

- MINIMUM COMPRESSIVE STRENGTH (f'c) AT THE END OF 28 DAYS SHALL BE AS FOLLOWS:
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| A. FOOTINGS (GRADE BEAMS) | 3000 PSI U.N.O. ON PLAN |
| B. FOUNDATION WALLS | 3000 PSI |
| C. SLABS-ON-GRADE | 4000 PSI |
| D. CONCRETE WALL PANELS (MINIMUM STRENGTH) | 4000 PSI |
- MAXIMUM WATER/CEMENT RATIO = 0.48 TO 0.50 FOR FOOTINGS AND 0.52 FOR SLABS-ON-GRADE AND PRECAST WALL PANELS
- SUMP LIMITS = 4" + 1"
- CONCRETE SHALL BE NORMAL WEIGHT (145 PCF), UNLESS NOTED OTHERWISE.
- CEMENTITIOUS MATERIALS CONTENT SHALL NOT BE LESS THAN 520 POUNDS PER CUBIC YARD. USE OF ANY FLY ASH IN FLOOR SLAB MIXES SHALL BE NO MORE THAN 20%.
2. AIR-ENTRAINED IS NOT REQUIRED FOR STRUCTURAL CONCRETE.
3. AGGREGATES SHALL COMPLY WITH ASTM C 33 AND SHALL BE FREE OF DELETERIOUS MATTER AND SHALL BE MADE OF COARSE LESTHONE OR GRANITE AGGREGATES.
4. MATERIALS OR ADMIXTURES SHALL NOT CONTAIN ANY CALCIUM CHLORIDE. IF ADMIXTURES ARE UTILIZED, THEY SHALL BE COMPATIBLE WITH OTHER ADMIXTURES AND MUST NOT CONTRIBUTE WATER-SOLUBLE CHLORIDE IONS EXCEEDING THOSE PERMITTED IN HARDENED CONCRETE.
5. REINFORCING STEEL SHALL MEET THE FOLLOWING:
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| A. DEFORMED BARS | ASTM A615, GRADE 60 |
| B. WELDABLE DEFORMED BARS | ASTM A706, GRADE 60 |
| C. WELDED WIRE FABRIC | ASTM A185 |
6. WHERE DOWELS ARE INDICATED BUT NOT SIZED, PROVIDE DOWELS THAT MATCH SIZE AND LOCATION OF MAIN REINFORCING STEEL AND LAP SPICE WITH THE MAIN REINFORCING STEEL. REINFORCING BARS SHALL BE SPLICED AS NOTED IN THE REINFORCING LAP SCHEDULE.
7. REFER TO ACI 318 LATEST EDITION FOR CONCRETE COVER, ACI 315 LATEST EDITION FOR DETAILING, FABRICATION, PLACEMENT AND SUPPORT PRACTICES, ACI 347 FOR FORMWORK, ACI 305 FOR HOT WEATHER CONCRETING, ACI 306 FOR COLD WEATHER CONCRETING, AND ACI 301 LATEST EDITION FOR STANDARD PRACTICE FOR MIXING AND PLACING CONCRETE. PROVIDE CONCRETE COVER DIMENSIONS IN SHOP DRAWINGS FOR STRUCTURAL ENGINEER REVIEW.
8. "C.J." INDICATES SAW CUT CONTRACTION JOINT OR DOWELED CONSTRUCTION JOINT IN SLAB-ON-GRADE. SLAB POURS SHALL BE SEPARATED BY A DOWELED CONSTRUCTION JOINT. CONTRACTION/CONSTRUCTION JOINTS SHALL BE LOCATED AS SHOWN ON PLANS OR AS DIRECTED BY THE STRUCTURAL ENGINEER.
9. PROVIDE CORNER BARS THAT MATCH CONTINUOUS REINFORCEMENT SIZE AND QUANTITY AT INTERSECTIONS AND CORNERS OF FOUNDATIONS.
10. REINFORCING BAR SUPPORTS SHALL BE BOLSTERS, CHAIRS, SPACERS AND OTHER DEVICES TO HOLD REINFORCING BARS AND WELDED WIRE REINFORCEMENT IN PLACE. MANUFACTURE BAR SUPPORTS FROM STEEL, PLASTIC OR PRECAST CONCRETE ACCORDING TO CRSI'S "MANUAL OF STANDARD PRACTICE" OF GREATER COMPRESSIVE STRENGTH THAN THE CONCRETE PLACED IN.
11. FORM-FACING PANELS THAT WILL BE EXPOSED TO VIEW SHALL BE CONSTRUCTED TO MINIMIZE THE NUMBER OF JOINTS AND SHALL BE MADE OF PLYWOOD, METAL OR OTHER APPROVED PANEL MATERIAL. PLYWOOD MUST COMPLY WITH DOC PS 1 AND BE CLASS 1 OR BETTER.
12. CHAMFER EXTERIOR CORNERS AND EDGES OF PERMANENTLY EXPOSED CONCRETE.
13. THE CONCRETE SLABS SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED FOR THE FINISHED STRUCTURE AND HAVE NOT BEEN DESIGNED FOR MEANS AND METHODS OF CONSTRUCTION, INCLUDING BUT NOT LIMITED TO, FORK LIFTS, MAN LIFTS, AND OTHER VEHICULAR TRAFFIC.
14. A VAPOR RETARDER NOT LESS THAN 10 MILS THICK SHALL BE INSTALLED ONLY AT AREAS NOTATED ON THE CONSTRUCTION DOCUMENTS. THE RETARDER SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATION WITH JOINTS USING THE MANUFACTURER'S RECOMMENDED ADHESIVE OR PRESSURE SENSITIVE JOINT TAPE AND INCLUDING THE MANUFACTURER'S PROPRIETARY PENETRATION FLASHING FOR ALL THROUGH-SLAB PENETRATIONS. LAP VAPOR RETARDER JOINTS 6 INCHES MINIMUM.
15. CONCRETE SLABS-ON-GRADE SHALL BE CONSTRUCTED WITH A HARD TROWEL FINISH AND BE FINISHED ACCORDING TO ASTM E 1155 TO ACHIEVE THE MINIMUM TOLERANCES BELOW:
- OVERALL VALUES: FF = 50 FL = 35
LOCAL VALUES: FF = 25 FL = 20
16. THE CONCRETE SLAB-ON-GRADE SHALL BE CURED WITH AN APPROVED CURING MATERIAL THAT HAS BEEN SUBMITTED AND APPROVED BY THE ARCHITECT AND ENGINEER OF RECORD. THE FLOOR SHALL BE CURED WITH ONE COAT OF HARDENER/DENSIFIER (ASHFORD FORMULA SEALER OR APPROVED ALTERNATE).
17. CONTRACTOR SHALL VERIFY ALL DIMENSIONS, OPENINGS, BLOCKOUTS, RECESSES, ELEVATIONS, ANCHOR RODS AND EMBED LOCATIONS PRIOR TO CONCRETE PLACEMENT. THE CONTRACTOR SHALL VERIFY WITH ARCHITECTURAL, STRUCTURAL AND MEP DRAWINGS FOR LOCATIONS OF REQUIRED COORDINATION ITEMS. CONTRACTOR SHALL CONTACT THE ARCHITECT OR ENGINEER IF AN ERROR OR OMISSION OCCURS AFTER CONCRETE PLACEMENT.
18. ANCHOR BOLTS AND EMBED PLATES SHALL BE TIED INTO THE REBAR CAGE AND HELD IN PLACE WITH A RIGID TEMPLATE TO PREVENT MOVEMENT DURING CONCRETE PLACEMENT.
19. NON-SHRINK GROUT SHALL BE PRE-MIXED, NON-SHRINKING WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI IN 28 DAYS CONFORMING TO USE SPECIFICATIONS NO. CRD-C621.

- | STRUCTURAL STEEL SHALL MEET THE FOLLOWING MINIMUM YIELD STRESS (Fy), UNLESS NOTED OTHERWISE: | | |
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| | YIELD | A992 SPECIFICATION |
| A. W, WT SHAPES: | 50 KSI | A992 |
| B. BARS, PLATES, CHANNELS, ANGLES: | 36 KSI | A36 |
| C. SQUARE, RECTANGULAR HSS: | 50 KSI | A500, GRADE C |
| D. ANCHOR RODS: | 36 KSI OR 55 KSI | F1554 |
| E. ALL-THREAD RODS: | 36 KSI | A36 |
| F. HEADED STUD ANCHORS: | 65 KSI TENSILE STRESS | A108, GRADES 1010-1020 |
| 2. ALL STRUCTURAL STEEL SHALL ADHERE TO THE DETAILING, FABRICATION AND ERECTION REQUIREMENTS OF THE LATEST EDITIONS OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS AND THE AISC CODE OF PRACTICE. | | |
| 3. BOLTS FOR STEEL BEAM AND COLUMN CONNECTIONS SHALL BE 3/4-1/2 INCH DIAMETER ASTM A325-N HIGH-STRENGTH BOLTS UNLESS NOTED OTHERWISE. ALL BOLTED CONNECTIONS ARE BEARING TYPE AND SHALL BE SNUG TIGHTENED UNLESS NOTED OTHERWISE. FOR PRETENSIONED OR SLIP-CRITICAL JOINTS, THE METHOD OF INSTALLATION SHALL BE TURN-OF-NUT WITH MATCH MARKING, TWIST-OFF-TYPE TENSION CONTROL BOLT ASSEMBLIES (ASTM F1852), OR DIRECT TENSION INDICATORS (ASTM F959). | | |
| 4. WELDING SHALL MEET ANSI / AWS D1.1, STRUCTURAL WELDING CODE LATEST REVISION. ELECTRODES SHALL BE E70XX, LOW HYDROGEN. ALL STRUCTURAL STEEL WELDS SHALL BE PERFORMED BY A AWS CERTIFIED WELDER. | | |
| 5. WELDS NOT SPECIFICALLY SIZED ON THE STRUCTURAL DRAWINGS SHALL BE THE MINIMUM SIZE PER THE LATEST AWS D1.1. | | |
| 6. PROVIDE DOUBLE NUTS AND DOUBLE WASHERS FOR STEEL COLUMN ANCHOR BOLTS TO ALLOW FOR ADJUSTMENT IN BASE PLATE ALIGNMENT. PROVIDE 1 1/2 INCH NON-SHRINK GROUT UNDER BASE PLATE AFTER ERECTION. USE 2 1/2 INCHES NON-SHRINK GROUT WHEN COLUMN ANCHOR BOLTS ARE 1 1/4 INCH DIAMETER OR LARGER. NON-SHRINK GROUT SHALL BE NON-METALLIC WITH A MINIMUM COMPRESSIVE STRENGTH OF 5,000 PSI AT 28 DAYS. | | |
| 7. SHEAR CONNECTORS SHALL BE A CARBON STEEL HEADED STUD TYPE ASTM A108 GRADES 1010 THRU 1020, AWS D1.1, TYPE B WITH ARC SHIELDS. | | |
| 8. ALL CONNECTIONS ON THE STRUCTURAL DRAWINGS, UNLESS NOTED OTHERWISE, SHALL BE DESIGNED AND DETAILED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED, EMPLOYED OR RETAINED BY THE STEEL FABRICATOR. THE DESIGN AND DETAILING SHALL COMPLY WITH ALL APPLICABLE CODES AND SPECIFICATION SECTIONS. | | |
| 9. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR INCLUDING THE COSTS FOR ALL MISCELLANEOUS STEEL IN THEIR BID REGARDLESS OF WHETHER THOSE ITEMS ARE INDICATED ON THE STRUCTURAL DRAWINGS. THESE COSTS SHALL INCLUDE BUT ARE NOT LIMITED TO MISCELLANEOUS STEEL ITEMS SHOWN ON ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS SUCH AS SHELF ANGLES, GLAZING SUPPORTS AND LINTELS. | | |
| 10. LEDGER ANGLES AND LINTELS IN EXTERIOR WALL SYSTEMS SHALL BE HOT DIPPED GALVANIZED PER ASTM A123. | | |
| 11. ALL STRUCTURAL STEEL SHALL HAVE A COAT OF LIGHT GRAY PAINT TO PROVIDE PROTECTION AND GOOD APPEARANCE. | | |

- STEEL JOISTS SHALL BE AS INDICATED ON THE PLANS AND SHALL BE IN CONFORMANCE WITH THE LATEST SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI) AND MEET THE FOLLOWING:
- A. JOISTS SHALL BE DESIGNED FOR THE UNIFORM LOAD CAPACITY (AS SPECIFIED IN THE SJI STANDARD LOAD TABLES) IN ADDITION TO THE CONCENTRATED LOADS SHOWN ON PLANS AND DETAILS.
 - B. JOISTS THAT SUPPORT CONCENTRATED LOADS SHALL HAVE THEIR CHORDS DESIGNED TO WITHSTAND ALL BENDING STRESSES, OR THE LOADS SHALL OCCUR WITHIN 3 INCHES OF JOIST PANEL POINTS, OR THE JOIST SHALL BE REINFORCED PER THE "JOIST REINFORCING DETAIL" SHOWN HEREIN. CONCENTRATED LOADS SHALL BE CENTERED ON JOISTS AND NOT ATTACHED TO THE EDGE OF CHORD ANGLES.
 - C. JOISTS SHALL RESIST THE NET UPLIFT PRESSURE AS INDICATED ON THE DETAILS 7 & 8/S4.1. THIS PRESSURE SHALL ACT ALONE. AN ALLOWABLE STRESS INCREASE IS NOT PERMITTED.
 - D. FOR ALL MEMBERS THAT REQUIRE SPECIFIC ORIENTATION, PROVIDE TAG AT ONE END AND DEFINE LOCATION OF TAGGED END ON ERECTION DRAWINGS.
 - E. JOIST MANUFACTURER SHALL DETERMINE THE SEAT DEPTH AND WIDTH OF BEARING AND COORDINATE THE SAME WITH THE STEEL FABRICATOR. THE FOLLOWING SEAT DEPTHS ARE ASSUMED ON THE DRAWINGS: 2 1/2 INCHES FOR K-SERIES JOISTS, 5 INCHES FOR LH SERIES JOISTS).
 - F. JOISTS SHALL BE FABRICATED TO PROVIDE OPENINGS FOR DUCTS AS SHOWN IN THE REQUIRED OPENING IN JOIST DETAIL.
2. K-SERIES AND LH-SERIES JOISTS SHALL BE WELDED TO SUPPORTING STEEL WITH MINIMUM 1/8 INCH FILLET WELDS 2 INCHES LONG EACH SIDE OR WITH TWO 1/2 INCH DIAMETER ASTM A307 BOLTS OR THE EQUIVALENT, UNLESS NOTED OTHERWISE. WHEN NEAR OR AT A COLUMN, BOLT JOIST TO SUPPORTING STEEL IN CONFORMANCE WITH OSHA.
 3. JOIST BRIDGING AND ERECTION STABILITY SHALL BE PROVIDED IN ACCORDANCE WITH THE OCCUPATIONAL SAFETY AND HAZARD ADMINISTRATION (OSHA) AND THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI).
 4. JOIST RTU LOADS ARE PROVIDED ON THE ROOF FRAMING PLAN, REFERENCE PLANS AND DETAILS FOR LOAD LOCATIONS, VALUES AND SUPPORT FRAMING.
 5. JOIST MANUFACTURER SHALL DESIGN THE COMPRESSION CHORD OF ALL JOISTS SUPPORTING ROOF TOP UNITS, SKY LIGHTS, AND OTHER STRUCTURES FOR AN UNBRACED LENGTH APPLICABLE TO THE CONDITIONS AT THE PROJECT WHERE THE UNBRACED LENGTH IS GREATER THAN THE SJI MAXIMUM. (REFERENCE ARCHITECTURAL AND MECHANICAL DRAWINGS)
 6. DESIGN JOISTS FOR INTERNAL ROOF DRAINLINE AND FIRE SPRINKLER LINE LOCATIONS, IF REQUIRED. ADD 50 PLF FOR 8 INCH DIAMETER AND SMALLER, ADD 75 PLF FOR 10 INCH DIAMETER, ADD 102 PLF FOR 12 INCH DIAMETER, ADD 122 PLF FOR 14 INCH DIAMETER, ADD 200 PLF FOR 18 INCH DIAMETER. REFERENCE MECHANICAL DRAWINGS FOR EXACT LOCATION. CONTRACTOR SHALL OBTAIN FIRE LINE LOCATIONS AND SIZES PRIOR TO SUBMITTAL OF JOIST SHOP DRAWINGS.
 7. JOIST DESIGNS SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED, EMPLOYED OR RETAINED BY THE JOIST MANUFACTURER.
 8. SHOP DRAWING SHALL BE REVIEWED BY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD PRIOR TO JOIST FABRICATION.
 9. PROVIDE JOISTS CAPABLE OF WITH STANDING DESIGN LOADS INDICATED WITH LIVE LOAD DEFLECTIONS NO GREATER THAN L/240 OF THE SPAN.
 10. JOISTS SHALL BE CAMBERED ACCORDING TO SJI'S "SPECIFICATIONS". JOIST AND JOIST GIRDERS SHALL BE SHOP PRIMED WITH MANUFACTURER'S STANDARD SHOP PRIMER.
- STEEL DECK
1. ROOF DECK
- A. ROOF DECK SHALL BE GALVANIZED TYPE "B". DEPTH SHALL BE AS SHOWN ON DRAWINGS. ROOF DECK SHALL BE BOTTOM PRIMED WHITE.
 - B. ROOF DECK IS REQUIRED TO ACT AS A DIAPHRAGM. CONNECTIONS SHALL BE IN ACCORDANCE WITH STEEL DECK INSTITUTE SPECIFICATIONS. REFER TO THE ROOF DIAPHRAGM CONNECTION DIAGRAM FOR ATTACHMENT.
 - C. DECKING SHALL BE CONTINUOUS OVER A MINIMUM OF (3) SPANS UNLESS NOTED OTHERWISE.
 - D. NO HANGING LOADS SHALL BE ATTACHED TO ROOF DECK.

POST INSTALLED ANCHORS:

- ANCHORS SHALL ONLY BE INSTALLED WHERE SPECIFIED ON THE CONTRACT DRAWINGS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO INSTALLING POST INSTALLED ANCHORS IN PLACE OF MISSING OR MIS-PLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REINFORCING.
- THE CONTRACTOR SHALL SUBMIT PRODUCT DATA WITH DESIGN VALUES AND PHYSICAL PROPERTIES FOR ALL POST INSTALLED ANCHORS. ADDITIONALLY, THE CONTRACTOR SHALL SUBMIT CERTIFIED ICC ES OR ESR REPORTS WHICH VERIFY COMPLIANCE WITH THE SPECIFIED CRITERIA.
- SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED ON THE CONTRACT DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER ALONG WITH CALCULATIONS THAT ARE SIGNED AND SEALED BY A QUALIFIED PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION AND LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERTINENT EQUIVALENT PERFORMANCE VALUES OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARDS AS REQUIRED BY THE BUILDING CODE.
- ALL HOLES SHALL BE DRILLED, DRY AND CLEANED AND ANCHORS SHALL BE INSTALLED IN ACCORDANCE PER ANCHOR MANUFACTURER'S WRITTEN SPECIFICATIONS. THE LATEST VERSION OF THE WRITTEN SPECIFICATION SHALL BE ON-SITE AND FOLLOWED DURING THE INSTALLATION OF THE ANCHORS.
- THE ANCHOR EMBEDMENT DEPTH SHALL BE DEFINED AS THE DEPTH FROM THE SURFACE FACE OF THE LOAD BEARING BASE MATERIAL TO THE DEEPEST PART OF THE ANCHOR AFTER THE ANCHOR HAS BEEN DRIVEN INTO THE HOLE, BUT NOT YET EXPANDED, IF APPLICABLE.
- ANCHORS AT ALL WEATHER EXPOSED LOCATIONS SHALL BE STAINLESS STEEL.
- NON-EPOXY BASED ADHESIVES SHALL BE USED WHEN BASE MATERIAL TEMPERATURE IS BELOW 40 DEGREES FAHRENHEIT.
- THE FOLLOWING CONCRETE MECHANICAL ANCHORS ARE ALLOWED FOR USE IN CRACKED AND UNCRACKED CONCRETE AND HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193.
 - SIMPSON STRONG-TIE "STRONG BOLT 2" (ICC-ES ESR-3037)
 - SIMPSON STRONG-TIE "TITEN HD" (ICC-ES ESR-2713)
 - HILTI "KWIK BOLT TZ" EXPANSION ANCHOR (ICC-ES ESR 1917)
 - HILTI "HSL-3" HEAVY DUTY EXPANSION ANCHOR (ICC-ES ESR 1545)
 - HILTI "HDA" UNDERCUT ANCHOR (ICC-ES ESR 1546)
 - HILTI "KWIK HUS EZ" EXPANSION ANCHOR (ICC-ES ESR 3027)
- THE FOLLOWING CONCRETE ADHESIVE ANCHORS ARE ALLOWED FOR USE IN CRACKED AND UNCRACKED CONCRETE AND HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308.
 - SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508)
 - HILTI "HIT-HY200" (ICC-ES ESR-1385)
 - HILTI "HIT-RE 500 V3" (ICC-ES ESR-3814)

MASONRY

- CONCRETE MASONRY UNITS SHALL MEET ASTM SPECIFICATION C90, WITH A MINIMUM UNIT COMPRESSIVE STRENGTH = 1900 PSI. THE SPECIFIED DESIGN COMPRESSIVE STRENGTH OF THE CONCRETE MASONRY ASSEMBLY (f'm) SHALL BE 1900 PSI.
- MORTAR SHALL BE A PREBLENDED DRY MIX CONFORMING TO ASTM C1714 AND MEETING THE PROPERTY SPECIFICATIONS OF ASTM C270 TYPE "S" MORTAR FOR BELOW GRADE. TYPE "N" MORTAR FOR ABOVE GRADE. MASONRY CEMENT SHALL NOT BE USED FOR MORTAR.
- GROUT SHALL MEET ASTM SPECIFICATION C476 AND HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2000 PSI.
 - LIMIT THE HEIGHT OF VERTICAL GROUT POURS TO 4'-0" OR THE DISTANCE BETWEEN BOND BEAMS, WHICHEVER IS LESS.
 - GROUTING SHALL BE A CONTINUOUS PROCEDURE FOR EACH LIFT. DO NOT ALLOW HORIZONTAL CONSTRUCTION JOINT TO FORM BY DISCONTINUING GROUTING.
 - VERTICAL GROUT POUR EXCEEDING 12 INCHES SHALL BE MECHANICALLY CONSOLIDATED USING A VIBRATOR WITH A MAXIMUM 3/4 INCH DIAMETER HEAD.
- SOLID GROUT HOLLOW MASONRY CELLS AS NOTED ON STRUCTURAL DRAWINGS. USE GROUT METHOD OF CONSTRUCTION CONFORMING TO REQUIREMENTS OF CURRENT MSJC. GROUT SPACE DIMENSIONS AND MAXIMUM POUR HEIGHTS SHALL COMPLY WITH MSJC.
- CONTRACTOR SHALL CLEAN THE GROUT SPACES SUCH THAT THEY ARE FREE OF MORTAR DROPPINGS, DEBRIS, LOOSE AGGREGATES AND ANY MATERIAL THAT WOULD PREVENT CONTINUITY OF THE GROUT.
- HORIZONTAL JOINT REINFORCEMENT SHALL BE LADDER TYPE. JOINT REINFORCEMENT SHALL BE SPACED AT 8 INCHES ON CENTER BELOW FINISHED FLOOR AND IN PARAPETS, AND 16 INCHES ON CENTER ABOVE FINISHED FLOOR.
- CONCRETE MASONRY SHALL BE LAID IN RUNNING BOND.
- CONCRETE MASONRY BELOW FINISHED FLOOR SHALL BE NORMAL WEIGHT UNITS AND SHALL HAVE ALL THE CELLS FULLY GROUTED. CONCRETE MASONRY ABOVE FINISHED FLOOR SHALL BE MEDIUM WEIGHT AND IS TO BE GROUTED ONLY AT REINFORCED CELLS AND BOND BEAMS, UNLESS NOTED OTHERWISE. ALL CELLS WITH REINFORCING OR EMBEDDED ITEMS SHALL BE GROUTED SOLID.
- REFERENCE WALL SECTIONS AND DETAILS FOR MISCELLANEOUS BOND BEAM LOCATIONS AND EMBEDDED ITEMS. USE OPEN KNOCK OUT BOND BEAM BLOCK. DO NOT USE TROUGH TYPE BLOCKS FOR BOND BEAMS. DO NOT CONTINUE BOND BEAM REINFORCING THROUGH CONTROL JOINTS, UNLESS NOTED OTHERWISE.
- REINFORCING STEEL SHALL MEET ASTM SPECIFICATION A615, GRADE 60. REINFORCING STEEL SHALL BE SPLICED AS NOTED IN THE REINFORCING LAP SCHEDULE.
- PROVIDE TEMPORARY BRACING FOR WALLS, LINTELS, AND OTHER MASONRY DURING ERECTION. BRACING SHALL BE DESIGNED IN ACCORDANCE WITH THE MASON CONTRACTORS ASSOCIATION OF AMERICA STANDARD PRACTICE FOR BRACING MASONRY WALLS UNDER CONSTRUCTION. DESIGN SHALL BE PERFORMED BY AN ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. BRACING SHALL REMAIN UNTIL ROOFING AND OTHER STRUCTURAL ELEMENTS ARE COMPLETE AND PROVIDE PERMANENT STABILITY.

DEFERRED STRUCTURAL SUBMITTALS

- THE FOLLOWING STRUCTURAL COMPONENTS SHALL BE DESIGNED AND SUBMITTED BY OTHERS FOR APPROVAL IN ACCORDANCE WITH DRAWINGS AND SPECIFICATIONS.
 - STRUCTURAL STEEL CONNECTIONS OF FRAMING AND BRACING ELEMENTS
 - STEEL JOISTS AND JOIST GIRDERS (CONTRACTOR SHALL OBTAIN FIRE LINE LOCATIONS AND SIZES PRIOR TO SUBMITTAL OF JOIST SHOP DRAWINGS.)
 - STEEL, SELF-SUPPORTING STAIRS AND HANDRAIL FRAMING
 - STOREFRONT AND CURTAINWALL FRAMING, ACCESSORIES AND ATTACHMENTS TO STRUCTURE
 - EXCAVATION SUPPORT
 - TEMPORARY BRACING AND SUPPORT
 - CONCRETE WALL PANEL REINFORCING
 - ROOF ACCESS LADDERS AND SAFETY CAGES
 - SEISMIC ANCHORAGE AND BRACING OF MEP COMPONENTS
- DOCUMENTS FOR DEFERRED STRUCTURAL SUBMITTAL ITEMS SHALL BE DESIGNED, SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER OF RECORD WHO SHALL REVIEW THEM AND FORWARD THEM TO THE BUILDING OFFICIAL AS REQUESTED WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

SHOP DRAWINGS

- SHOP DRAWINGS AND SUBMITTALS SHALL BE REVIEWED AND APPROVED BY THE CONTRACTOR PRIOR TO SUBMITTAL FOR THE ENGINEER'S REVIEW. THE STRUCTURAL ENGINEER'S REVIEW IS TO CHECK THE GENERAL CONFORMANCE OF THE SHOP DRAWINGS WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR ANY ALTERATIONS FROM THE CONTRACT DOCUMENTS WHICH MAY INCLUDE QUANTITIES, DIMENSIONAL ERRORS OR OTHER ERRORS AND OMISIONS IN THE SHOP DRAWINGS.
- SHOP DRAWINGS SHALL NOT BE REPRODUCTIONS OF THE CONTRACT DOCUMENTS.
- THE FOLLOWING STRUCTURAL COMPONENTS SHALL BE SUBMITTED AS A SHOP DRAWING FOR REVIEW:
 - CONCRETE MIX DESIGN AND MATERIALS
 - CONCRETE REINFORCING STEEL
 - CONCRETE FORMWORK
 - STRUCTURAL STEEL
 - STEEL JOISTS
 - STEEL ROOF DECK AND THEIR ATTACHMENTS.
 - ALL DEFERRED SUBMITTAL ITEMS

SPECIAL INSPECTIONS

- THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS PER SECTION 1704 OF THE IBC. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED IN THE PROJECT SPECIFICATIONS.
- SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS DONE IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO COMPLETION OF THAT PHASE OF WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE START OF WORK.
- THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE SPECIAL INSPECTOR REGARDING INDIVIDUAL INSPECTION FOR ITEMS LISTED ON THE STATEMENT OF SPECIAL INSPECTIONS AND AS NOTED ON THE BUILDING DEPARTMENT APPROVED PLANS. ADEQUATE NOTICE AND ACCESS TO APPROVED PLANS SHALL BE PROVIDED SO THAT THE SPECIAL INSPECTOR HAS TIME TO BECOME FAMILIAR WITH THE PROJECT.
- FABRICATORS OF STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1704.2 OF THE IBC.
- THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTION PER SECTION 1700 OF THE REFERENCED BUILDING CODE.
 - BOLTS & ANCHORS EMBEDDED IN CONCRETE
 - PLACEMENT OF REINFORCING STEEL IN CONCRETE
 - CONCRETE MIX DESIGN
 - CONCRETE FORMWORK
 - STRUCTURAL STEEL FABRICATIONS
 - STRUCTURAL STEEL BOLTING AND WELDING
 - ON SITE STRUCTURAL FRAMING
 - INSPECTION OF ROOF DECK ATTACHMENTS
 - SHEAR WALL ATTACHMENTS AND ANCHORS
 - POST INSTALLED ANCHORS
 - ON SITE SOILS, EXCAVATIONS, FILLING AND COMPACTION
 - ERECTION OF PRECAST CONCRETE MEMBERS

ABBREVIATIONS

A.B.	ANCHOR BOLTS
ACI	AMERICAN CONCRETE INSTITUTE
A.E.S.S.	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
A.F.F.	ABOVE FINISHED FLOOR
ARCH.	ARCHITECTURAL
BAL.	BALANCE
B.L.	BLOCK LINTEL
BLDG.	BUILDING
B.O.	BOTTOM OF
B.O.D.	BOTTOM OF DECK
BRG.	BEARING
C.J.	CONTRACTION JOINT
C.L.	CENTER LINE
CLR.	CLEAR
CMU	CONCRETE MASONRY UNIT
COL.	COLUMN
CONC.	CONCRETE
CONST.	CONSTRUCTION
CONT.	CONTINUOUS
D.B.A.	DEFORMED BAR ANCHOR
DIA.	DIAMETER
DWG.	DRAWING
E.F.	EACH FACE
E.J.	EXPANSION JOINT
ELEV.	ELEVATION
E.O.D.	EDGE OF DECK
E.O.S.	EDGE OF SLAB
EQ.	EQUH WAY
E.W.	EACH WAY
EXIST.	EXISTING
FDN.	FOUNDATION
F.F.E.	FINISHED FLOOR ELEV.
F.S.	FAR SIDE
FTG.	FOOTING
GA.	GAGE
GALV.	GALVANIZED
G.B.	GRADE BEAM
HORIZ.	HORIZONTAL
H.S.A.	HEADED STUD ANCHOR
IBC	INTERNATIONAL BUILDING CODE
INFO.	INFORMATION
J.B.E.	JOIST BEARING ELEVATION
J.T.	JOINT
K	UNIT OF 1,000 POUNDS (KIP)

KSI	KIPS PER SQUARE INCH
LBS.	POUNDS
L.H.	LONG LEG HORIZONTAL
LV.	LONG LEG VERTICAL
LONG.	LONGITUDINAL
MAX.	MAXIMUM
MECH.	MECHANICAL
MFR.	MANUFACTURER
MIN.	MINIMUM
MISC.	MISCELLANEOUS
N.I.C.	NOT IN CONTRACT
NO.	NUMBER
N.T.S.	NOT TO SCALE
N.S.	NEAR SIDE
O.C.	ON CENTER
O.D.	OUTSIDE DIAMETER
O.H.	OPPOSITE HAND
P.A.F.	POWER ACTUATED FASTENER
PCF	POUNDS PER CUBIC FOOT
PLF	POUNDS PER LINEAR FOOT
P.M.E.J.	PREMOLDED EXPANSION JOINT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
QTY.	QUANTITY
RE.	REFER
REINF.	REINFORCING
REQD.	REQUIRED
R.O.	ROUGH OPENING
RTU	ROOF TOP UNIT
SCHED.	SCHEDULE
S.D.S.	SELF-DRILLING SCREWS
SIM.	SIMILAR
SPECS.	SPECIFICATIONS
STD.	STANDARD
STL.	STEEL
T&B	TOP AND BOTTOM
T.O.	TOP OF
T.O.P.	TOP OF PIER
T.O.W.	TOP OF WALL
TRANS.	TRANSVERSE
TYP.	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
VERT.	VERTICAL
W.P.	WORK POINT
WT.	WEIGHT
W.W.R.	WELDED WIRE REINFORCEMENT

NOTE: THE CONTRACTOR SHALL PROVIDE A BASE BID PRICING BASED ON THE PANEL THICKNESS SHOWN ON THE DRAWINGS. ADDITIONALLY, THE BASE BID SHALL REFLECT 3.5 LBS PER SQUARE FEET OF REINFORCING STEEL WITH UNIT PRICING OF ANY ADD OR DEDUCT FROM AFOREMENTIONED TONNAGE. ANY VALUE ENGINEERING DEVIATIONS FOR THE WALLS PANELS FOR PANEL THICKNESS SHALL BE QUALIFIED AS A SEPARATE LINE ITEM IN THE CONTRACTOR'S BID.



CURRAN
ARCHITECTURE

5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317 . 288 . 0681
F :: 317 . 288 . 0753



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PROPERTIES

wallace



Wallace Engineering
Structural Consultants, Inc.

Structural and Civil Consultants
1741 McGee Street
Kansas City, Missouri 64108
816.421.8282, Fax 816.421.8338

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PROJECT INFORMATION

LEE'S SUMMIT LOGISTICS
BUILDING B LOT 2

NW CORNER TUDOR RD & MAINST
LEE'S SUMMIT, MO

ISSUE DATES

ISSUE	DATE
ISSUE FOR PERMIT	04.22.2022

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S0.1

GENERAL NOTES

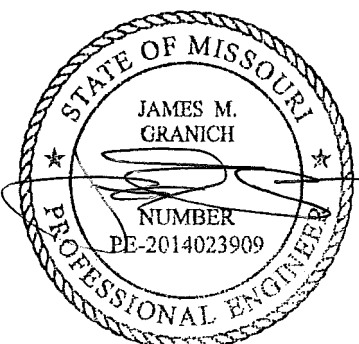


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ARCHITECTURE
5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317 . 288 . 0681
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BUILDING B LOT 2
NW CORNER TUDOR RD & MAINST
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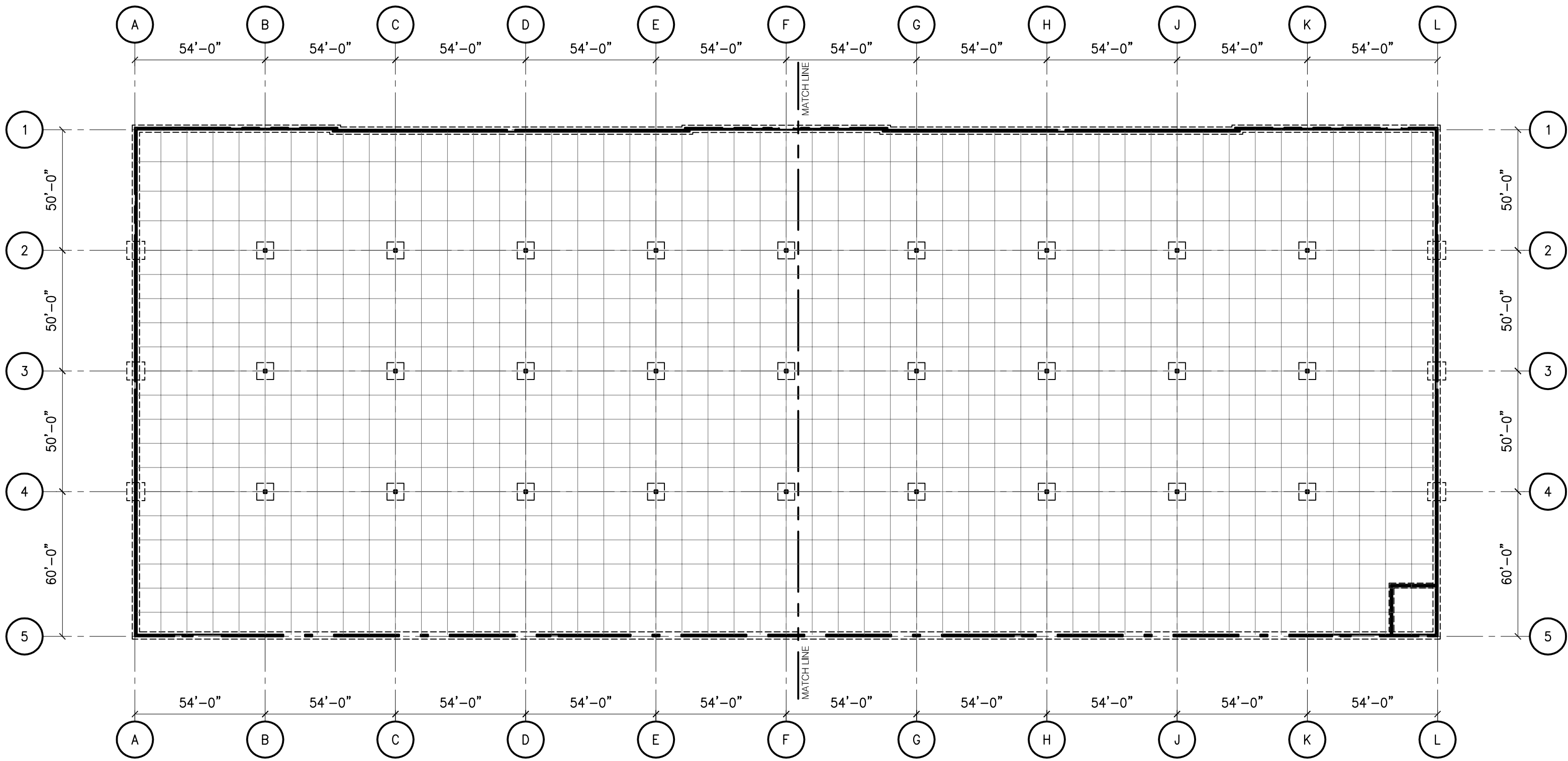
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S1.0

OVERALL FOUNDATION PLAN



1 OVERALL FOUNDATION PLAN
SCALE: 1"=40'-0"



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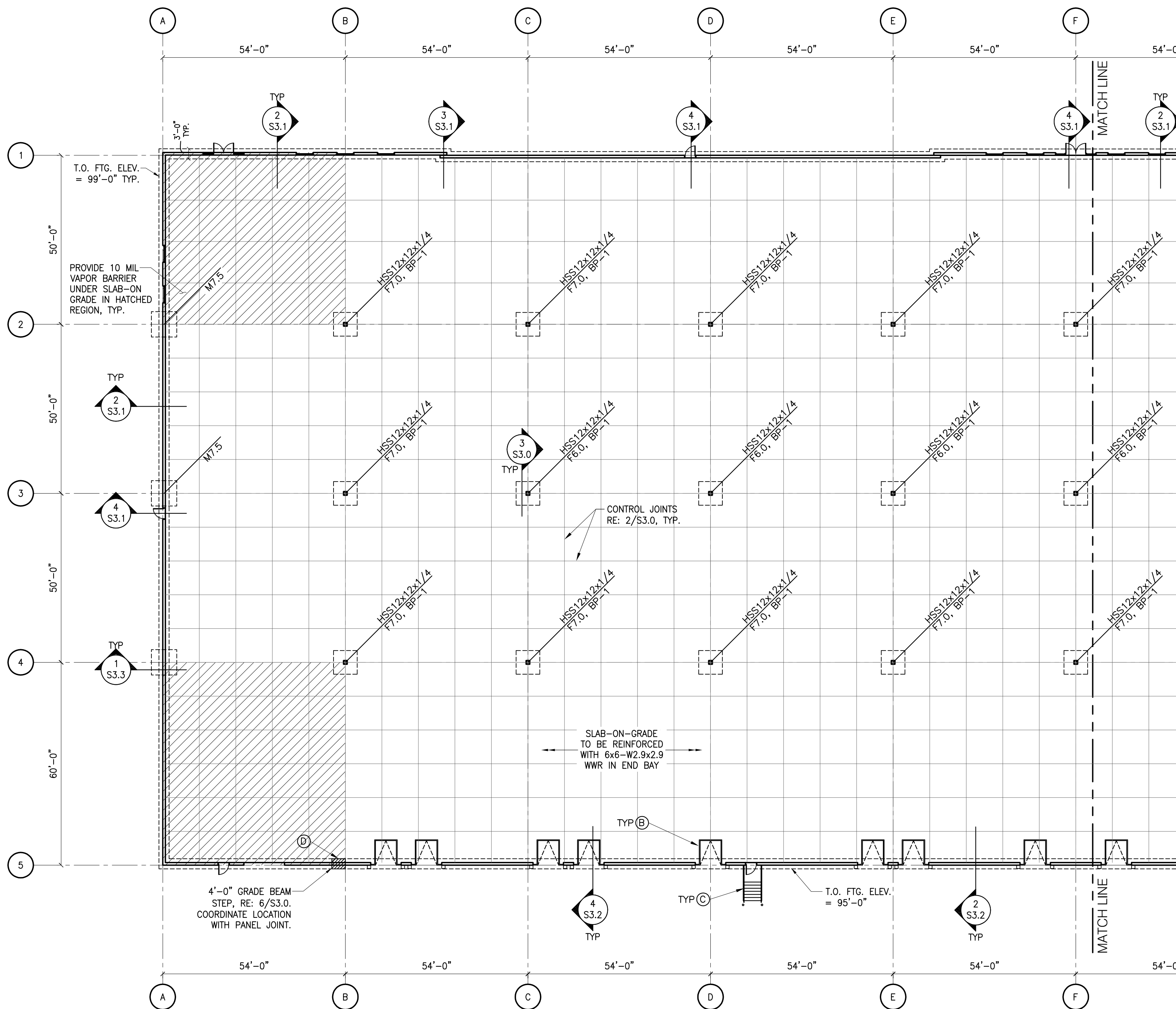
NW CORNER TUDOR RD & MAINST
LEE'S SUMMIT, MO

ENLARGED PARTIAL
FOUNDATION PLAN

1. CONCRETE SLAB-ON-GRADE, U.N.O., SHALL BE A 7" THICK UNREINFORCED SLAB (U.N.O.) OVER 4" ROCK, RE: THE GEOTECHNICAL REPORT. T.O. SLAB ELEV = 100'-0". SLAB TO BE SEALED WITH SINGLE COAT OF ASHFORD (OR EQUAL) FLOOR SEALANT.
2. THE CONCRETE SLABS SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED FOR THE FINISH FLOOR FINISHES AND HAVE NOT BEEN DESIGNED FOR MANS AND OTHER METHODS OF CONSTRUCTION INCLUDING BUT NOT LIMITED TO, FORK LIFTS, MAN LIFTS, AND OTHER VEHICULAR TRAFFIC. THE CONTRACTOR SHALL VERIFY THE SLAB DESIGN MEETS THE CONSTRUCTION NEEDS AND SHALL SUBMIT TO THE ENGINEER OF RECORD FOR REVIEW.
3. TOP OF FOOTING ELEV. = 99'-0, UNLESS NOTED OTHERWISE.
4. ALL PIPING OR CONDUITS THAT OCCUR THROUGH OR UNDER A GRADE BEAM OR FOOTING SHALL BE APPROVED BY THE ENGINEER OF RECORD PRIOR TO PLACEMENT. (RE: 4 & 5/3.0).
5. RE: 1/53.0 FOR REINFORCING LAP SCHEDULE.
6. RE: SHEET S3.0 FOR ADDITIONAL CONCRETE FOUNDATION DETAILS
7. ALL PRECAST PANELS SHALL BE 9 1/4" THICK, U.N.O.

- (A) DRAIN BLOCKOUT IN FOOTING, RE: 10&11/S3.0
- (B) DOCK PIT, RE: 5/S3.2. RE: ARCH. FOR LOCATIONS.
- (C) DOCK STAIRS RE: 1/S3.1. REFER TO ARCH DWGS FOR LOCATIONS AND TYPE OF STAIR
- (D) FOOTING STEP, RE: 6/S3.0
- (E) RAMP, RE: CIVIL DWGS.

1. F# = FOOTING MARK; RE: FOOTING SCHEDULE
2. C.J. = SAW CUT CONTROL JOINT; RE: DETAIL 2/S3.0
3. B.P. = BASE PLATE; RE: DETAIL 9/S3.0



1 ENLARGED PARTIAL FOUNDATION PLAN
SCALE: 1"=20'-0"



PLAN NOTES:

- CONCRETE SLAB-ON-GRADE, U.N.O., SHALL BE A 7" THICK UNREINFORCED SLAB (U.N.O.) OVER 4" ROCK, RE: THE GEOTECHNICAL REPORT, T.O. SLAB ELEV = 100'-0". SLAB TO BE SEALED WITH SINGLE COAT OF ASHFORD (OR EQUAL) FLOOR SEALANT. THE CONCRETE SLABS SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED FOR THE FINISHED STRUCTURE AND HAVE NOT BEEN DESIGNED FOR MEANS AND METHODS OF CONSTRUCTION, INCLUDING BUT NOT LIMITED TO, FORK LIFTS, MAN LIFTS, AND OTHER VEHICULAR TRAFFIC. THE CONTRACTOR SHALL VERIFY THE SLAB DESIGN MEETS THE CONSTRUCTION NEEDS AND SHALL SUBMIT TO THE ENGINEER OF RECORD FOR REVIEW.
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- RE: 1/S3.0 FOR REINFORCING LAP SCHEDULE.
- RE: SHEET S3.0 FOR ADDITIONAL CONCRETE FOUNDATION DETAILS
- ALL PRECAST PANELS SHALL BE 9 1/4" THICK, U.N.O.

PLAN REFERENCE NOTES:

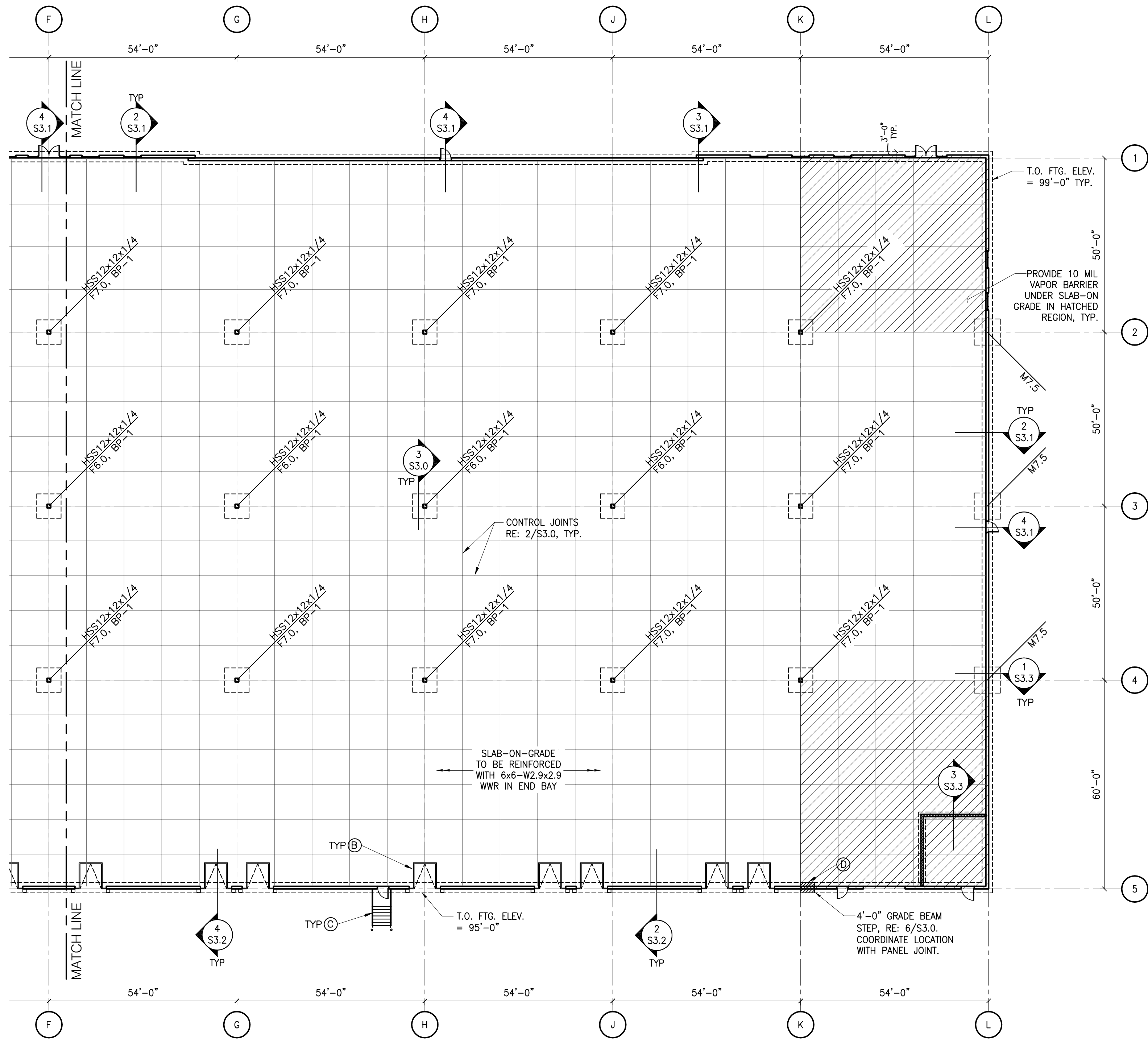
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(C) DOCK STAIRS RE: 1/S3.1. REFER TO ARCH DWGS FOR LOCATIONS AND TYPE OF STAIR
(D) FOOTING STEP, RE: 6/S3.0
(E) RAMP, RE: CIVIL DWGS.

LEGEND

1. F# = FOOTING MARK; RE: FOOTING SCHEDULE
2. C.J. = SAW CUT CONTROL JOINT; RE: DETAIL 2/S3.0
3. B.P. = BASE PLATE; RE: DETAIL 9/S3.0

SPOT FOOTING SCHEDULE

MARK	SIZE	REINFORCEMENT
M7.5	7'-6"x7'-6"x2'-6"	NO REINF. REQUIRED
F6.0	6'-0"x6'-0"x1'-3"	(6)-#6 EA. WAY
F7.0	7'-0"x7'-0"x1'-3"	(7)-#6 EA. WAY



1 ENLARGED PARTIAL FOUNDATION PLAN
SCALE: 1"=20'-0"

CURRAN
ARCHITECTURE
5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317 . 288 . 0681
F :: 317 . 288 . 0753

SCANNELL
PROPERTIES
wallace
Wallace Engineering
Structural Consultants, Inc.
Structural and Civil Consultants
1741 McGee Street
Kansas City, Missouri 64108
816.421.8282, Fax 816.421.8338

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NUMBER DE-2014023909
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PROJECT INFORMATION
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BUILDING B LOT 2
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LEE'S SUMMIT, MO

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S1.2
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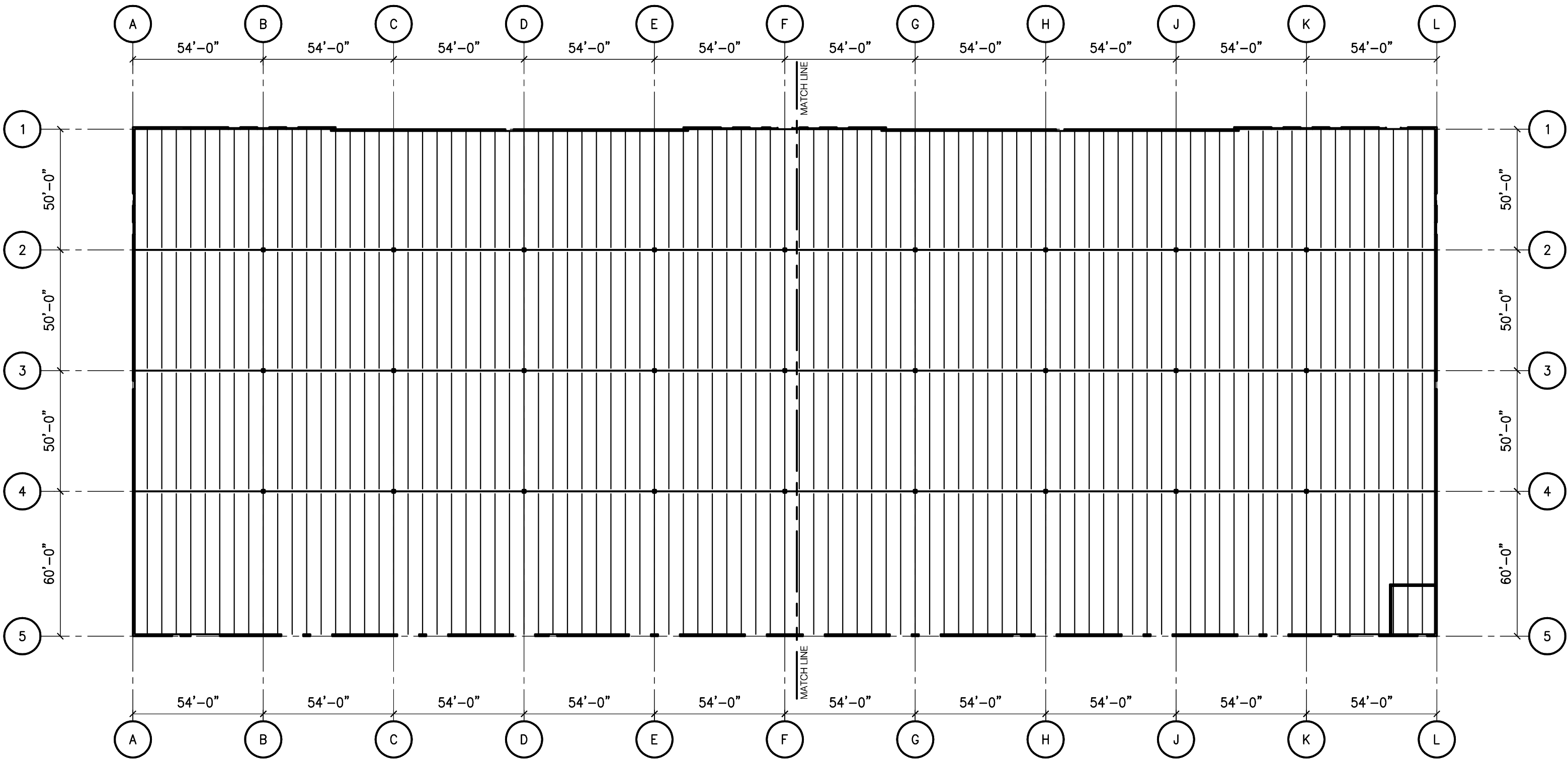
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PROJECT INFORMATION

LEE'S SUMMIT LOGISTICS
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1 OVERALL FRAMING PLAN
SCALE: 1"=40'-0"

ISSUE DATES

ISSUE	DATE
ISSUE FOR PERMIT	04.22.2022

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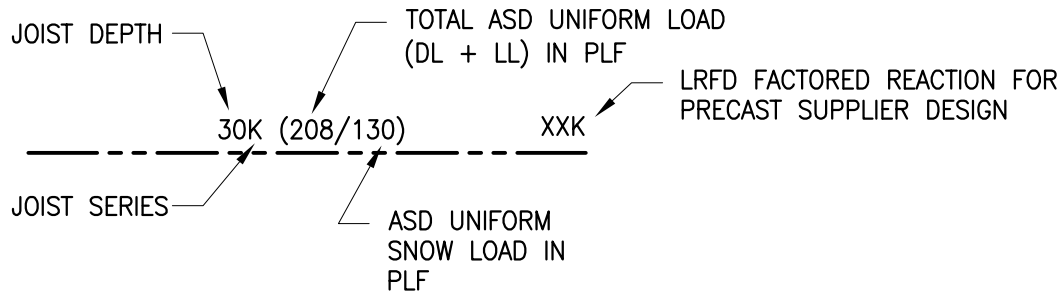
PLAN REFERENCE NOTES:

- (A) ROOF HATCH, RE: ARCH. PROVIDE ANGLE FRAME AT OPENING, RE: 8/54.0
- (B) JOIST SUPPLIER SHALL DESIGN JOISTS FOR AXIAL LOAD SHOWN.
- (C) DRAG STRUT SPLICE, RE: 9/54.0.
- (D) ROOF TOP EQUIPMENT, RE: ARCH./MEP. PROVIDE ANGLE FRAME AND CURB RE: 5/54.0 JOIST SUPPLIER SHALL ACCOUNT FOR LOAD SHOWN ON PLAN IN JOIST DESIGN.

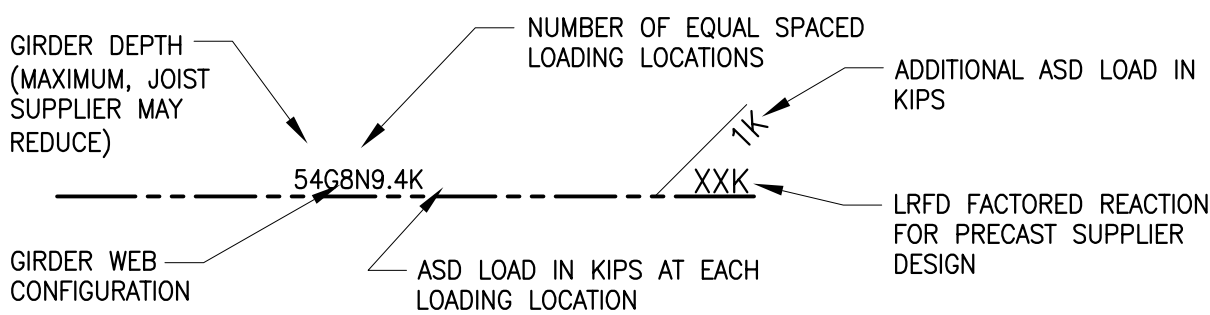
PLAN NOTES

1. ALL EDGE ANGLES SHALL BE CONTINUOUS AND SPLICED PER 6/54.0.
2. VERIFY ALL WALL OPENING, DIMENSIONS, JOINTS, BLOCKOUTS, REVEALS AND FUTURE KNOCK OUT PANELS WITH ARCHITECTURAL DRAWINGS.
3. NOTE TO JOIST MANUFACTURER: PROVIDE STANDARD BRIDGING COMPLYING WITH THE APPLICABLE STEEL JOIST INSTITUTE SPECIFICATIONS TYPICAL FOR GRAVITY AND UPLIFT LOADS SUPERIMPOSED ON ALL JOISTS. DIAGONAL BRIDGING SHALL BE PROVIDED BETWEEN ADJACENT JOISTS WHENEVER BOTTOM CHORD HORIZONTAL BRIDGING IS DISCONTINUOUS. (RE: 1 & 2/54.0)
4. ROOF DECK AND ROOF DECK ATTACHMENT SHALL BE PER SHEET S2.5.
5. RE: 3 AND 4/54.1 FOR ADDITIONAL PRECAST PANEL CONNECTION DETAILS
6. JOIST SHALL BE DESIGNED FOR ROOF TOP EQUIPMENT, RE: ARCH./MEP. PROVIDE ANGLE FRAME AND CURB, RE: 5/54.0. JOIST SUPPLIER SHALL ACCOUNT FOR LOAD SHOWN ON PLAN IN JOIST DESIGN.
7. JOIST AND JOIST GIRDER DEPTHS SHALL BE LIMITED SO THAT 32'-0" CLEAR HEIGHT TO BOTTOM OF STRUCTURE IS MAINTAINED

JOIST LEGEND



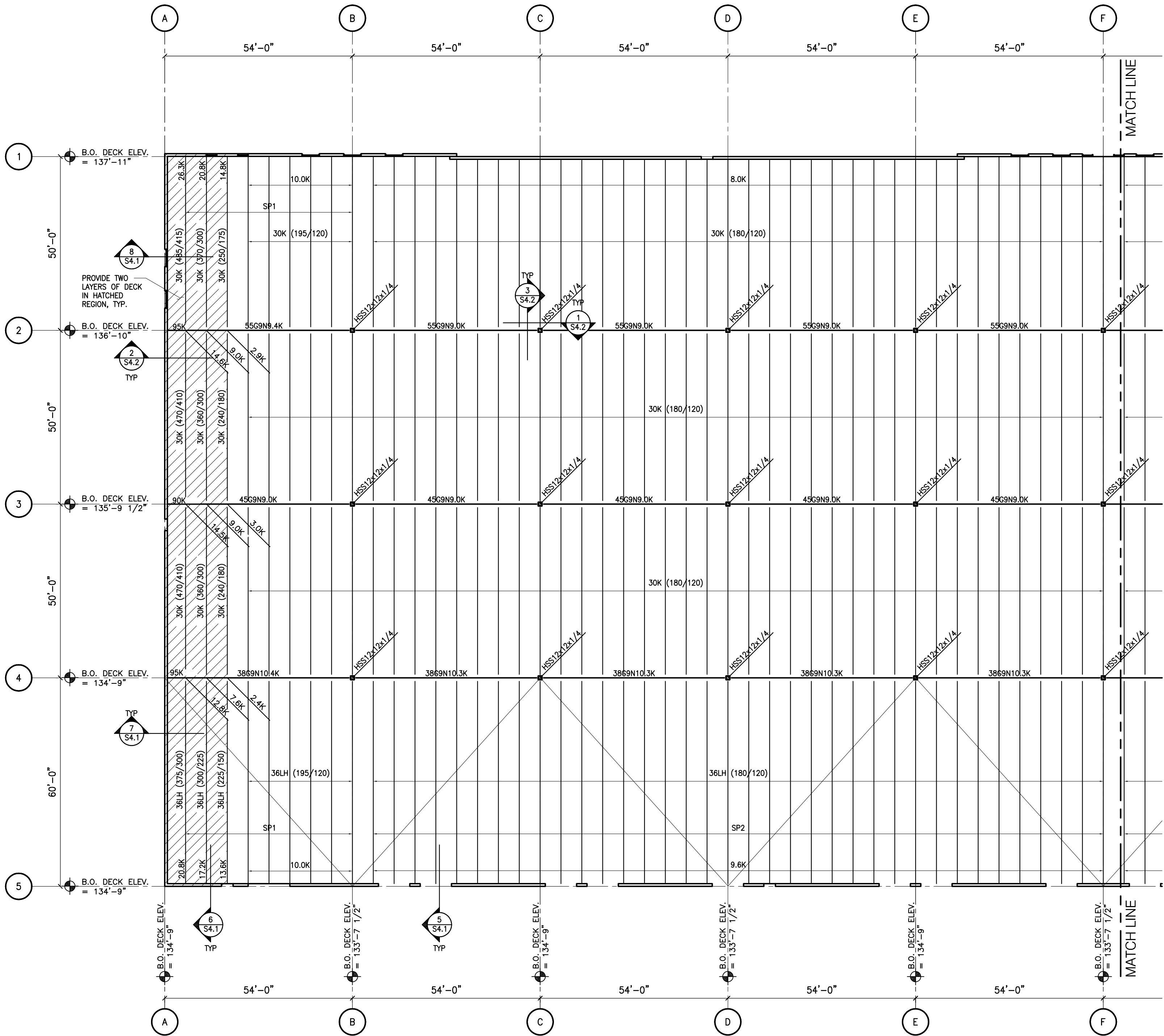
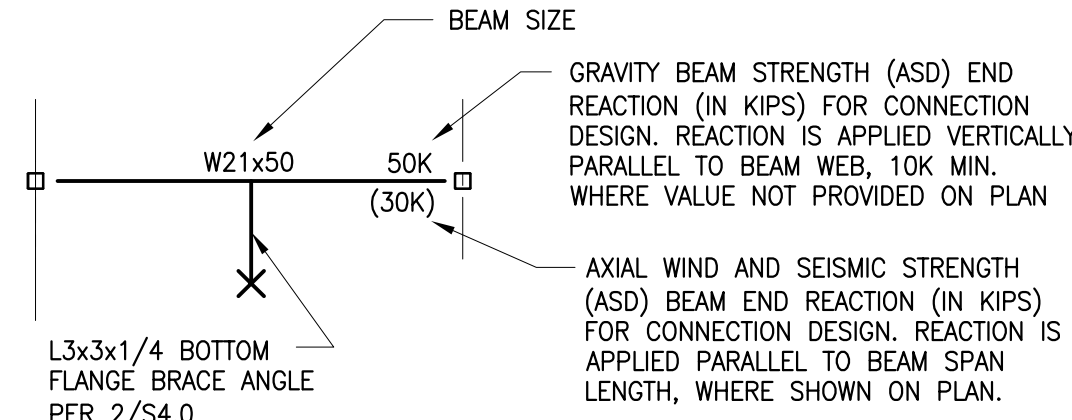
JOIST GIRDER LEGEND



BEAM REACTION LEGEND

STEEL FABRICATOR SHALL DESIGN THE BEAM CONNECTIONS FOR THE STRENGTH LEVEL LOADS (ASD) SHOWN ON THIS PLAN, TYP. (RE: 1/54.0)

USE MINIMUM TWO BOLT CONNECTION



1 ENLARGED PARTIAL FRAMING PLAN
SCALE: 1"=20'-0"

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ARCHITECTURE

5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317 . 288 . 0681
F :: 317 . 288 . 0753

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PROPERTIES

wallace

Wallace Engineering
Structural Consultants, Inc.
Structural and Civil Consultants
1741 McGee Street
Kansas City, Missouri 64108
816.421.8282, Fax 816.421.8338

CERTIFICATION

STATE OF MISSOURI
JAMES M. GRANCHI
NUMBER DE-2014023909
PROFESSIONAL ENGINEER

04/22/2022
Missouri COA #001268

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BUILDING B LOT 2

NW CORNER TUDOR RD & MAINST
LEE'S SUMMIT, MO

ISSUE DATES

ISSUE	DATE
ISSUE FOR PERMIT	04.22.2022

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S2.1
ENLARGED PARTIAL
FRAMING PLAN

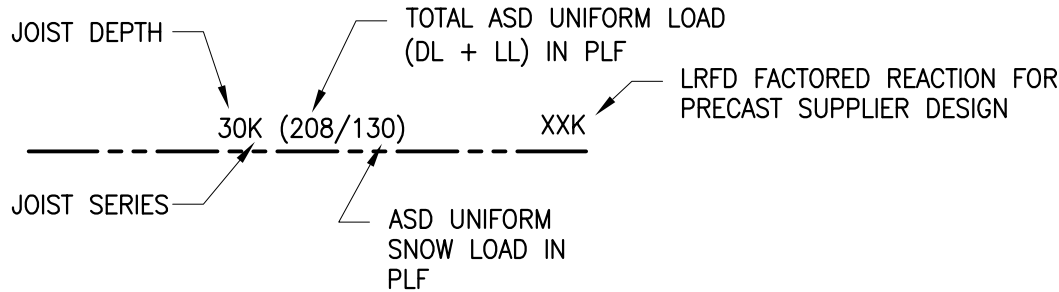
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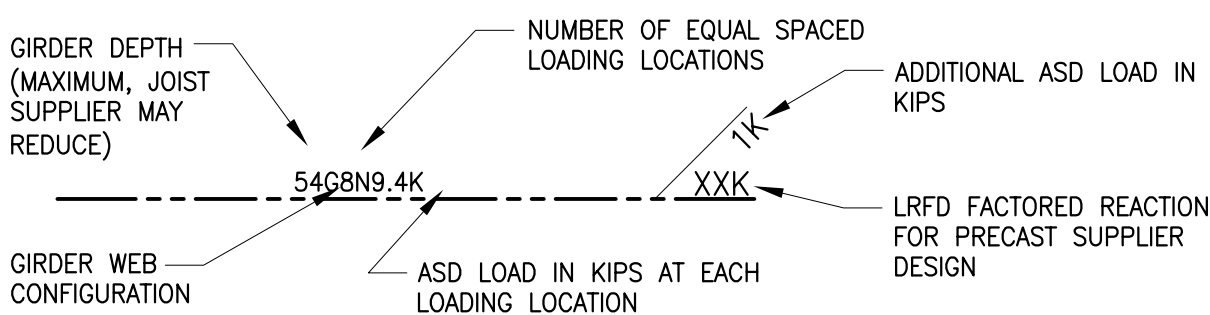
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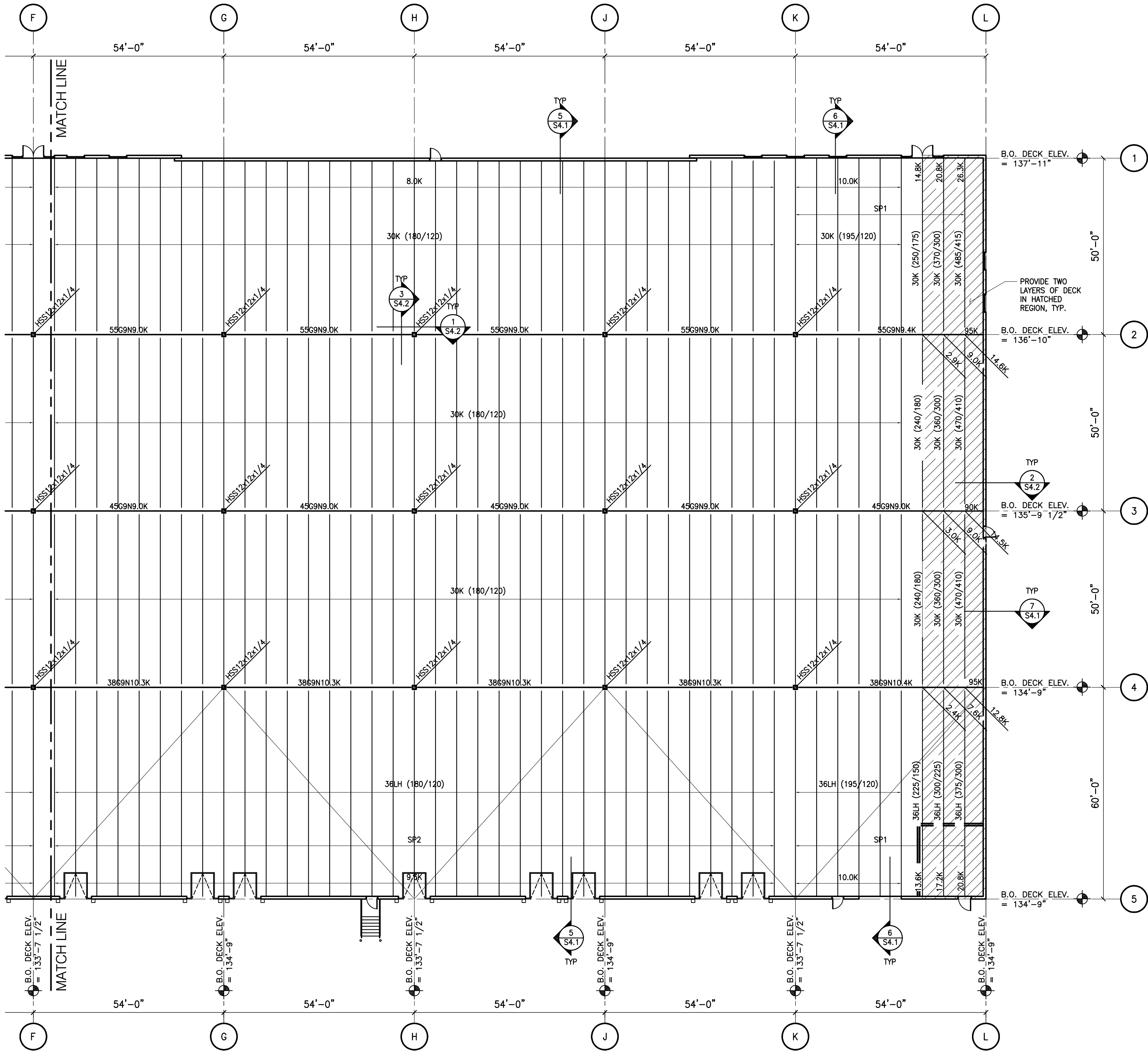
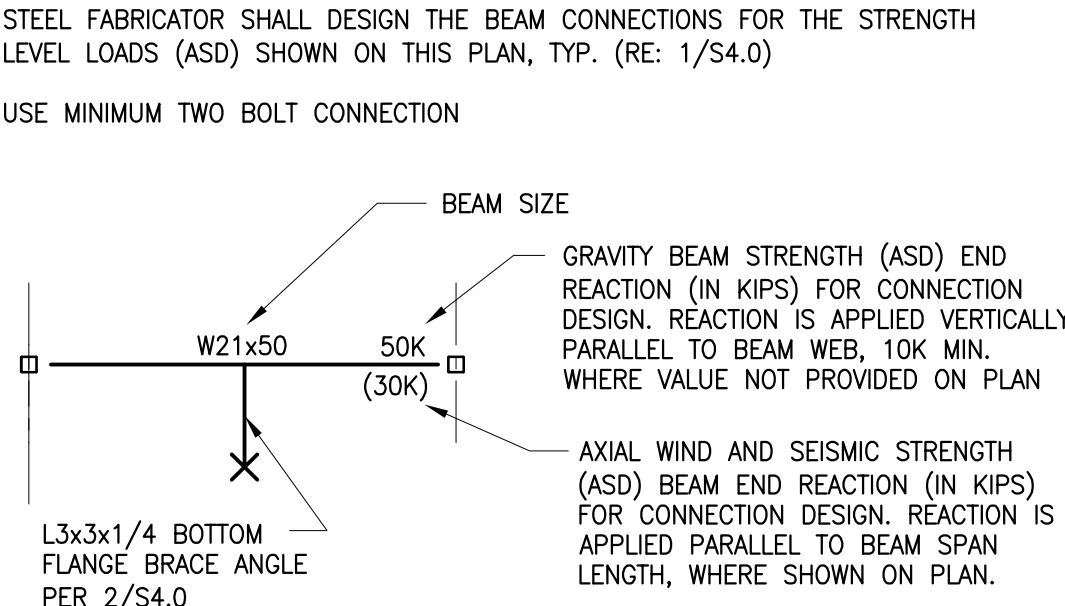
JOIST LEGEND



JOIST GIRDER LEGEND



BEAM REACTION LEGEND



1 ENLARGED PARTIAL FRAMING PLAN

SCALE: 1"=20'-0"

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5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317 . 288 . 0681
F :: 317 . 288 . 0753

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wallace

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BUILDING B LOT 2

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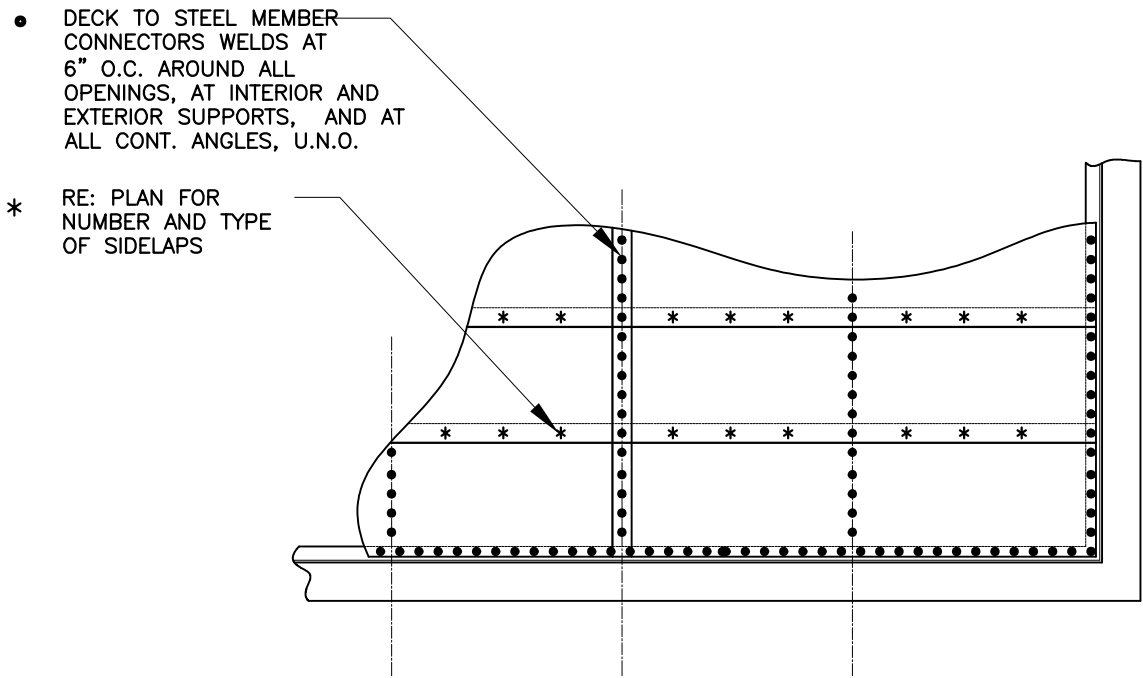
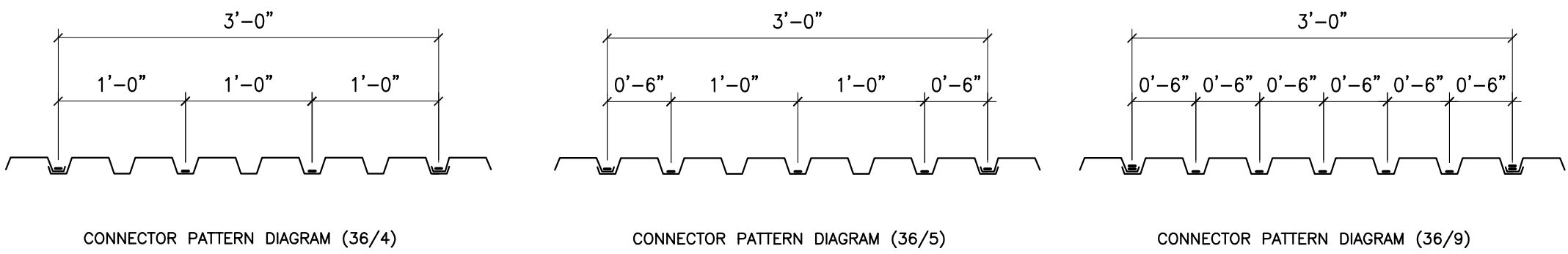
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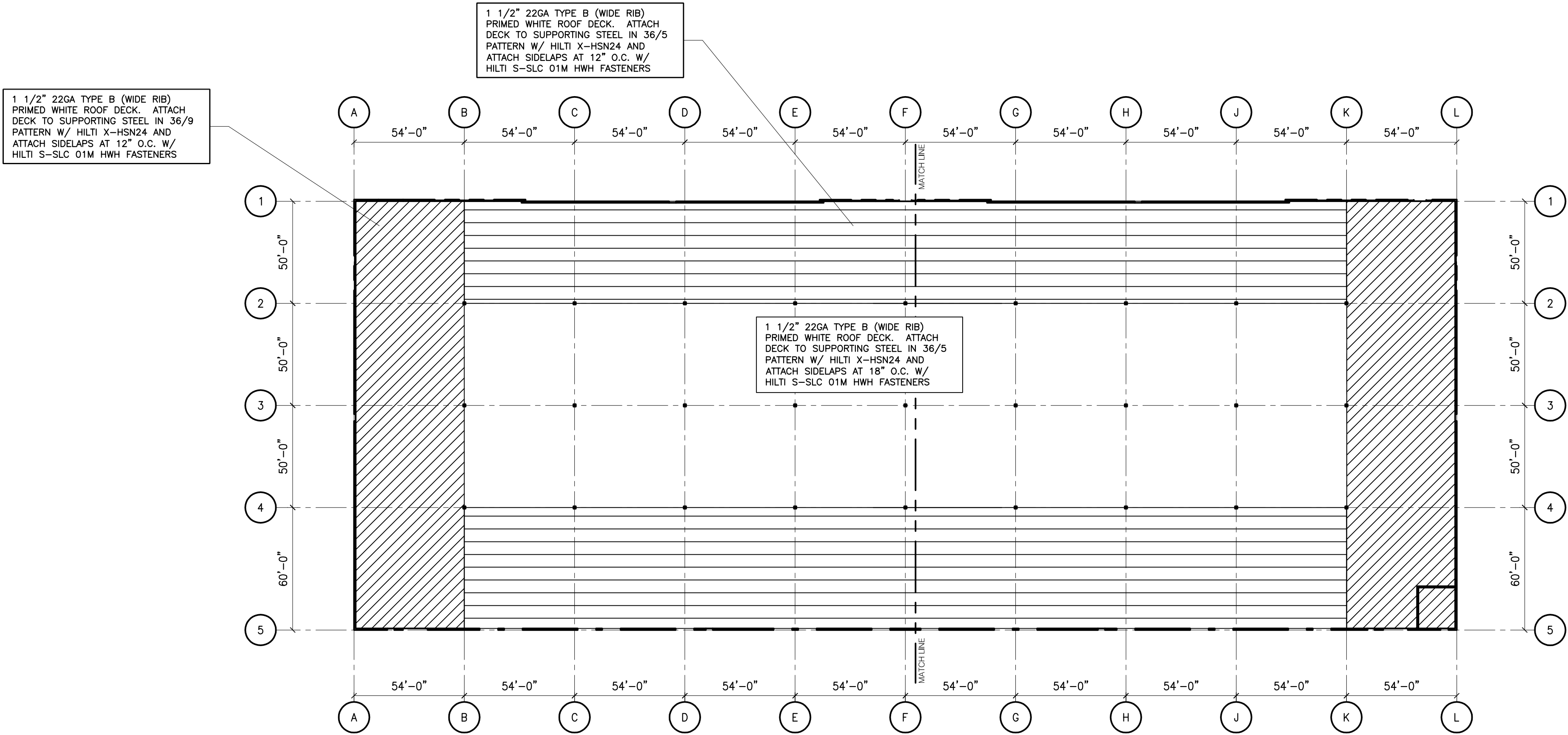
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S2.2
ENLARGED PARTIAL
FRAMING PLAN

NOTE:
CONTRACTOR SHALL COORDINATE THE TYPE OF PINS USED WITH THE THICKNESS OF THE JOISTS AND JOIST GIRDERS. FOR AREAS IN WHICH THE SUPPORTING STRUCTURE IS THICKER THAN 3/8", THE X-HSN24 PINS SHALL BE SUBSTITUTED FOR X-ENP-19-L15 PINS AT THE SAME FASTENING PATTERNS SPECIFIED.



ROOF DIAPHRAGM CONNECTION DIAGRAM



1 ROOF DECK ATTACHMENT
SCALE: 1"=40'-0"



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5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317 . 288 . 0681
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S2.3
ROOF DECK ATTACHMENT
PLAN



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INDIANAPOLIS, IN 46216
O :: 317 . 288 . 0681
F :: 317 . 288 . 0753



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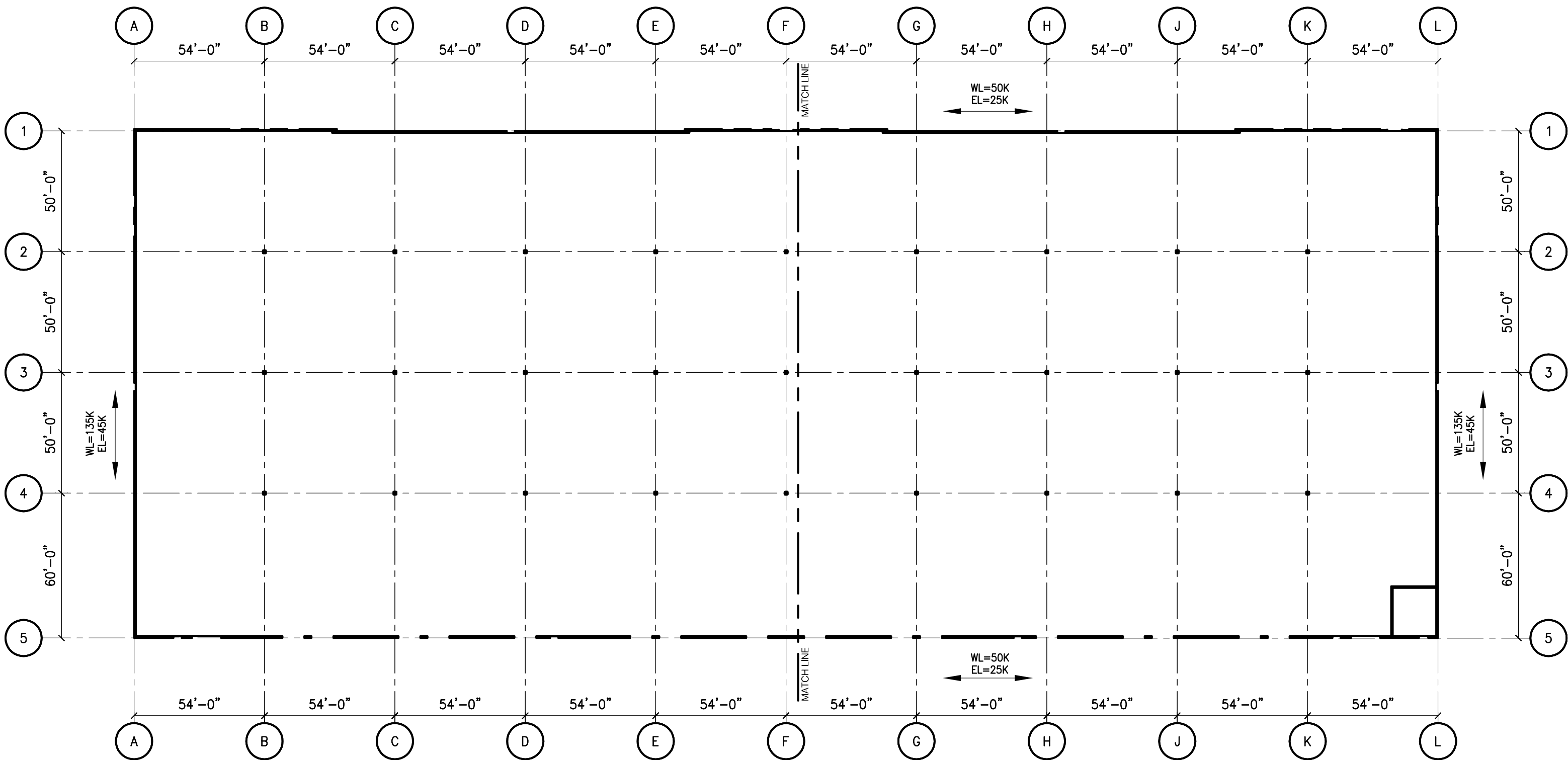
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S2.4

LATERAL LOAD PLAN

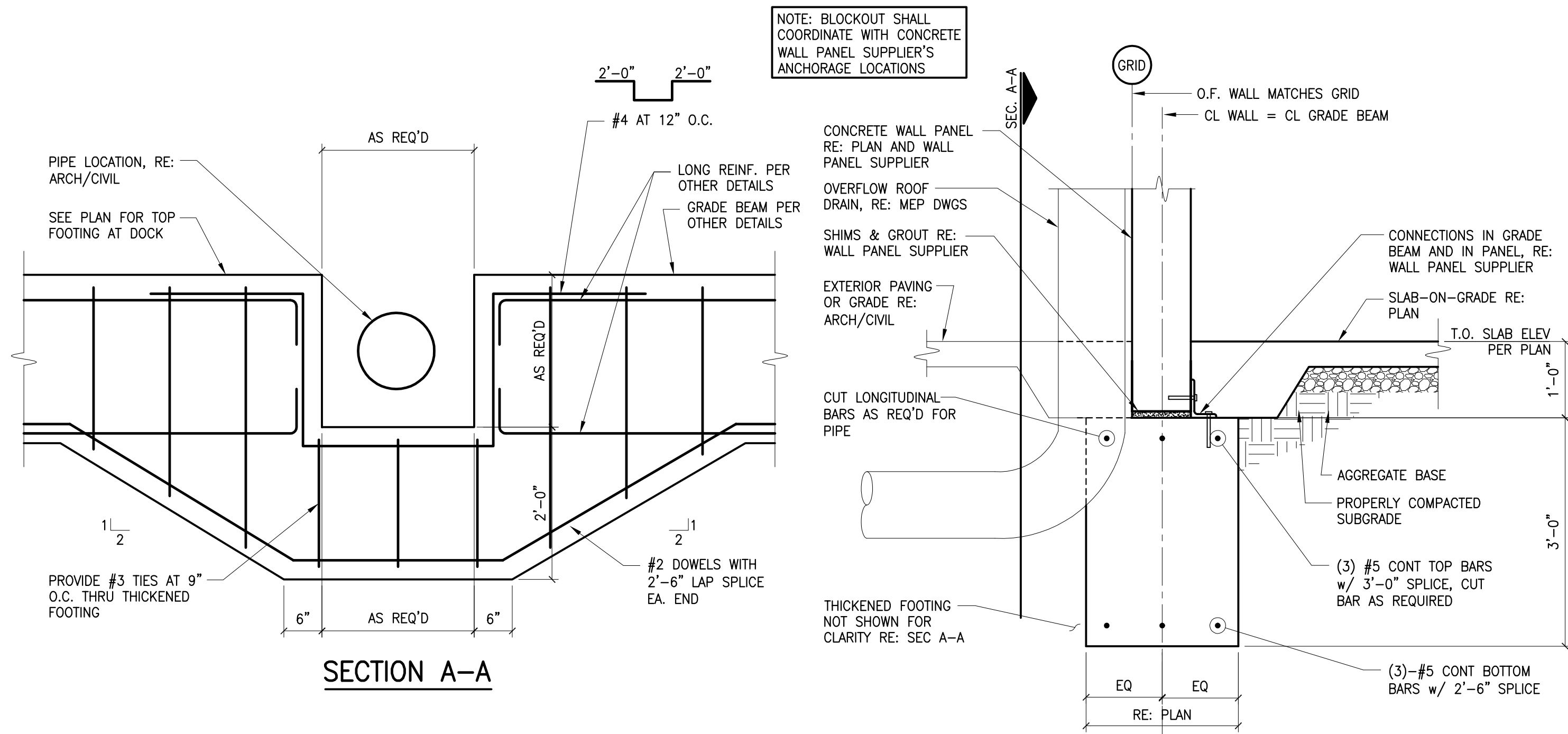
LOAD PLAN NOTES:

THE LATERAL SYSTEM OF THE BUILDING IS A COMBINATION OF SHEAR WALLS AND BRACED FRAMES. THE LOADS SHOWN ON THIS PLAN ARE THE ASD SEISMIC AND ASD WIND LOADS THAT ARE IMPARTED ON THE CONCRETE WALL PANELS, WHICH SHALL BE DESIGN AND DETAILED AS SHEAR WALLS. THE CONCRETE WALL SUPPLIER SHALL DESIGN THE PANELS TO RESIST THE LATERAL LOADS APPLIED AT THE ROOF DIAPHRAGM ELEVATION AS SHOWN PER OTHER DETAILS. THE ADDITIONAL SEISMIC LOAD INDUCED BY THE WEIGHT OF THE IN-PLANE PANELS AND OUT-OF-PLANE PANELS HAVE BEEN ACCOUNTED FOR IN THE FORCES SHOWN ON THE PLAN. THE CONCRETE WALL SUPPLIER SHALL DESIGN AND DETAIL THE CONNECTION OF THE CONCRETE PANELS TO THE FOUNDATION IN ORDER TO RESIST THE SHEAR AND UPLIFT FORCES FROM THE CONCRETE PANEL INTO THE FOUNDATION SYSTEM. THE FOUNDATION SYSTEM HAS BEEN DESIGNED FOR THE CONCRETE PANELS SHOWN TO ACT AS A COMPLETE SYSTEM ANY DEVIATIONS FROM THIS SHALL BE APPROVED BY THE ENGINEER OF RECORD.

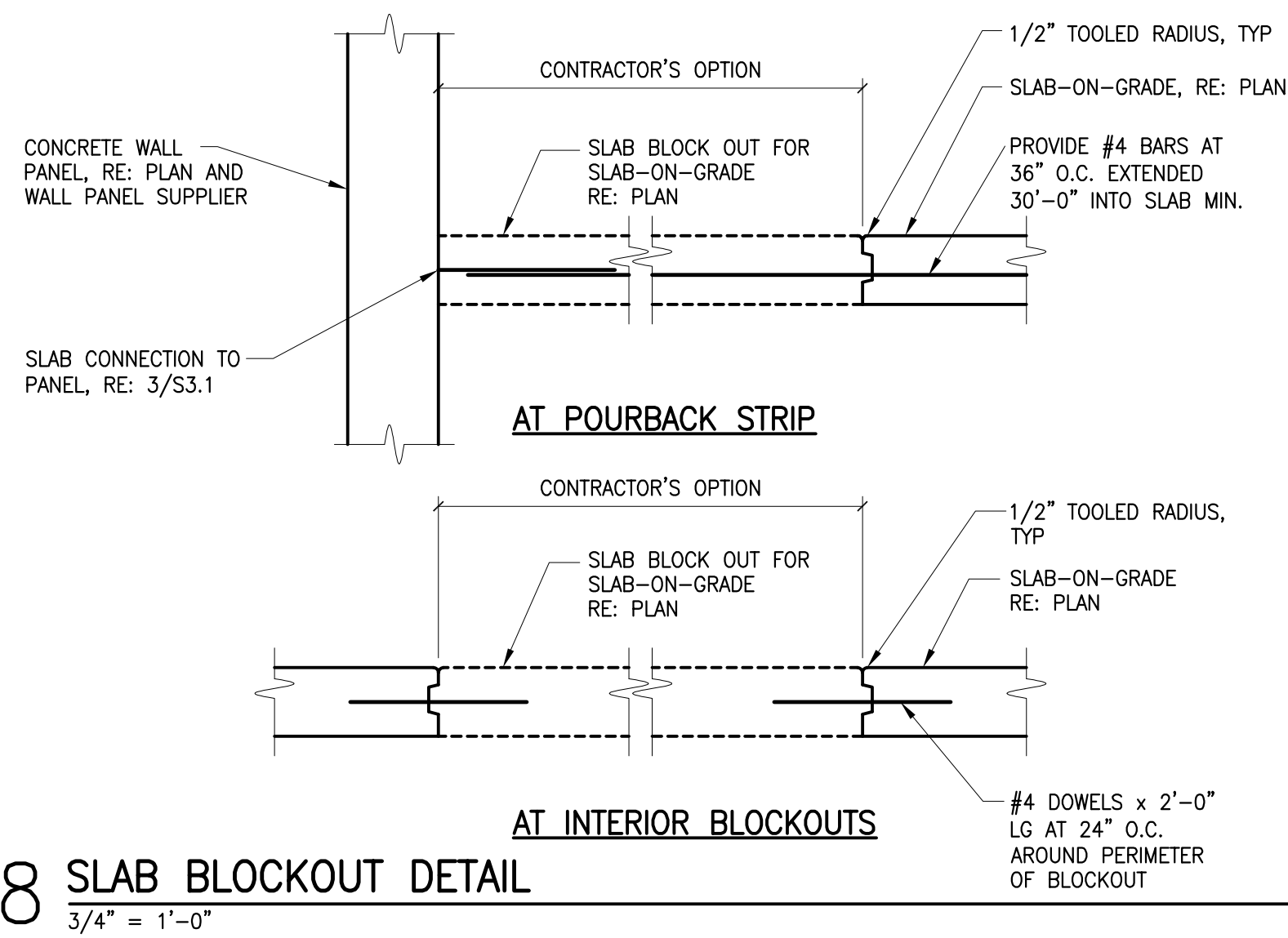


1 LATERAL LOAD PLAN
SCALE: 1/32"=1'-0"

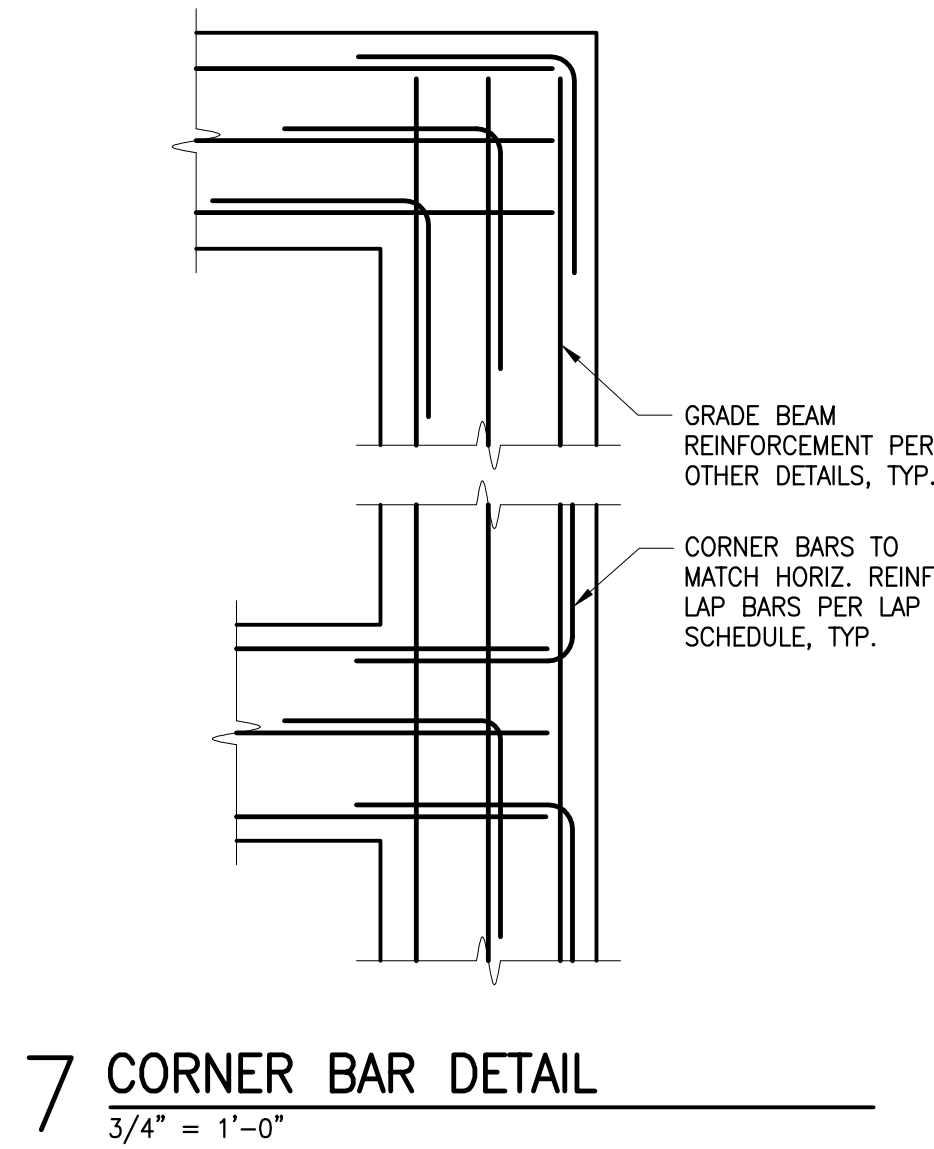




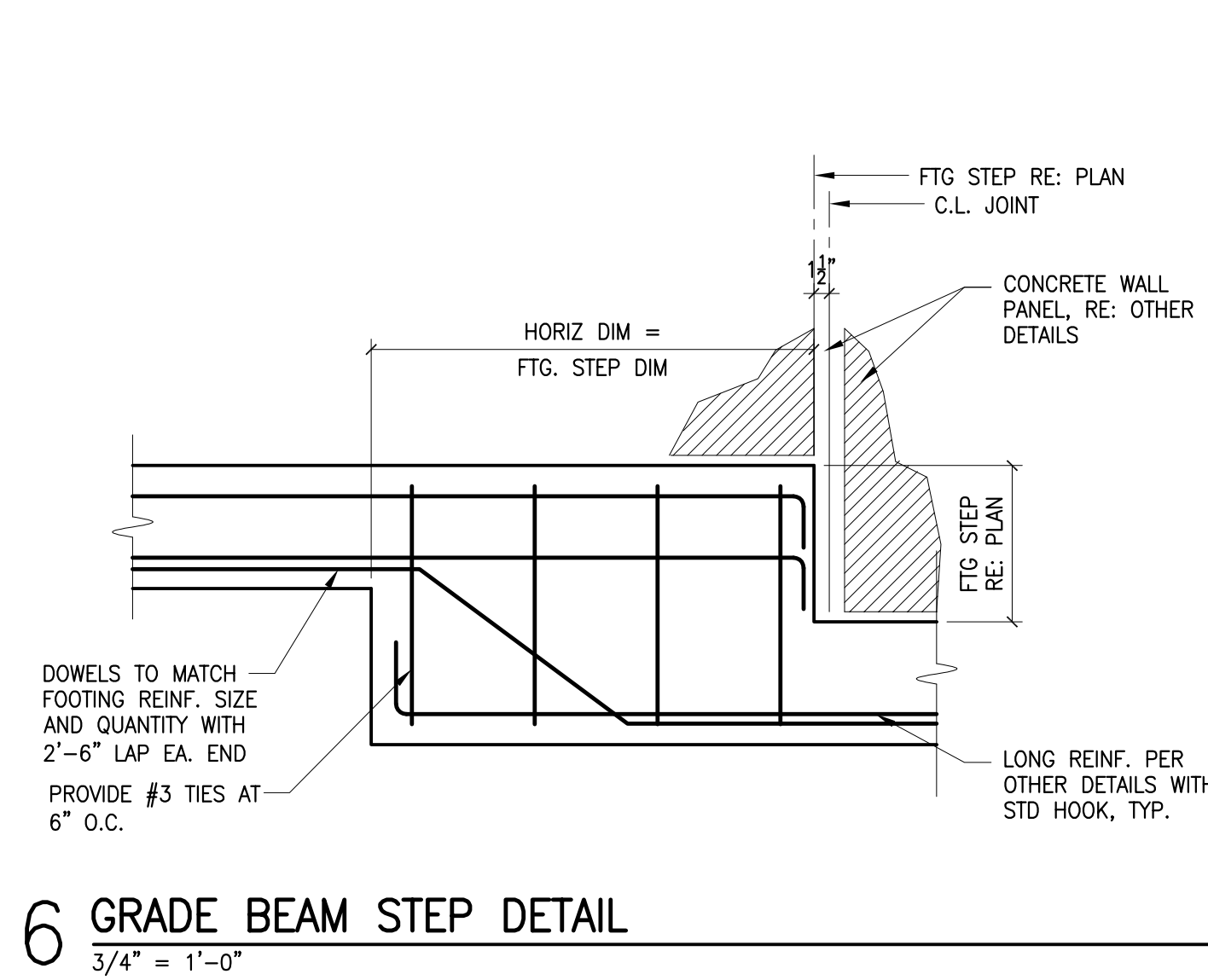
10 EXTERIOR FOOTING AT OVERFLOW DRAIN
3/4" = 1'-0"



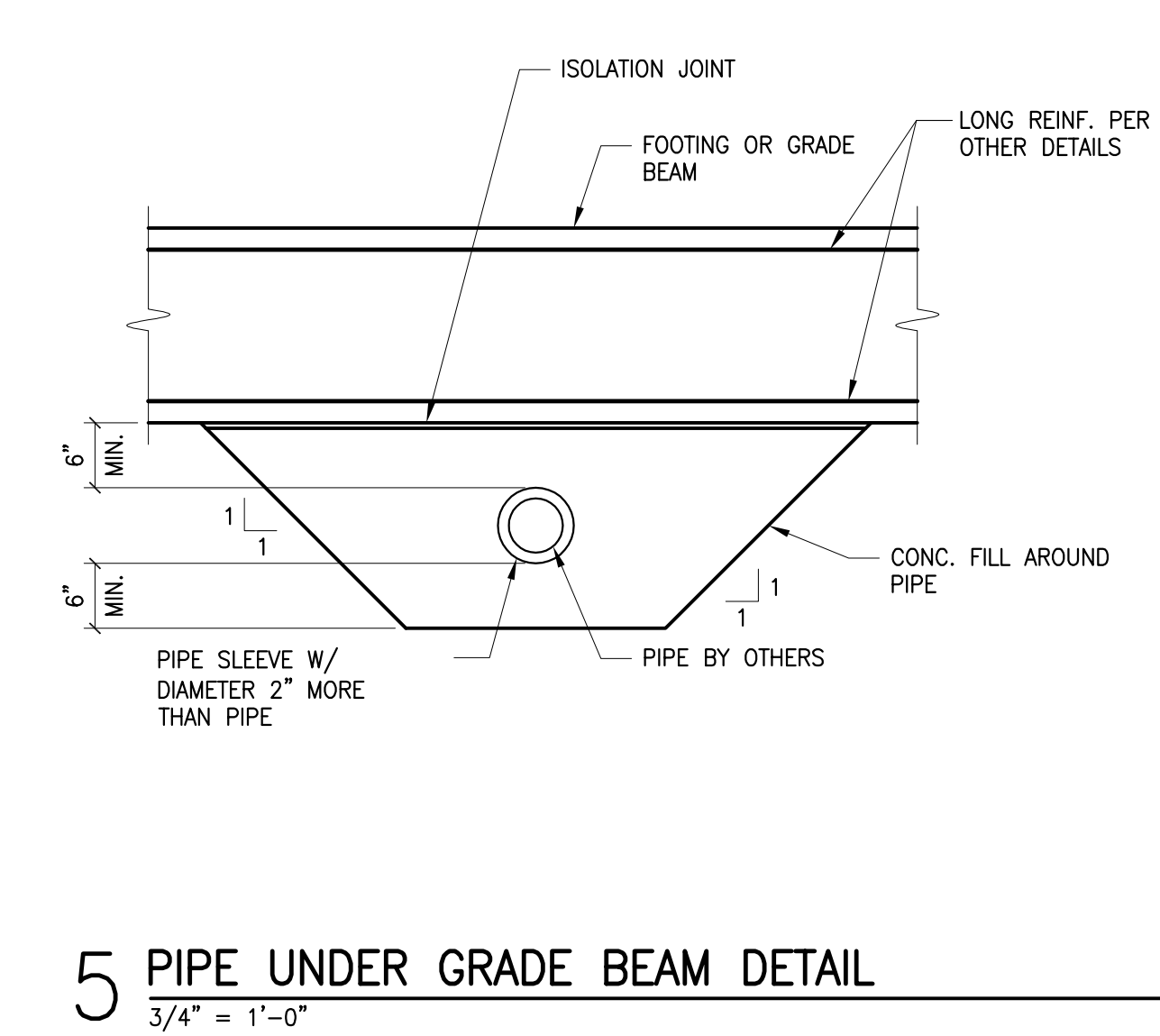
8 SLAB BLOCKOUT DETAIL
3/4" = 1'-0"



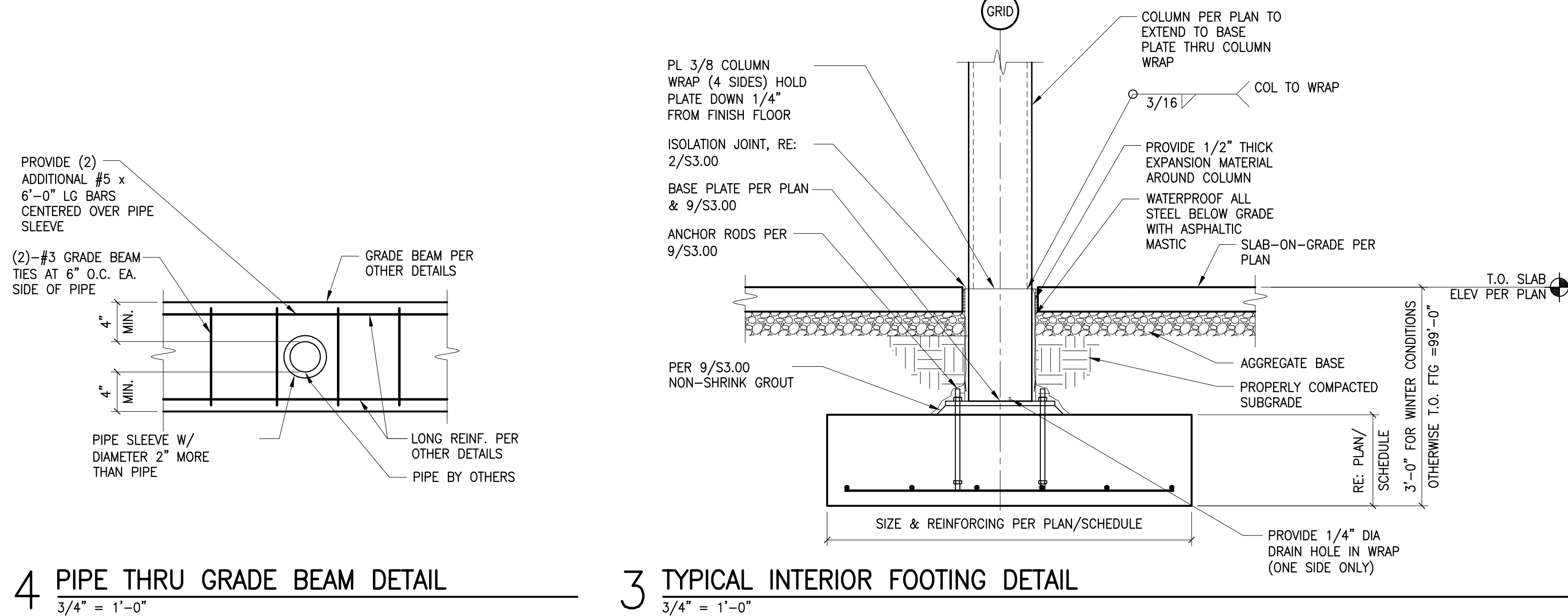
7 CORNER BAR DETAIL
3/4" = 1'-0"



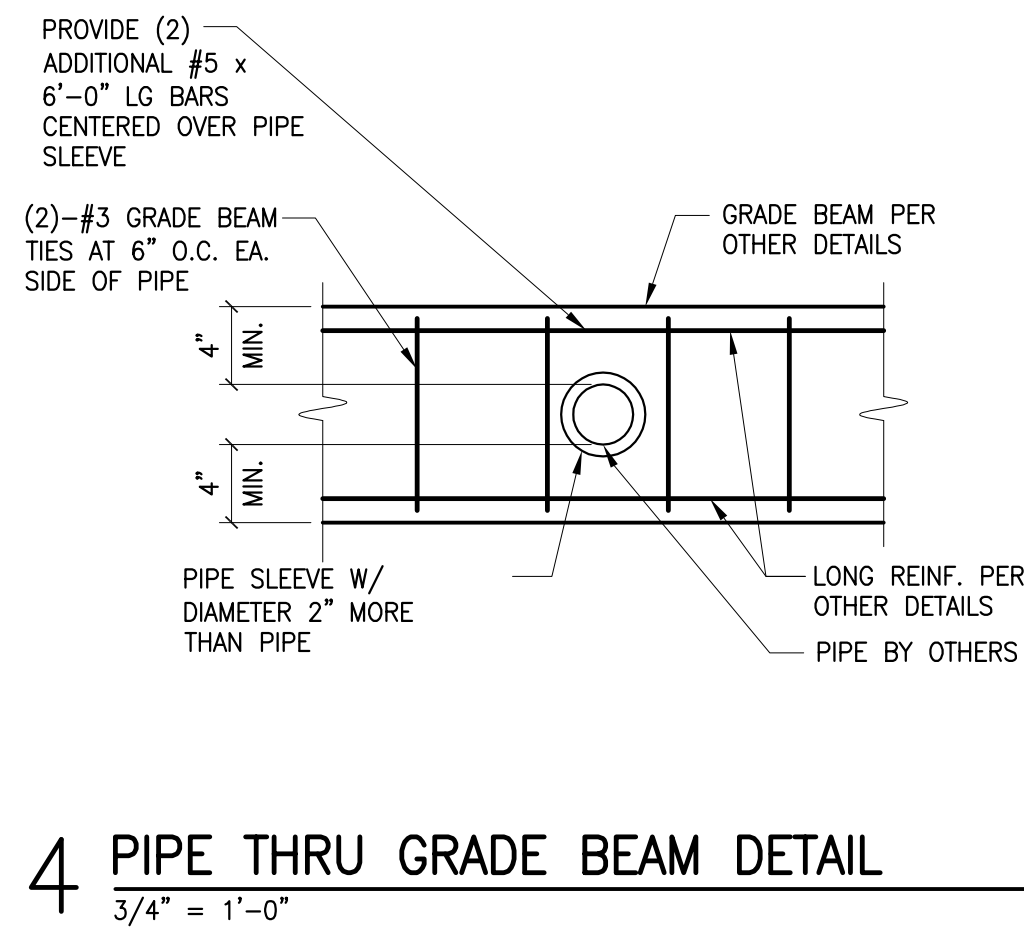
6 GRADE BEAM STEP DETAIL
3/4" = 1'-0"



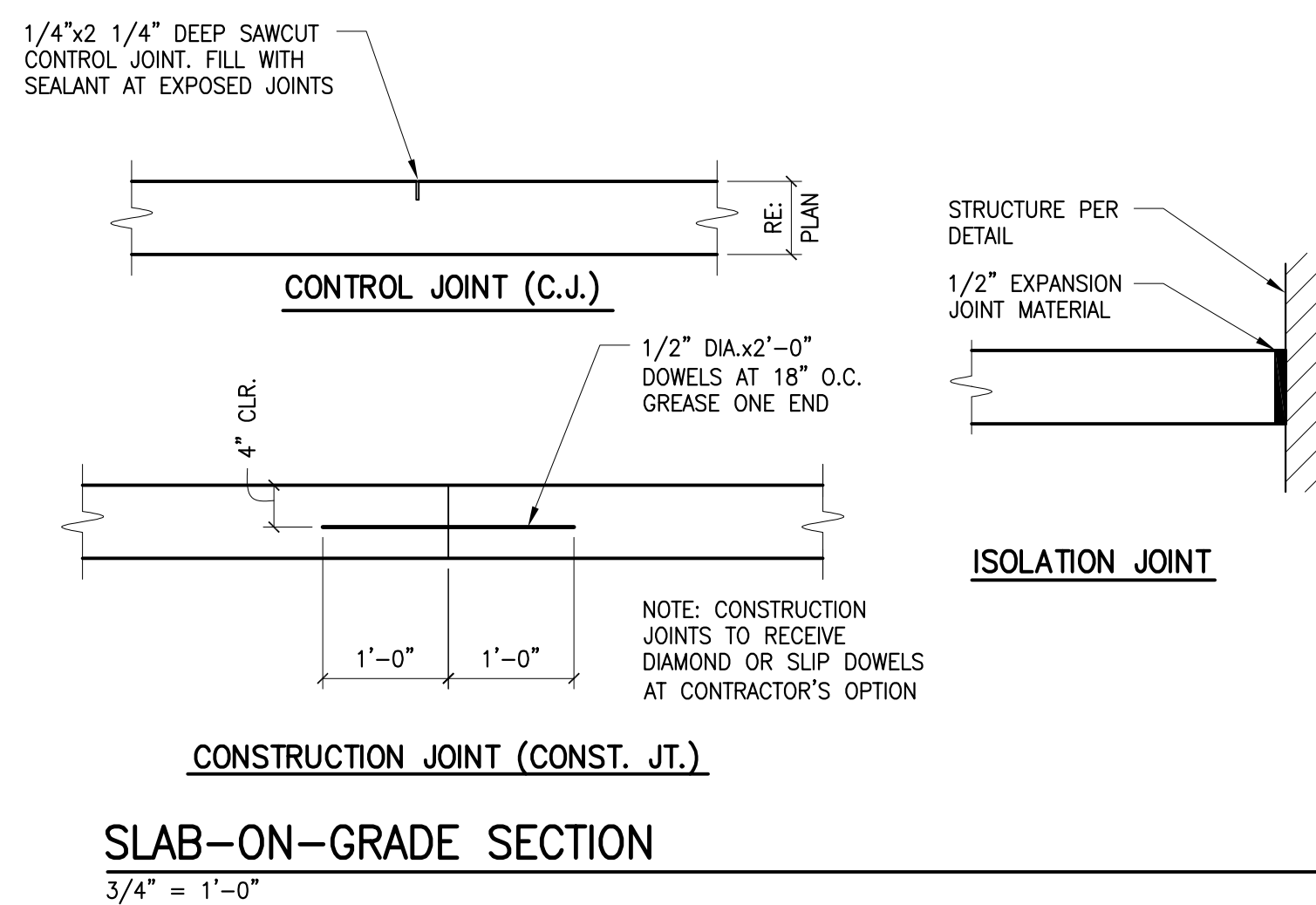
5 PIPE UNDER GRADE BEAM DETAIL
3/4" = 1'-0"



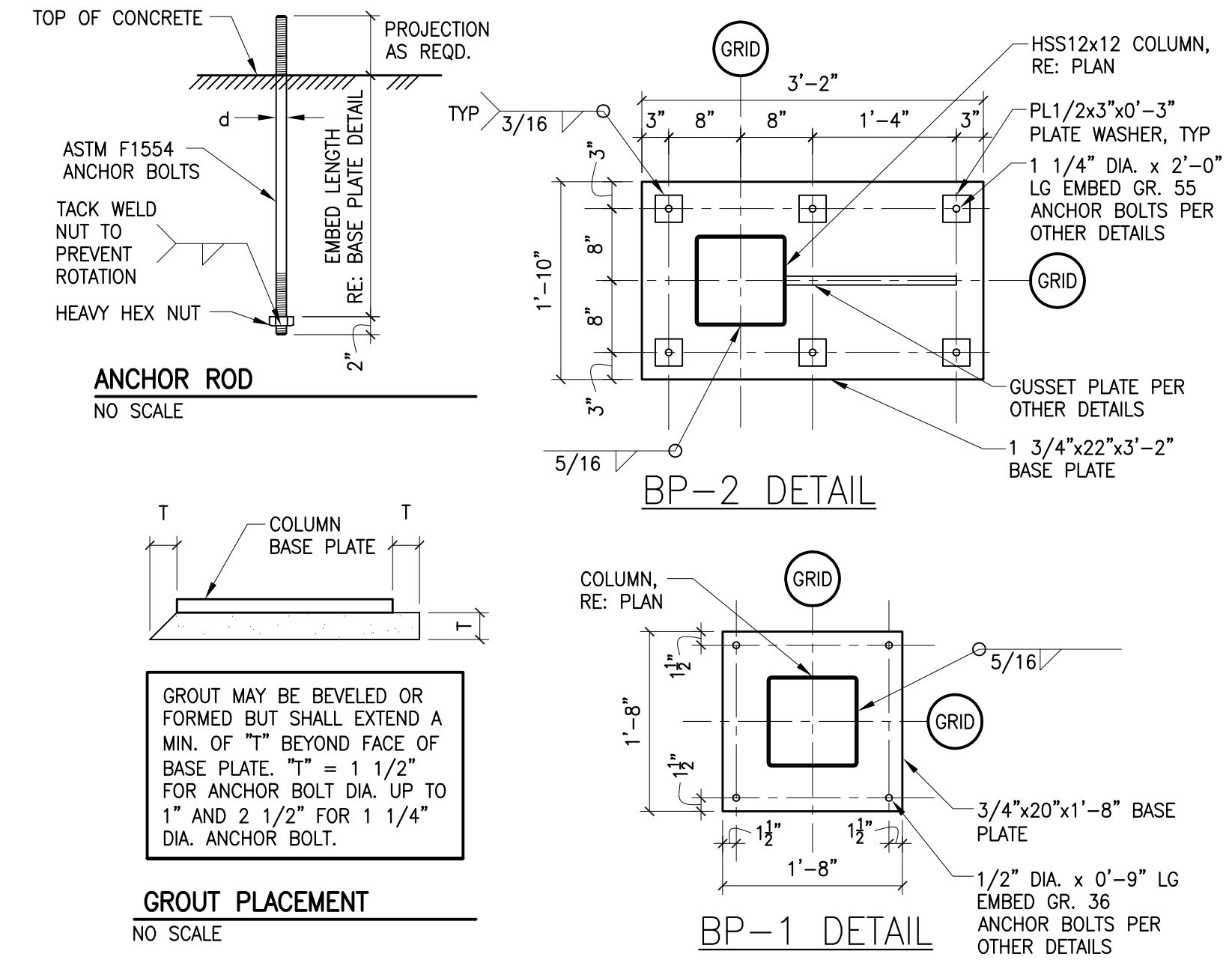
3 TYPICAL INTERIOR FOOTING DETAIL
3/4" = 1'-0"



4 PIPE THRU GRADE BEAM DETAIL
3/4" = 1'-0"



SLAB-ON-GRADE SECTION
3/4" = 1'-0"



9 ANCHOR ROD BASE PLATE DIAGRAMS AND SCHEDULE
3/4" = 1'-0"

STEEL REINF. LAP SCHEDULE (INCHES)						
BAR SIZE	CONCRETE					
	f'c = 3000 PSI		f'c = 4000 PSI		f'c = 5000 PSI	
	TOP	OTHER	TOP	OTHER	TOP	OTHER
#3	22	17	20	16	17	13
#4	29	22	27	21	23	17
#5	36	28	33	26	28	22
#6	43	33	40	31	34	26
#7	63	48	58	45	49	38
#8	72	55	66	51	56	43
#9	91	70	79	61	71	54

1 CONC. LAP SCHEDULE
3/4" = 1'-0"



CURRAN
ARCHITECTURE
5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317.288.0681
F :: 317.288.0753



wallace
Wallace Engineering
Structural Consultants, Inc.
Structural and Civil Consultants
1741 McGee Street
Kansas City, Missouri 64108
816.421.8282, Fax 816.421.8338

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PROJECT INFORMATION

LEE'S SUMMIT LOGISTICS
BUILDING B LOT 2
NW CORNER TUDOR RD & MAINST
LEE'S SUMMIT, MO

ISSUE DATES

ISSUE DATE
ISSUE FOR PERMIT 04.22.2022

210300

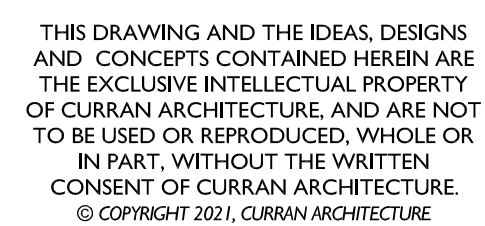
S3.0
FOUNDATION DETAILS



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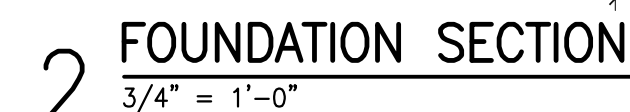
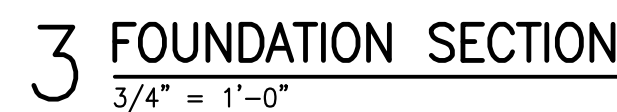
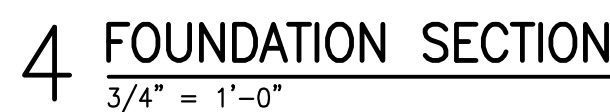


LEE'S SUMMIT LOGISTICS
BUILDING B LOT 2

NW CORNER TUDOR RD & MAINST
LEE'S SUMMIT, MO

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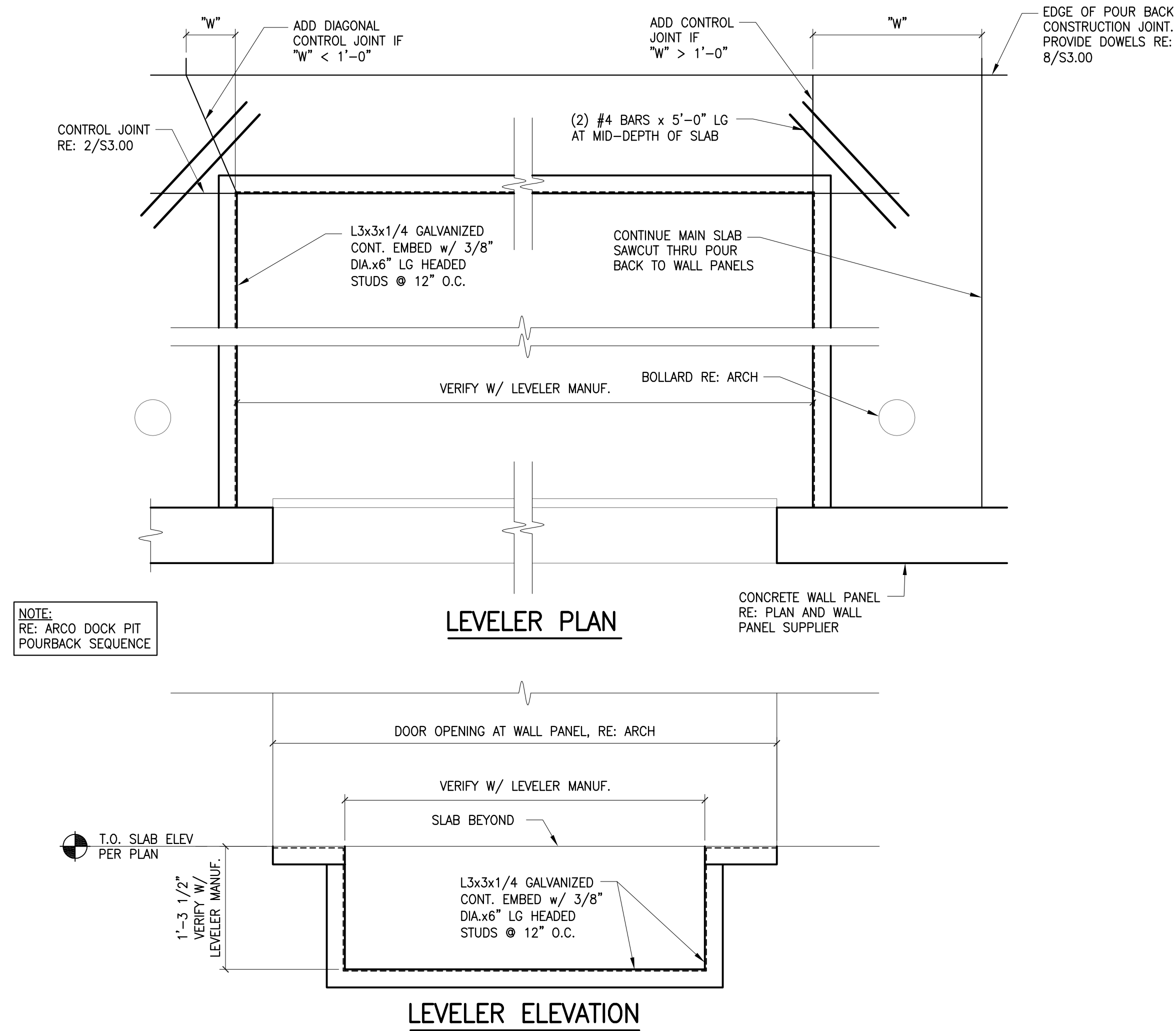
FOUNDATION DETAILS


$$3/4'' = 1' - 0''$$


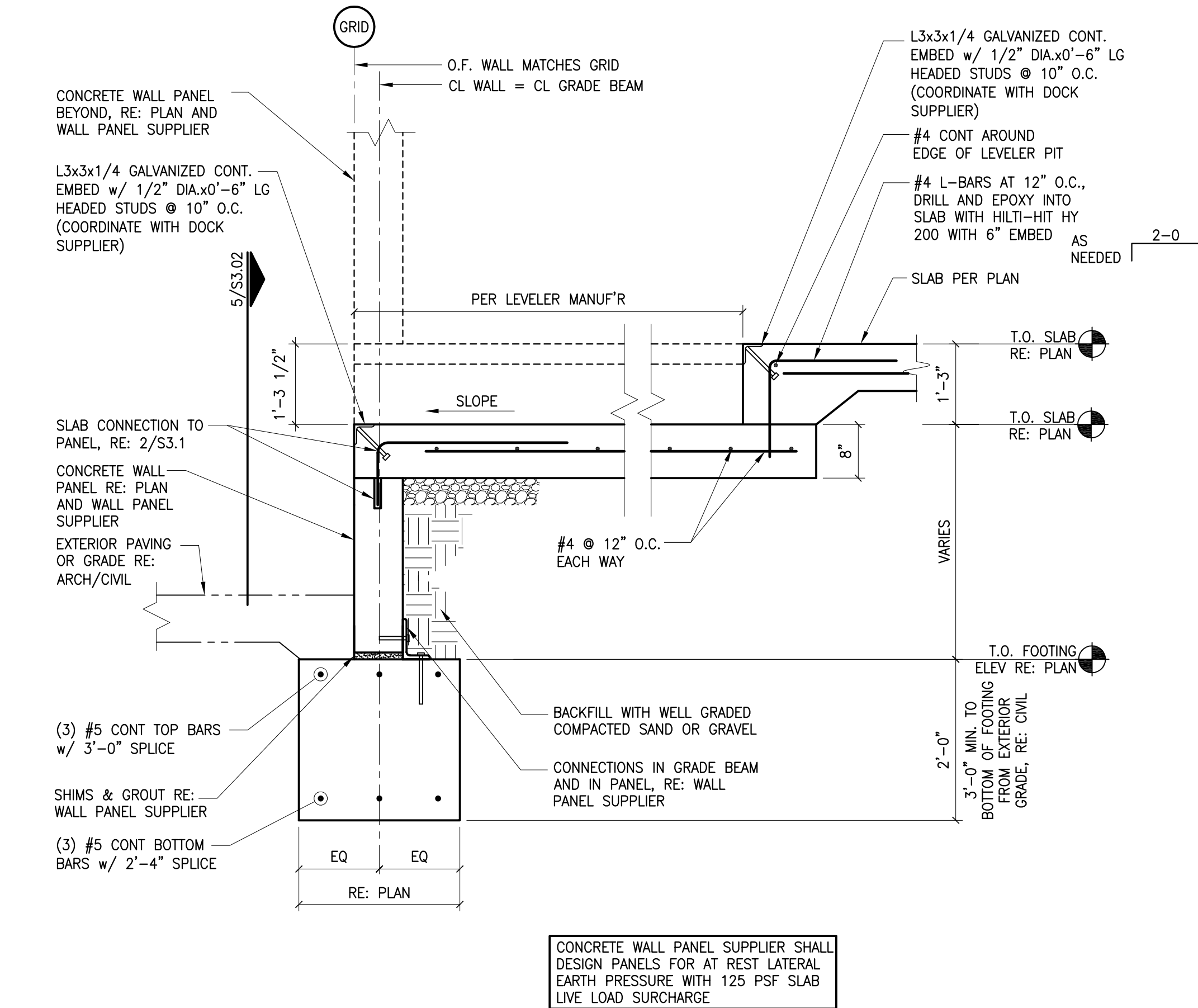
STRINGER: GALV. C12x20.7
HEADER: GALV. C12x20.7
TREADS: 1 1/4" DEEP BAR GRATING
LANDING DECK: 1 1/4" DEEP BAR GRATING



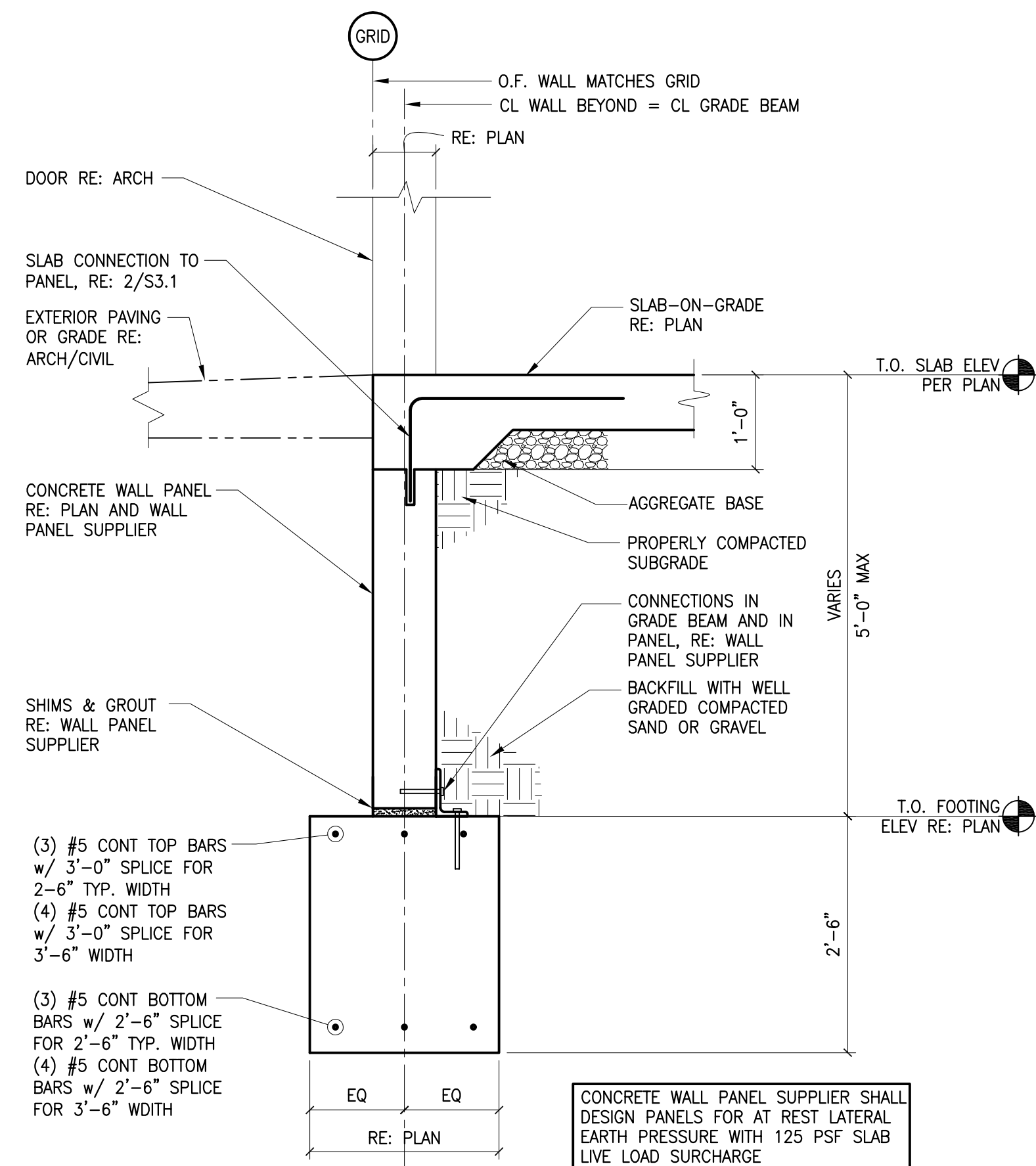
1 STAIR DETAILS
3/4" = 1'-0"



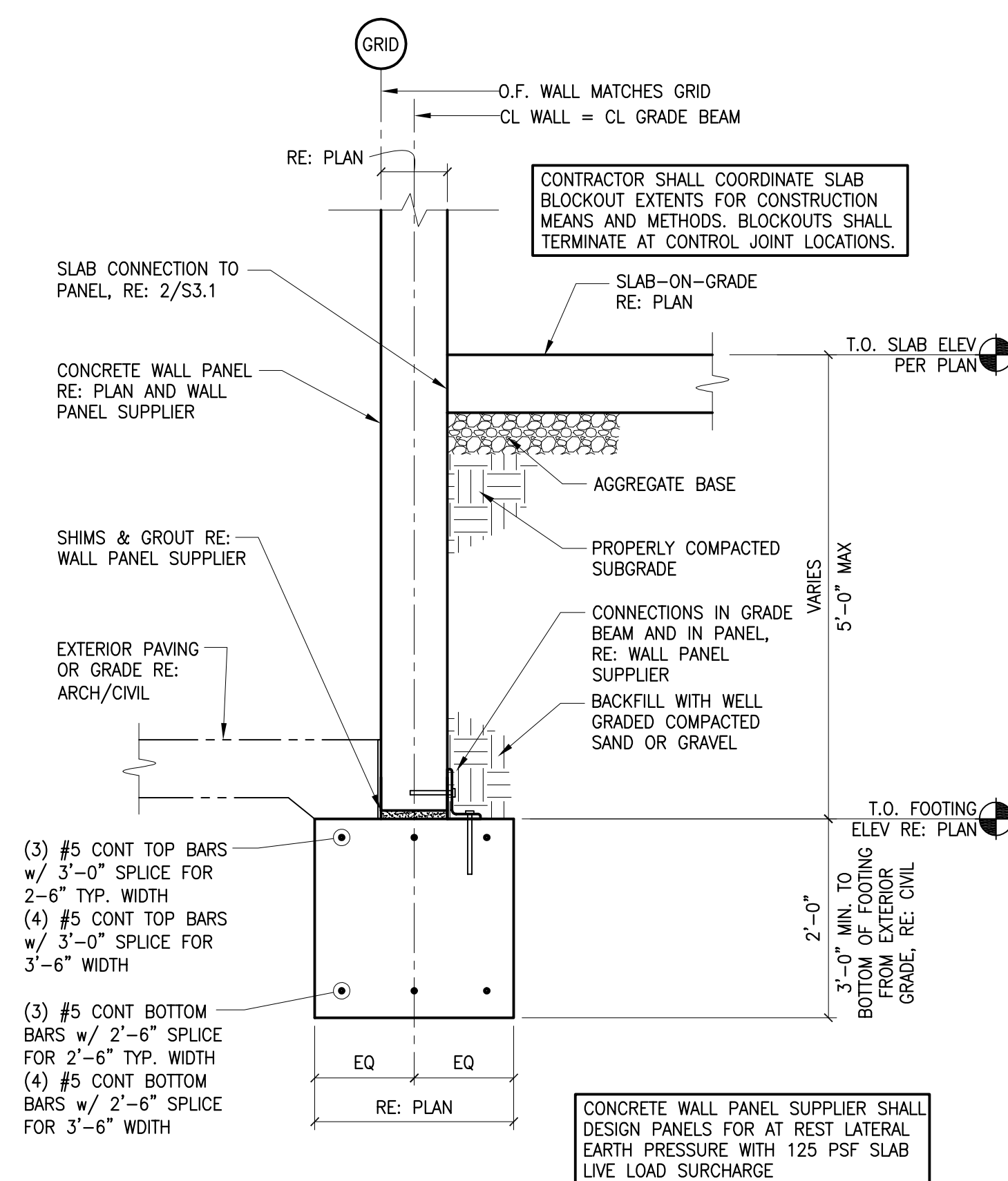
5 DOCK PIT SECTION
3/4" = 1'-0"



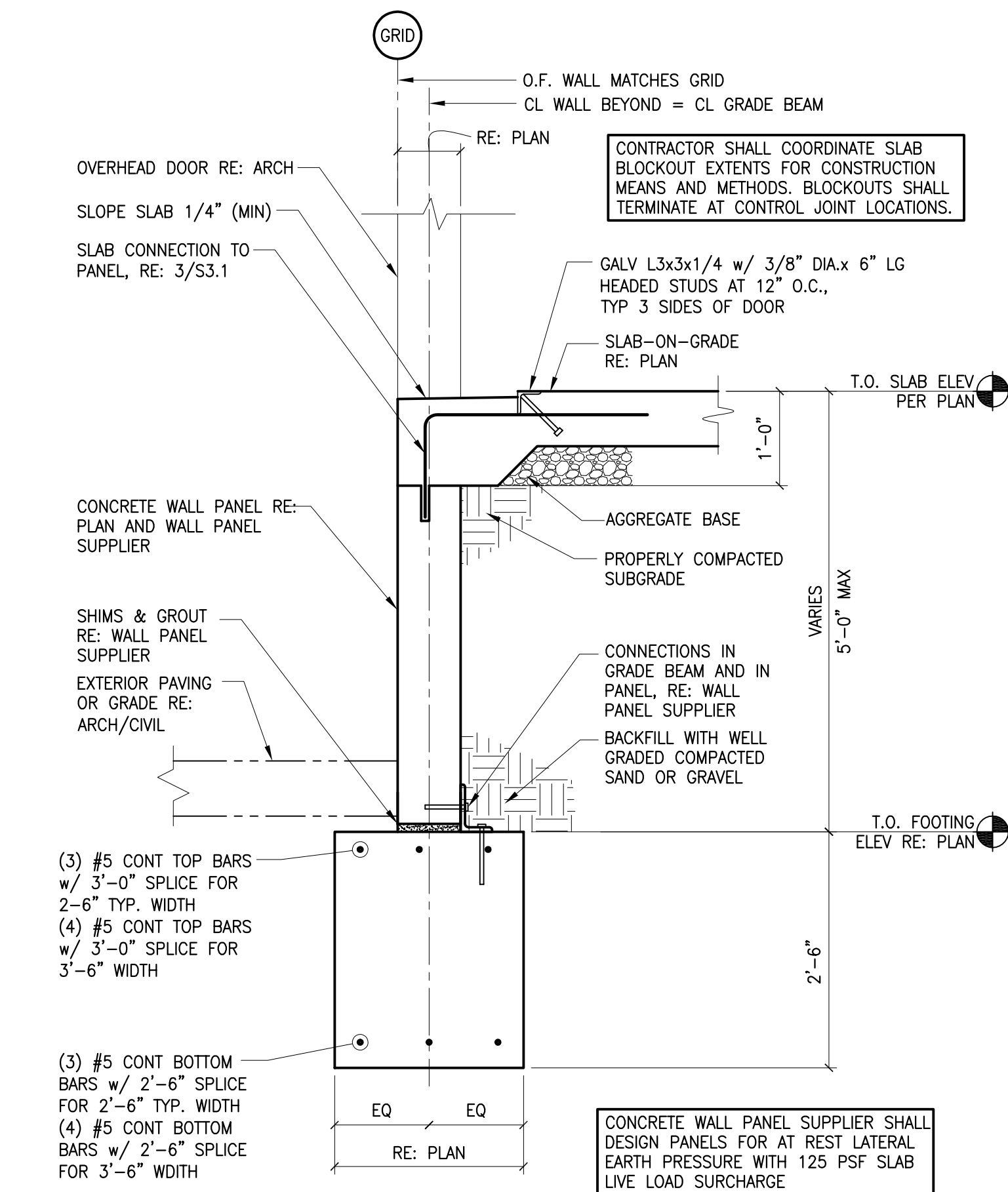
4 FOUNDATION SECTION
3/4" = 1'-0"



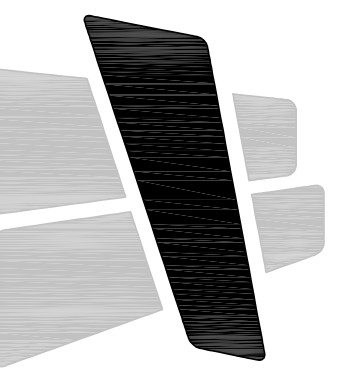
3 FOUNDATION SECTION
3/4" = 1'-0"



2 FOUNDATION SECTION AT DOCK WALL
3/4" = 1'-0"



1 FOUNDATION SECTION AT OVERHEAD DOOR
3/4" = 1'-0"



CURRAN
ARCHITECTURE

5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317 . 288 . 0681
F :: 317 . 288 . 0753



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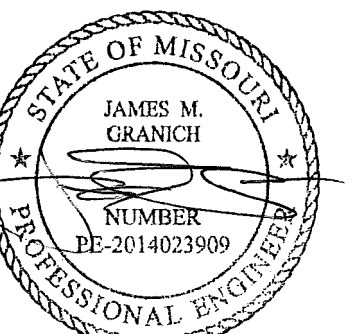
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Wallace Engineering
Structural Consultants, Inc.

Structural and Civil Consultants
1741 McGee Street
Kansas City, Missouri 64108
816.421.8282, Fax 816.421.8338

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PROJECT INFORMATION

**LEE'S SUMMIT LOGISTICS
BUILDING B LOT 2**

NW CORNER TUDOR RD & MAINST
LEE'S SUMMIT, MO

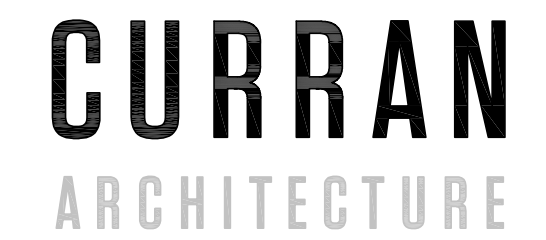
ISSUE DATES

ISSUE	DATE
ISSUE FOR PERMIT	04.22.2022

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S3.2

FOUNDATION DETAILS



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Wallace Engineering
Structural Consultants, Inc.

Structural and Civil Consultants
1741 McCreary Street
Kansas City, Missouri 64108
816.421.8282, Fax 816.421.8338

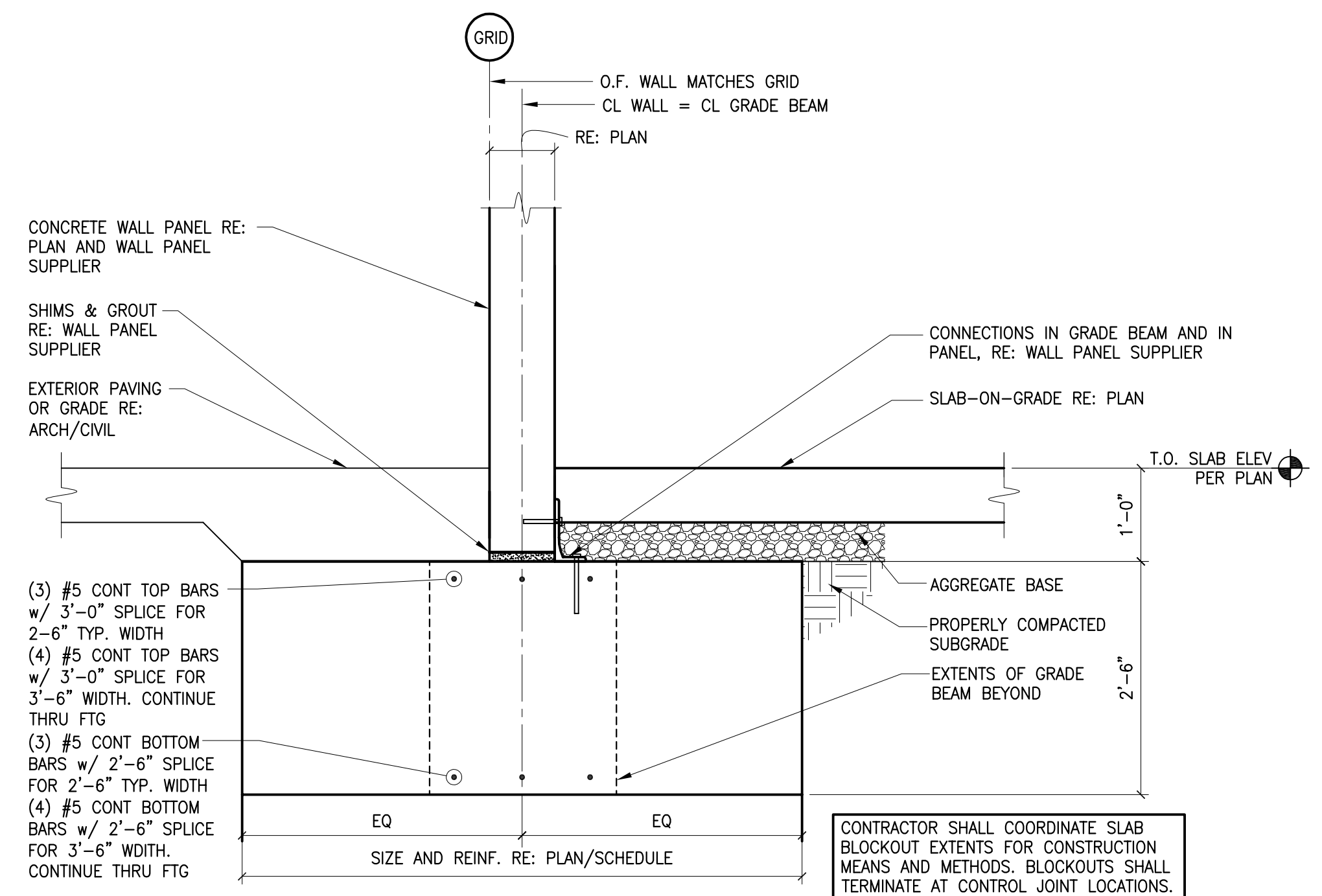
STATE OF MISSOURI
JAMES M. GRANICH
NUMBER
PE-2014023909
PROFESSIONAL ENGINEER

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LEE'S SUMMIT, MO

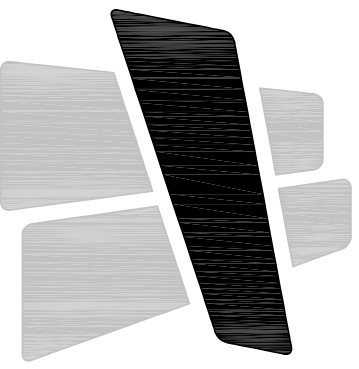
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FOUNDATION DETAILS



2 FOUNDATION SECTION
3/4" = 1'-0"

1 FOUNDATION SECTION
3/4" = 1'-0"



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ARCHITECTURE

5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317 . 288 . 0681
F :: 317 . 288 . 0753

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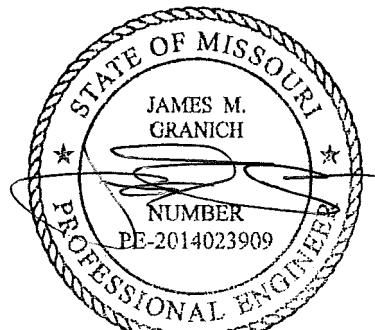
Structural and Civil Consultants

1741 McGee Street

Kansas City, Missouri 64108

816.421.8282, Fax 816.421.8338

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PROJECT INFORMATION

LEE'S SUMMIT LOGISTICS
BUILDING B LOT 2

NW CORNER TUDOR RD & MAINST
LEE'S SUMMIT, MO

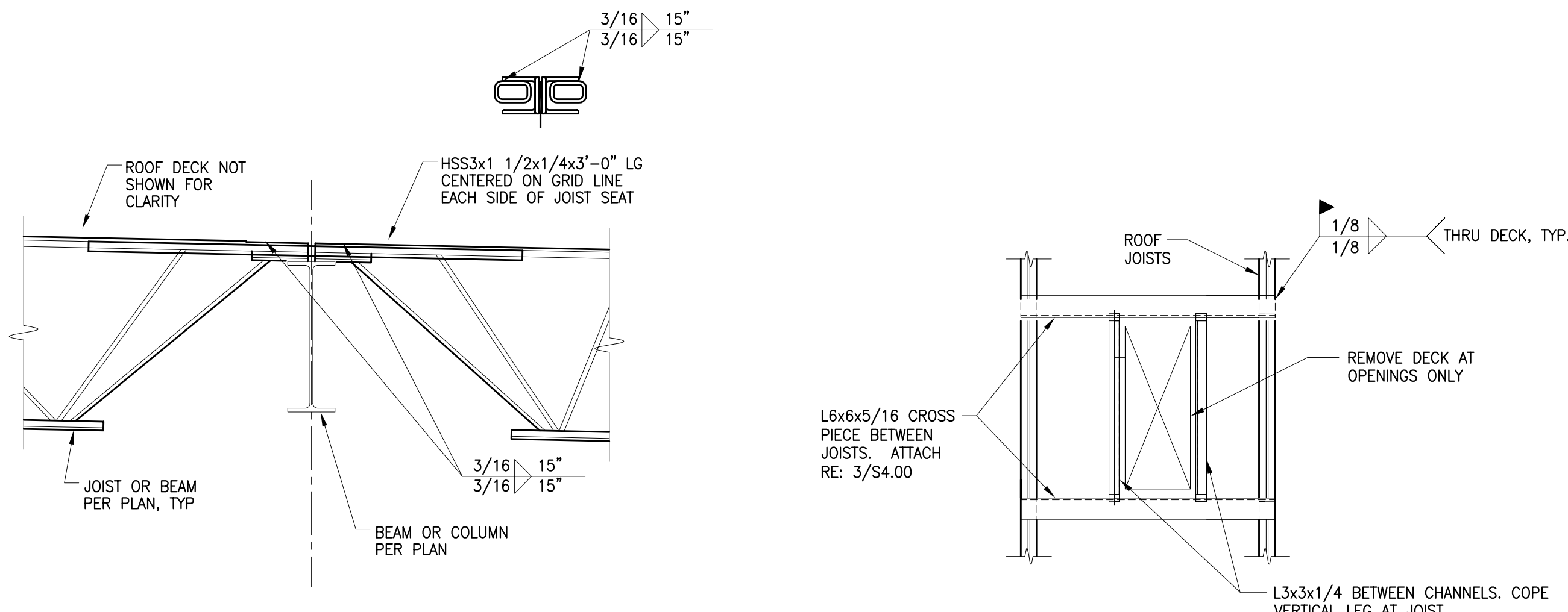
ISSUE DATES

ISSUE	DATE
ISSUE FOR PERMIT	04.22.2022

210300

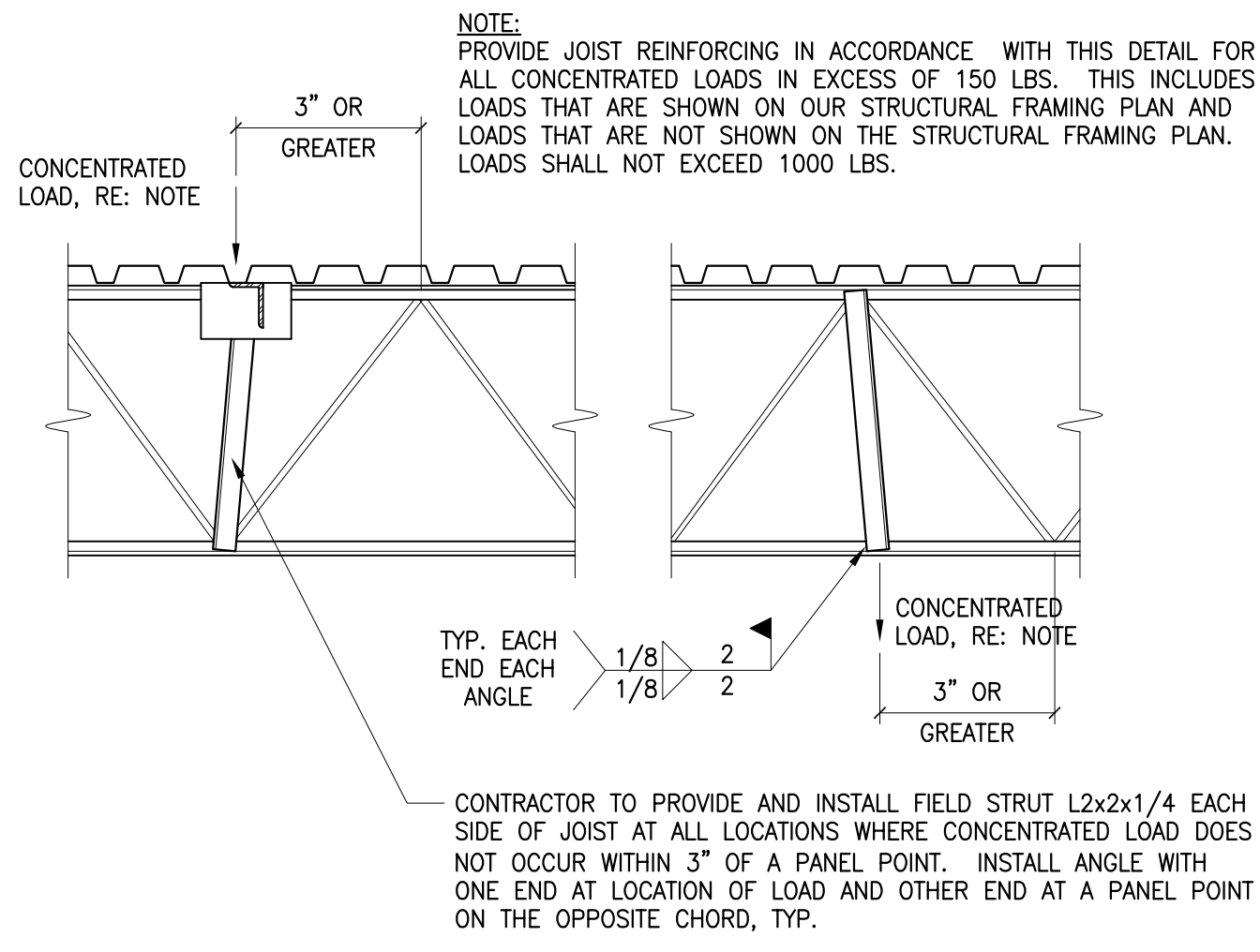
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FRAMING DETAILS

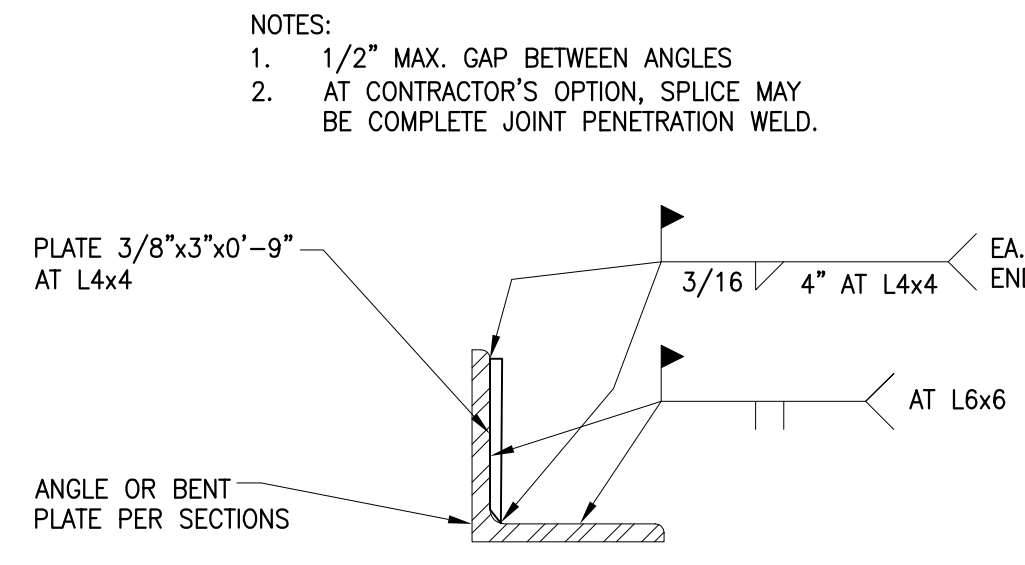


9 TYPICAL DRAG STRUT DETAIL AT EXPANSION JOINT
3/4" = 1'-0"

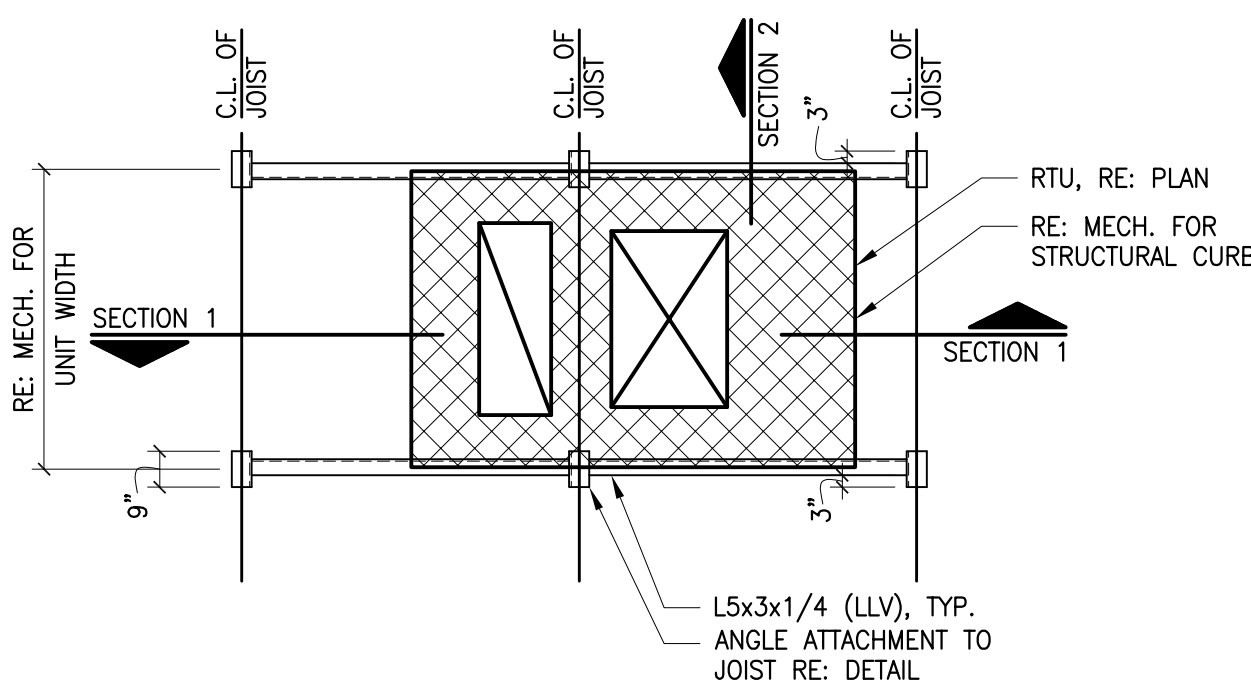
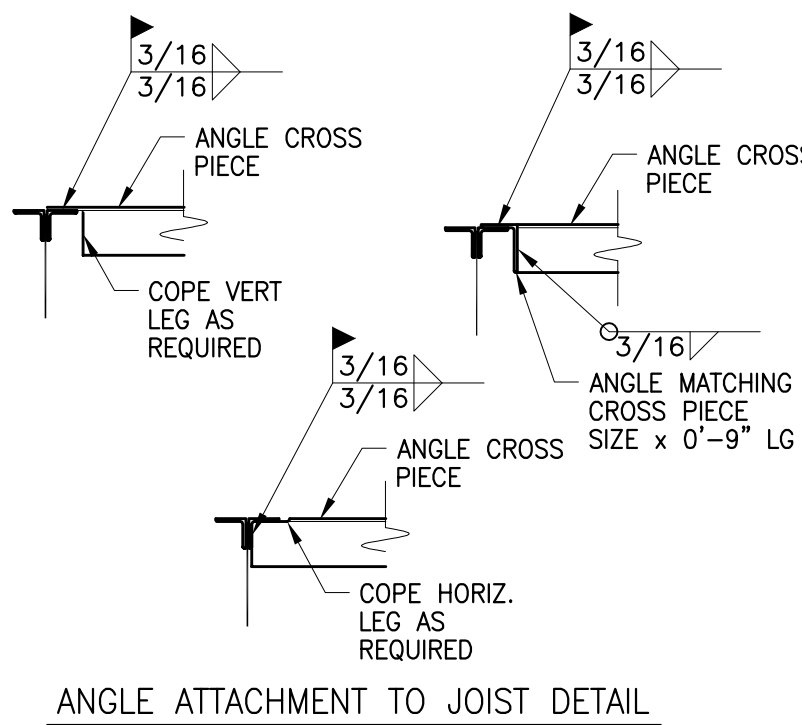
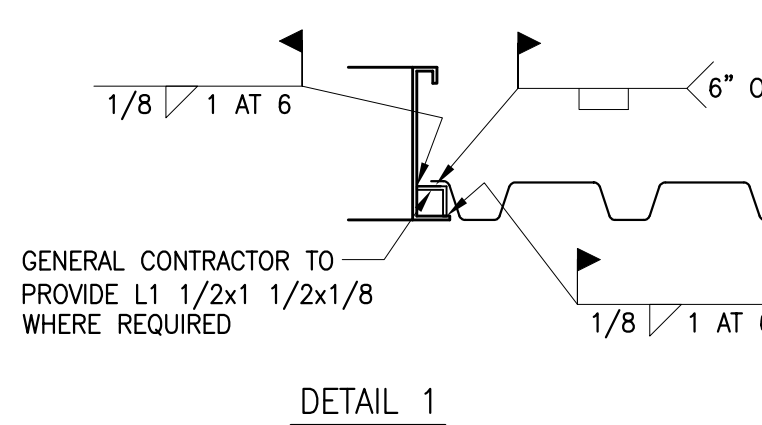
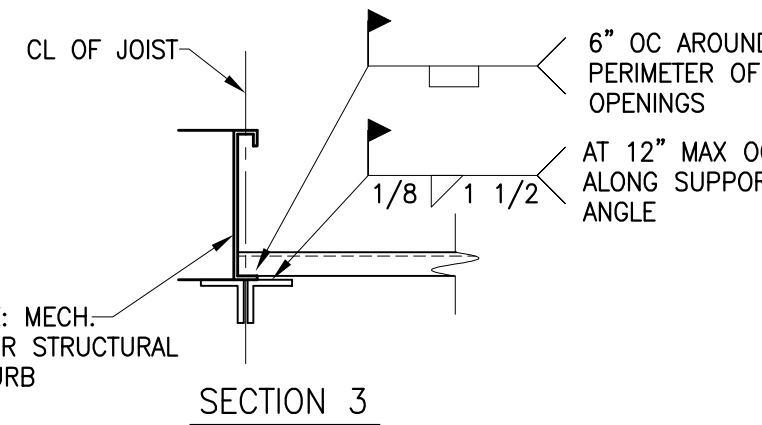
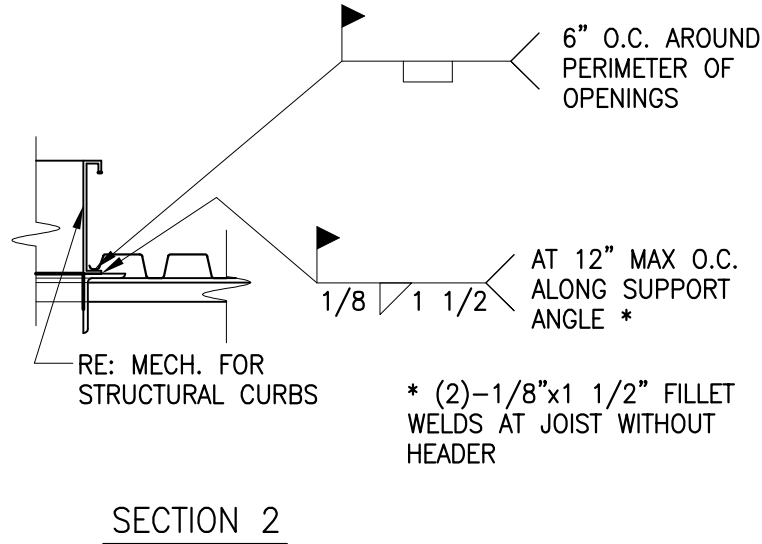
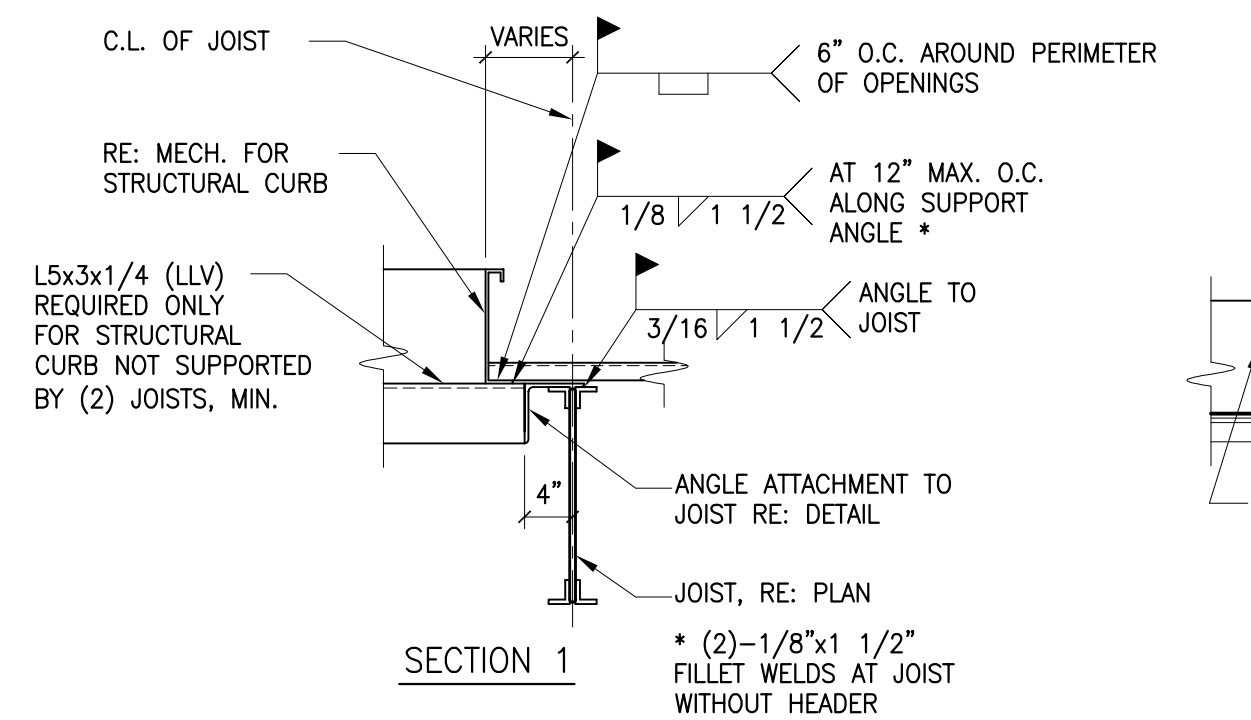
8 ROOF OPENING DETAIL
3/4" = 1'-0"



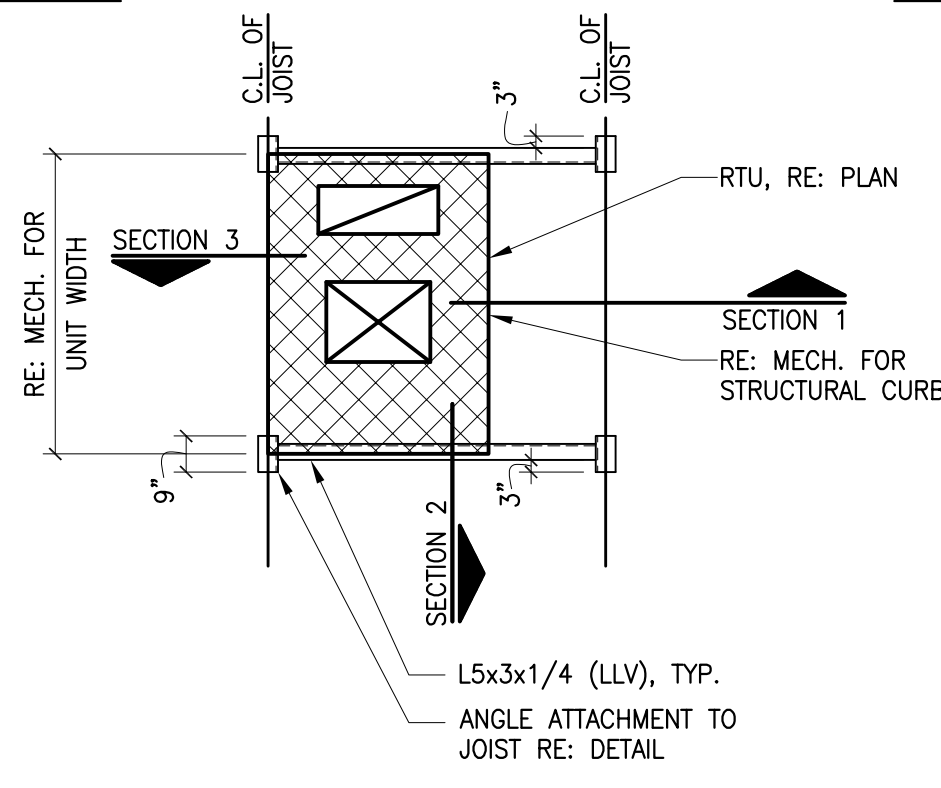
7 JOIST REINFORCING DETAIL
3/4" = 1'-0"



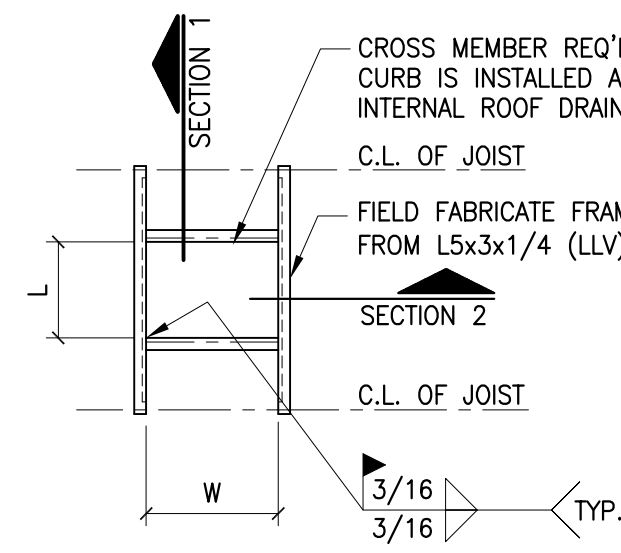
6 SPLICE DETAIL
3/4" = 1'-0"



TYP. AT UNIT SPANNING MULTIPLE JOISTS



TYP. AT UNIT BETWEEN JOISTS

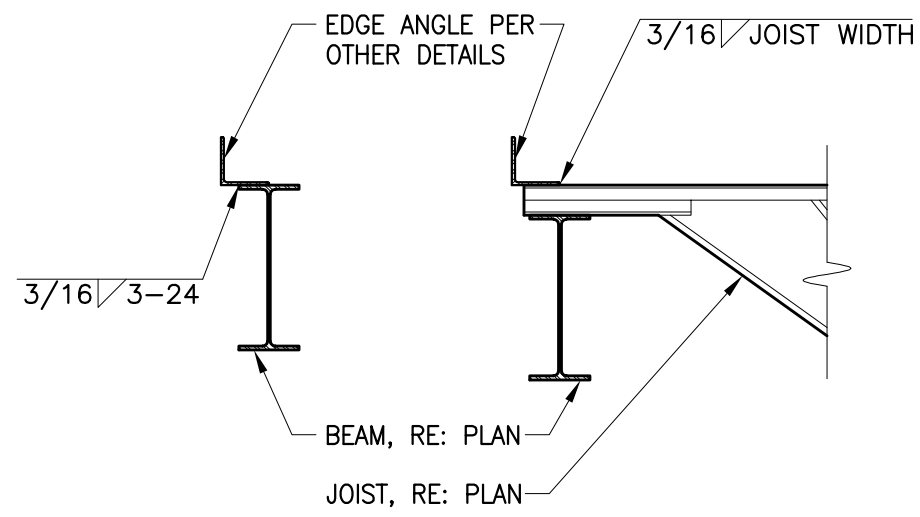


OPENING IN ROOF LARGER THAN 10"x10"

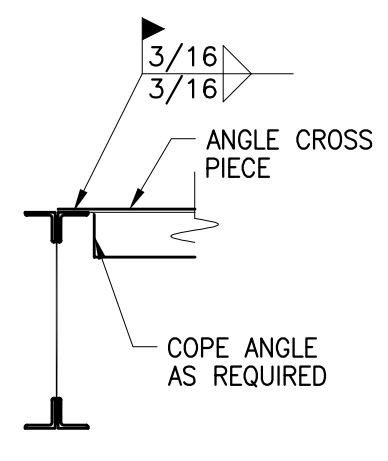
NOTES:
1. INSTALL CURBS, HEADERS, AND FRAMES AND WELD TO SUPPORT STEEL BEFORE DECK IS PLACED.
2. DESIGN JOISTS SUPPORTING RTU'S FOR TWO POINT LOADS. THE LOCATION OF THE LOADS AND THE SPACING BETWEEN THEM VARY. RE: RTU JOIST DIAGRAM THIS DETAIL AND ROOF FRAMING PLAN FOR POINT LOADS AND LOCATIONS.
3. RTU CURBS SHALL BE STRUCTURAL, DESIGNED TO SPAN BETWEEN JOISTS AND SUPPORT EDGES OF DECK. CURBS TO BE FABRICATED WITH LEDGE ANGLES (L2x2x1/4) AT MECHANICAL OPENINGS TO SUPPORT METAL DECK INSIDE OPENING NOT USED BY SUPPLY OR RETURN DUCT WORK. HEADERS ARE NOT REQUIRED FOR STRUCTURAL CURBS EXCEPT WHEN THE CURB DOES NOT SPAN BETWEEN TWO JOISTS OR THE CURB CANTILEVERS MORE THAN TWO FEET PAST JOIST.
4. ATTACH DECK AROUND OPENING PER ROOF DIAPHRAGM CONNECTION DETAIL.
5. IF CURB IS NOT PLACED WITHIN 3" OF A JOIST PANEL POINT, RE: JOIST REINFORCING DETAIL RE: 7/S4.00.
6. GENERAL CONTRACTOR SHALL COORDINATE RTU DIMENSIONS AND FRAMING LOCATIONS WITH THE STEEL FABRICATOR, MECHANICAL, AND ERECTION SUBCONTRACTORS.
7. STEEL SUPPLIER TO FURNISH STOCK ANGLE FOR FIELD FABRICATED SUPPORT FRAMES.
8. RE: DETAIL 1 FOR CONN. OF DECK PARALLEL TO CURB (WHERE REQ'D.).
9. RE: MECH. FOR ROOF TOP UNIT ANCHORAGE TO CURBS.

NOTE:

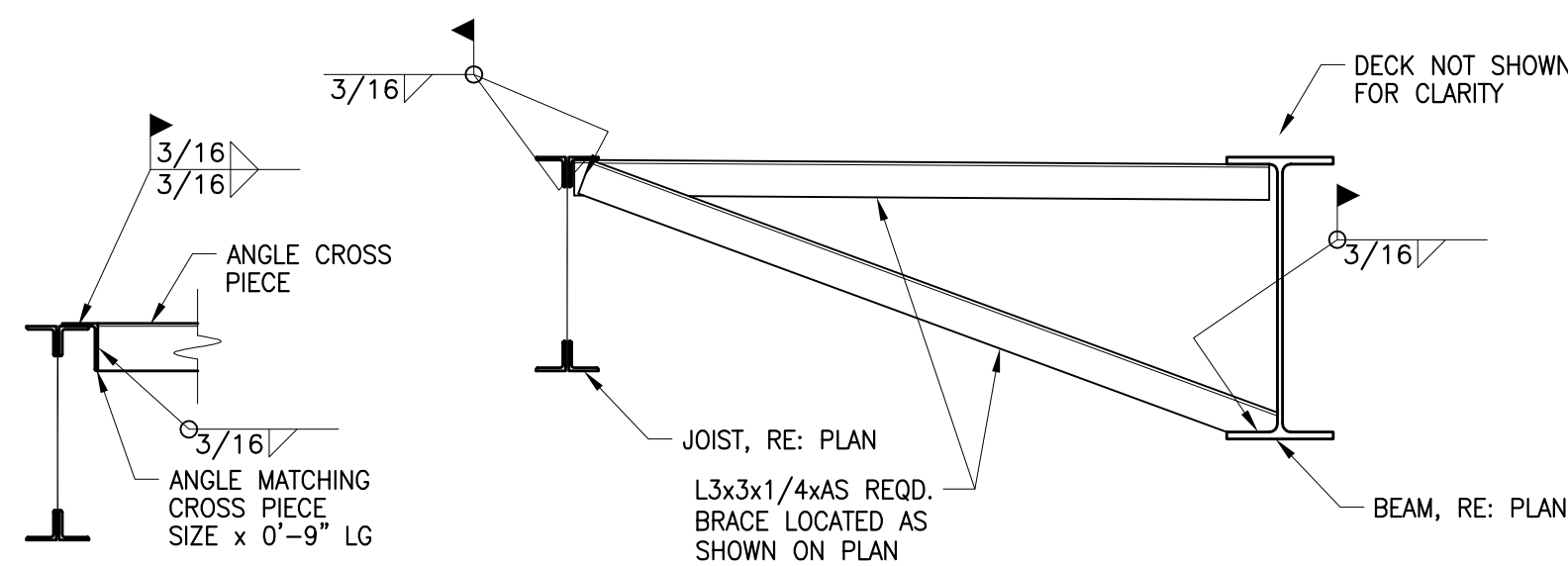
AT CONTRACTOR'S OPTION, WELDS MAY BE FIELD WELDS.



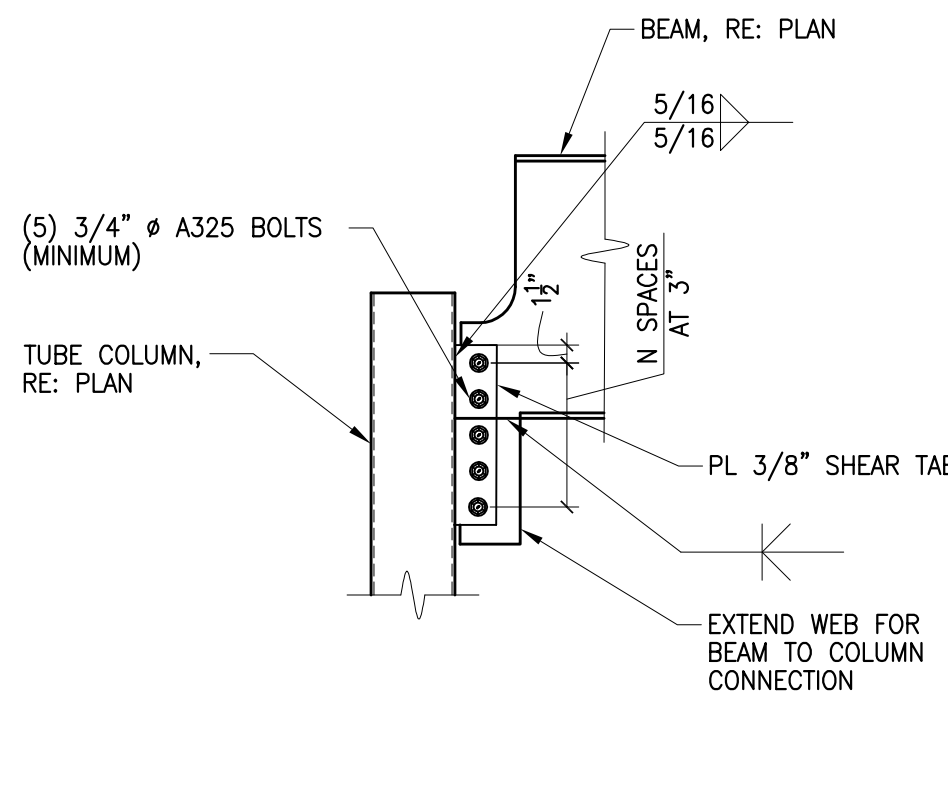
4 EDGE ANGLE CONNECTION DETAIL
3/4" = 1'-0"



3 ANGLE CONNECTION DETAILS
3/4" = 1'-0"



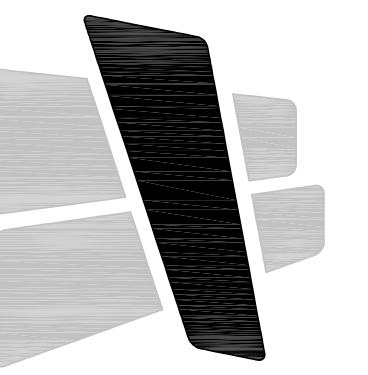
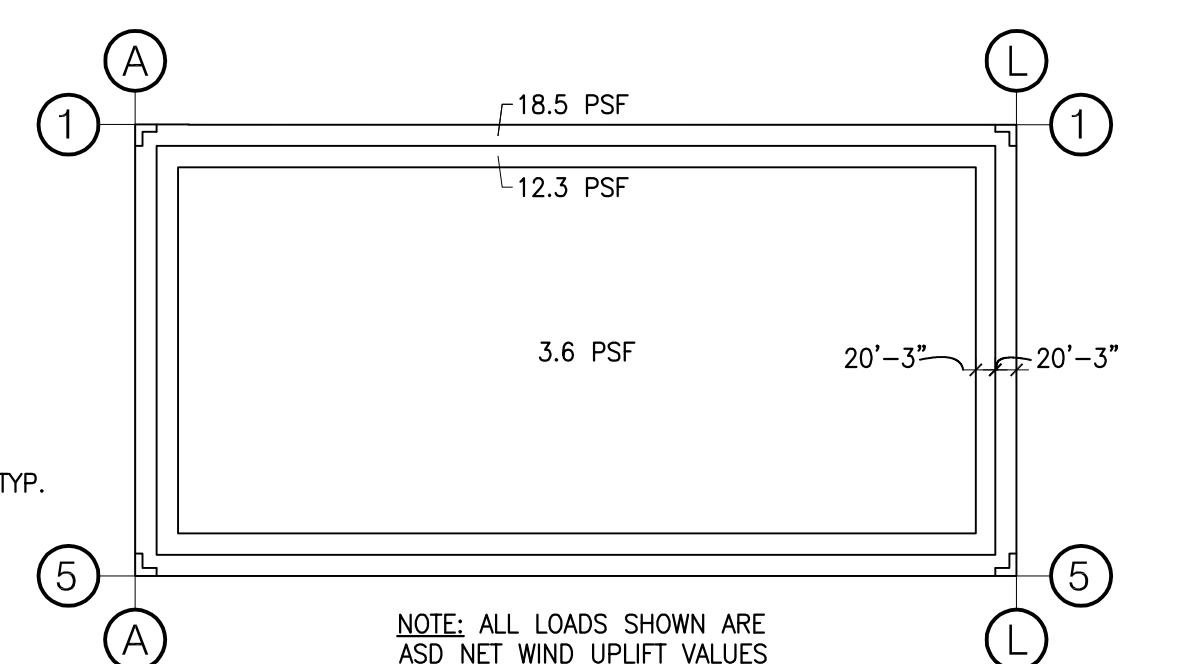
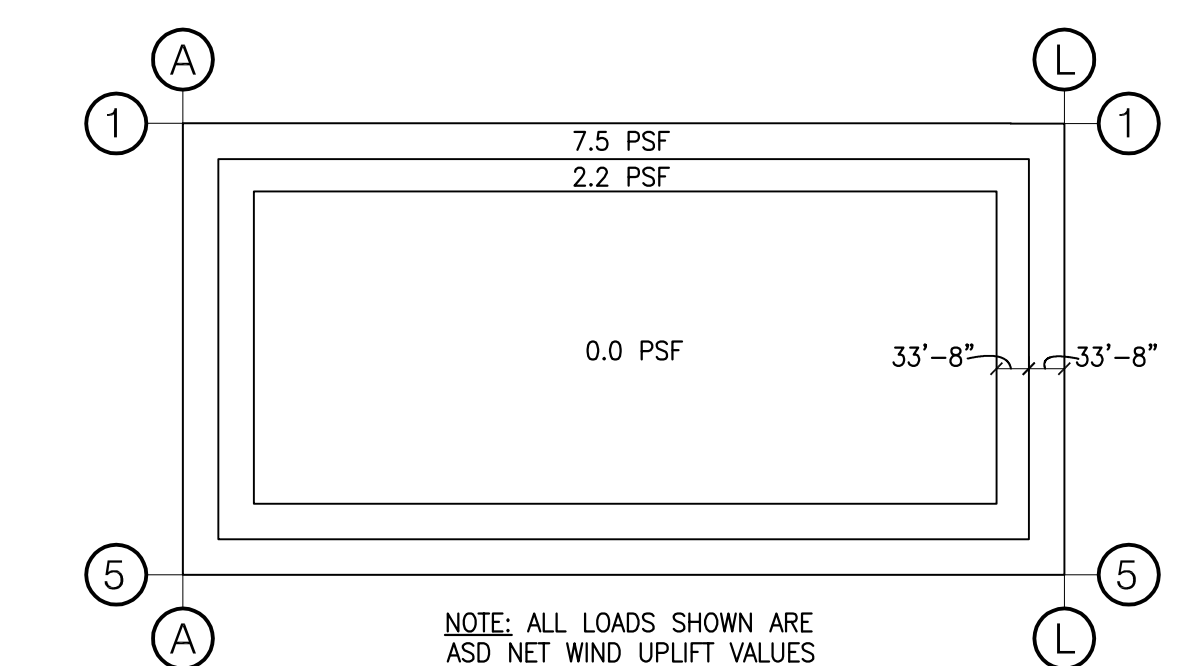
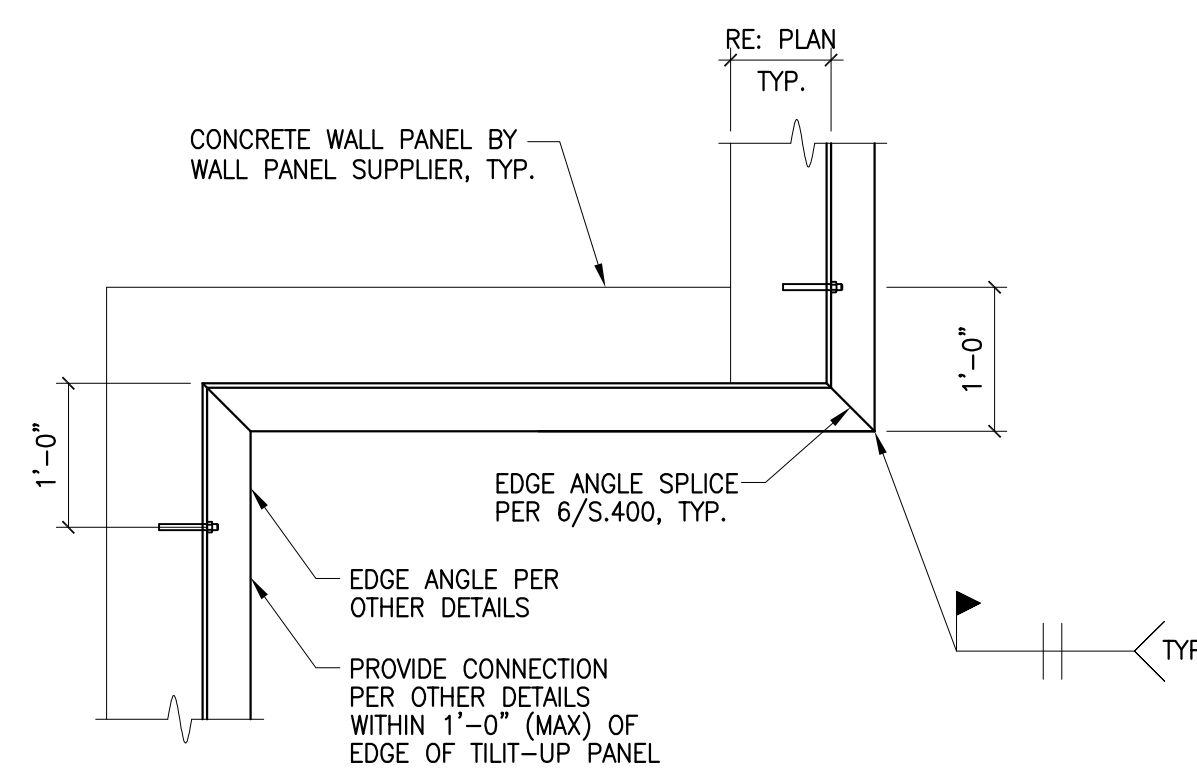
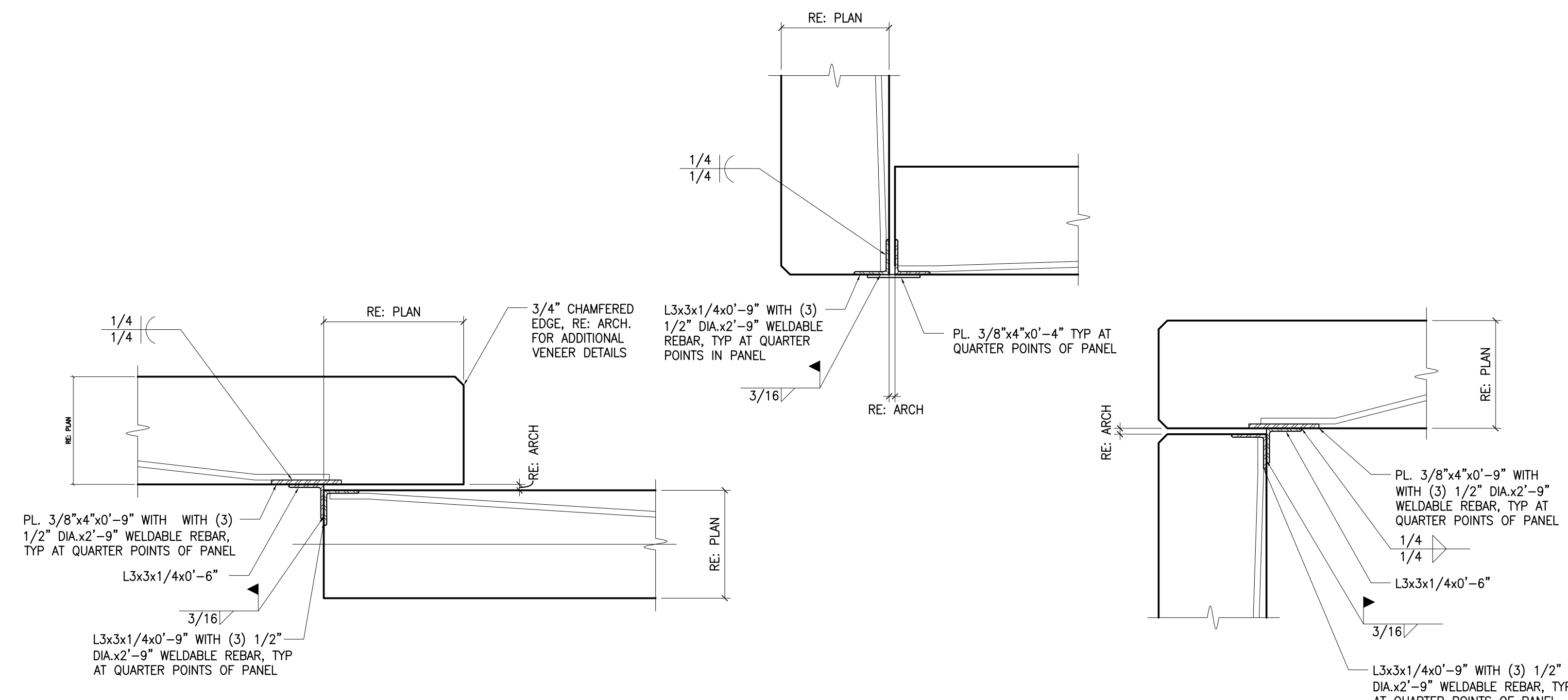
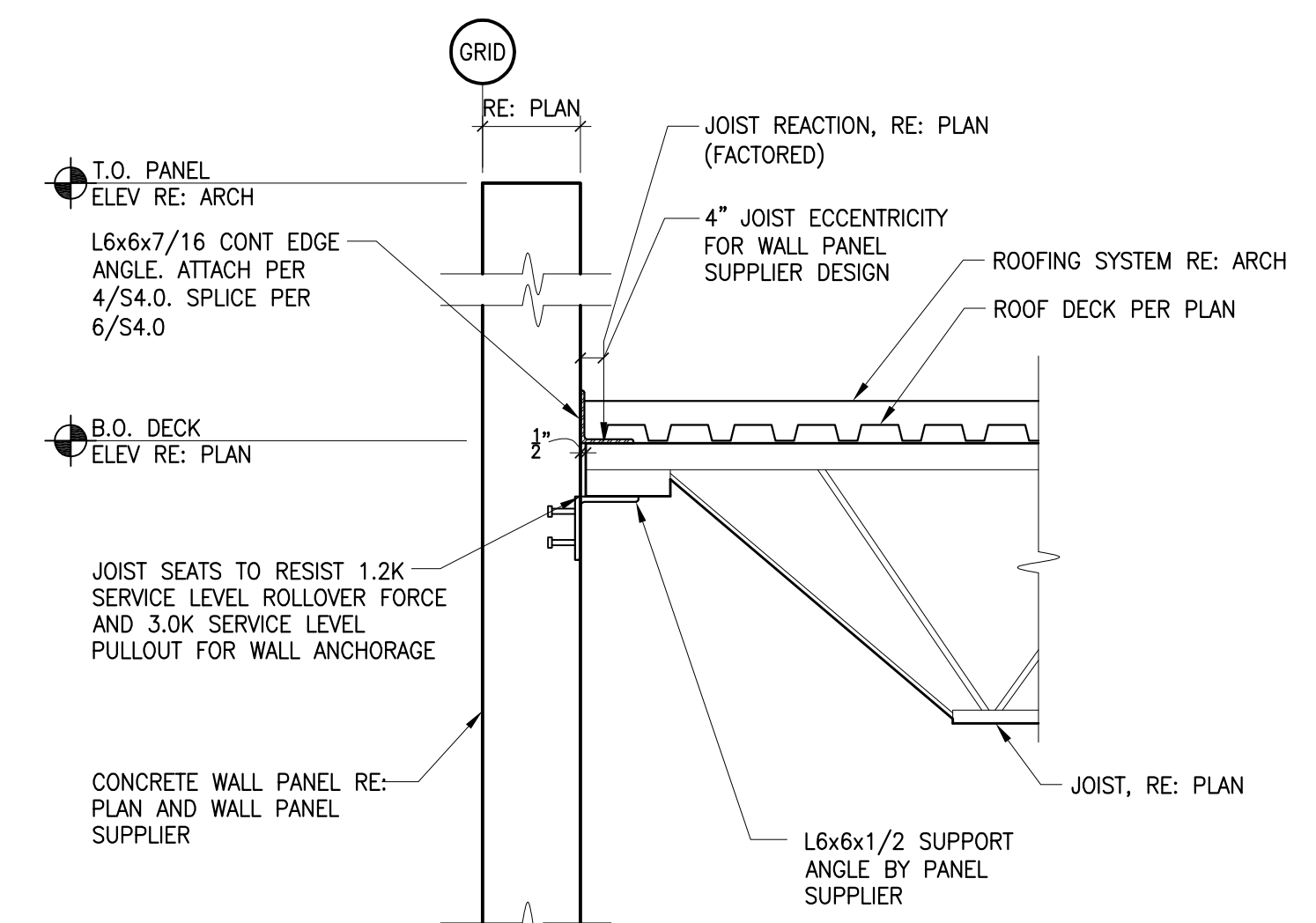
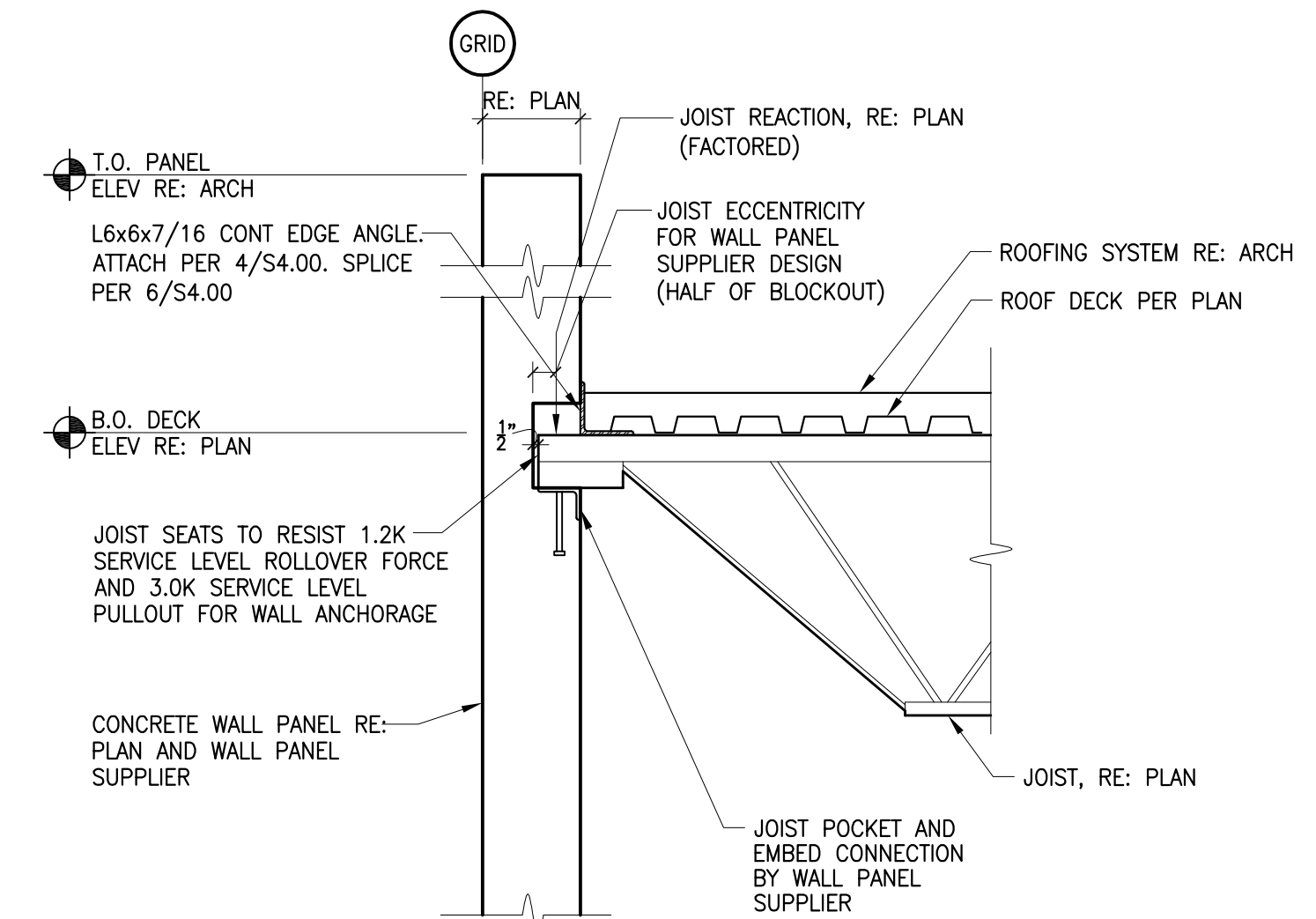
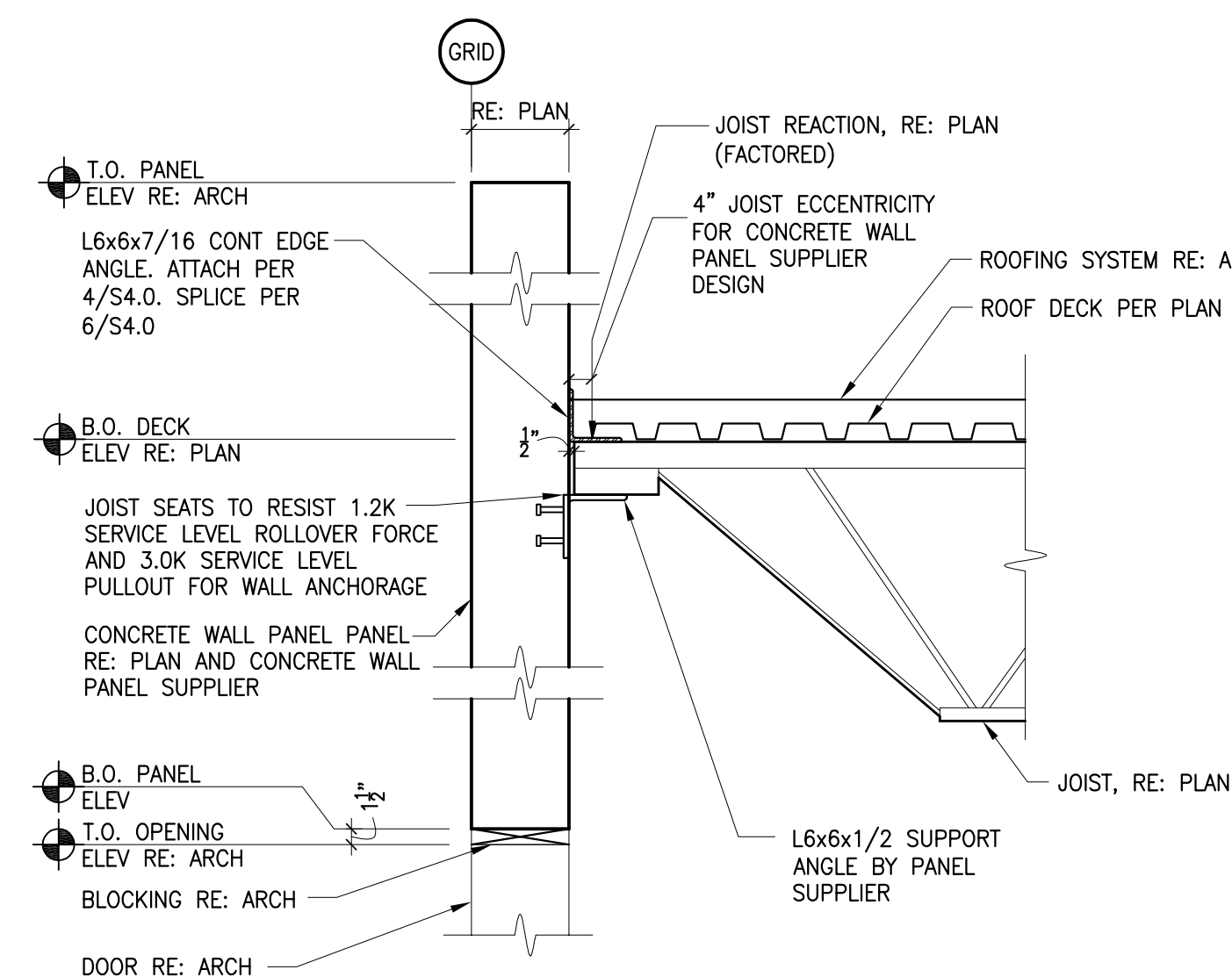
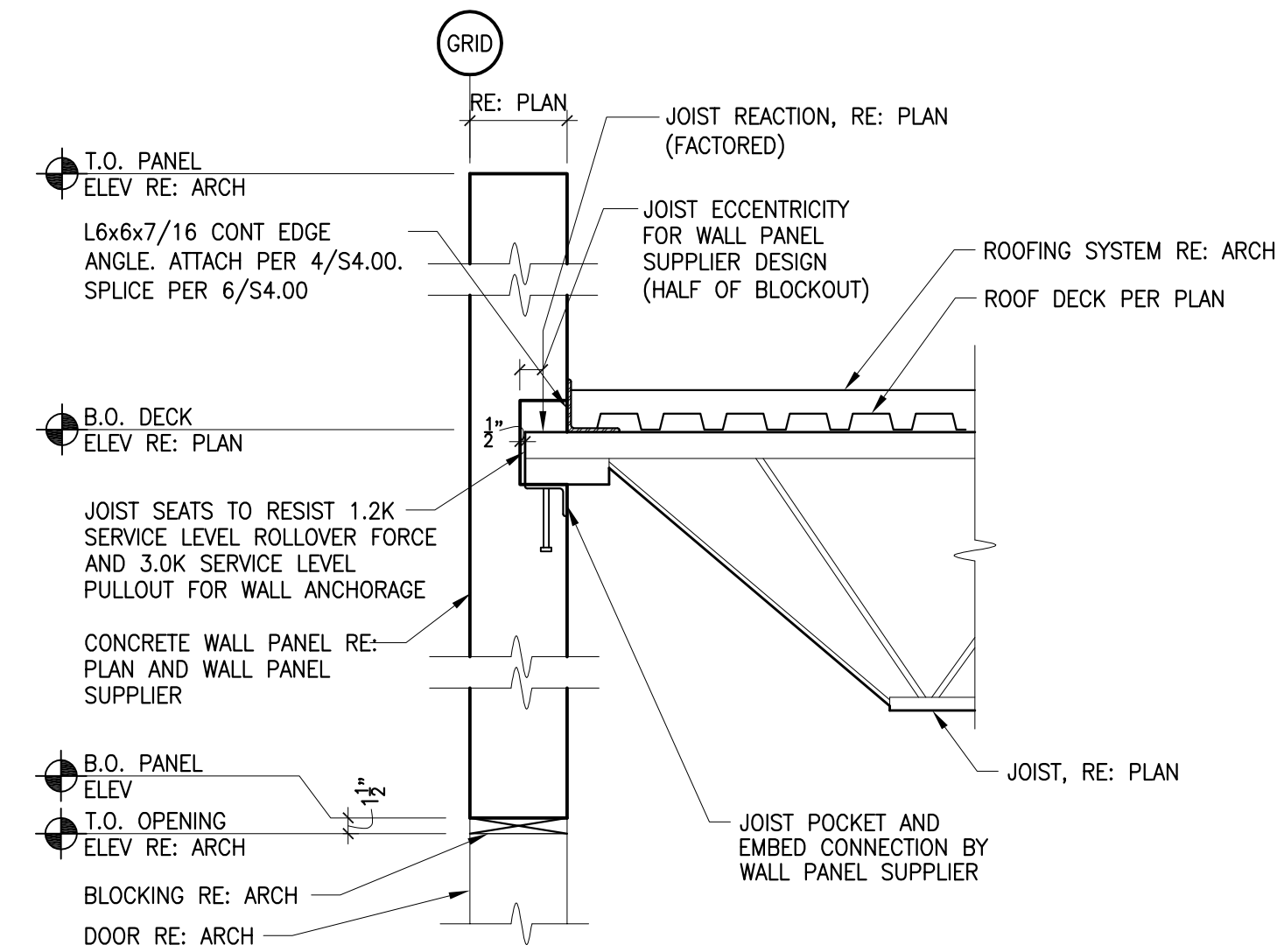
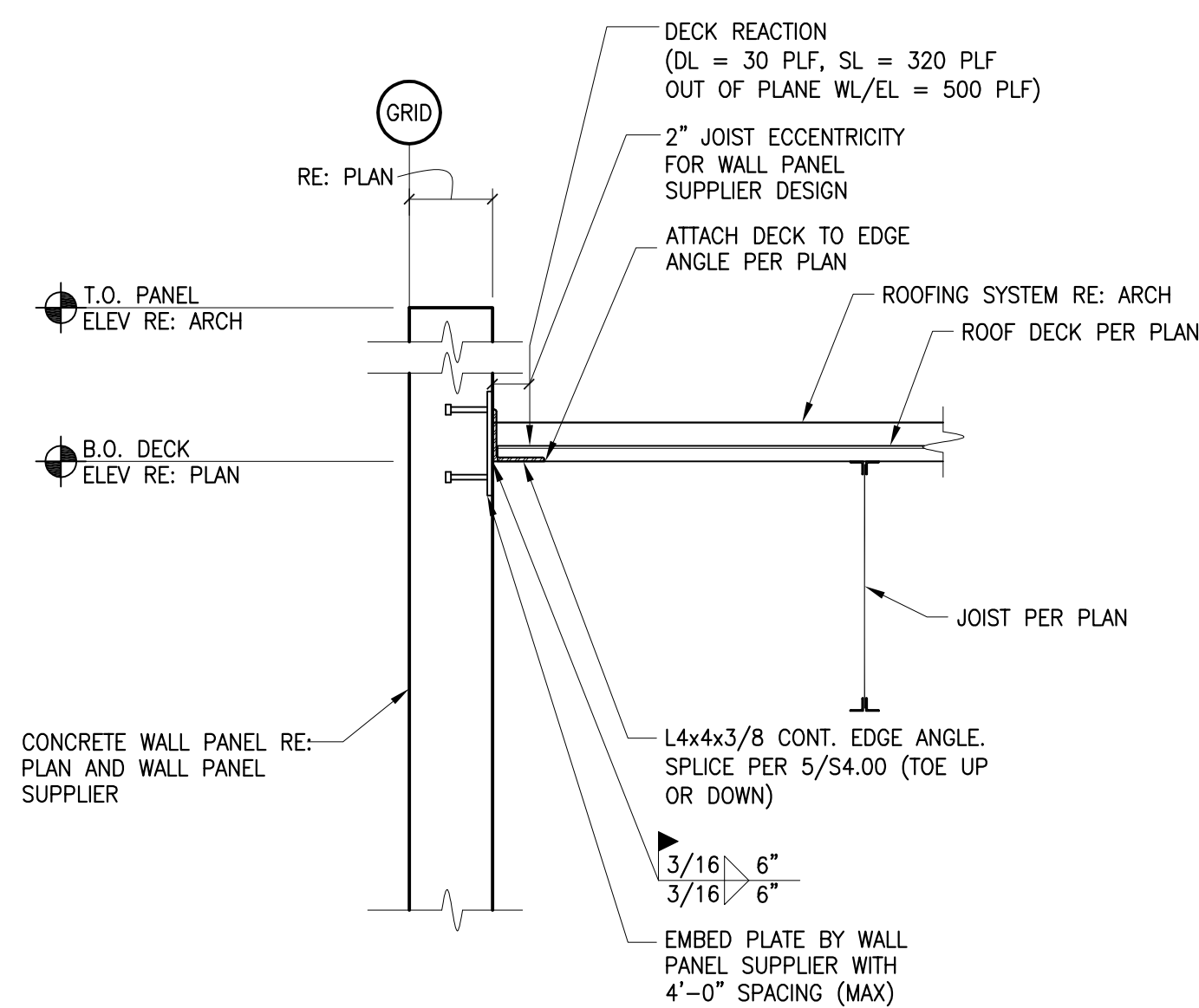
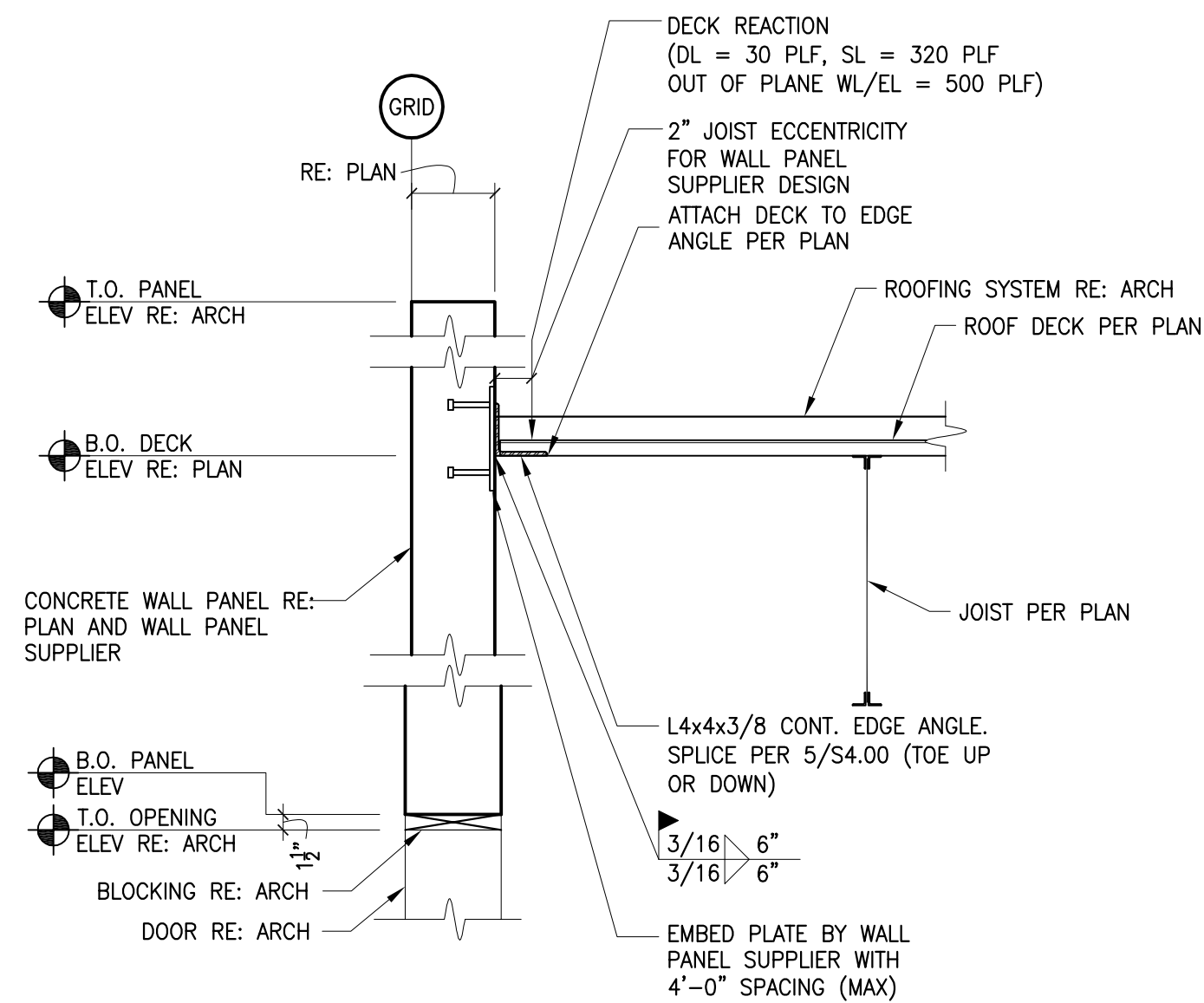
2 BOTTOM FLANGE BRACING DETAIL
3/4" = 1'-0"



1 BEAM CONNECTION DETAIL
3/4" = 1'-0"

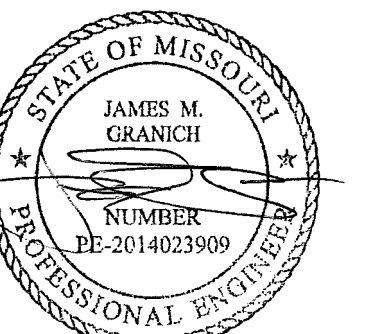
NOTES:

- ALL CONNECTIONS ON THE STRUCTURAL DRAWINGS, UNLESS NOTED OTHERWISE, SHALL BE DESIGNED AND DETAILED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED, EMPLOYED OR RETAINED BY THE STEEL FABRICATOR. THE DESIGN AND DETAILED SHALL COMPLY WITH ALL APPLICABLE CODES AND SPECIFICATION SECTIONS.
- CONNECTIONS SHOWN ARE FOR REFERENCE ONLY. FABRICATOR MAY USE OTHER AISC APPROVED CONNECTIONS.
- ALL BOLTS SHALL BE 3/4" DIAMETER A325 W/ HEAVY HEX NUTS, UNLESS NOTED OTHERWISE.
- ALL CONNECTIONS SHALL BE BEARING TYPE CONNECTIONS AND SHALL BE SNUG TIGHTENED UNLESS NOTED OTHERWISE.
- FOR BEAMS WITH AXIAL REACTIONS PER PLAN, CONNECTIONS SHALL BE DESIGNED AS FULLY TENSIONED SLIP CRITICAL PER AISC SPECIFICATIONS.



5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317.288.0681
F :: 317.288.0753

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LEE'S SUMMIT LOGISTICS
BUILDING B LOT 2

[illegible]

S4.1



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5719 LAWTON LOOP E. DR. #212
INDIANAPOLIS, IN 46216
O :: 317 . 288 . 0681
F :: 317 . 288 . 0753



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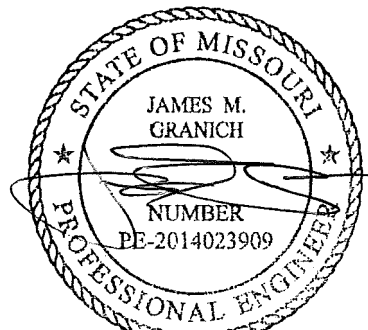
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Structural Consultants, Inc.

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PROJECT INFORMATION

LEE'S SUMMIT LOGISTICS
BUILDING B LOT 2

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LEE'S SUMMIT, MO

ISSUE DATES

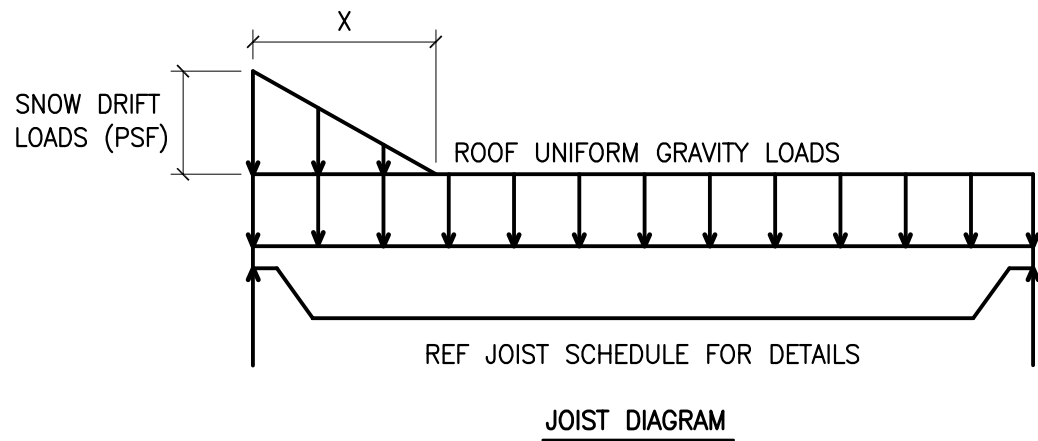
ISSUE	DATE
ISSUE FOR PERMIT	04.22.2022

210300

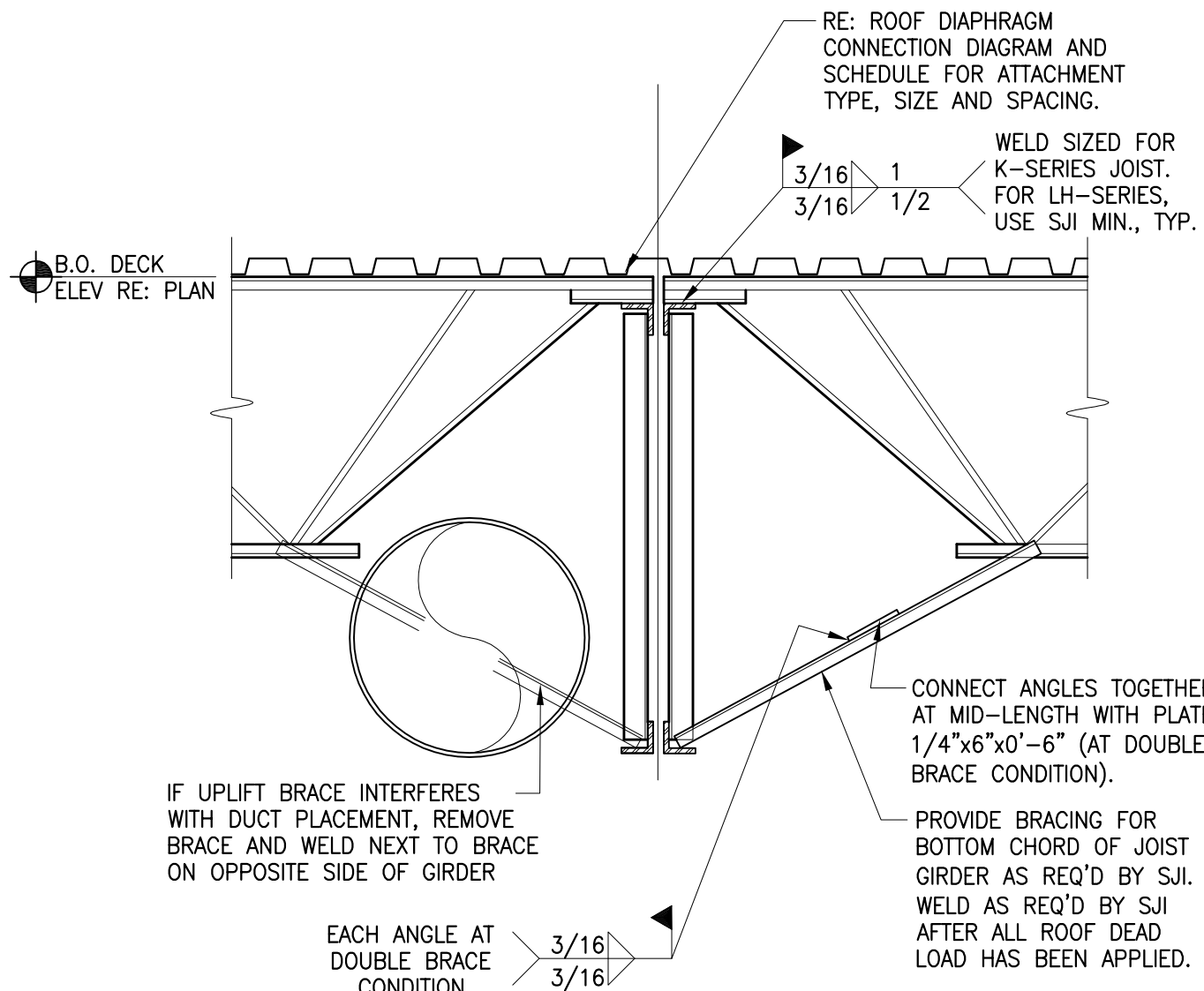
S4.2

FRAMING DETAILS

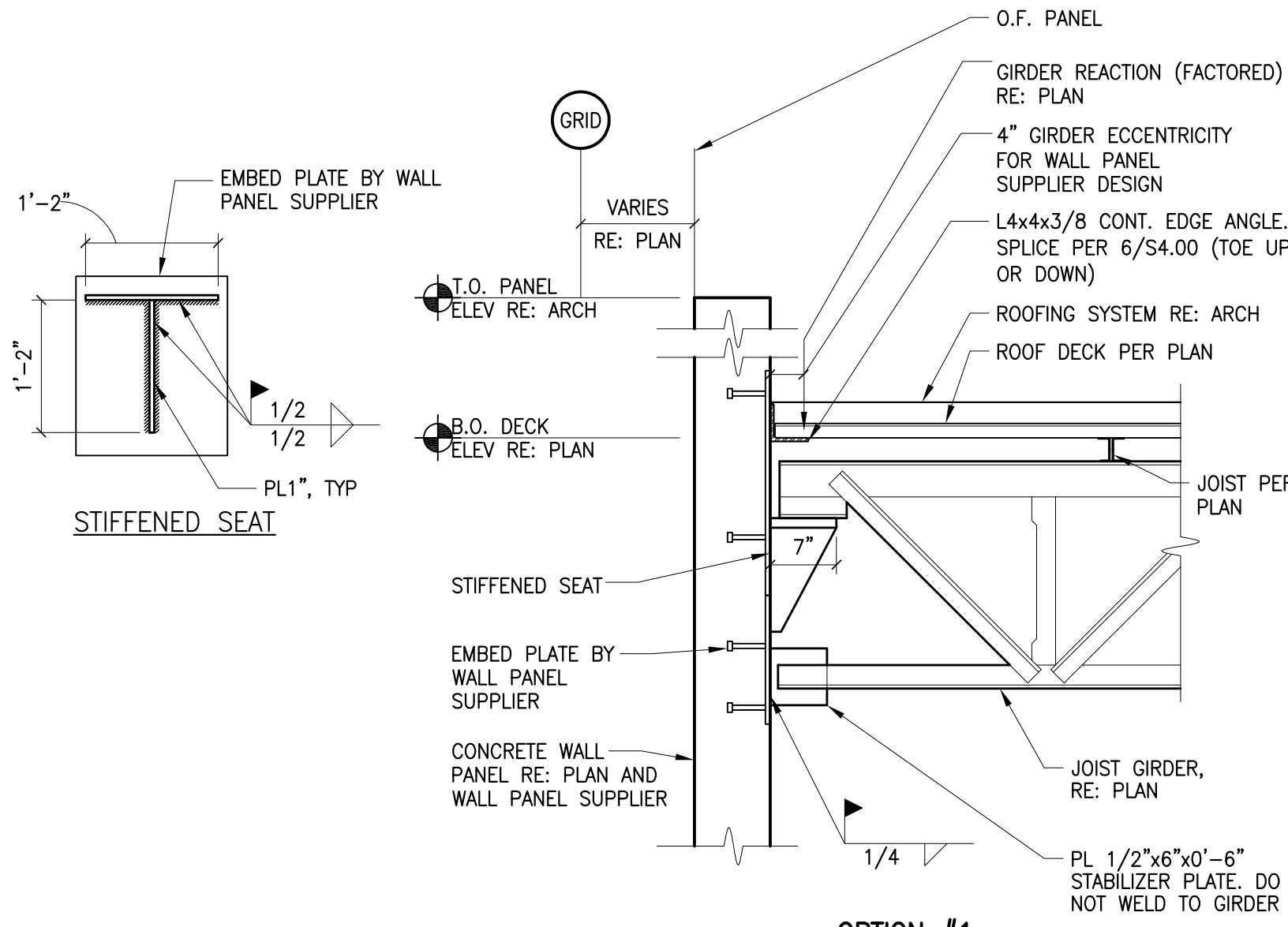
SPECIAL JOIST LOADS		
MARK	SNOW DRIFT (PSF)	SNOW WIDTH (X)
SP1	56.0	13'-6"
SP2	54.0	14'-10"



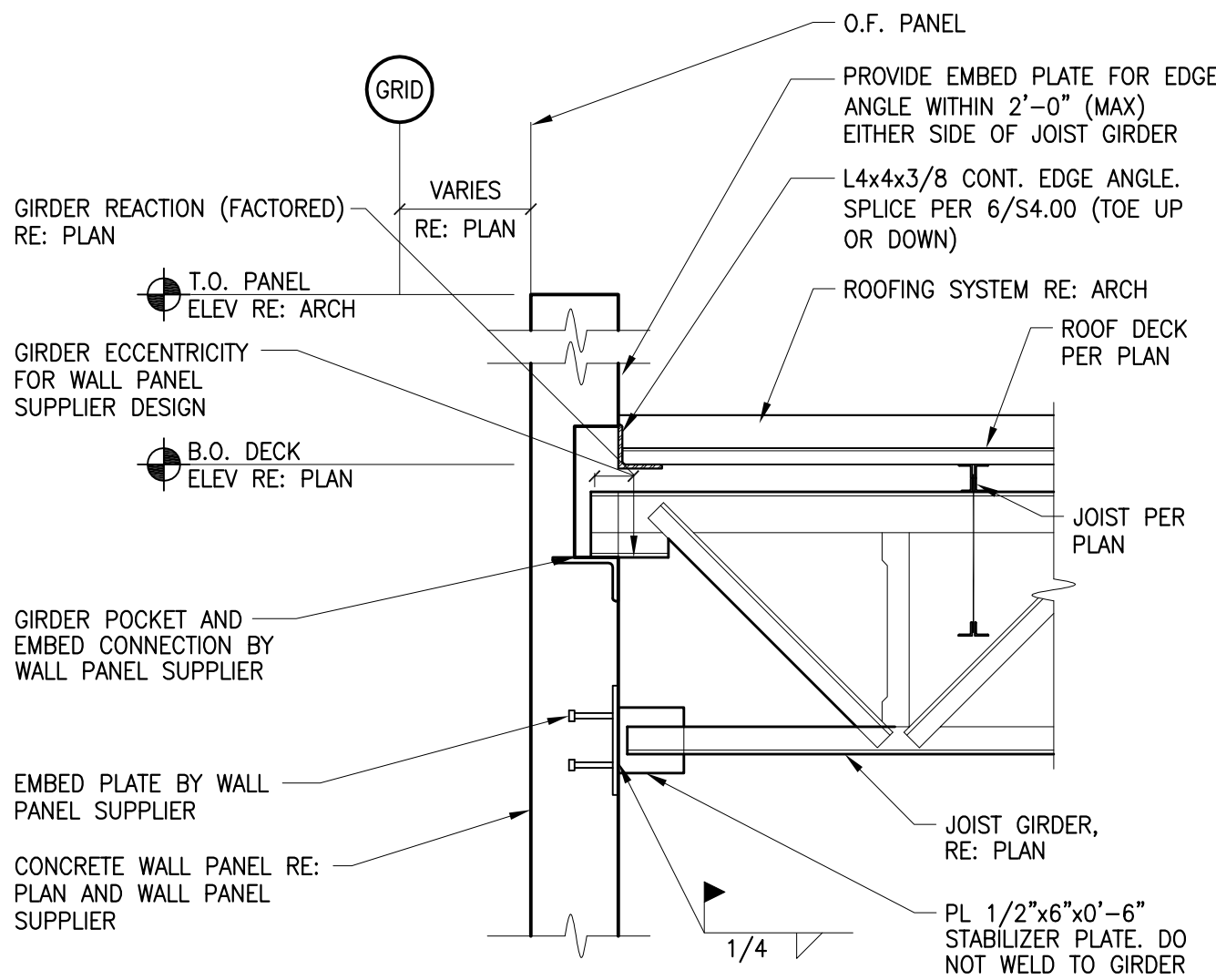
4 SPECIAL JOIST SCHEDULE
3/4" = 1'-0"



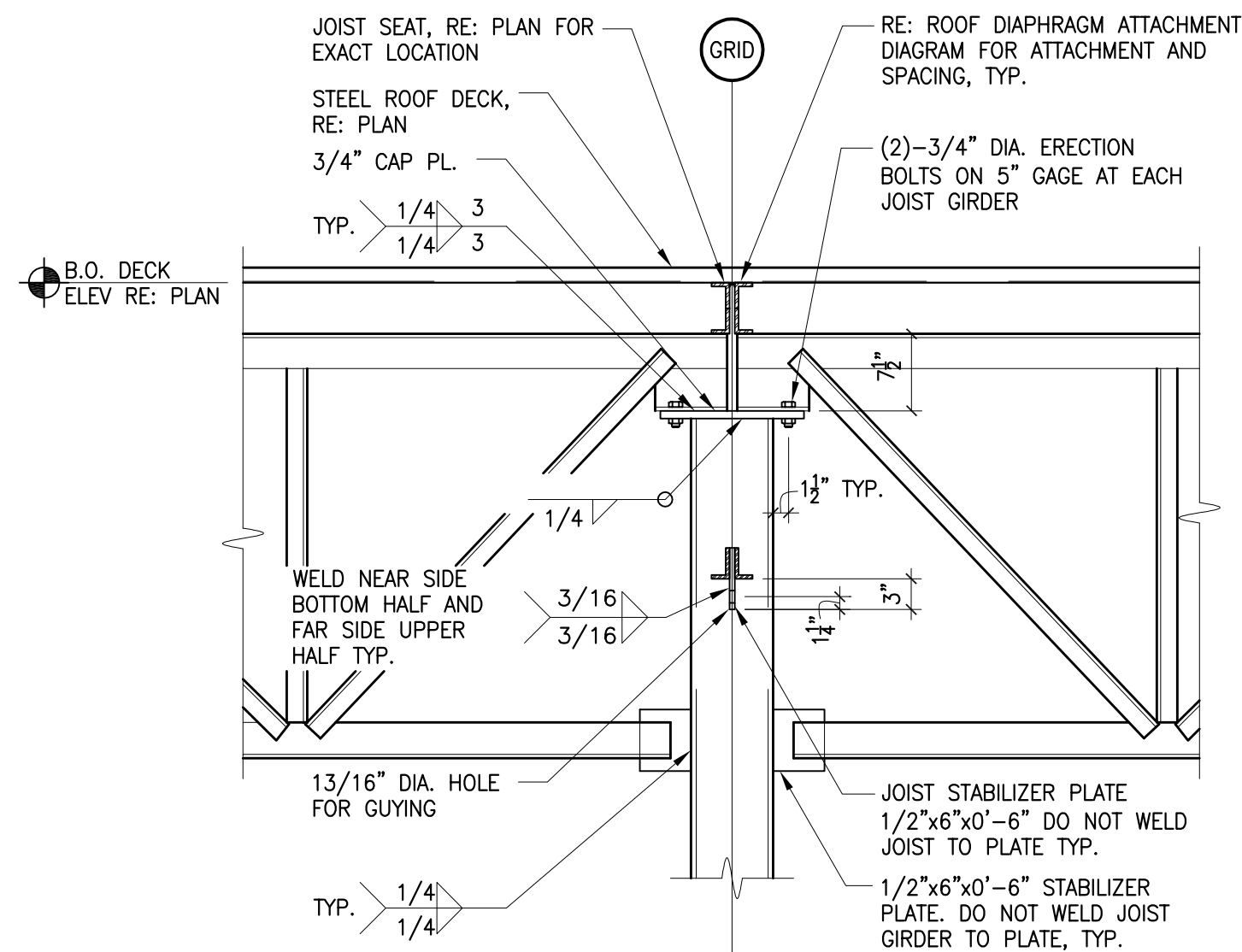
3 JOIST/JOIST GIRDER SECTION
3/4" = 1'-0"



OPTION #1



OPTION #2



1 JOIST GIRDER/COLUMN CONNECTION
3/4" = 1'-0"

2 GIRDER TO WALL PANEL FRAMING DETAIL
3/4" = 1'-0"