OF MIL	SSOLLAL
9) One H2.5T Simpson Stron	to truss connections. action (by others) of truss to bearing plat ig-Tie connectors recommended to com v and does not consider lateral forces.					*	SEVIER	wer ~ !

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



16023 Swingley Ridge Rd Chesterfield, MO 63017



ſ	Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 50	
						153048652	2
	P220321-P220321-02	C1	Half Hip Girder	2	2		
					_	Job Reference (optional)	
	Premier Building Supply (Sp	ringhill, KS), Spring Hills,	KS - 66083,	8	.610 s Ma	y 20 2022 MiTek Industries, Inc. Tue Jul 12 14:10:54 2022 Page 2	
			ID:j9POł	nljsFMTPD	QsTV6U05	5qz_mqA-8QVDvumVv3ee66j7ANx7cKLi2gErTwCbbAGGMayynf?	

NOTES-

15) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

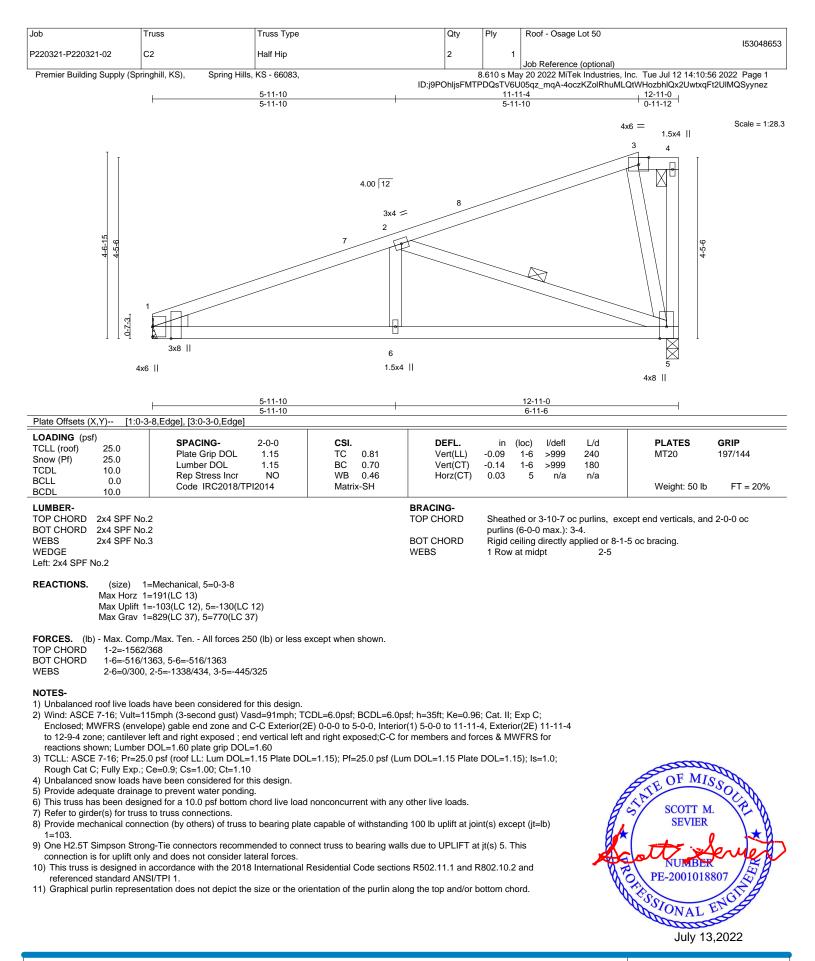
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-68, 3-4=-68, 1-5=-19

Concentrated Loads (lb)

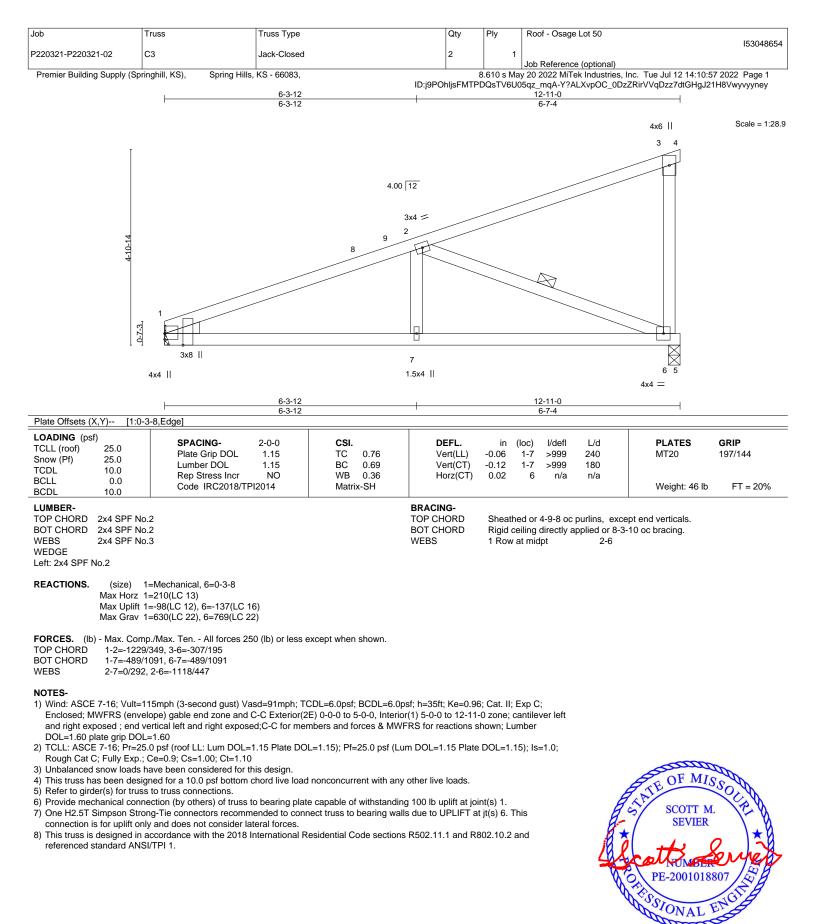
Vert: 10=-884(F) 11=-491(F) 12=-501(F) 13=-583(F) 14=-585(F)





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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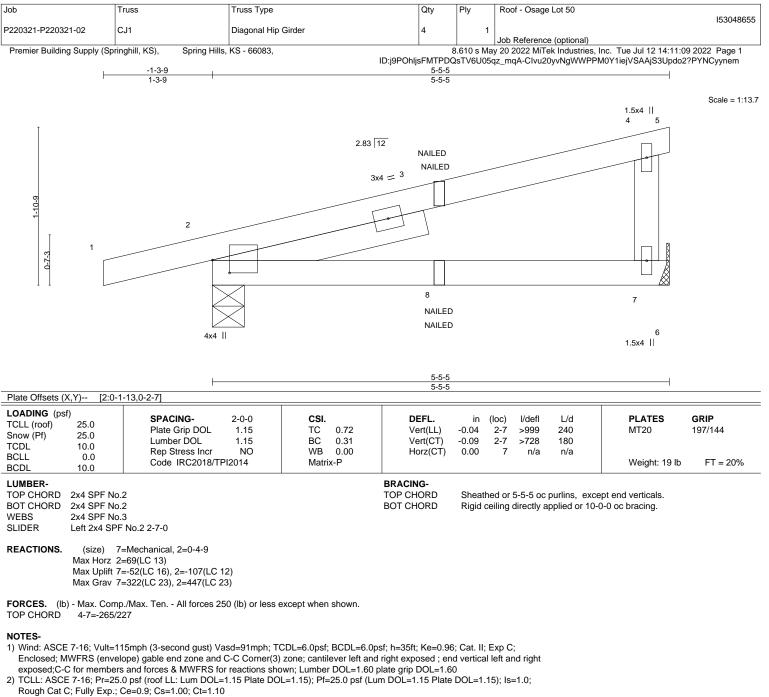


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E

July 13,2022



- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs
- non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7.
- 8) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) "NAILED" indicates 3-10d skew 45 to 135 degrees (0.148" x 3") toe-nails per NDS guidelines.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

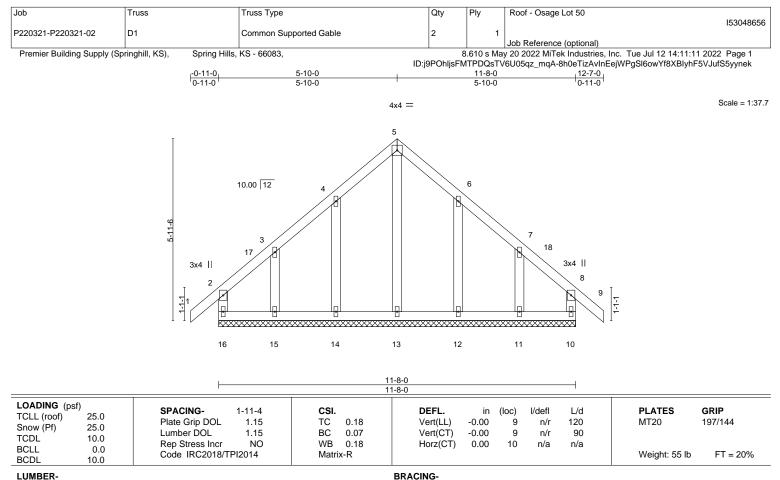
LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf) Vert: 1-4=-70, 4-5=-70, 2-6=-20



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16023 Swingley Ridge Rd Chesterfield, MO 63017



TOP CHORD

BOT CHORD

Sheathed or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

LUMBER-

TOP CHORD	2x4 SPF No.2
BOT CHORD	2x4 SPF No.2
WEBS	2x4 SPF No.3
OTHERS	2x4 SPF No.3

REACTIONS. All bearings 11-8-0.

Max Horz 16=181(LC 15) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 12 except 15=-130(LC 16), 11=-127(LC 17) Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 15, 11 except 14=305(LC 23), 12=305(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 4-5=-140/298. 5-6=-140/299

WFBS 5-13=-286/74, 4-14=-266/179, 6-12=-266/179

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-11-0 to 3-10-0, Exterior(2N) 3-10-0 to 5-10-0, Corner(3R) 5-10-0 to 10-10-0, Exterior(2N) 10-10-0 to 12-7-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.

7) All plates are 1.5x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

10) Gable studs spaced at 2-0-0 oc.

11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

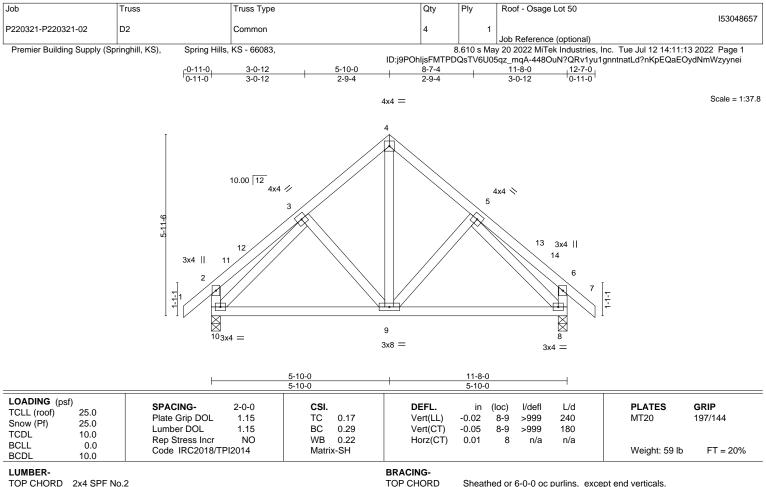
12) N/A

13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2

2x4 SPF No.3 WFBS

REACTIONS. (size) 10=0-3-8, 8=0-3-8 Max Horz 10=187(LC 15)

Max Uplift 10=-80(LC 16), 8=-80(LC 17) Max Grav 10=703(LC 23), 8=703(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- 3-4=-504/202, 4-5=-504/202, 2-10=-252/162, 6-8=-252/162 TOP CHORD
- BOT CHORD 9-10=-65/419 8-9=-35/419

WFBS 4-9=-146/295. 3-10=-531/82. 5-8=-531/82

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior(1) 4-1-0 to 5-10-0, Exterior(2R) 5-10-0 to 10-10-0, Interior(1) 10-10-0 to 12-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C: Fully Exp.: Ce=0.9: Cs=1.00: Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.

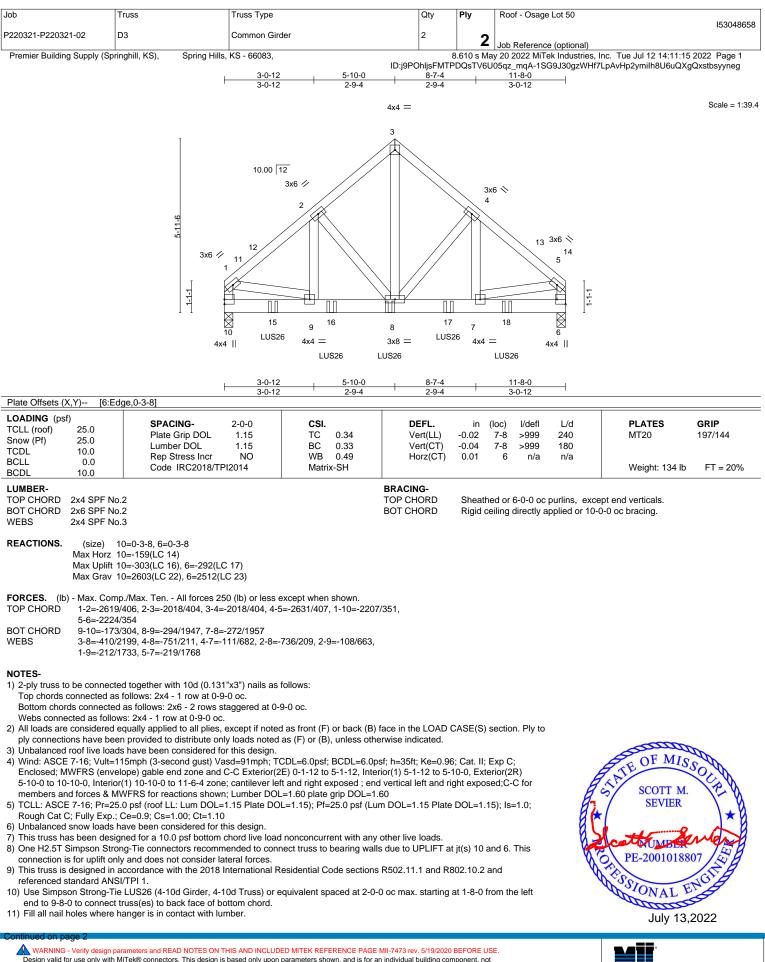
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

7) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 10 and 8. This connection is for uplift only and does not consider lateral forces.

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see
 MSIVTP11 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

16023 Swingley Ridge Rd Chesterfield, MO 63017

MiTek

Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 50	
					153048658	
P220321-P220321-02	D3	Common Girder	2	2		
				_	Job Reference (optional)	
Premier Building Supply (Spi	ringhill, KS), Spring Hills,	KS - 66083,	8	8.610 s Ma	y 20 2022 MiTek Industries, Inc. Tue Jul 12 14:11:15 2022 Page 2	
			ID:j9POhljsFMTPDQsTV6U05qz_mqA-1SG9J30gzWHf7LpAvHp2ymilh8U6uQXgQxstbsyyneg			

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-70, 3-5=-70, 6-10=-20

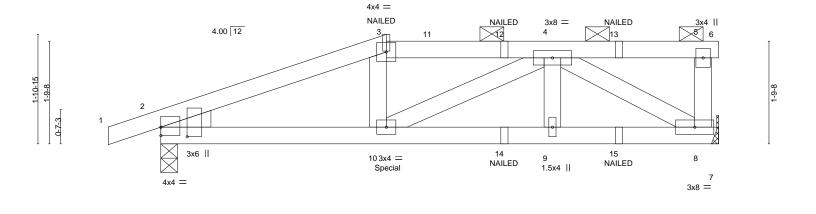
Concentrated Loads (lb)

Vert: 8=-772(B) 15=-772(B) 16=-772(B) 17=-772(B) 18=-772(B)





Scale = 1:20.1



	<u>3-11-4</u> 3-11-4		<u>6-10-1</u> 2-10-13			<u>9-8-14</u> 2-10-13	
Plate Offsets (X,Y) [2:0-0-0	0,0-1-12], [2:0-2-0,0-5-8]		21010			21010	
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCode IRC2018/TPI2014	CSI. TC 0.51 BC 0.66 WB 0.36 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)		l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 35 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.3 WEDGE Left: 2x4 SPF No.2		I	BRACING- TOP CHORD BOT CHORD	purlins (4-8-2		xcept end verticals, and	2-0-0 oc
Max Horz 2= Max Uplift 8= Max Grav 8=	-168(LC 13), 2=-199(LC 12) 904(LC 37), 2=946(LC 38)						
TOP CHORD 2-3=-1701/5 BOT CHORD 2-10=-509/15	/Max. Ten All forces 250 (lb) or less 6 03, 3-4=-1487/517 503, 9-10=-388/1205, 8-9=-388/1205 , 4-10=-138/444, 4-8=-1320/404	except when shown.					
 2) Wind: ASCE 7-16; Vult=115 Enclosed; MWFRS (envelop right exposed;C-C for memil 3) TCLL: ASCE 7-16; Pr=25.0 Rough Cat C; Fully Exp.; C. 4) Unbalanced snow loads have 5) This truss has been design non-concurrent with other li 6) Provide adequate drainage 7) This truss has been designt 8) Refer to girder(s) for truss ti 9) Provide mechanical connect 8=168. 10) One H2.5T Simpson Stron connection is for uplift only 11) This truss is designed in a referenced standard ANSI 12) Graphical purlin represent 13) "NAILED" indicates 3-10d 14 Anger(s) or other connect 	ve been considered for this design. ed for greater of min roof live load of 12 ve loads. to prevent water ponding. ed for a 10.0 psf bottom chord live load o truss connections. tion (by others) of truss to bearing plat ng-Tie connectors recommended to cor y and does not consider lateral forces. accordance with the 2018 International	E) zone; cantilever left a s shown; Lumber DOL= L=1.15); Pf=25.0 psf (Lu 2.0 psf or 2.00 times flat I nonconcurrent with any e capable of withstandir nect truss to bearing wa Residential Code sectio ientation of the purlin alo guidelines. ent to support concentra n device(s) is the respor	and right exposed ; e 1.60 plate grip DOL- Im DOL=1.15 Plate roof load of 25.0 pst / other live loads. Ing 100 lb uplift at join alls due to UPLIFT a Ins R502.11.1 and R ong the top and/or bother ted load(s) 312 lb do hasibility of others.	ind vertical left a =1.60 DOL=1.15); Is= i on overhangs it(s) except (jt=l t jt(s) 2. This 802.10.2 and ottom chord.	and 1.0; b)	STATE OF MI SCOTT M SEVIER NUMBER PE-2001018 PE-2001018 July 12	ETYOT
WARNING - Verify design para Design valid for use only with MiT a truss system. Before use, the bi building design. Bracing indicate is always required for stability and fabrication, storace, delivery, erec	meters and READ NOTES ON THIS AND INCLUD Tek® connectors. This design is based only upon uilding designer must verify the applicability of de d is to prevent buckling of individual truss web an d to prevent collapse with possible personal injury citon and bracing of trusses and truss systems, se m Truss Plate Institute, 2670 Crain Highway, Sui	parameters shown, and is for a sign parameters and properly d/or chord members only. Add and property damage. For ge ANSI/TPI1 Quality	an individual building com incorporate this design int ditional temporary and per	ponent, not o the overall manent bracing the	ponent	Mitek 16023 Swingley Ridge Chesterfield, MO 6301	

Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 50	
					153048659	
P220321-P220321-02	E1	Half Hip Girder	2	1		
					Job Reference (optional)	
Premier Building Supply (Springhill, KS), Spring Hills,		KS - 66083,	8.610 s May 20 2022 MiTek Industries, Inc. Tue Jul 12 14:11:22 2022 Page			
	ID:j9PO	hljsFMTPD	QsTV6U0	5qz_mqA-KoBonS63Jg9gTPsWpFRhkEVRAyod1dBi1X2lLyyyneZ		

LOAD CASE(S) Standard

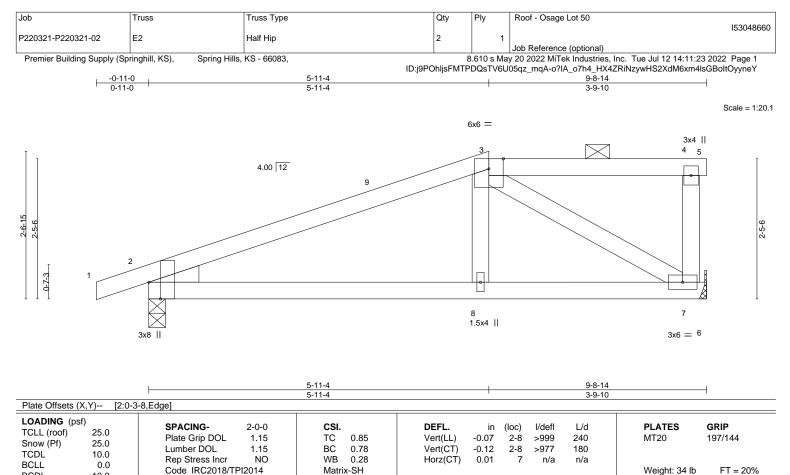
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-70, 3-5=-70, 5-6=-70, 2-7=-20

Concentrated Loads (lb)

Vert: 3=-118(F) 10=-312(F) 12=-118(F) 13=-118(F) 14=-19(F) 15=-19(F)





BC	JDL	10.0					
LU	JMBER-			BRACING-			
тс	OP CHORD	2x4 SPF No.2	2	TOP CHORD	Sheathed or 4-0-3 oc purlins, exce	pt end verticals, and 2-0-0 oc	
BC	OT CHORD	2x4 SPF No.2	2		purlins (6-0-0 max.): 3-5.	-	
W	EBS	2x4 SPF No.3	3	BOT CHORD	Rigid ceiling directly applied or 9-6-	0 oc bracing.	

WEDGE Left: 2x4 SPF No.2

REACTIONS. (size) 7=Mechanical, 2=0-3-8 Max Horz 2=99(LC 13) Max Uplift 7=-88(LC 12), 2=-124(LC 12) Max Grav 7=510(LC 37), 2=672(LC 38)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-751/281

BOT CHORD 2-8=-346/592, 7-8=-348/581

WEBS 3-7=-690/367

NOTES-

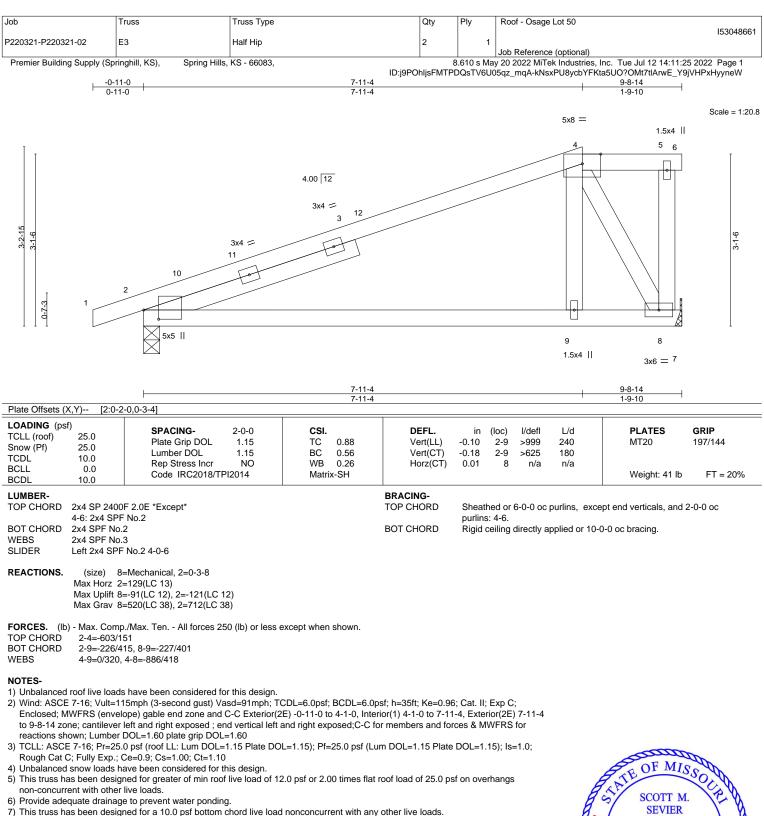
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior(1) 4-1-0 to 5-11-4, Exterior(2E) 5-11-4 to 9-8-14 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7.
- One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
 connection is for uplift only and does not consider lateral forces.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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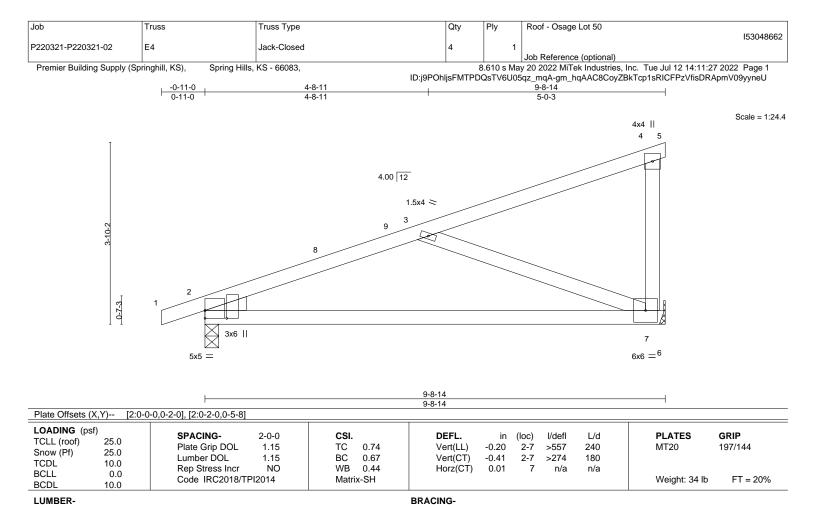


- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8.
- 10) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



July 13,2022





TOP CHORD

BOT CHORD

Sheathed or 5-10-2 oc purlins, except end verticals.

Rigid ceiling directly applied or 8-0-10 oc bracing.

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-872/352

 BOT CHORD
 2-7=-511/784

2x4 SPF No.2

2x4 SPF No 2

2x4 SPF No 3

WEBS 3-7=-767/495

NOTES-

TOP CHORD

BOT CHORD

WEBS 2x4 WEDGE Left: 2x4 SPF No.2 REACTIONS.

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior(1) 4-1-0 to 9-8-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

(size) 7=Mechanical, 2=0-3-8 Max Horz 2=161(LC 13)

Max Uplift 7=-103(LC 16), 2=-115(LC 12) Max Grav 7=602(LC 23), 2=575(LC 23)

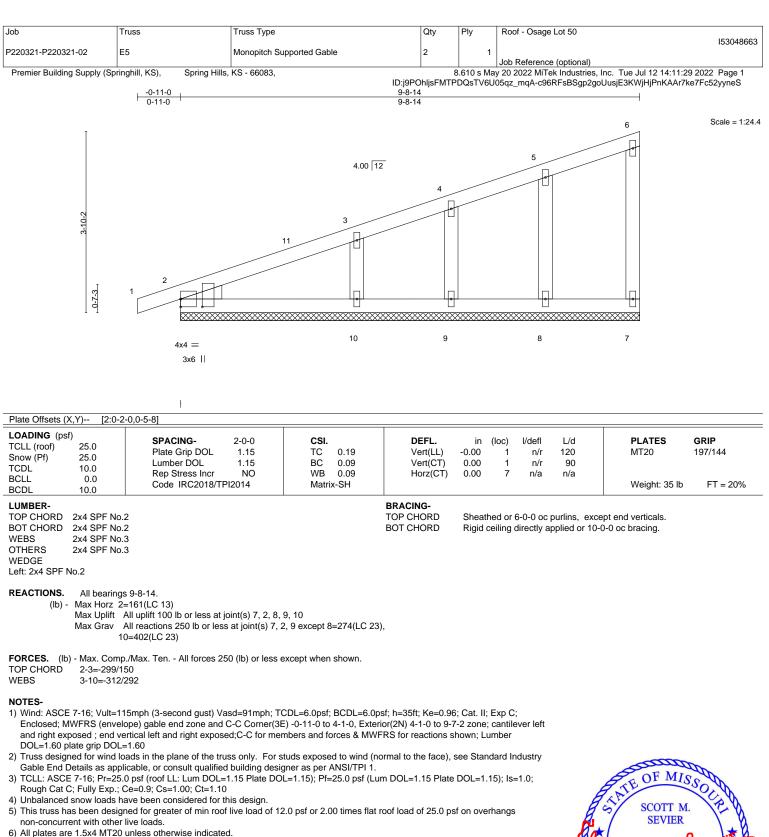
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Refer to girder(s) for truss to truss connections.

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=103.
- One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
 connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







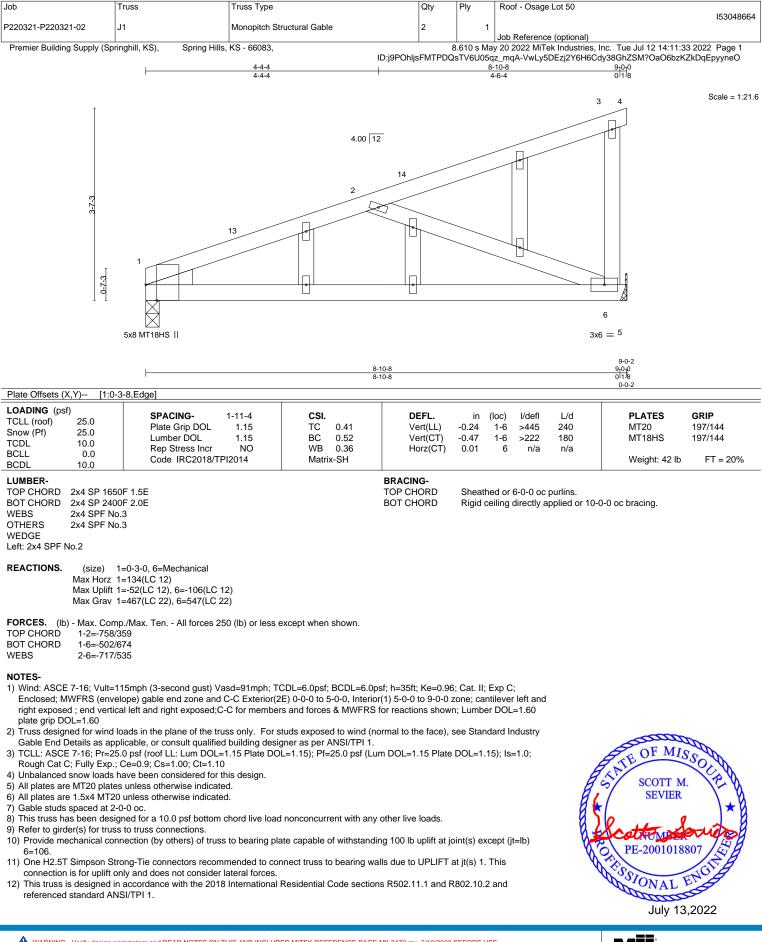
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) N/A

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

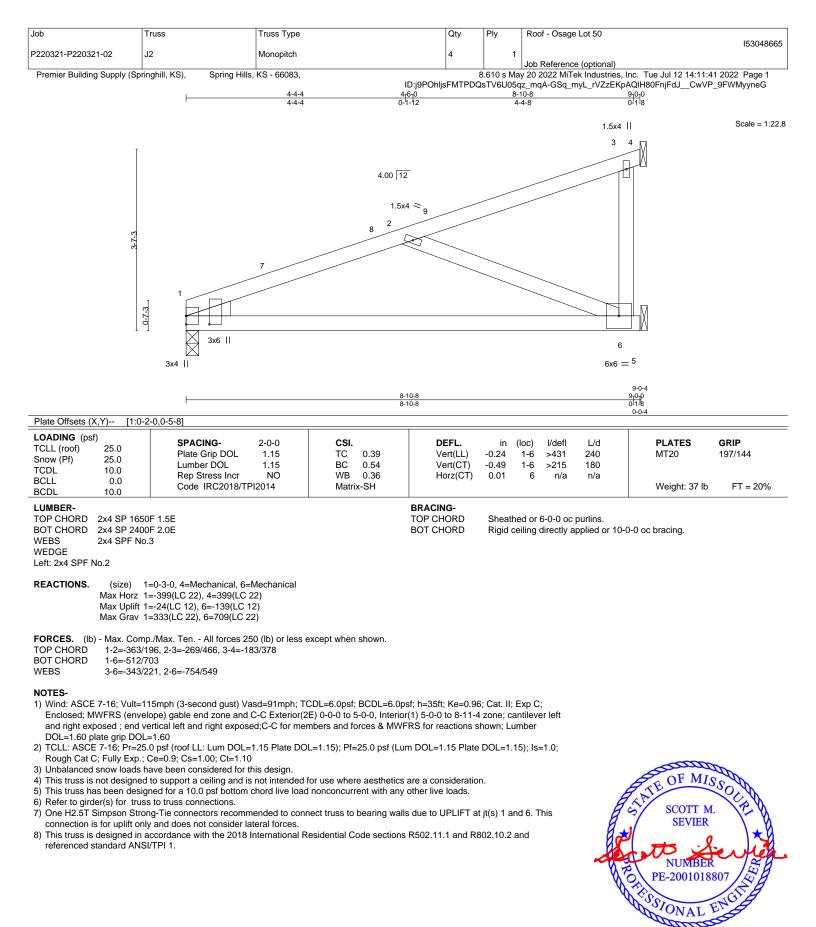






WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

16023 Swingley Ridge Rd Chesterfield, MO 63017

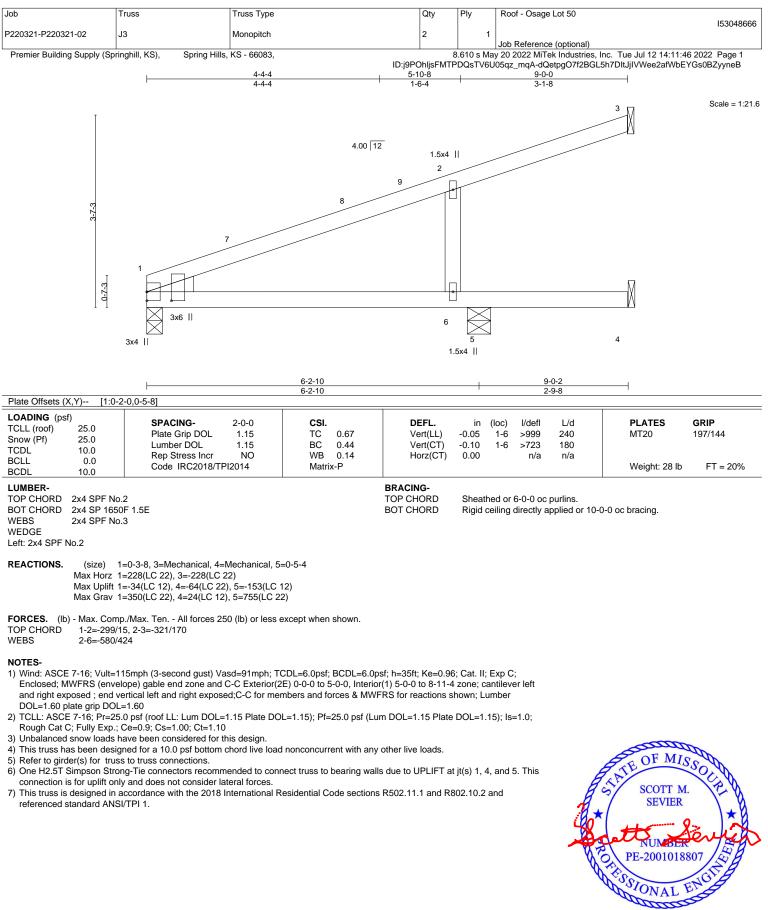


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



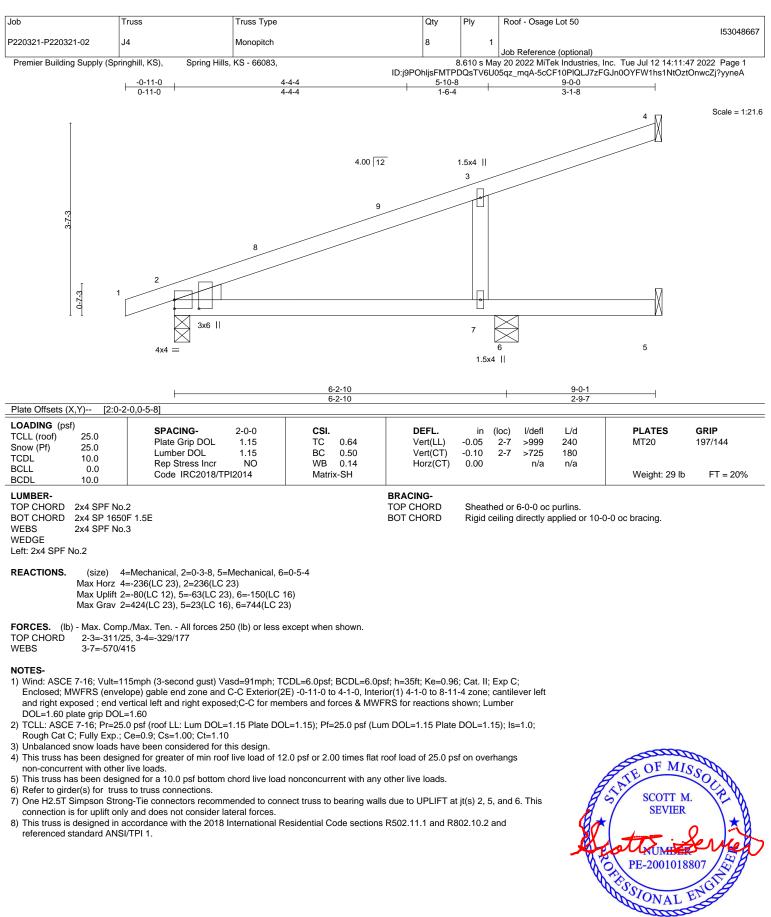
16023 Swingley Ridge Rd Chesterfield, MO 63017

July 13,2022



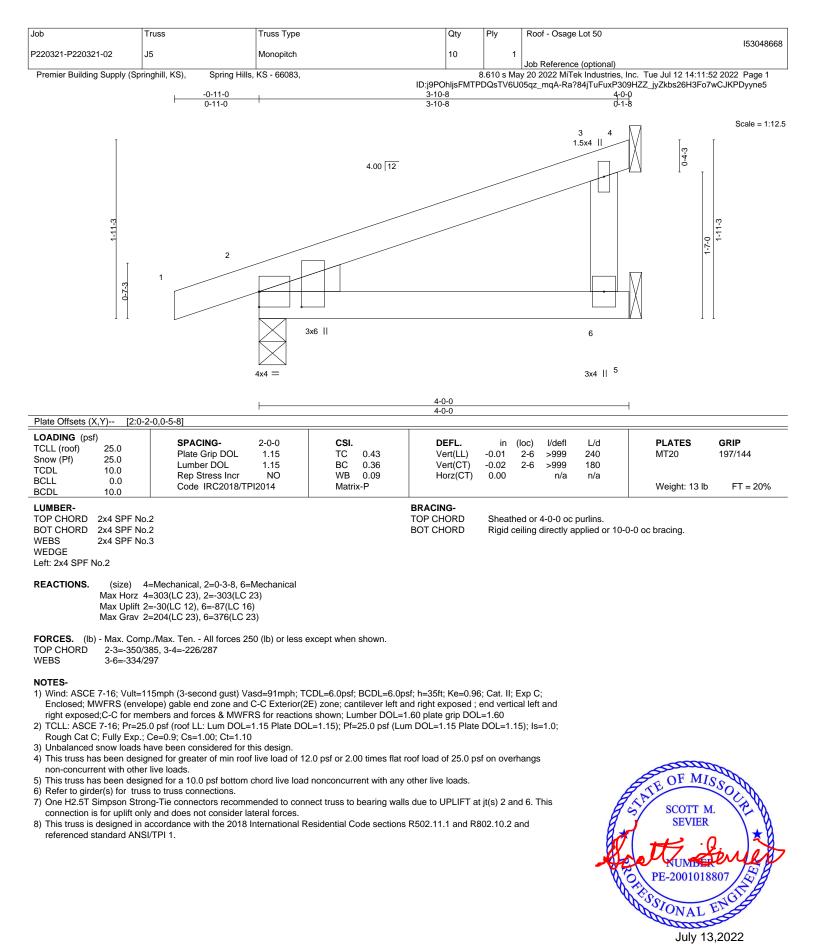
July 13,2022



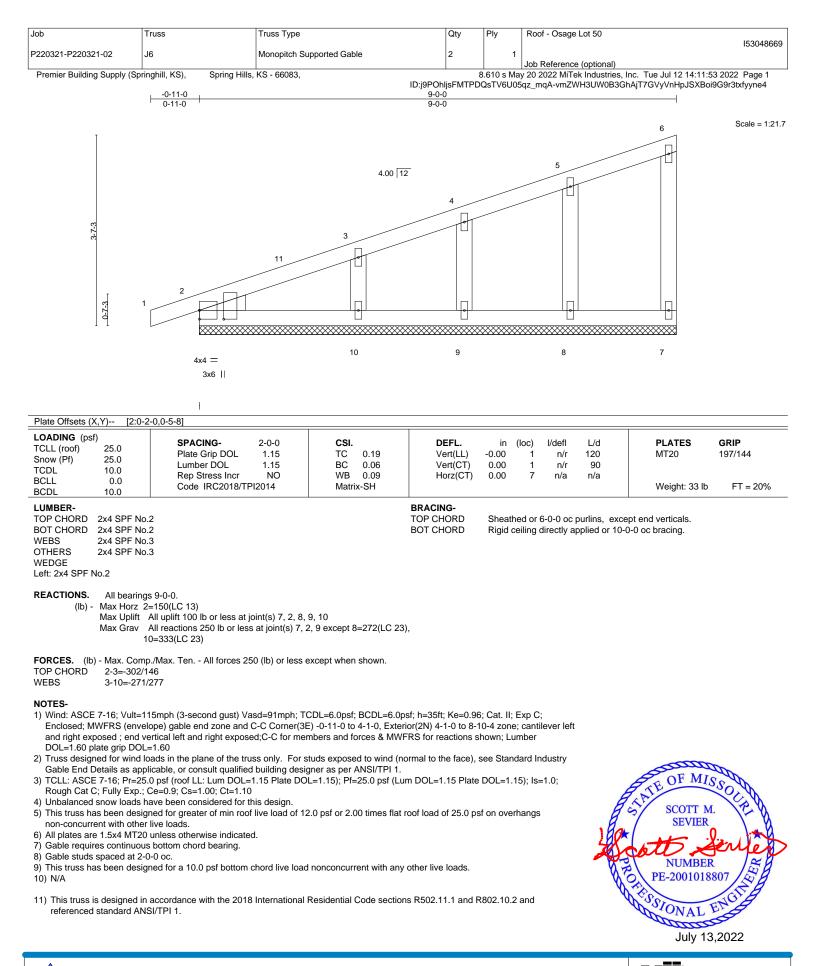


July 13,2022

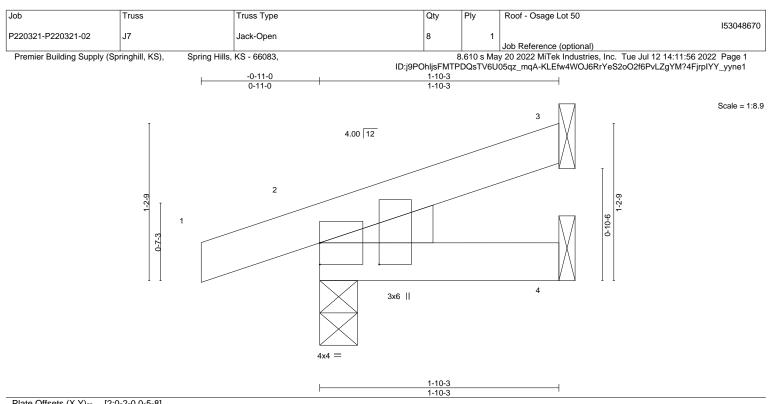








Mitek° 16023 Swingley Ridge Rd Chesterfield, MO 63017



LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCode IRC2018/TPI2014	CSI. TC 0.13 BC 0.03 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 -0.00	(loc) 2 2-4 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 7 lb	GRIP 197/144 FT = 20%
LUMBER-			BRACING-						

BOT CHORD

Sheathed or 1-10-3 oc purlins

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEDGE Left: 2x4 SPF No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=39(LC 12) Max Uplift 3=-32(LC 16), 2=-58(LC 12) Max Grav 3=65(LC 23), 2=214(LC 23), 4=37(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 8) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



July 13,2022



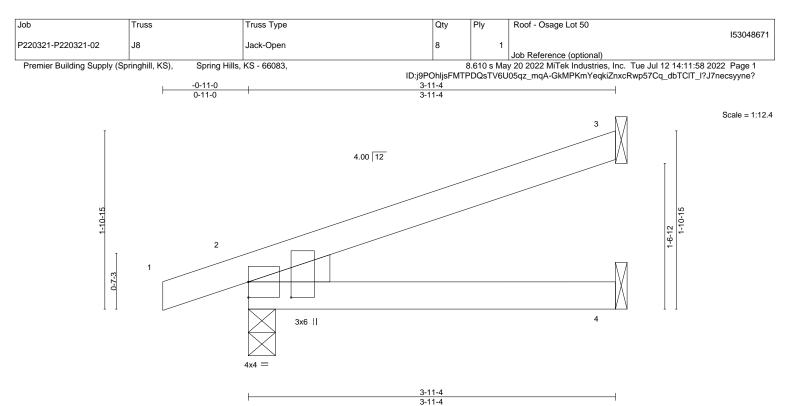


Plate Offsets (X,Y) [2:0	-2-0,0-5-8]								
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 DOUL 0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO	CSI. TC 0.42 BC 0.17 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.03 -0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCLL 0.0 BCDL 10.0	Code IRC2018/TPI2014	Matrix-P						Weight: 11 lb	FT = 20%
LUMBER-			BRACING-						

BOT CHORD

Sheathed or 3-11-4 oc purlins

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEDGE Left: 2x4 SPF No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=68(LC 12) Max Uplift 3=-70(LC 16), 2=-67(LC 12) Max Grav 3=188(LC 23), 2=343(LC 23), 4=77(LC 7)

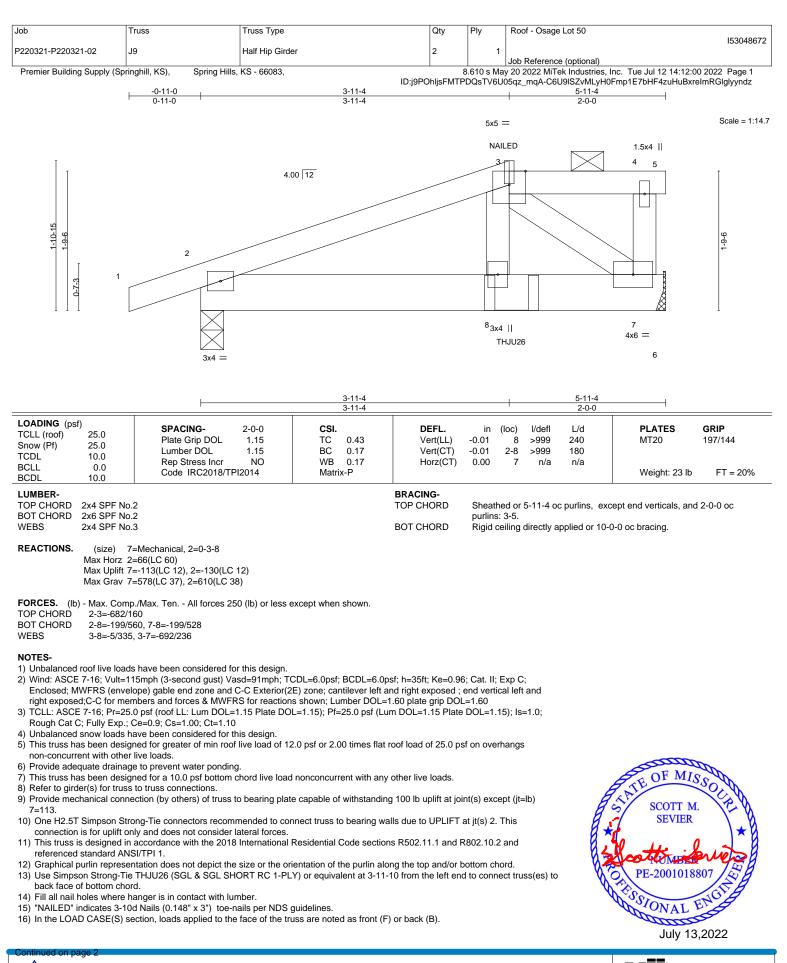
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 8) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







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MITEK° 16023 Swingley Ridge Rd Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 50
	10				153048672
P220321-P220321-02	J9	Half Hip Girder	2	1	
					Job Reference (optional)
Premier Building Supply (Spi	inghill, KS), Spring Hills,	KS - 66083,	8	.610 s Ma	y 20 2022 MiTek Industries, Inc. Tue Jul 12 14:12:00 2022 Page 2
		ID:j9PC	hljsFMTPI	DQsTV6U	05qz_mqA-C6U9ISZvMLyH0Fmp1E7bHF4zuHuBxreImRGlglyyndz

LOAD CASE(S) Standard

 Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-70, 3-4=-70, 4-5=-70, 2-6=-20
 Concentrated Leade (lb)

Concentrated Loads (lb) Vert: 3=-118(B) 8=-312(B)



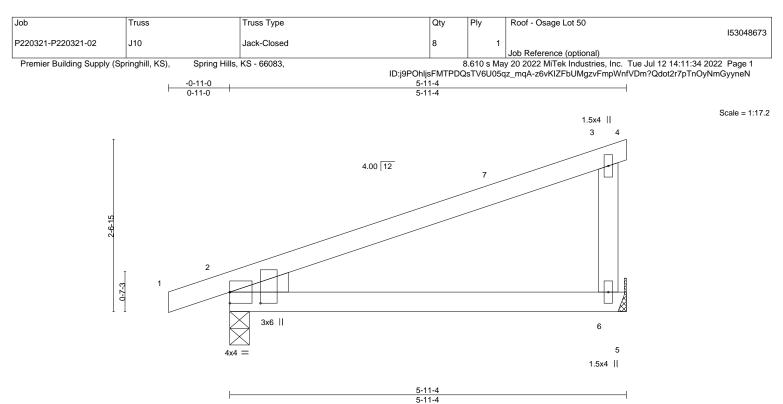


Plate Offsets (X,Y)	2:0-2-0,0-5-8]										
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 PCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 NO	CSI. TC BC WB	0.80 0.75 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.06 -0.12 0.00	(loc) 2-6 2-6 6	l/defl >999 >553 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCLL 0.0 BCDL 10.0	Code IRC2018/T	PI2014	Matri	k-P						Weight: 21 lb	FT = 20%
LUMBER-					BRACING-						

BOT CHORD

Sheathed or 5-11-4 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP 1650F 1.5E BOT CHORD 2x4 SPF No.2 2x4 SPF No 3 WFBS WEDGE Left: 2x4 SPF No.2

REACTIONS. (size) 6=Mechanical, 2=0-3-8 Max Horz 2=103(LC 13) Max Uplift 6=-64(LC 16), 2=-86(LC 12) Max Grav 6=376(LC 23), 2=451(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-6=-314/271

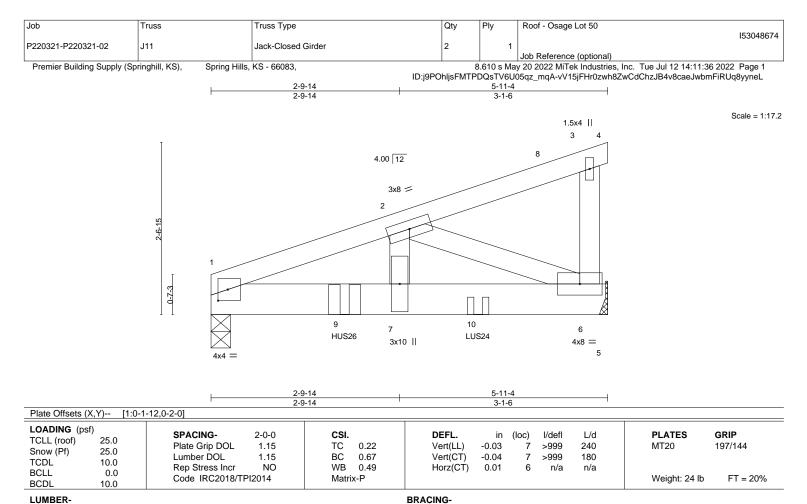
NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior(1) 4-1-0 to 5-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6.
- 8) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



July 13,2022





BOT CHORD

Sheathed or 4-2-1 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x6 SPF No 2

2x4 SPF No 3 WFBS REACTIONS.

(size) 1=0-3-8, 6=Mechanical Max Horz 1=99(LC 40)

Max Uplift 1=-151(LC 12), 6=-179(LC 16) Max Grav 1=1061(LC 22), 6=1072(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1830/430

BOT CHORD 1-7=-499/1646 6-7=-499/1646

2-7=-134/1090, 2-6=-1777/528 WFBS

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 5-1-12, Interior(1) 5-1-12 to 5-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C: Fully Exp.: Ce=0.9: Cs=1.00: Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=179
- 7) One H2.5T Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) Use Simpson Strong-Tie HUS26 (14-10d Girder, 6-10d Truss, Single Ply Girder) or equivalent at 2-0-0 from the left end to connect truss(es) to back face of bottom chord.
- 10) Use Simpson Strong-Tie LUS24 (4-SD9112 Girder, 2-SD9212 Truss, Single Ply Girder) or equivalent at 4-0-0 from the left end to connect truss(es) to back face of bottom chord.
- 11) Fill all nail holes where hanger is in contact with lumber.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).





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MiTek 16023 Swingley Ridge Rd Chesterfield, MO 63017

[Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 50
	D000004 D000004 00	14.4	lash Olasad Giadan			153048674
	P220321-P220321-02	J11	Jack-Closed Girder	2	1	
						Job Reference (optional)
	Premier Building Supply (Spi	ringhill, KS), Spring Hills,	KS - 66083,	8	.610 s Ma	y 20 2022 MiTek Industries, Inc. Tue Jul 12 14:11:36 2022 Page 2
			ID:j9PO	hljsFMTPI	QsTV6U	05qz_mqA-vV15jFHr0zwh8ZwCdChzJB4v8caeJwbmFiRUq8yyneL

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-70, 3-4=-70, 1-5=-20

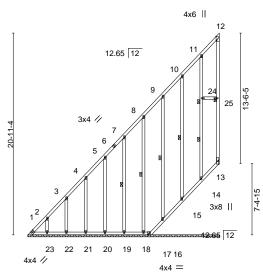
Concentrated Loads (lb) Vert: 9=-809(B) 10=-610(B)



Job	Truss	Truss Type	Qty	Ply	Roof - Osage Lot 50
					153048675
P220321-P220321-02	PG1	GABLE	2	1	
					Job Reference (optional)
Premier Building Supply (Sp	ringhill, KS), Spring Hills,	KS - 66083,		3.610 s Ma	y 20 2022 MiTek Industries, Inc. Tue Jul 12 14:12:04 2022 Page 1

ID:j9POhljsFMTPDQsTV6U05qz_mqA-5ujgbqcPQaSjVs3aG4CXR5EZTuAitXnuh3EzpWyyndv <u>19-10-6</u> 19-10-6 +

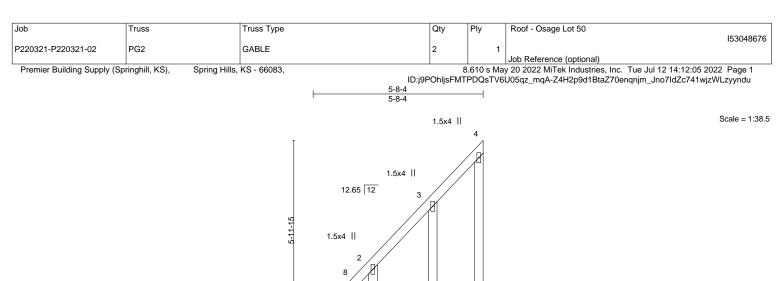
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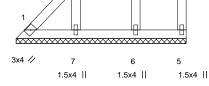


19-10-6 7-0-6 12-10-0 12-10-0

H

Plate Offsets (X,Y) [12:E	dge,0-0-8]	12-10-0	100	,			
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI. TC 0.78 BC 0.45 WB 0.67 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - -0.01 13	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 167 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SPF No BOT CHORD 2x4 SPF No WEBS 2x4 SPF No OTHERS 2x4 SPF No	2 3 3 *Except* 1-14: 2x4 SPF No.2		BRACING- TOP CHORD BOT CHORD WEBS JOINTS		ing: 13-14. pt 12-13	xcept end verticals. 0-0-0 oc bracing, Excep 3, 7-19, 8-18, 9-16, 10-1:	
(lb) - Max Horz 1 Max Uplift 1 2 1 Max Grav 6 FORCES. (lb) - Max. Comp TOP CHORD 1-2=-1258, 7-8=-709/7 BOT CHORD 16-17=-29		=-130(LC 16), 21=-131(L 6(LC 16), 16=-143(LC 16 23, 22, 21, 20, 19, 18, 1 4=304(LC 22) except when shown. , 4-5=-933/907, 5-7=-821 1=-381/408	_C 16), 6), 15=-118(LC 6, 15				
 Enclosed; MWFRS (enveand right exposed; end v DOL=1.60 plate grip DOL TCLL: ASCE 7-16; Pr=25 Rough Cat C; Fully Exp.; Unbalanced snow loads h All plates are 1.5x4 MT20 Gable requires continuous This truss has been design Bearing at joint(s) 13, 1, 1 formula. Building designes Provide mechanical connu 13=314, 16=143, 15=118 N/A 	0 psf (roof LL: Lum DOL=1.15 Plate DO Ce=0.9; Cs=1.00; Ct=1.10 ave been considered for this design. unless otherwise indicated. s bottom chord bearing. ned for a 10.0 psf bottom chord live load 7, 23, 22, 21, 20, 19, 18, 16, 15, 14 cons er should verify capacity of bearing surfar ection (by others) of truss to bearing plat	E) 0-4-1 to 5-4-1, Interior mbers and forces & MWF L=1.15); Pf=25.0 psf (Lur I nonconcurrent with any siders parallel to grain va ce. e capable of withstanding	r(1) 5-4-1 to 19-8-1 FRS for reactions s m DOL=1.15 Plate other live loads. lue using ANSI/TP g 100 lb uplift at join	0 zone; cantilev hown; Lumber DOL=1.15); Is= I 1 angle to grai	rer left =1.0; n	STATE OF MIS SCOTT M. SEVIER PE-20010188 PE-20010188	CHE A
	accordance with the 2018 International			802.10.2 and		July 13,	2022
Design valid for use only with M a truss system. Before use, the building design. Bracing indica is always required for stability a fabrication, storage, delivery, e	rameters and READ NOTES ON THIS AND INCLUD In Tek® connectors. This design is based only upon building designer must verify the applicability of de ted is to prevent buckling of individual truss web an and to prevent collapse with possible personal injury rection and bracing of trusses and truss systems, so from Truss Plate Institute, 2670 Crain Highway, Sui	parameters shown, and is for an sign parameters and properly in d/or chord members only. Addi v and property damage. For ger ee ANSI/TPI1 Quality	n individual building con accorporate this design in itional temporary and pe	nponent, not to the overall rmanent bracing g the	ponent	16023 Swingley Ridge F Chesterfield, MO 63017	łd





				1								
ADING (psf L (roof) w (Pf) DL L DL) 25.0 25.0 10.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TI	2-0-0 1.15 1.15 NO PI2014	CSI. TC BC WB Matri	0.53 0.03 0.09 x-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 27 lb	GRIP 197/144 FT = 20%
IBER-	10.0					BRACING-					1	

BOT CHORD

Sheathed or 5-8-4 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

LOAD TCLL Snow TCDL BCLL BCDL

TOP CHORD	2x4 SPF No.2
BOT CHORD	2x4 SPF No.2
WEBS	2x4 SPF No.3
OTHERS	2x4 SPF No.3

REACTIONS. All bearings 5-8-4.

(lb) - Max Horz 1=226(LC 13)

Max Uplif All uplif 100 lb or less at joint(s) 1, 5 except 6=-124(LC 16), 7=-136(LC 16) Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=294(LC 22), 7=274(LC 22)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-414/415, 2-3=-284/289

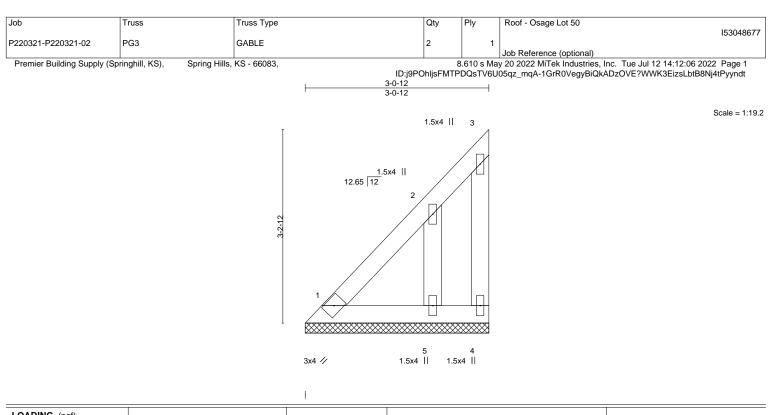
WEBS 3-6=-255/189

NOTES-

- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) N/A
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2018/TPI2014	CSI. TC 0.12 BC 0.02 WB 0.06 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 13 lb	GRIP 197/144 FT = 20%
LUMBER-			BRACING-						

BOT CHORD

Sheathed or 3-0-12 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 2x4 SPF No 3 WFBS OTHERS 2x4 SPF No.3

REACTIONS. (size) 1=3-0-12, 4=3-0-12, 5=3-0-12

Max Horz 1=112(LC 13)

Max Uplift 1=-19(LC 12), 4=-41(LC 15), 5=-112(LC 16)

Max Grav 1=101(LC 26), 4=33(LC 12), 5=235(LC 22)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;

Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

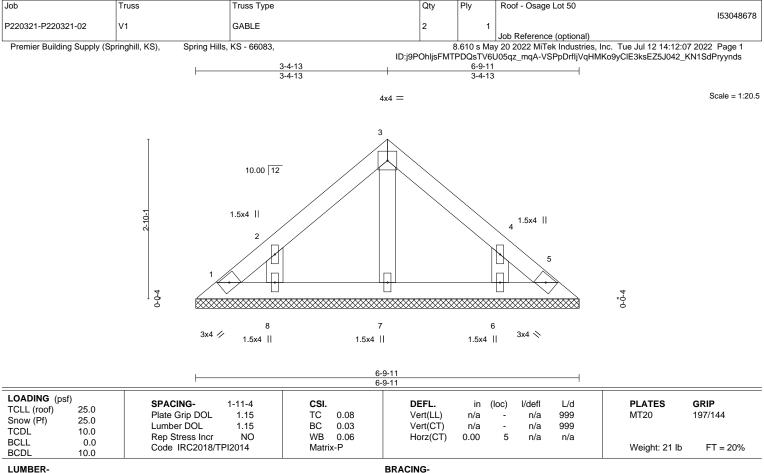
7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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⁶⁾ N/A



BOT CHORD

Sheathed or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 2x4 SPF No.3 OTHERS

REACTIONS. All bearings 6-9-11.

Max Horz 1=-65(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=253(LC 22), 6=253(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) N/A

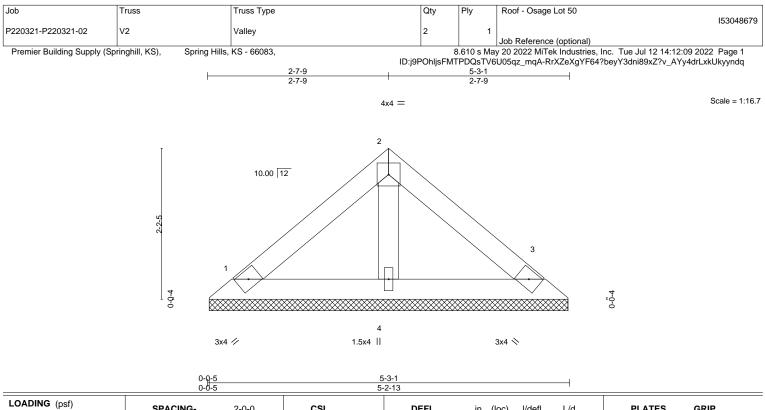
8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 25.0 25.0 10.0 0.0 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2018/TPI2014	CSI. TC 0.15 BC 0.05 WB 0.03 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 14 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD	2x4 SPF No.2			BRACING- TOP CHORD	Sheat	hed or	5-3-1 oc	purlins.		

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2OTHERS2x4 SPF No.3

REACTIONS. (size) 1=5-2-8, 3=5-2-8, 4=5-2-8

Max Horz 1=50(LC 13)

Max Uplift 1=-26(LC 16), 3=-32(LC 17)

Max Grav 1=157(LC 22), 3=157(LC 23), 4=166(LC 22)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

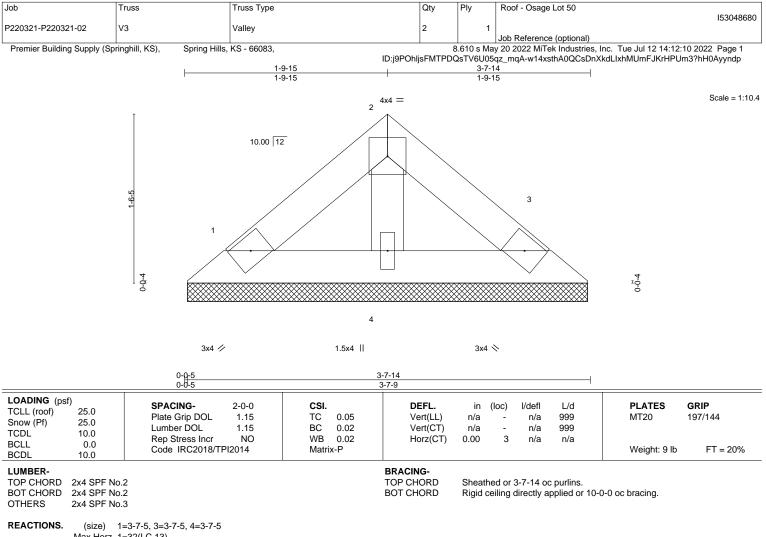
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





⁷⁾ N/A



Max Horz 1=32(LC 13)

Max Uplift 1=-17(LC 16), 3=-21(LC 17)

Max Grav 1=95(LC 22), 3=95(LC 23), 4=106(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

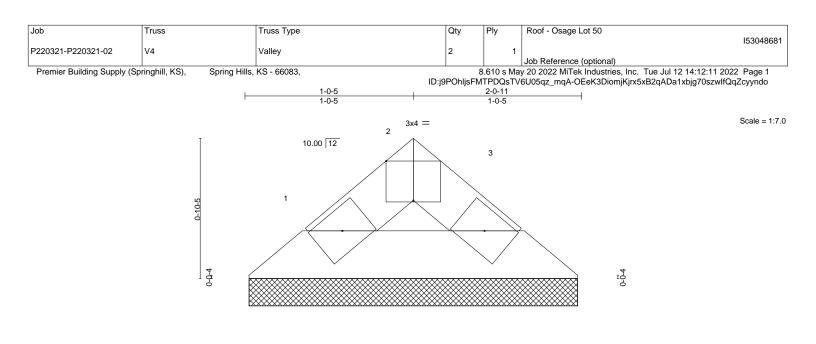
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) N/A

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

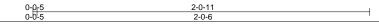






3x4 🥢

3x4 📎



LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0	0 0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 NO	CSI. TC BC WB	0.01 0.02 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 197/144
BCLL 0.0 BCDL 10.0		Code IRC2018/TF	PI2014	Matrix	x-P	. ,					Weight: 4 lb	FT = 20%

TOP CHORD

BOT CHORD

Sheathed or 2-0-11 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SPF No.2

BOT CHORD 2x4 SPF No.2

REACTIONS. (size) 1=2-0-1, 3=2-0-1

Max Horz 1=-14(LC 12) Max Uplift 1=-7(LC 16), 3=-7(LC 17)

Max Grav 1=60(LC 22), 3=60(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

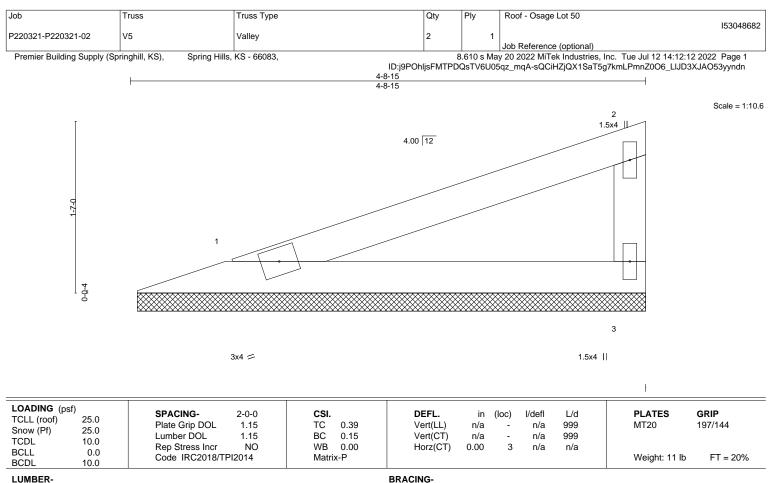


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⁷⁾ N/A



BOT CHORD

Sheathed or 4-8-15 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 2x4 SPF No.3 WFBS

REACTIONS. (size) 1=4-8-3, 3=4-8-3

Max Horz 1=57(LC 13)

Max Uplift 1=-29(LC 12), 3=-38(LC 16)

Max Grav 1=217(LC 22), 3=217(LC 22)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

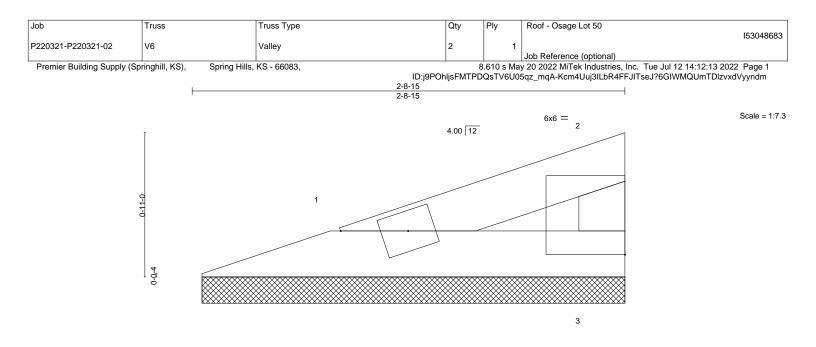
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) N/A

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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3x4 📁

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Plate Offsets ()	(,Y) [2:Edge,0)-1-13]										
LOADING (psf TCLL (roof) Snow (Pf) TCDL BCLL BCDL) 25.0 25.0 10.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TPI	2-0-0 1.15 1.15 NO 12014	CSI. TC BC WB Matri:	0.06 0.03 0.00 x-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 6 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.3				BRACING- TOP CHORD BOT CHORD	Sheathed or 2-8-15 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.							

REACTIONS. (size) 1=2-8-3, 3=2-8-3 Max Horz 1=26(LC 13) Max Uplift 1=-13(LC 12), 3=-17(LC 16) Max Grav 1=91(LC 22), 3=91(LC 22)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

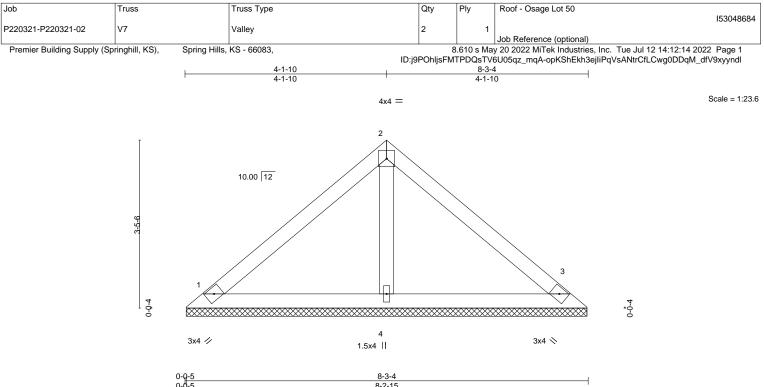
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) N/A
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







	0-0-5	8-2-15		
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 25.0 TCDL 10.0 BCLL 0.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNO	CSI. DEF TC 0.50 Verti BC 0.13 Verti WB 0.06 Horz	LL) n/a - n/a 999 CT) n/a - n/a 999	PLATES GRIP MT20 197/144
BCDL 10.0	Code IRC2018/TPI2014	Matrix-P BRACING		Weight: 24 lb FT = 20%

BOT CHORD

Sheathed or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 2x4 SPF No.3 OTHERS

REACTIONS. (size) 1=8-2-11, 3=8-2-11, 4=8-2-11

Max Horz 1=-84(LC 12) Max Uplift 1=-44(LC 16), 3=-54(LC 17)

Max Grav 1=286(LC 22), 3=286(LC 23), 4=291(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

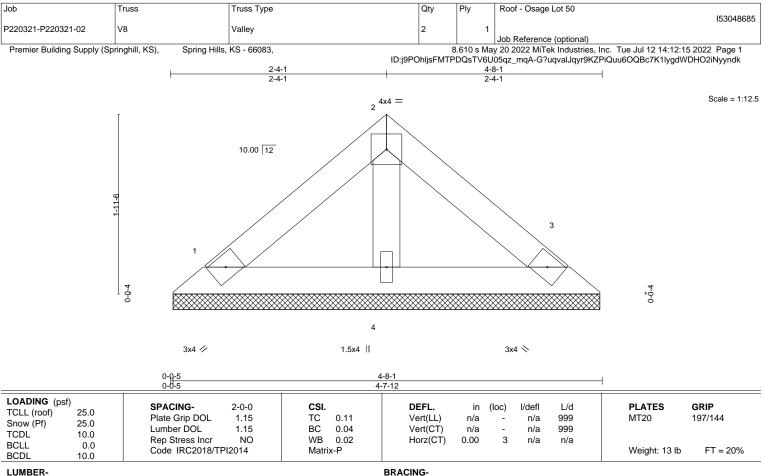
8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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⁷⁾ N/A



BOT CHORD

Sheathed or 4-8-1 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 2x4 SPF No.3 OTHERS

REACTIONS. (size) 1=4-7-7, 3=4-7-7, 4=4-7-7

Max Horz 1=-44(LC 12)

Max Uplift 1=-23(LC 16), 3=-28(LC 17)

Max Grav 1=134(LC 22), 3=134(LC 23), 4=144(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=0.96; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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⁷⁾ N/A

