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GENERAL NOTES

1. BUILDING AND DESIGN CODES:

A. 2018 INTERNATIONAL BUILDING CODE

B. ASCE 7-16: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES

C. ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES

D. ACI 318: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, LATEST EDITION

E. AISC 360: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, LATEST EDITION

F. AISI MANUAL OF STEEL CONSTRUCTION, LATEST EDITION

G. AISI 2007: SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS

H. AMERICAN WELDING SOCIETY, WELDING CODES

I. TMS 402/ACI 530/ASCE 6: BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, LATEST EDITION

J. NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION

2. OCCUPANCY CATEGORY

II

3. LIVE LOADS:

A. FLOOR

100 PSF

B. ROOF

20 PSF

4. WIND LOADS:

A. BASIC WIND SPEED (3 SECOND GUST)

115 MPH (ULTIMATE)

B. IMPORTANCE FACTOR, I

1.0

C. EXPOSURE CATEGORY

C

D. INTERNAL PRESSURE COEFFICIENT, GCpi

+/- 0.18

E. DESIGN WIND PRESSURES (COMPONENTS & CLADDING)

		SURFACE PRESSURE (PSF)		
		10 SF AREA	20 SF AREA	50 SF AREA
ROOF ZONES	1. INTERIOR	+16.0 / -46.8	+16.0 / -73.4	+16.0 / -73.4
	2. END	+16.0 / -44.1	+16.0 / -69.7	+16.0 / -69.7
	3. CORNER	+16.0 / -38.0	+16.0 / -61.2	+16.0 / -61.2
WALL ZONES	4. INTERIOR	+32.0 / -32.0	+32.0 / -32.0	+29.4 / -30.3
	5. END	+32.0 / -58.6	+32.0 / -58.6	+29.4 / -51.8

a. (+) VALUES INDICATE PRESSURES TOWARDS THE BUILDING. (-) VALUES INDICATE PRESSURES AWAY FROM THE BUILDING.

b. FOR SMALLER TRIBUTARY AREAS, LOADS WILL BE LARGER THAN SHOWN. CALCULATE PER CODE REQUIREMENTS.

c. WIDTH OF END ZONES

4.5 FT

F. ANALYSIS PROCEDURE

ENVELOPE PROCEDURE FOR LOW-RISE BUILDINGS

5. SNOW LOADS

A. GROUND SNOW LOAD, P_g

20 PSF

B. SNOW EXPOSURE FACTOR, C_e

1.0

C. IMPORTANCE FACTOR, I

1.0

D. THERMAL FACTOR, C_t

1.0

6. SEISMIC LOADS:

A. IMPORTANCE FACTOR, I

1.0

B. RISK CATEGORY

II

C. MAPPED SPECTRAL RESPONSE ACCELERATIONS

S_{0.2} = 0.101
S₁ = 0.069

D. SITE CLASS

C

E. DESIGN SPECTRAL RESPONSE ACCELERATIONS

S_{0.2} = 0.087
S_{0.1} = 0.069

F. SEISMIC DESIGN CATEGORY

B

G. BASIC SEISMIC FORCE RESISTING SYSTEM

N/S DIRECTION: STEEL ORDINARY MOMENT FRAME
E/W DIRECTION: PLYWOOD SHEAR WALLS

H. DESIGN BASE SHEAR

V = C_s x W

I. SEISMIC RESPONSE COEFFICIENT

N/S: C_s = 0.025
E/W: C_s = 0.011

J. RESPONSE MODIFICATION FACTOR

N/S: R = 3.5
E/W: R = 6.5

K. ANALYSIS PROCEDURE

EQUIVALENT LATERAL FORCE PROCEDURE

7. GENERAL REQUIREMENTS:

A. SPECIFICATIONS ARE PART OF THE CONSTRUCTION DOCUMENTS AND MUST BE USED IN CONJUNCTION WITH THE DRAWINGS. WHERE REQUIREMENTS INDICATED ON THE STRUCTURAL DRAWINGS DIFFER FROM THE SPECIFICATIONS, NOTIFY A/E.

B. VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO BEGINNING WORK OR FABRICATING MATERIALS. NOTIFY A/E OF DISCREPANCIES BEFORE PROCEEDING WITH ANY PHASE OF WORK.

C. VERIFY WITH OTHER DISCIPLINE DRAWINGS THE LOCATION OF CHASES, INSERTS, OPENINGS, SLEEVES, FINISHES, DEPRESSIONS, PADS, AND WALL OPENINGS.

D. DO NOT SCALE DRAWINGS FOR THE PURPOSE OF ESTABLISHING DIMENSIONS.

E. DETAILS LABELED "TYPICAL DETAILS" ON DRAWINGS APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SUCH DETAILS APPLY WHETHER OR NOT DETAILS ARE REFERENCED AT EACH LOCATION. NOTIFY A/E OF CONFLICTS REGARDING APPLICABILITY OF "TYPICAL DETAILS".

F. DO NOT LOAD THE SLAB-ON-GRADE OR SUPPORTED SLAB WITH ERECTION CRANES OR ERECTION EQUIPMENT. THE SLABS HAVE NOT BEEN DESIGNED FOR CRANE LOADS AND WILL REQUIRE AN INCREASE IN THICKNESS AND/OR REINFORCEMENT. SUBMIT FOR A/E REVIEW A PROPOSED CRANE SUPPORT PLAN FOR SLABS PRIOR TO COMMENCING WORK.

G. DO NOT STORE OR STACK CONSTRUCTION MATERIALS ON SUPPORTED SLABS, ELEVATED FLOORS, OR ROOFS IN EXCESS OF 80 PERCENT OF LIVE LOAD. GENERAL CONTRACTOR WILL BE RESPONSIBLE AND ENSURE THAT ALL SUB-CONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. AVOID IMPACT WHEN PLACING MATERIALS ON POURED OR ERECTED FLOORS OR ROOF.

H. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. UNLESS INDICATED OTHERWISE, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION. PROVIDE ALL MEASURES REQUIRED TO PROTECT THE STRUCTURE, WORKMEN, AND OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT ARE NOT LIMITED TO, BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR THE BUILDING, FORMS AND SCAFFOLDING, SHORING OF RETAINING WALLS AND OTHER TEMPORARY SUPPORTS AS REQUIRED.

I. PRINCIPAL OPENINGS THROUGH THE FRAMING ARE SHOWN ON DRAWINGS. EXAMINE THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR THE REQUIRED OPENINGS AND PROVIDE FOR REQUIRED OPENINGS WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT. VERIFY SIZE AND LOCATION OF ALL OPENINGS WITH THE MECHANICAL CONTRACTOR. BRING ANY DEVIATION FROM THE OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS TO THE ENGINEER'S ATTENTION FOR APPROVAL.

J. COORDINATE AND PROVIDE ALL MISCELLANEOUS FRAMING MEMBERS SHOWN ON THE ARCHITECTURAL DRAWINGS. THESE MEMBERS MAY NOT BE SHOWN ON THE STRUCTURAL DRAWINGS.

K. ARCHITECTURAL, MECHANICAL, AND ELECTRICAL COMPONENTS AND SYSTEMS SHALL BE DESIGNED AND CONSTRUCTED TO RESIST SEISMIC FORCES AS DETERMINED IN CHAPTER 13 OF ASCE 7.

 1. REFER TO THE GEOTECHNICAL REPORT AND SPECIFICATIONS FOR GENERAL REQUIREMENTS OF EARTHWORK, OVER EXCAVATION, SUBGRADE PREPARATION, FILL AND COMPACTION, WATERPROOFING AND OTHER PERTINENT REQUIREMENTS AND INFORMATION. 2. FOUNDATION DESIGNS AND SUBGRADE PREPARATION ARE BASED UPON THE RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT NUMBER 83225012 BY TERRAGON, DATED 09/02/2022. 3. FOOTING DESIGNS ARE BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF (FACTOR OF SAFETY = 3). THE ALLOWABLE SOIL BEARING PRESSURE MAY BE INCREASED BY ONE-THIRD WHEN CONSIDERING TOTAL LOADS, INCLUDING LOADS OF SHORT DURATION SUCH AS WIND FORCES. FOOTINGS SHALL BEAR IN SPECIFIED BEARING MATERIAL AS NOTED IN THE GEOTECHNICAL REPORT. 4. CONTRACTOR AND TESTING LABORATORY REPRESENTATIVE SHALL READ THE GEOTECHNICAL REPORT AND BECOME THOROUGHLY FAMILIAR WITH SITE AND SUBGRADE INFORMATION. CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING EXACT QUANTITIES OF CUT AND FILL FOR ESTIMATING AND CONSTRUCTION. SUBGRADE SHALL BE PREPARED AS NOTED IN THE GEOTECHNICAL REPORT. 5. ARRANGE FOR OWNER'S INDEPENDENT TESTING AGENCY TO MONITOR CUT AND FILL OPERATIONS, AND PERFORM FIELD DENSITY AND MOISTURE CONTENT TESTS TO VERIFY COMPACTION AND APPROVE FOOTING SUBGRADE PRIOR TO PLACING CONCRETE. CUT AND FILL OPERATIONS SHALL BE INSPECTED AND APPROVED BY A GEOTECHNICAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. 6. A QUALIFIED AND REGISTERED GEOTECHNICAL ENGINEER, LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED AND WORKING FOR THE TESTING LABORATORY, SHALL DETERMINE CONFORMANCE OF THE FOUNDATION BEARING STRATA WITH THE FOUNDATION DESIGN CRITERIA ABOVE, AND ALL OTHER CONTRACT DOCUMENTS, AND VERIFY SIZE, REINFORCING STEEL, THICKNESS, EMBEDMENT DEPTH, AND REMOVAL OF CUT MATERIAL. TESTING LABORATORY SHALL NOTIFY CONTRACTOR AND A/E OF ANY CONDITIONS NOT IN ACCORDANCE WITH FOUNDATION DESIGN CRITERIA OR CONTRACT DOCUMENTS. 7. USE ONLY STRUCTURAL FILL MATERIAL AS NOTED IN THE GEOTECHNICAL REPORT FOR FILL BELOW THE BUILDING. EXTEND FILL AT LEAST FIVE FEET BEYOND THE BUILDING PERIMETER ON ALL SIDES. REFER TO THE GEOTECHNICAL REPORT FOR THE DEPTH OF THE STRUCTURAL FILL MATERIAL AND COMPACTION REQUIREMENTS. 8. MAINTAIN SUBGRADE AND FILL MOISTURE CONTENT UNTIL FOUNDATIONS ARE PLACED. 9. DO NOT PLACE GRADE BEAMS, WALLS, FOOTINGS OR SLABS AGAINST SUBGRADE CONTAINING FREE WATER, FROST, OR ICE. 10. MAINTAIN PROPER SITE DRAINAGE DURING CONSTRUCTION TO ENSURE SURFACE RUNOFF AWAY FROM STRUCTURES AND TO PREVENT PONDING OF SURFACE RUNOFF NEAR THE STRUCTURES. 11. KEEP OPEN EXCAVATIONS AROUND BUILDING DRY. BACKFILL AGAINST FOUNDATIONS AND GRADE BEAMS AS SOON AS PRACTICAL. PUMP WATER OUT OF OPEN EXCAVATIONS IF FLOODING OCCURS PRIOR TO BACKFILLING. 12. PROTECT PIPES AND CONDUITS RUNNING THRU WALLS AND SLABS WITH 1/2 INCH EXPANSION MATERIAL. LOWER CONTINUOUS FOOTINGS AND GRADE BEAMS PERPENDICULAR TO PIPE RUNS. TO ALLOW PIPES TO PASS ABOVE THE FOOTINGS OR THOUGH THE GRADE BEAMS. ALTERNATIVELY, PROVIDE A CONCRETE JACKET IF PIPES ARE LOW ENOUGH TO BE PLACED BELOW THE FOOTINGS AND GRADE BEAMS. LOWER FOOTINGS AND GRADE BEAMS PARALLEL TO PIPE RUNS TO AVOID SURCHARGE ONTO ADJACENT TRENCH EXCAVATIONS. 13. PLACE FOUNDATIONS WITHIN 8 HOURS AFTER EXCAVATION. DO NOT LEAVE EXCAVATION OPEN OVERNIGHT. 14. AVOID DAMAGING EXISTING UNDERGROUND UTILITIES SUCH AS WATER MAINS, SANITARY SEWERS, BURIED CABLES, ETC., WHICH MIGHT EXTEND ACROSS OR ADJOINING THE SITE. 15. REFER TO CIVIL DRAWINGS FOR LIMITS OF EXCAVATIONS. | 1. ANCHORS INSTALLED IN HARDENED CONCRETE TO BE USED ONLY WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. CONTRACTOR SHALL OBTAIN APPROVAL FROM THE A/E PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. PLACE POST-INSTALLED ANCHORS SUCH THAT THEY AVOID CONFLICTING WITH EXISTING REBAR. WHERE INDICATED, PROVIDE THE FOLLOWING POST-INSTALLED ANCHOR: A. EPOXY ADHESIVE ANCHORS: SIMPSON SET-3G FOR ANCHORAGE TO CONCRETE a. ALLOWABLE ANCHOR SUBSTITUTIONS SHALL BE SUBMITTED TO THE A/E WITH INFORMATION DEMONSTRATING THAT THE ANCHOR SUBSTITUTION PROVIDES EQUAL OR GREATER PERFORMANCE VALUES. 2. INSTALL ANCHORS IN ACCORDANCE WITH THE CURRENT ICBO REPORT FOR THE ANCHORS AND THE MANUFACTURER'S RECOMMENDATIONS. ALL ANCHORS SHALL BE INSTALLED BY A MANUFACTURER CERTIFIED INSTALLER. 3. INSTALL ANCHORS PERPENDICULAR TO THE FACE OF THE CONCRETE. DEVIATION FROM PERPENDICULAR GREATER THAN 10 DEGREES IS UNACCEPTABLE. 4. CREATE A TEMPLATE AT EACH ANCHOR CONNECTION LOCATION PRIOR TO FABRICATING HOLES IN CONNECTION PLATES. MAKE TEMPLATE BY LOCATING EXISTING REBAR WITH THE HELP OF A PACHOMETER. REPOSITION ANCHORS A MAXIMUM OF 1 1/2 INCHES AS REQUIRED TO AVOID CONFLICTS WITH EXISTING REINFORCEMENT. 5. FILL ALL ABANDONED HOLES WITH EPOXY GROUT. 6. PROVIDE HOLES IN CONNECTION PLATES NO MORE THAN 1/16 OF AN INCH LARGER THAN THE ANCHOR DIAMETER. IF LARGER HOLES ARE NEEDED FOR ERECTION PURPOSES, PROVIDE PLATE WASHERS WELDED TO THE CONNECTION PLATE TO TRANSFER THE BOLT LOAD. 7. CLEAN DRILLED HOLES FREE OF DEBRIS/DUST AND INSPECT PRIOR TO APPLYING EPOXY AND INSTALLING ANCHORS. | 1. WOOD FRAMING SHALL MEET THE FOLLOWING MINIMUM STRESS PROPERTIES UNLESS NOTED OTHERWISE: | | MEMBER SIZE | BENDING F _x (PSI) | TENSION PARALLEL TO GRAIN F _t (PSI) | SHEAR PARALLEL TO GRAIN F _v (PSI) | COMPRESSION PERPENDICULAR TO GRAIN F _c (PSI) | COMPRESSION PARALLEL TO GRAIN F _c (PSI) | ELASTIC MODULUS E (PSI) | |----------------------------|-------------|------------------------------|--|--|---|--|-------------------------| | SOUTHERN PINE #2 OR BETTER | 2x6 | 1,000 | 600 | 175 | 565 | 1,400 | 1,400,000 | | | 2x10 | 800 | 475 | 175 | 565 | 1,300 | 1,400,000 | | | 2x12 | 750 | 450 | 175 | 565 | 1,250 | 1,400,000 | | ROSBORO GLULAM X-BEAM | SEE DWGS | 2,400 | - | 265 | 650 | - | 1,800,000 | 2. PROVIDE SIMPSON STRONG-TIE CONNECTORS OR EQUIVALENT FOR WOOD FRAMING CONNECTIONS TO SUPPORTING MEMBERS. USE STRONG-TIE CONNECTORS AND NAILS OF APPROPRIATE SIZE AND CAPACITY FOR THE SUPPORTED MEMBER AND INSTALL ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS. 3. ALL ROOF AND EXTERIOR WALL SHEATHING SHALL BE APA RATED CDX PLYWOOD WITH EXTERIOR GLUE (EXPOSURE 1) OR ORIENTED STRAND BOARD (OSB) WITH EXTERIOR GLUE (EXPOSURE 1), AND SHALL BEAR THE STAMP OF AN APPROVED TESTING AGENCY. 4. INSTALL ROOF SHEATHING WITH THE LONG DIMENSION OF THE PANEL PERPENDICULAR TO SUPPORTS UNLESS NOTED OTHERWISE, AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. STAGGER END JOINTS. 5. ALL ROOF SHEATHING SHALL BE APA RATED EXPOSURE 1 CDX PLYWOOD SHEATHING WITH A MINIMUM THICKNESS OF 5/8 INCH, DOC PS-1 OR PS-2. WITH A SPAN RATING OF AT LEAST 32/16 NAILED WITH 10d GALVANIZED COMMON NAILS AT 4 INCHES ON CENTER AT PANEL EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS. 10d NAILS SHALL HAVE A MINIMUM 0.148 INCH DIAMETER AND 1 1/2 INCH MINIMUM PENETRATION INTO SUPPORTING FRAMING. 6. OUTSIDE OF EXTERIOR WALLS SHALL BE SHEATHED WITH APA RATED EXPOSURE 1 OSB OR CDX PLYWOOD SHEATHING WITH A MINIMUM THICKNESS OF 5/8 INCH. DOC PS-1 OR PS-2. REFER TO S5.4 FOR FASTENING REQUIREMENTS OF SHEAR WALLS. IDENTIFIED ON S1.1. WALLS NOT IDENTIFIED AS SHEAR WALLS SHALL BE FASTENED TO WALL STUDS WITH 10d GALVANIZED COMMON NAILS AT 6 INCHES ON CENTER AT EDGES AND 12 INCHES IN CENTER AT INTERMEDIATE SUPPORTS. PROVIDE BLOCKING AT UNSUPPORTED PANEL EDGES. 10d NAILS SHALL HAVE A MINIMUM OF 0.148 INCH DIAMETER AND 1 1/2 INCH MINIMUM PENETRATION INTO SUPPORTING FRAMING. 7. INSTALL JOISTS, RAFTERS, HEADERS AND BEAMS "CROWN UP." 8. ALL JOISTS SHALL HAVE DIAGONAL BRIDGING OR FULL DEPTH BLOCKING AT 8 FEET ON CENTER MAXIMUM ALONG THE SPAN AND AT SUPPORTING BEAMS OR WALLS. 9. CUTTING, BORING OR NOTCHING OF FRAMING MEMBERS, IF REQUIRED, SHALL CONFORM TO THE LIMITATIONS PRESCRIBED BY THE IBC AND MAY BE DISALLOWED FOR SOME FRAMING MEMBERS BY THE A/E. 10. ALL WOOD IN CONTACT WITH CONCRETE AND EXTERIOR MASONRY SHALL BE PRESSURE TREATED. 11. REFER TO THE IBC FOR MINIMUM FASTENING CRITERIA. ALL NAILS TO BE COMMON WIRE SIZE. NAILING SHALL COMPLY WITH REQUIREMENTS OF NAILING SCHEDULE UNLESS NOTED OTHERWISE. 12. MOISTURE CONTENT OF ALL WOOD MEMBERS SHALL NOT EXCEED 19%. | 1. ALL TRUSSES CALLED OUT IN THE DRAWINGS SHALL BE PRE-ENGINEERED, MANUFACTURED TRUSSES. TRUSSES SHALL CONFORM TO THE SPACING, DIMENSIONS AND LAYOUTS CALLED OUT IN THESE NOTES AND ON THE PLANS AND SHALL BE DESIGNED FOR SPECIFIED LOADINGS. 2. MAXIMUM LIVE LOAD DEFLECTION FOR TRUSSES NOT TO EXCEED L/360. MAXIMUM TOTAL LOAD DEFLECTION NOT TO EXCEED L/240, WHICHEVER IS GREATER. 3. TRUSSES AND CONNECTOR PLATES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST REVISION OF THE TRUSS PLATE INSTITUTE SPECIFICATIONS. TRUSS MANUFACTURER SHALL DESIGN THE TRUSS TO WALL CONNECTIONS, U.N.O. ON THE DETAILS. PROVIDE FRAMING ANCHORS AND/OR TRUSS HANGERS AS REQUIRED AND AS SHOWN ON THE DRAWINGS. 4. PROVIDE TRUSS SHOP DRAWINGS, INSTALLATION DRAWINGS, AND CALCULATIONS PREPARED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH ALL APPLICABLE CODES, ORDINANCES, ETC. 5. CONTINUOUSLY BRACE AND SUPPORT TRUSSES DURING UNLOADING TO PREVENT EXCESSIVE STRESS ON THE JOINTS. DO NOT PERMIT TRUSSES TO DROP, SAG, OR BE SUPPORTED IN A DIRECTION PERPENDICULAR TO THE TRUSS PLANE. INSTALL TRUSSES IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS, INCLUDING PROPER HANDLING, SAFETY PRECAUTIONS, TEMPORARY BRACING DURING ERECTION AND ALL OTHER SAFEGUARDS. 6. INSTALL ALL PERMANENT CHORD BRACING REQUIRED BY TRUSS SHOP DRAWINGS (TYPICALLY 3 ROWS OF 2x4 - FULL LENGTH OF BUILDING). 7. INSPECT ALL TRUSSES AFTER INSTALLATION FOR DAMAGE. NOTIFY A/E IMMEDIATELY OF DAMAGED TRUSSES. REMOVE AND REPLACE ALL DAMAGED TRUSSES. 8. TRUSSES ARE A DEFERRED SUBMITTAL ITEM AND CONTRACTOR IS REQUIRED TO COMPLETE THE FOLLOWING: A. FIRST, THE TRUSS PLANS AND CALCULATIONS, SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED, SHALL BE SUBMITTED TO THE A/E FOR REVIEW BEFORE SUBMITTING TO THE BUILDING DEPARTMENT. SECOND, THE SHOP DRAWINGS SHALL BE SUBMITTED WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED BY THE A/E AND HAVE BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE BUILDING DESIGN. THE TRUSS DESIGN SHALL BE APPROVED BY THE BUILDING DEPARTMENT BEFORE THE TRUSSES ARE FABRICATED. GENERAL CONTRACTOR SHOULD PLAN FOR REVIEW TIME BY BOTH THE A/E AND BUILDING OFFICIAL IN THE CONSTRUCTION SCHEDULE. | 1. METAL STUDS USED AT THE COOK LINE WALL SHALL HAVE A YIELD STRENGTH OF 33KSI. 2. STUD SIZE, GAUGE, AND CONNECTION SHALL BE PER DETAILS. 3. TRACK SIZE, GAUGE, AND CONNECTION SHALL BE PER DETAILS. | 1. MASONRY (BRICK) VENEER SHALL BE ATTACHED WITH ADJUSTABLE WIRE TIES SPACED AT 16" O.C. VERTICALLY AND 16" O.C. HORIZONTALLY. ADDITIONAL TIES ALONG ALL OPENINGS GREATER THAN 16" ARE REQUIRED TO BE LOCATED WITHIN 12" OF OPENING AND SPACED AT 32" O.C. MAX. AROUND OPENING PERIMETER. 2. BRICK TIES SHALL BE MANUFACTURED BY HOHMANN & BARNARD, INC. MODEL NUMBER HB-213 OR APPROVED EQUAL. || ms consultants, inc. engineers, architects, planners 2221 Schrock Road Columbus, Ohio 43229 p 614.898.7100 f 614.898.7570 www.msconsultants.com | | PROTOTYPE: PT20M LEE'S SUMMIT, MO HOLLYWOOD ST LEE'S SUMMIT, MO 64082 | | NOTICE: THIS ARCHITECTURAL AND ENGINEERING DRAWING IS GIVEN IN CONFIDENCE AND SHALL BE USED ONLY PURSUANT TO THE AGREEMENT WITH THE ARCHITECT. NO OTHER USE, DISSEMINATION, OR DUPLICATION MAY BE MADE WITHOUT PRIOR WRITTEN CONSENT OF THE ARCHITECT. ALL COMMON LAW RIGHTS OF COPYRIGHT AND OTHERWISE ARE HEREBY SPECIFICALLY RESERVED. | | PROFESSIONAL OF RECORD: CRAIG E. METZGER NO. 2019031268 EXP DATE: 12/31/23 | | | REV | DESCRIPTION | DATE | |-----|-----------------------------|----------| | 1 | Building Permit Resubmittal | 09/15/22 | | | | | | | | | | | | | Project No.: 40497-21 Client Project No.: Drawing Title: Date: 06.29.2022 Designed: DMS Drawn: CLS Checked: CEM Phase: PERMIT SET Drawing No.: S0.2 | |